

The Town of East Gwillimbury

Stormwater Management Master Plan Municipal Class Environmental Assessment

W10-487

November 2012



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Executive Summary

Introduction

The Town of East Gwillimbury (the Town, “Owner”) has retained Cole Engineering Group Ltd. (Cole Engineering) to prepare a Stormwater Management Master Plan (SWMMP) in accordance to the Lake Simcoe Protection Plan 2009 by the Province of Ontario, through Phases 1 and 2 of a Master Plan Municipal Class Environmental Assessment (Class EA). The Town is located just south of Lake Simcoe and is bound by the Town of Georgina to the north, the Regional Municipality of Durham to the east, the Town of Newmarket and Town of Whitchurch-Stouffville to the south and the Township of King and County of Simcoe to the west. The study area is approximately 25,000 hectares (62,000 acres) and is mainly rural with the exception of the community villages of Holland Landing, Queensville, Sharon, Green Lane West and Mount Albert. The Town drains to four (4) subwatersheds: Black River subwatershed, Maskinonge River subwatershed, East Holland River subwatershed, and West Holland River subwatershed. Parts of the Greenbelt and the Oak Ridges Moraine are within the Town boundary but are mostly outside of the main settlement areas. There are existing Stormwater Management (SWM) works in place that consist of dry and wet ponds. The intent of the SWMMP is to develop the practical and implementable framework which balances the requirements of proposed and existing development with infrastructure requirements, economic, social and environmental constraints and opportunities.

SWMMP Plan Strategy

The objectives of the SWMMP is to investigate opportunities to prevent flooding, mitigate changes in water balance and reduce phosphorus loadings to Lake Simcoe, as well assess erosion issues and maintain / improve water quality. The report examines the existing environmental characteristics of the Town, land use changes, peak flow impacts, phosphorus loadings, water budget, and erosion analysis. Cole Engineering has also evaluated the potential impacts of climate change by developing a sensitivity analysis. Based on the results of each analysis, recommendations and the preferred alternative were developed for each settlement area. In addition, recommended SWM guidelines were established for future development within the Town.

An assessment of stormwater peak flows was conducted from existing and future conditions to evaluate the environmental impact of the future development in the Town. Subcatchment areas within the Town settlement areas were modelled under the 2 to 100-year storm events. Results indicate a general increase in peak flow as a result of the development proposed from the Official Plan. The current “post-to-pre” approach in terms of stormwater quantity control was applied to post-development drainage areas subject to increases in imperviousness from the future conditions model. As such, several drainage areas are required to be overcontrolled to unit flow control criteria in the event the “post-to-pre” approach was not sufficient. This scenario was mostly evident in the East and West Holland River subwatersheds where development has been intensified. It is recommended however, that the “post-to-pre” approach be continued to be imposed for new development in the Black River and Maskinonge River where most peak flows are reduced. It is recommended that a detailed unit flow study is to be investigated (specifically within the Holland River) and implemented in the future.

A comparison of phosphorus loadings is required to assess the environmental impact of development in terms of phosphorus on Lake Simcoe. All settlement areas are expected to see an increase in phosphorus loads as development increases. An integrated treatment train approach is recommended to mitigate the accumulation of phosphorus from proposed development (i.e. bioretention, infiltration trenches, permeable pavement, and perforated pipe systems). It is noted that specific mitigation measures to be used to reduce phosphorus accumulation are to be confirmed on a site specific basis at the functional design stages of development.

A water balance assessment was developed as a part of the study to establish the infiltration deficit as a result of future development. Soil type along with land use was utilized to calculate water balance for the pre and post development conditions. On an individual settlement basis an infiltration rate was formulated based on post development infiltration rates. Results of the analysis indicate that due to soil restrictions and land constraints, the use of Best Management Practices were recommended where applicable on a site specific basis (i.e. soakaway pits, bioretention, infiltration trenches and chambers, permeable pavement, and grassed swales).

Erosion analysis was based on the current creek survey data developed by the LSRCA in 2008. Most concern was due to stream bank erosion, sediment build up in the watercourses, and unrestricted livestock access. Traditional BMP measures, bank stabilization and riparian vegetation are recommended to decrease sedimentation and reduce flows. A detailed erosion analysis of creeks within the Town did not take place as a part of this study and the Town is recommended to investigate the opportunity for individual erosion assessments where future development will take place.

A sensitivity analysis was conducted to determine the potential impacts of climate change. Although the definition of climate change is still undefined, Cole Engineering has developed hydrologic models to determine the impacts on existing stormwater management facilities (SWMFs) if the effects of change would increase / decrease rainfall intensity, increase / decrease storm duration and under the application of wet and dry soil conditions. The results indicate that the changes in the conditions of storm events and hydrologic parameters on the existing SWMFs in the Town are generally sensitive in terms of peak outflows and storage volumes. A number of assumptions were made in regards to the effects of climate change on SWM and it is recommended that further study be developed to accurately assess climate change impacts.

As part of this project a software tool called SWMSOFT has been utilized to develop an inventory of all the existing SWMFs. With the use of this program, a maintenance schedule has been established to allow for all facilities to meet current Enhanced standards. The Town will be able to use this software to determine when facilities will need to be retrofitted and an approximate cost of the work needed. There are currently four (4) SWM ponds that are required to be tended to immediately in order to meet standards.

Public Information Centre / Stakeholder Consultation

A notice of commencement and notice of Public Information Centre (PIC) were completed as part of this study and were posted on the "Our Town News" section of the Town's website and published in the Town's newspaper, the Era Banner. In addition, the notices were mailed directly to affected stakeholders.

One (1) Public Information Centre (PIC) took place during the project. The PIC was held on March 3, 2011 to present the alternative strategies identified. It was attended by six (6) people mostly consisting of Town Council and Staff. The Project Team including representatives from Cole Engineering and the Town were in attendance to answer any questions that participants had.

The PIC presented the following elements:

- Study Overview & Background;
- Problem / Opportunity Statement;
- Overview of the Municipal Class Environmental Assessment Process;
- Alternative Solutions Being Considered; and,
- Next Steps.

No comments were received from public consultation and a notice of completion of this project is targeted at the second quarter of 2011.

SWM Recommendations

Future SWMFs will be designed to Ministry of the Environment (MOE) standards:

- Stormwater Quality Control: Enhanced – 80% TSS removal, 80% TP removal;
- Stormwater Quantity Control: Post-development peak flows to be reduced to pre-development levels for all storm events up to and including the 100-year storm. Unit-flow rates are to be applied for specific drainage areas requiring over-control to meet pre-development peak flows; **
- The Regional Storm should be controlled to the pre-development peak flow rate level *if* noted in the subwatershed study (LSRCA Technical Guidelines for Stormwater Management Submissions, November 2010);
- Site specific erosion control and detention time calculations based on individual fluvial assessment of the receiving stream (i.e. 24hr, 48hr, 72hr, etc.);
- Future developments will follow the guidelines as outlined in the Lake Simcoe Protection Plan (LSPP);
- BMPs will be implemented where applicable to establish a treatment train approach (i.e. source, conveyance and end-of-pipe treatment);
- Where opportunities exist, implement enhanced vegetation for shading, bottom draw outlets, cooling trenches at SWMF locations to reduce stormwater temperature;
- Where opportunities exist, reduce phosphorus discharge, improve infiltration and reduce erosion potential within settlement areas and expansion of settlement area boundaries;
- Where opportunities exist, implement rural retrofits for applicable areas outside of existing settlement boundaries; and,
- Proceed with retrofit opportunities and maintenance operations of select SWM facilities as identified by the LSRCA and the SWM Master Plan.

** It is recommended that an investigation regarding the implementation of unit flow criteria should be conducted in the future for the East and West Holland River subwatersheds to establish a reduction of peak flows.

Preferred Alternative

The preferred alternative in each settlement area is the Traditional SWM with BMP Implementation Strategy. Based on the future land use, the preferred alternative is to be combined with Traditional SWM with Urban or Rural Retrofits Strategy. The proposed BMP implementation and Urban / Rural Retrofits will be determined on a site specific basis at the functional design stages of development.

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1.0 Introduction

1.1. Background

In November 2010, the Town of East Gwillimbury (the Town, “Owner”) initiated a “Master Plan” Municipal Class Environmental Assessment (Class EA) to develop plans and strategies to govern the Stormwater Management (SWM) and practices within the Town limits. The intent of the Stormwater Management Master Plan (SWMMP) is to prepare a practical and implementable framework which balances the requirements of proposed and existing development with infrastructure requirements, economic, social and environmental constraints and opportunities. The Lake Simcoe Protection Plan was introduced by the Province of Ontario in June of 2009, and it called for all settlement areas to prepare and implement comprehensive SWMMP that would improve the management of stormwater for both existing and planned development. This report is prepared in the spirit of the Comprehensive Stormwater Management Master Plan Guidelines as well as with the Class EA which is described further in **Section 2.0**.

1.2. Location

The Town is located just south of Lake Simcoe and is bounded by the Town of Georgina to the north, the Regional Municipality of Durham to the east, the Town of Newmarket and the Town of Whitchurch-Stouffville to the south and the Township of King and County of Simcoe to the west. The study area is approximately 25,000 hectares (62,000 acres) and is mainly rural with the exception of the community villages of Holland Landing, Queensville, Sharon, Green Lane West and Mount Albert. The study area is illustrated below in **Figure 1-1**.

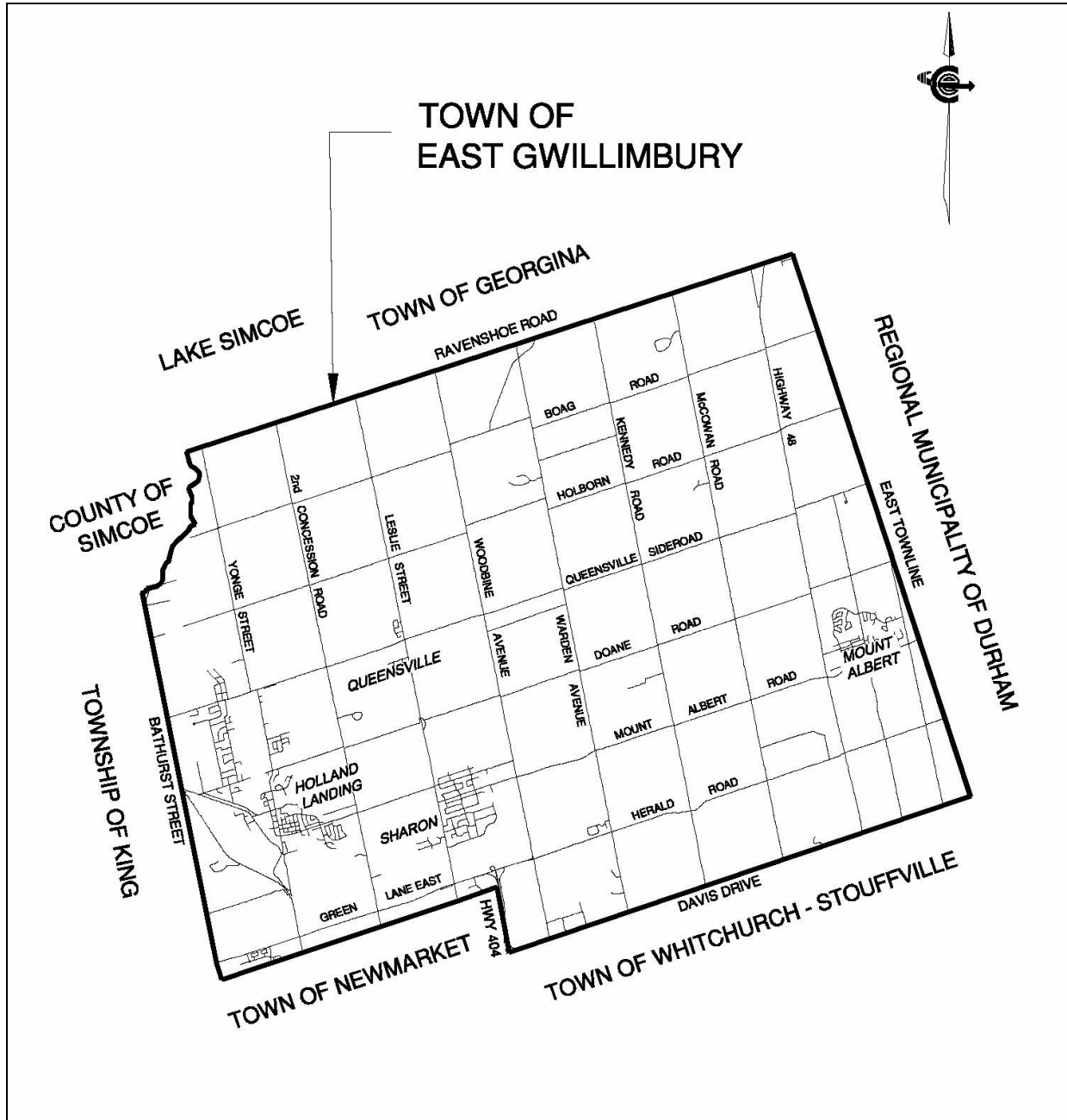


Figure 1-1 – Study Area

1.3. Study Overview

In 2010, the Town developed a Consolidated Official Plan (OP) Document outlining the proposed developments and standards to the year 2031. The OP's purpose is to provide direction and a policy framework for managing growth and land use decisions over the proposed planning period. It is expected that the Town will grow in population from 23,000 to 86,400 residents and the planned policy is to ensure appropriate growth towards a sustainable community. The Town's Thinking Green! Sustainability Strategy and Development Standards provides a foundation and guiding objectives for the policy and framework of the official plan. The five (5) pillars of the Strategic Plan adopted by the Town in 2005 outline the Council's vision for the future.

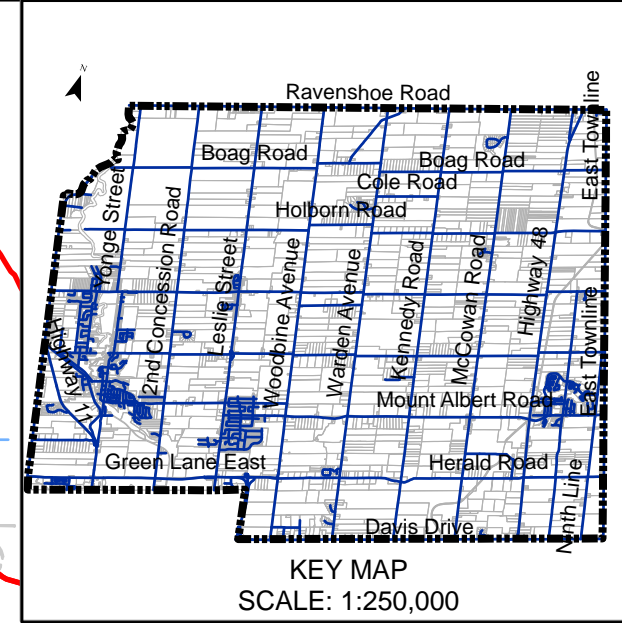
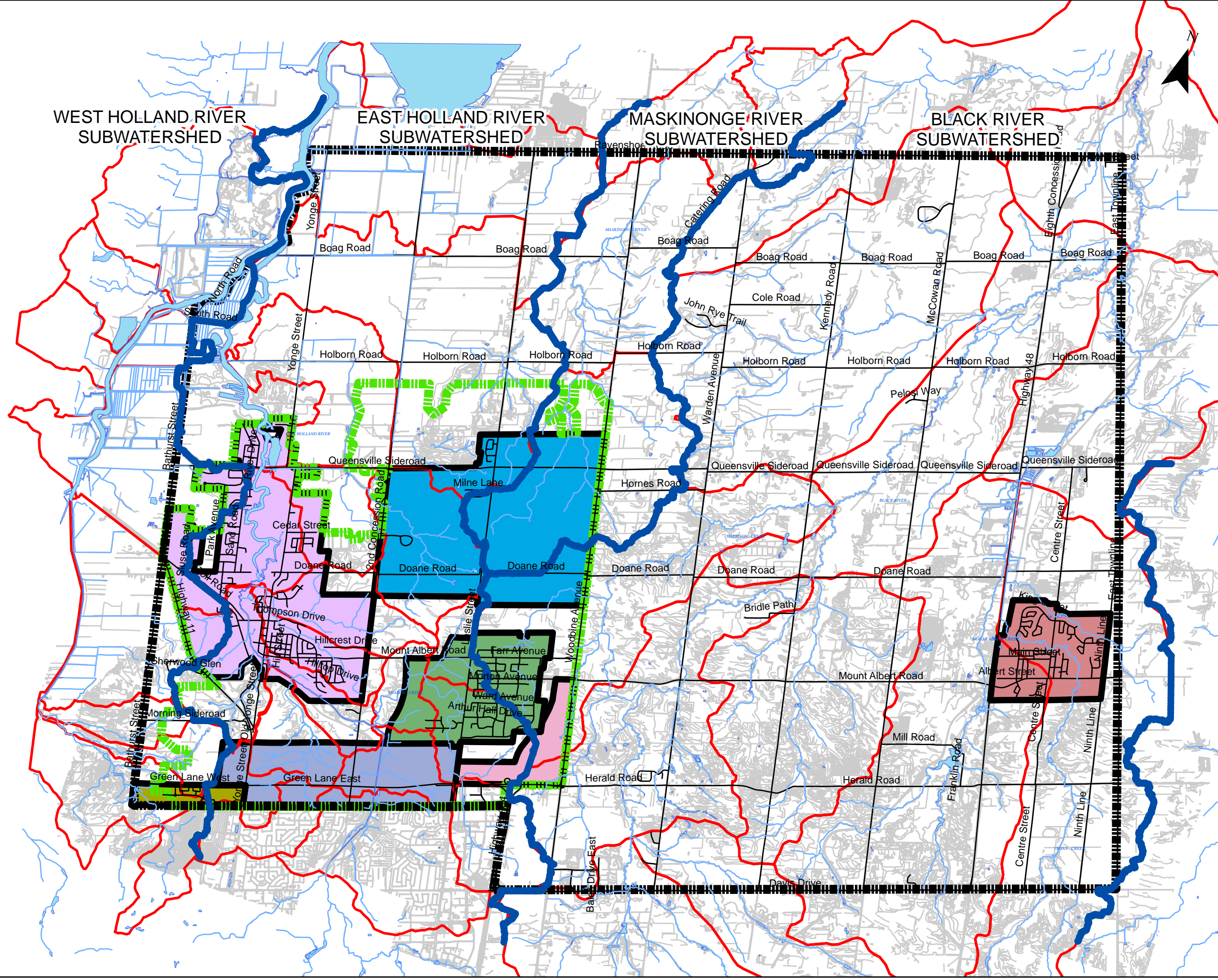
The five (5) pillars are as follows:

1. Protecting and Enhancing the Natural Environment;
2. Providing and Advocating for Quality Programs and Services to the Community;
3. Investing in Municipal Infrastructure;
4. Managing Growth to Ensure a Sustainable Community; and,
5. Supporting a Municipal Organization Focused on Excellence.
6. The Town's consolidated OP will provide a framework for the development of the SWMMP and outline existing constraints and opportunities for the implementation of SWM for existing and planned developments.

The Town is part of four (4) major subwatersheds within the Lake Simcoe Watershed:

1. The West Holland;
2. East Holland;
3. Maskinonge River; and,
4. Black River sub-watersheds.

The main drainage sheds are the Black River and East Holland River and the associated tributaries which ultimately drain to Lake Simcoe. The subwatershed boundaries and existing subcatchment areas are illustrated below in **Figure 1-2**.



LEGEND

- WATERSHED BOUNDARY
- TOWN BOUNDARY
- CENTRAL GROWTH AREA
- SUBCATCHMENT AREAS

COMMUNITY AREA

- GREEN LANE EAST COMMUNITY PLAN
- GREEN LANE WEST COMMUNITY PLAN
- HOLLAND LANDING - RIVER DRIVE PARK COMMUNITY PLAN
- MOUNT ALBERT COMMUNITY PLAN AREA
- QUEENSVILLE COMMUNITY PLAN
- SHARON COMMUNITY PLAN AREA

NOTES:

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Town of East Gwillimbury
Our town. Our future.

SCALE: 1:70,000

0 625 1,250 2,500 3,750 5,000 Meters

STORMWATER MANAGEMENT MASTER PLAN
DRAINAGE AREAS AND SUBWATSHED BOUNDARIES

1.4. Previous Studies

A number of previous studies have been developed within the study area.

These studies include:

- Queensville Community Master Plan Volume II – Master Drainage Plan - Town of East Gwillimbury, revised June 2000;
- Sharon Community OPA – 55 SWM Study - Town of East Gwillimbury, revised June 2005;
- Sharon Community Plan Master Environmental and Servicing Plan – MMM Group, February 2010;
- Growth Plan for the Greater Golden Horseshoe – Ministry of Public Infrastructure Renewal, 2006;
- Report on the Phosphorus Loads to Lake Simcoe – Lake Simcoe Conservation Authority, 2004-2007;
- Lake Simcoe Basin SWM and Retrofit Opportunities – Lake Simcoe Conservation Authority, 2007;
- Hydrology Report (Final) – Hydrologic and Hydraulic Modeling for the West Holland River, East Holland River and Maskinonge River Watersheds, CCL, June 30, 2005;
- Hydraulic Report (Final) – Hydrologic and Hydraulic Modeling for the West Holland River, East Holland River and Maskinonge River Watersheds, CCL, July 11, 2005;
- Black River Hydraulic Model Update – Completed as part of the Regulation Limit Mapping, LSRCA, 2005;
- West Holland River Subwatershed Plan – Lake Simcoe Region Conservation Authority, 2010;
- East Holland River Subwatershed Plan – Lake Simcoe Region Conservation Authority, 2010;
- Maskinonge River Subwatershed Plan – Lake Simcoe Region Conservation Authority, 2010; and,
- Black River Subwatershed Plan – Lake Simcoe Region Conservation Authority, 2010.

Key aspects of existing master plans within the Town are summarized below.

1.4.1. Queensville Community Master Plan, June 2000

Recommended Master SWM Plan Practices:

- General surface storage, vegetative, soft measures, special purpose and conservation / restoration classes to address stormwater quality concerns;
- Other management concerns pertaining to erosion control, water quantity control, and groundwater recharge impacts;
- Conveyance – dual drainage principle (minor and major stormwater drainage systems), 5 year flow;
- Lot Level Controls – Drainage swales, Dry swales (Soakaway Pits), Reduced Lot Grading to Maximize depression storage for groundwater recharge; and,

- End-of-pipe – Off-line SWMFs (provide water quality, erosion, quantity control), on-line SWMFs, on-site detention (industrial blocks), OGS, phosphorus control, Enhanced control.

Design Guidelines:

- Water Quality – Based on aquatic life, Enhanced (80% TSS Removal);
- Water Quantity – No increases to peak flow above pre-development level, where possible 10-15% over control;
- Erosion Control – storage of first flush (25mm rainfall volume 2-hour duration) for 24 hours; and,
- Infiltration encouraged.

It should be noted that the Queensville Community Master Plan (June 2000) was completed before the implementation of the Lake Simcoe Protection Plan (2009).

1.4.2. Sharon Community Plan – Master Environmental Servicing Plan (MESP), February 2010

Design Guidelines:

- Storm drainage conveyance system will be dual drainage (major and minor);
- Pipes will be sized to accommodate flows from a 1 in 5 year event at or below 90% of pipe capacity;
- Velocities 0.6 m/s, remaining under velocity of 3.0 m/s;
- Overland flow routes convey storm flows in excess of the 1 in 5 year event;
- Enhanced Water Quality;
- The SWM Strategy focuses on end-of-pipe solutions;
- 10 SWMF proposed west side of Leslie (all off-line); and,
- One (1) retrofit is proposed for water quality (East Holland River Subwatershed Study).

1.5. Data Gaps

The data collected for this project is from a combination of sources including the Town, LSRCA and a variety of other sources. Previous studies, models, files and other data required for this study is based on the information given. It should be noted that within those studies there are some gaps in available information as well as reports that have yet to be completed or reports that are unavailable to the public. Examples of data gaps are:

- Existing storm sewer information;
- Drainage area information outside of settlement areas;
- Uncontrolled outlets; and,
- Existing SWM Pond IDs and reports.

Cole Engineering has done its best to gather all the information available to bring this report together. Further and more detailed studies may need to be completed in order to fill in the missing information.

1.6. Problem and Opportunity Statements

The last four (4) decades of research, monitoring, and scientific studies show how human-related activities, including urban and rural uses, recreation and agriculture, have impaired the health of the Lake Simcoe watershed ecosystem through direct and indirect changes (LSPP, 2009). Further to the objectives identified in the Lake Simcoe Protection Plan, SWM policies call for comprehensive master plans to improve the management of stormwater for both existing and planned development.

Since the implementation of the Lake Simcoe Protection Plan, the Town has retained a consultant (Cole Engineering) to develop a SWMMP to define all anticipated works necessary to maintain, expand and improve the existing storm drainage system (including SWM ponds) while protecting the valued natural resources both within and beyond Town limits. The SWMMP will be prepared in accordance with the Class EA and be available for public review.

This project presents an opportunity to improve the management of stormwater for both existing and planned development which is based on changes in land use as outlined in the Consolidated OP 2031 for the Town (June 2010). An opportunity exists to implement a drainage strategy within the Town to meet the requirements as set out in the Lake Simcoe Protection Plan. While implementing drainage improvements, there will be opportunities to minimize ongoing erosion and sedimentation, phosphorus loadings and changes in water balance which may cause a negative impact on the Lake Simcoe watershed.

1.7. Objectives of the Project

The primary objective of the project is to meet the requirements of the Lake Simcoe Protection Plan (specifically *Policy 4.5 SA*) while considering the intentions of the Town's OP.

The objectives of each plan in detail are described below:

Objectives of the Lake Simcoe Protection Plan

The objectives of the Plan as set out in the Lake Simcoe Protection Act, 2008 are to:

- Protect, improve or restore the elements that contribute to the ecological health of the Lake Simcoe watershed, including, water quality, hydrology, key natural heritage features and their functions, and key hydrologic features and their functions;
- Restore a self-sustaining coldwater fish community in Lake Simcoe;
- Reduce loadings of phosphorus and other nutrients of concern to Lake Simcoe and its tributaries;
- Reduce the discharge of pollutants to Lake Simcoe and its tributaries;
- Respond to adverse effects related to invasive species and, where possible, to prevent invasive species from entering the Lake Simcoe watershed;
- Improve the Lake Simcoe watershed's capacity to adapt to climate change;
- Provide for ongoing scientific research and monitoring related to the ecological health of the Lake Simcoe watershed;
- Improve conditions for environmentally sustainable recreation activities related to Lake Simcoe and to promote those activities;

- Promote environmentally sustainable land and water uses, activities and development practices;
- Build on the protections for the Lake Simcoe watershed that are provided by provincial plans that apply in all or part of the Lake Simcoe watershed, including the Oak Ridges Moraine Conservation Plan and the Greenbelt Plan, and provincial legislation, including the Clean Water Act, 2006, the Conservation Authorities Act, the Ontario Water Resources Act, and the Planning Act; and,
- Pursue any other objectives set out in the Lake Simcoe Protection Plan.

Objectives of Policy 4.5-SA of the Lake Simcoe Protection Plan

Within five (5) years of the date the Plan comes into effect, municipalities, in collaboration with LSRCA, will prepare and implement comprehensive SWMMP's for each settlement area in the Lake Simcoe watershed. The SWMMP's will be prepared in accordance with the Class EA guidelines and will include:

- A characterization of existing environmental conditions on a subwatershed basis, consistent with any relevant subwatershed evaluations, if available;
- An evaluation of the cumulative environmental impact of stormwater from existing and planned *development*;
- A determination of the effectiveness of existing SWM works at reducing the negative impacts of stormwater on the environment, including consideration of the potential impacts of climate change on the effectiveness of the works;
- An examination of any stormwater retrofit opportunities that have already been identified by the municipality or the LSRCA for areas where stormwater is uncontrolled or inadequately controlled;
- The identification of additional SWM retrofit opportunities or improvements to existing SWM works that could improve the level of treatment within a particular *settlement area*;
- A description of existing or planned programs for regular maintenance of SWM works;
- An identification of the recommended approaches for SWM in each *settlement area*; and,
- An implementation plan for the recommended approaches.

Objectives of the Town's Official Plan

The policies of the Official Plan (OP) as it pertains to SWM are to promote and implement effective SWM that will help to reduce erosion; avoid downstream flash flooding; reduce nutrient, siltation and sediment loading; sustain fish habitat; and help to improve the quality of Lake Simcoe and its tributaries. Primary SWM objectives include:

- Ensuring that SWM systems are designed and constructed in accordance with sound environmental and engineering best practices;
- Pursuing and implementing sound SWM practices that will ensure adequate protection from flooding and erosion, maintain and/or improve water quality and enhance the environmental, aesthetic and recreational potential of watercourses; and,
- Reducing phosphorus loading to Lake Simcoe and its tributaries.

From review of the objectives as set out in each plan, the Town's SWMMP objectives are to:

- Investigate opportunities to prevent flooding within the Town;
- Identify existing erosion concerns and recommend erosion control measures;
- Investigate opportunities to maintain and/or improve water quality;
- Investigate opportunities to reduce phosphorus loadings to Lake Simcoe; and,
- Investigate opportunities to mitigate changes in water balance.

It is noted that a key objective of the Master Plan is to comply with the LSRCA Guidance document on 'Comprehensive SWMMP Guidelines' completed in April 2011. This Master Plan and the requirements herein are subject to the guidelines and criteria as outlined in this document.

1.8. Purpose of the Project File

This Project File documents the planning and design process followed and conclusions reached for the Town's SWMMP EA Study. In accordance with the Municipal Class EA planning and design process, the problems and opportunities associated with this study were investigated and documented. Subsequently, a number of alternative solutions were identified and evaluated. This information was presented to stakeholders at a Public Information Centre (PIC) on March 3, 2011. The Project File documents the EA process followed and is structured for ease of public review. In addition to the alternative evaluation, a detailed assessment of existing conditions was developed to meet the requirements of the Lake Simcoe Protection Plan (LSPP). The project file is intended to meet (where appropriate) the objectives as set out in the LSPP, the Town OP and the Municipal Class EA document.

2.0 Planning Context and the EA Planning Process

2.1. Municipal Class EA

The Municipal Class EA (2007) planning and design process was followed for this project as it allows the Town to meet the requirements of the Ontario Environmental Assessment Act (OEAA) for municipal infrastructure without having to either undertake an Individual Environmental Assessment or request a specific exemption for the project. The Class EA is a planning process approved under the OEAA for a class or group of undertakings including municipal infrastructure. Municipal projects included in the Class EA may be implemented without further approval under the OEAA, provided that the approved Class EA planning and design process (**Figure 2-1**) is followed.

EXHIBIT A.2

MUNICIPAL CLASS EA PLANNING AND DESIGN PROCESS

NOTE: This flow chart is to be read in conjunction with Part A of the Municipal Class EA

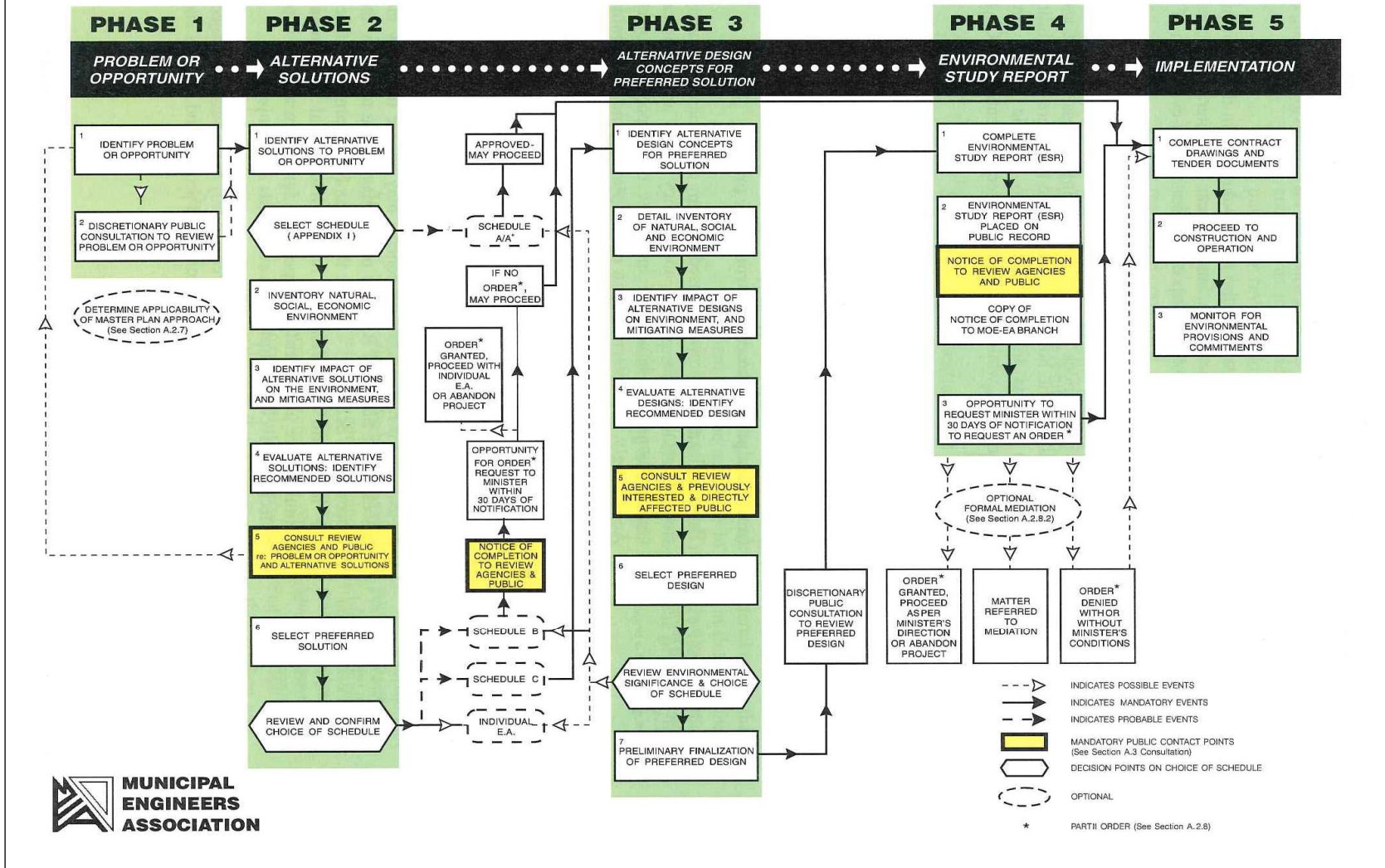


Figure 2-1 – Municipal Class EA Process

2.1.1. Four (4) Project Schedules

Since projects undertaken by municipalities vary in their environmental effects, the Class EA classifies these projects into four (4) schedules according to their environmental significance:

1. **Schedule 'A'**: Projects are limited in scale, have minimal adverse effects and include the majority of municipal maintenance and operational activities. These projects are approved and may proceed directly to Phase Five for implementation without following the other phases.
2. **Schedule 'A+'**: Projects are limited in scale and have minimal adverse effects. These projects are approved and may proceed directly to Phase Five for implementation without following the other phases. However, the public is to be advised prior to project implementation though there is no ability for the public to request a Part II Order.
3. **Schedule 'B'**: Projects have the potential for some adverse environmental effects. The municipality is required to undertake a screening process (Phases One and Two) involving mandatory contact with directly affected public and relevant review agencies to ensure that they are aware of the project and that their concerns are addressed. Schedule 'B' projects require that a Project File report be prepared and submitted for review by the public and review agencies. If there are no outstanding concerns, then the municipality may proceed to Phase Five for implementation.
4. **Schedule 'C'**: Projects have the potential for significant environmental effects and must proceed under the full planning and documentation procedures specified in the Class EA Document (Phases One to Four). Schedule 'C' projects require that an Environmental Study Report be prepared and submitted for review by the public and review agencies. If there are no outstanding concerns, then the municipality may proceed to Phase Five for implementation.

2.1.2. Master Plan Classification

As the Master Plan process does not fall into a specific schedule, they are specified as “long range plans which integrate infrastructure requirements for existing and future land use with environmental assessment planning principles. These plans examine an infrastructure system(s) or group of related projects in order to outline a framework for planning for subsequent projects and/or developments.”

Section A.2.7 of the Municipal Class EA document states:

“At a minimum, Master Plans address Phase 1 and 2 of the Municipal Class EA process”.

Since the Town’s SWMMP Class EA is a high level document relating to the future planning of SWM guidelines and principles, it is classified as a Master Plan project.

2.1.3. Master Plan EA Process

The following activities were carried out for this Study.

Phase One: Identify the Problem / Opportunity

This phase involves identifying the problem / opportunity to be addressed through the study and describing it in sufficient detail to lead to a clear problem / opportunity statement.

Phase Two: Identify and Evaluate Alternative Solutions to the Problem / Opportunity

This phase involves six (6) steps:

1. Identify reasonable alternative solutions to the problem / opportunity (**Section 8.0**);
2. Prepare a general inventory of the existing natural, social and economic environments in which the project is to occur (**Section 3.0**);
3. Identify the net positive and negative effects of each alternative solution, including mitigating measures (**Section 4.0**);
4. Evaluate the alternative solutions (**Section 9.4**);
5. Consult with review agencies and the public to solicit comment and input (**Section 11**); and,
6. Select or confirm the preferred solution(s).

It is noted that the evaluation and selection of a preferred solution is developed on a settlement area basis and is subject to individual site specific analysis during later phases of the EA process. All preferred solutions are to be confirmed at the detailed design stage.

2.2. Public Consultation

In order to fulfill the requirements for a Municipal Class EA, a Public Information Center (PIC) was held on March 3, 2011 at the East Gwillimbury Town Center in Sharon. **Section 11** describes the public consultation process in detail.

3.0 Existing Conditions

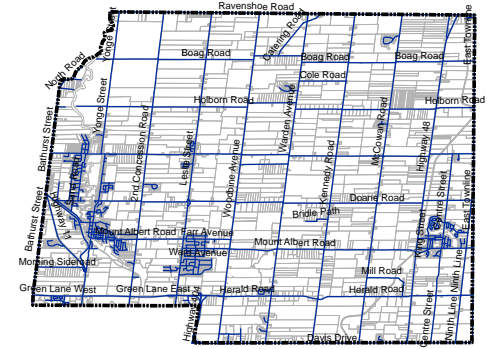
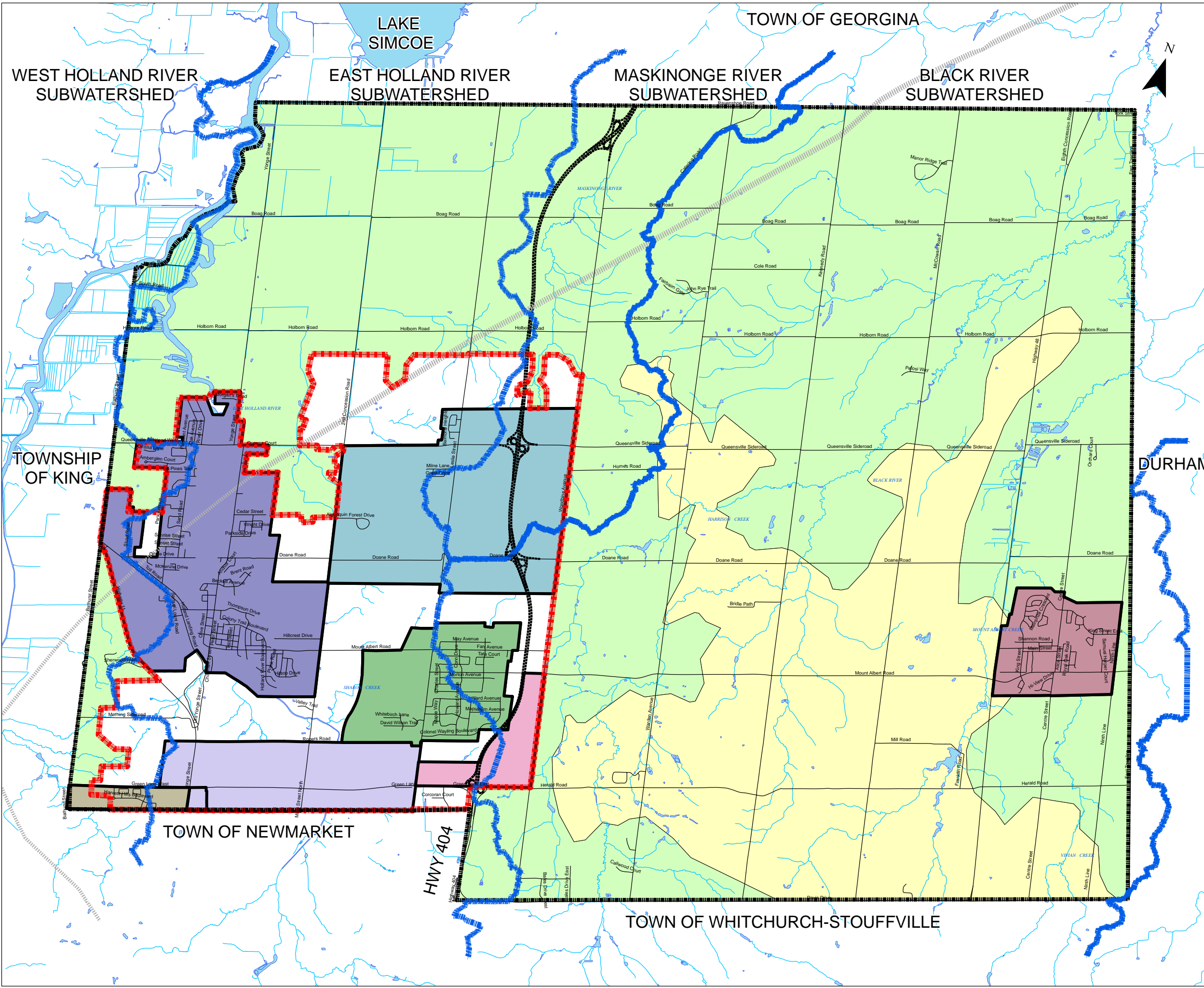
3.1. General

This chapter will summarize existing environmental conditions within the Town. It should be noted that for the purpose of this study, existing environmental conditions were based on a review of existing information and documentation specifically the Town consolidated OP 2031. The existing information reviewed is outlined in **Section 1.4** of this report. Further information on the characterization of existing environmental conditions can be found in the Town consolidated OP 2031.

3.2. Natural Environment

3.2.1. Natural Heritage

Natural Heritage features within the Town are identified as wetlands, seepage areas and springs, fish habitat, permanent and intermittent streams, lakes and significant woodlands, all of which need to be protected for the long term. Many of these areas are identified within the Oak Ridges Moraine and Greenbelt areas within the Town (**Figure 3-1**). Protected countryside, which includes the Oak Ridges Moraine and the Greenbelt, represents approximately 75% of the Town. In the OP, the Town supports permanent protection of the Natural Heritage System, key features of the Oak Ridges Moraine and Greenbelt, and the supplementary environmental corridors (**Figure 3-2**).



KEY MAP
SCALE: 1:150,000

LEGEND

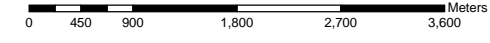
- SUBWATERSHED BOUNDARY
- TOWN BOUNDARY
- CENTRAL GROWTH AREA
- HYDRO CORRIDOR
- PROVINCIAL FREEWAY ROUTE (APPROVED)
- GREEN LANE WEST COMMUNITY PLAN
- HOLLAND LANDING - RIVER DRIVE PARK COMMUNITY PLAN
- MOUNT ALBERT COMMUNITY PLAN AREA
- QUEENSVILLE COMMUNITY PLAN
- SHARON COMMUNITY PLAN AREA
- HWY 404 EMPLOYMENT AREA
- GREEN LANE CORRIDOR COMMUNITY PLAN
- WATERCOURSE
- WATERBODY
- GREENBELT**
- OAK RIDGES MORAIN (ORM)
- PROTECTED COUNTRYSIDE

NOTES:

AS PER THE TOWN OF EAST GWILLIMBURY CONSOLIDATED OFFICIAL PLAN 2031



SCALE: 1:30,000



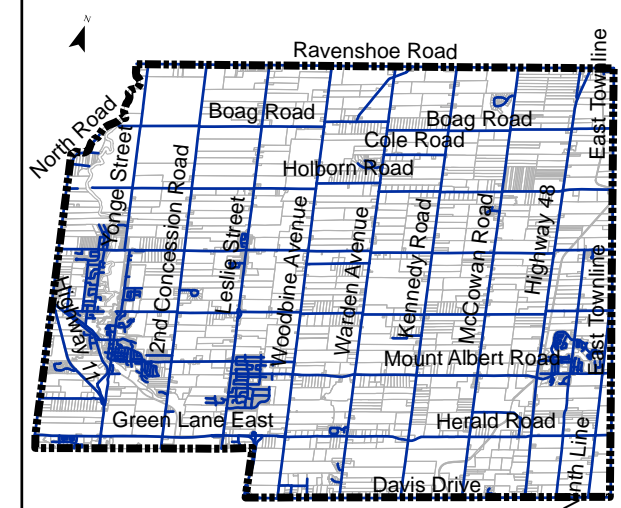
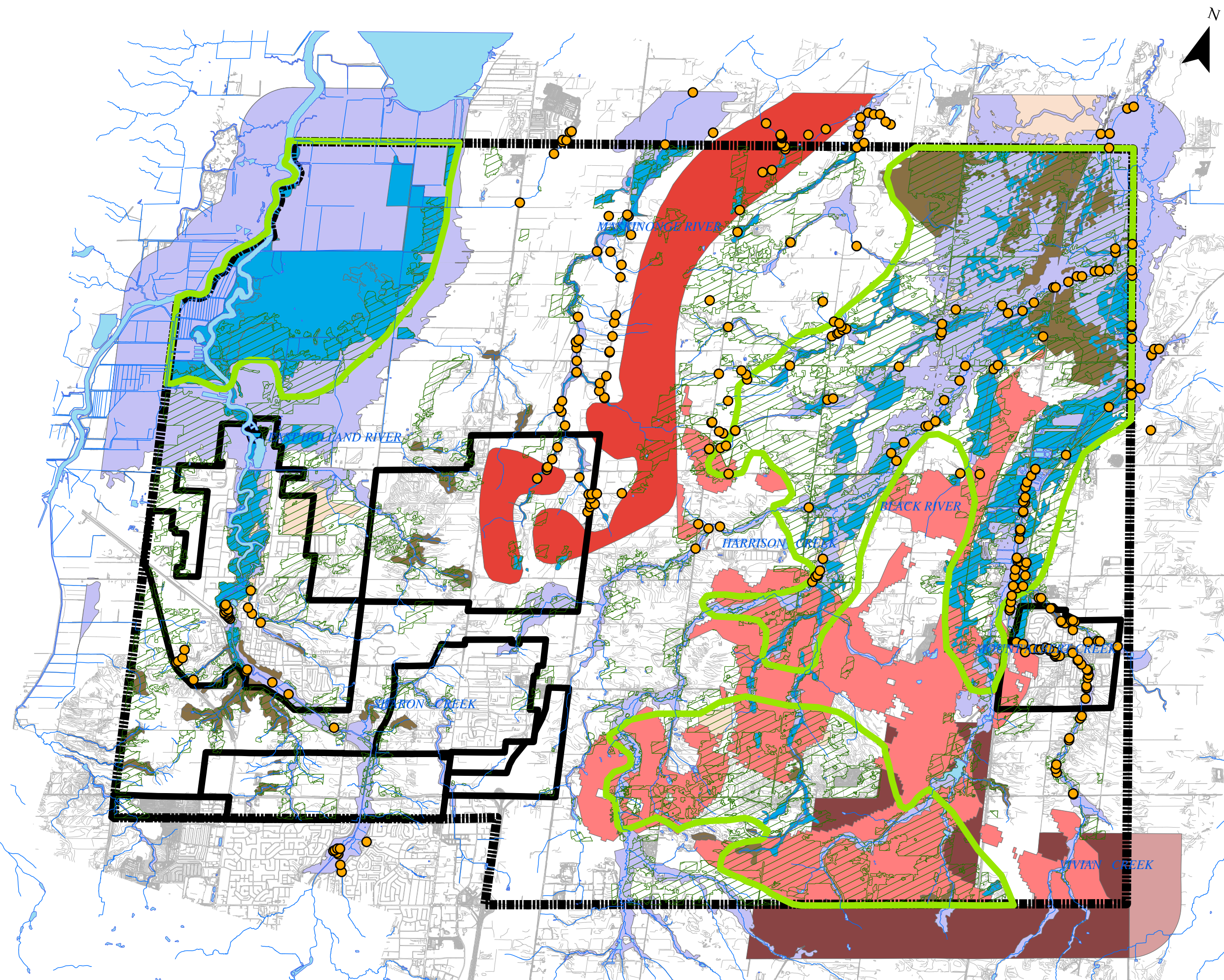
**STORMWATER MANAGEMENT
MASTER PLAN
LOCATION MAP**

FIGURE 3-1

DATE: JAN 2011

3.2.2. Natural Hazards

Natural hazards within the Town consist of erosion prone areas and areas within the floodplain. These areas present a natural threat to human safety and the environment. If stormwater is left uncontrolled the risk may increase. The floodplain areas were obtained from the LSRCA. Flooding risks generally occur around watercourses. The Town has numerous wetland areas. These wetlands are largely concentrated in the Holland Marsh area, part of the East Holland subwatershed. Wetlands are often subject to more than one hazard such as flooding, erosion and subsidence which make them poor sites for development. Many of these wetlands are deemed Provincially Significant wetlands in which the Provincial Policy Statement states that “Development and site alternation shall not be permitted on significant wetlands.” Significant wetlands, existing flood and erosion hazards are outlined in **Figure 3-2**.



KEY MAP
SCALE: 1:250,000

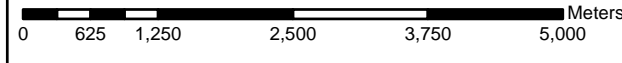
LEGEND

- TOWN BOUNDARY
- BIG WOODS
- REGIONALLY SIGNIFICANT FORESTS
- EROSION SITES
- PROVINCIALLY SIGNIFICANT WETLANDS
- ENVIRONMENTALLY SIGNIFICANT AREA (BIOLOGICAL)
- ENVIRONMENTALLY SIGNIFICANT AREA (PHYSICAL)
- SIGNIFICANT VALLEYLANDS
- HIGH AQUIFER VULNERABILITY
- MASKINONGE RIVER SIGNIFICANT GROUNDWATER RECHARGE
- PEFFERLAW INFILTRATION AREA
- VIVIAN INFILTRATION AREA
- FLOODPLAIN

NOTES:



SCALE: 1:70,000



**STORMWATER MANAGEMENT
MASTER PLAN**

NATURAL HAZARDS AND NATURAL HERITAGE AREAS

3.2.3. Groundwater Vulnerability

Groundwater vulnerability is also stated as aquifer vulnerability and refers to the shallow groundwater aquifer's susceptibility to contamination from both human and natural sources. The vulnerability of groundwater to contamination depends on soil types, water table elevation, contaminant concentration and the confined / unconfined nature of the aquifer. These areas need to be protected in order to reduce contamination and protect the groundwater. Measures to infiltrate stormwater that are in these areas require detailed studies to ensure that they are not impacting the Town's groundwater. Groundwater vulnerability sites within the Town are illustrated above in **Figure 3-2**.

3.2.4. Groundwater Recharge / Discharge

Discharge potential occurs in areas where the static water table intersects the ground surface. Groundwater discharge areas are often in low topographic areas and can be observed above in **Figure 3-2**. Discharge rates vary throughout the year due to seasonal and longer term changes in recharge and groundwater potentials. Groundwater is replenished as precipitation or snowmelt infiltrates into the ground surface. Significant recharge areas can be described as areas that can effectively move water from the surface through the unsaturated soil to replenish available groundwater resources. Once again infiltration measures in recharge areas require analysis to ensure minimal impact to groundwater. These areas within the Town are also indicated above in **Figure 3-2**.

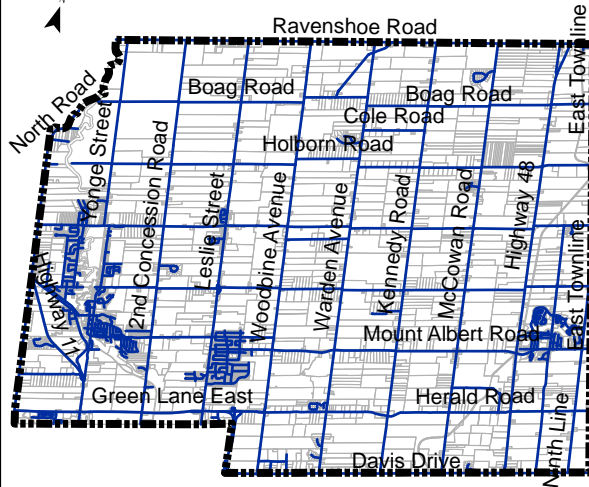
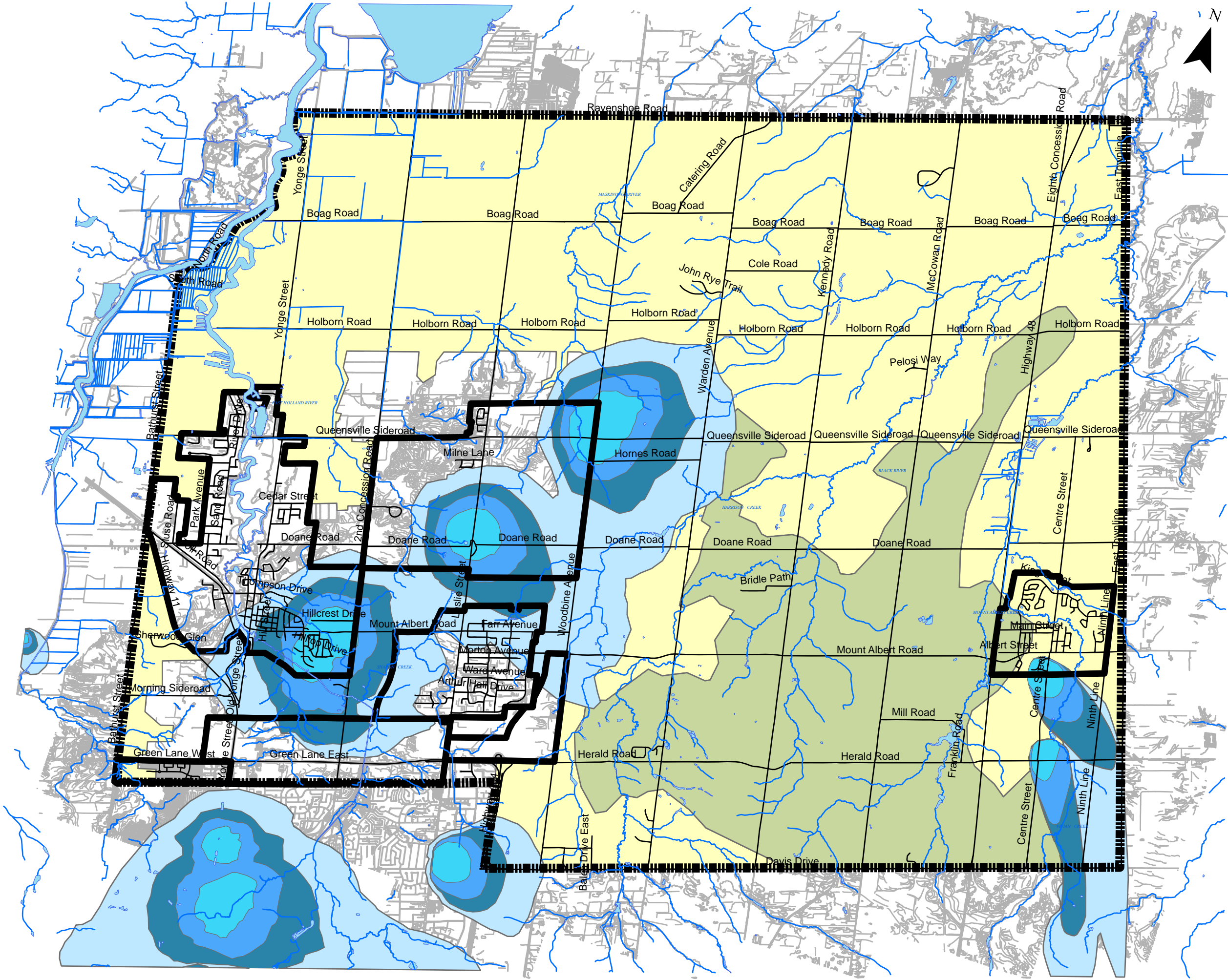
3.2.5. Well Head and Intake Protection Zones

Wellhead Protection Areas are zones established by the Region of York that are in the vicinity of domestic water supply wells. They are in place to protect groundwater quality from degradations and to ensure sources of water are not compromised in the future as a result of land use decisions. These areas are displayed by *time-of-travel zones* which represent the travel time for groundwater to reach the municipal well.

Wellhead protection areas are based on time travel zones as follows:

- 100 metre pathogen zones around each wellhead; and,
- Time of travel zones of 0 to 2 years, 2 to 5 years, 5 to 10 years and 10 to 25 years.

As per the Town's OP: "the storage or use of pathogen threats by new land uses, including the siting and development of SWM ponds and rapid infiltration basins or columns, except for storage of manure or personal family use, is prohibited within the 100 metre pathogen zone around each municipal well and may be restricted within the 100 metres to two (2) year time of travel zone". The wellhead and intake protection zones within the Town are located below in **Figure 3-3**.



KEY MAP
SCALE: 1:250,000

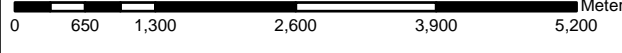
LEGEND

- TOWN BOUNDARY
- WELLHEAD PROTECTION AREA**
- 10 YEAR ZONE
- 25 YEAR ZONE
- 2 YEAR ZONE
- 5 YEAR ZONE
- GREENBELT**
- OAK RIDGES MORAIN (ORM)
- PROTECTED COUNTRYSIDE

NOTES:



SCALE: 1:70,000



**STORMWATER MANAGEMENT
MASTER PLAN**

WELLHEAD PROTECTION ZONES

3.2.6. Watercourses / Waterbodies

Significant watercourses within the Town include Holland River, Sharon Creek, Harrison's Creek, Vivian Creek, Mount Albert Creek, Maskinonge River, and Black River (**Figure 3-1**). Cook's Bay is situated NW to the Town, which connects to Lake Simcoe. In addition, other significant waterbodies include Soldier's Bay, Roger's Reservoir and Franklin Pond.

3.2.7. Significant Valleylands

Valleylands are considered to be the "backbone" of a watershed as it supports many important ecological functions such as channelling water and wildlife, and providing a connection between natural heritage features. Valleylands are natural depressions often associated with a river or a stream. In the town of East Gwillimbury, all four (4) subwatersheds contain valleylands. The largest valleyland is within the Black River subwatershed just north of the Oak Ridges Moraine associated with the main branch of the Black River (**Figure 3-2**). Within Secondary Plan Areas the Town identifies valleylands as follows;

- Width of 25m;
- Length of 50m;
- Slope of 15%; and,
- Height of 5 m.

Areas that meet these criteria within the settlement areas are not to be developed or have any type of site alteration. Field investigations are recommended to determine location and appropriate top-of-bank boundary.

3.2.8. Big Woods Policy Areas and Regionally Significant Forests

Regionally Significant forests within the Town consist of Brown Hill Regional Forest as well as Bendor & Graves Regional Forest. The Town has laid out policies to maintain a large percentage of woodland cover and plans for future replacement efforts. The Town has identified where there is a high percentage of woodland cover (**Figure 3-2**). In order to maintain these areas the Town has created the Big Woods Policy to ensure no net loss of woodland cover in these areas. The goal as set out in the OP is to promote and establish programs in increase forest cover of the Town to 30%.

3.2.9. Storm Water Quality and Quantity

To ensure the health and sustainability of the subwatersheds in both the Town and downstream areas, SWM practices are put in place.

The Town's OP outlines the future objectives in regards to SWM and is as follows:

- Ensure that SWM systems are designed and constructed in accordance with sound environmental and engineering best practices;
- Pursue and implement sound SWM practices that will ensure adequate protection from flooding and erosion, maintain and/or improve water quality and enhance the environmental, aesthetic and recreational potential of watercourses; and,

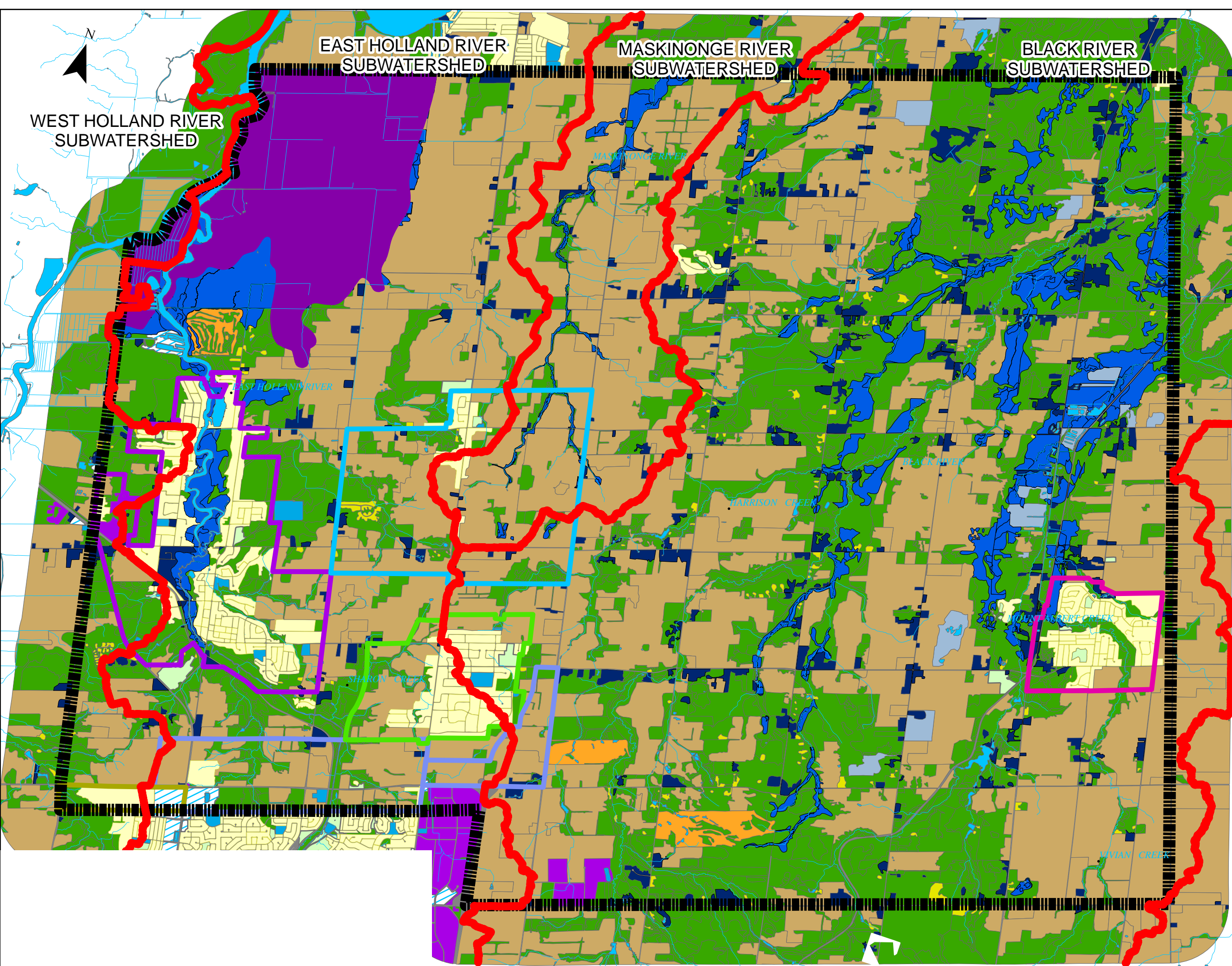
- Reduce phosphorus loading to Lake Simcoe and its tributaries.

Currently, there are no specific SWM standards set in place for the Town. As a part of this study, the development of policies and guidelines for management of the Town's storm drainage system based on current SWM Best Management Practices (BMPs) and development policies and guidelines to apply to future development has been established and further discussed in **Section 12.1.4**.

3.3. Socio-Economic Environment

3.3.1. Agricultural Lands

More than 25% of the Town is farmland and is important to the livelihood of many residents. Agriculture use is mostly for row crops as well as specialty crops grown in the marshes of Holland Landing. Lands designated for agriculture use by the Town are outlined below in **Figure 3-4**.

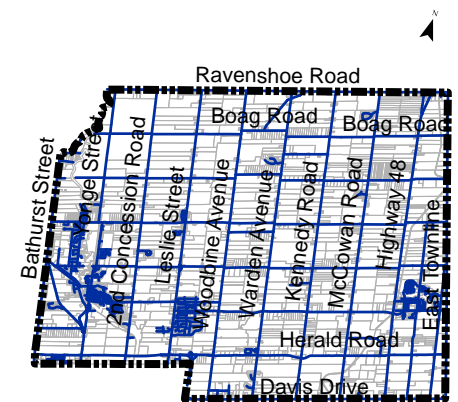


WEST HOLLAND RIVER
SUBWATERSHED

EAST HOLLAND RIVER
SUBWATERSHED

MASKINONGE RIVER
SUBWATERSHED

BLACK RIVER
SUBWATERSHED



KEY MAP
SCALE: 1:350,000

LEGEND

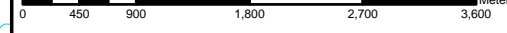
- SUBWATERSHED BOUNDARY
- TOWN BOUNDARY
- HOLLAND MARSH
- PROVINCIALLY SIGNIFICANT WETLANDS
- LANDUSE**
- ACTIVE AGGREGATE, PEAT EXTRACTION
- COMMERCIAL
- ESTATE RESIDENTIAL
- GOLF COURSE
- INDUSTRIAL
- INSTITUTIONAL
- INTENSIVE AGRICULTURE; NON-INTENSIVE AGRICULTURE
- MANICURED OPEN SPACE
- NATURAL HERITAGE FEATURE
- RAIL ROAD
- RURAL DEVELOPMENT
- LOW DENSITY RESIDENTIAL
- COMMUNITY AREA**
- GREEN LANE EAST
- GREEN LANE WEST
- HOLLAND LANDING
- MOUNT ALBERT
- QUEENSVILLE
- SHARON

NOTES:

Landuse data provided by the Lake Simcoe Region Conservation Authority



SCALE: 1:60,000



**STORMWATER MANAGEMENT
MASTER PLAN
EXISTING LAND USE**

3.3.2. Land Use

The Town encompasses a mix of land uses including fully serviced urban areas, partially-serviced suburban areas, rural hamlets, estate residential subdivisions and rural agricultural land. The urban areas of East Gwillimbury consist of Green Lane Corridor, Holland Landing, Mount Albert, Queensville and Sharon. A detailed discussion of existing and future land use is provided in **Section 4.2**.

3.4. Municipal Infrastructure

3.4.1. Inventory of Stormwater Management Facilities (SWMF)

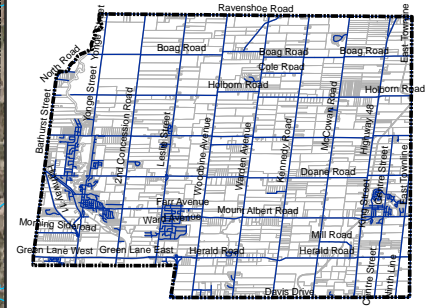
The existing SWMF in the Town were inventoried and surveyed by Cole Engineering as a part of this study. Existing SWMF that were accessible and approved by the Town were surveyed and characterized. The most common types of SWMFs in the Town include wet ponds, dry ponds and artificial wetlands. **Section 7.0** further describes the survey process and the database inventory (SWMSOft) utilized. The existing pond inventory is illustrated below in **Figure 3-5**.

WEST HOLLAND RIVER SUBWATERSHED

EAST HOLLAND RIVER SUBWATERSHED

MASKINONGE RIVER SUBWATERSHED

BLACK RIVER SUBWATERSHED



KEY MAP
SCALE: 1:175,000

LEGEND

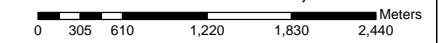
- COMPARISON POINTS
- ☆ UNCONTROLLED OUTFALL RETROFIT OPPORTUNITY
- ☆ EXISTING POND RETROFIT OPPORTUNITY
- PERMANENT FACILITIES UNDER PRIVATE OWNERSHIP
- CURRENT TOWN RETROFIT PROJECTS
- SUBWATERSHED BOUNDARY
- TOWN BOUNDARY
- SETTLEMENT AREA
- SURVEYED SWM FACILITY IN SWMSOFT**
- DRY POND
- WET POND
- STORMWATER DRAINAGE AREA**
- LEVEL 1 CONTROL
- QUANTITY CONTROL
- UNCONTROLLED
- WATERCOURSE
- 18 POND ID

NOTES:

IT IS NOTED THAT DATA GAPS EXIST FOR DRAINAGE AREAS FOR SWM PONDS SURVEYED OUTSIDE OF SETTLEMENT BOUNDARIES.



SCALE: 1:25,000



STORMWATER MANAGEMENT MASTER PLAN
STORMWATER MANAGEMENT FACILITIES

FIGURE 3-5 DATE: JAN 2012

IMAGE: First Base Solutions Inc, 2009 Orthophotography

3.4.2. Transportation Network

Expansion of Highway 404 is underway and the link between Highways 400 and 404 has been proposed. The extension would improve the capacity of the transportation network in northern York and Simcoe County to meet the forecasted increase in commuter and recreational travel needs. The alignment for the proposed link is conceptual in order to recognize a future route approved by the Provincial accordance with the Environmental Assessment Act and related Controlled Access Highway designation (**Figure 3-1**).

3.4.3. Municipal and Private Services

Throughout the communities of Queensville and Sharon municipal water is available as well as in Bales Drive Industrial Park, located southeast of Sharon. For Mount Albert, municipal water is not currently available for parts of Princess and Albert Streets. Municipal water is not currently available for the areas east of the Holland River and north of Doane Road in Holland Landing.

There are future plans for municipal sewers for the communities of Queensville and Sharon. There are no sewer services currently available for the Bales Drive Industrial Park, located southeast of Sharon. Municipal sewers are installed north of Mount Albert Road, excluding Samuel Harper Court, Haig Crescent and parts of Princess and Albert Streets in the community of Mount Albert. In Holland Landing, municipal sewers are currently located east of Holland River and south of Doane Road. Currently, no municipal sewers are available east of the Holland River and north of Doane Road. In the future, plans are being made for municipal sewers to be installed west of Holland River and south of Holland Landing Road.

3.4.4. Utility Corridors

A high voltage transmission line runs through the Town and is illustrated in **Figure 3-1**. Any future high voltage transmission lines should be relocated on the basis of a comprehensive study and in consultation with the Town. Utility infrastructure should be located to minimize any adverse social, land use, visual, environmental or safety impacts. Encouragement should be made to locate the new transmission lines along existing utility corridors or right-of-way (i.e. roads and railways) in order to minimize the fragmentation of properties. Where it is feasible, utility infrastructure should be buried within road allowances.

3.4.5. Summary of Environmental Conditions

The existing environmental conditions as described in **Section 3.0** provide a general characterization of the existing conditions which are consistent with the available subwatershed studies (i.e. East Holland River, West Holland River, Maskinonge River and Black River). Reference should be made to the LSRCA subwatershed studies for detailed information and evaluations on a subwatershed basis. In addition, reference should be made to the Town Consolidated Official Plan 2031 which provides further existing information on a planning perspective.

4.0 Evaluation of the Cumulative Environmental Impact of Stormwater

4.1. General

As set out in Policy 4.5 SA b. of the LSPP, an evaluation of the environmental impact of stormwater from existing and planned development is required based on the changes in land use from the future conditions of the Town's OP 2031.

4.2. Land Use Changes

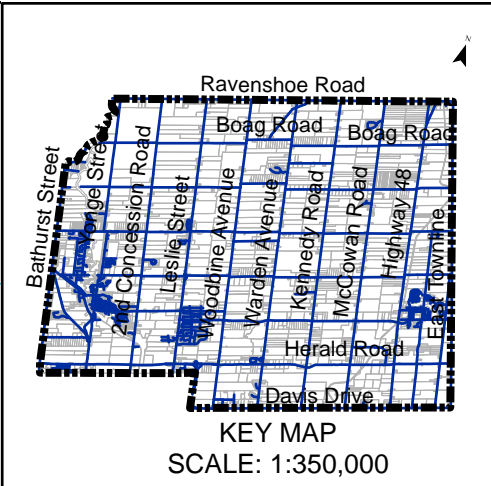
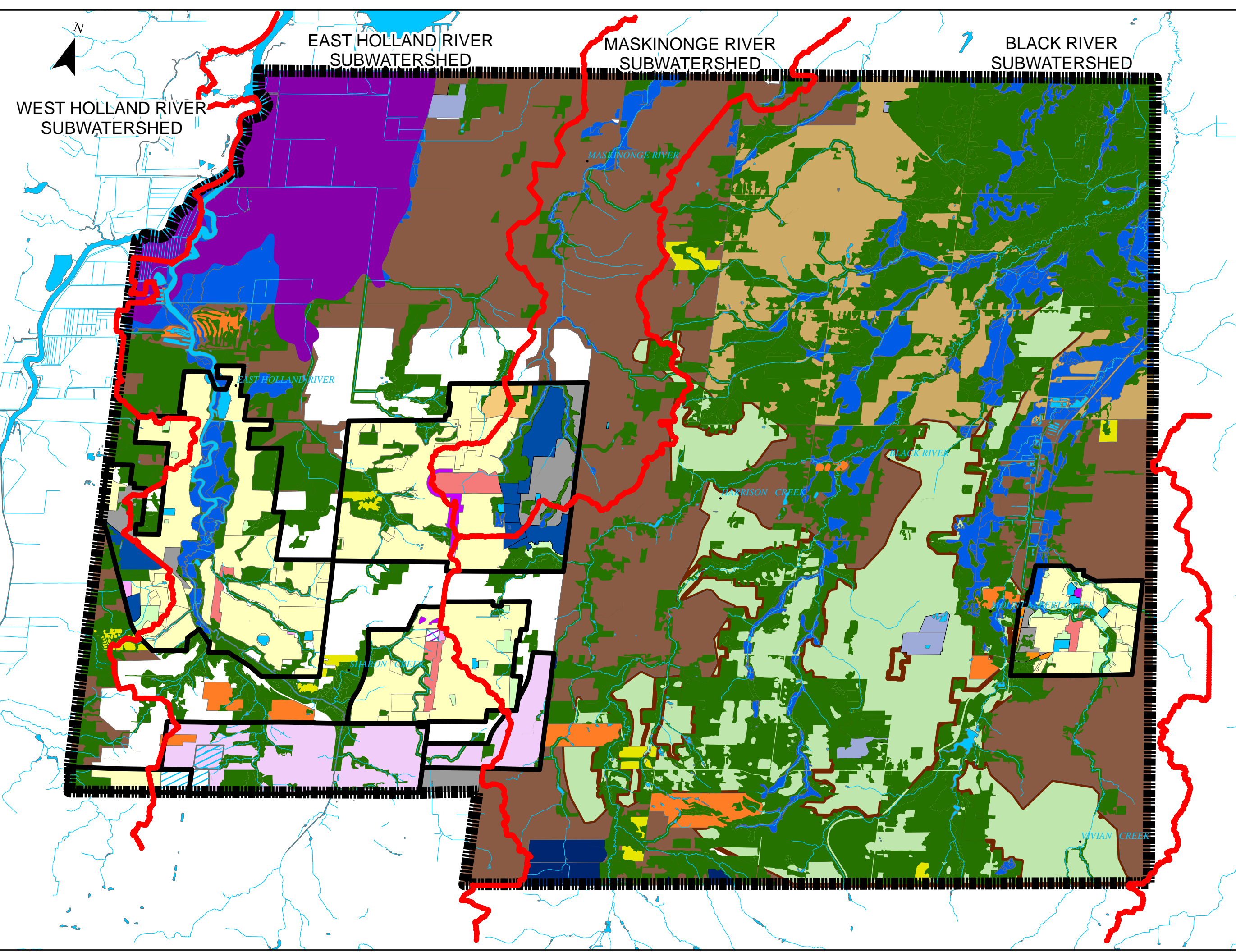
4.2.1. Existing Land Use

The Town is primarily rural, with a large percentage of the Town's land area used of agricultural purposes. Existing settlement areas include Sharon, Holland Landing, Queensville, Mount Albert and Green Lane West. Existing land uses within the Town can generally be classified into a number of categories, including agriculture, open space, environmental, low-medium residential, institutional, commercial and employment. The existing land use within the Town is illustrated in **Figure 3-4**. The existing land use in each drainage area is summarized in **Appendix A**.

4.2.2. Future Land Use

As outlined in the Town's new Consolidated Official Plan (June 2010), the Town will transition from a rural community of communities to a connected urban area, surrounded by a protected countryside. The Protected Countryside includes the Oak Ridges Moraine and Greenbelt Plan Area, which represent approximately 75% of the Town's land area. While this area will be protected from development, the Town's Central Growth Area will grow into a balanced urban area with a mix of residential, institutional, commercial and employment uses. The future land uses in the Town are illustrated below in **Figure 4-1**. The future land use in each drainage area is summarized in **Appendix B**.

Due to the changes in land use, a comparison of the impacts of stormwater from in relation to peak flows, phosphorus, water budget and erosion will be assessed in the following sections.



LEGEND

SUBWATERSHED BOUNDARY	AGGREGATE RESOURCE AREAS
TOWN BOUNDARY	RURAL INDUSTRIAL AREA
COMMUNITY PLAN AREAS	PRIME AGRICULTURAL AREA
WATERBODY	RURAL AREA
WATERCOURSE	ESTATE RESIDENTIAL AREA
HOLLAND MARSH	NEW SECONDARY PLAN AREAS
WETLANDS	OPEN SPACE
EPA	OAK RIDGES MORAINE
RECREATION AREAS	
MIXED USE RESIDENTIAL	
LOW DENSITY RESIDENTIAL	
VILLAGE CORE AREAS	
POST SECONDARY INSTITUTION	
INSTITUTIONAL	
PRESTIGE EMPLOYMENT	
COMMERCIAL	
COMMUNITY COMMERCIAL	
GENERAL EMPLOYMENT	
NEIGHBOURHOOD COMMERCIAL	

NOTES:
AS PER THE TOWN OF EG CONSOLIDATED OP 2031.

COLE ENGINEERING
Experience Enhancing Excellence

Town of East Gwillimbury
Our Town, Our Future

SCALE: 1:60,000

0 425 850 1,700 2,550 3,400 Meters

STORMWATER MANAGEMENT MASTER PLAN
FUTURE LAND USE

FIGURE 4-1 **DATE: JAN 2012**

4.3. Peak Flow Modelling

4.3.1. General

As part of Policy 4.5 SA of the LSPP, an assessment of stormwater peak flows from existing and future conditions is required to evaluate the environmental impact of the future development in the Town. The hydrologic models and reports received from LSRCA in February 2011 entitled *Hydrologic and Hydraulic Modeling For the West Holland River, East Holland River and Maskinonge River Watersheds* (CCL, 2005) and *Black River Hydraulic Model Update* (LSRCA, 2005) were reviewed and updated to include the revised land use in the Town.

It is noted that only the Future Condition hydrologic models were updated to evaluate the impacts of stormwater as part of this study. The Existing Condition model representative of land use from the end of year 2004 (CCL, 2005 and LSRCA, 2005) was unchanged.

4.3.2. Objectives

The objectives of the hydrologic model update are summarized as follows:

- Assess the changes in peak flows at each subcatchment from existing to future conditions;
- Verify if the future condition lumped approach (CCL, 2005) is required to be updated with the changes in land use within the Town;
- Apply an over control scenario if the future conditions lumped approach (CCL, 2005) does not meet post-to-pre criteria; and,
- Confirm the stormwater criteria and assumptions as outlined in the *Hydrologic and Hydraulic Modeling For the West Holland River, East Holland River and Maskinonge River Watersheds* (CCL, 2005) and *Black River Hydraulic Model Update* (LSRCA, 2005) are maintained with the future conditions update.

4.3.3. Model Background

The hydrologic model received from LSRCA which was reviewed and updated was modelled using Visual OTTHYMO Version.2.3. This modelling software allows users to model the storm effects and define the hydrologic input parameters of a given site area.

The main modelling inputs used in the update are described below:

- STANDHYD Command:
 - An area input command mainly used for impervious areas; and,
 - Impervious area input parameters may be assigned to this command.
- NASHYD Command:
 - An area input command mainly used for pervious areas; and,
 - Pervious area input parameters may be assigned to this command.
- ADDHYD Command:
 - An input command used to add two (2) areas together.

- ROUTE RESERVOIR Command:
 - A command used to represent a SWMF or control device; and,
 - The user may input a rating curve to represent a storage-discharge relationship of a SWMF.

It is noted that other existing modelling input commands (i.e. ROUTE CHANNEL and READHYD) were deemed acceptable and were not updated as part of this study.

4.3.4. Hydrologic Model Setup

To update the hydrologic model, an initial review was undertaken to determine the initial set up of the model. The received hydrologic model was developed using a number of approaches. The existing and future condition both incorporate the lumped approach described below:

1. Lumped Approach

All ponds within a subcatchment area are lumped (i.e. all ponds in a single catchment area were lumped into one single pond located at the outlet of the catchment). Under this approach, the discharge and active storage relationship for the lumped pond is determined through the sum of all ponds within a basin based on the return period (CCL, 2005).

Subcatchments are initially divided into two areas: the area that drains into the lumped ponds and the area that does not. The areas draining to the lumped ponds are attributed a numerical suffix of “1” after the subcatchment area ID and the areas not draining to the lumped ponds are given a numerical suffix of “2”.

It should be noted that the *Hydrologic and Hydraulic Modeling For the West Holland River, East Holland River and Maskinonge River Watersheds* (CCL, 2005) states that: “the future development within any given subwatershed may include the *approved future development* (future development with approved servicing and SWM plans containing approved future SWMFs) and the *additional future development* (the future development without servicing / SWM plans available). Therefore, the lumped pond within a specific subcatchment under the future landuse condition should include the lumped pond under the existing landuse condition, the storage-discharge relations of the approved future SWMFs, and the to-be-designed SWMFs’ storage-discharge relations for the additional future development”. The report also states that all SWMFs for additional future development are assumed to be off-line. For more information, reference should be made to the approved *Hydrologic and Hydraulic Modelling For the West Holland River, East Holland River and Maskinonge River Watersheds* Report (CCL, 2005).

To populate each lumped pond (ROUTE RESERVOIR Command), flows and storage values are required to generate the rating curve input parameter. The flow and storage values (i.e. storage-discharge relationship) are developed using average unit flows and unit active storage requirements as described below:

1. a) Post-to-Pre Approach

The “post-to-pre” peak flow control criterion is required for the design of SWMFs for additional future development under the 1:2 to 1:100 year design storms (CCL, 2005). In this Study, the return period control peak flows (rating curve input parameters) were estimated through the average unit flow relationship as per **Table 4-1** below.

Table 4-1 – Average Unit Flow Requirements for SWMFs

Average Unit Flow (m ³ /s/ha)						
25 mm Storm	1:2 Year Storm	1:5 Year Storm	1:10 Year Storm	1:25 Year Storm	1:50 Year Storm	1:100 Year Storm
0.0034	0.0194	0.0361	0.0500	0.0669	0.0840	0.0978

As per Table C9-A in the Hydrologic and Hydraulic Modeling For the West Holland River, East Holland River and Maskinonge River Watersheds (CCL 2005).

The average storage required for the design of SWMFs for additional future development were estimated in this Study based on the linear regression analysis conducted in the Hydrologic Study (CCL, 2005). The generalized unit active storage requirements were based on the weighted imperviousness, storm return period and control drainage area as per **Table 4-2** below (CCL, 2005).

Table 4-2 – Generalized Unit Active Storage Requirements for SWMFs

Unit Storage (m ³ /ha)							
Linear Regression Analysis Parameters	25 mm Storm	1:2 Year Storm	1:5 Year Storm	1:10 Year Storm	1:25 Year Storm	1:50 Year Storm	1:100 Year Storm
C1=Slope	1.867	2.422	2.136	2.106	1.915	1.553	1.201
C2=Intercept	38.173	32.362	86.332	123.051	174.221	227.826	280.180

As per Table C9-B in the Hydrologic and Hydraulic Modeling For the West Holland River, East Holland River and Maskinonge River Watersheds (CCL 2005).

Note: Unit Storage (m³/ha) = **C1*IMP+C2 = Slope*IMP+Intercept**

It is noted that the “post-to-pre” approach utilized in the original model was adopted in this Study to update the hydrologic model to future conditions.

1. b) Over-Control Approach

In the event the post-to-pre approach does not meet pre-development flow rates, the over-control approach will be applied to the stage-storage-discharge relationships at each future conditions subcatchment. In this approach, revised flows and storage values will be applied *to replace* the average unit flows and unit active storage populated from the post-to-pre approach as discussed above. The resulting peak flows (now below pre-development levels due to the revised stage-storage-discharge relationships) for each storm event will determine the over-control unit-flow rates required for each applicable drainage area. Subsequently, based on the total proposed development area within the drainage area, an over-control unit-flow rate will be generated.

An example of the approach is provided in **Table 4-3** for subcatchment area 9210. It is noted the “over-control” approach is *not* applied to drainage areas already meeting pre-development levels with the application of the “post-to-pre” approach.

Table 4-3 – Over-Control Approach Application to Stage-Storage-Discharge Curve Example (9210)

Event	Post-to-Pre Approach		Over-Control Approach	
	DISCH (m ³ /s)	STORAGE (ha-m)	DISCH (m ³ /s)	STORAGE (ha-m)
	0.0000	0.0000	0.0000	0.0000
25mm	0.2400	0.5431	0.2000	2.0000
2-year	1.3500	0.5855	1.9000	7.0000
5-year	2.5000	0.9171	2.0000	7.5000
10-year	3.4700	1.1672	2.3000	8.0000
25-year	4.6400	1.4936	3.5000	10.0000
50-year	5.8200	1.8113	4.5000	13.0000
100-year	6.7800	2.1218	5.5000	14.0000

For the above example, with a proposed development area of 176.4 ha, the unit-flow-rate during the 100-year condition is approximately 0.025 m³/s/ha or 25 L/s/ha. The resulting peak flows and unit flow rates for all storm events are summarized in **Table 4-4** below.

Table 4-4 – Over-Control Approach Application Unit-Flow Rate Example (9210)

CONDITION (DEVELOPED AREA = 176.4ha)	PEAK FLOW (m ³ /s)					
	STORM EVENT					
	2-YEAR	5-YEAR	10-YEAR	25-YEAR	50-YEAR	100-YEAR
Pre-Development	1.289	2.078	2.659	3.436	4.041	4.665
Post-Development (Uncontrolled)	16.874	23.724	28.316	34.230	38.702	43.242
Post-to-Pre Approach	8.021	9.711	11.387	13.600	15.259	16.942
Over-Control Approach	1.066	1.778	2.313	3.322	3.875	4.443
Unit-Flow Rate (m ³ /s/ha)	0.006	0.010	0.013	0.019	0.022	0.025

It should be noted the purpose of the over-control approach is to reduce peak flow rates to below pre-development levels *should* the post-to-pre approach not meet the required quantity control criteria. The over-control approach and supplementary unit-flow-rates are further discussed in **Section 4.3.7**.

4.3.5. Methodology

The existing hydrologic model consists of analyzing the peak flows from individual subcatchments within each subwatershed (East Holland River, West Holland River, Maskinonge River and Black River). As growth within the Town is limited to the existing settlement area boundaries (i.e. Queensville, Sharon, Holland Landing, Green Lane and Mount Albert), adjacent subcatchment areas that drain into or from the settlement areas are analyzed.

The hydrologic modelling update will be conducted through the following steps:

- Identify existing subcatchment areas intersecting each settlement area (i.e. Queensville and Holland Landing); and,

- Divide subcatchment areas into i) Area within settlement area; and ii) External area outside of settlement area (**Figure 4-2**) below.

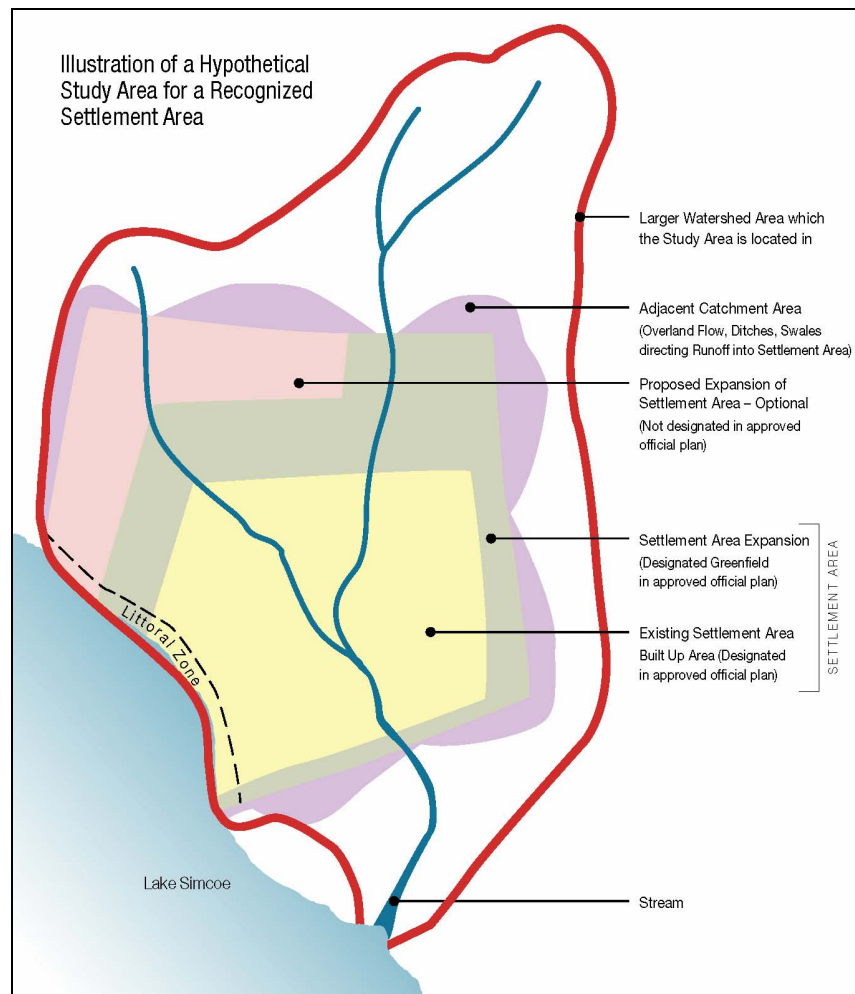


Figure 4-2 – Illustration of Adjacent Catchment Area to Settlement Area (Comprehensive SWM Master Plan Guidelines – April 2011)

- Revise hydrologic input parameters for all applicable drainage areas associated with the changes in land use from the Town's Consolidated OP 2031;
- If the imperviousness of a drainage area > 20%, a STANDHYD command will be implemented, otherwise a NASHYD command will be utilized;
- The existing command type of the external area will be maintained and the area adjusted for the external subcatchment areas; and,
- Compare the revised future peak flows to existing conditions and analyze results.

Due to the number of revisions to the hydrologic models, a numbering system is proposed for subcatchment areas added to the model. For example, if the subcatchment area 244 is located within the Queensville settlement area, the ID will end with a "9" and renumbered as 2449. The modelling numbering system is outlined in **Table 4-5**.

Table 4-5 – VO2 Modelling Numbering System

East Gwillimbury Settlement Area	Numerical Suffix
Queensville	9
Holland Landing	7
Sharon	3
Green Lane	4
Mount Albert	6

It should be noted that the numerical suffixes utilized in the hydrologic modelling and summarized above in **Table 4-5**, are arbitrary in nature and were chosen to not conflict with the existing numerical suffixes used in the Hydrologic and Hydraulic Modeling for the West Holland River, East Holland River and Maskinonge River Watersheds Report (CCL 2005).

To evaluate the effects on peak flows from future development, a number of additional assumptions were made for the purpose of the hydrologic analysis:

- All areas of development within the settlement areas of the Town will be controlled using the “post-to-pre” approach and drain into its own pond (storage-discharge inputs determined from unit flow and unit storage calculations);
- The lumped ponds and storage-discharge inputs remain the same as well as methodologies for calculating hydrologic input parameters;
- If the subcatchment area within the settlement area is less than the area to the lumped pond, the settlement area will be subtracted out and an additional STANDHYD or NASHYD command will be input into the model (**Figure 4-3**) below;

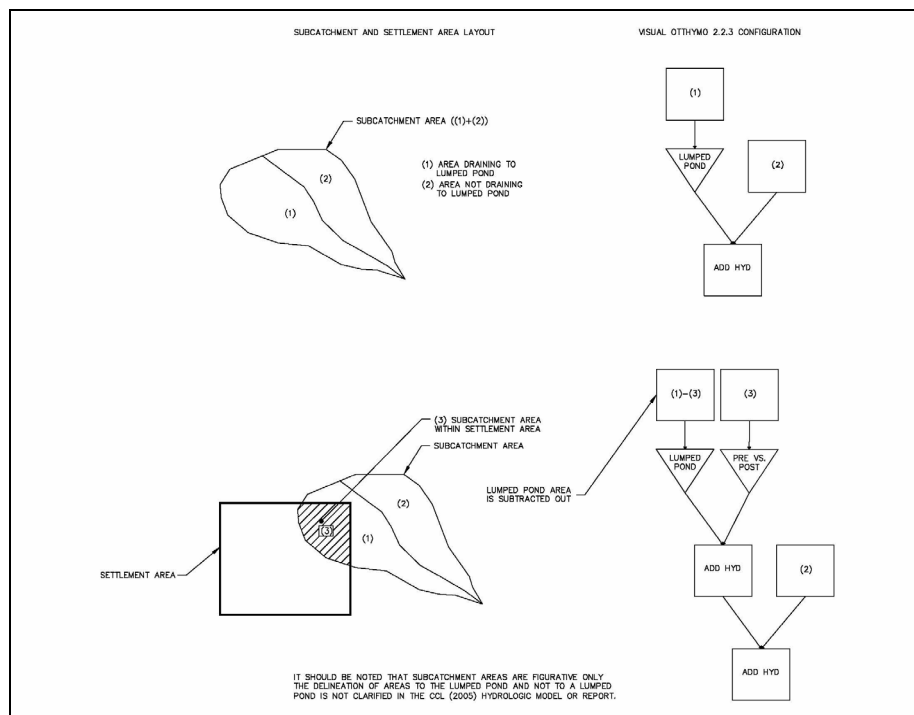


Figure 4-3 – Illustration of Settlement Area Layout and VO2 Configuration I

- If the subcatchment area within the settlement area is greater than the area to the lumped pond, the area to the lumped pond will be replaced (i.e. revised area, hydrologic parameters and numerical suffix). The difference in areas will be subtracted from the area not draining to the lumped pond (**Figure 4-4**) below; and,

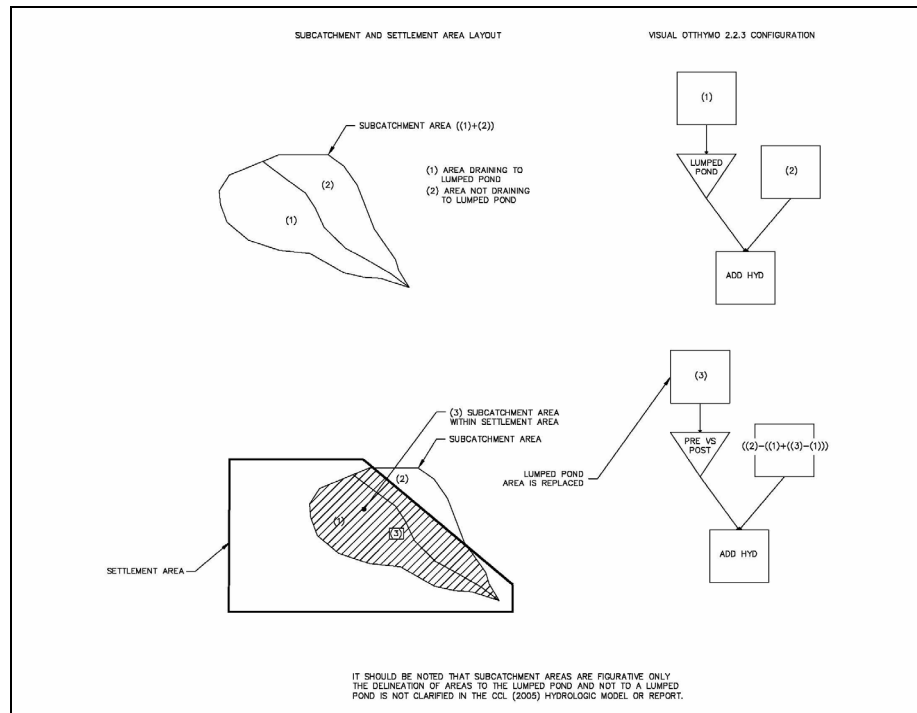


Figure 4-4 – Illustration of Settlement Area Layout and VO2 Configuration II

- All subcatchment areas within the settlement area will drain into its own “post-to-pre” ROUTE RESERVOIR command.

It should be noted that the Black Creek hydrologic model provided by the LSRCA does not include SWM controls (i.e. the lumped approach) and only the necessary pond inputs for the “post-to-pre” approach were input in the future conditions model.

4.3.6. Existing Conditions

The hydrologic model provided by LSRCA included the 12-hour SCS Type II storms for the Holland River and Maskinonge River subwatersheds and the 24-hour SCS Type II storms for the Black River subwatershed. The total rainfall for the subwatersheds during the 2-year to 100-year storm events are summarized below in **Table 4-6**, **Table 4-7**, and **Table 4-8**.

Table 4-6 – Total Rainfall in the Holland River Watershed

Event	12-Hour Holland River Design Storm Total Rainfall (mm)
2-year	42.00
5-year	54.40
10-year	62.70
25-year	73.10
50-year	80.80
100-year	88.50

Table 4-7 – Total Rainfall in the Black River Subwatershed

Event	24-Hour Black River Design Storm Total Rainfall (mm)
2-year	46.38
5-year	66.29
10-year	79.54
25-year	96.10
50-year	108.54
100-year	120.71

Table 4-8 – Total Rainfall in the Maskinonge River Subwatershed

Event	12-Hour Maskinonge River Design Storm Total Rainfall (mm)
2-year	42.00
5-year	54.40
10-year	62.70
25-year	73.10
50-year	80.80
100-year	88.50

An initial review of the existing conditions hydrologic model was conducted and no changes to the input parameters or storm files were made with the updates as a part of this study. The existing condition drainage area plan based on the Hydrologic and Hydraulic Modeling for the West Holland River, East Holland River and Maskinonge River Watersheds Report (CCL, 2005) and Black River Hydraulic Model Update (LSRCA, 2005) is below as **Figure 4-5**. The existing condition hydrologic input parameters sorted by subwatershed are provided in **Table 4-9**.

Table 4-9 – Existing Condition Hydrologic Input Parameters

Catchment	Subwatershed	Drainage Area (ha)	CN	TP (hr)	Comment
244	East Holland River	1908.71	61	2.200	Drains northwest to Holland River
236	East Holland River	494.49	54	1.236	Drains northwest to Holland River
238	East Holland River	311.70	53	1.563	Drains northwest to Holland River
232	East Holland River	314.80	58	0.997	Drains northwest to Holland River
216	East Holland River	145.77	75	0.522	Drains northwest to Holland River
240	East Holland River	434.37	46	3.603	Drains northwest to Holland River
242	East Holland River	657.88	52	5.374	Drains northwest to Holland River
228	East Holland River	310.54	61	1.116	Drains northwest to Holland River
226	East Holland River	237.58	75	1.053	Drains northwest to Holland River
220	East Holland River	161.10	73	1.501	Drains northwest to Holland River
230	East Holland River	78.80	69	N/A	Drains northwest to Holland River
234	East Holland River	267.16	39	N/A	Drains northwest to Holland River
218	East Holland River	152.25	69	0.793	Drains northwest to Holland River
210	East Holland River	218.27	80	1.079	Drains northwest to Holland River
214	East Holland River	316.95	62	N/A	Drains northwest to Holland River
224	East Holland River	140.45	79	N/A	Drains northwest to Holland River
222	East Holland River	219.50	79	N/A	Drains northwest to Holland River
208	East Holland River	692.30	70	N/A	Drains northwest to Holland River
212	East Holland River	328.40	69	N/A	Drains northwest to Holland River
342	West Holland River	1004.58	54	2.565	Drains northwest to Holland River

Catchment	Subwatershed	Drainage Area (ha)	CN	TP (hr)	Comment
422	West Holland River	780.20	54	1.948	Drains northwest to Holland River
416	West Holland River	439.30	64	1.286	Drains northwest to Holland River
418	West Holland River	182.79	64	1.052	Drains northwest to Holland River
414	West Holland River	188.00	64	N/A	Drains northwest to Holland River
1	Maskinonge River	1120.6	62.5	2.244	Drains northeast to Maskinonge River
3	Black River	2954.54	64	13.100	Drains northeast to Black River Subwatershed
4	Black River	2672.69	53	10.700	Drains northeast to Black River Subwatershed
6	Black River	1504.44	73	12.200	Drains northeast to Black River Subwatershed
7	Black River	1373.84	73	10.700	Drains northeast to Black River Subwatershed

Note: A CN of N/A indicates a STANDHYD command

The existing conditions model and design storms (provided by LSRCA) were modeled to analyze the peak flows from the 2 to 100 year events. Existing conditions peak flow for each subcatchment area are summarized in **Table 4-10**. Detailed model output sorted by subwatershed is provided in **Appendix C**.

Table 4-10 – Existing Condition Peak Flows

Subcatchment Area (Comparison Point ID)	Subwatershed	Peak Flow (m ³ /s)		
		2-year	5-year	100-year
Total Flow in Subcatchment Area 244 (9244)	East Holland River	3.274	5.505	13.625
Total Flow in Subcatchment Area 236 (9236)	East Holland River	1.022	1.753	4.497
Total Flow in Subcatchment Area 238 (9238)	East Holland River	0.520	0.893	2.301
Total Flow in Subcatchment Area 232 (9232)	East Holland River	0.870	1.488	3.790
Total Flow in Subcatchment Area 216 (9216)	East Holland River	1.235	2.043	4.797
Total Flow in Subcatchment Area 240 (9252)	East Holland River	0.304	0.523	1.357
Total Flow in Subcatchment Area 242 (9242)	East Holland River	0.402	0.685	1.747
Total Flow in Subcatchment Area 228 (9228)	East Holland River	0.879	1.493	3.743
Total Flow in Subcatchment Area 226 (9226)	East Holland River	1.169	1.920	4.479
Total Flow in Subcatchment Area 220 (9220)	East Holland River	0.592	0.975	2.291
Total Flow in Subcatchment Area 230 (8172)	East Holland River	2.277	5.478	14.897
Total Flow in Subcatchment Area 234 (9234)	East Holland River	2.402	3.511	7.740
Total Flow in Subcatchment Area 218 (9218)	East Holland River	0.743	1.247	3.023
Total Flow in Subcatchment Area 210 (9210)	East Holland River	1.289	2.078	4.665
Total Flow in Subcatchment Area 214 (9214)	East Holland River	3.693	6.812	20.817
Total Flow in Subcatchment Area 224 (9224)	East Holland River	4.872	6.985	16.438
Total Flow in Subcatchment Area 222 (8164)	East Holland River	5.052	7.127	13.404
Total Flow in Subcatchment Area 208 (8148)	East Holland River	16.017	23.444	54.869
Total Flow in Subcatchment Area 212 (8152)	East Holland River	7.249	10.779	27.704
Total Flow in Subcatchment Area 342 (9342)	West Holland River	1.208	2.057	5.230
Total Flow in Subcatchment Area 422 (9422)	West Holland River	1.144	1.954	4.992
Total Flow in Subcatchment Area 416 (9416)	West Holland River	1.251	2.108	5.193
Total Flow in Subcatchment Area 418 (9423)	West Holland River	0.600	1.014	2.512
Total Flow in Subcatchment Area 414 (8254)	West Holland River	2.277	3.718	11.915
Total Flow in Subcatchment Area 1 (8501)	Maskinonge River	4.905	8.001	19.073
Total Flow in Subcatchment Area 3 (515)	Black River	3.641	6.991	19.085
Total Flow in Subcatchment Area 4 (516)	Black River	2.718	5.388	15.685
Total Flow in Subcatchment Area 6 (518)	Black River	2.663	4.942	12.668
Total Flow in Subcatchment Area 7 (524)	Black River	2.719	5.050	12.958

Note: It is noted that peak flows are not directly additive due to varied time to peak.

4.3.7. Future Conditions

Future Conditions Drainage Characteristics

As stated in **Section 4.2**, the existing and future land uses within the Town are classified into a number of categories and mainly include agriculture, open water / wetlands, forest, open space, medium density residential, low density residential, estate lands, institutional, and commercial. To remain consistent with the original hydrologic model, the average imperviousness and directly connected imperviousness associated with each land use class were adopted from the Hydrologic and Hydraulic Modeling for the West Holland River, East Holland River and Maskinonge River Watersheds Report (CCL, 2005). The average imperviousness values are summarized below in **Table 4-11**.

Table 4-11 – Land Use Classes and Imperviousness

Land Use Class	Avg. Imperviousness	Directly Connected Imperviousness
High Density Residential	55%	25%
Medium Density Residential	45%	15%
Low Density Residential	30%	10%
Estate Residential	15%	5%
Commercial / Industrial / Institutional	75%	70%
Rural Developments	65%	25%
General Urban Areas	45%	15%
Open Space / Recreational	0%	0%
Pasture / Agricultural / Woodlots / Forest	0%	0%
Wetland / Meadows	0%	0%

Note: As per Table 1. Land Use Classes and their Associated Average (CCL, 2005)

It is noted that for consistency purposes, the average imperviousness values were taken from the Hydrologic and Hydraulic Modeling for the West Holland River, East Holland River and Maskinonge River Watersheds Report (CCL, 2005). The average imperviousness utilized from this study may be lower than representative of today's standards and the Town should investigate more recent imperviousness values from neighbouring municipalities when outlining the stormwater management standards. Detailed investigation of appropriate imperviousness values within the Town was not undertaken as a part of this study.

Similarly for rural areas, the same methodology for CN calculations were utilized for the revisions to the future conditions. The CN values associated with land use covers and hydrologic soil groups are provided in **Table 4-12**.

Table 4-12 – SCS Curve Number (CN II) for Land Use Classes and Hydrologic Soil Groups

Land Use Class	Hydrologic Soil Group			
	A Soils	B Soils	C Soils	D Soils
Forest / Woodlots	30	58	71	77
Agricultural / Crops	62	74	82	86
Pasture / Meadows	38	65	76	81
Wetland / Open Water	50	50	50	50
Lawn / Open Space	49	69	79	84
Impervious Area	98	98	98	98

Note: As per Table 2. SCS Curve Number (CNII) for Different Hydrologic Soil Groups and Landuse Covers (CCL, 2005)

The time to peak input parameter was calculated based on the methodology from the existing hydrological reports as well. An average of the results of five (5) times to peak (T_p) equations was utilized in conjunction with the subcatchment parameters (i.e. slope, length, width etc.). The equations consist of the HYMO two-parameter, HYMO three-parameter, Kirpich, Williams & Hann, and Bransby Williams and are described below (CCL, 2005):

HYMO two-parameter: $t_p = 0.016 * A^{0.31} * S^{-0.5}$

HYMO three-parameter: $t_p = 0.0086 * A^{0.422} * S^{-0.46} * (L/W)^{0.133}$

Kirpich Method: $t_p = 0.67 * 0.0195 * L^{0.77} * S^{-0.385} / 60$

Williams and Hann Method: $t_p = 6.54 * A^{0.39} * (S * 100)^{-0.50} / 60$

Bransby-Williams Method: $t_p = 0.67 * 0.057 * L / (S^{-0.50} * A^{0.10})$

Where:

- t_p is the time to peak (hour);
- A is the drainage area (ha);
- S is the slope (m/m);
- L is the watershed length (m); and,
- W is the watershed width (m).

Detailed CN, Total Imperviousness (TIMP), Directly Connected Imperviousness (XIMP), and time to peak calculations are provided in **Appendix D**. The future conditions drainage area plan and associated external areas are illustrated below in **Figure 4-6**.

As summarized in **Table 4-1** and **Table 4-2** in **Section 4.3.4**, the storage-discharge relationships for each ROUTE RESERVOIR input for the future conditions model was calculated based on unit equations. The unit flow and unit storage calculations for each subcatchment area are provided in **Appendix E**.

It is noted that as stated in the Hydrology Report (CCL, 2005), the input parameters Initial Abstraction (Ia) and number of linear reservoirs (N) has remained the same during the future conditions due to the model sensitivity of these parameters in the calibrated model.

Future Conditions Flow Summary

To evaluate the impacts of the future land use, a comparison of peak flows was conducted as per Policy 4.5 SA of the LSPP. The future peak flow condition results with no mitigation, with post-to-pre quantity control, and over-control are summarized below in **Table 4-13** to **Table 4-41**. Future conditions detailed model output for the 2 to 100-year storm events are provided in **Appendix F**.

Over-Control

As stated in **Section 4.3.4 1.b)**, several future drainage areas are subject to over control (beyond the “post-to-pre” approach) in order to meet pre-development peak flow rates. Over-control involves revising the controlling stage-storage-discharge relationships by applying additional storage or reducing flows for all storm events. Drainage areas subject to the required stage-storage-discharge relationship revisions are given a unit-flow rate ($\text{m}^3/\text{s}/\text{ha}$) criteria (for future developed area only) as indicated below summary tables **Table 4-13** to **Table 4-41** and given a notation of ¹.

Table 4-13– Post-Development Peak Flow for Subcatchment Area 244 (Comparison Point 9244)

1	2	3	4	5	6	7 ¹	8 = 4–7	9 = (4–7)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Qpost (over-control)	Diff.	% Diff.
	year	mm	m^3/s	m^3/s	m^3/s	m^3/s	m^3/s	
East Holland River (Area: 1908.71ha)	2	42	3.274	6.470	3.379	3.154	-0.120	-3.7%
	5	54	5.505	9.779	5.731	5.319	-0.186	-3.4%
	10	63	7.239	14.265	8.399	6.991	-0.248	-3.4%
	25	73	9.643	18.761	12.314	9.301	-0.342	-3.5%
	50	81	11.577	22.364	14.947	11.148	-0.429	-3.7%
	100	89	13.625	26.176	17.713	13.380	-0.245	-1.8%

¹ Over-control approach utilized. Refer to **Table 4-42** for required unit-flow rate.

Table 4-14 – Post-Development Peak Flow for Subcatchment Area 236 (Comparison Point 9236)

1	2	3	4	5	6	7 ¹	8 = 4–7	9 = (4–7)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Qpost (over-control)	Diff.	% Diff.
	year	mm	m^3/s	m^3/s	m^3/s	m^3/s	m^3/s	
East Holland River (Area: 494.5 ha)	2	42	1.022	6.248	1.452	0.886	-0.136	-15.3%
	5	54	1.753	11.729	3.558	1.748	-0.005	-0.3%
	10	63	2.329	15.238	5.336	2.137	-0.192	-9.0%
	25	73	3.138	21.179	7.563	3.117	-0.021	-0.7%
	50	81	3.795	28.894	9.138	3.778	-0.017	-0.4%

1	2	3	4	5	6	7 ¹	8 = 4-7	9 = (4-7)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Qpost (over-control)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
	100	89	4.497	34.062	10.731	4.297	-0.200	-4.7%

¹ Over-control approach utilized. Refer to **Table 4-42** for required unit-flow rate.

Table 4-15 – Post-Development Peak Flow for Subcatchment Area 238 (Comparison Point 9238)

1	2	3	4	5	6	7 = 4-6	8 = (4-6)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
East Holland River (Area: 311.70ha)	2	42	0.520	0.660	0.429	-0.091	-17.5%
	5	54	0.893	1.121	0.734	-0.159	-17.8%
	10	63	1.188	1.483	0.974	-0.214	-18.0%
	25	73	1.603	1.989	1.310	-0.293	-18.3%
	50	81	1.940	2.400	1.611	-0.329	-17.0%
	100	89	2.301	2.840	2.274	-0.027	-1.2%

Table 4-16 – Post-Development Peak Flow for Subcatchment Area 232 (Comparison Point 9232)

1	2	3	4	5	6	7 ¹	8 = 4-7	9 = (4-7)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Qpost (over-control)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
East Holland River (Area: 314.80ha)	2	42	0.870	2.994	0.433	0.662	-0.208	-31.4%
	5	54	1.488	5.082	0.856	1.449	-0.039	-2.7%
	10	63	1.972	6.731	1.219	1.965	-0.007	-0.4%
	25	73	2.653	10.545	3.410	2.602	-0.051	-2.0%
	50	81	3.203	12.714	4.930	3.123	-0.080	-2.6%
	100	89	3.790	15.023	6.075	3.784	-0.006	-0.2%

¹ Over-control approach utilized. Refer to **Table 4-42** for Unit-Flow rates.

Table 4-17 – Post-Development Peak Flow for Subcatchment Area 216 (Comparison Point 9216)

1	2	3	4	5	6	7 ¹	8 = 4-7	9 = (4-7)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Qpost (over-control)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
East Holland River (Area: 145.77ha)	2	42	1.235	2.832	1.140	1.046	-0.189	-18.1%
	5	54	2.043	5.006	1.973	1.777	-0.266	-15.0%
	10	63	2.652	7.338	2.965	2.550	-0.102	-4.0%
	25	73	3.476	9.924	4.217	3.449	-0.027	-0.8%
	50	81	4.123	11.838	5.085	4.084	-0.039	-1.0%
	100	89	4.797	13.845	6.021	4.562	-0.235	-5.2%

¹ Over-control approach utilized. Refer to **Table 4-42** for required unit-flow rate.

Table 4-18 – Post-Development Peak Flow for Subcatchment Area 240 (Comparison Point 9252)

1	2	3	4	5	6	7 ¹	8 = 4-7	9 = (4-7)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Qpost (over-control)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
East Holland River (Area: 434.37ha)	2	42	0.304	1.297	0.476	0.298	-0.006	-2.0%
	5	54	0.523	2.242	1.386	0.503	-0.020	-3.8%
	10	63	0.696	2.996	2.182	0.665	-0.031	-4.5%
	25	73	0.942	4.763	2.822	0.892	-0.050	-5.3%
	50	81	1.142	5.769	3.377	1.076	-0.066	-5.8%
	100	89	1.357	6.846	4.052	1.273	-0.084	-6.2%

¹ Over-control approach utilized. Refer to **Table 4-42** for required unit-flow rate.

Table 4-19 – Post-Development Peak Flow for Subcatchment Area 242 (Comparison Point 9242)

1	2	3	4	5	6	7 ¹	8 = 4-7	9 = (4-7)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Qpost (over-control)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
East Holland River (Area: 145.77ha)	2	42	0.402	0.396	0.405	0.393	-0.009	-2.2%
	5	54	0.685	0.676	0.688	0.671	-0.014	-2.0%
	10	63	0.908	0.896	0.908	0.889	-0.019	-2.1%

1	2	3	4	5	6	7 ¹	8 = 4-7	9 = (4-7)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Qpost (over-control)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
	25	73	1.221	1.205	1.216	1.197	-0.024	-2.0%
	50	81	1.476	1.456	1.467	1.446	-0.030	-2.0%
	100	89	1.747	1.724	1.734	1.713	-0.034	-1.9%

¹ Over-control approach utilized. Refer to **Table 4-42** for required unit-flow rate.

Table 4-20 – Post-Development Peak Flow for Subcatchment Area 228 (Comparison Point 9228)

1	2	3	4	5	6	7 ¹	8 = 4-7	9 = (4-7)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Qpost (over-control)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
East Holland River (Area: 310.54ha)	2	42	0.879	2.179	0.580	0.727	-0.152	-20.9%
	5	54	1.493	3.478	1.034	1.476	-0.017	-1.2%
	10	63	1.970	4.607	1.425	1.906	-0.064	-3.4%
	25	73	2.635	6.205	2.921	2.492	-0.143	-5.7%
	50	81	3.173	8.593	4.216	3.011	-0.162	-5.4%
	100	89	3.743	10.169	5.130	3.655	-0.088	-2.4%

¹ Over-control approach utilized. Refer to **Table 4-42** for required unit-flow rate.

Table 4-21– Post-Development Peak Flow for Subcatchment Area 226 (Comparison Point 9226)

1	2	3	4	5	6	7 ¹	8 = 4-7	9 = (4-7)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Qpost (over-control)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
East Holland River (Area: 237.6ha)	2	42	1.169	10.086	1.917	1.097	-0.072	-6.6%
	5	54	1.920	14.314	4.153	1.719	-0.201	-11.7%
	10	63	2.484	17.117	6.279	2.200	-0.284	-12.9%
	25	73	3.250	20.742	9.111	2.854	-0.396	-13.9%
	50	81	3.852	23.495	10.908	3.434	-0.418	-12.2%
	100	89	4.479	26.297	14.385	4.024	-0.455	-11.3%

¹ Over-control approach utilized. Refer to **Table 4-42** for required unit-flow rate.

Table 4-22 – Post-Development Peak Flow for Subcatchment Area 220 (Comparison Point 9220)

1	2	3	4	5	6	7 ¹	8 = 4-7	9 = (4-7)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Qpost (over-control)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
East Holland River (Area: 161.10ha)	2	42	0.592	2.650	0.611	0.547	0.045	-8.2%
	5	54	0.975	4.574	1.731	0.943	0.032	-3.4%
	10	63	1.264	5.676	2.361	1.230	0.034	-2.8%
	25	73	1.657	7.142	3.202	1.613	0.044	-2.7%
	50	81	1.967	8.277	3.785	1.917	0.05	-2.6%
	100	89	2.291	9.448	4.483	2.218	0.073	-3.3%

¹ Over-control approach utilized. Refer to **Table 4-42** for required unit-flow rate.

Table 4-23 – Post-Development Peak Flow for Subcatchment Area 230 (Comparison Point 8172)

1	2	3	4	5	6	7 = 4-6	8 = (4-6)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
East Holland River (Area: 78.80ha)	2	42	2.277	3.989	0.610	-1.667	-73.2%
	5	54	5.478	6.513	1.454	-4.024	-73.5%
	10	63	7.402	10.349	3.372	-4.030	-54.4%
	25	73	10.731	13.748	4.907	-5.824	-54.3%
	50	81	12.808	16.386	5.904	-6.904	-53.9%
	100	89	14.897	19.155	6.947	-7.950	-53.4%

Table 4-24 – Post-Development Peak Flow for Subcatchment Area 234 (Comparison Point 9234)

1	2	3	4	5	6	7 = 4-6	8 = (4-6)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
East Holland River (Area: 161.10ha)	2	42	2.402	0.412	0.273	-2.129	-88.6%
	5	54	3.511	0.701	0.465	-3.046	-86.8%
	10	63	4.606	0.928	0.613	-3.993	-86.7%

1	2	3	4	5	6	7 = 4-6	8 = (4-6)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
	25	73	5.792	1.245	0.828	-4.964	-85.7%
	50	81	6.738	1.502	1.366	-5.372	-79.7%
	100	89	7.740	1.776	1.765	-5.975	-77.2%

Table 4-25 – Post-Development Peak Flow for Subcatchment Area 218 (Comparison Point 9218)

1	2	3	4	5	6	7 ¹	8 = 4-7	9 = (4-7)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Qpost (over-control)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
East Holland River (Area: 152.25ha)	2	42	0.743	2.048	0.436	0.514	-0.229	-44.6%
	5	54	1.247	3.870	0.813	1.223	-0.024	-2.0%
	10	63	1.632	5.027	1.988	1.429	-0.203	-14.2%
	25	73	2.161	6.595	2.799	1.735	-0.426	-24.6%
	50	81	2.582	7.825	3.426	1.990	-0.592	-29.7%
	100	89	3.023	9.104	4.065	2.262	-0.761	-33.6%

¹ Over-control approach utilized. Refer to **Table 4-42** for required unit-flow rate.

Table 4-26 – Post-Development Peak Flow for Subcatchment Area 210 (Comparison Point 9210)

1	2	3	4	5	6	7 ¹	8 = 4-7	9 = (4-7)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Qpost (over-control)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
East Holland River (Area: 218.27ha)	2	42	1.289	16.874	8.021	1.066	-0.223	-20.9%
	5	54	2.078	23.724	9.711	1.778	-0.300	-16.9%
	10	63	2.659	28.316	11.387	2.313	-0.346	-15.0%
	25	73	3.436	34.230	13.600	3.322	-0.114	-3.4%
	50	81	4.041	38.702	15.259	3.875	-0.166	-4.3%
	100	89	4.665	43.242	16.942	4.443	-0.222	-5.0%

¹ Over-control approach utilized. Refer to **Table 4-42** for required unit-flow rate.

Table 4-27 – Post-Development Peak Flow for Subcatchment Area 214 (Comparison Point 9214)

1	2	3	4	5	6	7 = 4-6	8 = (4-6)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
West Holland River (Area: 316.95ha)	2	42	3.693	11.137	1.473	-2.220	-60.1%
	5	54	6.812	15.749	4.012	-2.800	-41.1%
	10	63	9.005	20.665	8.476	-0.529	-5.9%
	25	73	12.628	25.985	12.150	-0.478	-3.8%
	50	81	17.592	30.405	13.920	-3.672	-20.9%
	100	89	20.817	34.675	15.918	-4.889	-23.5%

Table 4-28 – Post-Development Peak Flow for Subcatchment Area 224 (Comparison Point 9224)

1	2	3	4	5	6	7 = 4-6	8 = (4-6)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
East Holland River (Area: 140.45ha)	2	42	4.872	8.951	2.122	-2.750	-56.4%
	5	54	6.985	13.076	4.116	-2.869	-41.1%
	10	63	9.955	15.776	6.595	-3.360	-33.8%
	25	73	12.468	19.191	8.011	-4.457	-35.7%
	50	81	14.420	21.793	9.852	-4.568	-31.7%
	100	89	16.438	24.448	12.139	-4.299	-26.2%

Table 4-29 – Post-Development Peak Flow for Subcatchment Area 222 (Comparison Point 8164)

1	2	3	4	5	6	7 = 4-6	8 = (4-6)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
East Holland River (Area: 219.50ha)	2	42	5.052	11.876	4.972	-0.080	-1.6%
	5	54	7.127	17.696	6.960	-0.167	-2.3%
	10	63	8.611	21.382	8.341	-0.270	-3.1%
	25	73	10.506	26.191	10.118	-0.388	-3.7%
	50	81	11.943	29.866	11.458	-0.485	-4.1%

1	2	3	4	5	6	7 = 4-6	8 = (4-6)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
	100	89	13.404	33.624	12.816	-0.588	-4.4%

Table 4-30 – Post-Development Peak Flow for Subcatchment Area 208 (Comparison Point 8148)

1	2	3	4	5	6	7 ¹	8 = 4-7	9 = (4-7)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Qpost (over-control)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
East Holland River (Area: 692.30ha)	2	42	16.017	22.060	16.040	15.899	-0.118	-0.7%
	5	54	23.444	32.387	23.473	23.258	-0.186	-0.8%
	10	63	29.923	49.068	30.100	29.922	-0.001	-0.003%
	25	73	41.282	64.924	41.399	41.170	-0.112	-0.3%
	50	81	47.476	74.637	47.602	47.333	-0.143	-0.3%
	100	89	54.869	84.666	54.001	53.690	-1.179	-2.1%

¹ Over-control approach utilized. Refer to **Table 4-42** for required unit-flow rate.

Table 4-31 – Post-Development Peak Flow for Subcatchment Area 212 (Comparison Point 8152)

1	2	3	4	5	6	7 = 4-6	8 = (4-6)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
East Holland River (Area: 328.40ha)	2	42	7.249	13.305	2.884	-4.365	-60.2%
	5	54	10.779	19.096	7.329	-3.45	-32.0%
	10	63	13.245	22.985	9.497	-3.748	-28.3%
	25	73	17.277	30.778	13.378	-3.899	-22.6%
	50	81	23.418	35.302	15.960	-7.458	-31.8%
	100	89	27.704	39.976	18.522	-9.182	-33.1%

Table 4-32 – Post-Development Peak Flow for Subcatchment Area 342 (Comparison Point 9342)

1	2	3	4	5	6	7 ¹	8 = 4-7	9 = (4-7)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Qpost (over-control)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
West Holland River (Area: 1004.58ha)	2	42	1.208	9.517	1.748	1.136	-0.072	-6.0%
	5	54	2.057	13.498	4.136	1.986	-0.071	-3.5%
	10	63	2.724	16.363	5.408	2.649	-0.075	-2.8%
	25	73	3.660	22.261	7.536	3.570	-0.090	-2.5%
	50	81	4.420	25.680	9.111	4.304	-0.116	-2.6%
	100	89	5.230	29.235	10.777	5.080	-0.150	-2.9%

¹ Over-control approach utilized. Refer to **Table 4-42** for required unit-flow rate.

Table 4-33 – Post-Development Peak Flow for Subcatchment Area 422 (Comparison Point 9422)

1	2	3	4	5	6	7 ¹	8 = 4-7	9 = (4-7)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Qpost (over-control)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
West Holland River (Area: 780.20ha)	2	42	1.144	1.746	1.23	1.070	-0.074	-6.5%
	5	54	1.954	2.717	2.093	1.829	-0.125	-6.4%
	10	63	2.591	3.408	2.791	2.438	-0.153	-5.9%
	25	73	3.488	4.492	3.763	3.292	-0.196	-5.6%
	50	81	4.215	5.652	4.836	3.986	-0.229	-5.4%
	100	89	4.992	6.698	6.032	4.726	-0.266	-5.3%

¹ Over-control approach utilized. Refer to **Table 4-42** for required unit-flow rate.

Table 4-34 – Post-Development Peak Flow for Subcatchment Area 416 (Comparison Point 9416)

1	2	3	4	5	6	7 ¹	8 = 4-7	9 = (4-7)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Qpost (over-control)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
West Holland River (Area:	2	42	1.251	1.600	0.913	1.005	-0.246	-24.5%
	5	54	2.108	2.674	2.243	1.882	-0.226	-12.0%
	10	63	2.770	3.500	2.980	2.523	-0.247	-9.8%

1	2	3	4	5	6	7 ¹	8 = 4-7	9 = (4-7)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Qpost (over-control)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
	25	73	3.686	4.634	4.048	3.388	-0.298	-8.8%
	50	81	4.419	5.537	4.816	4.090	-0.329	-8.0%
	100	89	5.193	6.486	5.810	4.758	-0.435	-9.1%

¹ Over-control approach utilized. Refer to **Table 4-42** for required unit-flow rate.

Table 4-35 – Post-Development Peak Flow for Subcatchment Area 418 (Comparison Point 9423)

1	2	3	4	5	6	7 ¹	8 = 4-7	9 = (4-7)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Qpost (over-control)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
West Holland River (Area: 182.79ha)	2	42	0.600	0.643	0.618	0.574	-0.026	-4.3%
	5	54	1.014	1.082	1.114	0.970	-0.044	-4.3%
	10	63	1.335	1.421	1.467	1.277	-0.058	-4.3%
	25	73	1.780	1.904	1.954	1.702	-0.078	-4.4%
	50	81	2.136	2.291	2.348	2.042	-0.094	-4.4%
	100	89	2.512	2.702	2.757	2.401	-0.111	-4.4%

¹ Over-control approach utilized. Refer to **Table 4-42** for required unit-flow rate.

Table 4-36 – Post-Development Peak Flow for Subcatchment Area 414 (Comparison Point 8254)

1	2	3	4	5	6	7 = 4-6	8 = (4-6)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
West Holland River (Area: 188.00ha)	2	42	2.277	5.638	1.301	-0.976	-42.9%
	5	54	3.718	9.550	3.037	-0.681	-18.3%
	10	63	5.494	11.831	3.960	-1.534	-27.9%
	25	73	7.551	14.891	5.059	-2.492	-33.0%
	50	81	10.313	17.405	5.696	-4.617	-44.8%
	100	89	11.915	19.912	6.329	-5.586	-46.9%

Table 4-37 – Post-Development Peak Flow for Subcatchment Area 1 (Comparison Point 8501)

1	2	3	4	5	6	7 = 4-6	8 = (4-6)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
Maskinonge River (Area: 1120.60ha)	2	42	4.905	20.072	3.162	-1.743	-35.5%
	5	54	8.001	31.066	5.451	-2.550	-31.9%
	10	63	10.376	37.993	7.901	-2.475	-23.9%
	25	73	13.665	47.202	11.383	-2.282	-16.7%
	50	81	16.295	54.353	13.894	-2.401	-14.7%
	100	89	19.073	61.757	16.323	-2.750	-14.4%

Table 4-38 – Post-Development Peak Flow for Subcatchment Area 3 (Comparison Point 515)

1	2	3	4	5	6	7 ¹	8 = 4-7	9 = (4-7)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Qpost (over-control)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
Black River (Area: 2954.54ha)	2	51	3.641	3.406	3.663	3.636	-0.005	-0.14%
	5	73	6.991	6.512	6.780	6.916	-0.075	-1.1%
	10	87	9.595	8.923	9.190	9.457	-0.138	-1.4%
	25	105	13.206	12.264	12.519	12.774	-0.432	-3.3%
	50	118	16.096	14.936	15.178	15.406	-0.690	-4.3%
	100	131	19.085	17.692	17.922	18.126	-0.959	-5.0%

¹ Over-control approach utilized. Refer to **Table 4-42** for required unit-flow rate.

Table 4-39 – Post-Development Peak Flow for Subcatchment Area 4 (Comparison Point 516)

1	2	3	4	5	6	7 ¹	8 = 4-7	9 = (4-7)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Qpost (over-control)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
Black River (Area: 2672.7ha)	2	51	2.718	2.666	2.811	2.717	-0.001	-0.04%
	5	73	5.388	5.267	5.415	5.348	-0.040	-0.74%
	10	87	7.532	7.353	7.498	7.456	-0.076	-1.0%

1	2	3	4	5	6	7 ¹	8 = 4-7	9 = (4-7)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Qpost (over-control)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
	25	105	10.578	10.316	10.454	10.448	-0.130	-1.2%
	50	118	13.066	12.736	12.866	12.890	-0.176	-1.3%
	100	131	15.685	15.280	15.402	15.442	-0.243	-1.5%

¹ Over-control approach utilized. Refer to **Table 4-42** for required unit-flow rate.

Table 4-40 – Post-Development Peak Flow for Subcatchment Area 6 (Comparison Point 518)

1	2	3	4	5	6	7 ¹	8 = 4-7	9 = (4-7)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Qpost (over-control)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
Black River (Area: 1504.44ha)	2	51	2.663	2.626	2.680	2.650	-0.013	-0.5%
	5	73	4.942	4.865	4.918	4.902	-0.040	-0.8%
	10	87	6.658	6.550	6.601	6.595	-0.063	-0.9%
	25	105	8.982	8.833	8.879	8.888	-0.094	-1.0%
	50	118	10.806	10.624	10.667	10.680	-0.126	-1.2%
	100	131	12.668	12.451	12.491	12.502	-0.166	-1.3%

¹ Over-control approach utilized. Refer to **Table 4-42** for required unit-flow rate.

Table 4-41 – Post-Development Peak Flow for Subcatchment Area 7 (Comparison Point 524)

1	2	3	4	5	6	7 ¹	8 = 4-7	9 = (4-7)/4
Subwatershed	Storm Event	Rainfall	Qpre	Qpost (no mitigation)	Qpost (post-to-pre approach)	Qpost (over-control)	Diff.	% Diff.
	year	mm	m ³ /s	m ³ /s	m ³ /s	m ³ /s	m ³ /s	
Black River (Area: 1373.84ha)	2	51	2.719	5.793	4.086	2.377	-0.342	-12.6%
	5	73	5.050	11.758	10.036	4.256	-0.794	-15.7%
	10	87	6.806	15.357	13.920	5.660	-1.146	-16.8%
	25	105	9.183	19.994	17.831	7.461	-1.722	-18.7%
	50	118	11.051	23.670	20.998	8.783	-2.268	-20.5%
	100	131	12.958	27.671	24.534	11.309	-1.649	-12.7%

¹ Over-control approach utilized. Refer to **Table 4-42** for required unit-flow rate.

It should be noted the total rainfall values used in the above quantity control analysis are taken from the original CCL hydrologic model (2005). Developed runoff volumes and peak flows in this SWMMP are based on this older data and as such should be used for reference only. Actual hydrologic data to be used during detailed design of future SWM pond are to be based on the most recent rainfall data and various storm durations in use at the time each future SWM facility is designed.

Results

Table 4-13 to **Table 4-41** above indicate the results of the hydrologic modelling for the future condition land use within the Town. For comparison purposes, columns were added to indicate the “no mitigation” scenario, “post-to-pre” approach and “over-control” approach results. It is noted that the future return peak flows at many comparison points are higher than the existing return period peak flows with the “post-to-pre” peak flow approach applied. This indicates that the “post-to-pre” peak flow control criterion alone may not be enough to limit the future return period peak flows to / below their existing levels (as seen in areas within the East Holland and West Holland River subwatersheds). As a result, the “over-control” approach was applied to comparison points unable to meet pre-development flow rates as indicated below each corresponding summary table. In addition, it should also be noted smaller storm events may reflect a larger difference in flows from the pre-development level and can be adjusted when controls are implemented during the functional stages of development. **Table 4-42** outlines the required unit-flow rates to be applied in each subcatchment area to reduce post-development peak flows to pre-development levels. Calculations of the unit-flow rates are provided in **Appendix F**.

Table 4-42 – Unit-Flow-Rate Summary

Subwatershed	Settlement Area	Sub. Area	Comp. Pt.	Prop. Dev. Area	Unit-Flow Rates (m ³ /s/ha)					
					Storm Event					
					2YR	5YR	10YR	25YR	50YR	100YR
East Holland River	Queensville	244	9244	90.5	0.035	0.059	0.077	0.103	0.123	0.148
East Holland River	Queensville	236	9236	301.2	0.061	0.120	0.146	0.213	0.259	0.294
East Holland River	Holland Landing	232	9232	86.7	0.008	0.017	0.023	0.030	0.036	0.044
East Holland River	Sharon	216	9216	34.0	0.031	0.052	0.075	0.101	0.120	0.134
East Holland River	Holland Landing	240	9252	10.1	0.030	0.050	0.066	0.088	0.107	0.126
East Holland River	Holland Landing	242	9242	15.5	0.025	0.043	0.057	0.077	0.093	0.111
East Holland River	Holland Landing	228	9228	43.6	0.017	0.034	0.044	0.057	0.069	0.084
East Holland River	Green Lane	226	9226	116.5	0.009	0.015	0.019	0.024	0.029	0.035
East Holland River	Green Lane	220	9220	27.1	0.020	0.035	0.045	0.060	0.071	0.082
East Holland River	Sharon	218	9218	42.1	0.012	0.029	0.034	0.041	0.047	0.054
East Holland River	Green Lane	210	9210	176.4	0.006	0.010	0.013	0.019	0.022	0.025
East Holland River	Green Lane	208	8148	54.4	0.292	0.428	0.550	0.757	0.870	0.987
West Holland River	Holland Landing	342	9342	50.4	0.023	0.039	0.053	0.071	0.085	0.101
West Holland River	Holland Landing	422	9422	52.3	0.020	0.035	0.047	0.063	0.076	0.090
West Holland River	Green Lane	416	9416	20.4	0.049	0.092	0.124	0.166	0.200	0.233
West Holland River	Green Lane	418	9423	8.7	0.066	0.111	0.147	0.196	0.235	0.276
Black River	Mount Albert	3	515	98.4	0.037	0.070	0.096	0.130	0.157	0.184
Black River	Mount Albert	4	516	22.6	0.120	0.237	0.330	0.462	0.570	0.683
Black River	Green Lane	6	518	32.3	0.082	0.152	0.204	0.275	0.331	0.387
Black River	Sharon	7	524	270.4	0.009	0.016	0.021	0.028	0.032	0.042

Although the application of unit-flow rates will result in reducing peak flow rates at this level of study within the Town, it is desirable to develop a detailed subwatershed based unit flow control criterion, rather than applying the generalized unit flow equations for the entire watershed (CCL, 2005). It is noted that the hydrologic results are analogous to the results in the Hydrologic and Hydraulic Modeling for the West Holland River, East Holland River and Maskinonge River Watersheds Report (CCL, 2005) for the ‘post-to-pre’ condition and thus recommendations adopted for peak flow control on a subwatershed level. It is noted the unit-flow rates summarized in **Table 4-42** are only applicable to the implementation of the Town of East Gwillimbury Official Plan 2031 at this level of study and detailed stormwater management is required for each new development as outlined by MOE, LSPP and Town standards. In addition, it should be noted the over-control approach in which the unit-flow rates are to be applied, should to be further refined on a subwatershed basis through a detailed unit flow study.

Summary

As indicated above in **Table 4-13** to **Table 4-41**, at some comparison points, the future return period peak flows are relatively close to the existing peak flows (i.e. $\pm 5.0\%$), and it indicates that the “post-to-pre” peak flow criteria applied to the design of the future SWM facilities does help to control the flood peak flows. As a result, it is recommended that a “post-to-pre” approach be continued to be imposed or enhanced for new developments *and* a unit-storage / unit-flow criteria be applied *until* a detailed unit flow / unit storage criteria is investigated and implemented, specifically within the Black River and Maskinonge River where impacts to peak flows are not as severe. The implementation of unit storage / unit flow criteria can potentially reduce flows within the watershed, reduce erosion downstream and decrease flood potential. It is recommended that a detailed unit flow study is to be investigated (specifically within the Holland River) and implemented in the future.

It should be noted that future SWM facilities within the Town are required to undergo detailed design and control stormwater to guidelines as outlined by the province, MOE and LSRCA. The effects of the future land use on peak flows should be analyzed at a detailed level where the implementation of future SWM facilities in the Town will control the increase in peak flows to each representative subwatershed. In addition, the most current rainfall data and storm durations should be utilized for each future SWM facility design. Further information on recommended SWM practices and SWM criteria is detailed in **Section 12.1.4**.

It should also be noted that a detailed assessment of the hydraulic study was not conducted as a part of this Master Plan. In addition, detailed storm sewer analysis was not conducted. To ensure that future development will not have a negative impact on the Town’s storm sewer system, it is recommended that a storm sewer analysis (i.e. flow capacity, infiltration / inflow) be conducted in the future.

4.4. Phosphorus Loading

4.4.1. General

As per the Lake Simcoe Protection Plan's "Phosphorus Reduction Strategy" and detailed in Policy 4.5 SA of the Plan, an assessment of phosphorus loading from existing and planned development is required for all settlement areas within the Town boundary. The Phosphorus Reduction Strategy states that "in order to achieve the ambitious reductions in phosphorus loadings proposed in the Plan, there is a need to reduce loadings from all sources that contribute to excess phosphorus throughout the Lake Simcoe watershed" (LSRCA, 2009). The proposed reduction will be achieved through innovative solutions which will be implemented from the preferred alternative discussed herein. This assessment will evaluate the existing and future phosphorus loadings within the Town's settlement areas and recommend potential mitigation measures to be included in future development work.

4.4.2. Background

Information

Phosphorus is a key water quality concern in Lake Simcoe. While some phosphorus is required to support a healthy aquatic ecosystem, too much of this nutrient leads to excessive growth of plants. Generally, the more phosphorus present in the system, the number of plants and size of the plants will increase. As this biomass dies, decomposing organisms use up available oxygen. Low levels of dissolved oxygen at the bottom of the lake, especially during summer months, are harmful to fish populations and other aquatic organisms. Although reductions in phosphorus have led to improved oxygen conditions in the lake, the improvements are not yet sufficient for the fish to fully sustain themselves naturally. It is therefore an imperative goal for phosphorus loadings to Lake Simcoe to be reduced alongside development for the near future.

Documentation

The following phosphorus loading assessment is based on the detailed study results published by the LSRCA entitled:

- Report on the Phosphorus Loads to Lake Simcoe – LSRCA, 2004-2007, (September 2010).

4.4.3. Methodology

The objectives of the phosphorus loading assessment are summarized as follows:

- Utilize the base case and growth scenario unit area phosphorus loadings from the *Report on the Phosphorus Loads to Lake Simcoe – LSRCA, 2004-2007, (September 2010)* to generate existing and future phosphorus loading rates for each subwatershed;
- Apply the unit area phosphorus loading rate for each subwatershed to each applicable settlement area within the Town; and,
- Assess the changes in phosphorus loadings at each settlement area from existing to future conditions.

It is noted that atmospheric phosphorus (a large portion of the total phosphorus loading to Lake Simcoe) was not accounted for in this study. A phosphorus loading sample calculation is provided below:

Phosphorus Loading Sample Calculation

As outlined in **Section 4.4.3**, phosphorus loading rates were calculated based on the base case and growth scenario unit area phosphorus loadings from the *Report on the Phosphorus Loads to Lake Simcoe – LSRCA, 2004-2007, (September 2010)* to generate existing and future phosphorus loading rates for each subwatershed. Based on the existing and future land use of each settlement area, loading calculations were developed.

A sample calculation for Mount Albert in the Black River subwatershed for a specific land use is provided below. It should be noted the sample calculation is not intended to be exhaustive and should be read in conjunction with detailed phosphorus loading calculations provided in **Appendix G**.

Mount Albert

- *Subwatershed: Black River; Catchment: 3; Land Use: Agriculture; Soil Type: Type B and Type C*

Pre-Development

Existing Load = 0.129 kg/ha (Agriculture)

- For each soil type (Type B and Type C), the existing load is calculated:

Total Existing Load = (89.7 ha x 0.129 kg/ha) + (59.5 ha x 0.129 kg/ha) = 19.3 kg/year

- Similarly, the total existing load for each land use category is added for Catchment 3:

Total Existing Load = 60.9 kg/year

- The total existing load for each drainage area within the settlement area is added:

Total = **87.4 kg/year**

Post-Development

- Similar to pre-development, the revised loading rates are applied to each land use:

Future Load = 0.727 kg/ha (Low Density Residential)

- For each soil type (Type C and Type B, Bottomland and Type A), the future load is calculated:

Total Future Load = (62.2 ha + 84.8 ha + 4.4 ha + 21.1 ha) x 0.727 kg/ha = 125.3 kg/year

- The total future load for each land use category is added for Catchment 3:

Total Future Load = 166.6 kg/year

- The total future load for each drainage area within the settlement area is added:

Total = **226.9 kg/year**

- Based on the difference in loading rate divided by the total proposed future development area, the unit TP removal rate is determined:

Mount Albert Unit TP Removal Rate (kg/year/ha) = (226.9 kg/year – 87.4 kg/year) / 36.9 ha = **3.78 kg/year/ha**

4.4.4. Phosphorus Loading Assessment

As stated above in **Section 4.4.3**, phosphorus loading rates were generated based on the calibrated loadings in the Estimation of Phosphorus Loadings to Lake Simcoe Report (LSRCA, September 2010). Similar to Appendix E of the Estimation of Phosphorus Loadings to Lake Simcoe Report (LSRCA, September 2010), the land use classification system has been condensed to the land use categories in the Town described in the existing and future conditions land use in **Section 3.0**. The revised land use classification system and resulting phosphorus loading rates are summarized below in **Table 4-43**. Detailed phosphorus loading unit rate calculations are provided in **Appendix G**.

Table 4-43 – Land Use Categories and Average Annual Phosphorus Loading Rates

CANWET Land Use Category	Existing / Future Condition Land Use	Existing (Base Scenario) / Future (Growth Scenario) Average Annual Phosphorus Loading Rate (kg/ha)			
		Black River	East Holland River	Maskinonge River	West Holland River
Hay-Pasture	Agriculture	0.129 / 0.131	0.251 / 0.247	0.130 / 0.129	0.165 / 0.166
Cropland					
Quarry					
Turf-Sod					
Tile Drainage					
Forest	Forest	0.001 / 0.001	0.004 / 0.004	0.000 / 0.000	0.003 / 0.003
Wetland	Open Water / Wetland	0.139 / 0.137	0.342 / 0.350	0.672 / 0.6226	0.657 / 0.653
Stream Banks					
Groundwater					
Transition	Open Space	0.341 / 0.374	0.385 / 0.478	0.012 / 0.013	1.215 / 1.357
Septics					
Polder					
Unpaved Road	Rural Development, Estate	0.153 / 0.153	0.041 / 0.041	0.083 / 0.068	0.021 / 0.021
Low Intensity Development					
High Intensity Development	Residential, Low Density Residential, Medium Density Residential, Commercial / Institutional	0.393 / 0.727	0.660 / 0.968	0.339 / 0.753	0.246 / 0.451

It is noted that every land use category in the Estimation of Phosphorus Loadings to Lake Simcoe (LSRCA, September 2010) has been incorporated in the phosphorus loading estimate to provide a balanced phosphorus loading calculation.

4.4.5. Existing Conditions

The primary phosphorus loads are generated from the existing developed settlement areas within the Town: Mount Albert, Sharon, Queensville, Holland Landing and Green Lane West. The existing condition subwatershed phosphorus loading rates summarized below in **Table 4-44** were applied to each settlement area on a drainage area basis. The resulting existing condition total phosphorus loadings based on the base case scenario are also provided in the table below. Detailed existing condition phosphorus loading calculations are provided in **Appendix H**.

Table 4-44 – Existing Phosphorus Loadings

Settlement Area	Total Annual Phosphorus Loading (kg/year)
Mount Albert	87.4
Sharon	198.4
Queensville	198.5
Holland Landing	489.9
Green Lane West	192.4

4.4.6. Future Conditions

Future condition land use categories and areas were applied to the future condition phosphorus loading rates from the growth case scenario in the Estimation of Phosphorus Loadings to Lake Simcoe (LSRCA, September 2010). The resulting future condition total phosphorus loadings are provided below in **Table 4-45**. Detailed future condition phosphorus loading calculations are provided in **Appendix I**.

Table 4-45 – Future Phosphorus Loadings

Settlement Area	Total Annual Phosphorus Loading (kg/year)
Mount Albert	226.9
Sharon	338.0
Queensville	857.2
Holland Landing	865.0
Green Lane Expansion Area	811.9

4.4.7. Phosphorus Loading Assessment Results

A comparison of phosphorus loadings is required to assess the environmental impact of development in terms of phosphorus on Lake Simcoe. The phosphorus loading without mitigation assessment is provided in **Table 4-46**.

Table 4-46 – Phosphorus Loading Comparison

Settlement Area (Area)	Existing Total Annual Phosphorus Loading (kg/year)	Future Total Annual Phosphorus Loading (kg/year)	Difference / (% Difference) (kg/year)	Total Proposed Development Area (ha)	Unit TP Removal Rate Required for Mitigation (kg/year/ha)
Mount Albert (358 ha)	87.4	226.9	+139.5 / (+160%)	36.9	3.78
Sharon (468 ha)	198.4	338.0	+139.6 / (+70%)	211.8	0.66
Queensville (1182 ha)	198.5	857.2	+664.7 / (+335%)	54.9	12.1
Holland Landing (1160 ha)	489.9	865.0	+375.1 / (+77%)	37.7	9.9
Green Lane Expansion Area (871 ha)	192.4	811.9	+619.5 / (+322%)	69	8.9

As indicated above in **Table 4-46**, the largest increase in phosphorus loading is in Queensville and Green Lane West (+335% and +322% respectfully) with an increase overall in phosphorus loadings within the Town. In order to mitigate the impacts of development, unit total phosphorus removal rates, SWM controls and innovative water quality approaches are required to reduce phosphorus loadings. The current Total Phosphorus (TP) removal rates accepted by the Ministry of Environment (MOE) for wet ponds and wetlands are 63% and 77% respectively (MOE Phosphorus Budget Tool, January 2012). It should be noted that The *Estimation of Phosphorus Loadings to Lake Simcoe (LSRCA, September 2010)* Report has sufficient information regarding BMPs and phosphorus reduction strategies which may be implemented in the Town.

Phosphorus Mitigation Recommendations

The results of this assessment indicate development will increase phosphorus loadings within each settlement area. It is recommended to implement unit phosphorus removal rates in an integrated treatment train approach (i.e. lot level, conveyance and end-of-pipe controls) to mitigate the accumulation of phosphorus from proposed development (i.e. bioretention, infiltration trenches, permeable pavement, perforated pipe systems, etc.). Further information on potential mitigation measures (and BMP's) is provided in **Section 8.2**. It is noted that specific mitigation measures to be used to reduce phosphorus accumulation are to be confirmed on a site specific basis at the functional design stages of development.

4.5. Water Budget

As per Section 4.8-SA in the Lake Simcoe Protection Plan, a water budget assessment is required under the Lake Simcoe Protection Plan for the existing and proposed land use in the Town. To characterize the environmental impacts of the future conditions, a water budget consisting of the assessment of rainfall, runoff and evapotranspiration to determine infiltration deficit will be developed.

4.5.1. General

To conduct the water budget, each settlement area is considered separately. The settlement areas of Mount Albert, Queensville, Holland Landing, Sharon, and the Green Lane Employment Expansion Area were analyzed. Existing and future development conditions for each settlement area are compared and a volume deficit is determined. As required by the Lake Simcoe Protection Plan, a development must minimize changes in annual infiltration between pre-development and post-development conditions. The water balance analysis for the Town under existing and future conditions was completed on an average annual basis. The detailed water balance calculations are provided in **Appendix J** and summarized in **Table 4-47** to **Table 4-52**.

4.5.2. Background

A major consequence of urbanization is the increase of impervious areas, which changes the hydrologic characteristics within each settlement area and thus increases the amount of stormwater runoff. The Town is expected to expand rapidly over the next 20 years which will change the land use and decrease the amount of infiltration. Urbanization also decreases the amount of evapotranspiration due to the reduction of pervious area. This can potentially cause flooding, increases in velocity of flow during storms, and an increase in annual flow. A water budget assessment is necessary to analyze the potential impacts of development from existing to future conditions.

4.5.3. Water Budget Assessment

The Town consists of 25 different underlying soil types. These soils are categorized with the following textures: sandy loam, loam, sand, clay loam, silt loam, and muck.

A figure outlining the current soil conditions is illustrated below in **Figure 4-7**. These soils were grouped together based on their hydrologic soil group and four (4) soil groups were created. Soil areas designated as Muck were assigned a soil type that was consistent with the most conservative soil type in that drainage area. Land use was separated into agriculture, open water grouped with provincially significant wetlands, forest, medium density residential, low density residential, and estate, institutional, commercial, and open space. Based on Chapter 3 of the MOE drainage manual, and the land zones as given by the Town, the existing and future water budget parameters were developed and land uses were grouped as follows: Agriculture was considered as moderately rooted crops, forest as mature forest, open space as pasture and shrubs, and the remaining as urban lawns. Open water and wetlands are removed from the water budget calculations as their area is consistent from existing to future development. The impervious areas were calculated by the same methodology as was completed in the Hydrologic and Hydraulic Modeling for the West Holland River, East Holland River and Maskinonge River Watersheds Report (CCL, 2005) for the watershed as prepared by the Lake Simcoe Region Conservation Authority.

The water budget assessment infiltration rates were calculated based on the sample calculations located on *Page 3-5 of Chapter 3 of the MOE SWMP Manual (2007)*. A water budget sample calculation for the Mount Albert settlement area is provided below to outline the methodology and process:

Water Budget Sample Calculation

A sample calculation for Mount Albert is provided below utilizing the process outlined on *Page 3-5 of Chapter 3 of the MOE SWMP Manual (2007)*. It should be noted the sample calculation is not intended to be exhaustive and should be read in conjunction with detailed water balance calculations provided in **Appendix J**.

Mount Albert

Pre-Development Conditions

- *Land Use: Forest, Soil Type: Type B*
- Based on land use and soil type, Evapotranspiration (ET) values were derived from *Table 3.1 of the MOE SWMP Manual (2007)*:

ET = 548 mm

- Surplus (mm) is calculated by subtracting the ET (mm) from the total annual precipitation (901.2 mm).

Surplus = Precipitation – ET = 901.2 mm – 548 mm = 353.2 mm

- Infiltration (mm) is calculated by multiplying Surplus (mm) by the cumulative infiltration factors derived from *Table 3.1 of the MOE SWMP Manual (2007)* based on topography, soils and cover. In this case, topography is flat land (0.3), soils are open sandy loam (0.4) and cover is woodland (0.2).

Infiltration = Surplus * (Cumulative Infiltration Factors) = 353.2 * (0.3+0.4+0.2) = 317.9 mm

- Average annual land use infiltration is calculated by multiplying total area of the land use by infiltration:

Average annual land use infiltration = 1.14 ha * 317.9 mm = 362.4 ha·mm = 3624 m³

- This method is repeated for each soil type and land use within all the drainage areas within the settlement area (refer to **Appendix J** of the SWMMP).
- The total annual site infiltration volume is the sum of all infiltration values in the settlement area.

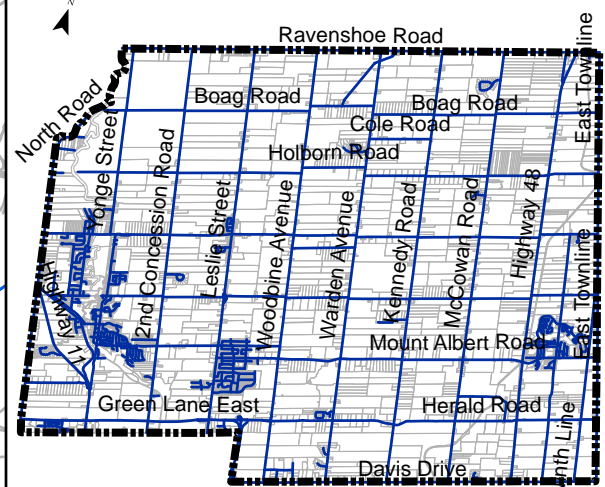
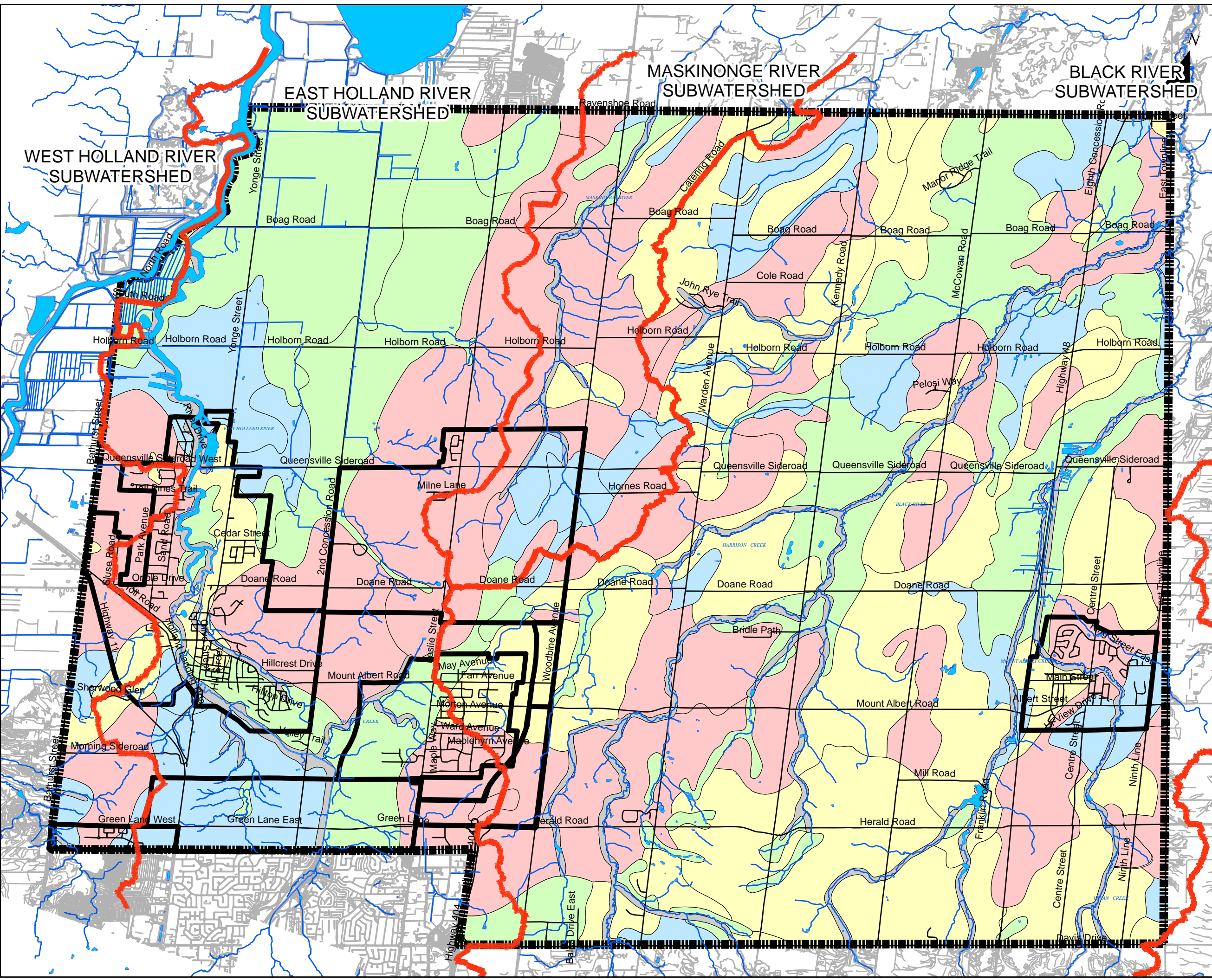
Total infiltration (mm) = **804805.5 m³** / 314.5 ha = 255.9 mm

- A similar approach is conducted for Post-Development – No Mitigation Conditions with larger impervious areas generating a reduced total infiltration (mm).

Total infiltration (mm) = **693192 m³** / 278 ha = 250 mm

- The annual infiltration rate required by each settlement area (Table 4-52) is the infiltration deficit divided by the total future development area:

Annual infiltration rate (m³/ha/year) = (804805.5 m³ – 693192 m³) / 36.9 ha = **3025 m³/ha/year**



KEY MAP
SCALE: 1:250,000

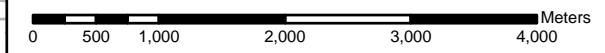
LEGEND

- TOWN BOUNDARY
- WATERSHED BOUNDARY
- COMMUNITY PLAN AREAS
- TYPE A SOILS
- TYPE B SOILS
- TYPE C SOILS
- TYPE D SOILS
- BOTTOM LAND

NOTES:



SCALE: 1:60,000



**STORMWATER MANAGEMENT
MASTER PLAN**

HYDROLOGIC SOIL GROUPS

Mount Albert Settlement Area

The settlement area of Mount Albert expects to undergo some development by 2031 according to the Town's OP. There will be an increase in commercial and industrial areas and a decrease in forest, and open space. The impervious area will increase from 12% to 22%. This increase of impervious area will create an infiltration deficit of 312.12 m³/ha.

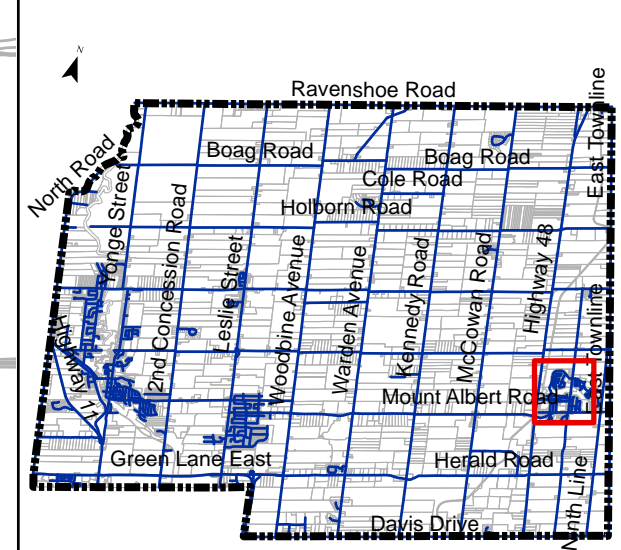
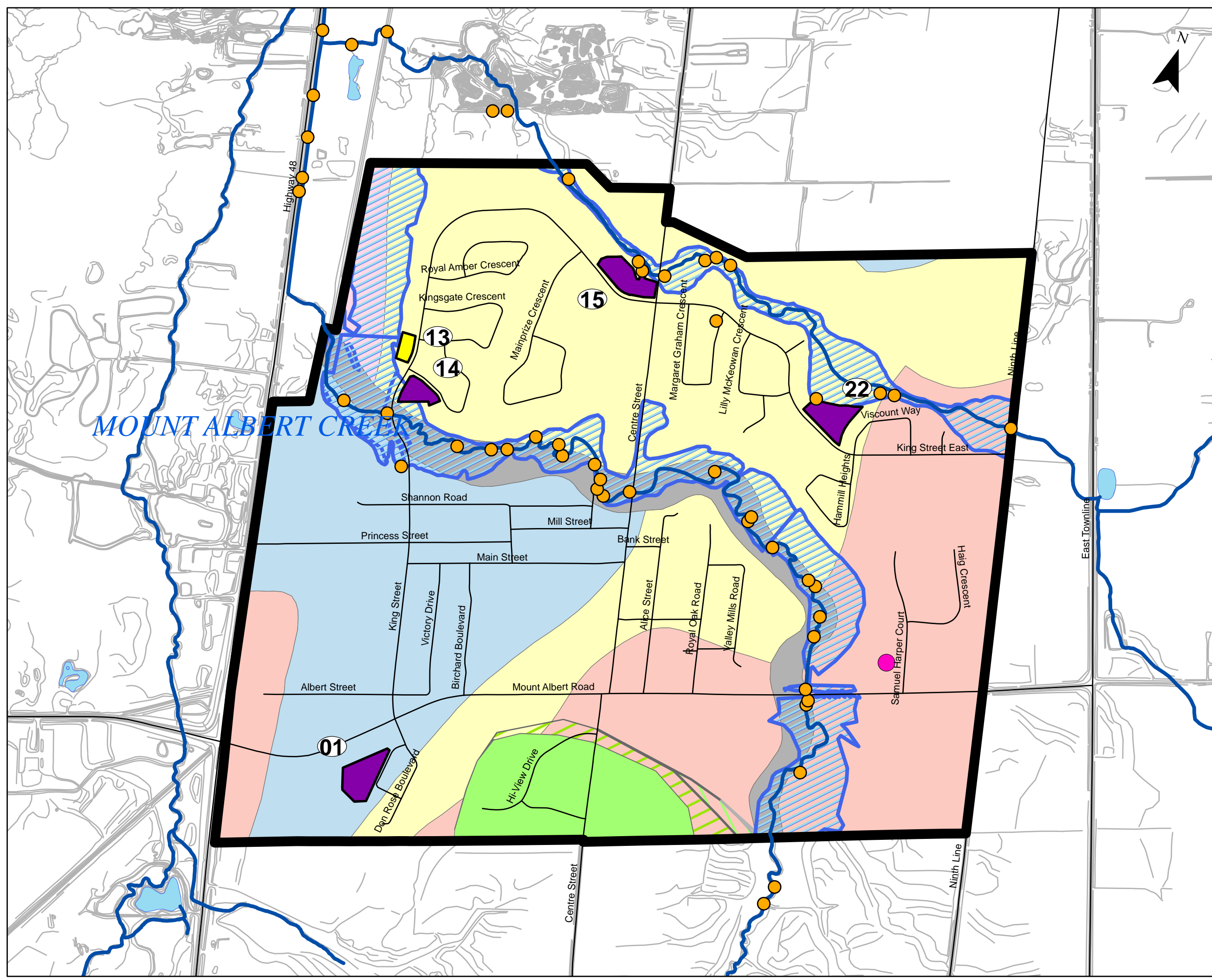
The Mount Albert settlement area contains all four (4) soil types (**Figure 4-8**) below, but consists mainly of Type A and B soils which are classified as well draining according to Table 3.1 of the SWM Planning and Design Manual (MOE SWM Planning and Design Manual (2003)). The water budget for Mount Albert is provided below in **Table 4-47**.

Table 4-47 – Mount Albert Water Budget

Pre-Development			
	Pervious Area	Impervious Area	Total
Area (ha)	314.5	43.1	357.6
Precipitation (mm)	901.2	901.2	
ET	535.67	90.12	
Surplus	365.53	811.08	
Total Infiltration (mm)	255.9	0	
Total Runoff (mm)	109.63	811.08	
Runoff (m ³)	344786.35	349575.48	694361
Infiltration (m ³)	804805.5	0	804805
Post-Development – No Mitigation			
	Pervious Area	Impervious Area	Total
Area (ha)	278	80	358
Precipitation (mm)	901	901	
ET	531	90	
Surplus	370	811	
Total Infiltration (mm)	250	0	
Total Runoff (mm)	120	811	
Runoff (m ³)	333965	648783	982748
Infiltration (m ³)	693192	0	693192

Queensville Settlement Area

Under existing conditions, Queensville is mostly agriculture with a small amount of land designated for residential. Queensville is anticipated to grow into a mixed use urban area, with a mix of residential, employment and commercial uses. The imperviousness will increase from 3% to 21%. Post-development rates must meet pre-development rates. The deficit is 315.26 m³/ha. Soil types in Queensville are of all four (4) types, and about half of the settlement area made of well draining soils (**Figure 4-9**) below. The water budget for Queensville is provided in **Table 4-47**.



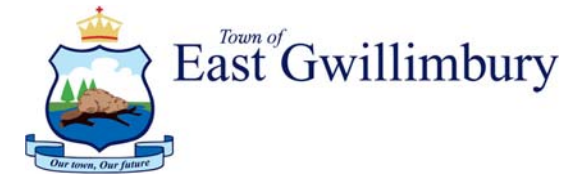
KEY MAP
SCALE: 1:250,000

LEGEND

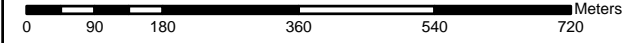
- | | | | |
|----------------------------------|-------------------|--|------------|
| | PROPOSED PONDS | | BOTTOMLAND |
| | EROSION SITES | | TYPE A |
| | EXISTING DRY POND | | TYPE B |
| | EXISTING WET POND | | TYPE C |
| | FLOODLINES | | TYPE D |
| WELLHEAD PROTECTION ZONES | | | |
| | 2 YEAR ZONES | | |
| | 5 YEAR ZONES | | |
| | 10 YEAR ZONES | | |
| | 25 YEAR ZONES | | |

NOTES:

It should be noted that the locations of proposed ponds may require further analysis



SCALE: 1:10,000

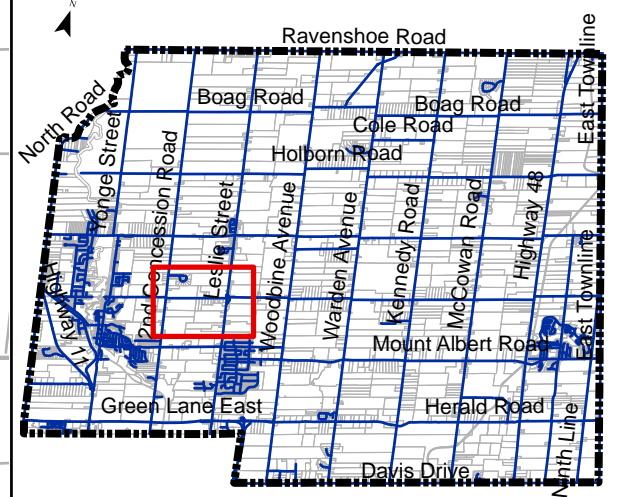
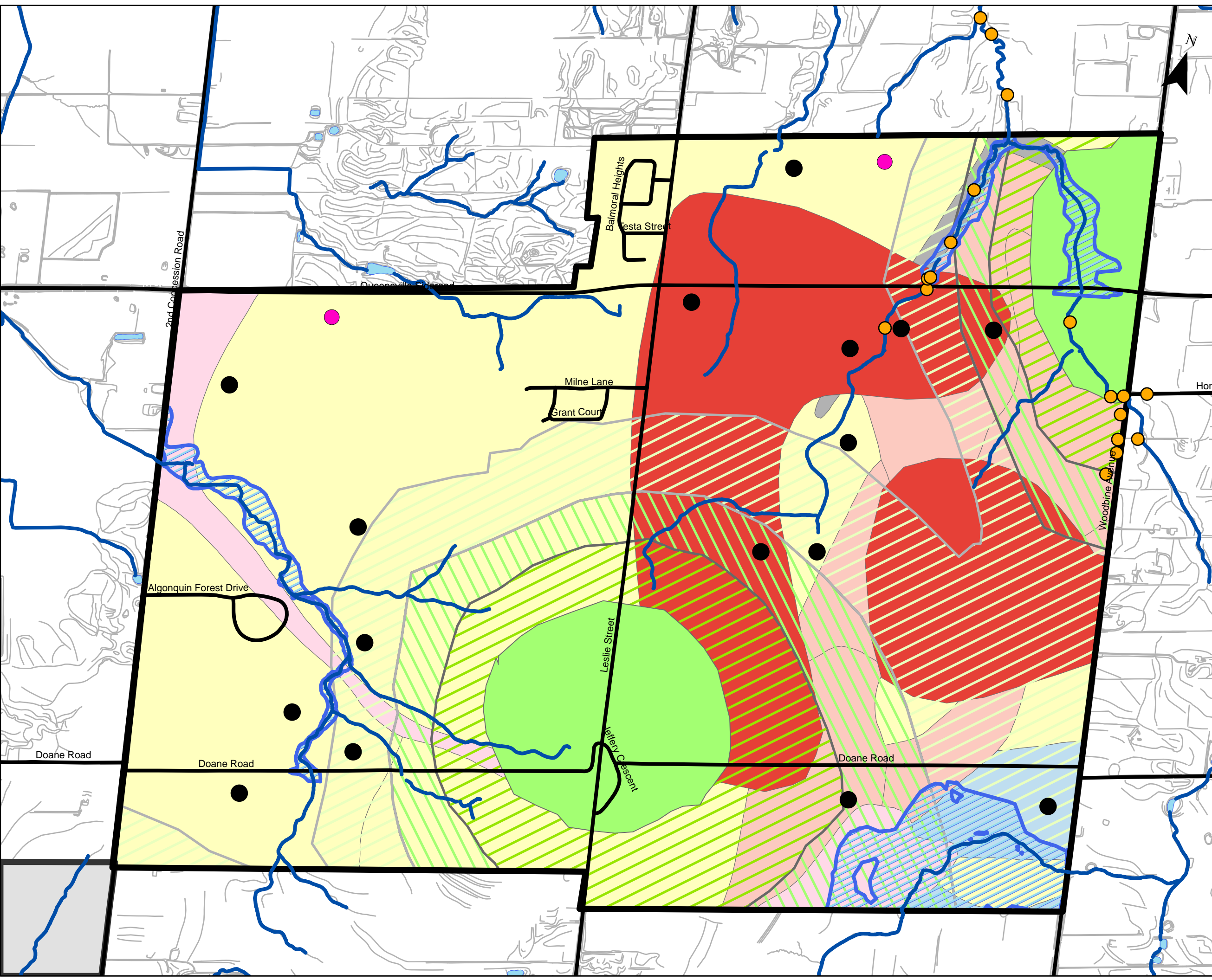


**STORMWATER MANAGEMENT
MASTER PLAN**

MOUNT ALBERT COMMUNITY AREA

FIGURE 4-8

DATE: JAN 2012



KEY MAP
SCALE: 1:250,000

LEGEND

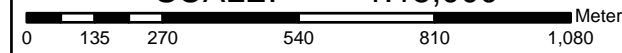
- EROSION SITES
- PROPOSED PONDS - MESP
- PROPOSED PONDS
- EXISTING DRY POND
- EXISTING WET POND
- FLOODLINES
- MASKINONGE RIVER SIG. GROUNDWATER RECHARGE
- 2 YEAR ZONES
- 5 YEAR ZONES
- 10 YEAR ZONES
- 25 YEAR ZONES
- SOIL TYPE
- BOTTOMLAND
- TYPE A
- TYPE B
- TYPE C
- TYPE D

NOTES:

IT SHOULD BE NOTED THE PROPOSED POND LOCATIONS MAY REQUIRE FURTHER ANALYSIS.



SCALE: 1:15,000



**STORMWATER MANAGEMENT
MASTER PLAN**

QUEENSVILLE COMMUNITY AREA

FIGURE 4-9

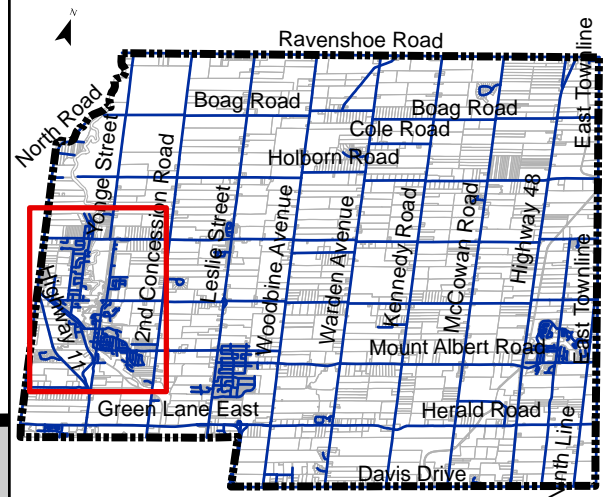
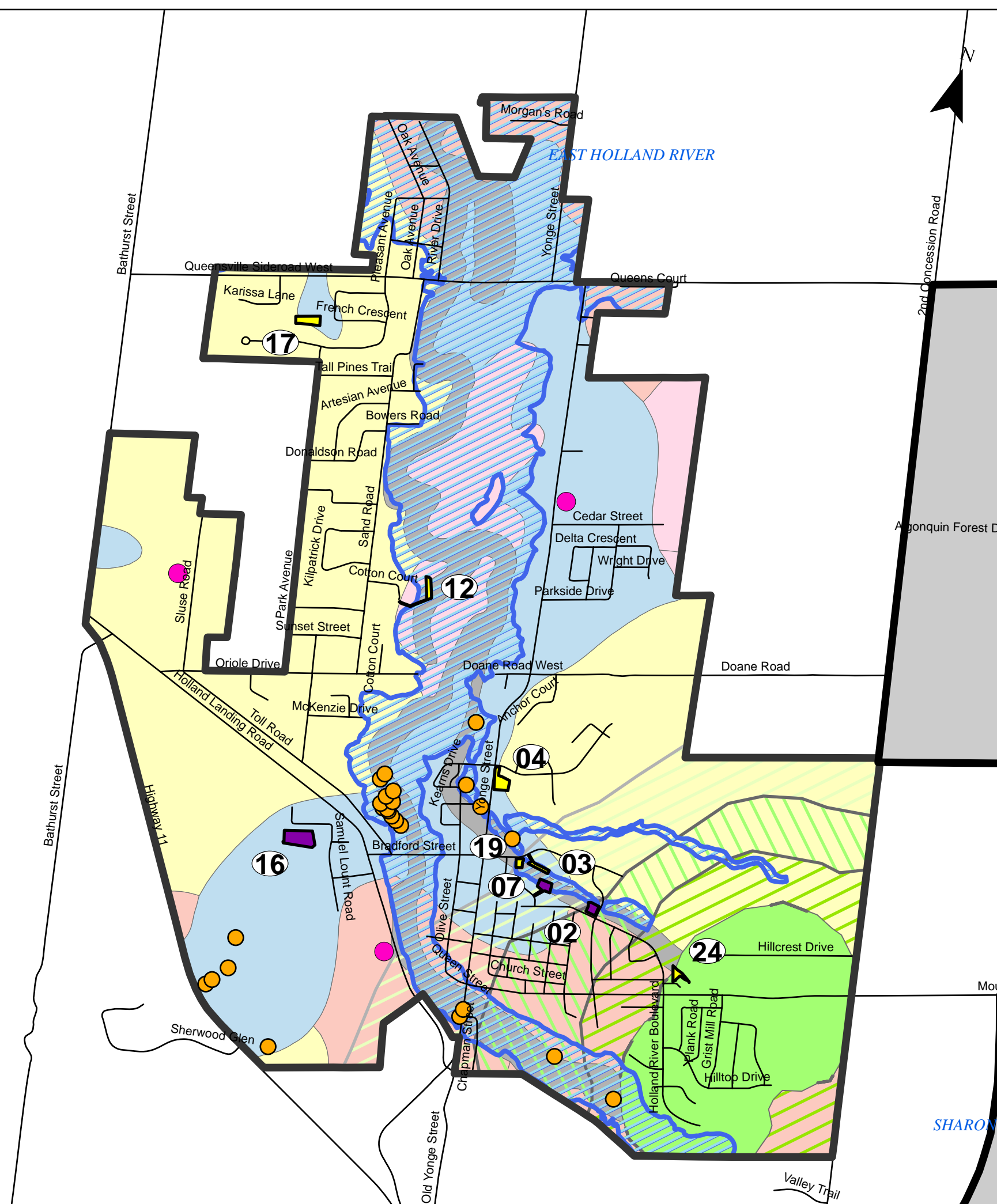
DATE: JAN 2012

Table 4-48 – Queensville Water Budget

Pre-Development			
	Pervious Area	Impervious Area	Total
Area (ha)	1147.6	34.5	1182.1
Precipitation (mm)	901.2	901.2	
ET	540.65	90.12	
Surplus	360.55	811.08	
Total Infiltration (mm)	251.99	0	
Total Runoff (mm)	108.56	811.08	
Runoff (m ³)	1245834.56	279822.6	1525657
Infiltration (m ³)	2891837.24	0	2891837
Post-Development – No Mitigation			
	Pervious Area	Impervious Area	Total
Area (ha)	935.8	246.3	1182.1
Precipitation (mm)	901.2	901.2	
ET	530.6	90.12	
Surplus	370.6	811.08	
Total Infiltration (mm)	269.2	0	
Total Runoff (mm)	101.4	811.08	
Runoff (m ³)	948901.2	1997690.04	2946591
Infiltration (m ³)	2519174	0	2519174

Holland Landing Settlement Area

This settlement area is partially developed under existing conditions. To accommodate future growth, Holland landing will acquire an employment and commercial area, as well a growth in residential lands. There is a large section of land that is designated as Provincially Significant Wetland or open water that surrounds the East Holland River (**Figure 4-10**) below. This area does not change. Imperviousness will increase 18% mostly due to the high percent impervious that commercial and employment areas have. The infiltration rate that the post-development is required to meet will ask for land to infiltration 273.46 m³/ha. The water budget for Holland Landing is provided in **Table 4-49**.



KEY MAP
SCALE: 1:250,000

LEGEND

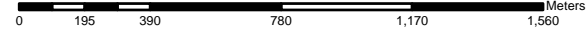
- | | | | |
|----------------------------------|----------------|--|-----------|
| | EROSION SITES | | SOIL TYPE |
| | PROPOSED PONDS | | TYPE A |
| | DRY POND | | TYPE B |
| | WET POND | | TYPE C |
| | FLOODLINES | | TYPE D |
| WELLHEAD PROTECTION ZONES | | | |
| | 2 YEAR ZONES | | |
| | 5 YEAR ZONES | | |
| | 10 YEAR ZONES | | |
| | 25 YEAR ZONES | | |

NOTES:

IT SHOULD BE NOTED THAT THE LOCATIONS OF THE PROPOSED PONDS MAY REQUIRE FURTHER ANALYSIS.



SCALE: 1:22,500



**STORMWATER MANAGEMENT
MASTER PLAN**

HOLLAND LANDING COMMUNITY AREA

FIGURE 4-10

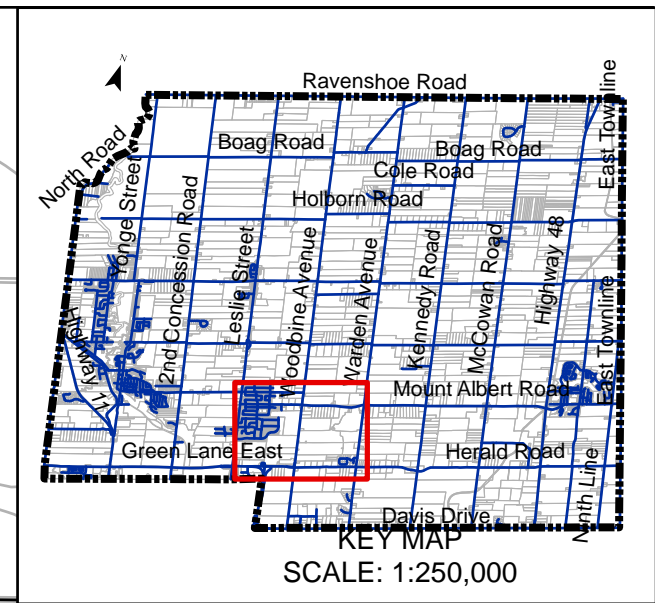
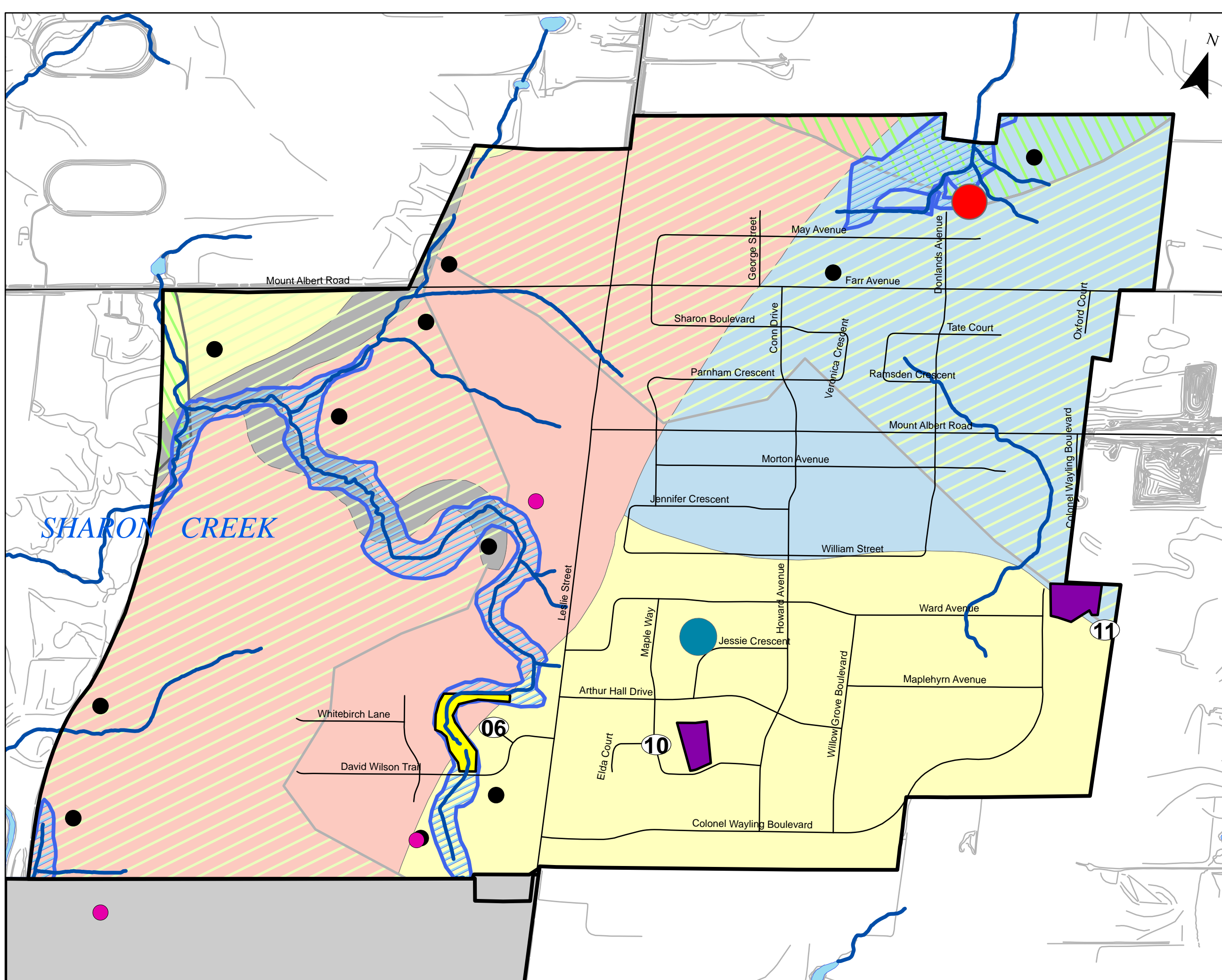
DATE: JAN 2012

Table 4-49 – Holland Landing Water Budget

Pre-Development			
	Pervious Area	Impervious Area	Total
Area (ha)	956.5	203.6	1160.1
Precipitation (mm)	901.2	901.2	
ET	535.3	90.12	
Surplus	365.9	811.08	
Total Infiltration (mm)	279.94	0	
Total Runoff (mm)	85.96	811.08	
Runoff (m ³)	822207.4	1651358.88	2473566
Infiltration (m ³)	2677626.1	0	2677626
Post-Development – No Mitigation			
	Pervious Area	Impervious Area	Total
Area (ha)	901.6	258.5	1160.1
Precipitation (mm)	901.2	901.2	
ET	529.3	90.12	
Surplus	371.9	811.08	
Total Infiltration (mm)	261.8	0	
Total Runoff (mm)	110.1	811.08	
Runoff (m ³)	992661.6	2096641.8	3089303
Infiltration (m ³)	2360389	0	2360389

Sharon Settlement Area

The Sharon Settlement Area is approximately half developed under existing conditions, but will grow to mostly residential area. The infiltration rate is 305.16 m³/ha, due to the increase in imperviousness (19% to 27%). The area made of well draining soils that are ideal for infiltration measures (**Figure 4-11**) below. The water budget for Sharon is provided in **Table 4-50**.



LEGEND

	PROPOSED PONDS		SOIL TYPE
	PROPOSED PONDS (MESP 2008)		TYPE A
	EROSION SITES		TYPE B
	TOWN IDENTIFIED WET PONDS		TYPE C
	TOWN IDENTIFIED DRY PONDS		TYPE D
	EXISTING DRY POND		
	EXISTING WET POND		
	Floodlines		
WELLHEAD PROTECTION ZONES			
	2 YEAR ZONES		
	5 YEAR ZONES		
	10 YEAR ZONES		
	25 YEAR ZONES		

NOTES:
IT SHOULD BE NOTED THE LOCATIONS OF PROPOSED PONDS MAY REQUIRE FURTHER ANALYSIS.

SCALE: 1:10,000

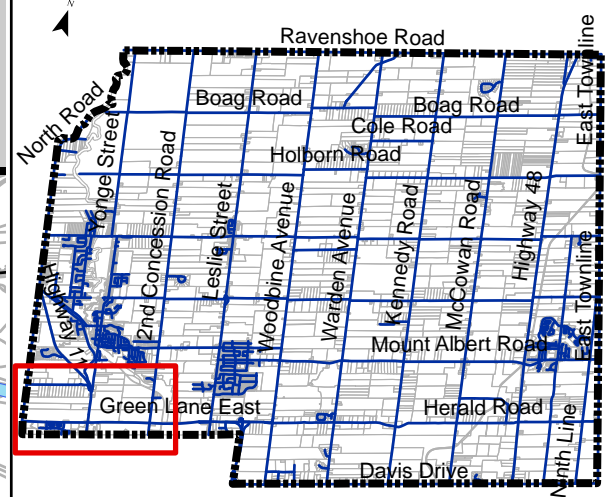
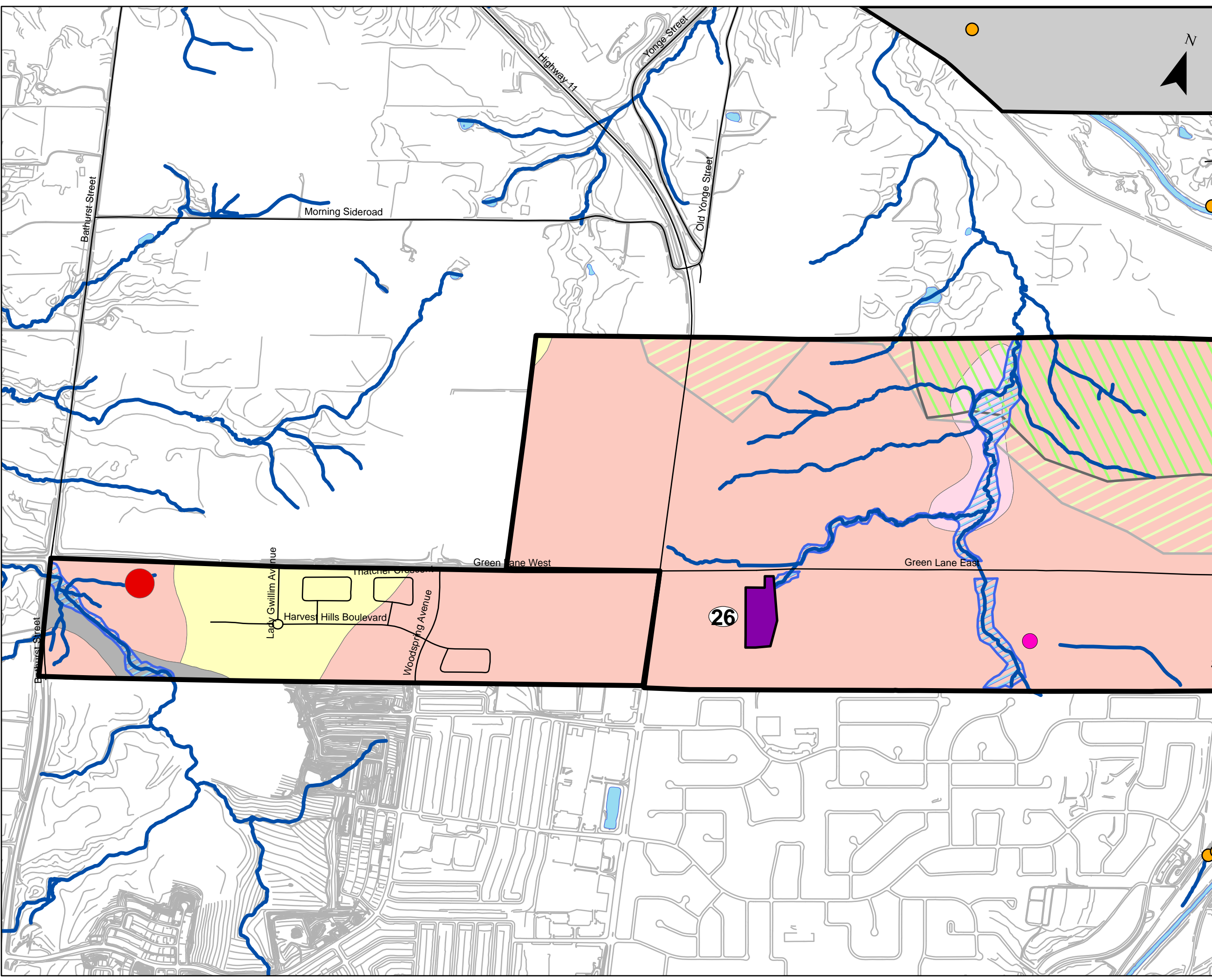
STORMWATER MANAGEMENT MASTER PLAN
SHARON COMMUNITY AREA

Table 4-50 – Sharon Water Budget

Pre-Development			
	Pervious Area	Impervious Area	Total
Area (ha)	379.1	88.9	468
Precipitation (mm)	901.2	901.2	
ET	532.85	90.12	
Surplus	368.35	811.08	
Total Infiltration (mm)	245.16	0	
Total Runoff (mm)	123.19	811.08	
Runoff (m ³)	467013.29	721050.12	1188063
Infiltration (m ³)	929401.56	0	929401
Post-Development – No Mitigation			
	Pervious Area	Impervious Area	Total
Area (ha)	341.4	126.6	468
Precipitation (mm)	901.2	901.2	
ET	524.8	90.12	
Surplus	376.4	811.08	
Total Infiltration (mm)	230.4	0	
Total Runoff (mm)	146	811.08	
Runoff (m ³)	498444	1026827.28	1525271
Infiltration (m ³)	786586	0	786586

Green Lane Expansion Areas

This area consists of the two (2) expansion areas along Green lane, including a proposed Community and Employment Area. Under existing conditions these lands are almost all agriculture with the exception of some residential and forested area. The future conditions will double the impervious area and require an infiltration rate of 456.21 m³/ha. Again this settlement area is comprised wholly of well draining soils ideal for infiltration measures (**Figure 4-12** and **Figure 4-13**) below. The water budget for the Green Lane Expansion Area is provided below in **Table 4-51**.



KEY MAP
SCALE: 1:125,000

LEGEND

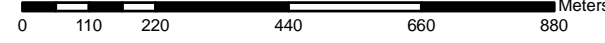
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|----------------------------------|---------------------------|------------------|-------------------|
| | EROSION SITES | | EXISTING DRY POND |
| | PROPOSED PONDS | | EXISTING WET POND |
| | TOWN IDENTIFIED WET PONDS | SOIL TYPE | |
| | FLOODLINES | | BOTTOMLAND |
| WELLHEAD PROTECTION ZONES | | | TYPE A |
| | 2 YEAR ZONES | | TYPE B |
| | 5 YEAR ZONES | | TYPE C |
| | 10 YEAR ZONES | | TYPE D |
| | 25 YEAR ZONES | | |

NOTES:

It should be noted that the locations of proposed ponds may require further analysis

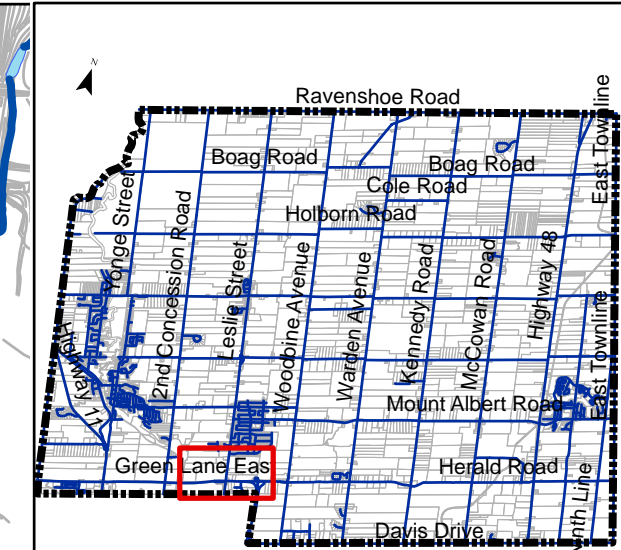
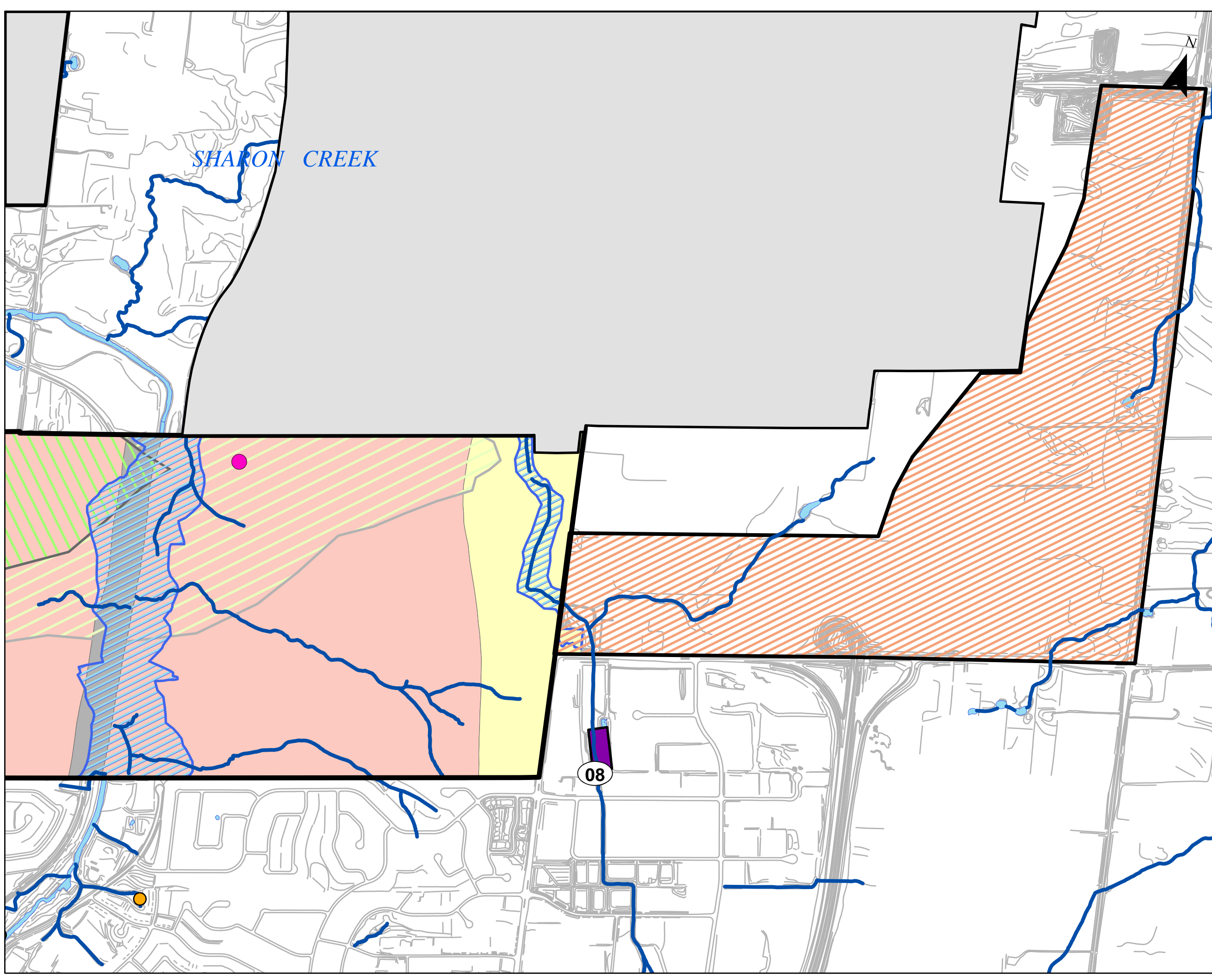


SCALE: 1:12,500



**STORMWATER MANAGEMENT
MASTER PLAN**
GREEN LANE WEST AND
GREEN LANE CORRIDOR

FIGURE 4-12 DATE: JAN 2012



KEY MAP
SCALE: 1:250,000

LEGEND

EROSION SITES	WELLHEAD PROTECTION ZONES
PROPOSED PONDS	2 YEAR ZONES
SECONDARY PLAN AREA	5 YEAR ZONES
EXISTING DRY POND	10 YEAR ZONES
EXISTING WET POND	25 YEAR ZONES
FLOODLINES	SOIL TYPE
	BOTTOMLAND
	TYPE A
	TYPE B
	TYPE C
	TYPE D

NOTES:
IT SHOULD BE NOTED THE LOCATIONS OF PROPOSED PONDS MAY REQUIRE FURTHER ANALYSIS.

SCALE: 1:12,500
0 110 220 440 660 880 Meters

**STORMWATER MANAGEMENT
MASTER PLAN**
GREEN LANE CORRIDOR AND
HIGHWAY 404 EMPLOYMENT AREA

Table 4-51 – Green Lane Expansion Areas Water Budget

Pre-Development			
	Pervious Area	Impervious Area	Total
Area (ha)	813.3	57.9	871.2
Precipitation (mm)	901.2	901.2	
ET	541.88	90.12	
Surplus	359.32	811.08	
Total Infiltration (mm)	227.6	0	
Total Runoff (mm)	131.72	811.08	
Runoff (m ³)	1071278.76	469615.32	1540894
Infiltration (m ³)	1851070.8	0	1851070
Post-Development – No Mitigation			
	Pervious Area	Impervious Area	Total
Area (ha)	744.3	126.9	871.2
Precipitation (mm)	901.2	901.2	
ET	530	90.12	
Surplus	371.2	811.08	
Total Infiltration (mm)	195.3	0	
Total Runoff (mm)	175.9	811.08	
Runoff (m ³)	1309223.7	1029260.52	2338484
Infiltration (m ³)	1453618	0	1453618

Detailed water balance calculations are provided in **Appendix J**.

Rural Areas

The rural part of East Gwillimbury is considered as unchanged and no development being done on this land. Rural areas consist of all soil types with the main landuse zoned for agricultural use. All landuses will remain the same; hence pre and post infiltration rates are matching.

Results

A summary of the water balance results are provided below in **Table 4-52**.

Table 4-52 – Water Budget Comparison

Settlement Area	Pre-Dev Runoff (m ³)	Post-Dev Runoff (m ³)	Increase / % Increase (m ³)	Pre-Dev Infiltration (m ³)	Post-Dev Infiltration (m ³)	Decrease / % Decrease (m ³)
Mount Albert	694362	982748	288386 / +41.5%	804806	693192	111614 / -13.9%
Queensville	1525657	2946591	1420934 / +93.1%	2891837	2519174	372663 / -12.9%
Holland Landing	2946591	3089303	142712 /	2519174	2360389	158785 /

Settlement Area	Pre-Dev Runoff (m ³)	Post-Dev Runoff (m ³)	Increase / % Increase (m ³)	Pre-Dev Infiltration (m ³)	Post-Dev Infiltration (m ³)	Decrease / % Decrease (m ³)
			+4.8%			-6.3%
Sharon	1188063	1525271	337208 / +28.4%	929402	786586	142816 / -15.4%
Green Lane Expansion Areas	1540894	2338484	797590 / +51.8%	1851071	1453618	397453 / -21.5%

A summary of the annual infiltration rates required by each settlement area (proposed development area only) are provided below in **Table 4-53**.

Table 4-53 – Annual Infiltration Rates Required by Each Settlement Area

Settlement Area	Future Development Area (ha)	Required Infiltration Rate (m ³ /ha/year)
Mount Albert	36.9	3025
Queensville	211.8	1760
Holland Landing	54.9	2892
Sharon	37.7	3788
Green Lane Expansion Areas	69	5760

Based on the analysis, a general increase in runoff will occur during the post-development condition with a reduction in infiltration. The largest increase in runoff occurs in Queensville and the greatest reduction in infiltration occurs in the Green Lane Employment Expansion Area. These areas will require the implementation of BMPs in an integrated treatment train approach to mitigate the loss of infiltration from the proposed changes in land use. Alternative BMP practices are further discussed in **Section 8.2**. It should be noted that specific mitigation measures are to be confirmed on a site specific basis at the functional design stage.

4.6. Erosion Analysis

4.6.1. General

Stormwater runoff can cause severe erosion concerns if uncontrolled runoff continues to drain into the watercourse with a lack of stabilization measures. Erosion can occur on stream banks from flow in the watercourse or runoff. This can change the characteristics of the stream by widening and decreasing the depth due to sediment accumulation. Sediments may also be carried in runoff from bare agricultural lands or construction sites.

The LSRCA and the Town are committed to decreasing the amount of sediment that is being carried into Lake Simcoe as well as its tributaries.

As per Policy 4.5 SA of the LSPP, an erosion analysis of existing and future conditions is required. This section of the SWMMP will detail the existing erosion study and relevant erosion analysis recommendations to be applied to future developments and studies.

4.6.2. Background

Erosion is of concern within in the Town due to the large percentage of agriculture land, and numerous watercourses. Agricultural lands are particularly vulnerable when they are bare (e.g. after tilling, or in the spring) and can contribute large amounts of sediment from runoff. In rural areas, farmers may remove vegetation along watercourses in order to maximize arable land and in doing so contribute to stream bank instability and erosion. In urban areas, impervious surfaces can increase the volume and velocity of stormwater runoff, leading to an increase in streambank erosion, and sediment deposition.

4.6.3. Existing Conditions

An erosion site survey was completed by the LSRCA in 2008. The LSRCA conducted a BMP Inventory, spanning 12 sections of the 18 subwatersheds within the Lake Simcoe basin.

The subwatershed studies of the East Holland River, West Holland River, Maskinonge River and Black River assess this study and provide details of the BMP opportunities where applicable. The purpose of the BMP inventory was to identify opportunities for the reduction of nutrients or improvements to fish habitat within the Lake Simcoe basin (LSRCA, 2010). As part of the study, existing erosion sites within the watercourses were identified. The erosion sites within the Town are depicted above in **Figure 3-2**.

4.6.4. Future Conditions

In order to mitigate existing erosion points and prevent future erosion sites the following recommendations have been made.

The mitigation of stream bank erosion will need to be completed on a site by site basis following a more detailed analysis. Proponents may suggest the most appropriate manner to solve the erosion such as the use of rip-rap or amoustone however; the LSRCA encourages the use of natural channel designs and “soft solutions”. This includes improving riparian vegetation that may have been removed or damaged. The vegetation helps to stabilize the banks decreasing erosion and phosphorus loading. Increasing the ‘buffer’ zone to at least 15 m around watercourses will help increase bank stability, filter excess nutrients from surface runoff, reduce soil erosion and improve water quality.

The implementation of traditional BMP measures will help control the volume of stormwater as well as mitigating sediment erosion concerns. Measures such as wet ponds, grassed swales, and wetlands enhance sediment removal by increasing detention times. These measures can also decrease urban runoff which is known to change the flow patterns in rivers increasing erosion. In addition, SWM pond retrofits may reduce erosion by eliminating uncontrolled outfalls and upgrading existing SWMF to higher protection levels.

Fluvial Assessments

Future developments within the Town will be required to conduct a fluvial geomorphological and erosion assessment of the receiving watercourse by a qualified fluvial geomorphologist. The objectives of the assessment will be to identify areas of ongoing channel adjustment and to develop appropriate mitigation measures to limit future impacts. As future developments will contribute runoff to a watercourse, an assessment is required in order to ensure that changes upstream as a result of development will not adversely impact the creeks.

It should be noted that the detail of the fluvial assessment (if required) should be confirmed with the LSRCAs along with any other applicable approval agencies.

A fluvial assessment of a receiving watercourse typically consists of:

- An analysis of existing geomorphic conditions:
 - Aerial photography (i.e. changes in channel movement over time);
 - Field work results (i.e. site photos, measurement of channel dimension parameters); and,
 - Cross sections and profiles of the channel.
- Rapid Field Assessments:
 - RSAT Evaluations;
 - Stability Index (SI) Interpretation; and,
 - Analysis of Qualitative Habitat Evaluation Index.
- Erosion Thresholds:
 - The point at which sustained flows will tend to entrain and transport sediment, specifically the D_{50} of the substrate material; and,
 - Development of critical shear stresses and critical flow (on reach basis).
- Flow Duration Analysis Recommendations.

The fluvial assessment should result in future modelling recommendations (i.e. event-based modelling vs. continuous modelling) and establish the critical flow for the receiving reach which is necessary to conduct the supplementary erosion exceedance analysis for the development. The erosion exceedance analysis will set the extended detention criteria for the site (i.e. 24hr, 48hr or 72hr extended detention) which will retain stormwater on site mitigating adverse erosion impacts downstream.

Smaller development sites that prove to have minimal impact to the watershed may only be required to maintain “good housekeeping” practices. These include monitoring mud tracked from the construction site, controlling silt and debris being washed into storm sewers and watercourses, and controlling wind blown dust. The Lake Simcoe Protection Plan is proposing policies to minimize the amount of dust erosion generated from construction sites that may impact future developments. Sites with greater impact to the watershed require greater sediment and erosion control similar to traditional BMP measures as well as maintaining “good housekeeping” practices. Erosion and sediment controls should be monitored during and after construction.

It should be noted that detailed erosion analysis of creeks within the Town will not take place as part of this study. The Town will investigate the opportunity for individual erosion assessments where development will take place (e.g. Sharon West development) or if another major opportunity for stabilization arises. The next two (2) phases of the EA Process will feature detailed erosion assessments on an individual development basis.

5.0 Effectiveness of Existing SWM Works

5.1. Existing SWM Assessment

5.1.1. General

As part of Policy 4.5 SA of the LSPP, a determination of the effectiveness of existing SWM works is required to assess the existing reduction of negative impacts of stormwater on the environment. This requirement involves the verification of the existing pond control structure works and a supplementary sensitivity analysis to examine the existing SWM effectiveness within the Town. See **Section 4.0** for the effectiveness of using a lumped pond approach for the different Community Areas in maintaining downstream peak flows. It is noted that **Section 6.0** through **Section 7.0** of this report detail the examination and verification of existing SWM works (i.e. quality of existing SWMF). The following section will only detail the sensitivity analysis requirement performed on the existing SWMF within the Town.

5.1.2. Background

A review of existing information was conducted and the hydrologic and SWM reports received from the Town that were reviewed for this assessment are listed below:

- Pond 23: Technical Design Brief SWMFs, York Region Industrial Subdivision 19T-94016, Cumming Cockburn Limited (CCL) (July 2004);
- Pond 17: SWM Analysis for Amberglen Estates Phase III File 19T-89091, A.M. Candaras Associates Inc. (December 1994);
- Pond 21: SWM Plan – Manors of Forest Ridge, Town of East Gwillimbury, URS Cole, Sherman (June 2001); and,
- Pond 18: SWM for Riverstone Estates Draft Plan 19T-89117, Greenland International Consulting Inc. (July 2002).

The pond locations used in the sensitivity analysis are illustrated on **Figure 3-5**. It is noted that to perform the sensitivity analysis, only select existing SWMF were reviewed and implemented. The sensitivity analysis methodology is further explained in **Section 5.1.4** of this report.

5.1.3. Objectives

To assess the effects of climate change on the existing SWM works, a hydrologic model will be established to reproduce the SWMF designs of select SWMF within the Town. A sensitivity analysis will then be applied to determine the effects on the representative peak flows and storage volumes of the existing SWMF.

The objectives of the verification and sensitivity analysis of the existing ponds are summarized as follows:

- Verify the hydrologic input parameters (i.e. Drainage Area, CN, TIMP, XIMP, and rating curve) from select existing SWMF within the Town where applicable;
- Develop a sensitivity analysis by adjusting CN values and the 100-year storm event rainfalls and intensities by factors of 25% to simulate the effects of climate change; and,
- Comment on the effects on quantity performance (i.e. peak flows and storage volumes) of the existing SWM works within the Town including a number of assumptions regarding climate change.

5.1.4. Methodology

To conduct the sensitivity analysis, a balanced assessment of existing SWMF characteristics must be developed. The sensitivity analysis will be established through the following seven (7) steps:

- Identify four (4) existing SWMF within Town boundaries which are designed to receive runoff from:
 - A large urbanized drainage area (**Pond 23**);
 - A small urbanized drainage area (**Pond 17**);
 - A large rural drainage area (**Pond 21**); and,
 - A small rural drainage area (**Pond 18**).
- Utilize existing information from existing SWM reports and drainage area plans to establish the corresponding hydrologic input parameters for each SWMF;
- Replicate the hydrologic model with Visual OTTHYMO v2.3. and run under the 100-year storm event;
- Adjust the duration of the 100-year storm event (from 12hr to 6hr and 24hr) to simulate the effects of climate change on short and long duration storm events;
- Adjust the 100-year storm event rainfall intensity by factors of 25% by modifying the storm parameter (RFACT) to simulate the effects of climate change on low and high intensity storm events;
- Adjust the CN input parameter (from $CN_{AMC II}$ (average) to $CN_{AMC I}$ (dry) and $CN_{AMC III}$ (wet) to simulate the effects of climate change on the antecedent moisture conditions of the soils; and,
- Comment on the resulting effectiveness of the existing SWMF (i.e. changes in peak flow and storage volumes) in response to climate change.

5.1.5. Existing SWM Input Parameters

The existing SWM input parameters for the four (4) selected SWM ponds are summarized below in **Table 5-1**.

Table 5-1 – Sensitivity Analysis SWM Input Parameters

Pond ID	Subwatershed	Subcatchment Type (ID)	Area (ha)	CN	TIMP (%)	XIMP (%)	Tp (hr)	SLPI (%)	LGI (m)
23	Black River	STANDHYD (3010)	26.95	76.0	79	79	-	1.5	423.87
		STANDHYD (3020)	5.36	62.0	87	87	-	1.0	189.03
17	East Holland River	STANDHYD (101)	2.60	60.0	25	12	-	1.5	131.00
		STANDHYD (102)	3.75	60.0	1	1	-	1.0	157.00
		NASHYD (103)	2.23	60.0	-	-	0.20	-	-
		NASHYD (104)	1.96	60.0	-	-	0.55	-	-
21	Black River	STANDHYD (204)	1.46	65.5	27	13.5	-	1.7	145.00
		STANDHYD (206)	2.65	65.5	24.5	12.3	-	1.8	340.00
		STANDHYD (207)	4.86	65.5	17.5	8.8	-	0.5	450.00
		STANDHYD (210)	1.64	55.0	27	8.8	-	0.5	30.00
		NASHYD (203)	1.91	57.3	-	-	0.09	-	-
		NASHYD (205)	5.45	58.7	-	-	0.31	-	-
		NASHYD (208)	4.31	57.0	-	-	0.54	-	-
		NASHYD (209)	3.63	56.3	-	-	0.58	-	-
		NASHYD (101)	30.00	54.2	-	-	0.18	-	-
18	Black River	STANDHYD (1)	5.22	50.0	20	20	-	1.0	186.50
		NASHYD (2)	3.63	75.0	-	-	0.06	-	-
		NASHYD (3)	1.52	80.0	-	-	0.05	-	-

In addition, the existing SWMF rating curves are summarized below in **Table 5-2** to **Table 5-5**.

Table 5-2 – Rating Curve for Pond 23

Subwatershed	Outflow	Storage
	(m ³ /s)	(ha·m)
Black River	0.000	0.000
	0.014	0.152
	0.037	0.307
	0.058	0.465

Subwatershed	Outflow	Storage
	(m ³ /s)	(ha·m)
	0.073	0.627
	0.086	0.793
	0.097	0.975
	0.302	1.161
	0.318	1.349
	0.334	1.540
	0.349	1.735
	0.363	1.932
	0.376	2.133
	1.088	2.337
	2.379	2.544
	4.046	2.754
6.017	2.967	

Table 5-3 – Rating Curve for Pond 17

Subwatershed	Outflow	Storage
	(m ³ /s)	(ha·m)
East Holland River	0.000	0.0000
	0.007	0.0015
	0.017	0.0925
	0.410	0.1756

Table 5-4 – Rating Curve for Pond 21

Subwatershed	Outflow	Storage
	(m ³ /s)	(ha·m)
Black River	0.000	0.000
	0.000	0.000
	0.003	0.146
	0.020	0.263
	0.071	0.419
	0.112	0.555
	0.151	0.751
	0.173	0.905

Subwatershed	Outflow	Storage
	(m ³ /s)	(ha·m)
	0.184	1.010
	0.185	1.038
	0.187	1.205
	0.189	1.381

Table 5-5 – Rating Curve for Pond 18

Subwatershed	Outflow	Storage
	(m ³ /s)	(ha·m)
Black River	0.000	0.000
	0.007	0.425
	0.064	0.430
	0.073	0.465
	0.096	0.525
	0.139	0.580
	0.192	0.615
	0.254	0.651

It is noted that the Pond IDs correspond to the pond survey IDs described in **Section 7.0** (as implemented by SWMSOft) and a figure detailing the pond locations is provided as **Figure 3-5**.

5.2. Hydrologic Modelling Results

The 100-year storm (SCS Type II 12hour) for the Black River and East Holland River subwatersheds were run to evaluate the existing outflow and storage volume used in each SWMF. This scenario of hydrologic modelling is called the 'base model'. The resulting peak flows and storage volumes of the base model are summarized below in **Table 5-6**.

Detailed hydrologic model output is included in **Appendix K**.

Table 5-6 – Post-Development Base Model Peak Flow and Storage Volumes of Existing Ponds

Pond ID	Storm Event	Rainfall	Q _{OUTFLOW}	Storage Used
	year	mm	m ³ /s	ha·m
23	100	121	1.312	2.3738
17	100	89	0.242	0.1402
21	100	121	0.198	2.1785
18	100	121	0.004	0.2723

5.3. Sensitivity Analysis

In support of the evaluation of the existing SWM works within the Town, a sensitivity analysis was conducted of the modelling. The resulting peak flow hours and storage volumes of the base model were evaluated against a variation in storm event duration, rainfall intensity and CN values. The revised CN values for each SWMF are summarized below in **Table 5-7**.

Table 5-7 – Sensitivity Analysis Variation in CN Value

Pond ID	Subwatershed	Subcatchment Type (ID)	Area (ha)	CN _{AMCII}	CN _{AMCI}	CN _{AMCIII}
23	Black River	STANDHYD (3010)	26.95	76.0	58.0	88.0
		STANDHYD (3020)	5.36	62.0	42.0	79.0
17	East Holland River	STANDHYD (101)	2.60	60.0	40.0	77.0
		STANDHYD (102)	3.75	60.0	40.0	77.0
		NASHYD (103)	2.23	60.0	40.0	77.0
		NASHYD (104)	1.96	60.0	40.0	77.0
21	Black River	STANDHYD (204)	1.46	65.5	46.0	82.0
		STANDHYD (206)	2.65	65.5	46.0	82.0
		STANDHYD (207)	4.86	65.5	46.0	82.0
		STANDHYD (210)	1.64	55.0	35.0	73.0
		NASHYD (203)	1.91	57.3	37.0	74.0
		NASHYD (205)	5.45	58.7	39.0	76.5
		NASHYD (208)	4.31	57.0	37.0	74.0
		NASHYD (209)	3.63	56.3	36.0	76.5
18	Black River	STANDHYD (1)	5.22	50.0	30.0	70.0
		NASHYD (2)	3.63	75.0	56.0	88.0
		NASHYD (3)	1.52	80.0	63.0	91.0

The results of the sensitivity analysis for peak outflows and storage volumes of each evaluated SWMF are presented below in **Table 5-8** and **Table 5-9**.

Table 5-8 – Sensitivity Analysis Results (Peak Flow)

Condition / Adjustment	Peak Flow (m ³ /s) / Percent Change from Base Model			
	Pond 23	Pond 17	Pond 21	Pond 18
	(Large Urban)	(Small Urban)	(Large Rural)	(Small Rural)
Base Model / None	1.312	0.242	0.198	0.004
6hr / 100-year Storm Duration	0.322 / -75.5%	0.129 / -46.7%	0.137 / -30.8%	0.002 / -50.0%
24hr / 100-year Storm Duration	1.008 / -23.2%	0.887 / +266.5%	0.199 / +0.5%	0.005 / +25.0%

	Peak Flow (m ³ /s) / Percent Change from Base Model			
-25% / Rainfall Intensity	0.367 / -72.0%	0.090 / -62.8%	0.188 / -5.1%	0.003 / -25.0%
+25% / Rainfall Intensity	3.201 / +144.0%	0.426 / +76.0%	0.210 / +6.1%	0.006 / +50.0%
AMC I Conditions / CN	1.110 / -15.4%	0.078 / -67.8%	0.187 / -5.6%	0.003 / -25.0%
AMC III Conditions / CN	1.465 / +11.7%	0.469 / +93.8%	0.212 / +7.1%	0.006 / +50.0%

Table 5-9 – Sensitivity Analysis Results (Storage Volumes)

Condition / Adjustment	Storage Volume (ha-m) / Percent Change from Base Model			
	Pond 23	Pond 17	Pond 21	Pond 18
	(Large Urban)	(Small Urban)	(Large Rural)	(Small Rural)
Base Model / None	2.3738	0.1402	2.1785	0.2723
6hr / 100-year Storm Duration	1.4025 / -40.9%	0.1161 / -17.2%	0.6826 / -68.7%	0.1056 / -61.2%
24hr / 100-year Storm Duration	2.3143 / -2.5%	0.2771 / +97.6%	2.2347 / +2.6%	0.2931 / +7.6%
-25% / Rainfall Intensity	1.9950 / -16.0%	0.1080 / -23.0%	1.2766 / -41.4%	0.1820 / -33.2%
+25% / Rainfall Intensity	2.6535 / +11.8%	0.1793 / +27.9%	3.2137 / +47.5%	0.3716 / +36.5%
Dry AMC I Conditions / CN	2.3421 / -1.3%	0.1055 / -24.8%	1.1871 / -45.5%	0.1996 / -26.7%
Wet AMC III Conditions / CN	2.3975 / +1.0%	0.1883 / +34.3%	3.3906 / +55.6%	0.3694 / +35.7%

It is noted that the variable conditions as summarized above in **Table 5-8** and **Table 5-9** outline the assumptions by which climate change may have an impact on existing SWMF. Detailed climate change assessments were not completed as part of this study.

Results

By inspection and as shown above in **Table 5-8** and **Table 5-9**, the peak outflows are generally sensitive for a shorter duration storm for all drainage areas, where modelling indicates that a 25% change from the Base Model value results in a greater than 25% change in the peak flow, either positive or negative. Similarly, longer storm duration is the most sensitive to the peak flows from a SWMF draining a small urbanized area. The storage volumes generally decrease during short storm events as expected and increase (with the exception of the large urban area) during longer duration storms. To summarize, the existing ponds would operate at normal conditions for short storm events (i.e. peak flows and storage volumes are adequate) and during longer storm events the pond outflows and storage volumes would generally increase.

Alternatively, the peak flows are generally not sensitive to a 25% reduction and increase in rainfall intensity for rural areas. Peak flows from SWMFs draining urban areas however are very sensitive to decreases and increases in rainfall intensity. Pond volumes are generally not sensitive to changes in rainfall intensity for ponds draining urban areas and are sensitive in rural areas. In addition, moisture conditions of the draining soils are generally sensitive to ponds draining smaller drainage areas for both peak outflows and storage volumes. This would suggest that larger ponds would still operate at a normal level during saturated ground conditions as a result of climate change.

The sensitivity analysis provided above in **Table 5-8** and **Table 5-9** indicates that the changes in the conditions of storm events and hydrologic parameters on the existing SWMF in the Town are generally sensitive in terms of peak outflows and storage volumes.

It should be noted that the sensitivity analysis performed is to simulate the assumed hydrologic instability based on climate change. Climate change is variable in nature and cannot be wholly assessed as part of this analysis.

5.4. Climate Change

The LSRCA realizes that the concept of Climate change is still an evolving term but needs to be addressed as it is expected to influence, directly and indirectly all elements of the Lake Simcoe Watershed. Climate change may potentially impact the Town by having an increase in phosphorus loading, increase in sediment and contaminants in flood transportation, reduction in ground water flows, variation in stream flow regimes, and changes in precipitations, lake levels, erosion, and ice cover. The Conservation Authority would like municipalities to investigate comprehensive master plans which will consider the potential impacts of climate change on the effectiveness of SWM works. This is detailed in the above sections. Future risk assessment and adaptation planning may need to be completed once the impacts of climate change have been confirmed.

6.0 Examination of Stormwater Retrofit Opportunities

6.1. General

The Lake Simcoe Basin SWM and Retrofit Opportunities (2007) Report by the LSRCA has identified the need for a complete, consistent, and contemporary data set associated with stormwater runoff in the Lake Simcoe Watershed. This data set includes all urban catchments, outlets, existing SWMFs and locations, and phosphorus loads associated with stormwater runoff. This report also identifies retrofit opportunities including facilities that can be upgraded, or areas that can support a SWMF.

The retrofit opportunities in the Town include:

- Three (3) in Sharon;
- 20 in Holland Landing; and,
- Six (6) in Mount Albert.

A detailed list, including type of retrofit, size of SWMP, phosphorus reduction rate, and an estimated cost is available in the report which is publicly accessible on the LSRCA website (www.lsrca.on.ca). The existing SWM retrofit opportunities within the Town including uncontrolled outlets and areas of a potential SWMF retrofits are illustrated above in **Figure 3-5**.

6.2. Examination of Existing Retrofit Strategy

Alongside the LSRCA, the Town has identified areas and existing facilities that will require a retrofit. Most of these facilities have been identified in the Lake Simcoe Basin SWM and Retrofit Opportunities (2007) and previous reports and studies completed by the LSRCA. The Town and LSRCA have paired to advance projects where funding is available through the Lake Simcoe Clean Up Fund. This is an ongoing effort and the overall goal is to permit all existing facilities within the Town to meet Enhanced requirements.

6.3. Retrofit Strategy Development

Generally, in the absence of a Town retrofit strategy, the development of a new retrofit strategy is required as part of the Policy 4.5 SA of the LSPP. However, as per discussion with LSRCA staff, a new retrofit strategy is not required at this time due to the overall 2007 strategy completed by LSRCA.

The existing areas requiring retrofit should correspond to the areas identified by the LSRCA in the Simcoe Basin SWM and Retrofit Opportunities (2007) Report. The development of the retrofit strategy of existing SWMFs will result from the implementation of SWMSoft SWMF software. Existing SWMFs were surveyed and recommendations based on existing operating conditions are developed as a part of the SWM Maintenance Program detailed in **Section 7.0** of this report. In addition, the SWMSoft Inspection Report and Bathymetric Surveys are included in **Appendix L**.

7.0 SWM Maintenance Program

7.1. Introduction

The Town is responsible for the operations and management of 26 identified SWMFs including both wet and dry ponds. As part of the Great Golden Horseshoe, East Gwillimbury is expected to grow from 23,000 to 86,400 residents by 2031. To effectively manage this population growth, the Town has requested the examination of current SWMFs as part of this Master Plan.

Cole Engineering was retained by the Town to conduct a SWMF Assessment and Maintenance Study to facilitate the on-going management of these SWMFs, and promote effective management techniques. This report presents a summary of findings collected.

7.1.1. Objective

The objective of this section is to:

- Collect field and existing document information of all the Town-owned facilities;
- Conduct a detailed field assessment and evaluate all of SWMFs and their components;
- Estimate available permanent pool volumes in assumed wet ponds;
- Assess the operation and maintenance requirements to keep the facilities in proper operating condition;
- Forecast future maintenance requirements including tasks, resources, schedules and costs; and,

- Implement a database management system to facilitate on-going operations and maintenance.

A good maintenance plan should be proactive, demonstrating due diligence by calling for a comprehensive collection and analysis of data. It should prioritize inspections and maintenance works for repairs and restorations. Performing accurate assessments of individual facilities is necessary to determine the requirements for staff, physical and budget resources required for maintenance. In the long run, proper planning reduces liabilities and costs.

7.2. Background

The Town currently is responsible for the operation and management of 26 assumed SWMFs, and by 2031, the Town will be responsible for a multitude of SWMFs both wet and dry.

Most of the ponds are designed and constructed by developers in accordance with current SWMF design standards and ownership is transferred to the Town once the construction associated with the development area is complete.

7.3. Function and Maintenance of SWMFs

SWMFs have been introduced to mitigate the impacts of urban runoff from existing and new development areas.

Depending on their design, SWMFs can provide:

- Flood protection;
- Water quality treatment;
- Erosion control;
- Base flow augmentation;
- Infiltration;
- Spill management;
- Aesthetics; and,
- Buffer between urbanized areas and/or natural areas.

The following sections describe the basic functions, along with related maintenance activities, that can keep SWMFs operating as intended.

7.3.1. Applicable Legislation, Regulations and Guidance Documents

The following section provides discussion on legislation, regulation and guidance documents which are currently applicable to the construction, operation and maintenance of SWMFs from the federal, provincial, municipal level.

Fisheries Act

The Department of Fisheries and Oceans (DFO) is the implementing agency of the *Federal Fisheries Act*. Section 35 of the Act prohibits the Harmful, Alteration, Disruption and Destruction (HADD) of fish habitat and Section 36 of the Act prohibits the release of substances deleterious to fish habitat. Where a HADD cannot be avoided a Letter of Authorization may be issued by the DFO detailing required mitigation and habitat compensation measures. During the permit issuing process DFO determines whether approval under the *Canadian Environmental Assessment Act* (CEAA) is required for the project. This determination would be based on the decision regarding a HADD.

LSRCA has an agreement with the DFO to identify fish habitat impacts within their watersheds. In addition, the CAs provides co-ordination and assistance to DFO for review and assessment of projects, which may impact fisheries and/or fishery habitat. In general SWMFs are not considered fish habitat, however, natural wetlands, lakes and the creeks connected to SWMFs are. As such, it is the responsibility of the Town to ensure that proposed works which may affect wetlands, lakes and creeks do not have a negative impact on the natural fish habitat.

Canadian Environmental Assessment Act

In most cases, Federal Environmental Assessments are not applicable for work associated with SWMFs, however, one can be triggered if a designated licence, permit, or approval such as an authorization for a HADD under the *Fisheries Act* is required. In general, an environmental assessment is a process to predict the environmental effects of proposed initiatives before they are carried out. They can be fairly straight forward or be subject to a comprehensive study reviewed at the federal level depending on the potential environmental impacts.

Ontario Environmental Assessment Act

Establishment of new SWMFs is subject to the Municipal Class Environmental Assessment Process described by the Municipal Engineers Associations, 2007 in accordance with the *Ontario Environmental Assessment Act*.

The Class EA planning takes into consideration the protection of all aspects of the natural, social and economic environment as well as long-term planning for the mitigation of any adverse effects during both construction and commissioning. The Class EA process also includes consultation with the Public, First Nations, Government Agencies, local interest groups and review bodies to obtain input and feedback and to ultimately attain general acceptance for the preferred alternative.

The design and construction of most SWMFs would follow the Schedule A Class EA process since they would be required as a condition of approval of site plan, or plan of subdivision and would come into effect under the *Planning Act* prior to the construction of the facility. Modifications or retrofits to SWMFs are subject to a Schedule A+ Class EA. Most other new SWMFs would follow the Schedule B Class EA process.

Places to Grow Act and the Greenbelt Act

"*Places to Grow*" is the Ontario Government's initiative to promote sustainable growth and development in the Greater Golden Horseshoe. The plan outlines the framework for infrastructure including water and wastewater infrastructure. It supports the policies identified in the Greenbelt and encourages innovative SWM actions.

Ontario's Greenbelt is an area of permanently protected green space, farmland, forests, wetlands and watersheds. The Greenbelt Plan covers approximately 1.8 million acres of environmentally sensitive and agricultural land in the Golden Horseshoe. SWMFs within the Greenbelt are subject to the requirements of the Greenbelt Plan which are aimed at the protection of key natural features and key hydrologic features.

Lakes and Rivers Improvements Act

The Ministry of Natural Resources (MNR) is the lead agency responsible for the management of fish resources in Ontario. MNR approval is required under the *Lakes and Rivers Improvement Act* for the placement of structures to convey and dissipate flows. Potential changes to the rivers streams or stormwater ponds may be required MNR approval.

Ontario Water Resources Act

As per the Ministry of the Environment (MOE) Certificate of Approval (C of A) issued under Section 53 of the *Ontario Water Resources Act*, owners of SWMFs are responsible for maintaining them in proper working condition.

This means operating as per the original design specifications and includes all components in the original design such as inlet and outlet structures, side slopes, sediment forebay, shading measures, erosion protection lining, manhole covers and the approved volumes.

The C of A clearly outlines the legal requirements where the owner must:

“ensure that the works will be operated, maintained, funded, staffed, and equipped in a manner enabling compliance with the terms and conditions...such that the environment is protected and deterioration, loss, injury, or damage to any person or property is prevented”

With regards to sediment removal, the owner must understand that:

“regular removal of sediment from the approved SWM works is required to mitigate the impacts of sediment on the downstream receiving watercourse. It is also required to ensure that adequate storage is maintained in the SWMFs at all times, as required by the design”

Within a municipal context, developers typically build and own the SWMFs as part of the development block, and they are responsible for maintaining them in proper operating condition. However, the responsibility shifts when the municipality assumes ownership. Prior to transferring ownership to the Town, an as-built survey should be completed to ensure that the facility was built and maintained in accordance with the C of A. A full clean out should also be conducted on the facility to return it to the full permanent pool volume as per the original design requirement, unless the survey indicates a minimal amount of accumulated sediment (Drake & Guo 2008).

The MOE provides the following Guidance Documents to understand the requirements of SWMFs and assist with the application for a C of A:

- MOE. SWM Planning and Design Manual. March 2003; and,
- MOE. Section 53, Ontario Water Resources Act. Sample Application Package for a Sanitary Sewage and Stormwater Collection System & SWMF Certificate of Approval. June 2009.

Ontario Municipal Act

The *Municipal Act* empowers individual municipalities to adopt by-laws, policies and guidelines to suit the goals and needs of each local community. The Town has developed standards for SWMFs as described in Section 4 of the Subdivision Design Manual, dated February 2005. The manual outlines the requirements imposed by the Town for the design and preliminary maintenance needs for new SWMFs.

Conservation Authorities Act

Section 28 enables the Conservation Authority to make Regulations prohibiting, regulating or requiring the permission of the authority for straightening, changing, diverting or interfering in any way with the existing channel of a river, creek, stream or watercourse, or for changing or interfering in any way with a wetland.

The Lake Simcoe Region Conservation Authority (LSRCA) is authorized through O.Reg. 179/06, Regulation of Development, Interference with Wetlands and Alterations to Shorelines and Watercourses.

Through this Regulation, the CA may prohibit work from taking place within valley and stream corridors, wetlands and associated areas of interference. Any work which falls within the regulated areas described as follows is subject to this regulation:

- Straightening, changing, diverting or interfering in any way with the existing channel of a river, creek, stream or watercourse, or for changing or interfering in any way with a wetland;
- Development, if in the opinion of the authority, the control of flooding, erosion, dynamic beaches or pollution or the conservation of land may be affected by the development; and,
- Where Development is defined as:
 - The construction, reconstruction, erection or placing of a building or structure of any kind;
 - Any change to a building or structure that would have the effect of altering the use or potential use of the building or structure, increasing the size of the building or structure or increasing the number of dwelling units in the building or structure;
 - Site grading; and/or,
 - The temporary or permanent placing, dumping or removal of any material, originating on the site or elsewhere.

Endangered Species Act

In general, engineered SWMFs such as wet ponds and dry ponds are not considered fish habitat, however, they may discharge to creeks which provide habitat for endangered species. In such situations, provisions need to be made to protect the habitat and ensure that no impacts occur. Wetlands and lakes, however, can be considered fish habitat and it would be necessary to consider the requirements under this act. If it is suspected that endangered species are within the study area, the MNR should be consulted to determine if specific measures are required.

Environmental Protection Act

Proper management of SWMFs will require periodic maintenance to restore the permanent pool volume. Sediment quality, management and disposal of impacted dredged material should adhere to MOE requirements:

- MOE. Guidance on Sampling and Analytical Methods for Use at Contaminated Sites in Ontario. December 1996;
- O.Reg. 153/04 as amended by O.Reg. 511/09 – *Records of Site Condition*; and,
- O.Reg. 347/90 as amended by O.Reg. 558/00 – General Waste Management.

Oak Ridges Moraine Conservation Plan

Any development that is to occur on lands under the Oak Ridges Moraine Conservation Plan (ORMCP) legislation must be accompanied by a stormwater management plan as set out in section 46 of the Oak Ridges Moraine Conservation Plan. The objectives of a stormwater management plan are to; maintain groundwater quantity and quality; protect aquatic species and their habitat; prevent increases in stream channel erosion; and; prevent any increase in flood risk. The ORMCP would prefer to have an integrated treatment train approach that uses a planned sequence of methods for controlling stormwater and minimising impacts.

Lake Simcoe Protection Plan

Further discussion is provided above in **Section 1.0**.

7.4. Methodology

The following section describes in detail the methodology undertaken to assess each facility's maintenance needs and collected data. During the course of the study, design drawings and reports were scanned into electronic files to allow instant access from the software.

These files were also used to populate the database with all relevant information regarding the SWMFs. All of the field data necessary to complete the study was collected from winter 2010 to present. This data includes facilities, components, inspection results, maintenance recommendations, and bathymetric survey results.

7.4.1. Database Development

The design of the database was completed early in the project. The design involved merging data from the Town with Cole Engineering's SWMF management database, SWMSOft. Most of the data from the Town database consisted of facility information (name, type, function and location) as well as data referring to its functionality (drainage area, slope, and drainage length). It should be noted that the data collected had gaps and require further refinement. Additional tabs consisted of five (5) main groups:

1. Attributes;
2. Inspection;
3. Maintenance;
4. Hydrology; and,
5. Resources.

7.4.2. Attribute Data

The attribute data set contains the physical characteristics of a facility or component, for example, its location or parent component. The relationships between facilities and components (inlet, outlet, and berm) and between a parent component (inlet) and its child component (baffle blocks, headwall, and grate) were stored so that components could be easily found and logically stored in a tree view. This was limited to a degree of two (2) to reduce unnecessary complexity in the system.

7.4.3. Inspection Data

Inspection information was stored after each inspection of the facility. This data includes the inspector, condition of the component, comments by the inspector, date of inspection, condition ranking and photo.

Components assessed include:

- General facility appearance;
- Inlet and outlet structures;
- Low flow channels;
- Emergency overland spillway;
- Vegetation;
- Access road and walkways;
- Perimeter fencing;
- Any unusual situations within the facility such as the presence of erosion, unsafe conditions, nuisance issues, encroachments, poor water quality, etc.;
- Sediment accumulation; and
- Public safety.

The condition ranking system is the basis for the maintenance assessment of the facilities and components. A condition ranking between 1 and 5 was given, where:

1 = Excellent (the component has no deterioration);

2 = Satisfactory (some wear is noticed, but does not affect the functionality of the component);

3 = Attention Required (the component is still functioning but has minor problems that may prevent the component from functioning properly during extreme events – some simple upkeep is required);

4 = Non-Functional (the component is no longer functioning as designed); and,

5 = Safety Hazard (the component presents a safety hazard either because it allows access to restricted areas, e.g. a grate on a pipe is not secure, or the component is structurally unsound e.g. erosion of the access road).

7.4.4. Maintenance Data

Maintenance information was stored when a maintenance task was opened after an inspection. The information stored when a task was opened consists of the maintenance task, the description, type of task (inspection, upkeep, repair, or replacement), a photograph of the component, and any resources needed to complete the task. Resources are discussed in the following section.

When a task is closed, the information on how long the task took, as well as when it was completed can be stored with the original budget and the final cost of completing the tasks. Photographs taken during maintenance can also be stored with the closed maintenance task. By incorporating photographs into every aspect of the system, any user from any department can have a good idea about the current state of a component, as well as how it changes over time.

7.4.5. Resource Data

Internal and external resources available to the Town play an important role in the maintenance of a facility. Resources are used to perform tasks such as repairs and sediment removal. Resources are separated into four (4) groups: Town staff, materials, contractors, and consultants. Each resource is given a cost as well as a unit in terms of time or frequency when tasks are assigned. This allows the number of units of a given resource to be summed in order to render the total cost of the maintenance task. It is important to note that if the rate of a resource is changed, the cost of all open maintenance tasks is also changed in the software. Once the task is closed, the budget for the task is stored together with the actual cost. Thus, a comparison can be made between budgeted and actual maintenance costs, which can be used for future planning.

7.4.6. File Data Collection

Collecting file data is an important step in preparing the maintenance plan. The Town provided numerous scanned reports with information pertaining to the maintenance of the facilities or their components. All reports, drawings, briefs, and files were reviewed and any facility or drainage area information was added to the database. Existing data gaps are further explained in **Section 1.5**. All drawings and all main bodies, and as much of the appendices as possible of each report were scanned. The scanned images are stored such that they are easy to access from the software, thereby facilitating the search for information.

7.4.7. Field Data Collection

All field data was collected via a GPS or total station survey, digital camera, and inspection sheets. If an assumed permanent pool was associated with the facility, then a bathymetric survey was completed with the Cole Engineering-designed bathymetric rod and boat.

Inspections

During an inspection, each component's location was captured with the GPS station. This data was used to place an icon on the Master drawing, giving a complete picture of the facility. Also, at least one photograph of each component was taken so that future inspections can be compared. Photos, besides of each component, included overviews of SWMFs, upstream and downstream embankment faces, inlets to the SWMF and downstream outfalls from the SWMF and any issues associated with the SWMF. These photos allowed for maintenance tasks to be assigned without the need for further visits.

When performing the visual inspection, all field staff used a general inspection form, included in the SWMSOft report. The Pond ID, subcomponent and parent component were entered, along with descriptions of the component and its location. A ranking from 1 to 5 was selected to indicate the condition of the component and a space for any comments related to the ranking was provided for rankings 3 and higher. A default satisfactory description is available in the software when entering inspection results. The SWMSOft Inspection Report is included in **Appendix L**.

Bathymetric Survey

As mentioned above, if a facility has an assumed permanent pool component, a bathymetric survey was completed as well as the component survey. Bathymetric surveys assessed the quantity of sediment accumulated in the pond. Two (2) methods were used to collect the data, with the second method being used only when the equipment was available.

For both methods, the equipment used included a portable boat, several sections of floating rope, tent anchors, spray paint, and two (2) rods, one (1) neutrally buoyant and the other made of metal. In the first method, a measuring tape and total station survey equipment were also required. In the second method, global positioning system (GPS) survey equipment was used instead of a total station.

The first step in the process consisted of examining the site for suitable cross-sections that would provide enough points to develop the model, in locations where there were likely changes in depth. In most cases, cross-sections spaced evenly every 10 – 15 m across the entire pond were completed. Anchor points were sprayed or flagged, numbered, and surveyed along the water's edge where the cross-section meets the water. Upon completion of the staking, a rope was strung across the cross-section to keep the boat in a straight line.

In the first method, the measuring tape was secured to the anchor. The rope was used to keep the boat in a straight line between the cross-section points and the tape measure was used to ensure a point was taken every 5 m (2.5 m if the cross-section was very narrow). The total station survey equipment was used to take an X-Y coordinate position in relation to a benchmark determined on the SWMF. In the second method, GPS survey equipment was used, and UTM coordinates were recorded at every point and sediment and pond bottom depth were measured. These measurements were taken approximately every 5 m.

To measure the sediment depth, the neutrally buoyant bathymetric rod was allowed to slowly sink until it rested on the sediment. Once the reading was taken, the metal rod was threaded through the neutrally buoyant rod and the sediment until it reached the original bottom of the pond. This process continued across the entire pond to complete the bathymetric survey.

The bathymetric survey provided multiple cross-sections across the entire pond. These points were then compiled to create a digital terrain model of the two surfaces. This is explained in the following section and detailed in **Appendix L**.

7.4.8. Permanent Pool Volume Analysis

Cole Engineering began analyzing the permanent pool volume using the coordinates of each pond as well as each point in the cross-sections inputted into the software. The software then generated the Northing, Easting, and an elevation points for the sediment and pond bottom for each measurement. Next, the points were imported into AutoCAD Civil 3D and into Digital Terrain Models that we generated to create an accurate model of each pond. From this model, we calculated the current sediment volume and the remaining permanent pool volume. Generated drawings for each facility show the top of sediment contour lines and several cross-sections of the pond. The volumes of both permanent pool and active storage, if available, are also displayed. Drawings are included in the Bathymetric Surveys provided in **Appendix L**.

Assess Maintenance Requirements

After completing inspections, all components with a condition rating of 3, 4, or 5 were evaluated for their maintenance needs. To create the maintenance task and to allocate the resources needed to complete it, photographs and comments made by the inspector were used. For each task, a related photograph taken during the inspection was selected in order that those performing the maintenance would have a visual of what the task would entail. While adding a due date and reminder date was possible, this was not done since the Town could more appropriately provide future work schedules.

7.5. Results

7.5.1. Inventory

A comprehensive inventory of the Town's SWMFs can be found in the SWMSOft report and a key map is provided in **Figure 6**. This inventory lists the ponds by Pond ID and provides information regarding the facility name, pond type (i.e. wet pond, and dry pond.), facility function, facility type (offline or online), facility status (assumed or unassumed), and whether or not a bathymetric survey was completed.

7.5.2. Inspections

A summary of inspection results for all ponds, listed by Pond ID, is generated by SWMSOft and is included in the SWMSOft report in **Appendix L**. All ponds with at least one (1) component ranking 3, 4 or 5 are highlighted in **Table 7-2** as they require immediate maintenance.

7.5.3. Permanent Pool Volume Analysis

Based on the bathymetric surveys and permanent pool volume analysis, the sediment volume, remaining permanent pool volume, and total pond volumes are displayed in the SWMSOft report along with a recommended sediment removal timeframe. Bathymetric drawings for the ponds that are over 40% full of sediment are provided in the SWMSOft report. The MOE *SWM Planning and Design Manual* (2003) recommends storage volumes based on drainage area and MOE protection level, which determines the efficiency at which the pond removes sediment. Drainage area and protection level information was not available for all ponds in order to calculate the necessary sediment volume to be removed so as a general rule, ponds with >50% sediment accumulation was considered to require immediate sediment removal.

The ponds identified as requiring immediate sediment removal are 01, 08, 10 and 17. Pond 15 is recommended to have sediment removed within the next five years.

Upon review of the design reports available for these ponds, it appears that the amount of sediment accumulation exceeded initial predictions. It is possible that proper erosion control plans were not implemented during construction of the subdivision. In discussion with the Town, it was determined that the ponds had not been dredged prior to assumption by the Town, which may account for some of the excess accumulated sediment. It should be noted that some of the ponds assumed by the Town were designed and built before the MOE guidelines went into effect. Before a sediment plan is attempted, we recommend reviewing the possibility of a retrofit that would update the facility to current standards as the area surrounding the pond will, in any case, need to be disturbed and any necessary equipment will already have to be in place. The SWMSOft Inspection Report and Bathymetric Surveys are included in **Appendix L**.

7.6. Sediment Removal Process

7.6.1. Background Review

Since each SWMF has specific site characteristics, it is important to conduct a thorough Background Review to identify approvals requirements, evaluate alternative approaches and develop an optimal sediment removal program.

Review of Available Information

In order to obtain a good understanding of the scope of the program, certain documents should be reviewed in detail including:

- A comprehensive bathymetric survey which outlines the estimated quantities requiring removal;
- SWM Design Drawing, or Drainage Plans; and
- Other reports on retrofits and maintenance.

Field Investigations

A site visit should be conducted to collect additional information about the existing site conditions. As part of the site visit: sediment sampling, topographic survey, condition survey of the pond structures, SWMFs areas and adjacent properties, possible access routes, tree protection / removal requirements, utilities, temporary storage area of sediments, etc. should be assessed.

Public Consultation / Liaison

Communication with the local community is a key component in the successful execution any major program. It will be important to inform adjacent private property owners of the work to be carried out on the SWM facilities. Information to be provided to the private property owners will include, but not limited, to the following:

- Construction access to the SWM facility;
- Method of sediment removal and dewatering;
- Potential generation of dust during dewatering operations;
- Possible noise level due to sediment removal and dewatering operation;
- Odour impact;
- Duration of sediment removal and dewatering operation; and,
- Timing of sediment removal and dewatering operation.

Any required easement and construction access through private properties will be identified in the early stages of the design and discussed with affected property owners.

Agencies Consultation and Approvals

Permitting and approval requirements for each pond may differ depending on Site conditions (i.e. on-line vs. off-line, within regulated areas, or if significant environmental impacts are anticipated). It will be important to consider what legislation and regulations are applicable. **Table 7-1** below provides a discussion on potential legislation applicable in the management of SWMFs. The key regulatory agencies which may be involved with sediment removal processes are included below in **Table 7-1**.

Table 7-1 – Discussion on Potential Legislation

Agency	Applicable Approvals
Department of Fisheries and Oceans	<p>Wetlands, lakes, and on-line ponds which provide fish habitat would need to be reviewed for federal requirements.</p> <p>If a Harmful, Alteration, Disruption and Destruction (HADD) of fish habitat or a release of substances deleterious to fish habitat is anticipated, the DFO should be consulted. The DFO can also determine whether approval under the Canadian <i>Environmental Assessment Act</i> (CEAA) is required for the project.</p>
Ministry of Environment	<p>Sediment removal is considered a regular maintenance activity and would generally require a Schedule A Environmental Assessment unless if environmental impacts are anticipated. Other work may trigger other requirements and should be reviewed on an individual basis.</p> <p>Sediment quality, management and disposal of impacted dredged material should adhere to MOE requirements:</p> <p>MOE. Guidance on Sampling and Analytical Methods for Use at Contaminated</p>

Agency	Applicable Approvals
	Sites in Ontario. December 1996. O.Reg. 153/04 as amended by O.Reg. 511/09 - <i>Records of Site Condition</i> O.Reg. 347/90 as amended by O.Reg. 558/00 – General Waste Management
Ministry of Natural Resources	In most cases, MNR involvement is not anticipated for the management of sediments in SWMFs, unless changes to flows in rivers, creeks or discharges from SWMFs are expected. If it is suspected that endangered species are within the project area, the MNR should be consulted to determine if specific measures are required.
Lake Simcoe Region Conservation Authority	Some SWMFs fall within regulated areas which are mapped by the CAs who regulate and may prohibit work from taking place within valley and stream corridors, wetlands and associated areas of interference and the Lake Ontario waterfront.
York Region	Construction access may be required on Regional property. Tree Removal / Protection By-Law may need to be reviewed as part of the development of the sediment removal.

Management of Sediments

During field investigations, sediment samples should be collected to develop a basic understanding of the physical and chemical properties of the sediment materials.

Sampling should be conducted using a protocol and methodology as described in the MOE Guidance on Sampling and Analytical Methods for Use at Contaminated Sites in Ontario Document, dated December 1996.

Analysis to characterize sediment properties should include:

- Grain size analysis;
- Inorganic parameters and nutrients (Moisture content, Total Organic Carbon, TKN, ammonia, nitrate, nitrite, phosphorus);
- Full suite of analysis as per the O.Reg. 347/90 as amended by O.Reg. 558/00; and,
- Full suite of analysis as per the O.Reg. 153/04 as amended by O.Reg. 511/09.

All analysis shall be conducted at a Canadian Association for Laboratory Accreditation (CALA) Accredited Laboratory. Understanding the physical properties can help in evaluating the various sediment removal options.

Comparison of analytical results with criteria defined by O.Reg. 558/00 can determine whether materials are classified as hazardous wastes. If materials are classified as hazardous wastes, then disposal options are limited to hazardous waste receivers. If materials are not hazardous wastes, then various disposal options can be considered.

Comparison of analytical results with the “soil criteria” defined by O.Reg. 511/09 (Soil, Ground Water and Sediment Standards for Use Under Part XV.1 of the Environmental Protection Act, July 2009) can help in establishing whether sediments meet acceptable environmental criteria for different types of land-uses. According to the amended regulations, non-hazardous soils meeting certain land-use criteria can be transported as fill material to another property with equal or less stringent criteria.

Sediment sampling conducted prior to removal can provide a general understanding of conditions which can be expected during removal, however, additional sampling of materials as they are excavated will be required if they are to be transported off-site to ensure they are disposed of in accordance with all applicable statutory and receiver requirements. O.Reg. 511/09 requires at least one (1) soil sample shall be analyzed for each 160 cubic metres of soil for the first 5,000 cubic metres and at least one sample for each additional 300 cubic metres of soil.

7.6.2. Design and Tender

Methodology Review and Selection

The understanding of physical and chemical site conditions should be used to develop a comprehensive program. Various sediment removal alternatives should be evaluated to identify the preferred approach taking into consideration:

- Excavation (Conventional and/or Hydraulic);
- Dewatering (Drying Beds / Geo-Synthetic Bags / Belt Filter Press / Centrifuge); and,
- Disposal of Dewatered Sediments (On-site / Offsite).

The “preferred solution” will base on the best possible fit of removal and disposal methodologies under the existing conditions at each location and consider:

- Duration;
- Construction and equipment access;
- Work hours;
- Stockpiling of removed sediments;
- Pond dewatering;
- Sediment quality sampling;
- Disposal route;
- Odour Impact;
- Noise Impact;
- Aesthetic Impact; and
- Overall Cost.

Sediment Removal Options

For sediment removal both mechanical as well as hydraulic dredging methods are available. Both methods carry their advantages and disadvantages.

Conventional Excavation

Different excavating machines could be deployed such as tracked or wheeled front loader, shovel, back hoe, and backfill blade or bulldozer. The method of sediment drying / dewatering is also a factor in the selection. The size of the pond and the physical properties of the sediments is a key factor in the selection of the excavating equipment. Conventional excavation is very economical where the pond has a possibility and capacity to drain. The pond is drained and sediments are left exposed for drying naturally or suitable methods are adopted for removal of moisture contents in addition to a natural drying process. Conventional excavation is very cost effective where noise, odour or aesthetics is not a sensitive issue. This is the most desirable option because of the ease of operation and cost implication. The down side is that a longer duration is required for drying of sediments and non-availability of the SWMF for its desired function during that period.

Hydraulic Excavation

Hydraulic dredging is the best way to remove sediments without draining the SWMFs. One (1) option available includes auger dredging with an adjustable mud shield. These auger dredges are available in different sizes and are suitable for large volumes over longer distances. Dredges can be manoeuvred in the water by means of propellers, cables, or mechanical legs.

The auger dredge works by having an auger-like device cut into the sediments while, at the same time, a powerful pump is used to extract the resulting mixture of sediment and pond water. This changes the nature of the sediments from being semi-solid to be a slurry or “dredgeate”. The advantage of this situation is the slurry / dredgeate is relatively easy to pump from the floating dredge to a land based processing facility. The disadvantage is that extra water is required to create pumpable slurry and it must be removed before it is hauled away for disposal or reuse.

Sediment Dewatering

In order to determine a suitable dewatering site several aspects need to be investigated including:

- Proximity to the pond;
- Distance to the majority of dredging activities;
- Accessibility of large trucks;
- Adequacy of existing features and the magnitude of upgrading which would be required to provide all features needed for the dewatering equipment;
- Availability of power, and,
- Impact on land based environment and park users.

Settling Ponds / Conventional Drying Beds

Settling ponds offer the most cost effective and least noisy method available for the reduction of moisture content from the sediments. Disadvantages when using settling ponds include the duration, odour and aesthetics. Other options include conventional drying bed method. Available area for construction of drying ponds or bed is also an important factor to be considered for the selection of this method.

Geo-Synthetic Filter Bags

Geo-Synthetic Filter Bags are large bags made of very strong geo-fabric with maximum 500 microns filter opening. The dredgeate / slurry treated with flocculating compound is pumped into the filter bag. The fine sediments flocculate together and settle in the bag. Centrate is separated from dredgeate and filtered out of the filter bag. A constant head is maintained for expulsion of water from the filter bag (disposal of the centrate is a separate issue). The slurry is constantly pumped into the bag to keep a positive pressure on the escaping fluid. The bag is left for several days before more slurry is pumped into the bag. This process is repeated until the bag is full of sediment. The sediment filled bag is pierced and the sediments are disposed off at site or off site using conventional methods.

This method can be slow and the drying time is dependent upon the physical properties of the sediments, weather, surface area exposed to atmosphere, flocculating agent used, and area available for the filter bags (more filter bags means less dewatering time).

Mechanical Dewatering

These processes are already in use at waste water treatment facilities for the dewatering of sludge. The commonly used equipment is a plate and frame filter press, belt filter press and centrifuge.

The plate and frame filter press is a batch process, removal of moisture content is highest, dry solid content are in the range of 30 % to 52 %, disadvantage is intermittent operation and low production. The belt filter press is a labour intensive continuous operation. The resultant solid contents are up to 25 % and the production is low.

The centrifuge has an advantage of continuous operation with a high production rate. In water treatment and wastewater treatment industry, centrifuges are common at large treatment plants where high production rates, cleanliness and odour control (easier because centrifuge is mostly closed) are important. Centrifuges are most cost effective (\$/ton processed) than any other process described above. The disadvantages of mechanical dewatering are cost, noise and power requirements.

Centrate Handling

It is anticipated if mechanical dewatering is selected; polymers will be used to accelerate the flocculation process. Once dredgeate is separated into residual solid content and centrate, some form of "polymer quenching" will be required before the water is returned to the pond.

Tender Documents

Tender documents should include all forms of tender, contract price schedules and contractual schedules such as, "General and Supplementary Conditions", "Special Provisions". It should address project specific items such as:

- Location plan;
- Site access routes;
- Construction area restriction plans showing staging and storage areas
- Erosion and silt protection;
- Excavation volumes;
- Bathymetric surveys;
- Existing and proposed typical cross-sections,

- Sediment quality;
- Dewatering techniques;
- Disposal methodologies;
- Material specifications and sundry construction;
- Retrofit / repairs to the existing components of the pond structure; and,
- Rehabilitation of eroded banks.

Construction methodology plans and details:

- Erosion and sediment control plans and project schedule and sequencing details;
- Site restoration plans (including seeding / sodding, erosion control matting, etc.);
- Landscaping (including bioengineering and habitat features) plans and details;
- Restoration Plans, Details and Specifications for hard and soft site elements;
- Detailed Cost Estimate, this includes for each pond site the accessibility of site, location of disposal, volume of sediment and quality of sediment;
- Flow bypass measures for period with rainfall;
- Short and long term monitoring requirement;
- Operations and Maintenance Manual; and,
- Detailed Design Report (including all documentation of calculations and recommendations).

7.6.3. Construction Supervision and Contract Administration

Tender and Award

The Tender and award process may involve Pre-Tender bidders meetings, responding to questions, preparing addenda, and contractor selection. Evaluation of construction bids should consider the ability of the contractor to complete the work within the specified timeframe, the cost, and the contractor's experience with pond projects and innovative or value added knowledge or technologies offered.

Pre-Construction

A Pre-Construction meeting should be held on-site to identify site specific constraints, work areas, staging areas and no disturbance areas. A detailed Pre-Construction survey, including digital pictures and videos, should be completed to document existing conditions. A Health and Safety audit should be conducted, with the results of the survey and audit should be included in the Pre-Construction Report.

Contract Administration and Construction Inspection Services

After a construction phasing and staging plan has been developed and a qualified contractor retained, it is important to develop and maintain a working construction scheduling document to track the progress of the construction and ensure that possible delays are minimized or avoided entirely. Environmental monitoring also plays an important role in managing risk to the Town during the construction period.

Sediment removal projects are often high public profile projects and communication between the field staff, contract administrators, project manager and the Town is critical. Periodic formal environmental inspections will occur during landscaping and at other times throughout the construction stages. Various specialists including a site inspector, environmental inspector and ecological specialists need to work together during the different stages of the program. Inspection services should include:

Contract administration and site inspections should include:

- Scheduling, co-ordinating and attending project review meetings including a pre-construction review meeting;
- Regular on-site inspections which include reporting;
- Liaising with the contractor and evaluating any out-of-scope requests;
- Ensuring that all environmental protection measures are acceptable and functioning as designed and identifying deficiencies to be corrected by the contractor;
- Maintaining a daily diary of construction events documenting the progress of the work and to substantiate the quality and quantity of the work performed;
- Providing comments to the contractor's proposed procedures, methods and construction phasing to ensure compliance with design and contract requirements;
- Managing claims, notices of intent to claim, disputes and questions relating to contractor performance, quality of the work and interpretation of the contract documents;
- Addressing inquiries and request for information from external Agencies, adjacent landowners and members of the public; and,
- Reviewing and processing payment certification of Substantial and Total Performance for *Construction Lien Act* purposes.

Record Drawings and Project Completion

During the close-out period, close attention must be paid to the project documentation. In particular, any deficiencies must be identified, defined and the contractor notified. The deficiency must ultimately be rectified prior to the release of securities or other bonds supplied by the contractor. A final thorough review of the contract documents and the constructed works is required at this time. As-built conditions will be surveyed and design drawings updated to reflect the current conditions.

7.7. Maintenance

A summary of maintenance tasks for all ponds with at least one component ranking 3, 4 or 5 is provided below in **Table 7-2**.

Table 7-2 – Summary of Maintenance per Pond with Associated Cost

Pond ID	Component	Condition	Maintenance Task	Est. Cost
01	Facility	3	Prepare plan and remove sediment	\$120,000.00
	Outlet	3	Fill in Erosion	\$250.00
02	Grate	3	Refasten grating to headwall	\$425.00
03	Inlet	3	Clean up debris and vegetation	\$400.00
05	Facility	3	Remove debris	\$800.00
	Grate Outlet	3	Clean up debris and vegetation	\$400.00
06	Vegetation	3	Remove / Replace dead vegetation	\$800.00
	Culvert	3	Cover Graffiti	\$500.00
07	Grate	3	Grate cleanup	\$200.00
	Outlet	4	Clean up debris and vegetation	\$900.00

Pond ID	Component	Condition	Maintenance Task	Est. Cost
	Vegetation	3	Remove / Replace dead vegetation	\$400.00
08	Facility	3	Prepare plan and remove sediment	\$213,100.00
	Inlet	3	Add a grate to structure	\$2,500.00
	Vegetation	3	Remove / Replace dead vegetation	\$400.00
10	Grate	3	Lock grate	\$200.00
	Facility	3	Prepare plan and remove sediment	\$199,800.00
11	Vegetation	3	Remove / Replace dead vegetation	\$4,000.00
	Fence	3	Repair broken links and fence supports	\$400.00
	Headwall	3	Paint over graffiti	\$500.00
	Fence	3	Repair broken links	\$425.00
	Fence	3	Repair broken links and fence supports	\$400.00
	Intermediate Quantity Inlet	3	Clean up debris and vegetation	\$400.00
12	Sanitary Manhole	3	Investigate location	\$200.00
14	Grate	3	Refasten grating to headwall	\$400.00
15	Fence	3	Repair broken links and fence supports	\$425.00
16	Facility	3	Continue Trapping Efforts	\$1,000.00
	Manhole	3	Clean Debris from manhole	\$3,000.00
	Inlet 3	3	Clean up debris and vegetation	\$400.00
17	Facility	3	Prepare plan and remove sediment	\$44,500.00
18	Facility	3	Remove debris	\$400.00
	Outfall	3	Clear Debris	\$400.00
19	Quality Outlet	3	Repair structure.	\$3,000.00
	Vegetation	3	Remove / Replace dead vegetation	\$800.00
20	Inlet	3	Fill in erosion.	\$500.00
21	Facility	3	Perform spring inspection	\$250.00
	Headwall	3	Clear Graffiti	\$450.00
	Inlet	3	Cover graffiti.	\$450.00
	Outlet	3	Clear Structures of any debris	\$400.00
	Outlet Manhole	3	Clear Vegetation	\$3,000.00
22	Inlet	3	Sample water to determine quality.	\$700.00
23	Gate	3	Replace lock on structure.	\$225.00
24	Pipe	4	Clean up debris, sediment and vegetation	\$800.00
25	Vegetation	3	Remove / Replace dead vegetation	\$800.00
26	Vegetation	3	Remove / Replace dead vegetation	\$800.00
	Headwall	3	Paint over graffiti	\$500.00

Condition rankings are further explained in **Section 7.4.3**. The cost associated with the maintenance required by each pond has been generated by SWMSOft based on the resource cost listed above in **Table 7-2**. It is noted that the cost associated with the above noted improvements are to be confirmed by the Town and may be updated within the SWMSOft software. The SWMSOft Inspection Report and Bathymetric Surveys is included in **Appendix L**.

7.7.1. Potential Improvements

This section discusses the common findings and recommendations across the SWMFs.

Unlocked Grates and Hatches

A large number of grates covering inlet and outlet pipes and hatches are not secure and pose a safety hazard. These grates should either be locked or bolted shut to prevent entry and remove the safety hazard. Bolting grates shut is the preferred method of securing the grates to avoid the need to manage the keys to all of the locks at different facilities.

Graffiti

Graffiti is a common aesthetic issue at SWMFs, and is most commonly found on concrete surfaces such as the headwalls of inlets and outlets. The recommended solution is to paint over the graffiti with grey paint, which is cost effective and obscures the graffiti. It is also possible to sand blast the concrete to remove the graffiti, however this would be more costly and may require the notification of homeowners in the surrounding area because of the noise and dust that would be created during the removal.

Litter

Litter is another common problem at SWMFs, and it can affect the facility's water quality as well as its aesthetics. The recommended solution is to dispatch Town work crews to remove the debris. In addition, the Town may consider placing additional garbage cans at the locations where litter is a recurring problem.

Vegetation

Overgrown vegetation at the inlet and outlets is a common problem at SWMFs, and can compromise the facility's function in controlling water levels. The recommended solution is to remove the overgrowth using hand tools.

Dead vegetation presents a problem in terms of aesthetics. It also poses as a possible safety risk if the tree is in danger of falling, or acts as a potential tripping hazard. Dead vegetation that presents a hazard should be removed and the area should be revegetated.

Beaver Damage

Several SWMFs show evidence of beaver damage. The presence of a beaver in the vicinity of a SWMF is a concern because the beaver may cause damage to surrounding vegetation or construct a dam that inhibits the proper function of the SWMF. Possible solutions to this problem are to either remove the beaver from the facility, or to place barriers that prevent the beaver from obstructing the flow of water.

The recommended solution is to hire a trapper licensed by the Ontario Ministry of Natural Resources (OMNR) to remove the beaver because the costs associated with installing and maintaining the barriers is greater than the cost to remove the beaver. Once the beaver is removed, prevention measures should be implemented to discourage beavers from inhabiting the facility in the future (Town of Richmond Hill, 2010).

Beaver dam removal is generally done using hand tools or equipment such as backhoes after the beaver is removed. The removal of the dam must be done carefully as it can negatively impact fish and fish habitat by decreasing the water level upstream of the dam, as well as releasing sediment and large volumes of water downstream (Fisheries and Oceans Canada 2010). Removal practices must follow the Fisheries and Oceans Canada – Ontario Operational Statement (2010) and avoid contravention with subsection 35(1) of the *Fisheries Act*: “No person shall carry on any work or undertaking that results in the harmful alteration, disruption or destruction of fish habitat”.

If the conditions of the Operational Statement for beaver dam removal cannot be met, then the LSRCA should be contacted to discuss removal options to remain in compliance with the *Fisheries Act*.

Algae Blooms

Algae blooms, or rapid and excessive growth of algae, commonly occur in ponds. Algae blooms occur when conditions are extremely favourable to the growth of algal populations; these conditions include excess nutrients, low dissolved oxygen levels, and an abundance of sunlight. As algal blooms decompose, they remove a large amount of dissolved oxygen from the pond. The lack of oxygen can result in damage to the pond’s ecosystem, and the decline of biota in the pond. Algal blooms are also unsightly, can cause foul odours, and negatively affect the water quality of the ponds (Clark 2008).

There are a number of different ways to control the algae population of a pond and prevent algae blooms from forming.

The most common methods involve killing the algae, limiting the amount of sunlight that enters the pond, and increasing the oxygen levels of the pond (Lynch 2009). Increasing the dissolved oxygen content in the pond is an effective way to control algae populations by addressing the underlying cause, and can be achieved by placing aeration equipment in the pond. The increased oxygen levels prevent algae growth, and lessen the impact of algae decomposition on the surrounding ecosystem (Clark, 2008).

The recommended solution is to install a sub-surface aeration system in the ponds affected by algal blooms. Aeration systems are relatively easy to maintain and will effectively control the algae population in a pond. The aeration system would have to be wind powered, because of the limited or no access to power at most pond locations. Wind powered aeration systems are widely available and commonly used to aerate ponds.

7.8. Costs

7.8.1. Regular Maintenance

There are two (2) main categories of regular maintenance:

- Debris / vegetation removal; and,

- Minor repair.

Most of the debris and vegetation removal centres on the inlet and outlet structures and require half a day's worth of work for a crew of two (2) people. This has been assumed to cost approximately \$400.00 for Town operation staff to complete. Minor repairs generally involve repairing baffle blocks, replacing grates in front of pipes or mending fences. These tasks were budgeted to take between one (1) and three (3) days, depending on the severity of the damage. Such items were estimated to cost between \$1000.00 and \$4000.00 each. For unassumed ponds, a cost of \$200.00 was assigned to the facility to account for discussion for the coordination of repair efforts with the developers. It should be noted that a number of assumptions were made on the costs of improvements. The Town is required to update the cost summary as a function of SWMSOft before costing and implementing the improvements.

7.8.2. Sediment Removal

The cost of removing sediment depends on:

- Accessibility of the site;
- Location for disposal;
- Volume of sediment; and,
- Quality of sediment.

The cost for removing sediment can be low, approximately \$100.00/m³ if the site is easily accessible, the sediment can be disposed of locally, and an analysis of sediment quality passes the Guidelines for Contaminated Sites. However, as some municipalities have experienced, the cost can increase more than 30-fold, if the sediment does not pass the guidelines and must be disposed of in a registered landfill. In addition, costs for sediment removal are typically based on a weight or mass of material basis rather than a volume of material basis, and the density of sediments may vary. Note that measurements obtained in this study correspond to in-situ volume measurements under saturated conditions. As such, the volume-based cost estimates include an intrinsic factor of safety.

The initial observations of the facilities deemed suitable for sediment removal did not reveal any problems that would cause removal costs to increase by an appreciable amount.

Preliminary estimates therefore determine the approximate cost of removal by multiplying the volume of accumulated sediment by \$100.00. An additional amount of \$25,000.00 is applied to all sediment removal costs to cover the expense of preparing a sediment removal plan. The plan includes site and equipment selection, permits, transportation, and sampling.

7.8.3. Cost Summary

The cost that is associated with the maintenance of each pond outlined in **Table 7-3** has been calculated by SWMSOft. The costs are based estimated amounts for each task to be completed along with the cost of the crew and hours to complete the task. These amounts have been entered in by Cole Engineering but can be adjusted by the Town. A full description of the amounts associated with each task is outlined in below in **Table 7-3**.

Table 7-3 – Cost Summary of SWMSoft Resource

Resource	Description	Cost per Unit*	Unit
General Inspector	City staff to perform seasonal inspections.	250	Use
Fencing	Black vinyl covered fence	5	Foot
Riprap	8 inch stone used around inlet / outlet	150	Cu. Metre
Weeping tile	Weeping tile with sock	15	Metre
General Contractor	Non-specific contractor to do simple repairs outside scope of city labourers	1	Dollar
General Consultant	Non-specific consultant to assess various problems outside of city expertise	1	Dollar
City Official	Non-specific city employee to consult with local residents	1	Dollar
2 Person Crew	2 Person City Work Crew (8 Hours)	800	Day
3 Person Contract Crew	3 Person Contract Work Crew (10 hours)	3000	Day
Site Inspector	Performs advanced inspections	600	Use
Water Sampling	Generic company for testing water samples	700	Use
Front-end Loader	Standard Front-end loader for use with 2 Person City Work Crew	800	Day
Miscellaneous	Undefined materials to be allocated to task	1	Dollar
Engineer	City Engineer	50	Hour
Lock	A padlock	25	Use
Paint	Paint	50	Gallon
Soil	Soil to use as fill	10	Cu. Ft.
Erosion Fabric	Erosion fabric	10	Metre
Grass Seed	Grass seed	50	Kilogram
Fence Repair Contractor	A contractor to perform structural repairs on fences	250	Use
Buoy Rope	Rope to attach the buoy to the emergency station, as well as safety rope around the buoy	50	Use
Plastic Pipe Cap	A cap to cover plastic riser pipes	50	Use
Buoy	Red floating buoy	50	Use
Hazard Sign	A sign	100	Use
Pest Control [Beaver]	A pest control company	1000	Use
Plastic Pipe	Plastic piping	10	Foot
Hoop	Emergency equipment hoop	100	Use
Pest control	Removal of problem animals	1	Dollar
Grate	A grate to cover a large outlet pipe	1000	Use
Aeration Equipment	A sub-surface aerator powered by a windmill	2500	Use

* The unit costs listed are an estimated cost for the resources. It is recommended that the Town update the unit rates.

8.0 Development of Long List of Alternatives

8.1. General

In order to identify the solution that best encompasses the study's principles, goals and objectives a long list of alternatives was generated. The alternatives consider both existing and future land uses within the Town that consists of rural and urban areas.

The approach for developing and evaluating alternatives is consistent with the requirements of the planning and design process for Master Planning projects described in the Municipal Class EA (Municipal Engineers Association, June, 2000). It involves reviewing Phase One work (i.e. Identification of the Problem) and undertaking Phase Two (i.e. Establishing Existing Conditions, Identification of Long List of Alternatives, Development and Assessment of Alternative Management Strategies and Selection of a Preferred Strategy). In addition, consultation with stakeholders is a necessary step in this process.

8.2. Overview of Alternatives

A range of alternative solutions was developed in order to address the identified problem and opportunity. The alternatives identified for evaluation are summarized in the following categories:

- Source Controls;
- Conveyance Controls;
- End of Pipe Measures; and,
- Restoration Measures.

Even though the "Do Nothing" alternative does not address the Problem / Opportunity Statement, the Class EA document mandates its consideration in all Class EAs as a means of providing a benchmark for evaluating the other alternative solutions.

The following subsections briefly describe each of these alternative solutions.

8.2.1. Source Controls

This alternative involves physical measures that are located at the beginning of the drainage system; generally on private property. Source controls will reduce stormwater runoff generated from impervious surfaces that mostly occur in urban areas. Source controls can be used in a variety of different land uses such as residential, industrial and commercial. Source controls can be retrofitted into existing areas and implemented into proposed urban areas. Source control measures include:

Roof Downspout Disconnection

Simple downspout disconnection involves directing flow from roof downspouts to a pervious area that drains away from the building. This prevents stormwater from directly entering the storm sewer system or flowing across a "connected impervious surface, such as a driveway, that drains to a storm sewer. Simple downspout disconnection requires a minimum flow path length across the pervious area of 5 metres.

When the infiltration rate of the soil in the pervious area is less than 15 mm/hr (i.e. hydraulic conductivity of less than 1×10^{-6} cm/s), the area should be tilled to a depth of 300 mm and amended with compost to achieve an organic content in the range of 8 to 15% by weight or 30 to 40% by volume.



Bioretention

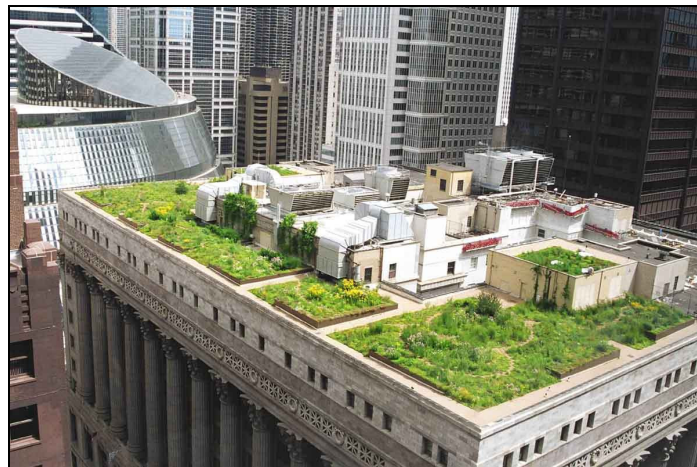
As a stormwater filter and infiltration practice, bioretention temporarily stores, treats and infiltrates runoff. Depending on native soil infiltration rate and physical constraints, the system may be designed without an underdrain for full infiltration, with an underdrain for partial infiltration, or with an impermeable liner and underdrain for filtration only, which can also be referred to as a biofilter. The primary component of a bioretention practice is the filter bed which is a mixture of sand, fines and organic material. Other important elements of bioretention include a mulch ground cover and plants adapted to the conditions of stormwater practice. Pretreatment, such as a settling forebay, vegetated filter strip, or stone diaphragm, often precedes the bioretention to remove particles that would otherwise clog the filter bed. Bioretention is designed to capture small storm events or the water quality storage requirement. An overflow bypass is necessary to pass large storm events.

Bioretention can be adapted to fit into many different development contexts and provides a convenient area for snow storage and treatment. In a low density development, it might have a soft edge and gentle slopes, while a high density application might have a hard edge with vertical slopes.



Green Roofs (i.e. Roof Gardens)

Green roofs, also known as “living roofs” or “rooftop gardens”, consist of a thin layer of vegetation and growing medium installed on top of a conventional flat or sloped roof. Green roofs are publicized for their benefits to cities, as they improve energy efficiency, reduce urban heat island effects, and create greenspace for passive recreation or aesthetic enjoyment. To a water resources manager, they are attractive for their water quality, water balance, and peak flow control benefits. From a hydrologic perspective, the green roof acts like a lawn or meadow by storing rainwater in the growing medium and ponding areas. Excess rainfall enters underdrains and overflow points and is conveyed in the building drainage system. After the storm, a large portion of the stored water is evapotranspired by the plants, evaporates or slowly drains away.



Soakaway Pits, Infiltration Trenches and Chambers

On sites suitable for underground stormwater infiltration practices, there are a variety of facility design options to consider, such as soakaway pits, infiltration trenches and infiltration chambers.

Soakaway pits are rectangular or circular excavations lined with geotextile fabric and filled with clean granular stone or other void forming material, which receives runoff from a perforated pipe inlet and allow it to infiltrate into the native soil. They typically service individual lots and receive only roof and walkway runoff but can also be designed to receive overflows from rainwater harvesting systems. Soakaways can also be referred to as infiltration galleries, dry wells or soakaway pits.

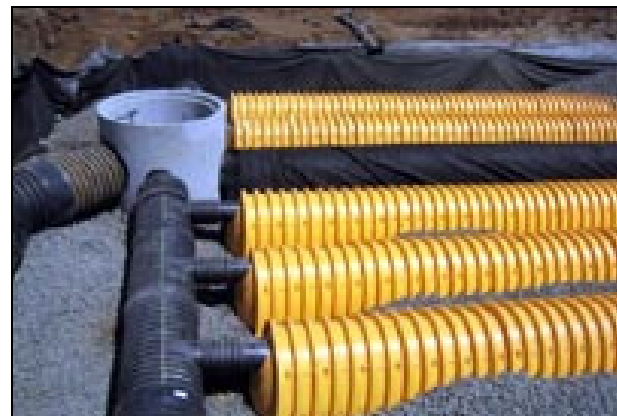


Infiltration trenches are rectangular trenches lined with geotextile fabric and filled with clean granular stone or other void forming material. Like soakaways, they typically service an individual lot and receive only roof and walkway runoff. This design variation on soakaways is well suited to sites where available space for infiltration is limited to narrow strips of land between buildings or properties, or along road rights-of-way. They can also be referred to as infiltration galleries or linear soakaways.



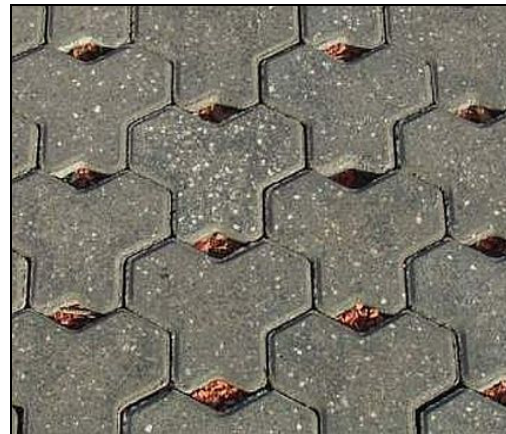
Infiltration chambers are another design variation on soakaways.

They include a range of proprietary manufactured modular structures installed underground, typically under parking or landscaped areas that create large void spaces for temporary storage of stormwater runoff and allow it to infiltrate into the underlying native soil. Structures typically have open bottoms, perforated side walls and optional underlying granular stone reservoirs. They can be installed individually or in series in trench or bed configurations. They can infiltrate roof, walkway, parking lot and road runoff with adequate pretreatment. Due to the large volume of underground void space they create in comparison to a soakaway of the same dimensions, and the modular nature of their design, they are well suited to sites where available space for other types of BMPs is limited, or where it is desirable for the facility to have little or no surface footprint (e.g., high density development contexts). They can also be referred to as infiltration tanks.



Permeable Pavement

Permeable pavements, an alternative to traditional impervious pavement, allow stormwater to drain through them and into a stone reservoir where it is infiltrated into the underlying native soil or temporarily detained. They can be used for low traffic roads, parking lots, driveways, pedestrian plazas and walkways. Permeable pavement is ideal for sites with limited space for other surface stormwater BMPs. Depending on the native soils and physical constraints, the system may be designed with no underdrain for full infiltration, with an underdrain for partial infiltration, or with an impermeable liner and underdrain for a no infiltration or detention and filtration only practice. Permeable paving allows for filtration, storage, or infiltration of runoff, and can reduce or eliminate surface stormwater flows compared to traditional impervious paving surfaces like concrete and asphalt.



Rainwater Harvesting (i.e. Rain Barrels, Cisterns, etc.)

Rainwater harvesting is the process of intercepting, conveying and storing rainfall for future use. Harvesting rainwater for domestic uses has been practical in rural Ontario for well over a century. Interest in adapting this practice to urban areas is increasing as it provides combined benefits of conserving potable water and reducing stormwater runoff. When harvested rainwater is used to irrigate landscaped areas, the water is either evapotranspirated by vegetation or infiltrated into the soil, thereby helping to maintain predevelopment water balance.



8.2.2. Conveyance Controls

This alternative involves controlling stormwater as it travels along the drainage system. The Town contains a variety of existing drainage paths that would lead to retrofitting opportunities or with evaluation, the opportunity for new drainage paths. The goal of this alternative is to reduce erosion and storm water runoff. Conveyance controls can consist of swales, ditches, culverts, catch basins, manholes, and storm sewers. Some conveyance control measures include:

Grassed Swales

Grassed swales are vegetated open channels designed to convey, treat and attenuate stormwater runoff (also referred to as enhanced vegetated swales). Check dams and vegetation in the swale slows the water to allow sedimentation, filtration through the root zone and soil matrix, evapotranspiration, and infiltration into the underlying native soil. Simple grassed channels or ditches have long been used for stormwater conveyance, particularly for roadway drainage.

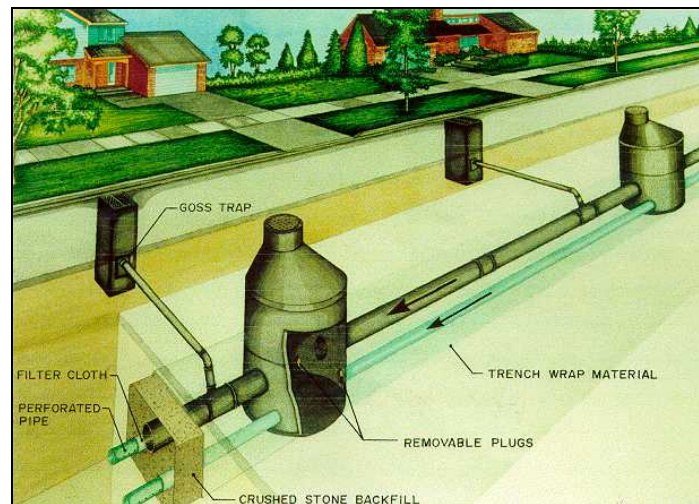
Grassed swales incorporate design features such as modified geometry and check dams that improve the contaminant removal and runoff reduction functions of simple grassed channel and roadside ditch designs. A dry swale is a design variation that incorporates an engineered soil media bed and optional perforated pipe underdrain system. Grassed swales are not capable of providing the same water balance and water quality benefits as dry swales, as they lack the engineered soil media and storage capacity of that best management practice.



Perforated Pipe Systems

Perforated pipe systems can be thought of as long infiltration trenches or linear soakaway pits that are designed for both conveyance and infiltration of stormwater runoff. They are underground stormwater conveyance systems designed to attenuate runoff volume and thereby, reduce contaminant loads to receiving waters. They are composed of perforated pipes installed in gently sloping granular stone beds that are lined with geotextile fabric that allow infiltration of runoff into the gravel bed and underlying native soil while it is being conveyed from source areas or other BMPs to an end-of-pipe facility or receiving waterbody. Perforated pipe systems can be used in place of conventional storm sewer pipes where topography, water table depth, and runoff quality conditions are suitable.

They are suitable for treating runoff from roofs, walkways, parking lots and low to medium traffic roads, with adequate pretreatment. A design variation can include perforated catchbasins, where the catchbasin sump is perforated to allow runoff to infiltrate into the underlying native soil. Perforated pipe systems can also be referred to as pervious pipe systems, exfiltration systems, clean water collector systems and percolation drainage systems.



Vegetated Filter Strips

Vegetated filter strips (i.e. buffer strips and grassed filter strips) are gently sloping, densely vegetated areas that treat runoff as sheet flow from adjacent impervious areas. They function by slowing runoff velocity and filtering out suspended sediment and associated pollutants, and by providing some infiltration into underlying soils. Originally used as an agricultural treatment practice, filter strips have evolved into an urban SWM practice. Vegetation may be comprised of a variety of trees, shrubs and native plants to add aesthetic value as well as water quality benefits. With proper design and maintenance, filter strips can provide relatively high pollutant removal. Maintaining sheet flow into the filter strip through the use of a level spreading device is essential.



8.2.3. End of Pipe Measures

This alternative consists of managing stormwater at the end of a storm sewer system prior to discharging to a stream.

They usually consist of man-made measures such as stormwater ponds or constructed wetlands. This alternative would look at opportunities for retrofit of existing ponds, or if there is a need for more. End of pipe measures include Best Management Practices.

SWM Ponds (i.e. Wet Ponds)

Wet ponds are the most common end-of-pipe SWMF employed in Ontario. They are less land-intensive than wetland systems and are normally reliable in operation, especially during adverse conditions (e.g., winter / spring).

This reliability can be attributed to:

- Performance does not depend on soil characteristics;
- The permanent pool minimizes re-suspension;
- The permanent pool minimizes blockage of the outlet;
- Biological removal of pollutants occurs (including phosphorus); and,
- The permanent pool provides extended settling.

Wet ponds can be designed to efficiently provide for water quality, erosion and quantity control, reducing the need for multiple end-of-pipe facilities. Wet ponds can be designed with extensive landscaping and associated recreational amenities, contributing to the character of the community and enhancing its marketability.



Dry Ponds

Dry ponds have no permanent pool of water. As such, while they can be effectively used for erosion control and flood control, the removal of stormwater contaminants in these facilities is purely a function of the detention time in the pond. For a 24 hour retention period, this normally means a lower contaminant removal (the inter-event settling time does not exist). While achieving this for smaller drainage areas can be difficult (because of orifice size considerations), the use of dry ponds in larger catchments (especially in retrofit situations) may have greater potential than had previously been thought. There are no documented performance monitoring data for dry ponds with longer detention times, however, and re-suspension of settled material remains a concern. As such, the use of dry ponds (for water quality control) remains largely restricted to retrofits, where temperature is an overriding concern, and situations where other more effective SWMP types are infeasible. Dry ponds may be used as part of an overall treatment train approach.



Constructed Wetlands

The constructed wetland is one of the preferred end-of-pipe SWM facilities for water quality enhancement. Wetlands are normally more land-intensive than wet ponds because of their shallower depth (both in the permanent pool and in the active storage zone). They are suitable for providing the storage needed for erosion control purposes, but will generally be limited in their quantity (i.e. flood) control role because of the restrictions on active storage depth.

Benefits of constructed wetlands are similar to wet ponds and include:

- Performance does not depend on soil characteristics;
- The permanent pool minimizes re-suspension;
- The permanent pool minimizes blockage of the outlet;
- Biological removal of pollutants occurs (including phosphorus); and,
- The permanent pool provides extended settling.



8.2.4. Restoration Measures

This would include the restoration or enhancement of existing habitats, both rural and urban within the Town. Stream Restoration Programs, Aquatic Habitat Enhancement and Terrestrial Habitat Enhancement are examples of restoration measures that attenuate erosion and promote infiltration.



8.2.5. Rural Measures

East Gwillimbury has a large percentage of land dedicated to rural uses. This alternative is simply Best Management Practices aimed for rural properties. There are structural based and non-structural based practices.

Structural Best Management Practices: Aimed to reduce runoff and pollution from agricultural practices this would include proper manure storage and handling, feedlot runoff controls, and internal wetland systems to control feedlot runoff and milkhouse wastes.



Non-Structural Best Management Practices for Rural Areas: Includes livestock fencing, buffer strips, conservation tillage and nutrient management. These measures would address non-point pollution sources.



9.0 Development and Assessment of Alternative Solutions

9.1 Identification / Description of Alternative Solutions

As an initial step, the Project Team identified and described alternative solutions, or functionally different ways of addressing the problem / opportunity statement, as described in **Section 1.6**. Any “reasonable” alternative was included initially. All alternatives were considered equally for discussion purposes and evaluation as seen in **Section 10.0**.

9.2. General

The development of Alternative SWM Strategies is necessary in order to determine the effectiveness of each strategy with respect to protecting, enhancing and restoring the natural resources of the watersheds located within the Town under existing and planned land use changes.

For the purposes of the SWMMP, a SWM Strategy was defined as a set of BMPs which, when implemented collectively, will attempt to address impacts associated with change in land uses within the watersheds. The land uses under consideration include existing urban and rural land uses and proposed development within the settlement areas.

The assessment was undertaken using results from the modelling as well as taking into account various factors such as social, economic and environmental criteria which are defined to further develop specific components of the Alternative Strategies. In addition, the assessment will look to find an alternative which minimizes, to the extent possible, the impact on the community, natural environment and the economy.

The approach used for developing and evaluating alternatives is, where appropriate, consistent with the planning and design process for Master Planning projects as described in the Class EA document. The approach has been used for measures which are located outside proposed development areas (i.e. stream restoration works, stormwater pond retrofit works).

For other measures, such as the construction of SWM measures for proposed developments, or the implementation of a Town wide program (i.e. disconnection of roof downspouts) general direction as to the types of measure, or proposed programs will be provided in each alternative.

9.3. Development of Alternative SWM Strategies

The Town is comprised of mainly rural area. The Protected Countryside, which includes the Oak Ridges Moraine and Greenbelt Plan, represents approximately 75% of the Town's land area.

Results from the Existing Conditions as assessed in **Section 3.0** indicate the existing environmental conditions are not meeting the requirements as set out in the Lake Simcoe Protection Plan.

The assessment of existing conditions suggest that the Alternative Management Strategies, if they are to be affective, must deal with impacts associated with the existing urban and rural land uses as well as proposed land uses. In addition, the problem statement identifies the need to improve the SWM within existing developments as well as planned. The strategies must also incorporate the principles of the Lake Simcoe Protection Plan, specifically Section 4.8-DP and the requirements regarding phosphorus removal and mitigation of changes in water balance.

A total of five (5) Alternative Management Strategies have been brought forward for assessment. The five strategies are defined as:

1. Do Nothing;
2. Traditional SWM Strategy;
3. Traditional SWM with BMP Implementation Strategy;

4. Traditional SWM with Urban Retrofits Strategy; and,
5. Traditional SWM with Rural Retrofits Strategy.

Table 9-1 below has been developed to summarize the potential effects of each alternative with respect to flow volume, phosphorus loadings, infiltration reduction and erosion potential. These impacts would occur within the Town as a result of the proposed land use change.

9.3.1. Do Nothing

The “Do Nothing” alternative would involve leaving the Town as is. No SWM works are carried out in any of the existing urban or rural lands or within any proposed development or redevelopment areas. An assessment to the impacts associated with the implementation of this strategy is required as part of undertaking a Municipal Class Environmental Assessment Study.

As seen in **Table 9-1**, this strategy would result in higher flow volumes, increased flooding, increase production of phosphorus, an increase in erosion potential and infiltration reduction.

9.3.2. Traditional SWM Strategy

The Traditional SWM Strategy is the implementation of SWM in proposed development or redevelopment areas. The SWM works would consist of the construction of SWM ponds for quantity, quality and erosion control as per MOE guidelines demonstrate.

Proposed developments would be serviced by conventional storm sewer systems and limited source control measures would be implemented such as downspout disconnection.

This alternative would result in higher flow volumes, an increase in erosion potential, and an increase in production of phosphorus and infiltration reduction.

9.3.3. Traditional SWM with BMP Implementation Strategy

This Strategy is consistent with the Traditional SWM Strategy as it will apply to proposed development and redevelopment areas. A variety of BMP source controls, conveyance controls and end-of-pipe measures will be implemented for all proposed areas of development. Alternative developments (i.e. LEED certified developments, Low Impact Developments) would also be considered.

A series of assumptions for the types of Best Management Practices to be implemented with this Strategy were made in order to include this Strategy as an Alternative. The types of BMP measures to be implemented are further explained in **Section 8.2**.

9.3.4. Traditional SWM with Urban Retrofits Strategy

This strategy is consistent with the Traditional SWM Strategy in that conventional SWM strategies would be implemented within proposed development or redevelopment areas. In addition to traditional SWM, a variety of source, conveyance and end-of-pipe measures will be implemented in existing urban areas.

This strategy is highly based on the amount of participating landowners to implement the proposed retrofits at the source level. The types of urban retrofits to be implemented in this strategy are further explained in **Section 8.2**.

9.3.5. Traditional SWM with Rural Retrofits Strategy

This strategy is consistent with the Traditional SWM Strategy in that conventional SWM strategies would be implemented within proposed development or redevelopment areas. In addition to traditional SWM, a variety of source, conveyance and end-of-pipe measures will be implemented in existing rural areas.

Similar to the Traditional SWM with Urban Retrofit Strategy, rural retrofit implementation is also based on the amount of participating rural landowners to implement the proposed retrofits at the source level. The types of rural retrofits to be implemented in this strategy are further explained above in **Section 8.2.5**.

Table 9-1 – SWM Strategy Alternatives

Preliminary Criteria	SWM Strategy Alternatives				
	Do Nothing	Traditional	Traditional w / BMPs	Traditional w / Urban Retrofits	Traditional w / Rural Retrofits
Land Uses where Strategy is to be Applied	None	Proposed Development and Redevelopment Areas	Proposed Development and Redevelopment Areas	Proposed Development and Redevelopment Areas and Existing Urban Areas	Proposed Development and Redevelopment Areas and Existing Rural Areas
Flows	Increase in Runoff	Increase in Flow Volume	Reduction in Flow Volume	Reduction in Urban Areas	Reduction in Rural Areas
Phosphorus Loading	Current Condition	Increase	Reduction	Reduction in Urban Areas	Current Condition in Urban Areas Reduction in Rural Areas
Infiltration	Current Condition	Reduction	Increase	Increase in Urban Areas	Current Condition
Erosion Potential	Current Condition	Increase	Reduction	Reduction in Urban Areas	Current Condition

9.4. Development of Evaluation Categories and Criteria

Evaluation criteria were developed to reflect the definition of “environment” provided in the OEAA and the specific circumstances associated with this project. Criteria that were considered were divided into categories, as listed below in **Table 9-2**.

Table 9-2 – Preliminary Evaluation Criteria

Technical
Opportunity to reduce peak flows into Lake Simcoe
Opportunity to decrease erosion of watercourses
Opportunity to improve water quality
Opportunity to reduce phosphorus loadings into Lake Simcoe
Opportunity to mitigate changes in water balance
Natural Heritage Features
Provisions of direct and indirect fish habitat
Potential to improve terrestrial habitats
Impacts to natural hazard features
Social Environment
Ability to improve public health and safety
Impacts to private properties
Impacts to public property
Cultural Environment
Impacts to built and cultural heritage landscape
Impacts to archaeological resources
Economic Environment
Capital costs
Operation and Maintenance costs
Risk management
Impact on agricultural land use

It is noted that the evaluation criteria was applied on an individual settlement area basis where the key implementations of the alternative SWM strategies could be effectively applied. The preferred alternatives for each settlement area are based on existing site constraints and the feasibility of implementing SWM improvements to each area. The preferred alternative strategy is outlined in **Section 10.0**.

10.0 Preferred Alternative Strategy

The preferred alternative strategy is to provide site specific recommendations on a settlement area basis where growth is expected to occur as part of the Town Consolidated Official Plan 2031. The following sections detail the preferred alternative within each settlement area, the opportunities and constraints associated in the implementation of the strategy and information inclusive to future developments within each settlement area. In addition, a summary of the site specific criteria is provided for each settlement area. It is noted the recommendations provided are indicative of the Master Plan process and that future developments require detailed analysis of existing site conditions and approval by the LSRCA and other approval agencies before implementing any recommendations described herein.

10.1. Site Specific Recommendations

Based on existing conditions, the Town's OP, and the results of **Section 4.0**, recommendations and criteria for each settlement area have been developed.

10.1.1. Mount Albert

General

It is proposed within the Community of Mount Albert, a small amount of growth will occur in the projected future. The existing land use is 12% imperviousness and the area is planned to develop which will lead to a 10% increase. This increase is due to the addition of an employment area and some institutional designated land.

It is important to note that within this settlement boundary there are areas of high aquifer vulnerability and wellhead protection areas that should be avoided when attempting to infiltrate stormwater runoff (**Figure 4-8**). The Region of York Official Plan should be consulted when determining the regulations surrounding the implementation of SWM ponds (Region of York 2010). There are also several erosion sites in Mount Albert, specifically along the Mount Albert Creek. Controlling stormwater will help to mitigate erosion by reducing peak flows, but other measures such as adding riparian vegetation may need to be considered and can also help reduce phosphorus loading.

Three (3) SWM facilities are proposed to handle stormwater from new developments. Feasible locations are outlined above in **Figure 4-8**. Proposed SWM facilities should be located outside of the floodplain and 2-year wellhead protection zones. It should be noted these ponds will have to undergo a detailed design process as discussed in **Section 4.3.7**. It is recommended that the Town monitor groundwater levels to allow for measures such as infiltration trenches.

Quantity Control

As discussed in **Section 4.3.7**, quantity control should be enforced using "post-to-pre" criteria with the exception of the drainage areas outlined in **Table 4-42** in which unit-flow rates should be applied.

Quality Control

Stormwater quality control should be enforced using Ministry of Environment criteria and developments should provide 80% TSS Removal and Enhanced water quality levels.

As discussed in **Section 4.4**, stormwater quality control in terms of phosphorus control in the Mount Albert settlement area should be enforced using criteria of a total phosphorus removal rate of 3.78 kg/year/ha.

Water Budget

As discussed in **Section 4.5**, in order to maintain the existing infiltration rates, future developments will require to maintain an infiltration rate of 3025 m³/ha within the Mount Albert settlement area.

Best Management Practices

Recognizing that Mount Albert consists mainly of well draining soils, BMPs such as infiltration trenches and infiltration galleries may be implemented for new developments. For existing rural areas, it is recommended rural BMPs be considered were applicable.

The stormwater management criteria and recommendations for the Mount Albert settlement area are summarized in **Table 10-1** below.

Table 10-1 – Mount Albert Recommendation Summary

Quantity Control	Water Quality	Water Budget	Best Management Practices
<ul style="list-style-type: none"> ▪ Post development flows to be controlled to pre-development peak flow rates (“post-to-pre”) ▪ Unit-flow rates as summarized in Table 4-42 are to be applied for specific drainage areas in Mount Albert 	<ul style="list-style-type: none"> ▪ Water quality is to be controlled to Enhanced levels (i.e. 80% TSS removal). ▪ A required phosphorous removal rate of 3.78 kg/year/ha is required for the Mount Albert settlement area. 	<ul style="list-style-type: none"> ▪ A required infiltration rate of 3025 m³/ha is required for the Mount Albert settlement area. 	<ul style="list-style-type: none"> ▪ It is recommended that infiltration measures and Rural BMPs, including livestock fencing and buffer strips, be implemented where applicable.

It should be noted that all requirements of the SWM guidelines should be met. The SWM guidelines can be found in **Section 12.1.4**.

Preferred SWM Strategy

Based on the above, the preferred SWM strategy for Mount Albert is the *Traditional SWM with BMP Implementation Strategy in combination with Traditional SWM with Rural Retrofits Strategy*. Retrofit opportunities are further discussed in **Section 7.0** and illustrated above in **Figure 3-5**. Where applicable, it is recommended to provide BMPs in areas where soils and groundwater levels permit on a future development basis. A figure of the constraints and opportunities for the implementation of BMPs located within Mount Albert is provided above as **Figure 4-8**.

10.1.2. Queensville

General

The Queensville area will see some large changes in growth by 2031 according to the Town of East Gwillimbury Consolidated Official Plan. Queensville is currently mainly an agriculture area containing a small amount of residential land with 3% imperviousness. It plans to grow to a multi-functional area with prestige employment areas, medium density and low density residential, as well as a village core area. The future settlement area will be approximately 21% imperviousness. The area contains all four (4) types of soil, but is less than half well draining soils. In addition, there is a large area that is designated wellhead protection area.

There are no existing SWM facilities in Queensville and it is recommended that future SWM facilities that will service this area should comply with Lake Simcoe Protection Plan guidelines for quality and quantity control. Four (4) ponds have been proposed based on the expected growth of the area. Feasible locations are outlined above in **Figure 4-9**. Where SWM facilities cannot be built, (i.e. in wellhead protection areas and groundwater recharged zones), the implementation of site specific BMP's are recommended.

New SWM facilities are prohibited within the 2-year Wellhead Protection Zones as per the Region of York Official Plan (Region of York 2010). Within the Queensville settlement area there is also an area designated for Maskinonge River Significant Groundwater Recharge. It is important to note that this area can be directly connected to the groundwater supply hence stormwater BMP measures should only be implemented after additional analysis is completed.

There are no known erosion sites within this settlement area, but stream bank stability should be investigated to prevent future erosion due to the increase in growth and peak runoff.

Quantity Control

As discussed in **Section 4.3.7**, quantity control should be enforced using “post-to-pre” criteria with the exception of the drainage areas outlined in **Table 4-42** in which unit-flow rates should be applied.

Quality Control

Stormwater quality control should be enforced using Ministry of Environment criteria and developments should provide 80% TSS Removal and Enhanced water quality levels.

As discussed in **Section 4.4**, stormwater quality control in terms of phosphorus control in the Queensville settlement area should be enforced using criteria of a total phosphorus removal rate of 0.66 kg/year/ha.

Water Budget

As discussed in **Section 4.5**, in order to maintain the existing infiltration rates, future developments will require to maintain an infiltration rate of 1760 m³/ha within the Queensville settlement area.

Best Management Practices

Recognizing that the Queensville settlement area consists mainly of poor draining soils, areas where SWM facilities cannot be built, site specific BMP's are recommended. It is recommended further investigation is required at a detailed level for areas where BMP's will be proposed.

The stormwater management criteria and recommendations for the Queensville settlement area are summarized in **Table 10-2** below.

Table 10-2 – Queensville Recommendation Summary

Quantity Control	Water Quality	Water Budget	Best Management Practices
<p>Post development flows to be controlled to pre-development peak flow rates (“post-to-pre”).</p> <p>Unit-flow rates as summarized in Table 4-42 are to be applied for specific drainage areas in Queensville.</p>	<p>Water quality is to be controlled to Enhanced levels (i.e. 80% TSS removal).</p> <p>A required phosphorus removal rate of 0.66 kg/year/ha is required for the Queensville settlement area.</p>	<p>A required infiltration rate of 1760 m³/ha is required for the Queensville settlement area.</p>	<p>It is recommended to provide BMPs in areas where soils and groundwater levels permit on a future development basis.</p>

It should be noted that all requirements of the SWM guidelines should be met. The SWM guidelines can be found in **Section 12.1.4**.

Preferred SWM Strategy

Based on the above, the preferred SWM strategy for Queensville is the *Traditional SWM with BMP Implementation Strategy*. Where applicable, it is recommended to provide BMPs in areas where soils and groundwater levels permit on a future development basis. A figure of the constraints and opportunities for the implementation of BMPs located within Queensville is provided above as **Figure 4-9**.

10.1.3. Holland Landing

General

Holland Landing is currently comprised mostly of low density residential, with some commercial and natural areas with an overall imperviousness of 17%. This settlement area will expand its residential and commercial land while maintaining a wetland and natural area. Imperviousness is expected to rise to approximately 23%. Holland Landing has a large area of well draining soils with a designated wellhead protection area in the southeast region.

The East Holland River flows north-south through Holland Landing and stream bank erosion and sedimentation is of concern in some areas. It is recommended to dredge areas that have been identified with sedimentation build up, and stabilize stream banks where erosion is of concern. These recommendations will assist in decreasing phosphorus loadings into the river and into Lake Simcoe.

Three (3) SWM facilities are proposed and illustrated in the constraints and opportunities figure for Holland Landing (**Figure 4-10**). It should be noted any new facilities are prohibited in the 2-year Wellhead Protection Zones as per the Region of York Official Plan (Region of York 2010) and their location will require further detailed analysis before implementation.

Quantity Control

As discussed in **Section 4.3.7**, quantity control should be enforced using “post-to-pre” criteria with the exception of the drainage areas outlined in **Table 4-42** in which unit-flow rates should be applied.

Quality Control

Stormwater quality control should be enforced using Ministry of Environment criteria and developments should provide 80% TSS Removal and Enhanced water quality levels.

As discussed in **Section 4.4**, stormwater quality control in terms of phosphorus control in the Holland Landing settlement area should be enforced using criteria of a total phosphorus removal rate of 12.1 kg/year/ha.

Water Budget

As discussed in **Section 4.5**, in order to maintain the existing infiltration rates, future developments will require to maintain an infiltration rate of 2892 m³/ha within the Holland Landing settlement area.

Best Management Practices

Recognizing that Holland Landing consists mainly of well draining soils, it is recommended to provide BMPs in areas where soils and groundwater levels permit on a future development basis. It should be noted that Holland Landing is also highly urbanized and the implementation of BMPs will be subject to space restrictions.

The stormwater management criteria and recommendations for the Holland Landing settlement area are summarized in **Table 10-3** below.

Table 10-3 – Holland Landing Recommendation Summary

Quantity Control	Water Quality	Water Budget	Best Management Practices
<p>Post development flows to be controlled to pre-development peak flow rates (“post-to-pre”).</p> <p>Unit-flow rates as summarized in Table 4-42 are to be applied for specific drainage areas in Holland Landing.</p>	<p>Water quality is to be controlled to Enhanced levels (i.e. 80% TSS removal).</p> <p>A required phosphorus removal rate of 12.1 kg/year/ha is required for the Holland Landing settlement area.</p>	<p>A required infiltration rate of 2892 m³/ha is required for the Holland Landing settlement area.</p>	<p>It is recommended to provide BMPs in areas where soils and groundwater levels permit on a future development basis.</p>

It should be noted that all requirements of the SWM guidelines should be met. The SWM guidelines can be found in **Section 12.1.4**.

Preferred SWM Strategy

Based on the above, the preferred SWM strategy for Holland Landing is the *Traditional SWM with BMP Implementation Strategy* in combination with *Traditional SWM with Urban Retrofits Strategy*. Retrofit opportunities are further discussed in **Section 7.0** and illustrated above in **Figure 4-10**. Where applicable, it is recommended to provide BMPs in areas where soils and groundwater levels permit on a future development basis. A figure of the constraints and opportunities for the implementation of BMPs located within Holland Landing is provided above in **Figure 4-10**.

10.1.4. Sharon

General

Sharon Community is expected to expand into almost completely residential with 27% imperviousness from its current state which is approximately 19% impervious. According to the Sharon Community MESP (February 2010), there are low permeable soils present that will restrict the type of BMP measures put into place.

There are no identified erosion concerns within the settlement area but regular inspection should be completed for receiving watercourses to maintain good condition. There are no areas that have been identified as high aquifer vulnerability and only a very small portion that contains 10 year and 25 year zones of wellhead protection. In said areas, there are guidelines surrounding SWM Facilities but are expected to have minor impacts to planning efforts. Consultation of the Region of York Official Plan and Sharon Community MESP (February 2010) is recommended.

Existing SWM Facilities failing to meet Enhanced criteria should be upgraded according to the SWMSoft Maintenance Schedule detailed in **Section 7.0**. Two (2) ponds have been proposed based on land use and stormwater management restrictions and are illustrated in **Figure 4-11**. Further detailed study is recommended on a site specific basis for each new development.

Quantity Control

As discussed in **Section 4.3.7**, quantity control should be enforced using “post-to-pre” criteria with the exception of the drainage areas outlined in **Table 4-42** in which unit-flow rates should be applied.

Quality Control

Stormwater quality control should be enforced using Ministry of Environment criteria and developments should provide 80% TSS Removal and Enhanced water quality levels.

As discussed in **Section 4.4**, stormwater quality control in terms of phosphorus control in the Sharon settlement area should be enforced using criteria of a total phosphorus removal rate of 9.9 kg/year/ha.

Water Budget

As discussed in **Section 4.5**, in order to maintain the existing infiltration rates, future developments will require to maintain an infiltration rate of 3788 m³/ha within the Sharon settlement area.

Best Management Practices

Recognizing that Sharon consists of mainly low permeable soils, BMPs such as infiltration-enhancing measures in addition to the use of infiltration trenches should be examined when additional site-specific information is available. BMPs such as infiltration trenches would fill and overflow quickly leaving the BMP ineffective. The available grades in the area are generally 2% or higher, limiting the application of this type of control. However, some lot-level controls, such as downspout disconnection are applicable to all areas, while others, such as filter strips adjacent to open space areas and grassed swales adjacent to suitable land uses have limited application. The use of infiltration-enhancing measures in addition to the use of infiltration trenches should be examined at the functional servicing plan stage when additional site-specific information and detailed design issues will allow a greater potential for implementation.

The stormwater management criteria and recommendations for the Sharon settlement area are summarized in **Table 10-4** below.

Table 10-4 – Sharon Recommendation Summary

Quantity Control	Water Quality	Water Budget	Best Management Practices
<p>Post development flows to be controlled to pre-development peak flow rates (“post-to-pre”).</p> <p>Unit-flow rates as summarized in Table 4-42 are to be applied for specific drainage areas in Sharon.</p>	<p>Water quality is to be controlled to Enhanced levels (i.e. 80% TSS removal).</p> <p>A required phosphorous removal rate of 9.9 kg/year/ha is required for the Sharon settlement area.</p>	<p>A required infiltration rate of 3788 m³/ha is required for the Sharon settlement area.</p>	<p>It is recommended that the possibility of infiltration measures such as infiltration trenches, be examined before being implemented.</p>

It should be noted that all requirements of the SWM guidelines should be met. The SWM guidelines can be found in **Section 12.1.4**.

Preferred SWM Strategy

Based on the above, the preferred SWM strategy for Sharon is the *Traditional SWM with BMP Implementation Strategy* in combination with *Traditional SWM with Urban Retrofits Strategy*. Retrofit opportunities are further discussed in **Section 7.0** and illustrated in **Figure 4-11**. Where applicable, it is recommended to provide BMPs in areas where soils and groundwater levels permit on a future development basis. A figure of the constraints and opportunities for the implementation of BMPs located within Sharon is provided as **Figure 4-11**.

10.1.5. Green Lane Expansion Areas

General

The Green Lane Expansion Area is designed to accommodate the growth of the Town of East Gwillimbury by providing appropriate employment and commercial areas. Existing conditions have designated this area as primarily agricultural and some rural residential.

There is currently only one (1) SWM pond in this area that is meeting Enhanced criteria. It is recommended that as growth continues, additional SWM ponds be considered to service the highly impervious areas. Only one (1) major waterway exists in the employment area with no identified erosion concerns. As development increases, runoff volume will increase and stream bank instability may occur. It is recommended to maintain a buffer zone of at least 15 m around the creek and uphold existing vegetation (or increase vegetation) to ensure adequate stability, infiltration, and phosphorus removal.

Four (4) SWM facilities are proposed as seen above in **Figure 4-12** and **Figure 4-13**. A further detailed study is recommended on a site specific basis to determine the location, size and number of ponds.

Quantity Control

As discussed in **Section 4.3.7**, quantity control should be enforced using “post-to-pre” criteria with the exception of the drainage areas outlined in **Table 4-42** in which unit-flow rates should be applied. Based on the amount of industrial and commercial area proposed in the Green Lane Expansion Area, developments should meet unit-flow rates to reduce peak flows entering the receiving watercourse. It is noted a detailed unit-storage / unit-flow study should be conducted on a sub-watershed level to confirm restrictions on watercourse discharge.

Quality Control

Stormwater quality control should be enforced using Ministry of Environment criteria and developments should provide 80% TSS Removal and Enhanced water quality levels.

As discussed in **Section 4.4**, stormwater quality control in terms of phosphorus control in the Green Lane Expansion Area should be enforced using criteria of a total phosphorus removal rate of 8.9 kg/year/ha.

Water Budget

As discussed in **Section 4.5**, in order to maintain the existing infiltration rates, future developments will require to maintain an infiltration rate of 5760 m³/ha within the Green Lane Expansion Area.

Best Management Practices

Traditional BMP measures are recommended to be implemented in this area, and also innovative measures such as permeable pavement and green roofs. Urban BMPs as infiltration-enhancing measures in addition to the use of infiltration trenches and downspout disconnection should be examined when additional site-specific information is available.

The stormwater management criteria and recommendations for the Green Lane Expansion Area settlement area are summarized in **Table 10-5** below.

Table 10-5 – Green Lane Expansion Area Recommendation Summary

Quantity Control	Water Quality	Water Budget	Best Management Practices
<p>Post development flows to be controlled to pre-development peak flow rates (“post-to-pre”).</p> <p>Unit-flow rates as summarized in Table 4-22 are to be applied for specific drainage areas in the Green Lane Expansion Area.</p>	<p>Water quality is to be controlled to Enhanced levels (i.e. 80% TSS removal).</p> <p>A required phosphorus removal rate of 8.9 kg/year/ha is required for the Green Lane Expansion Area.</p>	<p>A required infiltration rate of 5760 m³/ha is required for the Green Lane Expansion Area.</p>	<p>It is recommended that the possibility of infiltration measures such as infiltration trenches, be examined before being implemented.</p>

It should be noted that all requirements of the SWM guidelines should be met. The SWM guidelines can be found in **Section 12.1.4**.

Preferred SWM Strategy

Based on the above, the preferred SWM strategy for the Green Lane and Employment Expansion Area is the *Traditional SWM with BMP Implementation Strategy* in combination with *Traditional SWM with Urban Retrofits Strategy*. Retrofit opportunities are further discussed in **Section 7.0** and illustrated above in **Figure 4-12** and **Figure 4-13**. Where applicable, it is recommended to provide BMPs in areas where soils and groundwater levels permit on a future development basis. A figure of the constraints and opportunities for the implementation of BMPs located within the Green Lane and Employment Expansion Area is provided above in **Figure 4-12** and **Figure 4-13**.

10.1.6. Rural Areas

The rural areas reside outside the above mentioned settlement areas in the Town of East Gwillimbury and are not part of the central growth area. There is no development proposed in this area, therefore land use will remain the same. In addition, approximately 75% of the Town is within the Oak Ridges Moraine and Protected Countryside. If the Town decides to grow further and develop into these areas, site specific studies are recommended. The implementation of rural retrofit opportunities as identified in **Section 8.2.5** should also be investigated.

Similar to the restrictions mentioned above, areas such as well head protection areas, high aquifer vulnerability, poor draining soils, and floodlines are all present in the rural areas. There is also land within the Oak Ridge Moraine that will have further restrictions on development according to the Oak Ridges Moraine Policies. It is noted that funding is available for rural retrofits as explained in **Section 12.4** and it is recommended that the Town investigate opportunities to retrofit rural areas on a site specific basis in the future.

10.2. Preferred Alternative Recommendation Summary

A summary of the recommendations presented in **Section 10.1** based on the preferred alternative for each settlement area was developed and listed below in **Table 10-6**.

Table 10-6 – Preferred Alternative Recommendation Summary

Settlement Area	Preferred Alternative	Recommended BMPs* (where appropriate)	Rationale
Mount Albert	Traditional SWM with BMP Implementation Traditional SWM with Rural Retrofits	Rural BMPs: Livestock fencing Buffer strips	Very small amount of growth Rural area will be maintained Within Oak Ridges Moraine
Queensville	Traditional SWM with BMP Implementation	Downspout disconnection Infiltration trenches Grassed swales	Proposed residential area Very small existing urban area

Settlement Area	Preferred Alternative	Recommended BMPs* (where appropriate)	Rationale
Holland Landing	Traditional SWM with BMP Implementation Traditional SWM with Urban Retrofits	Downspout disconnection Infiltration trenches Bioretention Rainwater harvesting Vegetated filter strips	Moderate amount of growth of residential and commercial Retrofit existing urban infrastructure where appropriate
Sharon	Traditional SWM with BMP Implementation Traditional SWM with Urban Retrofits	Downspout disconnection Grassed swales Vegetated filter strips	Moderate amount of growth Retrofit existing urban infrastructure where appropriate Few BMPs can be implemented due to poor infiltration
Green Lane Expansion Areas	Traditional SWM with BMP Implementation	Green roofs Infiltration chambers Permeable pavement Rainwater harvesting	Proposed commercial / industrial area Small existing urban area

*Recommended BMPs are based on settlement area specific conditions

10.3. Overall SWM Recommendations

Future SWMFs will be designed to MOE standards:

- Stormwater Quality Control: Enhanced – 80% TSS removal, 80% TP removal;
- Stormwater Quantity Control: Post-development peak flows to be reduced to pre-development levels for all storm events up to and including the 100-year storm. Unit-flow rates are to be applied for specific drainage areas requiring overcontrol to meet pre-development peak flows;**
- The Regional Storm should be controlled to the pre-development peak flow rate level *if* noted in the subwatershed study (LSRCA Technical Guidelines for Stormwater Management Submissions, November 2010);
- Site specific erosion control and detention time calculations based on individual fluvial assessment of the receiving stream (i.e. 24hr, 48hr, 72hr, etc.);
- Future developments will follow the guidelines as outlined in the Lake Simcoe Protection Plan (LSPP);
- BMPs will be implemented where applicable to establish a treatment train approach (i.e. source, conveyance and end-of-pipe treatment);
- Where opportunities exist, implement enhanced vegetation for shading, bottom draw outlets, cooling trenches at SWMF locations to reduce stormwater temperature;
- Where opportunities exist, reduce phosphorus discharge, improve infiltration and reduce erosion potential within settlement areas and expansion of settlement area boundaries;

- Where opportunities exist, implement rural retrofits for applicable areas outside of existing settlement boundaries; and,
- Proceed with retrofit opportunities and maintenance operations of select SWM facilities as identified by the LSRCA and the SWM Master Plan.

** It is recommended that an investigation regarding the implementation of detailed unit flow criteria should be conducted in the future for the East and West Holland River subwatersheds to establish a reduction of peak flows.

11.0 Public Consultation

11.1. Consultation Activities

In order to fulfill the requirements of a municipal class EA, several opportunities were given to general public, residents, agencies, and interested stakeholders for learning, sharing and responding to the project. The Municipal Class EA requires the Proponent to undertake two (2) mandatory points of public contact during Phase Two (Alternative Solutions). The Project Team has exceeded the mandatory number of public contacts, with the following opportunities for comment provided:

- Notice of Commencement;
- Notice of Public Information Centre;
- Public Information Centre; and,
- Notice of Completion.

11.1.1. Notice of Commencement

The Notice of Commencement was prepared and distributed to stakeholders and review agencies between December 20, 2010 and February 9, 2011. In addition, the notice was posted on the Town's website under "Our Town News". The notice informs stakeholders of the SWMMMP being initiated in the Town and the study area that may be affected. The notice provides background information including the purpose, objectives and process. It also notifies the public about the consultation process consisting of one (1) Public Information Center (PIC) and the Public Education Campaign in place and gives locations of where further information can be found. Furthermore, the contact information for the Town's Project Manager and Cole Engineering's Project Manager were made available to the public to engage any initial feedback on the project.

A copy of the Notice of Commencement is provided in **Appendix M**.

11.1.2. Notice of Public Information Centre

A Notice of PIC was prepared and posted on the Town's website under "Our Town News". A copy of the Notice of PIC is available in **Appendix N**. The notice provided a description of the project, provided the details of the PIC, and included a request for comments and input. Contact information was provided for the Town's Project Manager and Cole Engineering's Project Engineer was also provided to encourage the submission of comments.

11.1.3. Public Information Centre

One (1) Public Information Centre (PIC) took place during the project. The PIC was held on March 3, 2011. It was attended by six (6) people mostly consisting of Town council and staff. The Project Team including representatives from Cole and the Town were in attendance to answer any questions that participants had.

The PIC presented the following elements:

- Study Overview and Background;
- Problem / Opportunity Statement;
- Overview of the Municipal Class Environmental Assessment Process;
- Alternative Solutions Being Considered; and,
- Next Steps.

The display panels presented at the PIC can be found in **Appendix O**. The PIC sign-in sheet and comment form are enclosed as **Appendix P**. It is noted that no comments were received as a result of the PIC.

11.2. First Nations Consultation

A letter and Notice of Commencement was sent to the Consultation Assessment Coordinator with Lands, Resources and Consultations at Métis Nation of Ontario. The project notice is currently being reviewed by the community. The impact that this statement may have could restrict the implementation of BMP measures on certain lands. It is expected that these impacts will be minor. A letter received from Indian and Northern Affairs Canada is included as **Appendix Q**.

11.3. Consultation with LSRCA

An initial meeting was held with staff of the LSRCA on January 14, 2011. The purpose of this meeting was to assess LSRCA approval requirements for the Lake Simcoe Protection Plan and future permits to be acquired during the Master Plan EA process.

The meeting minutes which provide preliminary comments on the Master Plan EA, which can be found in **Appendix R** with key points summarized below:

- LSRCA agrees with the intent of the EA process;
- LSRCA requires the SWMMP to follow the requirements of the Lake Simcoe Protection Plan; and,
- LSRCA prefers the development of the SWMMP to follow the Municipal Master Plan EA Process with regards to alternatives and public consultation.

11.4. Consultation with the Ministry of the Environment

A covering letter and the Notice of Commencement were sent to the Ministry of the Environment (MOE) on February 9, 2011. A letter was received from the MOE, dated March 8, 2010, confirming their requirements and suggestions for the EA process. This letter is included in **Appendix S**.

12.0 Implementation

12.1. Policy Considerations

12.1.1. General

As part of Policy 4.5 SA of the LSPP, recommended policies and SWM standards are required for future developments within the Town. Policies examined are discussed below and future SWM guidelines are discussed in **Section 12.1.4**.

12.1.2. Road Salts

An imminent concern in regards to the degradation of water quality is the accumulation of road salts within surface water. The CEPA defines road salt containing chloride as toxic, and therefore need to be managed in order to protect the Town's drinking water and aquatic life. Analysis of the Lake Simcoe Watershed indicates that chloride levels have remained constant or increased in most subwatershed and with the expect growth may further increase. It is recommended that the use, storage, and application of road salt and the disposal of snow be conducted in accordance to the Code of Practice for the Environmental Management of Road Salts (*Environment Canada 2004*). In reference to road salt application timing, schedules can be modelled after the Region of York or the Code of Practice. Alternatives to road salt should be explored such as the use of sand. It should be noted that when sand or other sediments are used, road sweeping is necessarily to be completed in a timely manner to avoid said sediments being carried into local watercourses. The monitoring of road salt usage requires a separate study to determine the best practices for the Town's need.

12.1.3. Water Temperature

The combination of warmer runoff from impervious areas and reduction in groundwater infiltration can produce severely elevated temperatures in the receiving streams, which can contribute to reductions in dissolved oxygen and create conditions outside of the thermal tolerance limits for desirable fish species and other aquatic life.

Chapter 4.4 of the *MOE SWMP Design Manual* states that "an increase in stormwater temperature is inevitable if an area is developed (i.e. urbanization causes stormwater temperature increases). This observation is based on current development practices." Mitigation by way of cooling can be provided if surface water is to be released on lawns or pervious areas before released from the site. In addition, subsurface trench outlets (such as those recommended in **Section 8.2**) has been suggested by MOE (Chapter 4.4) as a technique which reduces stormwater temperature. It is recommended that stormwater be discharged through underground trenches or onto pervious surfaces, where practical, before being released into watercourses. In addition, opportunities to implement SWM pond bottom draw outlets, cooling trenches and enhanced vegetation for shading be investigated for the reduction of stormwater temperature in future developments. It is noted that further analysis regarding the increases / decreases in stormwater temperature may be required during the functional design stage to accurately assess the changes in stormwater temperature as a result of development.

12.1.4. SWM Guidelines

As stated in **Section 10.3**, the overall SWM guidelines for the Town should be as follows:

- Future SWMFs will be designed to MOE standards;
- Stormwater Quality Control: Enhanced - 80% TSS removal, 80% TP removal;
- Stormwater Quantity Control: Post-development peak flows to be reduced to pre-development levels for all storm events up to and including the 100-year storm; **
- Site specific erosion control and detention time calculations based on individual fluvial analysis of the receiving stream (i.e. 24hr, 48hr, 72hr, etc.);
- Future developments will follow the standards as outlined in the Lake Simcoe Protection Plan (LSPP);
- BMPs will be implemented where applicable to establish a treatment train approach (i.e. source, conveyance and end-of-pipe treatment);
- Where opportunities exist, implement enhanced vegetation for shading, bottom draw outlets, cooling trenches at SWMF locations to reduce stormwater temperature;
- Where opportunities exist, reduce phosphorus discharge, improve infiltration and reduce erosion potential within settlement areas and expansion of settlement area boundaries;
- Where opportunities exist, implement rural retrofits for applicable areas outside of existing settlement boundaries; and,
- Proceed with retrofit opportunities and maintenance operations of select SWM facilities as identified by the LSRCA and **Appendix L** of the SWM Master Plan.

It should be noted that although “post-to-pre” stormwater quantity control criteria is recommended for future developments, this level of control may not reduce flows in the watershed to pre-development levels as per the hydrologic analysis in **Section 4.3. It is recommended that unit-flow rates be applied for areas requiring overcontrol to meet pre-development peak flow rates. An investigation regarding the implementation of detailed unit flow criteria should be conducted in the future for the East Holland and West Holland River subwatersheds to establish a reduction of peak flows.

12.2. Public Consultation / Education

It should be noted that SWM can be more effective with the addition of a public education program. A program should be focused on informing residents and businesses about the use of best management practices and low-impact development measures. The LSRCA is committed to continuing community and public outreach to engage the communities and encourage their participation in developing the future implementation plans. The LSRCA currently runs workshops and other outreach programs that the Town could use as a foundation to an education program. Moreover, The Town can model programs after those of The Region of York, such as their Water for Tomorrow Website or their Rain Barrel program to assist homeowners in reusing stormwater in their gardens.

12.3. Recommended Amendments to Official Plan

There are no current amendments to the Town consolidated OP 2031 that have been identified while carrying out this SWMMP.

12.4. Sources of Funding

The Ontario Soil and Crop Improvement Association in partnership with Agriculture and Agri-Food Canada, Ontario Ministry of Agriculture, and Food and Rural Affairs have developed a four (4) year program aimed to help farmers adopt Best Management Practices in four (4) main areas: environment and climate, food safety and traceability, business development and biosecurity. The Canada-Ontario Farm Stewardship Program will provide information, training, workshops, technical advice, as well as cost-sharing opportunities. Farmers can receive aid in proper manure and milkhouse waste storage, stream ditch and floodplain management, wetlands and wildlife ponds and soil management. Applications for the final year of funding will come up in November 2011, but it is highly encouraged to contact a local representative as soon as possible.

Further information regarding this funding program is available on the Ontario Soil and Crop Improvement Association website (<http://www.ontariosoilcrop.org/default.htm>).

LSRCA also has a similar program that provides landowners with funding and technical assistance for environmental projects on their land. LEAP (Landowner Environmental Assistance Program) can cover up to 100% for projects such as planting trees and shrubs, managing manure, controlling cropland erosion, and upgrading SWM ponds. Applications are made on an individual project basis usually by the landowner. Staff members are available to answer any questions regarding the planning and application process and it is recommended to contact the local stewardship representative before an application is submitted.

Further information regarding this funding program can be found on the LSRCA website (<http://www.lsrca.on.ca/leap>). Note that in the past this program has been very popular so it is recommended that applications be submitted early in the year.

APPENDIX A
Existing Land Use

Catchment No.	Soil Type	Agriculture (ha)	Forest (ha)	Open Water/Wetlands (ha)	Rural Development (ha)	Residential (ha)	Estate (ha)	Commercial (ha)	Institutional (ha)	Open Space (ha)
North Alberta										
3B	Type D	0.0	5.7	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	Bottomland	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	19.1
	Type B	89.7	1.1	6.5	0.0	57.2	0.0	0.0	0.0	0.0
4B	Type C	59.5	0.0	0.0	0.0	29.6	0.0	0.0	0.0	0.0
	Type B	4.1	8.0	0.0	0.0	4.7	0.0	0.0	0.0	0.0
	Type C	0.6	1.6	2.6	0.0	14.9	0.0	0.0	0.0	0.0
Total	Type A	19.1	5.4	5.7	0.0	37.0	0.0	0.0	0.0	0.0
		173.0	21.8	14.8	0.0	143.6	0.0	0.0	0.0	19.1
TOTAL										
North West										
238	Type B	0.0	0.0	0.0	4.7	0.0	0.0	0.0	0.0	0.0
236	Type D	12.6	18.5	0.0	1.8	0.0	2.3	0.0	0.0	0.0
	Type B	204.8	147.1	8.9	3.8	0.0	12.9	0.0	0.0	0.0
232	Type B	1.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
216	Type B	3.7	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
244	Type B	69.5	32.9	0.0	0.0	41.9	0.0	0.0	0.0	0.0
7B	Type D	5.8	4.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	Type A	20.8	7.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	Type B	59.5	3.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	Type C	48.6	3.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0
1M	Bottomland	0.0	0.0	17.8	0.0	0.0	0.0	0.0	0.0	0.0
	Type B	208.0	17.1	2.7	2.8	35.4	2.9	0.0	0.0	12.5
	Type C	170.4	19.8	0.0	0.0	0.0	1.1	0.0	0.0	0.0
Total		805.5	254.7	27.4	12.9	77.3	19.2	0.0	0.0	12.5
TOTAL										
Holland Landing										
342	Type B	57.3	56.4	0.0	28.3	130.6	0.0	0.0	2.0	0.0
	Type A	0.8	0.7	0.0	0.0	10.6	0.0	0.0	0.0	4.8
240	Type B	0.0	6.9	0.0	1.1	16.0	0.0	0.0	0.0	0.0
	Type C	0.0	5.4	2.2	0.0	19.7	0.0	0.0	0.0	0.0
	Bottomland	0.0	0.0	11.0	0.0	0.0	0.0	0.0	0.0	0.0
242	Type A	0.0	0.0	0.0	0.0	13.0	0.0	0.0	0.0	0.0
	Type C	0.0	0.0	0.0	0.0	2.4	0.0	0.0	0.0	0.0
238	Type A	0.0	32.4	0.0	0.0	21.4	0.0	0.0	0.0	0.0
	Bottomland	0.0	0.0	1.4	0.0	0.0	0.0	0.0	0.0	0.0
	Type C	0.0	5.0	0.0	0.0	12.8	0.0	0.0	0.0	0.0
	Type B	0.0	8.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	Type D	0.0	16.3	0.0	0.0	10.3	0.0	0.0	0.0	0.0
234	Bottomland	0.0	0.0	25.9	0.0	0.0	0.0	0.0	0.0	0.0
	Type A	0.0	13.7	15.7	0.0	73.5	0.0	0.0	0.7	0.0
	Type D	0.0	0.0	33.8	0.6	2.8	0.0	0.0	0.0	0.0
	Type B	0.0	6.9	0.0	0.0	19.2	0.0	0.0	0.0	2.5
422	Type B	0.0	14.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	Type A	0.0	11.6	3.5	0.0	0.0	0.0	0.0	0.7	32.6
	Type C	0.0	2.5	0.0	0.0	0.0	0.0	0.0	0.0	0.0
232	Type A	0.0	0.0	0.0	0.0	3.6	0.0	0.0	0.0	0.0
	Type B	67.4	46.0	9.8	1.7	51.2	0.0	0.0	4.4	0.0
230	Type B	22.5	0.0	4.7	0.0	28.7	0.0	0.0	0.0	0.0
	Bottomland	0.0	0.0	15.9	0.0	0.0	0.0	0.0	0.0	0.0
	Type C	22.1	5.4	6.1	1.5	54.0	0.7	0.0	3.8	13.2
	Type A	0.0	0.0	1.9	0.0	29.4	0.0	0.0	0.0	0.0
220	Bottomland	0.0	0.0	9.6	0.0	0.0	0.0	0.0	0.0	0.0
	Type C	2.7	9.3	0.0	0.2	22.0	0.0	0.0	0.0	21.0
226	Bottomland	0.0	0.0	7.1	0.0	0.0	0.0	0.0	0.0	0.0
	Type C	0.0	1.2	0.0	1.3	0.0	0.0	0.0	0.0	0.0
228	Bottomland	0.0	0.0	12.9	0.0	0.0	0.0	0.0	0.0	0.0
	Type C	20.0	9.5	2.0	0.8	28.2	0.0	0.0	0.0	9.2
	Type B	0.0	1.9	1.1	6.4	0.0	0.0	0.0	0.0	2.2
	Type A	0.0	5.2	0.0	0.0	9.2	0.0	0.0	0.0	0.0
Total		192.8	259.1	164.6	41.9	588.5	0.7	0.0	11.6	85.5
TOTAL										
Sharon										
218	Type B	10.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	Bottomland	0.0	0.0	6.1	0.0	0.0	0.0	0.0	0.0	0.0
	Type C	27.3	27.7	0.0	0.0	7.6	0.0	0.0	0.0	0.0
216	Type B	0.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	Bottomland	0.0	0.0	4.2	0.0	0.0	0.0	0.0	0.0	0.0
	Type C	34.2	2.8	0.0	0.0	27.1	0.0	0.0	0.0	0.0
214	Type A	0.0	0.0	0.0	0.0	8.2	0.0	0.0	0.0	0.0
	Bottomland	0.0	0.0	7.6	0.0	0.0	0.0	0.0	0.0	0.0
	Type C	21.1	11.4	0.0	0.0	24.5	0.0	0.0	0.0	0.0
210	Type B	0.0	0.0	1.6	0.0	89.2	0.0	0.0	0.0	0.0
	Type A	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
7B	Type C	5.1	0.0	0.0	0.0	15.3	0.0	0.0	0.0	0.0
	Type B	0.8	0.0	0.0	0.0	32.9	0.0	0.0	0.0	0.0
	Type A	30.5	0.0	0.0	0.0	72.1	0.0	0.0	0.0	0.0
Total		130.0	41.7	19.5	0.0	296.3	0.0	0.0	0.0	0.0
TOTAL										

Catchment No.	Soil Type	Agriculture (ha)	Forest (ha)	Open Water/Wetlands (ha)	Rural Development (ha)	Residential (ha)	Estate (ha)	Commercial (ha)	Institutional (ha)	Open Space (ha)
	Type B	5.9	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
418	Type C	11.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	Bottomland	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
418	Type C	5.8	10.6	0.0	0.0	0.0	0.0	0.0	0.0	0.0
414	Type C	2.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
228	Type C	1.3	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	Type C	72.2	5.8	0.0	7.0	0.3	0.0	35.3	0.0	5.1
224	Type D	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	Type C	75.0	16.5	0.0	0.0	0.0	0.0	0.0	0.0	5.4
226	Type D	4.1	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	Bottomland	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
220	Type C	5.7	1.4	0.0	4.1	0.0	0.0	0.0	0.0	1.7
	Type C	19.1	11.8	0.0	0.0	3.3	0.0	3.2	0.0	12.5
222	Type D	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	Bottomland	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
208	Type C	54.6	14.0	0.0	0.0	1.6	0.0	0.0	0.0	0.0
	Bottomland	0.0	0.0	5.1	0.0	0.0	0.0	0.0	0.0	0.0
	Type C	153.4	19.6	0.0	0.0	0.9	0.0	0.0	0.0	0.0
210	Type B	10.9	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	Type C	2.4	0.0	0.0	0.0	0.8	0.0	0.0	0.0	0.0
214	Type B	74.7	15.4	0.0	12.5	1.6	0.0	0.0	0.0	0.0
212	Type B	13.2	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
6B	Type B	30.7	10.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	Type B	82.9	0.0	0.0	0.0	0.0	0.0	0.0	0.0	8.4
7B	Type A	19.5	0.0	0.0	17.2	0.0	0.0	0.0	0.0	0.0
Total Area		645.3	105.1	231.8	40.8	8.4	0.0	38.5	0.0	33.1
									TOTAL	1,103.0

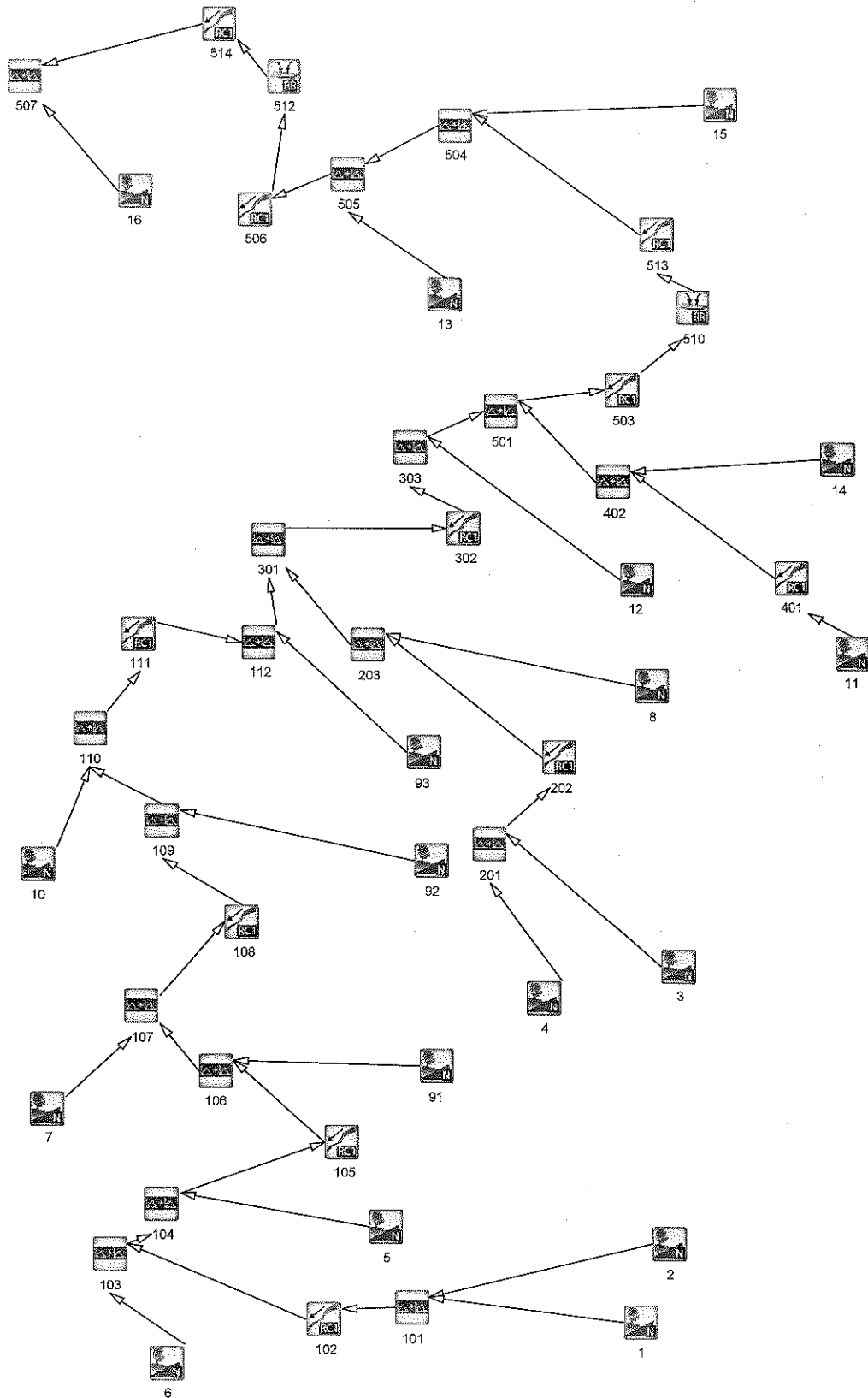
APPENDIX B
Future Land Use

Cachment No.	Soil Type	Agriculture (ha)	Open Water/Wetlands (ha)	Forest (ha)	Med. Density Residential (ha)	Low Density Residential (ha)	Estate (ha)	Institutional (ha)	Commercial (ha)	Open Space (ha)
General Land Use Categories										
3B	Type C	0.0	0.0	0.0	0.0	62.2	0.0	0.0	0.0	22.6
	Type B	0.0	4.9	0.0	1.9	84.8	0.0	7.7	0.0	41.0
	Type D	0.0	5.7	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	Bottomland	0.0	3.4	0.0	0.0	4.4	0.0	0.0	0.3	15.1
	Type A	0.0	0.8	0.0	0.0	21.1	0.0	0.0	3.0	1.2
4B	Type C	2.7	0.0	0.0	0.0	14.5	0.0	0.0	7.1	0.1
	Type B	0.0	0.0	0.0	0.0	8.2	0.0	2.6	0.3	3.5
	Type A	6.2	0.0	0.0	0.0	19.1	0.0	2.2	25.1	0.5
Total		8.94	14.78	0.00	1.89	214.31	0.00	12.62	35.88	83.94
General Land Use Categories										
TOTAL 372.4										
General Land Use Categories										
1M	Type B	0.0	2.7	0.0	12.7	150.4	0.0	1.6	67.4	33.8
	Type C	0.0	4.7	0.0	0.0	0.1	0.0	1.4	139.7	34.0
	Type D	0.0	0.0	0.0	0.0	0.0	0.0	0.2	12.5	9.8
	Bottomland	0.0	16.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	Type B	0.0	0.0	0.0	0.0	112.7	0.0	0.0	19.7	10.9
244	Type B	0.0	3.4	49.2	11.1	273.8	13.2	0.0	0.0	24.8
236	Type D	0.0	0.0	12.8	0.0	22.6	1.1	0.0	0.0	0.0
238	Type B	0.0	0.0	0.0	0.0	2.0	2.7	0.0	0.0	0.0
232	Type B	0.0	0.0	0.0	0.0	1.2	0.0	0.0	0.0	0.6
7B	Type B	0.0	0.0	0.0	0.8	30.2	0.0	0.1	18.4	10.0
	Type D	0.0	0.0	0.0	0.0	23.4	0.0	0.0	5.1	18.0
	Type A	0.0	0.0	0.0	0.0	0.0	0.0	0.0	18.0	12.7
	Type C	0.0	0.0	0.0	0.0	0.0	0.0	0.0	19.6	2.2
	Type B	0.0	0.0	0.0	0.0	3.5	0.0	0.0	0.0	0.0
Total		0.0	27.4	62.1	24.6	619.9	17.0	3.3	298.5	156.8
TOTAL 1209.5										
General Land Use Categories										
240	Type C	0.0	4.9	2.6	0.0	20.4	0.0	0.0	0.0	0.0
	Type A	0.0	2.3	0.0	0.0	14.2	0.0	0.0	0.0	0.0
	Bottomland	0.0	13.0	0.1	0.0	10.0	0.0	0.0	0.0	0.0
	Type B	0.0	0.0	5.9	0.0	15.3	0.0	0.0	0.0	0.2
	Type B	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
242	Type C	0.0	0.0	2.1	0.0	3.1	0.0	0.0	0.0	0.0
Type A	0.0	0.0	0.2	0.0	0.0	0.0	0.0	0.0	0.0	
342	Type B	1.8	0.0	20.3	0.0	111.0	0.0	2.9	98.4	2.3
Type A	0.0	0.0	0.5	0.0	10.6	0.0	0.0	1.0	2.7	
422	Type B	0.0	0.0	0.0	0.0	0.0	0.0	0.0	7.8	6.2
	Type A	0.0	0.0	0.0	0.0	45.2	0.0	0.0	0.0	5.4
	Type C	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	2.2
	Type A	0.0	6.9	0.0	0.0	41.0	0.0	0.0	2.3	1.9
228	Type B	0.0	2.8	0.2	0.0	4.7	0.0	0.0	5.2	0.0
	Type C	0.0	3.0	3.7	0.0	26.8	0.0	0.0	0.0	0.0
	Type D	0.0	2.0	0.0	0.0	7.7	0.0	0.0	0.0	0.0
	Bottomland	0.0	17.2	0.0	0.0	0.0	0.0	0.0	0.0	4.7
	Type C	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	3.4
226	Bottomland	0.0	0.1	0.0	0.0	0.0	0.0	0.0	0.0	7.0
220	Bottomland	0.0	2.8	0.0	0.0	0.0	0.0	0.0	0.0	9.5
	Type C	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	3.2
	Type D	0.0	0.0	0.0	0.0	39.5	0.0	0.6	0.0	2.2
	Type D	0.0	0.0	0.0	1.0	66.9	0.0	4.1	0.0	11.2
230	Bottomland	0.0	0.0	0.0	0.4	0.0	0.0	0.0	0.0	25.6
	Type A	0.0	2.9	0.0	0.0	28.7	0.0	0.0	0.0	0.0
	Type B	0.0	0.0	0.0	0.1	33.2	0.0	0.9	0.0	0.0
	Type B	0.0	0.0	27.9	0.0	140.3	0.0	0.0	0.0	7.5
232	Type A	0.0	0.0	2.4	0.0	5.9	0.0	0.0	0.0	0.0
	Bottomland	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
	Type B	0.0	9.3	6.1	0.0	49.3	0.0	0.0	3.4	5.8
234	Type A	0.0	12.4	9.4	0.0	67.0	0.0	0.0	0.0	0.0
	Type D	0.0	44.2	8.0	0.0	0.0	0.0	0.0	0.0	4.5
	Bottomland	0.0	40.8	0.0	0.0	0.0	0.0	0.0	0.0	7.9
	Type B	0.0	0.0	8.5	0.0	0.0	0.0	0.0	0.0	0.0
238	Type A	0.0	0.0	23.8	0.0	28.6	0.0	0.0	0.0	0.1
	Type C	0.0	0.0	1.5	0.0	7.4	0.0	0.0	0.0	0.0
	Bottomland	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.1
	Type D	0.0	0.0	16.0	0.0	2.8	0.0	0.0	0.0	0.3
	Total		1.8	164.6	137.1	1.4	779.7	0.0	8.5	116.8
TOTAL 1324.7										
General Land Use Categories										
218	Type D	0.0	0.3	11.3	0.0	42.2	0.0	0.0	0.0	0.0
	Bottomland	0.0	0.4	4.9	0.0	0.0	0.0	0.0	0.0	0.9
	Type B	0.0	0.0	1.9	0.0	7.5	0.0	0.0	0.0	0.0
210	Type D	0.0	0.0	0.0	0.0	11.1	0.0	0.0	0.0	0.0
214	Type D	0.0	8.3	0.0	0.0	49.7	0.0	0.0	0.0	0.5
	Type B	0.0	1.6	0.0	0.0	85.6	0.0	0.0	0.0	1.6
	Type A	0.0	0.0	0.0	0.0	3.0	0.0	0.0	0.0	0.0
	Bottomland	0.0	4.1	0.0	0.0	0.0	0.0	0.0	0.0	3.9
	Type B	0.0	0.0	0.0	0.0	27.8	0.0	0.0	0.0	8.0
7B	Type A	0.0	0.0	0.0	0.0	105.6	0.0	1.8	0.0	12.5
	Type D	0.0	0.0	0.0	0.0	23.2	0.0	0.0	0.0	0.0
	Type D	0.0	0.0	0.0	0.0	6.3	0.0	0.0	0.0	0.0
216	Type D	0.0	2.7	0.0	0.0	58.5	0.0	0.0	0.0	0.0
	Type B	0.0	0.0	0.0	0.0	0.9	0.0	0.0	0.0	0.0
	Bottomland	0.0	2.0	0.0	0.0	0.0	0.0	0.0	0.0	1.3
	Total		0.0	19.5	18.1	0.0	419.4	0.0	1.8	0.0
TOTAL 487.5										

Cachment No.	Soil Type	Agriculture (ha)	Open Water/Wetlands (ha)	Forest (ha)	Med. Density Residential (ha)	Low Density Residential (ha)	Estate (ha)	Institutional (ha)	Commercial (ha)	Open Space (ha)
416	Type C	0.0	0.0	2.6	0.0	6.3	0.0	0.0	0.0	29.6
	Bottomland	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	15.1
	Type B	0.0	0.0	0.0	0.0	14.2	0.0	0.0	0.0	21.6
418	Type B	0.0	0.0	0.0	0.0	5.5	0.0	0.0	0.0	0.0
	Type C	0.0	0.0	0.0	0.0	3.2	0.0	0.0	0.0	0.0
414	Type C	0.0	0.0	0.0	0.0	2.1	0.0	0.0	0.0	0.0
	Type D	0.0	0.0	0.0	0.0	9.2	0.0	0.0	77.1	0.0
224	Type C	0.0	0.0	0.0	0.0	0.0	0.0	0.0	1.5	0.0
	Type D	0.0	0.0	0.0	0.0	3.2	0.0	0.0	39.6	0.0
222	Type C	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.4	0.0
	Type D	0.0	0.0	0.0	0.0	0.0	0.0	0.0	24.5	0.0
208	Type D	0.0	0.0	0.0	0.0	0.0	0.0	0.0	28.2	0.0
	Bottomland	0.0	0.0	0.0	0.0	0.0	0.0	0.0	3.4	0.0
226	Type C	0.0	0.0	0.0	0.0	0.0	0.0	0.0	108.0	0.0
	Type D	0.0	0.0	0.0	0.0	0.0	0.0	0.0	9.8	0.0
210	Type C	0.0	0.0	0.0	0.0	6.0	0.0	0.0	49.8	0.0
	Type D	0.0	0.0	0.0	0.0	0.0	0.0	0.0	117.0	0.0
	Bottomland	0.0	5.1	6.0	0.0	0.0	0.0	0.0	10.0	0.0
	Type B	0.0	0.0	0.0	0.0	0.0	0.0	0.0	10.9	0.0
220	Type C	0.0	0.0	0.0	0.0	0.0	0.0	0.0	12.8	0.0
	Bottomland	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.5	0.0
214	Type D	0.0	0.0	0.0	0.0	0.0	0.0	0.0	3.1	0.0
	Type B	0.0	0.0	0.0	0.0	0.0	0.0	0.0	89.9	0.0
6	Type B	0.0	0.0	0.0	0.0	0.0	0.0	0.0	32.3	0.0
7	Type B	0.0	0.0	0.0	0.0	0.0	0.0	0.0	56.8	0.0
	Type A	0.0	0.0	0.0	0.0	0.0	0.0	0.0	79.0	0.0
212	Type B	0.0	0.0	0.0	0.0	0.0	0.0	0.0	4.4	0.0
Total		0.0	5.1	2.6	0.0	43.6	0.0	0.0	758.7	66.3
									TOTAL	876.3

APPENDIX C
Existing Conditions Hydrologic Model Output

BLACK RIVER SUBWATERSHED



**EXISTING
BLACK RIVER 2-100 YEAR STORMS**

```

V V I SSSS U U A L
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
WV I SSSS UUUU A A LLLL

OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
OOO T T H H Y Y M M OOO Company Serial

```

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***** DETAILED OUTPUT *****

Input filename: C:\program files\Visual Gchymo 2.4r\VO2\voin.dat
Output filename: C:\Users\jscott\AppData\Local\Temp\88073fbd-731f-4992-8e5b-76831528f07f\Scenario.out
Summary filename: C:\Users\jscott\AppData\Local\Temp\88073fbd-731f-4992-8e5b-76831528f07f\Scenario.sum

DATE: 08/22/2012 TIME: 11:56:20

USER:

COMMENTS: _____

** SIMULATION NUMBER: 1 **

READ STORM | Filename: C:\Users\jscott\AppData
Local\Temp\88073fbd-731f-4992-8e5b-76831528f07f\44f96c2

|
| Ptotal= 51.24 mm | Comments: 2yr/24hr

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	0.00	6.50	4.35	12.75	1.79	19.00	0.26
0.50	0.26	6.75	4.35	13.00	1.79	19.25	0.26
0.75	0.26	7.00	4.35	13.25	1.79	19.50	0.26
1.00	0.26	7.25	4.35	13.50	1.79	19.75	0.26
1.25	0.26	7.50	4.35	13.75	1.79	20.00	0.26
1.50	0.26	7.75	4.35	14.00	1.79	20.25	0.26
1.75	0.26	8.00	4.35	14.25	1.79	20.50	0.26
2.00	0.26	8.25	4.35	14.50	1.02	20.75	0.26
2.25	0.26	8.50	11.78	14.75	1.02	21.00	0.26
2.50	0.26	8.75	11.78	15.00	1.02	21.25	0.26
2.75	0.26	9.00	11.78	15.25	1.02	21.50	0.26
3.00	0.26	9.25	11.78	15.50	1.02	21.75	0.26
3.25	0.26	9.50	11.78	15.75	1.02	22.00	0.26
3.50	0.26	9.75	11.78	16.00	1.02	22.25	0.26
3.75	0.26	10.00	11.78	16.25	1.02	22.50	0.26
4.00	0.26	10.25	11.78	16.50	0.51	22.75	0.26
4.25	0.26	10.50	3.33	16.75	0.51	23.00	0.26
4.50	1.54	10.75	3.33	17.00	0.51	23.25	0.26
4.75	1.54	11.00	3.33	17.25	0.51	23.50	0.26
5.00	1.54	11.25	3.33	17.50	0.51	23.75	0.26
5.25	1.54	11.50	3.33	17.75	0.51	24.00	0.26
5.50	1.54	11.75	3.33	18.00	0.51	24.25	0.26
5.75	1.54	12.00	3.33	18.25	0.51		
6.00	1.54	12.25	3.33	18.50	0.26		
6.25	1.54	12.50	1.79	18.75	0.26		

CALIB | Area (ha)=3094.49 Curve Number (CN)= 74.0
NASHVD (0016) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 18.10

Unit Hyd Qpeak (cms)= 6.704

PEAK FLOW (cms)= 4.001 (i)
TIME TO PEAK (hrs)= 30.000
RUNOFF VOLUME (mm)= 15.782
TOTAL RAINFALL (mm)= 51.240
RUNOFF COEFFICIENT = 0.308

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB | Area (ha)=1627.59 Curve Number (CN)= 65.0
NASHVD (0013) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 15.50

Unit Hyd Qpeak (cms)= 4.118

PEAK FLOW (cms)= 1.790 (i)
TIME TO PEAK (hrs)= 27.500
RUNOFF VOLUME (mm)= 11.683
TOTAL RAINFALL (mm)= 51.240
RUNOFF COEFFICIENT = 0.228

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB | Area (ha)=1059.88 Curve Number (CN)= 73.0
NASHVD (0015) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 9.30

Unit Hyd Qpeak (cms)= 4.469

PEAK FLOW (cms)= 2.358 (i)
TIME TO PEAK (hrs)= 20.750
RUNOFF VOLUME (mm)= 15.252
TOTAL RAINFALL (mm)= 51.240
RUNOFF COEFFICIENT = 0.298

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB | Area (ha)=1501.52 Curve Number (CN)= 61.0
NASHVD (0012) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 11.50

Unit Hyd Qpeak (cms)= 5.120

PEAK FLOW (cms)= 1.873 (i)
TIME TO PEAK (hrs)= 23.250
RUNOFF VOLUME (mm)= 10.248
TOTAL RAINFALL (mm)= 51.240
RUNOFF COEFFICIENT = 0.200

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB | Area (ha)= 381.73 Curve Number (CN)= 69.0
NASHVD (0093) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 14.90

Unit Hyd Qpeak (cms)= 1.005

PEAK FLOW (cms)= 0.496 (i)
TIME TO PEAK (hrs)= 26.750
RUNOFF VOLUME (mm)= 13.334
TOTAL RAINFALL (mm)= 51.240
RUNOFF COEFFICIENT = 0.260

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB | Area (ha)=2842.76 Curve Number (CN)= 64.0
NASHVD (0010) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 10.80

Unit Hyd Qpeak (cms)= 10.322

PEAK FLOW (cms)= 4.127 (i)
TIME TO PEAK (hrs)= 22.500
RUNOFF VOLUME (mm)= 11.306
TOTAL RAINFALL (mm)= 51.240
RUNOFF COEFFICIENT = 0.221

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB | Area (ha)=1373.84 Curve Number (CN)= 73.0
NASHVD (0007) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 10.70

Unit Hyd Qpeak (cms)= 5.035

PEAK FLOW (cms)= 2.719 (1)
 TIME TO PEAK (hrs)= 22.250
 RUNOFF VOLUME (mm)= 15.252
 TOTAL RAINFALL (mm)= 51.240
 RUNOFF COEFFICIENT = 0.298

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0005) Area (ha)=1444.47 Curve Number (CN)= 60.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 8.00

Unit Hyd Opeak (cms) = 7.080

PEAK FLOW (cms)= 2.348 (1)
 TIME TO PEAK (hrs)= 19.500
 RUNOFF VOLUME (mm)= 9.918
 TOTAL RAINFALL (mm)= 51.240
 RUNOFF COEFFICIENT = 0.194

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0006) Area (ha)=1504.44 Curve Number (CN)= 73.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 12.20

Unit Hyd Opeak (cms) = 4.836

PEAK FLOW (cms)= 2.663 (1)
 TIME TO PEAK (hrs)= 24.000
 RUNOFF VOLUME (mm)= 15.252
 TOTAL RAINFALL (mm)= 51.240
 RUNOFF COEFFICIENT = 0.298

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0001) Area (ha)=2873.64 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 11.00

Unit Hyd Opeak (cms) = 10.244

PEAK FLOW (cms)= 3.721 (1)
 TIME TO PEAK (hrs)= 22.750
 RUNOFF VOLUME (mm)= 10.248
 TOTAL RAINFALL (mm)= 51.240
 RUNOFF COEFFICIENT = 0.200

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0002) Area (ha)= 988.72 Curve Number (CN)= 58.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 9.10

Unit Hyd Opeak (cms) = 4.261

PEAK FLOW (cms)= 1.357 (1)
 TIME TO PEAK (hrs)= 20.750
 RUNOFF VOLUME (mm)= 9.289
 TOTAL RAINFALL (mm)= 51.240
 RUNOFF COEFFICIENT = 0.181

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0101) |
 1 + 2 = 3 |
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID= 1 (0001): 2873.64 3.721 22.75 10.25
 + ID= 2 (0002): 988.72 1.357 20.75 9.29
 =====
 ID = 3 (0101): 3862.36 5.044 22.25 10.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0102) |
 IN= 2--> OUT= 1 |
 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning	
0.00	250.00	0.0700	
43.00	240.00	0.0700 / 0.0350	Main Channel
44.00	239.20	0.0350	Main Channel
45.00	239.20	0.0350	Main Channel
46.00	240.00	0.0350 / 0.0700	Main Channel
63.50	241.00	0.0700	
71.00	250.00	0.0700	

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.40	239.60	.642E+04	0.5	0.60	221.73
0.80	240.00	.171E+05	1.8	1.15	155.20
1.39	240.59	.764E+05	8.0	1.12	159.62
1.98	241.18	.214E+06	23.7	1.19	150.23
2.56	241.76	.385E+06	51.8	1.44	123.91
3.15	242.35	.575E+06	90.1	1.68	106.38
3.74	242.94	.784E+06	138.5	1.89	94.40
4.33	243.53	.101E+07	196.9	2.08	85.71
4.92	244.12	.126E+07	265.6	2.26	79.08
5.51	244.71	.153E+07	344.7	2.42	73.81
6.09	245.29	.181E+07	434.5	2.57	69.50
6.68	245.88	.212E+07	535.2	2.71	65.99
7.27	246.47	.244E+07	647.3	2.84	62.81
7.86	247.06	.278E+07	771.1	2.97	60.13
8.45	247.65	.314E+07	906.8	3.09	57.77
9.04	248.24	.352E+07	1054.8	3.20	55.67
9.62	248.82	.392E+07	1215.5	3.32	53.79
10.21	249.41	.434E+07	1389.1	3.42	52.09
10.80	250.00	.478E+07	1576.1	3.53	50.53

<---- hydrograph ----> <-pipe / channel-->

	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (0101)	3862.36	5.04	22.25	10.00	1.11	1.13
OUTFLOW : ID= 1 (0102)	3862.36	4.79	25.00	10.00	1.08	1.13

ADD HYD (0103) |
 1 + 2 = 3 |
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID= 1 (0005): 1504.44 2.663 24.00 15.25
 + ID= 2 (0102): 3862.36 4.787 25.00 10.00
 =====
 ID = 3 (0103): 5366.80 7.438 24.75 11.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0104) |
 1 + 2 = 3 |
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID= 1 (0005): 1444.47 2.348 19.50 9.92
 + ID= 2 (0103): 5366.80 7.438 24.75 11.47
 =====
 ID = 3 (0104): 6811.27 9.390 23.25 11.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0105) |
 IN= 2--> OUT= 1 |
 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning	
0.00	237.00	0.1000	
330.00	235.00	0.1000 / 0.0350	Main Channel
331.00	234.20	0.0350	Main Channel
332.00	234.20	0.0350	Main Channel
333.00	235.00	0.0350 / 0.1000	Main Channel
425.00	235.10	0.1000	
698.00	236.00	0.1000	
1101.00	239.00	0.1000	

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.13	234.33	.490E+03	0.1	0.64	82.66
0.27	234.47	.112E+04	0.3	0.93	56.72
0.40	234.60	.189E+04	0.7	1.14	45.94
0.53	234.73	.280E+04	1.2	1.33	39.64
0.67	234.87	.385E+04	1.8	1.49	35.34
0.80	235.00	.504E+04	2.6	1.63	32.16
0.95	235.15	.642E+04	3.3	1.75	29.66
1.11	235.31	.808E+04	4.3	1.84	27.51
1.26	235.46	.997E+04	5.3	1.91	25.64
1.42	235.62	.120E+05	6.9	2.00	23.91
1.57	235.77	.144E+05	9.0	2.06	22.40

1.72	235.92	.812E+06	172.6	0.67	78.45
1.88	236.08	.107E+07	250.3	0.74	71.19
2.03	236.23	.135E+07	347.8	0.81	64.71
2.18	236.38	.165E+07	461.9	0.88	59.65
2.34	236.54	.198E+07	593.4	0.95	55.58
2.49	236.69	.233E+07	742.8	1.01	52.21
2.65	236.85	.270E+07	910.6	1.06	49.36
2.80	237.00	.309E+07	1097.6	1.12	46.91

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLW : ID= 2 (0104) 6811.27 9.39 23.25 11.14 1.00 0.45
OUTFLOW: ID= 1 (0105) 6811.27 9.11 25.75 11.14 0.99 0.45

```

```

CALIB (0091) | Area (ha)= 281.97 Curve Number (CN)= 65.0
NASHYD (0091) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 5.80

```

```

Unit Hyd Qpeak (cms)= 1.906
PEAK FLOW (cms)= 0.689 (i)
TIME TO PEAK (hrs)= 17.000
RUNOFF VOLUME (mm)= 11.683
TOTAL RAINFALL (mm)= 51.240
RUNOFF COEFFICIENT = 0.228

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (0106) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0091): 281.97 0.689 17.00 11.68
+ ID2= 2 (0105): 6811.27 9.111 25.75 11.14
=====
ID = 3 (0106): 7093.24 9.411 25.25 11.17

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (0107) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0007): 1373.84 2.719 22.25 15.25
+ ID2= 2 (0106): 7093.24 9.411 25.25 11.17
=====
ID = 3 (0107): 8467.08 12.012 24.75 11.83

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (0108) |
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

```

```

<----- DATA FOR SECTION ( 1.1) ----->
Distance Elevation Manning
0.00 234.00 0.1000
400.00 231.00 0.1000 /0.0350 Main Channel
401.00 229.70 0.0350 Main Channel
402.00 229.70 0.0350 Main Channel
404.00 230.50 0.0350 /0.1000 Main Channel
495.00 231.00 0.1000
1234.00 256.00 0.1000

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.20 229.90 .151E+04 0.1 0.24 395.61
0.40 230.10 .377E+04 0.2 0.35 274.62
0.60 230.30 .677E+04 0.5 0.43 222.06
0.80 230.50 .105E+05 0.9 0.50 190.52
1.03 230.73 .437E+05 2.0 0.27 356.49
1.27 230.97 .134E+06 5.0 0.21 448.69
1.50 231.20 .278E+06 11.4 0.23 405.59
1.73 231.43 .473E+06 21.6 0.26 364.80
1.97 231.67 .718E+06 36.0 0.29 332.76
2.20 231.90 .101E+07 55.1 0.31 306.87
2.43 232.13 .136E+07 79.4 0.33 285.44
2.67 232.37 .176E+07 109.6 0.36 267.34
2.90 232.60 .221E+07 145.9 0.38 251.83
3.13 232.83 .270E+07 189.0 0.40 238.35
3.37 233.07 .325E+07 239.3 0.42 226.51
3.60 233.30 .385E+07 297.1 0.44 216.02
3.83 233.53 .450E+07 363.0 0.46 206.64
4.07 233.77 .520E+07 437.3 0.48 198.20

```

```

4.30 234.00 .595E+07 520.5 0.50 190.55
<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLW : ID= 2 (0107) 8467.08 12.01 24.75 11.83 1.51 0.24
OUTFLOW: ID= 1 (0108) 8467.08 9.75 30.50 11.83 1.44 0.23

```

```

CALIB (0092) | Area (ha)=1102.32 Curve Number (CN)= 64.0
NASHYD (0092) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 16.50

```

```

Unit Hyd Qpeak (cms)= 2.620
PEAK FLOW (cms)= 1.109 (i)
TIME TO PEAK (hrs)= 28.500
RUNOFF VOLUME (mm)= 11.306
TOTAL RAINFALL (mm)= 51.240
RUNOFF COEFFICIENT = 0.221

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (0109) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0092): 1102.32 1.109 28.50 11.31
+ ID2= 2 (0108): 8467.08 9.754 30.50 11.83
=====
ID = 3 (0109): 9569.40 10.851 30.25 11.77

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (0110) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0010): 2842.76 4.127 22.50 11.31
+ ID2= 2 (0109): 9569.40 10.851 30.25 11.77
=====
ID = 3 (0110): 12412.16 14.103 28.50 11.66

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (0111) |
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

```

```

<----- DATA FOR SECTION ( 1.1) ----->
Distance Elevation Manning
0.00 236.00 0.1000
275.00 232.00 0.1000
1039.00 230.00 0.1000 /0.0350 Main Channel
1040.00 229.20 0.0350 Main Channel
1041.00 229.20 0.0350 Main Channel
1043.00 230.00 0.0350 /0.1000 Main Channel
1117.00 230.60 0.1000
1325.00 234.00 0.1000

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.20 229.40 .985E+03 0.0 0.14 431.34
0.40 229.60 .251E+04 0.1 0.20 298.03
0.60 229.80 .457E+04 0.3 0.25 239.97
0.80 230.00 .717E+04 0.6 0.29 205.17
1.07 230.27 .754E+05 2.2 0.11 561.90
1.33 230.53 .272E+06 8.6 0.11 527.12
1.60 230.80 .594E+06 22.7 0.14 436.37
1.87 231.07 .103E+07 46.2 0.16 371.32
2.13 231.33 .158E+07 80.7 0.18 325.64
2.40 231.60 .224E+07 127.8 0.20 291.74
2.67 231.87 .301E+07 189.0 0.22 265.46
2.93 232.13 .389E+07 271.8 0.25 238.27
3.20 232.40 .481E+07 376.4 0.28 212.79
3.47 232.67 .576E+07 496.0 0.31 193.49
3.73 232.93 .674E+07 630.4 0.33 178.30
4.00 233.20 .776E+07 779.4 0.36 165.99
4.27 233.47 .881E+07 943.0 0.38 155.79
4.53 233.73 .990E+07 1121.1 0.41 147.16
4.80 234.00 .110E+08 1313.9 0.43 139.76

```

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLW : ID= 2 (0110) ***** 14.10 28.50 11.66 1.44 0.12

```

OUTFLOW: ID= 1 (0111) ***** 11.33 35.25 11.66 1.38 0.12

ADD HYD (0112)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0093):	381.73	0.496	26.75	13.33
+ ID2= 2 (0111):	12412.16	11.327	35.25	11.66
ID = 3 (0112):	12793.89	11.725	35.00	11.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0008)	Area (ha)	Curve Number (CN)
NASHYD (0008)	1549.97	64.0
ID= 1 DT=15.0 min	5.00	3.10
U.H. Tp(hrs)	9.10	

Unit Hyd Qpeak (cms) = 6.679

PEAK FLOW (cms)	2.273 (i)
TIME TO PEAK (hrs)	20.750
RUNOFF VOLUME (mm)	9.918
TOTAL RAINFALL (mm)	51.240
RUNOFF COEFFICIENT	0.194

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0003)	Area (ha)	Curve Number (CN)
NASHYD (0003)	2954.54	64.0
ID= 1 DT=15.0 min	5.00	3.10
U.H. Tp(hrs)	13.10	

Unit Hyd Qpeak (cms) = 8.844

PEAK FLOW (cms)	3.641 (i)
TIME TO PEAK (hrs)	25.000
RUNOFF VOLUME (mm)	11.306
TOTAL RAINFALL (mm)	51.240
RUNOFF COEFFICIENT	0.221

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0004)	Area (ha)	Curve Number (CN)
NASHYD (0004)	2672.69	53.0
ID= 1 DT=15.0 min	5.00	3.10
U.H. Tp(hrs)	10.70	

Unit Hyd Qpeak (cms) = 9.795

PEAK FLOW (cms)	2.718 (i)
TIME TO PEAK (hrs)	22.500
RUNOFF VOLUME (mm)	7.876
TOTAL RAINFALL (mm)	51.240
RUNOFF COEFFICIENT	0.154

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0201)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0003):	2954.54	3.641	25.00	11.31
+ ID2= 2 (0004):	2672.69	2.718	22.50	7.88
ID = 3 (0201):	5627.23	6.301	23.75	9.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0202) Routing time step (min)'= 15.00

Distance	Elevation	Manning
0.00	239.00	0.1000
690.00	230.00	0.1000 / 0.0350
691.00	229.20	0.0350
692.00	229.20	0.0350
693.00	229.50	0.0350 / 0.1000
1190.00	230.00	0.1000
1417.00	255.00	0.1000

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.30	229.50	347E+04	0.4	0.84	136.57
0.80	230.00	864E+06	45.2	0.36	318.78
1.30	230.50	265E+07	274.1	0.71	161.20
1.80	231.00	459E+07	651.8	0.97	117.24
2.30	231.50	667E+07	1165.3	1.20	95.34
2.80	232.00	899E+07	1810.2	1.40	81.89
3.30	232.50	113E+08	2585.1	1.57	72.65
3.80	233.00	138E+08	3490.4	1.73	65.85
4.30	233.50	165E+08	4527.4	1.89	60.59
4.80	234.00	193E+08	5697.8	2.03	56.38
5.30	234.50	222E+08	7003.8	2.16	52.91
5.80	235.00	253E+08	8447.7	2.28	50.00
6.30	235.50	286E+08	10032.3	2.40	47.52
6.80	236.00	320E+08	11760.1	2.52	45.36
7.30	236.50	356E+08	13634.1	2.63	43.46
7.80	237.00	393E+08	15657.0	2.73	41.78
8.30	237.50	431E+08	17831.7	2.84	40.28
8.80	238.00	471E+08	20161.3	2.93	38.92
9.30	238.50	512E+08	22648.7	3.03	37.69

Hydrograph

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (0201)	5627.23	6.30	23.75	9.68	0.37
OUTFLOW: ID= 1 (0202)	5627.23	6.02	27.00	9.68	0.36

ADD HYD (0203)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0008):	1549.97	2.273	20.75	9.92
+ ID2= 2 (0202):	5627.23	6.023	27.00	9.68
ID = 3 (0203):	7177.20	7.869	25.25	9.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0301)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0112):	12793.89	11.725	35.00	11.71
+ ID2= 2 (0203):	7177.20	7.869	25.25	9.73
ID = 3 (0301):	19971.09	17.981	31.00	11.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0302) Routing time step (min)'= 15.00

Distance	Elevation	Manning
0.00	240.00	0.0700
110.00	239.00	0.0700
161.00	238.00	0.0700
250.00	237.00	0.0700
420.00	236.00	0.0700
560.00	235.00	0.0700
589.00	234.40	0.0700 / 0.0350
589.10	233.60	0.0350
594.00	233.60	0.0350
594.10	234.40	0.0350 / 0.0700
615.00	235.00	0.0700
740.00	236.00	0.0700
860.00	236.50	0.0700
1340.00	237.00	0.0700
1460.00	238.00	0.0700
1470.00	239.00	0.0700
1510.00	240.00	0.0700

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.27	233.87	770E+04	1.0	0.74	132.49
0.53	234.13	155E+05	2.9	1.10	88.48
0.80	234.40	234E+05	5.5	1.37	71.19
1.15	234.75	636E+05	11.6	1.07	91.11
1.50	235.10	169E+06	24.5	0.85	114.97
1.85	235.45	431E+06	54.5	0.74	131.62
2.20	235.80	882E+06	117.3	0.78	125.39
2.55	236.15	153E+07	218.3	0.83	117.06
2.90	236.50	246E+07	375.3	0.89	109.30
3.25	236.85	394E+07	579.0	0.86	113.45
3.60	237.20	612E+07	1010.3	0.97	101.01

3.95	237.55	.851E+07	1631.3	1.12	86.99
4.30	237.90	.111E+08	2391.8	1.27	77.05
4.65	238.25	.137E+08	3337.5	1.42	68.52
5.00	238.60	.164E+08	4498.7	1.58	61.71
5.35	238.95	.192E+08	5671.0	1.73	56.40
5.70	239.30	.220E+08	6955.1	1.85	52.75
6.05	239.65	.249E+08	8355.2	1.96	49.76
6.40	240.00	.280E+08	9884.7	2.07	47.18

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (0301) ***** 17.98 31.00 11.00 1.32 0.95
OUTFLOW : ID= 1 (0302) ***** 17.74 33.25 11.00 1.31 0.95

```

```

-----
| ADD HYD (0303) |
| 1 + 2 = 3 |
-----
ID1= 1 (0012): 1501.52 1.873 23.25 10.25
+ ID2= 2 (0302): 19971.09 17.744 33.25 11.00
-----
ID = 3 (0303): 21472.61 18.990 32.50 10.95

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0011) | Area (ha)=3099.17 Curve Number (CN)= 57.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
| U.H. Tp(hrs)= 11.10 |
-----
Unit Hyd Qpeak (cms) = 10.949
PEAK FLOW (cms) = 3.491 (i)
TIME TO PEAK (hrs) = 23.000
RUNOFF VOLUME (mm) = 8.989
TOTAL RAINFALL (mm) = 51.240
RUNOFF COEFFICIENT = 0.175

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ROUTE CHN (0401) |
| IN= 2--> OUT= 1 | Routing time step (min) = 15.00

```

```

<----- DATA FOR SECTION ( 1.1) ----->
Distance Elevation Manning
0.00 236.00 0.0500
72.00 235.00 0.0500
492.00 230.00 0.0500
581.00 229.20 0.0500 /0.0350 Main Channel
582.00 228.40 0.0350 Main Channel
583.00 228.40 0.0350 Main Channel
584.00 229.20 0.0350 /0.0500 Main Channel
588.00 229.20 0.0500
607.00 230.00 0.0500
627.00 235.00 0.0500

```

```

----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.27 228.67 .302E+04 0.1 0.21 666.69
0.53 228.93 .756E+04 0.3 0.30 465.92
0.80 229.20 .136E+05 0.6 0.37 377.99
1.16 229.56 .111E+06 3.0 0.23 616.66
1.52 229.92 .358E+06 11.5 0.27 519.91
1.89 230.29 .740E+06 30.1 0.35 410.09
2.25 230.65 .122E+07 59.5 0.41 342.49
2.61 231.01 .180E+07 100.6 0.47 298.58
2.97 231.37 .248E+07 154.6 0.53 267.22
3.34 231.74 .326E+07 222.9 0.58 243.40
3.70 232.10 .413E+07 306.6 0.63 224.51
4.06 232.46 .510E+07 406.9 0.68 209.05
4.42 232.82 .617E+07 524.8 0.72 196.10
4.79 233.19 .734E+07 661.4 0.77 185.06
5.15 233.55 .861E+07 817.9 0.81 175.49
5.51 233.91 .998E+07 995.2 0.85 167.10
5.87 234.27 .114E+08 1194.3 0.89 159.67
6.24 234.64 .130E+08 1416.3 0.93 153.03
6.60 235.00 .147E+08 1662.2 0.96 147.05

```

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (0011) 3099.17 3.49 23.00 8.99 1.18 0.23
OUTFLOW : ID= 1 (0401) 3099.17 2.54 32.75 8.99 1.09 0.25

```

```

-----
| CALIB |
| NASHYD (0014) | Area (ha)=2163.09 Curve Number (CN)= 69.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
| U.H. Tp(hrs)= 25.10 |
-----

```

Unit Hyd Qpeak (cms) = 3.379

```

PEAK FLOW (cms) = 1.737 (i)
TIME TO PEAK (hrs) = 37.000
RUNOFF VOLUME (mm) = 13.334
TOTAL RAINFALL (mm) = 51.240
RUNOFF COEFFICIENT = 0.260

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (0402) |
| 1 + 2 = 3 |
-----
ID1= 1 (0014): 2163.09 1.737 37.00 13.33
+ ID2= 2 (0401): 3099.17 2.543 32.75 8.99
-----
ID = 3 (0402): 5262.26 4.239 33.75 10.77

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (0501) |
| 1 + 2 = 3 |
-----
ID1= 1 (0303): 21472.61 18.990 32.50 10.95
+ ID2= 2 (0402): 5262.26 4.239 33.75 10.77
-----
ID = 3 (0501): 26734.87 23.213 32.75 10.91

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (0503) |
| IN= 2--> OUT= 1 | Routing time step (min) = 15.00

```

```

<----- DATA FOR SECTION ( 1.1) ----->
Distance Elevation Manning
0.00 237.00 0.0700
425.00 235.00 0.0700
445.00 234.00 0.0700
480.00 233.50 0.0700
1535.00 233.50 0.0700 /0.0350 Main Channel
1536.50 232.70 0.0350 Main Channel
1537.50 232.70 0.0350 Main Channel
1540.00 233.50 0.0350 /0.0700 Main Channel
1605.00 233.20 0.0700
1610.00 233.10 0.0700
2135.00 233.10 0.0700
2440.00 233.50 0.0700
2505.00 233.50 0.0700
2530.00 234.00 0.0700
2535.00 235.00 0.0700
2540.00 236.00 0.0700
2605.00 238.00 0.0700
2645.00 241.00 0.0700

```

```

----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.20 232.90 .385E+03 0.2 0.55 38.92
0.40 233.10 .103E+04 0.6 0.80 26.75
0.60 233.30 .159E+04 39.5 0.32 67.02
0.80 233.50 .366E+04 133.3 0.47 45.74
1.03 233.73 .977E+04 428.7 0.56 37.97
1.27 233.97 .160E+05 914.3 0.74 29.09
1.50 234.20 .222E+05 1556.9 0.90 23.78
1.73 234.43 .285E+05 2336.2 1.05 20.32
1.97 234.67 .348E+05 3240.2 1.20 17.88
2.20 234.90 .411E+05 4260.9 1.33 16.07
2.43 235.13 .474E+05 5359.1 1.45 14.75
2.67 235.37 .539E+05 6537.6 1.56 13.74
2.90 235.60 .605E+05 7819.4 1.66 12.90
3.13 235.83 .673E+05 9203.8 1.76 12.19
3.37 236.07 .743E+05 10683.6 1.85 11.59
3.60 236.30 .814E+05 12247.8 1.93 11.07
3.83 236.53 .887E+05 13912.8 2.01 10.62
4.07 236.77 .961E+05 15679.0 2.09 10.22
4.30 237.00 .104E+06 17546.8 2.17 9.85

```

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (0501) ***** 23.21 32.75 10.91 0.52 0.43

```

OUTFLOW: ID= 1 (0503) ***** 23.14 33.75 10.91 0.52 0.43

RESERVOIR (0510)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	27.8300	83.9600
3.8000	14.0500	*****	101.7400
11.0800	33.6500	*****	145.2700
20.8900	61.8900	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
*****	23.142	33.75	10.91
*****	20.176	41.25	10.91

PEAK FLOW REDUCTION [Qout/Qin](%)= 87.18
 TIME SHIFT OF PEAK FLOW (min)=450.00
 MAXIMUM STORAGE USED (ha.m.)= 59.8355

ROUTE CHN (0513)
IN= 2--> OUT= 1

Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning
0.00	237.00	0.0700
425.00	235.00	0.0700
445.00	234.00	0.0700
480.00	233.50	0.0700
1535.00	233.50	0.0700 /0.0350 Main Channel
1536.50	232.70	0.0350 Main Channel
1537.50	232.70	0.0350 Main Channel
1540.00	233.50	0.0350 /0.0700 Main Channel
1605.00	233.20	0.0700
1610.00	233.10	0.0700
2135.00	233.10	0.0700
2440.00	233.50	0.0700
2505.00	233.50	0.0700
2530.00	234.00	0.0700
2535.00	235.00	0.0700
2540.00	236.00	0.0700
2605.00	238.00	0.0700
2645.00	241.00	0.0700

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.20	232.90	.291E+03	0.2	0.55	29.43
0.40	233.10	.777E+03	0.6	0.80	20.23
0.60	233.30	1.20E+06	39.5	0.32	50.68
0.80	233.50	.277E+06	133.3	0.47	34.59
1.03	233.73	.739E+06	428.7	0.56	28.72
1.27	233.97	1.21E+07	914.3	0.74	22.00
1.50	234.20	1.68E+07	1556.9	0.90	17.98
1.73	234.43	2.15E+07	2336.2	1.05	15.37
1.97	234.67	2.63E+07	3240.2	1.20	13.52
2.20	234.90	3.11E+07	4260.9	1.33	12.15
2.43	235.13	3.59E+07	5359.1	1.45	11.15
2.67	235.37	4.08E+07	6537.6	1.56	10.39
2.90	235.60	4.58E+07	7813.4	1.66	9.76
3.13	235.83	5.09E+07	9203.8	1.76	9.22
3.37	236.07	5.62E+07	10683.6	1.85	8.76
3.60	236.30	6.15E+07	12247.8	1.93	8.37
3.83	236.53	6.70E+07	13912.8	2.01	8.03
4.07	236.77	7.27E+07	15679.0	2.09	7.73
4.30	237.00	7.84E+07	17546.8	2.17	7.45

<---- hydrograph ----> <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
*****	20.18	41.25	10.91	0.50	0.46
*****	20.15	42.25	10.91	0.50	0.46

ADD HYD (0504)
1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0015): 1059.88	2.358	20.75	15.25
+ ID2= 2 (0513): 26794.87	20.153	42.25	10.91
ID = 3 (0504): 27794.75	20.464	41.75	11.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0505)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0013): 1627.59	1.790	27.50	11.68	
+ ID2= 2 (0504): 27794.75	20.464	41.75	11.08	
ID = 3 (0505): 29422.34	21.562	40.50	11.11	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0506)
IN= 2--> OUT= 1

Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning
0.00	251.00	0.0500
323.00	238.00	0.0500
400.00	229.00	0.0500
545.00	228.00	0.0500
602.00	226.00	0.0500
604.00	225.80	0.0500 /0.0350 Main Channel
605.00	225.00	0.0350 Main Channel
609.00	225.00	0.0350 Main Channel
610.00	225.80	0.0350 /0.0500 Main Channel
614.00	226.00	0.0500
650.00	227.00	0.0500
810.00	229.50	0.0500
1855.00	240.00	0.0500
2617.00	259.00	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.80	225.80	.292E+05	3.5	0.87	139.36
2.13	227.13	.441E+06	50.6	0.84	145.20
3.45	228.45	.197E+07	262.2	0.97	125.42
4.78	229.78	.557E+07	971.2	1.27	95.57
6.11	231.11	1.06E+08	3236.5	1.61	75.63
7.43	232.43	1.69E+08	4425.6	1.90	63.77
8.76	233.76	2.47E+08	7361.8	2.17	55.90
10.08	235.08	3.38E+08	11229.7	2.42	50.22
11.41	236.41	4.44E+08	16121.0	2.65	45.87
12.74	237.74	5.63E+08	22124.8	2.86	42.40
14.06	239.06	6.97E+08	29005.2	3.03	40.02
15.39	240.39	8.46E+08	37431.0	3.23	37.66
16.72	241.72	1.01E+09	47908.6	3.47	34.97
18.04	243.04	1.17E+09	59742.1	3.71	32.73
19.37	244.37	1.35E+09	72949.0	3.94	30.83
20.69	245.69	1.53E+09	87549.6	4.16	29.20
22.02	247.02	1.73E+09	*****	4.37	27.79
23.35	248.35	1.93E+09	*****	4.58	26.55
24.67	249.67	2.14E+09	*****	4.77	25.45

<---- hydrograph ----> <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
*****	21.56	40.50	11.11	1.31	0.86
*****	21.31	43.25	11.11	1.30	0.86

RESERVOIR (0512)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
*****	0.0000	*****	16.2850
27.2000	3.7650	*****	20.9160
76.8000	7.7410	*****	25.9120
*****	11.0650	*****	36.6600
*****	11.9150	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
*****	21.307	43.25	11.11
*****	21.301	43.50	11.11

PEAK FLOW REDUCTION [Qout/Qin](%)= 99.97
 TIME SHIFT OF PEAK FLOW (min)= 15.00
 MAXIMUM STORAGE USED (ha.m.)= 2.9485

ROUTE CHN (0514)
IN= 2--> OUT= 1

Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning
0.00	230.00	0.0500
400.00	229.00	0.0500
545.00	228.00	0.0500
602.00	226.00	0.0500
604.00	225.80	0.0500 /0.0350 Main Channel

605.00 225.00 0.0350 Main Channel
 609.00 225.00 0.0350 Main Channel
 610.00 225.80 0.0350 /0.0500 Main Channel
 614.00 226.00 0.0500
 650.00 227.00 0.0500
 810.00 229.50 0.0500

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.20	225.20	.243E+04	0.3	0.39	121.91
0.40	225.40	.515E+04	1.1	0.59	80.58
0.60	225.60	.815E+04	2.1	0.74	64.02
0.80	225.80	.114E+05	3.5	0.87	54.69
1.05	226.05	.184E+05	6.1	0.94	50.48
1.29	226.29	.346E+05	10.3	0.85	55.99
1.54	226.54	.620E+05	17.4	0.80	69.38
1.79	226.79	.101E+06	28.3	0.81	59.18
2.03	227.03	.151E+06	43.8	0.83	57.35
2.28	227.28	.215E+06	63.9	0.85	55.99
2.53	227.53	.295E+06	91.4	0.89	53.82
2.77	227.77	.391E+06	127.0	0.93	51.35
3.02	228.02	.504E+06	170.6	0.97	49.24
3.27	228.27	.644E+06	215.5	0.96	49.84
3.51	228.51	.821E+06	279.7	0.97	48.92
3.76	228.76	.103E+07	363.1	1.00	47.47
4.01	229.01	.128E+07	466.2	1.04	45.89
4.25	229.25	.159E+07	572.0	1.03	46.41
4.50	229.50	.198E+07	719.1	1.04	45.95

----- hydrograph ----->

INFLOW : ID= 2 (0512)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
*****	21.30	43.50	11.11	1.63	0.80	
OUTFLOW : ID= 1 (0514)	*****	21.25	44.50	11.11	1.63	0.80

ADD HYD (0507)

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0016):	3094.49	4.001	30.00	15.78
+ ID2= 2 (0514):	29422.34	21.253	44.50	11.11
*****	*****	*****	*****	*****
ID = 3 (0507):	32516.83	23.955	43.00	11.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ** SIMULATION NUMBER: 2 **

READ STORM

Filename: C:\Users\jccott\AppData
 Local\Temp\
 88073fbd-731f-4992-8e5b-76831528f0f7\631bf607
 Ptotal= 72.50 mm
 Comments: Syr/24hr

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.25	0.00	6.50	6.16	12.75	2.54	19.00	0.36
0.50	0.36	6.75	6.16	13.00	2.54	19.25	0.36
0.75	0.36	7.00	6.16	13.25	2.54	19.50	0.36
1.00	0.36	7.25	6.16	13.50	2.54	19.75	0.36
1.25	0.36	7.50	6.16	13.75	2.54	20.00	0.36
1.50	0.36	7.75	6.16	14.00	2.54	20.25	0.36
1.75	0.36	8.00	6.16	14.25	2.54	20.50	0.36
2.00	0.36	8.25	6.16	14.50	1.45	20.75	0.36
2.25	0.36	8.50	16.68	14.75	1.45	21.00	0.36
2.50	0.36	8.75	16.68	15.00	1.45	21.25	0.36
2.75	0.36	9.00	16.68	15.25	1.45	21.50	0.36
3.00	0.36	9.25	16.68	15.50	1.45	21.75	0.36
3.25	0.36	9.50	16.68	15.75	1.45	22.00	0.36
3.50	0.36	9.75	16.68	16.00	1.45	22.25	0.36
3.75	0.36	10.00	16.68	16.25	1.45	22.50	0.36
4.00	0.36	10.25	16.68	16.50	0.73	22.75	0.36
4.25	0.36	10.50	4.71	16.75	0.73	23.00	0.36
4.50	2.18	10.75	4.71	17.00	0.73	23.25	0.36
4.75	2.18	11.00	4.71	17.25	0.73	23.50	0.36
5.00	2.18	11.25	4.71	17.50	0.73	23.75	0.36
5.25	2.18	11.50	4.71	17.75	0.73	24.00	0.36
5.50	2.18	11.75	4.71	18.00	0.73	24.25	0.36
5.75	2.18	12.00	4.71	18.25	0.73		
6.00	2.18	12.25	4.71	18.50	0.36		
6.25	2.18	12.50	2.54	18.75	0.36		

CALIB NASHVD (0016) Area (ha)=3094.49 Curve Number (CN)= 74.0

ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 18.10

Unit Hyd Qpeak (cms)= 6.704

PEAK FLOW (cms)= 7.377 (1)
 TIME TO PEAK (hrs)= 29.750
 RUNOFF VOLUME (mm)= 29.068
 TOTAL RAINFALL (mm)= 72.500
 RUNOFF COEFFICIENT = 0.401

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHVD (0013) Area (ha)=1627.59 Curve Number (CN)= 65.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 15.50

Unit Hyd Qpeak (cms)= 4.118

PEAK FLOW (cms)= 3.423 (1)
 TIME TO PEAK (hrs)= 27.250
 RUNOFF VOLUME (mm)= 22.305
 TOTAL RAINFALL (mm)= 72.500
 RUNOFF COEFFICIENT = 0.308

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHVD (0015) Area (ha)=1059.88 Curve Number (CN)= 73.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 9.30

Unit Hyd Qpeak (cms)= 4.469

PEAK FLOW (cms)= 4.382 (1)
 TIME TO PEAK (hrs)= 20.500
 RUNOFF VOLUME (mm)= 28.222
 TOTAL RAINFALL (mm)= 72.500
 RUNOFF COEFFICIENT = 0.389

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHVD (0012) Area (ha)=1501.52 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 11.50

Unit Hyd Qpeak (cms)= 5.120

PEAK FLOW (cms)= 3.633 (1)
 TIME TO PEAK (hrs)= 23.250
 RUNOFF VOLUME (mm)= 19.819
 TOTAL RAINFALL (mm)= 72.500
 RUNOFF COEFFICIENT = 0.273

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHVD (0093) Area (ha)= 381.73 Curve Number (CN)= 69.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 14.90

Unit Hyd Qpeak (cms)= 1.005

PEAK FLOW (cms)= 0.936 (1)
 TIME TO PEAK (hrs)= 26.750
 RUNOFF VOLUME (mm)= 25.087
 TOTAL RAINFALL (mm)= 72.500
 RUNOFF COEFFICIENT = 0.346

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHVD (0010) Area (ha)=2842.76 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 10.80

Unit Hyd Qpeak (cms)= 10.322

PEAK FLOW (cms)= 7.833 (1)
 TIME TO PEAK (hrs)= 22.250
 RUNOFF VOLUME (mm)= 21.658
 TOTAL RAINFALL (mm)= 72.500
 RUNOFF COEFFICIENT = 0.299

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHVD (0007) Area (ha)=1373.84 Curve Number (CN)= 73.0 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10 U.H. Tp(hrs)= 10.70

Unit Hyd Qpeak (cms)= 5.035 PEAK FLOW (cms)= 5.050 (i) TIME TO PEAK (hrs)= 22.000 RUNOFF VOLUME (mm)= 28.222 TOTAL RAINFALL (mm)= 72.500 RUNOFF COEFFICIENT = 0.389

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHVD (0005) Area (ha)=1444.47 Curve Number (CN)= 60.0 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10 U.H. Tp(hrs)= 8.00

Unit Hyd Qpeak (cms)= 7.080 PEAK FLOW (cms)= 4.575 (i) TIME TO PEAK (hrs)= 19.250 RUNOFF VOLUME (mm)= 19.238 TOTAL RAINFALL (mm)= 72.500 RUNOFF COEFFICIENT = 0.265

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHVD (0006) Area (ha)=1504.44 Curve Number (CN)= 73.0 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10 U.H. Tp(hrs)= 12.20

Unit Hyd Qpeak (cms)= 4.836 PEAK FLOW (cms)= 4.942 (i) TIME TO PEAK (hrs)= 23.750 RUNOFF VOLUME (mm)= 28.222 TOTAL RAINFALL (mm)= 72.500 RUNOFF COEFFICIENT = 0.389

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHVD (0001) Area (ha)=2873.64 Curve Number (CN)= 61.0 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10 U.H. Tp(hrs)= 11.00

Unit Hyd Qpeak (cms)= 10.244 PEAK FLOW (cms)= 7.220 (i) TIME TO PEAK (hrs)= 22.500 RUNOFF VOLUME (mm)= 19.819 TOTAL RAINFALL (mm)= 72.500 RUNOFF COEFFICIENT = 0.273

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHVD (0002) Area (ha)= 988.72 Curve Number (CN)= 58.0 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10 U.H. Tp(hrs)= 9.10

Unit Hyd Qpeak (cms)= 4.261 PEAK FLOW (cms)= 2.658 (i) TIME TO PEAK (hrs)= 20.500 RUNOFF VOLUME (mm)= 18.121 TOTAL RAINFALL (mm)= 72.500 RUNOFF COEFFICIENT = 0.250

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0101) 1 + 2 = 3 AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm) ID= 1 (0001): 2873.64 7.220 22.50 19.82

+ ID2= 2 (0002): 988.72 2.658 20.50 18.12 ID = 3 (0101): 3862.36 9.813 22.00 19.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CEN (0102) IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (1.1) Distance Elevation Manning 0.00 250.00 0.0700 43.00 240.00 0.0700 /0.0350 Main Channel 44.00 239.20 0.0350 Main Channel 45.00 239.20 0.0350 Main Channel 46.00 240.00 0.0350 /0.0700 Main Channel 63.50 241.00 0.0700 71.00 250.00 0.0700

TRAVEL TIME TABLE

DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME (m) (m) (cu.m.) (cms) (m/s) (min) 0.40 239.60 .642E+04 0.5 0.80 221.73 0.80 240.00 .171E+05 1.8 1.15 155.20 1.39 240.59 .764E+05 8.0 1.12 159.62 1.98 241.18 .214E+06 23.7 1.19 150.23 2.56 241.76 .385E+06 51.8 1.44 123.91 3.15 242.35 .575E+06 90.1 1.68 106.38 3.74 242.94 .784E+06 138.5 1.89 94.40 4.33 243.53 .101E+07 196.9 2.08 85.71 4.92 244.12 .126E+07 265.6 2.26 79.08 5.51 244.71 .153E+07 344.7 2.42 73.81 6.09 245.29 .181E+07 434.5 2.57 69.50 6.68 245.88 .212E+07 535.2 2.71 65.89 7.27 246.47 .244E+07 647.3 2.84 62.81 7.86 247.06 .278E+07 771.1 2.97 60.13 8.45 247.65 .314E+07 906.8 3.09 57.77 9.04 248.24 .352E+07 1054.8 3.20 55.67 9.62 248.82 .392E+07 1215.5 3.32 53.79 10.21 249.41 .434E+07 1389.1 3.42 52.09 10.80 250.00 .478E+07 1576.1 3.53 50.53

hydrograph AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL (ha) (cms) (hrs) (mm) (m) (m/s) INFLOW : ID= 2 (0101) 3862.36 9.81 22.00 19.38 1.46 1.12 OUTFLOW: ID= 1 (0102) 3862.36 9.30 24.75 19.38 1.44 1.12

ADD HYD (0103) 1 + 2 = 3 AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm) ID= 1 (0006): 1504.44 4.942 23.75 28.22 + ID2= 2 (0102): 3862.36 9.297 24.75 19.38 ID = 3 (0103): 5366.80 14.216 24.50 21.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0104) 1 + 2 = 3 AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm) ID= 1 (0005): 1444.47 4.575 19.25 19.24 + ID2= 2 (0103): 5366.80 14.216 24.50 21.86 ID = 3 (0104): 6811.27 18.023 23.25 21.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CEN (0105) IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (1.1) Distance Elevation Manning 0.00 237.00 0.1000 330.00 235.00 0.1000 /0.0350 Main Channel 331.00 234.20 0.0350 Main Channel 332.00 234.20 0.0350 Main Channel 333.00 235.00 0.0350 /0.1000 Main Channel 425.00 235.10 0.1000 698.00 236.00 0.1000 1101.00 239.00 0.1000

TRAVEL TIME TABLE

DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME

(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.13	234.33	490E+03	0.1	0.64	82.66
0.27	234.47	.112E+04	0.3	0.93	56.72
0.40	234.60	.189E+04	0.7	1.14	45.94
0.53	234.73	.260E+04	1.2	1.33	39.64
0.67	234.87	.385E+04	1.8	1.49	35.34
0.80	235.00	.504E+04	2.6	1.63	32.16
0.95	235.15	.442E+05	6.3	0.45	116.42
1.11	235.31	.138E+06	17.7	0.44	120.51
1.26	235.46	.247E+06	38.2	0.49	107.48
1.42	235.62	.400E+06	69.8	0.55	95.60
1.57	235.77	.589E+06	114.0	0.61	86.08
1.72	235.92	.812E+06	172.6	0.67	78.45
1.88	236.08	.107E+07	250.3	0.74	71.19
2.03	236.23	.135E+07	347.8	0.81	64.71
2.18	236.38	.165E+07	461.9	0.88	59.65
2.34	236.54	.198E+07	593.4	0.95	55.58
2.49	236.69	.233E+07	742.8	1.01	52.21
2.65	236.85	.270E+07	910.6	1.06	49.36
2.80	237.00	.309E+07	1097.6	1.12	46.91

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<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLW : ID= 2 (0104) 6811.27 18.02 23.25 21.31 1.11 0.44
OUTFLOW: ID= 1 (0105) 6811.27 17.48 25.50 21.31 1.10 0.44

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| CALIB |
| NASHVD (0091) | Area (ha)= 281.97 Curve Number (CN)= 65.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
| U.H. Tp(hrs)= 5.80

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Unit Hyd Qpeak (cms)= 1.906
PEAK FLOW (cms)= 1.324 (i)
TIME TO PEAK (hrs)= 16.750
RUNOFF VOLUME (mm)= 22.305
TOTAL RAINFALL (mm)= 72.500
RUNOFF COEFFICIENT = 0.308

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(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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| ADD HYD (0106) |
| 1 + 2 = 3 |
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0091): 281.97 1.324 16.75 22.31
+ ID2= 2 (0105): 6811.27 17.480 25.50 21.31
=====
ID = 3 (0106): 7093.24 18.055 25.00 21.35

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ADD HYD (0107) |
| 1 + 2 = 3 |
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0091): 1373.84 5.050 22.00 28.22
+ ID2= 2 (0106): 7093.24 18.055 25.00 21.35
=====
ID = 3 (0107): 8467.08 22.895 24.25 22.46

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ROUTE CHN (0108) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

```

```

<----- DATA FOR SECTION ( 1.1) ----->
Distance Elevation Manning
0.00 234.00 0.1000
400.00 231.00 0.1000 /0.0350 Main Channel
401.00 229.70 0.0350 Main Channel
402.00 229.70 0.0350 Main Channel
404.00 230.50 0.0350 /0.1000 Main Channel
495.00 231.00 0.1000
1234.00 256.00 0.1000

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.20 229.90 .151E+04 0.1 0.24 395.61
0.40 230.10 .377E+04 0.2 0.35 274.62
0.60 230.30 .677E+04 0.5 0.43 222.06
0.80 230.50 .105E+05 0.9 0.50 190.52
1.03 230.73 .437E+05 2.0 0.27 356.49
1.27 230.97 .134E+06 5.0 0.21 448.69

```

1.50	231.20	.278E+06	11.4	0.23	405.59
1.73	231.43	.473E+06	21.6	0.26	364.80
1.97	231.67	.718E+06	36.0	0.29	332.76
2.20	231.90	.101E+07	55.1	0.31	306.87
2.43	232.13	.136E+07	79.4	0.33	285.44
2.67	232.37	.176E+07	109.6	0.36	267.34
2.90	232.60	.221E+07	145.9	0.38	251.83
3.13	232.83	.270E+07	189.0	0.40	238.35
3.37	233.07	.325E+07	239.3	0.42	226.51
3.60	233.30	.385E+07	297.1	0.44	216.02
3.83	233.53	.450E+07	363.0	0.46	206.64
4.07	233.77	.520E+07	437.3	0.48	198.20
4.30	234.00	.595E+07	520.5	0.50	190.55

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLW : ID= 2 (0107) 8467.08 22.90 24.25 22.46 1.75 0.26
OUTFLOW: ID= 1 (0108) 8467.08 19.18 29.50 22.46 1.68 0.25

```

```

| CALIB |
| NASHVD (0092) | Area (ha)=1102.32 Curve Number (CN)= 64.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
| U.H. Tp(hrs)= 16.50

```

```

Unit Hyd Qpeak (cms)= 2.620
PEAK FLOW (cms)= 2.128 (i)
TIME TO PEAK (hrs)= 28.250
RUNOFF VOLUME (mm)= 21.658
TOTAL RAINFALL (mm)= 72.500
RUNOFF COEFFICIENT = 0.299

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

| ADD HYD (0109) |
| 1 + 2 = 3 |
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0092): 1102.32 2.128 28.25 21.66
+ ID2= 2 (0108): 8467.08 19.183 29.50 22.46
=====
ID = 3 (0109): 9569.40 21.301 29.50 22.37

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ADD HYD (0110) |
| 1 + 2 = 3 |
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0110): 2842.76 7.933 22.25 21.66
+ ID2= 2 (0109): 9569.40 21.301 29.50 22.37
=====
ID = 3 (0110): 12412.16 27.735 27.75 22.21

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ROUTE CHN (0111) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

```

```

<----- DATA FOR SECTION ( 1.1) ----->
Distance Elevation Manning
0.00 236.00 0.1000
275.00 232.00 0.1000
1039.00 230.00 0.1000 /0.0350 Main Channel
1040.00 229.20 0.0350 Main Channel
1041.00 229.20 0.0350 Main Channel
1043.00 230.00 0.0350 /0.1000 Main Channel
1117.00 230.60 0.1000
1325.00 234.00 0.1000

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.20 229.40 .985E+03 0.0 0.14 431.34
0.40 229.60 .251E+04 0.1 0.20 298.03
0.60 229.80 .457E+04 0.3 0.25 239.97
0.80 230.00 .717E+04 0.6 0.29 205.17
1.07 230.27 .754E+05 2.2 0.11 561.90
1.33 230.53 .272E+06 8.6 0.11 527.12
1.60 230.80 .594E+06 22.7 0.14 436.37
1.87 231.07 .103E+07 46.2 0.16 371.52
2.13 231.33 .158E+07 80.7 0.18 325.64
2.40 231.60 .224E+07 127.8 0.20 291.74
2.67 231.87 .301E+07 189.0 0.22 265.46
2.93 232.13 .389E+07 271.8 0.25 238.27

```


3.20	232.40	.481E+07	276.4	0.28	212.79
3.47	232.67	.576E+07	496.0	0.31	193.49
3.73	232.93	.674E+07	630.4	0.33	178.30
4.00	233.20	.776E+07	779.4	0.36	165.99
4.27	233.47	.881E+07	943.0	0.38	155.79
4.53	233.73	.990E+07	1121.1	0.41	147.16
4.80	234.00	.110E+08	1313.9	0.43	139.76

<---- hydrograph ----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLW : ID= 2 (0110) *****	27.74	27.75	22.21	1.66	0.14
OUTFLOW: ID= 1 (0111) *****	22.99	33.50	22.21	1.60	0.14

ADD HYD (0112) |

1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)	(mm)
ID1= 1 (0093):	381.73	0.936	26.75	25.09
+ ID2= 2 (0111):	12412.16	22.990	33.50	22.21
ID = 3 (0112):	12793.89	23.791	33.25	22.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB

NASHVD (0008)	Area	(ha)=1549.97	Curve Number	(CN)= 60.0
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)=	3.10
	U.H. Tp(hrs)=	9.10		

Unit Hyd Qpeak (cms)= 6.679

PEAK FLOW	(cms)=	4.426 (1)
TIME TO PEAK	(hrs)=	20.500
RUNOFF VOLUME	(mm)=	19.238
TOTAL RAINFALL	(mm)=	72.500
RUNOFF COEFFICIENT	=	0.265

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

NASHVD (0003)	Area	(ha)=2954.54	Curve Number	(CN)= 64.0
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)=	3.10
	U.H. Tp(hrs)=	13.10		

Unit Hyd Qpeak (cms)= 8.844

PEAK FLOW	(cms)=	6.991 (1)
TIME TO PEAK	(hrs)=	24.750
RUNOFF VOLUME	(mm)=	21.658
TOTAL RAINFALL	(mm)=	72.500
RUNOFF COEFFICIENT	=	0.299

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

NASHVD (0004)	Area	(ha)=2672.69	Curve Number	(CN)= 53.0
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)=	3.10
	U.H. Tp(hrs)=	10.70		

Unit Hyd Qpeak (cms)= 9.795

PEAK FLOW	(cms)=	5.388 (1)
TIME TO PEAK	(hrs)=	22.250
RUNOFF VOLUME	(mm)=	15.564
TOTAL RAINFALL	(mm)=	72.500
RUNOFF COEFFICIENT	=	0.215

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0201) |

1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)	(mm)
ID1= 1 (0003):	2954.54	6.991	24.75	21.66
+ ID2= 2 (0004):	2672.69	5.388	22.25	15.56
ID = 3 (0201):	5627.23	12.267	23.75	18.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0202) |

IN= 2--> OUT= 1 | Routing time step (min)= 15.00

DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning
0.00	239.00	0.1000
690.00	230.00	0.1000 /0.0350 Main Channel
691.00	229.20	0.0350 Main Channel
692.00	229.20	0.0350 Main Channel
693.00	229.50	0.0350 /0.1000 Main Channel
1190.00	230.00	0.1000
1417.00	255.00	0.1000

TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.30	229.50	.347E+04	0.4	0.84	136.57
0.80	230.00	.864E+06	45.2	0.36	318.78
1.30	230.50	.265E+07	274.1	0.71	161.20
1.80	231.00	.459E+07	651.8	0.97	117.24
2.30	231.50	.667E+07	1165.3	1.20	95.34
2.80	232.00	.889E+07	1810.2	1.40	81.89
3.30	232.50	.113E+08	2585.1	1.57	72.65
3.80	233.00	.138E+08	3490.4	1.73	65.85
4.30	233.50	.165E+08	4527.4	1.89	60.59
4.80	234.00	.193E+08	5697.8	2.03	56.38
5.30	234.50	.222E+08	7003.8	2.16	52.91
5.80	235.00	.253E+08	8447.7	2.28	50.00
6.30	235.50	.286E+08	10032.3	2.40	47.52
6.80	236.00	.320E+08	11760.1	2.52	45.36
7.30	236.50	.356E+08	13634.1	2.63	43.46
7.80	237.00	.393E+08	15657.0	2.73	41.78
8.30	237.50	.431E+08	17831.7	2.84	40.28
8.80	238.00	.471E+08	20161.3	2.93	38.92
9.30	238.50	.512E+08	22648.7	3.03	37.69

<---- hydrograph ----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLW : ID= 2 (0201) 5627.23	12.27	23.75	18.76	0.43	0.62
OUTFLOW: ID= 1 (0202) 5627.23	11.56	27.50	18.76	0.42	0.63

ADD HYD (0203) |

1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)	(mm)
ID1= 1 (0008):	1549.97	4.426	20.50	19.24
+ ID2= 2 (0203):	5627.23	11.563	27.50	18.76
ID = 3 (0203):	7177.20	15.026	25.50	18.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0301) |

1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)	(mm)
ID1= 1 (0112):	12793.89	23.791	33.25	22.29
+ ID2= 2 (0203):	7177.20	15.026	25.50	18.87
ID = 3 (0301):	19971.09	36.610	30.50	21.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0302) |

IN= 2--> OUT= 1 | Routing time step (min)= 15.00

DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning
0.00	240.00	0.0700
110.00	239.00	0.0700
161.00	238.00	0.0700
250.00	237.00	0.0700
420.00	236.00	0.0700
560.00	235.00	0.0700
589.00	234.40	0.0700 /0.0350 Main Channel
589.10	233.60	0.0350 Main Channel
594.00	233.60	0.0350 Main Channel
594.10	234.40	0.0350 /0.0700 Main Channel
615.00	235.00	0.0700
740.00	236.00	0.0700
860.00	236.50	0.0700
1340.00	237.00	0.0700
1460.00	238.00	0.0700
1470.00	239.00	0.0700
1510.00	240.00	0.0700

TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.30	239.50	.347E+04	0.4	0.84	136.57
0.80	240.00	.864E+06	45.2	0.36	318.78
1.30	240.50	.265E+07	274.1	0.71	161.20
1.80	241.00	.459E+07	651.8	0.97	117.24
2.30	241.50	.667E+07	1165.3	1.20	95.34
2.80	242.00	.889E+07	1810.2	1.40	81.89
3.30	242.50	.113E+08	2585.1	1.57	72.65
3.80	243.00	.138E+08	3490.4	1.73	65.85
4.30	243.50	.165E+08	4527.4	1.89	60.59
4.80	244.00	.193E+08	5697.8	2.03	56.38
5.30	244.50	.222E+08	7003.8	2.16	52.91
5.80	245.00	.253E+08	8447.7	2.28	50.00
6.30	245.50	.286E+08	10032.3	2.40	47.52
6.80	246.00	.320E+08	11760.1	2.52	45.36
7.30	246.50	.356E+08	13634.1	2.63	43.46
7.80	247.00	.393E+08	15657.0	2.73	41.78
8.30	247.50	.431E+08	17831.7	2.84	40.28
8.80	248.00	.471E+08	20161.3	2.93	38.92
9.30	248.50	.512E+08	22648.7	3.03	37.69

(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.27	233.87	.770E+04	1.0	0.74	132.49
0.53	234.13	.155E+05	2.9	1.10	88.48
0.80	234.40	.234E+05	5.5	1.37	71.19
1.15	234.75	.636E+05	11.6	1.07	91.11
1.50	235.10	.159E+06	24.5	0.85	114.97
1.85	235.45	.431E+06	54.5	0.74	131.62
2.20	235.80	.862E+06	117.3	0.78	125.39
2.55	236.15	.153E+07	219.3	0.83	117.06
2.90	236.50	.246E+07	375.3	0.89	109.30
3.25	236.85	.394E+07	579.0	0.86	113.45
3.60	237.20	.612E+07	1010.3	0.97	101.01
3.95	237.55	.851E+07	1631.3	1.12	86.99
4.30	237.90	.111E+08	2391.8	1.27	77.05
4.65	238.25	.137E+08	3337.5	1.42	68.52
5.00	238.60	.164E+08	4438.7	1.58	61.71
5.35	238.95	.192E+08	5671.0	1.73	56.40
5.70	239.30	.220E+08	6955.1	1.85	52.75
6.05	239.65	.249E+08	8355.2	1.96	49.76
6.40	240.00	.280E+08	9884.7	2.07	47.18

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLW : ID= 2 (0301) ***** 36.61 30.50 21.06 1.64 0.80
OUTFLOW: ID= 1 (0302) ***** 35.93 33.00 21.06 1.63 0.80

```

ADD HYD (0303)	1	2	3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (0301):	1501.52	3.633	23.25	19.82			
+ ID2= 2 (0302):	19971.09	35.930	33.00	21.06			
ID = 3 (0303):	21472.61	38.373	32.00	20.97			

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0011)	Area (ha)	(ha)=3099.17	Curve Number (CN)= 57.0
ID= 1 DT=15.0 min	1a	(mm)= 5.00	# of Linear Res.(N)= 3.10
	U.H. Tp(hrs)=	11.10	

Unit Hyd Qpeak (cms)= 10.949

```

PEAK FLOW (cms)= 6.850 (i)
TIME TO PEAK (hrs)= 22.750
RUNOFF VOLUME (mm)= 17.584
TOTAL RAINFALL (mm)= 72.500
RUNOFF COEFFICIENT = 0.243

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (0401)	Routing time step (min)'= 15.00
------------------	---------------------------------

----- DATA FOR SECTION (1.1) -----			
Distance	Elevation	Manning	
0.00	236.00	0.0500	
72.00	235.00	0.0500	
492.00	230.00	0.0500	
581.00	229.20	0.0500 / 0.0350	Main Channel
582.00	228.40	0.0350	Main Channel
583.00	228.40	0.0350	Main Channel
584.00	229.20	0.0350 / 0.0500	Main Channel
588.00	229.20	0.0500	
607.00	230.00	0.0500	
627.00	235.00	0.0500	

----- TRAVEL TIME TABLE -----						
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)	
0.27	228.67	.302E+04	0.1	0.21	666.69	
0.53	228.93	.756E+04	0.3	0.30	465.92	
0.80	229.20	.136E+05	0.6	0.37	377.99	
1.16	229.56	.111E+06	3.0	0.23	616.66	
1.52	229.92	.358E+06	11.5	0.27	519.91	
1.89	230.29	.740E+06	30.1	0.35	410.09	
2.25	230.65	.122E+07	59.5	0.41	342.49	
2.61	231.01	.180E+07	100.6	0.47	298.58	
2.97	231.37	.248E+07	154.6	0.53	267.22	
3.34	231.74	.326E+07	222.9	0.58	243.40	
3.70	232.10	.413E+07	306.6	0.63	224.51	
4.06	232.46	.510E+07	406.9	0.68	209.05	
4.42	232.82	.617E+07	524.8	0.72	196.10	
4.79	233.19	.734E+07	661.4	0.77	185.06	
5.15	233.55	.861E+07	817.9	0.81	175.49	
5.51	233.91	.998E+07	995.2	0.85	167.10	

5.87	234.27	.114E+08	1194.3	0.89	159.67
6.24	234.64	.130E+08	1416.3	0.93	153.03
6.60	235.00	.147E+08	1662.2	0.96	147.05

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLW : ID= 2 (0011) 3099.17 6.85 22.75 17.58 1.33 0.25
OUTFLOW: ID= 1 (0401) 3099.17 4.86 30.50 17.58 1.24 0.24

```

CALIB NASHYD (0014)	Area (ha)	(ha)=2163.09	Curve Number (CN)= 69.0
ID= 1 DT=15.0 min	1a	(mm)= 5.00	# of Linear Res.(N)= 3.10
	U.H. Tp(hrs)=	25.10	

Unit Hyd Qpeak (cms)= 3.379

```

PEAK FLOW (cms)= 3.271 (i)
TIME TO PEAK (hrs)= 36.750
RUNOFF VOLUME (mm)= 25.087
TOTAL RAINFALL (mm)= 72.500
RUNOFF COEFFICIENT = 0.346

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0402)	1	2	3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (0014):	2163.09	3.271	36.75	25.09			
+ ID2= 2 (0401):	3099.17	4.859	30.50	17.58			
ID = 3 (0402):	5262.26	7.958	32.25	20.67			

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0501)	1	2	3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (0303):	21472.61	38.373	32.00	20.97			
+ ID2= 2 (0402):	5262.26	7.958	32.25	20.67			
ID = 3 (0501):	26734.87	46.330	32.00	20.91			

Unit Hyd Qpeak (cms)= 10.949

```

PEAK FLOW (cms)= 6.850 (i)
TIME TO PEAK (hrs)= 22.750
RUNOFF VOLUME (mm)= 17.584
TOTAL RAINFALL (mm)= 72.500
RUNOFF COEFFICIENT = 0.243

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (0503)	Routing time step (min)'= 15.00
------------------	---------------------------------

----- DATA FOR SECTION (1.1) -----			
Distance	Elevation	Manning	
0.00	237.00	0.0700	
425.00	235.00	0.0700	
445.00	234.00	0.0700	
480.00	233.50	0.0700	
1535.00	233.50	0.0700 / 0.0350	Main Channel
1536.50	232.70	0.0350	Main Channel
1537.50	232.70	0.0350	Main Channel
1540.00	233.50	0.0350 / 0.0700	Main Channel
1605.00	233.20	0.0700	
1610.00	233.10	0.0700	
2135.00	233.10	0.0700	
2440.00	233.50	0.0700	
2505.00	233.50	0.0700	
2530.00	234.00	0.0700	
2535.00	235.00	0.0700	
2540.00	236.00	0.0700	
2605.00	238.00	0.0700	
2645.00	241.00	0.0700	

----- TRAVEL TIME TABLE -----						
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)	
0.20	232.90	.385E+03	0.2	0.55	38.92	
0.40	233.10	.103E+04	0.6	0.80	26.75	
0.60	233.30	.159E+06	39.5	0.32	67.02	
0.80	233.50	.366E+06	133.3	0.47	45.74	
1.03	233.73	.977E+06	428.7	0.56	37.97	
1.27	233.97	.160E+07	914.3	0.74	29.09	
1.50	234.20	.222E+07	1556.9	0.90	23.78	
1.73	234.43	.285E+07	2336.2	1.05	20.32	
1.97	234.67	.348E+07	3240.2	1.20	17.88	
2.20	234.90	.411E+07	4260.9	1.33	16.07	
2.43	235.13	.474E+07	5359.1	1.45	14.75	
2.67	235.37	.539E+07	6537.6	1.56	13.74	

2.90	235.60	.605E+07	7819.4	1.66	12.90
3.13	235.83	.673E+07	9203.8	1.76	12.19
3.37	236.07	.743E+07	10683.6	1.85	11.59
3.60	236.30	.814E+07	12247.8	1.93	11.07
3.83	236.53	.887E+07	13912.8	2.01	10.62
4.07	236.77	.961E+07	15679.0	2.09	10.22
4.30	237.00	1.04E+08	17546.8	2.17	9.85

<---- hydrograph ----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (0501)	46.33	32.00	20.91	0.61	0.33
OUTFLOW : ID= 1 (0503)	46.17	33.00	20.91	0.61	0.33

RESERVOIR (0510)

IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	27.8300	83.9600
3.8000	14.0500	*****	101.7400
11.0800	33.6500	*****	145.2700
20.8900	61.8900	0.0000	0.0000

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0503)	46.169	33.00	20.91
OUTFLOW : ID= 1 (0510)	47.562	30.75	20.91

PEAK FLOW REDUCTION [Qout/Qin](%)=103.02
TIME SHIFT OF PEAK FLOW (min)=*****
MAXIMUM STORAGE USED (ha.m.)= 84.4928

**** WARNING : HYDROGRAPH PEAK WAS NOT REDUCED.
CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.

ROUTE CHN (0513)

IN= 2--> OUT= 1

Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning
0.00	237.00	0.0700
425.00	235.00	0.0700
445.00	234.00	0.0700
480.00	233.50	0.0700
1535.00	233.50	0.0700 /0.0350 Main Channel
1536.50	232.70	0.0350 Main Channel
1537.50	232.70	0.0350 Main Channel
1540.00	233.50	0.0350 /0.0700 Main Channel
1605.00	233.20	0.0700
1610.00	233.10	0.0700
2135.00	233.10	0.0700
2440.00	233.50	0.0700
2505.00	233.50	0.0700
2510.00	234.00	0.0700
2535.00	235.00	0.0700
2540.00	236.00	0.0700
2605.00	238.00	0.0700
2645.00	241.00	0.0700

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.20	232.90	.291E+03	0.2	0.55	29.43
0.40	233.10	.777E+03	0.6	0.80	20.23
0.60	233.30	1.20E+06	39.5	0.32	50.68
0.80	233.50	2.77E+06	133.3	0.47	34.59
1.03	233.73	.739E+06	428.7	0.56	28.72
1.27	233.97	1.21E+07	914.3	0.74	22.00
1.50	234.20	1.68E+07	1556.9	0.90	17.98
1.73	234.43	2.15E+07	2336.2	1.05	15.37
1.97	234.67	2.63E+07	3240.2	1.20	13.52
2.20	234.90	3.11E+07	4260.9	1.33	12.15
2.43	235.13	3.59E+07	5350.1	1.45	11.15
2.67	235.37	4.08E+07	6537.6	1.56	10.39
2.90	235.60	4.58E+07	7819.4	1.66	9.76
3.13	235.83	5.09E+07	9203.8	1.76	9.22
3.37	236.07	5.62E+07	10683.6	1.85	8.76
3.60	236.30	6.15E+07	12247.8	1.93	8.37
3.83	236.53	6.70E+07	13912.8	2.01	8.03
4.07	236.77	7.27E+07	15679.0	2.09	7.73
4.30	237.00	7.84E+07	17546.8	2.17	7.45

<---- hydrograph ----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (0510)	47.56	30.75	20.91	0.62	0.33
OUTFLOW : ID= 1 (0513)	45.86	34.50	20.91	0.61	0.33

ADD HYD (0504)

1 + 2 = 3

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0015):	1059.88	4.382	20.50
+ ID2= 2 (0513):	26734.87	45.862	34.50
ID = 3 (0504):	27794.75	47.406	34.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0505)

1 + 2 = 3

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0013):	1627.59	3.423	27.25
+ ID2= 2 (0504):	27794.75	47.406	34.50
ID = 3 (0505):	29422.34	50.345	34.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0506)

IN= 2--> OUT= 1

Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning
0.00	251.00	0.0500
323.00	238.00	0.0500
400.00	229.00	0.0500
545.00	228.00	0.0500
602.00	226.00	0.0500
604.00	225.80	0.0500 /0.0350 Main Channel
605.00	225.00	0.0350 Main Channel
609.00	225.00	0.0350 Main Channel
610.00	225.80	0.0350 /0.0500 Main Channel
614.00	226.00	0.0500
650.00	227.00	0.0500
810.00	229.50	0.0500
1855.00	240.00	0.0500
2617.00	259.00	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.80	225.80	.292E+05	3.5	0.87	139.36
2.13	227.13	.441E+06	50.6	0.84	145.20
3.45	228.45	.197E+07	262.2	0.97	125.42
4.78	229.78	.557E+07	971.2	1.27	95.57
6.11	231.11	1.05E+08	2326.5	1.61	75.63
7.43	232.43	1.69E+08	4425.6	1.90	63.77
8.76	233.76	2.47E+08	7361.8	2.17	55.90
10.08	235.08	3.38E+08	11229.7	2.42	50.22
11.41	236.41	4.44E+08	16121.0	2.65	45.87
12.74	237.74	5.63E+08	22124.8	2.86	42.40
14.06	239.06	6.97E+08	29005.2	3.03	40.02
15.39	240.39	8.46E+08	37433.0	3.23	37.66
16.72	241.72	1.01E+09	47908.6	3.47	34.97
18.04	243.04	1.17E+09	59742.1	3.71	32.73
19.37	244.37	1.35E+09	72949.0	3.94	30.83
20.69	245.69	1.53E+09	87549.6	4.16	29.20
22.02	247.02	1.73E+09	*****	4.37	27.79
23.35	248.35	1.93E+09	*****	4.58	26.55
24.67	249.67	2.14E+09	*****	4.77	25.45

<---- hydrograph ----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (0505)	47.42	37.75	21.25	2.12	0.84
OUTFLOW : ID= 1 (0506)	47.42	37.75	21.25	2.03	0.84

RESERVOIR (0512)

IN= 2--> OUT= 1

DT= 15.0 min

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	*****	16.2850
27.2000	3.7650	*****	20.9160
76.8000	7.7410	*****	25.9120
*****	11.0650	*****	36.6600
*****	11.9150	0.0000	0.0000

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0506)	47.421	37.75	21.25
OUTFLOW : ID= 1 (0512)	47.408	38.00	21.25

PEAK FLOW REDUCTION [Qout/Qin](%)= 99.97
TIME SHIFT OF PEAK FLOW (min)= 15.00

MAXIMUM STORAGE USED (ha.m.)= 5.3851

ROUTING TIME STEP (min)= 15.00

DATA FOR SECTION (1.1) table with columns: Distance, Elevation, Manning, and Main Channel

TRAVEL TIME TABLE table with columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV.TIME

hydrograph table with columns: AREA, QPEAK, TPEAK, R.V., MAX DEPTH, MAX VEL

ADD HYD (0507) table with columns: AREA, QPEAK, TPEAK, R.V.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

SIMULATION NUMBER: 3

READ STORM File name and path information

Hydrograph plot data table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN

Hydrograph data table with columns: Time, Flow, Time, Flow

CALIB NASHYD (0016) Area (ha)=3094.49 Curve Number (CN)= 74.0

Unit Hyd Qpeak (cms)= 6.704

PEAK FLOW (cms)= 9.907 (i) TIME TO PEAK (hrs)= 29.750

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0013) Area (ha)=1627.59 Curve Number (CN)= 65.0

Unit Hyd Qpeak (cms)= 4.118

PEAK FLOW (cms)= 4.688 (i) TIME TO PEAK (hrs)= 27.250

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0015) Area (ha)=1059.88 Curve Number (CN)= 73.0

Unit Hyd Qpeak (cms)= 4.469

PEAK FLOW (cms)= 5.908 (i) TIME TO PEAK (hrs)= 20.500

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0012) Area (ha)=1501.52 Curve Number (CN)= 61.0

Unit Hyd Qpeak (cms)= 5.120

PEAK FLOW (cms)= 5.015 (i) TIME TO PEAK (hrs)= 23.000

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0093) Area (ha)= 381.73 Curve Number (CN)= 69.0

Unit Hyd Qpeak (cms)= 1.005

PEAK FLOW (cms)= 1.271 (i) TIME TO PEAK (hrs)= 26.500

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHVD (0010) Area (ha)=2842.76 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 10.80

Unit Hyd Qpeak (cms)= 10.322
 PEAK FLOW (cms)= 10.894 (1)
 TIME TO PEAK (hrs)= 22.250
 RUNOFF VOLUME (mm)= 29.699
 TOTAL RAINFALL (mm)= 86.660
 RUNOFF COEFFICIENT = 0.343

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHVD (0007) Area (ha)=1373.84 Curve Number (CN)= 73.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 10.70

Unit Hyd Qpeak (cms)= 5.035
 PEAK FLOW (cms)= 6.806 (1)
 TIME TO PEAK (hrs)= 22.000
 RUNOFF VOLUME (mm)= 37.974
 TOTAL RAINFALL (mm)= 86.660
 RUNOFF COEFFICIENT = 0.438

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHVD (0005) Area (ha)=1444.47 Curve Number (CN)= 60.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 8.00

Unit Hyd Qpeak (cms)= 7.080
 PEAK FLOW (cms)= 6.332 (1)
 TIME TO PEAK (hrs)= 19.250
 RUNOFF VOLUME (mm)= 26.568
 TOTAL RAINFALL (mm)= 86.660
 RUNOFF COEFFICIENT = 0.307

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHVD (0006) Area (ha)=1504.44 Curve Number (CN)= 73.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 12.20

Unit Hyd Qpeak (cms)= 4.836
 PEAK FLOW (cms)= 6.658 (1)
 TIME TO PEAK (hrs)= 23.500
 RUNOFF VOLUME (mm)= 37.974
 TOTAL RAINFALL (mm)= 86.660
 RUNOFF COEFFICIENT = 0.438

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHVD (0001) Area (ha)=2873.64 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 11.00

Unit Hyd Qpeak (cms)= 10.244
 PEAK FLOW (cms)= 9.967 (1)
 TIME TO PEAK (hrs)= 22.500
 RUNOFF VOLUME (mm)= 27.323
 TOTAL RAINFALL (mm)= 86.660
 RUNOFF COEFFICIENT = 0.315

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHVD (0002) Area (ha)= 988.72 Curve Number (CN)= 58.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 9.10

Unit Hyd Qpeak (cms)= 4.261
 PEAK FLOW (cms)= 3.689 (1)

TIME TO PEAK (hrs)= 20.500
 RUNOFF VOLUME (mm)= 25.108
 TOTAL RAINFALL (mm)= 86.660
 RUNOFF COEFFICIENT = 0.290

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0101)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0001): 2873.64 9.967 22.50 27.32
 + ID2= 2 (0002): 988.72 3.689 20.50 25.11
 =====
 ID = 3 (0101): 3862.36 13.565 21.75 26.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0102)
 IN= 2--> OUT= 1
 Routing time step (min)= 15.00

----- DATA FOR SECTION (1.1) -----
 Distance Elevation Manning
 0.00 250.00 0.0700
 43.00 240.00 0.0700 /0.0350 Main Channel
 44.00 239.20 0.0350 Main Channel
 45.00 239.20 0.0350 Main Channel
 46.00 240.00 0.0350 /0.0700 Main Channel
 63.50 241.00 0.0700
 71.00 250.00 0.0700

----- TRAVEL TIME TABLE -----
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
 (m) (m) (cu.m.) (cms) (m/s) (min)
 0.40 239.60 .642E+04 0.5 0.60 221.73
 0.80 240.00 .171E+05 1.8 1.15 155.20
 1.39 240.59 .764E+05 8.0 1.12 159.62
 1.98 241.18 .214E+06 23.7 1.19 150.23
 2.56 241.76 .385E+06 51.8 1.44 123.91
 3.15 242.35 .575E+06 90.1 1.68 106.38
 3.74 242.94 .784E+06 138.5 1.89 94.40
 4.33 243.53 .101E+07 196.9 2.08 85.71
 4.92 244.12 .135E+07 265.6 2.26 79.08
 5.51 244.71 .153E+07 344.7 2.42 73.81
 6.09 245.29 .181E+07 434.5 2.57 69.50
 6.68 245.88 .212E+07 535.2 2.71 65.89
 7.27 246.47 .244E+07 647.3 2.84 62.81
 7.86 247.06 .278E+07 771.1 2.97 60.13
 8.45 247.65 .314E+07 906.8 3.09 57.77
 9.04 248.24 .352E+07 1054.8 3.20 55.67
 9.62 248.82 .392E+07 1215.5 3.32 53.79
 10.21 249.41 .434E+07 1389.1 3.42 52.09
 10.80 250.00 .478E+07 1576.1 3.53 50.53

----- hydrograph ----- <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0101) 3862.36 13.56 21.75 26.76 1.60 1.14
 OUTFLOW : ID= 1 (0102) 3862.36 12.88 24.75 26.76 1.57 1.14

ADD HYD (0103)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0006): 1504.44 6.658 23.50 37.97
 + ID2= 2 (0102): 3862.36 12.877 24.75 26.76
 =====
 ID = 3 (0103): 5366.80 19.505 24.25 29.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0104)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0005): 1444.47 6.332 19.25 26.57
 + ID2= 2 (0103): 5366.80 19.505 24.25 29.90
 =====
 ID = 3 (0104): 6811.27 24.795 23.00 29.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0105)
 IN= 2--> OUT= 1
 Routing time step (min)= 15.00

<----- DATA FOR SECTION (1.1) ----->			
Distance	Elevation	Manning	
0.00	237.00	0.1000	
330.00	235.00	0.1000 / 0.0350	Main Channel
331.00	234.20	0.0350	Main Channel
332.00	234.20	0.0350	Main Channel
333.00	235.00	0.0350 / 0.1000	Main Channel
425.00	235.10	0.1000	
698.00	236.00	0.1000	
1101.00	239.00	0.1000	

402.00	229.70	0.0350	Main Channel
404.00	230.50	0.0350 / 0.1000	Main Channel
495.00	231.00	0.1000	
1234.00	256.00	0.1000	

<----- TRAVEL TIME TABLE ----->						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.13	234.33	.490E+03	0.1	0.64	82.66	
0.27	234.47	.112E+04	0.3	0.93	66.72	
0.40	234.60	.189E+04	0.7	1.14	45.94	
0.53	234.73	.280E+04	1.2	1.33	39.64	
0.67	234.87	.385E+04	1.8	1.49	35.34	
0.80	235.00	.504E+04	2.6	1.63	32.16	
0.95	235.15	.442E+05	6.3	0.45	116.42	
1.11	235.31	.128E+06	17.7	0.44	120.51	
1.26	235.46	.247E+06	38.2	0.49	107.48	
1.42	235.62	.400E+06	69.8	0.55	95.60	
1.57	235.77	.589E+06	114.0	0.61	86.08	
1.72	235.92	.812E+06	172.6	0.67	78.45	
1.88	236.08	.107E+07	250.3	0.74	71.19	
2.03	236.23	.135E+07	347.8	0.81	64.71	
2.18	236.38	.165E+07	461.9	0.88	59.65	
2.34	236.54	.198E+07	593.4	0.95	55.58	
2.49	236.69	.233E+07	742.8	1.01	52.21	
2.65	236.85	.270E+07	910.6	1.06	49.36	
2.80	237.00	.309E+07	1097.6	1.12	46.91	

<----- hydrograph ----> <-pipe / channel-->						
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (0104) 6811.27	24.80	23.00	29.19	1.16	0.45	
OUTFLOW : ID= 1 (0105) 6811.27	24.17	25.00	29.19	1.16	0.45	

CALIB						
NASHVD (0091)						
Area	(ha)	(ha)	Curve Number	(CN)=	65.0	
ID= 1 DT=15.0 min	Ia	(mm)=	5.00	# of Linear Res.(N)=	3.10	
	U.H. Tp(hrs)=	5.80				

Unit Hyd Qpeak (cms) = 1.906

PEAK FLOW (cms) = 1.818 (i)
 TIME TO PEAK (hrs) = 16.750
 RINOFF VOLUME (mm) = 30.529
 TOTAL RAINFALL (mm) = 86.660
 RINOFF COEFFICIENT = 0.352

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0106)						
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (0091):	281.97	1.818	16.75	30.53		
+ ID2= 2 (0105):	6811.27	24.168	25.00	29.19		
ID = 3 (0106):	7093.24	25.007	24.50	29.25		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0107)						
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (0007):	1373.84	6.806	22.00	37.97		
+ ID2= 2 (0106):	7093.24	25.007	24.50	29.25		
ID = 3 (0107):	8467.08	31.569	24.00	30.66		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (0108)	
IN= 2-->	OUT= 1
Routing time step (min)'= 15.00	

<----- DATA FOR SECTION (1.1) ----->			
Distance	Elevation	Manning	
0.00	234.00	0.1000	
400.00	231.00	0.1000 / 0.0350	Main Channel
401.00	229.70	0.0350	Main Channel

<----- TRAVEL TIME TABLE ----->						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.20	229.90	.151E+04	0.1	0.24	395.61	
0.40	230.10	.377E+04	0.2	0.35	274.62	
0.60	230.30	.677E+04	0.5	0.43	222.06	
0.80	230.50	.105E+05	0.9	0.50	190.52	
1.03	230.73	.437E+05	2.0	0.27	356.49	
1.27	230.97	.134E+06	5.0	0.21	448.69	
1.50	231.20	.278E+06	11.4	0.23	405.59	
1.73	231.43	.473E+06	21.6	0.26	364.80	
1.97	231.67	.718E+06	36.0	0.29	332.76	
2.20	231.90	.101E+07	55.1	0.31	306.87	
2.43	232.13	.136E+07	79.4	0.33	285.44	
2.67	232.37	.176E+07	109.6	0.36	267.34	
2.90	232.60	.221E+07	145.9	0.38	251.83	
3.13	232.83	.270E+07	189.0	0.40	238.35	
3.37	233.07	.325E+07	239.3	0.42	226.51	
3.60	233.30	.385E+07	297.1	0.44	216.02	
3.83	233.53	.450E+07	363.0	0.46	206.64	
4.07	233.77	.520E+07	437.3	0.48	198.20	
4.30	234.00	.595E+07	520.5	0.50	190.55	

<----- hydrograph ----> <-pipe / channel-->						
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (0107) 8467.08	26.60	29.00	30.66	1.81	0.27	
OUTFLOW : ID= 1 (0108) 8467.08	26.60	29.00	30.66	1.81	0.27	

CALIB						
NASHVD (0092)						
Area	(ha)	(ha)	Curve Number	(CN)=	64.0	
ID= 1 DT=15.0 min	Ia	(mm)=	5.00	# of Linear Res.(N)=	3.10	
	U.H. Tp(hrs)=	16.50				

Unit Hyd Qpeak (cms) = 2.620

PEAK FLOW (cms) = 2.919 (i)
 TIME TO PEAK (hrs) = 28.250
 RINOFF VOLUME (mm) = 29.698
 TOTAL RAINFALL (mm) = 86.660
 RINOFF COEFFICIENT = 0.343

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0109)						
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1 = 1 (0092):	1102.32	2.919	28.25	29.70		
+ ID2= 2 (0108):	8467.08	26.605	29.00	30.66		
ID = 3 (0109):	9569.40	29.518	29.00	30.55		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0110)						
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1 = 1 (0010):	2842.76	10.894	22.25	29.70		
+ ID2= 2 (0109):	9569.40	29.518	29.00	30.55		
ID = 3 (0110):	12412.16	38.502	27.50	30.36		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (0111)	
IN= 2-->	OUT= 1
Routing time step (min)'= 15.00	

<----- DATA FOR SECTION (1.1) ----->			
Distance	Elevation	Manning	
0.00	236.00	0.1000	
275.00	232.00	0.1000	
1039.00	230.00	0.1000 / 0.0350	Main Channel
1040.00	229.20	0.0350	Main Channel
1041.00	229.20	0.0350	Main Channel
1043.00	230.00	0.0350 / 0.1000	Main Channel
1117.00	230.60	0.1000	
1325.00	234.00	0.1000	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.20	229.40	.985E+03	0.0	0.14	431.34
0.40	229.60	.251E+04	0.1	0.20	298.03
0.60	229.80	.457E+04	0.3	0.25	239.97
0.80	230.00	.727E+04	0.6	0.29	205.17
1.07	230.27	.754E+05	2.2	0.11	561.90
1.33	230.53	.272E+06	8.6	0.11	527.12
1.50	230.80	.594E+06	22.7	0.14	436.37
1.87	231.07	.103E+07	46.2	0.16	371.32
2.13	231.33	.158E+07	80.7	0.18	325.64
2.40	231.60	.224E+07	127.8	0.20	291.74
2.67	231.87	.301E+07	189.0	0.22	265.46
2.93	232.13	.389E+07	271.8	0.25	238.27
3.20	232.40	.481E+07	376.4	0.28	212.79
3.47	232.67	.576E+07	496.0	0.31	193.49
3.73	232.93	.674E+07	630.4	0.33	178.30
4.00	233.20	.776E+07	779.4	0.36	165.99
4.27	233.47	.881E+07	943.0	0.38	155.79
4.53	233.73	.990E+07	1121.1	0.41	147.16
4.80	234.00	.110E+08	1313.9	0.43	139.76

<---- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0110) ***** 38.50 27.50 30.36 1.78 0.15
 OUTFLOW : ID= 1 (0111) ***** 32.15 32.50 30.36 1.71 0.15

ADD HYD (0112)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0093):	381.73	1.271	26.50	34.06
+ ID2= 2 (0111):	12412.16	32.146	32.50	30.36
ID = 3 (0112):	12793.89	33.270	32.50	30.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)=
NASHYD (0008)	1549.97			60.0
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res. (N)= 3.10	
	U.H. Tp	(hrs)= 9.10		

Unit Hyd Qpeak (cms) = 6.679

PEAK FLOW (cms) = 6.123 (i)
 TIME TO PEAK (hrs) = 20.500
 RUNOFF VOLUME (mm) = 26.568
 TOTAL RAINFALL (mm) = 86.660
 RUNOFF COEFFICIENT = 0.307

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)=
NASHYD (0003)	2954.54			64.0
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res. (N)= 3.10	
	U.H. Tp	(hrs)= 13.10		

Unit Hyd Qpeak (cms) = 8.844

PEAK FLOW (cms) = 9.595 (i)
 TIME TO PEAK (hrs) = 24.750
 RUNOFF VOLUME (mm) = 29.698
 TOTAL RAINFALL (mm) = 86.660
 RUNOFF COEFFICIENT = 0.343

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)=
NASHYD (0004)	2672.69			53.0
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res. (N)= 3.10	
	U.H. Tp	(hrs)= 10.70		

Unit Hyd Qpeak (cms) = 9.795

PEAK FLOW (cms) = 7.532 (i)
 TIME TO PEAK (hrs) = 22.250
 RUNOFF VOLUME (mm) = 21.728
 TOTAL RAINFALL (mm) = 86.660
 RUNOFF COEFFICIENT = 0.251

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0201)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0003):	2954.54	9.595	24.75	29.70
+ ID2= 2 (0004):	2672.69	7.532	22.25	21.73
ID = 3 (0201):	5627.23	16.974	23.50	25.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0202) | Routing time step (min)'= 15.00
 IN= 2--> OUT= 1 |

Distance	Elevation	Manning
0.00	239.00	0.1000
690.00	230.00	0.1000 / 0.0350
691.00	229.20	0.0350
692.00	229.20	0.0350
693.00	229.50	0.0350 / 0.1000
1190.00	230.00	0.1000
1417.00	255.00	0.1000

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.30	229.50	.347E+04	0.4	0.84	136.57
0.80	230.00	.864E+06	45.2	0.36	318.78
1.30	230.50	.265E+07	274.1	0.71	161.20
1.80	231.00	.459E+07	651.8	0.97	117.24
2.30	231.50	.667E+07	1165.3	1.20	95.34
2.80	232.00	.899E+07	1815.2	1.40	81.89
3.30	232.50	.113E+08	2585.1	1.57	72.65
3.80	233.00	.138E+08	3490.4	1.73	65.85
4.30	233.50	.165E+08	4527.4	1.89	60.59
4.80	234.00	.193E+08	5697.8	2.03	56.38
5.30	234.50	.222E+08	7003.8	2.16	52.91
5.80	235.00	.253E+08	8447.7	2.28	50.00
6.30	235.50	.286E+08	10032.3	2.40	47.52
6.80	236.00	.320E+08	11760.1	2.52	45.36
7.30	236.50	.356E+08	13634.1	2.63	43.46
7.80	237.00	.393E+08	15657.0	2.73	41.78
8.30	237.50	.431E+08	17831.7	2.84	40.28
8.80	238.00	.471E+08	20161.3	2.93	38.92
9.30	238.50	.512E+08	22648.7	3.03	37.69

<---- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0201) 5627.23 16.97 23.50 25.91 0.48 0.56
 OUTFLOW : ID= 1 (0202) 5627.23 15.82 27.75 25.91 0.47 0.57

ADD HYD (0203)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0008):	1549.97	6.123	20.50	26.57
+ ID2= 2 (0202):	5627.23	15.822	27.75	25.91
ID = 3 (0203):	7177.20	20.481	25.50	26.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0301)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0112):	12793.89	33.270	32.50	30.47
+ ID2= 2 (0203):	7177.20	20.481	25.50	26.05
ID = 3 (0301):	19971.09	51.473	30.00	28.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0302) | Routing time step (min)'= 15.00
 IN= 2--> OUT= 1 |

Distance	Elevation	Manning
0.00	240.00	0.0700
110.00	239.00	0.0700
161.00	238.00	0.0700
250.00	237.00	0.0700
420.00	236.00	0.0700
560.00	235.00	0.0700

589.00	234.40	0.0700	/0.0350	Main Channel
589.10	233.60	0.0350		Main Channel
594.00	233.60	0.0350	/0.0700	Main Channel
594.10	234.40	0.0350	/0.0700	Main Channel
615.00	235.00	0.0700		
740.00	236.00	0.0700		
860.00	236.50	0.0700		
1340.00	237.00	0.0700		
1460.00	238.00	0.0700		
1470.00	239.00	0.0700		
1510.00	240.00	0.0700		

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.27	233.87	.770E+04	1.0	0.74	132.49
0.53	234.13	.155E+05	2.9	1.10	88.48
0.80	234.40	.234E+05	5.5	1.37	71.19
1.15	234.75	.636E+05	11.6	1.07	91.11
1.50	235.10	.169E+06	24.5	0.85	114.97
1.85	235.45	.431E+06	54.5	0.74	131.62
2.20	235.80	.882E+06	117.3	0.78	125.39
2.55	236.15	.153E+07	218.3	0.83	117.06
2.90	236.50	.246E+07	375.3	0.89	109.30
3.25	236.85	.394E+07	579.0	0.86	113.45
3.60	237.20	.612E+07	1010.3	0.97	101.01
3.95	237.55	.851E+07	1631.3	1.12	86.99
4.30	237.90	.111E+08	2391.8	1.27	77.05
4.65	238.25	.137E+08	3337.5	1.42	68.52
5.00	238.60	.164E+08	4438.7	1.58	61.71
5.35	238.95	.192E+08	5671.0	1.73	56.40
5.70	239.30	.220E+08	6955.1	1.85	52.75
6.05	239.65	.249E+08	8355.2	1.96	49.76
6.40	240.00	.280E+08	9884.7	2.07	47.18

<---- hydrograph ----> <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0301) *****	51.47	30.00	28.88	1.81	0.75
OUTFLOW : ID= 1 (0302) *****	50.29	32.75	28.88	1.80	0.75

----- ADD HYD (0303) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0012):	1501.52	5.015	23.00	27.32
+ ID2= 2 (0302):	19971.09	50.289	32.75	28.88

ID = 3 (0303):	21472.61	53.703	32.00	28.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- CALLIB (0011) -----

NASHYD (0011)	Area (ha)	(ha)=3099.17	Curve Number (CN)= 57.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 3.10	
	U.H. Tp(hrs)= 11.10		

Unit Hyd Qpeak (cms)= 10.949

PEAK FLOW (cms)= 9.517 (1)
 TIME TO PEAK (hrs)= 22.750
 RUNOFF VOLUME (mm)= 24.402
 TOTAL RAINFALL (mm)= 86.660
 RUNOFF COEFFICIENT = 0.282

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- ROUTE CHN (0401) -----

IN= 2--> OUT= 1

Routing time step (min)= 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning
0.00	236.00	0.0500
72.00	235.00	0.0500
492.00	230.00	0.0500
581.00	229.20	0.0500 /0.0350
582.00	228.40	0.0350
583.00	228.40	0.0350
584.00	229.20	0.0350 /0.0500
588.00	229.20	0.0500
607.00	230.00	0.0500
627.00	235.00	0.0500

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.27	228.67	.302E+04	0.1	0.21	666.69
0.53	228.93	.756E+04	0.3	0.30	465.92

0.80	229.20	.136E+05	0.6	0.37	377.99
1.16	229.56	.111E+06	3.0	0.23	616.66
1.52	229.92	.358E+06	11.5	0.27	519.91
1.89	230.29	.740E+06	30.1	0.35	410.09
2.25	230.65	.122E+07	59.5	0.41	342.49
2.61	231.01	.180E+07	100.6	0.47	298.58
2.97	231.37	.248E+07	154.6	0.53	267.22
3.34	231.74	.326E+07	222.9	0.58	243.40
3.70	232.10	.413E+07	306.6	0.63	224.51
4.06	232.46	.510E+07	406.9	0.68	209.05
4.42	232.82	.617E+07	524.8	0.72	196.10
4.79	233.19	.734E+07	661.4	0.77	185.06
5.15	233.55	.861E+07	817.9	0.81	175.49
5.51	233.91	.998E+07	995.2	0.85	167.10
5.87	234.27	.114E+08	1194.3	0.89	159.67
6.24	234.64	.130E+08	1416.3	0.93	153.03
6.60	235.00	.147E+08	1662.2	0.96	147.05

<---- hydrograph ----> <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0011) 3099.17	9.52	22.75	24.40	1.44	0.26
OUTFLOW : ID= 1 (0401) 3099.17	6.86	30.00	24.40	1.33	0.25

----- CALLIB (0014) -----

NASHYD (0014)	Area (ha)	(ha)=2163.09	Curve Number (CN)= 69.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 3.10	
	U.H. Tp(hrs)= 25.10		

Unit Hyd Qpeak (cms)= 3.379

PEAK FLOW (cms)= 4.442 (1)
 TIME TO PEAK (hrs)= 36.750
 RUNOFF VOLUME (mm)= 34.061
 TOTAL RAINFALL (mm)= 86.660
 RUNOFF COEFFICIENT = 0.393

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- ADD HYD (0402) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0014):	2163.09	4.442	36.75	34.06
+ ID2= 2 (0401):	3099.17	6.858	30.00	24.40

ID = 3 (0402):	5262.26	11.022	31.50	28.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ADD HYD (0501) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0303):	21472.61	53.703	32.00	28.77
+ ID2= 2 (0402):	5262.26	11.022	31.50	28.37

ID = 3 (0501):	26734.87	64.720	32.00	28.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ROUTE CHN (0503) -----

IN= 2--> OUT= 1

Routing time step (min)= 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning
0.00	237.00	0.0700
425.00	235.00	0.0700
445.00	234.00	0.0700
480.00	233.50	0.0700
1535.00	233.50	0.0700 /0.0350
1536.50	232.70	0.0350
1537.50	232.70	0.0350
1540.00	233.50	0.0350 /0.0700
1605.00	233.20	0.0700
1610.00	233.10	0.0700
2135.00	233.10	0.0700
2440.00	233.50	0.0700
2505.00	233.50	0.0700
2530.00	234.00	0.0700
2535.00	235.00	0.0700
2540.00	236.00	0.0700
2605.00	238.00	0.0700
2645.00	241.00	0.0700

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.20	232.90	.385E+03	0.2	0.55	38.92
0.40	233.10	.103E+04	0.6	0.80	26.75
0.60	233.30	.159E+06	39.5	0.32	67.02
0.80	233.50	.366E+06	133.3	0.47	45.74
1.03	233.73	.977E+06	428.7	0.56	37.97
1.27	233.97	.160E+07	914.3	0.74	29.09
1.50	234.20	.222E+07	1556.9	0.90	23.78
1.73	234.43	.285E+07	2336.2	1.05	20.32
1.97	234.67	.348E+07	3240.2	1.20	17.88
2.20	234.90	.411E+07	4260.9	1.33	16.07
2.43	235.13	.474E+07	5359.1	1.45	14.75
2.67	235.37	.539E+07	6537.6	1.56	13.74
2.90	235.60	.605E+07	7819.4	1.66	12.90
3.13	235.83	.673E+07	9203.8	1.76	12.19
3.37	236.07	.743E+07	10683.6	1.85	11.59
3.60	236.30	.814E+07	12247.8	1.93	11.07
3.83	236.53	.887E+07	13912.8	2.01	10.62
4.07	236.77	.961E+07	15679.0	2.09	10.22
4.30	237.00	.104E+08	17546.8	2.17	9.85

		<---- hydrograph ---->				<-pipe / channel-->	
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW: ID= 2 (0501)	*****	64.72	32.00	28.69	0.65	0.35	
OUTFLOW: ID= 1 (0503)	*****	64.52	32.75	28.69	0.65	0.35	

RESERVOIR (0510)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	27.8300	83.9600
3.8000	14.0500	*****	101.7400
11.0800	33.6500	*****	145.2700
20.8900	61.8900	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (0503)	*****	64.519	32.75	28.69
OUTFLOW: ID= 1 (0510)	*****	64.516	32.75	28.69

PEAK FLOW REDUCTION (Qout/Qin) = 100.00
TIME SHIFT OF PEAK FLOW (min) = 0.00
MAXIMUM STORAGE USED (ha.m.) = 85.0259

ROUTE CHN (0513)
IN= 2--> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (1.1) ----->						
Distance	Elevation	Manning				
0.00	237.00	0.0700				
425.00	235.00	0.0700				
445.00	234.00	0.0700				
480.00	233.50	0.0700				
1535.00	233.50	0.0700 / 0.0350	Main Channel			
1536.50	232.70	0.0350	Main Channel			
1537.50	232.70	0.0350	Main Channel			
1540.00	233.50	0.0350 / 0.0700	Main Channel			
1605.00	233.20	0.0700				
1610.00	233.10	0.0700				
2135.00	233.10	0.0700				
2440.00	233.50	0.0700				
2505.00	233.50	0.0700				
2530.00	234.00	0.0700				
2535.00	235.00	0.0700				
2540.00	236.00	0.0700				
2605.00	238.00	0.0700				
2645.00	241.00	0.0700				

TRAVEL TIME TABLE					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.20	232.90	.291E+03	0.2	0.55	29.43
0.40	233.10	.777E+03	0.6	0.80	20.23
0.60	233.30	.120E+06	39.5	0.32	50.68
0.80	233.50	.277E+06	133.3	0.47	34.59
1.03	233.73	.739E+06	428.7	0.56	28.72
1.27	233.97	.121E+07	914.3	0.74	22.00
1.50	234.20	.168E+07	1556.9	0.90	17.98
1.73	234.43	.215E+07	2336.2	1.05	15.37
1.97	234.67	.263E+07	3240.2	1.20	13.52
2.20	234.90	.311E+07	4260.9	1.33	12.15
2.43	235.13	.359E+07	5359.1	1.45	11.15
2.67	235.37	.408E+07	6537.6	1.56	10.39
2.90	235.60	.458E+07	7819.4	1.66	9.76
3.13	235.83	.509E+07	9203.8	1.76	9.22
3.37	236.07	.562E+07	10683.6	1.85	8.76
3.60	236.30	.615E+07	12247.8	1.93	8.37
3.83	236.53	.670E+07	13912.8	2.01	8.03

4.07	236.77	.727E+07	15679.0	2.09	7.73
4.30	237.00	.784E+07	17546.8	2.17	7.45

		<---- hydrograph ---->				<-pipe / channel-->	
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW: ID= 2 (0510)	*****	64.52	32.75	28.69	0.65	0.35	
OUTFLOW: ID= 1 (0513)	*****	64.42	33.50	28.69	0.65	0.35	

ADD HYD (0504)
1 + 2 = 3

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0015):	1059.88	5.908	20.50	37.97
+ ID2= 2 (0513):	26734.87	64.424	33.50	28.69
ID = 3 (0504):	27794.75	66.841	33.00	29.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0505)
1 + 2 = 3

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0013):	1627.59	4.688	27.25	30.53
+ ID2= 2 (0504):	27794.75	66.841	33.00	29.05
ID = 3 (0505):	29422.34	71.059	32.75	29.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0506)
IN= 2--> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (1.1) ----->				
Distance	Elevation	Manning		
0.00	251.00	0.0500		
323.00	238.00	0.0500		
400.00	229.00	0.0500		
545.00	228.00	0.0500		
602.00	226.00	0.0500		
604.00	225.80	0.0500 / 0.0350	Main Channel	
605.00	225.00	0.0350	Main Channel	
609.00	225.00	0.0350	Main Channel	
610.00	225.80	0.0350 / 0.0500	Main Channel	
614.00	226.00	0.0500		
650.00	227.00	0.0500		
810.00	229.50	0.0500		
1855.00	240.00	0.0500		
2617.00	259.00	0.0500		

TRAVEL TIME TABLE					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.80	225.80	.292E+05	3.5	0.87	139.36
2.13	227.13	.441E+06	50.6	0.84	145.20
3.45	228.45	.197E+07	262.2	0.97	125.42
4.78	229.78	.557E+07	971.2	1.27	95.57
6.11	231.11	.106E+08	2326.5	1.61	75.63
7.43	232.43	.169E+08	4425.6	1.90	63.77
8.76	233.76	.247E+08	7361.8	2.17	55.90
10.08	235.08	.338E+08	11229.7	2.42	50.22
11.41	236.41	.444E+08	16121.0	2.65	45.87
12.74	237.74	.563E+08	22124.8	2.86	42.40
14.06	239.06	.697E+08	29005.2	3.03	40.02
15.39	240.39	.846E+08	37433.0	3.23	37.66
16.72	241.72	.101E+09	47908.6	3.47	34.97
18.04	243.04	.117E+09	59742.1	3.71	32.73
19.37	244.37	.135E+09	72949.0	3.94	30.83
20.69	245.69	.153E+09	87549.6	4.16	29.20
22.02	247.02	.173E+09	*****	4.37	27.79
23.35	248.35	.193E+09	*****	4.58	26.55
24.67	249.67	.214E+09	*****	4.77	25.45

		<---- hydrograph ---->				<-pipe / channel-->	
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW: ID= 2 (0505)	*****	71.06	32.75	29.13	2.25	0.85	
OUTFLOW: ID= 1 (0506)	*****	68.60	35.75	29.13	2.24	0.85	

RESERVOIR (0512)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	*****	16.2850

27.2000 3.7650 | ***** 20.9160
 76.8000 7.7410 | ***** 25.9120
 ***** 11.0650 | ***** 36.6600
 ***** 11.9150 | 0.0000 0.0000

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (0506) ***** 68.596 35.75 29.13
 OUTFLOW: ID= 1 (0512) ***** 68.582 36.00 29.13

PEAK FLOW REDUCTION [Qout/Qin](%)= 99.98
 TIME SHFT OF PEAK FLOW (min)= 15.00
 MAXIMUM STORAGE USED (ha.m.)= 7.0829

ROUTER CHN (0514) |
 IN= 2--> OUF= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning	
0.00	230.00	0.0500	
400.00	229.00	0.0500	
545.00	228.00	0.0500	
602.00	226.00	0.0500	
604.00	225.80	0.0500 /0.0350	Main Channel
605.00	225.00	0.0350	Main Channel
609.00	225.00	0.0350	Main Channel
610.00	225.80	0.0350 /0.0500	Main Channel
614.00	226.00	0.0500	
650.00	227.00	0.0500	
810.00	229.50	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.20	225.20	.243E+04	0.3	0.39	121.91
0.40	225.40	.515E+04	1.1	0.59	80.58
0.50	225.60	.815E+04	2.1	0.74	64.02
0.80	225.80	.114E+05	3.5	0.87	54.69
1.05	226.05	.184E+05	6.1	0.94	50.48
1.29	226.29	.346E+05	10.3	0.85	55.99
1.54	226.54	.620E+05	17.4	0.80	59.38
1.79	226.79	.101E+06	28.3	0.81	59.18
2.03	227.03	.151E+06	43.8	0.83	57.35
2.28	227.28	.215E+06	63.9	0.85	55.99
2.53	227.53	.295E+06	91.4	0.89	53.82
2.77	227.77	.391E+06	127.0	0.93	51.35
3.02	228.02	.504E+06	170.6	0.97	49.24
3.27	228.27	.644E+06	215.5	0.96	49.84
3.51	228.51	.821E+06	279.7	0.97	48.92
3.76	228.76	.103E+07	363.1	1.00	47.47
4.01	229.01	.128E+07	466.2	1.04	45.89
4.25	229.25	.159E+07	572.0	1.03	46.41
4.50	229.50	.198E+07	719.1	1.04	45.95

<---- hydrograph ----> <-pipe / channel-->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (0512) *****	68.58	36.00	29.13	2.32	0.86
OUTFLOW: ID= 1 (0514) *****	68.30	37.00	29.13	2.31	0.86

ADD HYD (0507) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0016): 3094.49 9.907 29.75 39.02
 + ID2= 2 (0514): 29422.34 68.301 37.00 29.13
 ID = 3 (0507): 32516.83 77.034 36.50 30.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ** SIMULATION NUMBER: 4 **

READ STORM | Filename: C:\Users\jascott\AppData
 | ata\Local\Temp\
 | 88073fbd-731f-4992-8e5b-76831528f0f7\06048376
 Ptotal=104.50 mm | Comments: 25yr/24hr

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	0.00	6.50	8.88	12.75	3.66	19.00	0.52
0.50	0.52	6.75	8.88	13.00	3.66	19.25	0.52
0.75	0.52	7.00	8.88	13.25	3.66	19.50	0.52
1.00	0.52	7.25	8.88	13.50	3.66	19.75	0.52
1.25	0.52	7.50	8.88	13.75	3.66	20.00	0.52

1.50	0.52	7.75	8.88	14.00	3.66	20.25	0.52
1.75	0.52	8.00	8.88	14.25	3.66	20.50	0.52
2.00	0.52	8.25	8.88	14.50	2.09	20.75	0.52
2.25	0.52	8.50	24.04	14.75	2.09	21.00	0.52
2.50	0.52	8.75	24.04	15.00	2.09	21.25	0.52
2.75	0.52	9.00	24.04	15.25	2.09	21.50	0.52
3.00	0.52	9.25	24.04	15.50	2.09	21.75	0.52
3.25	0.52	9.50	24.04	15.75	2.09	22.00	0.52
3.50	0.52	9.75	24.04	16.00	2.09	22.25	0.52
3.75	0.52	10.00	24.04	16.25	2.09	22.50	0.52
4.00	0.52	10.25	24.04	16.50	1.05	22.75	0.52
4.25	0.52	10.50	6.79	16.75	1.05	23.00	0.52
4.50	3.14	10.75	6.79	17.00	1.05	23.25	0.52
4.75	3.14	11.00	6.79	17.25	1.05	23.50	0.52
5.00	3.14	11.25	6.79	17.50	1.05	23.75	0.52
5.25	3.14	11.50	6.79	17.75	1.05	24.00	0.52
5.50	3.14	11.75	6.79	18.00	1.05	24.25	0.52
5.75	3.14	12.00	6.79	18.25	1.05		
6.00	3.14	12.25	6.79	18.50	0.52		
6.25	3.14	12.50	3.66	18.75	0.52		

CALIB (0016) | Area (ha)=3094.49 Curve Number (CN)= 74.0
 NASHYD (0016) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 18.10

Unit Hyd Qpeak (cms)= 6.704

PEAK FLOW (cms)= 13.323 (i)
 TIME TO PEAK (hrs)= 29.500
 RUNOFF VOLUME (mm)= 52.453
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.502

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0013) | Area (ha)=1627.59 Curve Number (CN)= 65.0
 NASHYD (0013) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 15.50

Unit Hyd Qpeak (cms)= 4.118

PEAK FLOW (cms)= 6.437 (i)
 TIME TO PEAK (hrs)= 27.000
 RUNOFF VOLUME (mm)= 41.902
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.401

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0015) | Area (ha)=1059.88 Curve Number (CN)= 73.0
 NASHYD (0015) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 9.30

Unit Hyd Qpeak (cms)= 4.469

PEAK FLOW (cms)= 7.975 (i)
 TIME TO PEAK (hrs)= 20.500
 RUNOFF VOLUME (mm)= 51.179
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.490

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0012) | Area (ha)=1501.52 Curve Number (CN)= 61.0
 NASHYD (0012) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 11.50

Unit Hyd Qpeak (cms)= 5.120

PEAK FLOW (cms)= 6.945 (i)
 TIME TO PEAK (hrs)= 23.000
 RUNOFF VOLUME (mm)= 37.803
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.362

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0093) | Area (ha)= 381.73 Curve Number (CN)= 69.0
 NASHYD (0093) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 ID= 1 DT=15.0 min |

U.H. Tp(hrs)= 14.90
 Unit Hyd Qpeak (cms)= 1.005
 PEAK FLOW (cms)= 1.731 (i)
 TIME TO PEAK (hrs)= 26.500
 RUNOFF VOLUME (mm)= 46.346
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.444

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0010) | Area (ha)=2842.76 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 10.80

Unit Hyd Qpeak (cms)= 10.322
 PEAK FLOW (cms)= 15.001 (i)
 TIME TO PEAK (hrs)= 22.250
 RUNOFF VOLUME (mm)= 40.847
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.391

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0007) | Area (ha)=1373.84 Curve Number (CN)= 73.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 10.70

Unit Hyd Qpeak (cms)= 5.035
 PEAK FLOW (cms)= 9.183 (i)
 TIME TO PEAK (hrs)= 22.000
 RUNOFF VOLUME (mm)= 51.179
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.490

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0005) | Area (ha)=1444.47 Curve Number (CN)= 60.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 8.00

Unit Hyd Qpeak (cms)= 7.080
 PEAK FLOW (cms)= 8.792 (i)
 TIME TO PEAK (hrs)= 19.250
 RUNOFF VOLUME (mm)= 36.827
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.352

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0006) | Area (ha)=1504.44 Curve Number (CN)= 73.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 12.20

Unit Hyd Qpeak (cms)= 4.836
 PEAK FLOW (cms)= 8.982 (i)
 TIME TO PEAK (hrs)= 23.500
 RUNOFF VOLUME (mm)= 51.179
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.490

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0001) | Area (ha)=2873.64 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 11.00

Unit Hyd Qpeak (cms)= 10.244
 PEAK FLOW (cms)= 13.804 (i)
 TIME TO PEAK (hrs)= 22.500
 RUNOFF VOLUME (mm)= 37.803
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.362

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0002) | Area (ha)= 988.72 Curve Number (CN)= 58.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 9.10

Unit Hyd Qpeak (cms)= 4.261
 PEAK FLOW (cms)= 5.139 (i)
 TIME TO PEAK (hrs)= 20.500
 RUNOFF VOLUME (mm)= 34.930
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.334

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0101) |
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0001): 2873.64 13.804 22.50 37.80
 + ID2= 2 (0002): 988.72 5.139 20.50 34.93
 ID= 3 (0101): 3862.36 18.820 21.75 37.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0102) |
 IN= 2---> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning	
0.00	250.00	0.0700	
43.00	240.00	0.0700 /0.0350	Main Channel
44.00	239.20	0.0350	Main Channel
45.00	239.20	0.0350	Main Channel
46.00	240.00	0.0350 /0.0700	Main Channel
63.50	241.00	0.0700	
71.00	250.00	0.0700	

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.40	239.60	642E+04	0.5	0.80	221.73
0.80	240.00	171E+05	1.8	1.15	155.20
1.39	240.59	764E+05	8.0	1.12	159.62
1.98	241.18	214E+06	23.7	1.19	150.23
2.56	241.76	385E+06	51.8	1.44	123.91
3.15	242.35	575E+06	90.1	1.68	106.38
3.74	242.94	784E+06	138.5	1.89	94.40
4.33	243.53	101E+07	196.9	2.08	85.71
4.92	244.12	126E+07	265.6	2.26	79.08
5.51	244.71	153E+07	344.7	2.42	73.81
6.09	245.29	181E+07	434.5	2.57	69.50
6.68	245.88	212E+07	535.2	2.71	65.89
7.27	246.47	244E+07	647.3	2.84	62.81
7.86	247.06	278E+07	771.1	2.97	60.13
8.45	247.65	314E+07	906.8	3.09	57.77
9.04	248.24	352E+07	1054.8	3.20	55.67
9.62	248.82	392E+07	1215.5	3.32	53.79
10.21	249.41	434E+07	1389.1	3.42	52.09
10.80	250.00	478E+07	1576.1	3.53	50.53

<---- hydrograph ----> <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (0101)	3862.36	18.82	21.75	37.07	1.79	1.16
OUTFLOW: ID= 1 (0102)	3862.36	17.90	24.50	37.07	1.76	1.16

ADD HYD (0103) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0006): 1504.44 8.982 23.50 51.18
 + ID2= 2 (0102): 3862.36 17.900 24.50 37.07
 ID= 3 (0103): 5366.80 26.849 24.25 41.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0104) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.

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      (ha)   (cms)   (hrs)   (mm)
ID1= 1 (0005): 1444.47  8.792  19.25  36.83
+ ID2= 2 (0103): 5366.80 26.849  24.25  41.02
=====
ID = 3 (0104): 6811.27 34.219  23.00  40.13
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (0105) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
-----

```

```

<----- DATA FOR SECTION ( 1.1) ----->
Distance      Elevation      Manning
0.00           237.00           0.1000
330.00         235.00           0.1000 /0.0350 Main Channel
331.00         234.20           0.0350 Main Channel
332.00         234.20           0.0350 Main Channel
333.00         235.00           0.0350 /0.1000 Main Channel
425.00         235.10           0.1000
698.00         236.00           0.1000
1101.00        239.00           0.1000
-----

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH  ELEV  VOLUME  FLOW RATE  VELOCITY  TRAV.TIME
(m)    (m)    (cu.m.) (cms)      (m/s)     (min)
0.13   234.33  .490E+03  0.1         0.64      82.66
0.27   234.47  .112E+04  0.3         0.93      56.72
0.40   234.60  .189E+04  0.7         1.14      45.94
0.53   234.73  .280E+04  1.2         1.33      39.64
0.67   234.87  .385E+04  1.8         1.49      35.34
0.80   235.00  .504E+04  2.6         1.63      32.16
0.95   235.15  .442E+05  6.3         0.45      116.42
1.11   235.31  .138E+06  17.7        0.44      120.51
1.26   235.46  .247E+06  38.2        0.49      107.48
1.42   235.62  .400E+06  69.8        0.55      95.60
1.57   235.77  .589E+06  114.0       0.61      86.08
1.72   235.92  .812E+06  172.6       0.67      78.45
1.88   236.08  .107E+07  250.3       0.74      71.19
2.03   236.23  .135E+07  347.8       0.81      64.71
2.18   236.38  .165E+07  461.9       0.88      59.65
2.34   236.54  .198E+07  593.4       0.95      55.58
2.49   236.69  .233E+07  742.8       1.01      52.21
2.65   236.85  .270E+07  910.6       1.06      49.36
2.80   237.00  .309E+07  1097.6      1.12      46.91
-----

```

```

<----- hydrograph -----> <-pipe / channel->
AREA  QPEAK  TPEAK  R.V.  MAX DEPTH  MAX VEL
(ha)  (cms)  (hrs)  (mm)  (m)         (m/s)
INFLOW : ID= 2 (0104) 6811.27 34.22 21.00 40.13 1.23 0.48
OUTFLOW : ID= 1 (0105) 6811.27 33.48 24.50 40.13 1.23 0.48
-----

```

```

-----
| CALIB (0091) |
| NASHYD (0091) | Area (ha)= 281.97 Curve Number (CN)= 65.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
| U.H. Tp(hrs)= 5.80
-----

```

Unit Hyd Qpeak (cms)= 1.906

```

PEAK FLOW (cms)= 2.502 (i)
TIME TO PEAK (hrs)= 16.750
RUNOFF VOLUME (mm)= 41.902
TOTAL RAINFALL (mm)= 104.500
RUNOFF COEFFICIENT = 0.401
-----

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (0106) |
| 1 + 2 = 3 | AREA  QPEAK  TPEAK  R.V.
      (ha)  (cms)  (hrs)  (mm)
ID1= 1 (0091): 281.97 2.502 16.75 41.90
+ ID2= 2 (0105): 6811.27 33.479 24.50 40.13
=====
ID = 3 (0106): 7093.24 34.680 24.25 40.20
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (0107) |
| 1 + 2 = 3 | AREA  QPEAK  TPEAK  R.V.
      (ha)  (cms)  (hrs)  (mm)
ID1= 1 (0007): 1373.84 9.183 22.00 51.18
+ ID2= 2 (0106): 7093.24 34.680 24.25 40.20
=====
ID = 3 (0107): 8467.08 43.582 23.75 41.98
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (0108) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
-----

```

```

<----- DATA FOR SECTION ( 1.1) ----->
Distance      Elevation      Manning
0.00           234.00           0.1000
400.00         231.00           0.1000 /0.0350 Main Channel
401.00         229.70           0.0350 Main Channel
402.00         229.70           0.0350 Main Channel
404.00         230.50           0.0350 /0.1000 Main Channel
495.00         231.00           0.1000
1234.00        256.00           0.1000
-----

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH  ELEV  VOLUME  FLOW RATE  VELOCITY  TRAV.TIME
(m)    (m)    (cu.m.) (cms)      (m/s)     (min)
0.20   229.90  .151E+04  0.1         0.24      395.61
0.40   230.10  .377E+04  0.2         0.35      274.62
0.60   230.30  .677E+04  0.5         0.43      222.06
0.80   230.50  .105E+05  0.9         0.50      190.52
1.03   230.73  .437E+05  2.0         0.27      356.49
1.27   230.97  .134E+06  5.0         0.21      448.69
1.50   231.20  .278E+06  11.4        0.23      405.59
1.73   231.43  .473E+06  21.6        0.26      364.80
1.97   231.67  .718E+06  36.0        0.29      332.76
2.20   231.90  .101E+07  55.1        0.31      306.87
2.43   232.13  .136E+07  79.4        0.33      285.44
2.67   232.37  .176E+07  109.6       0.36      267.94
2.90   232.60  .221E+07  145.9       0.38      251.83
3.13   232.83  .270E+07  189.0       0.40      238.35
3.37   233.07  .325E+07  239.3       0.42      226.51
3.60   233.30  .385E+07  297.1       0.44      216.02
3.83   233.53  .450E+07  363.0       0.46      206.64
4.07   233.77  .520E+07  437.3       0.48      198.20
4.30   234.00  .595E+07  520.5       0.50      190.55
-----

```

```

<----- hydrograph -----> <-pipe / channel->
AREA  QPEAK  TPEAK  R.V.  MAX DEPTH  MAX VEL
(ha)  (cms)  (hrs)  (mm)  (m)         (m/s)
INFLOW : ID= 2 (0107) 8467.08 43.58 23.75 41.98 2.06 0.29
OUTFLOW : ID= 1 (0108) 8467.08 37.20 28.50 41.98 1.98 0.29
-----

```

```

-----
| CALIB (0092) |
| NASHYD (0092) | Area (ha)=1102.32 Curve Number (CN)= 64.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
| U.H. Tp(hrs)= 16.50
-----

```

Unit Hyd Qpeak (cms)= 2.620

```

PEAK FLOW (cms)= 4.017 (i)
TIME TO PEAK (hrs)= 28.000
RUNOFF VOLUME (mm)= 40.847
TOTAL RAINFALL (mm)= 104.500
RUNOFF COEFFICIENT = 0.391
-----

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (0109) |
| 1 + 2 = 3 | AREA  QPEAK  TPEAK  R.V.
      (ha)  (cms)  (hrs)  (mm)
ID1= 1 (0092): 1102.32 4.017 28.00 40.85
+ ID2= 2 (0108): 8467.08 37.198 28.50 41.98
=====
ID = 3 (0109): 9569.40 41.213 28.50 41.85
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (0110) |
| 1 + 2 = 3 | AREA  QPEAK  TPEAK  R.V.
      (ha)  (cms)  (hrs)  (mm)
ID1= 1 (0010): 2842.76 15.001 22.25 40.85
+ ID2= 2 (0109): 9569.40 41.213 28.50 41.85
=====
ID = 3 (0110): 12412.16 53.808 27.00 41.62
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (0111) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
-----

```

Distance	Elevation	Manning	
0.00	236.00	0.1000	
275.00	232.00	0.1000	
1039.00	230.00	0.1000 / 0.0350	Main Channel
1040.00	229.20	0.0350	Main Channel
1041.00	229.20	0.0350	Main Channel
1043.00	230.00	0.0350 / 0.1000	Main Channel
1117.00	230.60	0.1000	
1325.00	234.00	0.1000	

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.20	229.40	.985E+03	0.0	0.14	431.34
0.40	229.60	.251E+04	0.1	0.20	298.03
0.60	229.80	.457E+04	0.3	0.25	239.97
0.80	230.00	.717E+04	0.6	0.29	205.17
1.07	230.27	.754E+05	2.2	0.11	561.90
1.33	230.53	.272E+06	8.6	0.11	527.12
1.60	230.80	.894E+06	22.7	0.14	436.37
1.87	231.07	.103E+07	46.2	0.16	371.32
2.13	231.33	.158E+07	80.7	0.18	325.64
2.40	231.60	.224E+07	127.8	0.20	291.74
2.67	231.87	.301E+07	189.0	0.22	265.46
2.93	232.13	.389E+07	271.8	0.25	238.27
3.20	232.40	.481E+07	376.4	0.28	212.79
3.47	232.67	.576E+07	496.0	0.31	193.49
3.73	232.93	.674E+07	630.4	0.33	178.30
4.00	233.20	.776E+07	779.4	0.36	165.99
4.27	233.47	.881E+07	943.0	0.38	155.79
4.53	233.73	.990E+07	1121.1	0.41	147.16
4.80	234.00	.110E+08	1313.9	0.43	139.76

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0110)	53.81	27.00	41.62	1.93	0.17
OUTFLOW : ID= 1 (0111)	45.70	32.00	41.62	1.86	0.16

ADD HYD (0112)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0093)	381.73	1.731	26.50	46.35
+ ID2= 2 (0111)	12412.16	45.695	32.00	41.62
ID= 3 (0112)	12793.89	47.260	31.50	41.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0008)	Area (ha)	Curve Number (CN)
NASHYD (0008)	1549.97	64.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 3.10
	U.H. Tp(hrs)= 9.10	

Unit Hyd Qpeak (cms)= 6.679

PEAK FLOW (cms)= 8.499 (i)
 TIME TO PEAK (hrs)= 20.250
 RUNOFF VOLUME (mm)= 36.827
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.352

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0003)	Area (ha)	Curve Number (CN)
NASHYD (0003)	2954.54	64.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 3.10
	U.H. Tp(hrs)= 13.10	

Unit Hyd Qpeak (cms)= 8.844

PEAK FLOW (cms)= 13.206 (i)
 TIME TO PEAK (hrs)= 24.750
 RUNOFF VOLUME (mm)= 40.847
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.391

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0004)	Area (ha)	Curve Number (CN)
NASHYD (0004)	2672.69	53.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 3.10
	U.H. Tp(hrs)= 10.70	

Unit Hyd Qpeak (cms)= 9.795

PEAK FLOW (cms)= 10.578 (i)
 TIME TO PEAK (hrs)= 22.250
 RUNOFF VOLUME (mm)= 30.486
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.292

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0201)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0003)	2954.54	13.206	24.75	40.85
+ ID2= 2 (0004)	2672.69	10.578	22.25	30.49
ID= 3 (0201)	5627.23	23.574	23.50	35.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0202)	ROUTING TIME STEP (min)
IN= 2--> OUT= 1	15.00

Distance	Elevation	Manning	
0.00	239.00	0.1000	
690.00	230.00	0.1000 / 0.0350	Main Channel
691.00	229.20	0.0350	Main Channel
692.00	229.20	0.0350	Main Channel
693.00	229.50	0.0350 / 0.1000	Main Channel
1190.00	230.00	0.1000	
1417.00	255.00	0.1000	

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.30	229.50	.347E+04	0.4	0.84	136.57
0.80	230.00	.864E+06	45.2	0.36	318.78
1.30	230.50	.265E+07	274.1	0.71	161.20
1.80	231.00	.459E+07	651.8	0.97	117.24
2.30	231.50	.667E+07	1165.3	1.20	95.34
2.80	232.00	.899E+07	1810.2	1.40	81.89
3.30	232.50	.113E+08	2585.1	1.57	72.65
3.80	233.00	.138E+08	3490.4	1.73	65.85
4.30	233.50	.165E+08	4527.4	1.89	60.59
4.80	234.00	.193E+08	5697.8	2.03	56.38
5.30	234.50	.222E+08	7003.8	2.16	52.91
5.80	235.00	.253E+08	8447.7	2.28	50.00
6.30	235.50	.286E+08	10032.3	2.40	47.52
6.80	236.00	.320E+08	11760.1	2.52	45.36
7.30	236.50	.356E+08	13634.1	2.63	43.46
7.80	237.00	.393E+08	15657.0	2.73	41.78
8.30	237.50	.431E+08	17831.7	2.84	40.28
8.80	238.00	.471E+08	20161.3	2.93	38.92
9.30	238.50	.512E+08	22648.7	3.03	37.69

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0201)	5627.23	23.57	23.50	35.93	0.56
OUTFLOW : ID= 1 (0202)	5627.23	21.66	28.25	35.93	0.54

ADD HYD (0203)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0008)	1549.97	8.499	20.25	36.83
+ ID2= 2 (0202)	5627.23	21.663	28.25	35.93
ID= 3 (0203)	7177.20	27.881	26.00	36.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0301)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0112)	12793.89	47.260	31.50	41.76
+ ID2= 2 (0203)	7177.20	27.881	26.00	36.12
ID= 3 (0301)	19971.09	72.818	30.00	39.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (0302) |
| IN= 2--> OTR= 1 | Routing time step (min)'= 15.00
-----

```

```

<----- DATA FOR SECTION ( 1.1) ----->
Distance Elevation Manning
0.00 240.00 0.0700
110.00 239.00 0.0700
161.00 238.00 0.0700
250.00 237.00 0.0700
420.00 236.00 0.0700
560.00 235.00 0.0700
589.00 234.40 0.0700 /0.0350 Main Channel
589.10 233.60 0.0350 Main Channel
594.00 233.60 0.0350 Main Channel
594.10 234.40 0.0350 /0.0700 Main Channel
615.00 235.00 0.0700
740.00 236.00 0.0700
860.00 236.50 0.0700
1340.00 237.00 0.0700
1460.00 238.00 0.0700
1470.00 239.00 0.0700
1510.00 240.00 0.0700

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.27 233.87 .770E+04 1.0 0.74 132.49
0.53 234.13 .155E+05 2.9 1.10 88.48
0.80 234.40 .234E+05 5.5 1.37 71.19
1.15 234.75 .636E+05 11.6 1.07 91.11
1.50 235.10 .169E+06 24.5 0.85 114.97
1.85 235.45 .431E+06 54.5 0.74 131.62
2.20 235.80 .882E+06 117.3 0.78 125.39
2.55 236.15 .153E+07 218.3 0.83 117.06
2.90 236.50 .246E+07 375.3 0.89 109.30
3.25 236.85 .394E+07 579.0 0.86 113.45
3.60 237.20 .612E+07 1010.3 0.97 101.01
3.95 237.55 .851E+07 1631.3 1.12 86.99
4.30 237.90 .111E+08 2391.8 1.27 77.05
4.65 238.25 .137E+08 3337.5 1.42 68.52
5.00 238.60 .164E+08 4438.7 1.58 61.71
5.35 238.95 .192E+08 5671.0 1.73 56.40
5.70 239.30 .220E+08 6955.1 1.85 52.75
6.05 239.65 .249E+08 8355.2 1.96 49.76
6.40 240.00 .280E+08 9884.7 2.07 47.18

```

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (0301) ***** 72.82 30.00 39.74 1.95 0.75
OUTFLOW : ID= 1 (0302) ***** 71.37 32.00 39.74 1.94 0.75

```

```

-----
| ADD HYD (0303) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
+ ID1= 1 (0012): 1531.52 6.945 23.00 37.80
+ ID2= 2 (0302): 19971.09 71.374 32.00 39.74
=====
ID = 3 (0303): 21472.61 76.208 31.50 39.60

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHVD (0011) | Area (ha)=3099.17 Curve Number (CN)= 57.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
| U.H. Tp(hrs)= 11.10

```

```

Unit Hyd Qpeak (cms) = 10.949
PEAK FLOW (cms) = 13.278 (i)
TIME TO PEAK (hrs) = 22.500
RUNOFF VOLUME (mm) = 34.008
TOTAL RAINFALL (mm) = 104.500
RUNOFF COEFFICIENT = 0.325

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ROUTE CHN (0401) |
| IN= 2--> OTR= 1 | Routing time step (min)'= 15.00
-----

```

```

<----- DATA FOR SECTION ( 1.1) ----->
Distance Elevation Manning
0.00 236.00 0.0500
72.00 235.00 0.0500
492.00 230.00 0.0500
581.00 229.20 0.0500 /0.0350 Main Channel

```

```

582.00 228.40 0.0350 Main Channel
583.00 228.40 0.0350 Main Channel
584.00 229.20 0.0350 /0.0500 Main Channel
588.00 229.20 0.0500
607.00 230.00 0.0500
627.00 235.00 0.0500

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.27 228.67 .302E+04 0.1 0.21 666.69
0.53 228.93 .756E+04 0.3 0.30 465.92
0.80 229.20 .136E+05 0.6 0.37 377.99
1.16 229.56 .111E+06 3.0 0.23 616.66
1.52 229.92 .358E+06 11.5 0.27 519.91
1.89 230.29 .740E+06 30.1 0.35 410.09
2.25 230.65 .122E+07 59.5 0.41 342.49
2.61 231.01 .180E+07 100.6 0.47 298.58
2.97 231.37 .248E+07 154.6 0.53 267.22
3.34 231.74 .326E+07 222.9 0.58 243.40
3.70 232.10 .413E+07 306.6 0.63 224.51
4.06 232.46 .510E+07 406.9 0.68 209.05
4.42 232.82 .617E+07 524.8 0.72 196.10
4.79 233.19 .734E+07 661.4 0.77 185.06
5.15 233.55 .861E+07 817.9 0.81 175.49
5.51 233.91 .998E+07 995.2 0.85 167.10
5.87 234.27 .114E+08 1194.3 0.89 159.67
6.24 234.64 .130E+08 1416.3 0.93 153.03
6.60 235.00 .147E+08 1662.2 0.96 147.05

```

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (0011) 3099.17 13.28 22.50 34.01 1.56 0.28
OUTFLOW : ID= 1 (0401) 3099.17 9.81 29.00 34.01 1.45 0.26

```

```

-----
| CALIB |
| NASHVD (0014) | Area (ha)=2163.09 Curve Number (CN)= 69.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
| U.H. Tp(hrs)= 25.10

```

Unit Hyd Qpeak (cms) = 3.379

```

PEAK FLOW (cms) = 6.045 (i)
TIME TO PEAK (hrs) = 36.500
RUNOFF VOLUME (mm) = 46.346
TOTAL RAINFALL (mm) = 104.500
RUNOFF COEFFICIENT = 0.444

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| ADD HYD (0402) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
+ ID1= 1 (0014): 2163.09 6.045 36.50 46.35
+ ID2= 2 (0401): 3099.17 9.807 29.00 34.01
=====
ID = 3 (0402): 5262.26 15.378 31.00 39.08

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| ADD HYD (0501) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
+ ID1= 1 (0303): 21472.61 76.208 31.50 39.60
+ ID2= 2 (0402): 5262.26 15.378 31.00 39.08
=====
ID = 3 (0501): 26734.87 91.569 31.50 39.50

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| ROUTE CHN (0503) |
| IN= 2--> OTR= 1 | Routing time step (min)'= 15.00
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<----- DATA FOR SECTION ( 1.1) ----->
Distance Elevation Manning
0.00 237.00 0.0700
425.00 235.00 0.0700
445.00 234.00 0.0700
480.00 233.50 0.0700
1535.00 233.50 0.0700 /0.0350 Main Channel
1536.50 232.70 0.0350 Main Channel
1537.50 232.70 0.0350 Main Channel
1540.00 233.50 0.0350 /0.0700 Main Channel

```

1605.00	233.20	0.0700
1610.00	233.10	0.0700
2135.00	233.10	0.0700
2440.00	233.50	0.0700
2505.00	233.50	0.0700
2530.00	234.00	0.0700
2535.00	235.00	0.0700
2540.00	236.00	0.0700
2605.00	238.00	0.0700
2645.00	241.00	0.0700

----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.20	232.90	.385E+03	0.2	0.55	38.92
0.40	233.10	.103E+04	0.6	0.80	26.75
0.60	233.30	.159E+06	39.5	0.32	67.02
0.80	233.50	.366E+06	133.3	0.47	45.74
1.03	233.73	.977E+06	428.7	0.56	37.97
1.27	233.97	.160E+07	914.3	0.74	29.09
1.50	234.20	.222E+07	1556.9	0.90	23.78
1.73	234.43	.285E+07	2336.2	1.05	20.32
1.97	234.67	.348E+07	3240.2	1.20	17.88
2.20	234.90	.411E+07	4260.9	1.33	16.07
2.43	235.13	.474E+07	5359.1	1.45	14.75
2.67	235.37	.539E+07	6537.6	1.56	13.74
2.90	235.60	.605E+07	7819.4	1.66	12.90
3.13	235.83	.673E+07	9203.8	1.76	12.19
3.37	236.07	.743E+07	10683.6	1.85	11.59
3.60	236.30	.814E+07	12247.8	1.93	11.07
3.83	236.53	.887E+07	13912.8	2.01	10.62
4.07	236.77	.961E+07	15679.0	2.09	10.22
4.30	237.00	1.04E+08	17546.8	2.17	9.85

<---- hydrograph ----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLW : ID= 2 (0501) *****	91.57	31.50	39.50	0.71	0.39
OUTFLOW: ID= 1 (0503) *****	91.37	32.00	39.50	0.71	0.39

RESERVOIR (0510)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	27.8300	83.9600
3.8000	14.0500	*****	101.7400
11.0800	33.6500	*****	145.2700
20.8900	61.8900	0.0000	0.0000

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
INFLW : ID= 2 (0503) *****	91.366	32.00	39.50
OUTFLOW: ID= 1 (0510) *****	91.365	32.25	39.50

PEAK FLOW REDUCTION [Qout/Qin](%)=100.00
TIME SHIFT OF PEAK FLOW (min)= 15.00
MAXIMUM STORAGE USED (ha.m.)= 85.8059

ROUTE CHN (0513)
IN= 2--> OUT= 1

Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning
0.00	237.00	0.0700
425.00	235.00	0.0700
445.00	234.00	0.0700
480.00	233.50	0.0700
1535.00	233.50	0.0700 / 0.0350 Main Channel
1536.50	232.70	0.0350 Main Channel
1537.50	232.70	0.0350 Main Channel
1540.00	233.50	0.0350 / 0.0700 Main Channel
1605.00	233.20	0.0700
1610.00	233.10	0.0700
2135.00	233.10	0.0700
2440.00	233.50	0.0700
2505.00	233.50	0.0700
2530.00	234.00	0.0700
2535.00	235.00	0.0700
2540.00	236.00	0.0700
2605.00	238.00	0.0700
2645.00	241.00	0.0700

----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.20	232.90	.291E+03	0.2	0.55	29.43
0.40	233.10	.777E+03	0.6	0.80	20.23
0.60	233.30	.120E+06	39.5	0.32	50.68
0.80	233.50	.277E+06	133.3	0.47	34.59
1.03	233.73	.739E+06	428.7	0.56	28.72

1.27	233.97	.121E+07	914.3	0.74	22.00
1.50	234.20	.168E+07	1556.9	0.90	17.98
1.73	234.43	.215E+07	2336.2	1.05	15.37
1.97	234.67	.263E+07	3240.2	1.20	13.52
2.20	234.90	.311E+07	4260.9	1.33	12.15
2.43	235.13	.359E+07	5359.1	1.45	11.15
2.67	235.37	.408E+07	6537.6	1.56	10.39
2.90	235.60	.458E+07	7819.4	1.66	9.76
3.13	235.83	.509E+07	9203.8	1.76	9.22
3.37	236.07	.562E+07	10683.6	1.85	8.76
3.60	236.30	.615E+07	12247.8	1.93	8.37
3.83	236.53	.670E+07	13912.8	2.01	8.03
4.07	236.77	.727E+07	15679.0	2.09	7.73
4.30	237.00	.784E+07	17546.8	2.17	7.45

<---- hydrograph ----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLW : ID= 2 (0510) *****	91.36	32.25	39.50	0.71	0.39
OUTFLOW: ID= 1 (0513) *****	91.35	32.50	39.50	0.71	0.39

ADD HYD (0504)

1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0015):	1059.88	7.975	20.50	51.18
+ ID2= 2 (0513):	26734.87	91.349	32.50	39.50
*****	*****	*****	*****	*****
ID = 3 (0504):	27794.75	94.824	32.25	39.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0505)

1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0013):	1627.59	6.437	27.00	41.90
+ ID2= 2 (0504):	27794.75	94.824	32.25	39.94
*****	*****	*****	*****	*****
ID = 3 (0505):	29422.34	100.762	31.50	40.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0506)

IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning
0.00	251.00	0.0500
323.00	238.00	0.0500
400.00	229.00	0.0500
545.00	228.00	0.0500
602.00	226.00	0.0500
604.00	225.80	0.0500 / 0.0350 Main Channel
605.00	225.00	0.0350 Main Channel
609.00	225.00	0.0350 Main Channel
610.00	225.80	0.0350 / 0.0500 Main Channel
614.00	226.00	0.0500
650.00	227.00	0.0500
810.00	229.50	0.0500
1855.00	240.00	0.0500
2617.00	259.00	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.80	225.80	.292E+05	3.5	0.87	139.36
2.13	227.13	.441E+06	50.6	0.84	145.20
3.45	228.45	.197E+07	262.2	0.97	125.42
4.78	229.78	.557E+07	971.2	1.27	95.57
6.11	231.11	.106E+08	2326.5	1.61	75.63
7.43	232.43	.169E+08	4425.6	1.90	63.77
8.76	233.76	.247E+08	7361.8	2.17	55.90
10.08	235.08	.338E+08	11229.7	2.42	50.22
11.41	236.41	.444E+08	16121.0	2.65	45.87
12.74	237.74	.563E+08	22124.8	2.86	42.40
14.06	239.06	.697E+08	29005.2	3.03	40.02
15.39	240.39	.846E+08	37433.0	3.23	37.66
16.72	241.72	.101E+09	47908.6	3.47	34.97
18.04	243.04	.117E+09	59742.1	3.71	32.73
19.37	244.37	.135E+09	72949.0	3.94	30.83
20.69	245.69	.153E+09	87549.6	4.16	29.20
22.02	247.02	.173E+09	*****	4.37	27.79
23.35	248.35	.193E+09	*****	4.58	26.55
24.67	249.67	.214E+09	*****	4.77	25.45

<---- hydrograph ----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
------	-------	-------	------	-----------	---------

(ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0505) ***** 100.76 31.50 40.05 2.44 0.86
 OUTFLOW : ID= 1 (0506) ***** 97.99 34.50 40.05 2.42 0.86

RESERVOIR (0512)
 DT= 15.0 min
 OUTFLOW STORAGE OUTFLOW STORAGE
 (cms) (ha.m.) (cms) (ha.m.)
 0.0000 0.0000 ***** 16.2850
 27.2000 3.7650 ***** 20.9160
 76.8000 7.7410 ***** 25.9120
 ***** 11.0650 ***** 36.6600
 ***** 11.9150 0.0000 0.0000

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (0506) ***** 97.993 34.50 40.05
 OUTFLOW : ID= 1 (0512) ***** 97.979 34.75 40.05

PEAK FLOW REDUCTION [Qout/Qin](%)= 99.99
 TIME SHIFT OF PEAK FLOW (min)= 15.00
 MAXIMUM STORAGE USED (ha.m.)= 9.1351

ROUTE CHN (0514)
 IN= 2--> OUT= 1
 Routing time step (min)'= 15.00

DATA FOR SECTION (1.1) -----
 Distance Elevation Manning
 0.00 230.00 0.0500
 400.00 229.00 0.0500
 545.00 228.00 0.0500
 602.00 226.00 0.0500
 604.00 225.80 0.0500 /0.0350 Main Channel
 605.00 225.00 0.0350 Main Channel
 609.00 225.00 0.0350 Main Channel
 610.00 225.80 0.0350 /0.0500 Main Channel
 614.00 226.00 0.0500
 650.00 227.00 0.0500
 810.00 229.50 0.0500

TRAVEL TIME TABLE -----
 DEPTH ELV VOLIME FLOW RATE VELOCITY TRAV TIME
 (m) (m) (cu.m.) (cms) (m/s) (min)
 0.20 225.20 243E+04 0.3 0.39 121.91
 0.40 225.40 515E+04 1.1 0.59 80.58
 0.60 225.60 815E+04 2.1 0.74 64.02
 0.80 225.80 114E+05 3.5 0.87 54.69
 1.05 226.05 184E+05 6.1 0.94 50.48
 1.29 226.29 346E+05 10.3 0.85 55.99
 1.54 226.54 620E+05 17.4 0.80 59.38
 1.79 226.79 101E+06 28.3 0.81 59.18
 2.03 227.03 151E+06 43.8 0.83 57.35
 2.28 227.28 215E+06 63.9 0.85 55.99
 2.53 227.53 295E+06 91.4 0.89 53.82
 2.77 227.77 391E+06 127.0 0.93 51.35
 3.02 228.02 504E+06 170.6 0.97 49.24
 3.27 228.27 644E+06 215.5 0.96 49.84
 3.51 228.51 821E+06 279.7 0.97 48.92
 3.76 228.76 103E+07 363.1 1.00 47.47
 4.01 229.01 128E+07 466.2 1.04 45.89
 4.25 229.25 159E+07 572.0 1.03 46.41
 4.50 229.50 198E+07 719.1 1.04 45.95

hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0512) ***** 97.98 34.75 40.05 2.57 0.89
 OUTFLOW : ID= 1 (0514) ***** 97.63 35.50 40.05 2.57 0.89

ADD HYD (0507)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0016): 3094.49 13.323 29.50 52.45
 + ID2= 2 (0514): 29422.34 97.631 35.50 40.05
 ID = 3 (0507): 32516.83 109.854 35.25 41.23

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ** SIMULATION NUMBER: 5 **

READ STORM Filename: C:\Users\jaccott\AppData
 Local\Temp\ata\locat\Temp\
 88073fbd-731f-4992-8e5b-76831528f0f7\0bbba830
 Ptotal=117.80 mm Comments: 50yr/24hr

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	0.00	6.50	10.01	12.75	4.12	19.00	0.59
0.50	0.59	6.75	10.01	13.00	4.12	19.25	0.59
0.75	0.59	7.00	10.01	13.25	4.12	19.50	0.59
1.00	0.59	7.25	10.01	13.50	4.12	19.75	0.59
1.25	0.59	7.50	10.01	13.75	4.12	20.00	0.59
1.50	0.59	7.75	10.01	14.00	4.12	20.25	0.59
1.75	0.59	8.00	10.01	14.25	4.12	20.50	0.59
2.00	0.59	8.25	10.01	14.50	2.36	20.75	0.59
2.25	0.59	8.50	27.09	14.75	2.36	21.00	0.59
2.50	0.59	8.75	27.09	15.00	2.36	21.25	0.59
2.75	0.59	9.00	27.09	15.25	2.36	21.50	0.59
3.00	0.59	9.25	27.09	15.50	2.36	21.75	0.59
3.25	0.59	9.50	27.09	15.75	2.36	22.00	0.59
3.50	0.59	9.75	27.09	16.00	2.36	22.25	0.59
3.75	0.59	10.00	27.09	16.25	2.36	22.50	0.59
4.00	0.59	10.25	27.09	16.50	1.18	22.75	0.59
4.25	0.59	10.50	7.66	16.75	1.18	23.00	0.59
4.50	3.53	10.75	7.66	17.00	1.18	23.25	0.59
4.75	3.53	11.00	7.66	17.25	1.18	23.50	0.59
5.00	3.53	11.25	7.66	17.50	1.18	23.75	0.59
5.25	3.53	11.50	7.66	17.75	1.18	24.00	0.59
5.50	3.53	11.75	7.66	18.00	1.18	24.25	0.59
5.75	3.53	12.00	7.66	18.25	1.18		
6.00	3.53	12.25	7.66	18.50	0.59		
6.25	3.53	12.50	4.12	18.75	0.59		

CALIB
 NASHYD (0016) Area (ha)=3094.49 Curve Number (CN)= 74.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 18.10

Unit Hyd Qpeak (cms)= 6.704

PEAK FLOW (cms)= 15.998 (1)
 TIME TO PEAK (hrs)= 29.500
 RUNOFF VOLUME (mm)= 62.976
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.535

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0013) Area (ha)=1627.59 Curve Number (CN)= 65.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 15.50

Unit Hyd Qpeak (cms)= 4.118

PEAK FLOW (cms)= 7.834 (1)
 TIME TO PEAK (hrs)= 27.000
 RUNOFF VOLUME (mm)= 50.983
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.433

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0015) Area (ha)=1059.88 Curve Number (CN)= 73.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 9.30

Unit Hyd Qpeak (cms)= 4.469

PEAK FLOW (cms)= 9.598 (1)
 TIME TO PEAK (hrs)= 20.250
 RUNOFF VOLUME (mm)= 61.544
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.522

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0012) Area (ha)=1501.52 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 11.50

Unit Hyd Qpeak (cms)= 5.120

PEAK FLOW (cms)= 8.498 (1)

TIME TO PEAK (hrs)= 23.000
 RUNOFF VOLUME (mm)= 46.236
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.392

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHVD (0093) Area (ha)= 381.73 Curve Number (CN)= 69.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 14.90

Unit Hyd Qpeak (cms)= 1.005

PEAK FLOW (cms)= 2.094 (i)
 TIME TO PEAK (hrs)= 26.250
 RUNOFF VOLUME (mm)= 56.073
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.476

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHVD (0010) Area (ha)=2842.76 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 10.80

Unit Hyd Qpeak (cms)= 10.322

PEAK FLOW (cms)= 18.288 (i)
 TIME TO PEAK (hrs)= 22.000
 RUNOFF VOLUME (mm)= 49.766
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.422

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHVD (0007) Area (ha)=1373.84 Curve Number (CN)= 73.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 10.70

Unit Hyd Qpeak (cms)= 5.035

PEAK FLOW (cms)= 11.051 (i)
 TIME TO PEAK (hrs)= 21.750
 RUNOFF VOLUME (mm)= 61.544
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.522

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHVD (0005) Area (ha)=1444.47 Curve Number (CN)= 60.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 8.00

Unit Hyd Qpeak (cms)= 7.080

PEAK FLOW (cms)= 10.776 (i)
 TIME TO PEAK (hrs)= 19.250
 RUNOFF VOLUME (mm)= 45.099
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.383

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHVD (0006) Area (ha)=1504.44 Curve Number (CN)= 73.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 12.20

Unit Hyd Qpeak (cms)= 4.836

PEAK FLOW (cms)= 10.806 (i)
 TIME TO PEAK (hrs)= 23.500
 RUNOFF VOLUME (mm)= 61.544
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.522

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

NASHVD (0001) Area (ha)=2873.64 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 11.00

Unit Hyd Qpeak (cms)= 10.244

PEAK FLOW (cms)= 16.894 (i)
 TIME TO PEAK (hrs)= 22.250
 RUNOFF VOLUME (mm)= 46.236
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.392

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHVD (0002) Area (ha)= 988.72 Curve Number (CN)= 58.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 U.H. Tp(hrs)= 9.10

Unit Hyd Qpeak (cms)= 4.261

PEAK FLOW (cms)= 6.314 (i)
 TIME TO PEAK (hrs)= 20.250
 RUNOFF VOLUME (mm)= 42.880
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.364

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0101)
 1 + 2 = 3 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID= 1 (0001): 2873.64 16.894 22.25 46.24
 + ID= 2 (0002): 988.72 6.314 20.25 42.88
 ID = 3 (0101): 3862.36 23.056 21.75 45.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0102)
 IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

Distance	Elevation	Manning	
0.00	250.00	0.0700	
43.00	240.00	0.0700 / 0.0350	Main Channel
44.00	239.20	0.0350	Main Channel
45.00	239.20	0.0350	Main Channel
46.00	240.00	0.0350 / 0.0700	Main Channel
63.50	241.00	0.0700	
71.00	250.00	0.0700	

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.40	239.60	.642E+04	0.5	0.80	221.73
0.80	240.00	.171E+05	1.8	1.15	155.20
1.39	240.59	.764E+05	8.0	1.12	159.62
1.98	241.18	.214E+06	23.7	1.19	150.23
2.56	241.76	.385E+06	51.8	1.44	123.91
3.15	242.35	.575E+06	90.1	1.68	106.38
3.74	242.94	.784E+06	138.5	1.89	94.40
4.33	243.53	.101E+07	196.9	2.08	85.71
4.92	244.12	.126E+07	265.6	2.26	79.08
5.51	244.71	.153E+07	344.7	2.42	73.81
6.09	245.29	.181E+07	434.5	2.57	69.50
6.68	245.88	.212E+07	535.2	2.71	65.89
7.27	246.47	.244E+07	647.3	2.84	62.81
7.86	247.06	.278E+07	771.1	2.97	60.13
8.45	247.65	.314E+07	906.8	3.09	57.77
9.04	248.24	.352E+07	1054.8	3.20	55.67
9.62	248.82	.392E+07	1215.5	3.32	53.79
10.21	249.41	.434E+07	1389.1	3.42	52.09
10.80	250.00	.478E+07	1576.1	3.53	50.53

INFLOW : ID=	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
2 (0101)	3862.36	23.06	21.75	45.38	1.95	1.18	
OUTFLOW: ID=	1 (0102)	3862.36	21.96	24.25	45.38	1.91	1.18

ADD HYD (0103)
 1 + 2 = 3 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)

ID1= 1 (0006): 1504.44 10.806 23.50 61.54
 + ID2= 2 (0102): 3862.36 21.964 24.25 45.38
 =====
 ID = 3 (0103): 5366.80 32.735 24.00 49.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0104)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0005): 1444.47 10.776 19.25 45.10
 + ID2= 2 (0103): 5366.80 32.735 24.00 49.91
 =====
 ID = 3 (0104): 6811.27 41.803 22.75 48.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0105)
 IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (1.1) -----
 Distance Elevation Manning
 0.00 237.00 0.1000
 330.00 235.00 0.1000 /0.0350 Main Channel
 331.00 234.20 0.0350 Main Channel
 332.00 234.20 0.0350 Main Channel
 333.00 235.00 0.0350 /0.1000 Main Channel
 425.00 235.10 0.1000
 698.00 236.00 0.1000
 1101.00 239.00 0.1000

----- TRAVEL TIME TABLE -----
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
 (m) (m) (cu.m.) (cms) (m/s) (min)
 0.13 234.33 490E+03 0.1 0.64 82.66
 0.27 234.47 112E+04 0.3 0.93 56.72
 0.40 234.60 189E+04 0.7 1.14 45.94
 0.53 234.73 280E+04 1.2 1.33 39.64
 0.67 234.87 385E+04 1.8 1.49 35.34
 0.80 235.00 504E+04 2.6 1.63 32.16
 0.95 235.15 442E+05 5.3 0.45 116.42
 1.11 235.31 138E+06 17.7 0.44 120.51
 1.26 235.46 247E+06 38.2 0.49 107.48
 1.42 235.62 400E+06 69.8 0.55 95.60
 1.57 235.77 589E+06 114.0 0.61 86.08
 1.72 235.92 812E+06 172.6 0.67 78.45
 1.88 236.08 107E+07 250.3 0.74 71.19
 2.03 236.23 135E+07 347.8 0.81 64.71
 2.18 236.38 165E+07 461.9 0.88 59.65
 2.34 236.54 198E+07 593.4 0.95 55.58
 2.49 236.69 233E+07 742.8 1.01 52.21
 2.65 236.85 270E+07 910.6 1.06 49.36
 2.80 237.00 309E+07 1097.6 1.12 46.91

----- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0104) 6811.27 41.80 22.75 48.89 1.28 0.49
 OUTFLOW: ID= 1 (0105) 6811.27 40.96 24.50 48.89 1.27 0.49

CALIB (0091) | Area (ha)= 281.97 Curve Number (CN)= 65.0
 NASHYD (0091) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 5.80

Unit Hyd Qpeak (cms)= 1.906
 PEAK FLOW (cms)= 3.048 (l)
 TIME TO PEAK (hrs)= 16.500
 RUNOFF VOLUME (mm)= 50.983
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.433

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0106)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0091): 281.97 3.048 16.50 50.98
 + ID2= 2 (0105): 6811.27 40.957 24.50 48.89
 =====
 ID = 3 (0106): 7093.24 42.433 24.00 48.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0107)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0007): 1373.84 11.051 21.75 61.54
 + ID2= 2 (0106): 7093.24 42.433 24.00 48.97
 =====
 ID = 3 (0107): 8467.08 53.176 23.75 51.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0108)
 IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (1.1) -----
 Distance Elevation Manning
 0.00 234.00 0.1000
 400.00 231.00 0.1000 /0.0350 Main Channel
 401.00 229.70 0.0350 Main Channel
 402.00 229.70 0.0350 Main Channel
 404.00 230.50 0.0350 /0.1000 Main Channel
 495.00 231.00 0.1000
 1234.00 256.00 0.1000

----- TRAVEL TIME TABLE -----
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
 (m) (m) (cu.m.) (cms) (m/s) (min)
 0.20 229.90 151E+04 0.1 0.24 395.61
 0.40 230.10 377E+04 0.2 0.35 274.62
 0.60 230.30 678E+04 0.5 0.43 222.06
 0.80 230.50 105E+05 0.9 0.50 190.52
 1.03 230.73 437E+05 2.0 0.27 356.49
 1.27 230.97 134E+06 5.0 0.21 448.69
 1.50 231.20 278E+06 11.4 0.23 405.59
 1.73 231.43 473E+06 21.6 0.26 364.80
 1.97 231.67 718E+06 36.0 0.29 332.76
 2.20 231.90 101E+07 55.1 0.31 306.87
 2.43 232.13 136E+07 79.4 0.33 285.44
 2.67 232.37 176E+07 109.6 0.36 267.34
 2.90 232.60 221E+07 145.9 0.38 251.83
 3.13 232.83 270E+07 189.0 0.40 238.35
 3.37 233.07 325E+07 239.3 0.42 226.51
 3.60 233.30 385E+07 297.1 0.44 216.02
 3.83 233.53 450E+07 363.0 0.46 206.64
 4.07 233.77 520E+07 437.3 0.48 198.20
 4.30 234.00 595E+07 520.5 0.50 190.55

----- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0107) 8467.08 53.18 23.75 51.01 2.18 0.31
 OUTFLOW: ID= 1 (0108) 8467.08 45.63 28.25 51.01 2.08 0.30

CALIB (0092) | Area (ha)=1102.32 Curve Number (CN)= 64.0
 NASHYD (0092) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 16.50

Unit Hyd Qpeak (cms)= 2.620
 PEAK FLOW (cms)= 4.895 (l)
 TIME TO PEAK (hrs)= 28.000
 RUNOFF VOLUME (mm)= 49.766
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.422

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0109)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0092): 1102.32 4.895 28.00 49.77
 + ID2= 2 (0108): 8467.08 45.631 28.25 51.01
 =====
 ID = 3 (0109): 9569.40 50.525 28.25 50.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0110)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)

ID1= 1 (0010): 2842.76 18.288 22.00 49.77
 + ID2= 2 (0109): 9569.40 50.525 28.25 50.67
 =====
 ID = 3 (0110): 12412.16 66.080 26.75 50.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0111) |
 IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning	
0.00	236.00	0.1000	
275.00	232.00	0.1000	
1039.00	230.00	0.1000 /0.0350	Main Channel
1040.00	229.20	0.0350	Main Channel
1041.00	229.20	0.0350	Main Channel
1043.00	230.00	0.0350 /0.1000	Main Channel
1117.00	230.60	0.1000	
1325.00	234.00	0.1000	

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.20	229.40	.985E+03	0.0	0.14	431.34
0.40	229.60	.251E+04	0.1	0.20	298.03
0.60	229.80	.457E+04	0.3	0.25	239.97
0.80	230.00	.717E+04	0.6	0.29	205.17
1.07	230.27	.754E+05	2.2	0.11	561.90
1.33	230.53	.272E+06	8.6	0.11	527.12
1.60	230.80	.594E+06	22.7	0.14	436.37
1.87	231.07	.103E+07	46.2	0.16	371.32
2.13	231.33	.158E+07	80.7	0.18	325.64
2.40	231.60	.224E+07	127.8	0.20	291.74
2.67	231.87	.301E+07	189.0	0.22	265.46
2.93	232.13	.389E+07	271.8	0.25	238.27
3.20	232.40	.481E+07	376.4	0.28	212.79
3.47	232.67	.576E+07	496.0	0.31	193.49
3.73	232.93	.674E+07	630.4	0.33	178.30
4.00	233.20	.776E+07	779.4	0.36	165.99
4.27	233.47	.881E+07	943.0	0.38	155.79
4.53	233.73	.990E+07	1121.1	0.41	147.16
4.80	234.00	.110E+08	1313.9	0.43	139.76

<----- hydrograph -----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (0110) *****	66.08	26.75	50.62	2.02	0.17
OUTFLOW: ID= 1 (0111) *****	56.29	31.50	50.62	1.95	0.17

ADD HYD (0112) |
 1 + 2 + 3 | AREA QPEAK TPEAK R.V.

(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0093):	381.73	2.094	26.25 56.07
+ ID2= 2 (0111):	12412.16	56.291	31.50 50.62
=====			
ID = 3 (0112):	12793.89	58.221	31.00 50.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0008) | Area (ha)=1549.97 Curve Number (CN)= 60.0
 NASHVD (0008) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 9.10

Unit Hyd Qpeak (cms)= 6.679
 PEAK FLOW (cms)= 10.418 (1)
 TIME TO PEAK (hrs)= 20.250
 RUNOFF VOLUME (mm)= 45.099
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.383

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0003) | Area (ha)=2954.54 Curve Number (CN)= 64.0
 NASHVD (0003) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 13.10

Unit Hyd Qpeak (cms)= 8.844
 PEAK FLOW (cms)= 16.096 (1)
 TIME TO PEAK (hrs)= 24.500
 RUNOFF VOLUME (mm)= 49.766

TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.422

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0004) | Area (ha)=2672.69 Curve Number (CN)= 53.0
 NASHVD (0004) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 10.70

Unit Hyd Qpeak (cms)= 9.795
 PEAK FLOW (cms)= 13.066 (1)
 TIME TO PEAK (hrs)= 22.000
 RUNOFF VOLUME (mm)= 37.639
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.320

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0201) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)

ID1= 1 (0003):	2954.54	16.096	24.50 49.77
+ ID2= 2 (0004):	2672.69	13.066	22.00 37.64
=====			
ID = 3 (0201):	5627.23	28.905	23.25 44.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0202) |
 IN= 3--> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning	
0.00	239.00	0.1000	
690.00	230.00	0.1000 /0.0350	Main Channel
691.00	229.20	0.0350	Main Channel
692.00	229.20	0.0350	Main Channel
693.00	229.50	0.0350 /0.1000	Main Channel
1190.00	230.00	0.1000	
1417.00	255.00	0.1000	

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.30	229.50	.347E+04	0.4	0.64	136.57
0.80	230.00	.864E+06	45.2	0.36	318.78
1.30	230.50	.265E+07	274.1	0.71	161.20
1.80	231.00	.459E+07	651.8	0.97	117.24
2.30	231.50	.667E+07	1165.3	1.20	95.34
2.80	232.00	.889E+07	1810.2	1.40	81.89
3.30	232.50	.113E+08	2585.1	1.57	72.65
3.80	233.00	.138E+08	3490.4	1.73	65.85
4.30	233.50	.165E+08	4527.4	1.89	60.59
4.80	234.00	.193E+08	5697.8	2.03	56.38
5.30	234.50	.222E+08	7003.8	2.16	52.91
5.80	235.00	.253E+08	8447.7	2.28	50.00
6.30	235.50	.286E+08	10032.3	2.40	47.52
6.80	236.00	.320E+08	11760.1	2.52	45.36
7.30	236.50	.356E+08	13634.1	2.63	43.46
7.80	237.00	.393E+08	15657.0	2.73	41.78
8.30	237.50	.431E+08	17831.7	2.84	40.28
8.80	238.00	.471E+08	20161.3	2.93	38.92
9.30	238.50	.512E+08	22648.7	3.03	37.69

<----- hydrograph -----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (0201) 5627.23	28.91	23.25	44.01	0.62	0.45
OUTFLOW: ID= 1 (0202) 5627.23	26.27	28.75	44.01	0.59	0.47

ADD HYD (0203) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)

ID1= 1 (0008):	1549.97	10.418	20.25 45.10
+ ID2= 2 (0202):	5627.23	26.270	28.75 44.01
=====			
ID = 3 (0203):	7177.20	33.663	26.00 44.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| ADD HYD (0301) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
ID1= 1 (0111): 12793.89 50.221 31.00 50.78
+ ID2= 2 (0203): 7177.20 33.663 26.00 44.24
=====
ID = 3 (0301): 19971.09 89.797 29.50 48.43
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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| ROUTE CHN (0302) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)'= 15.00
-----

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-----
|<----- DATA FOR SECTION ( 1.1) ----->|
| Distance Elevation Manning |
| 0.00 240.00 0.0700 |
| 110.00 239.00 0.0700 |
| 161.00 238.00 0.0700 |
| 250.00 237.00 0.0700 |
| 420.00 236.00 0.0700 |
| 560.00 235.00 0.0700 |
| 589.00 234.40 0.0700 /0.0350 Main Channel |
| 589.10 233.60 0.0350 Main Channel |
| 594.00 233.60 0.0350 Main Channel |
| 594.10 234.40 0.0350 /0.0700 Main Channel |
| 615.00 235.00 0.0700 |
| 740.00 236.00 0.0700 |
| 860.00 236.50 0.0700 |
| 1340.00 237.00 0.0700 |
| 1460.00 238.00 0.0700 |
| 1470.00 239.00 0.0700 |
| 1510.00 240.00 0.0700 |
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|<----- TRAVEL TIME TABLE ----->|
| DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME |
| (m) (m) (cu.m.) (cms) (m/s) (min) |
| 0.27 233.87 .770E+04 1.0 0.74 132.49 |
| 0.53 234.13 .155E+05 2.9 1.10 88.48 |
| 0.80 234.40 .234E+05 5.5 1.37 71.19 |
| 1.15 234.75 .636E+05 11.6 1.07 91.11 |
| 1.50 235.10 .169E+06 24.5 0.85 114.97 |
| 1.85 235.45 .431E+06 54.5 0.74 131.62 |
| 2.20 235.80 .882E+06 117.3 0.78 125.39 |
| 2.55 236.15 .153E+07 218.3 0.83 117.06 |
| 2.90 236.50 .246E+07 375.3 0.89 109.30 |
| 3.25 236.85 .394E+07 579.0 0.86 113.45 |
| 3.60 237.20 .612E+07 1010.3 0.97 101.01 |
| 3.95 237.55 .851E+07 1631.3 1.12 86.99 |
| 4.30 237.90 .111E+08 2391.8 1.27 77.05 |
| 4.65 238.25 .137E+08 3337.5 1.42 68.52 |
| 5.00 238.60 .164E+08 4438.7 1.58 61.71 |
| 5.35 238.95 .192E+08 5671.0 1.73 56.40 |
| 5.70 239.30 .220E+08 6955.1 1.85 52.75 |
| 6.05 239.65 .249E+08 8355.2 1.96 49.76 |
| 6.40 240.00 .280E+08 9884.7 2.07 47.18 |
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|<----- hydrograph -----> |<-pipe / channel->|
| AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL |
| (ha) (cms) (hrs) (mm) (m) (m/s) |
INFLOW : ID= 2 (0301) ***** 89.80 29.50 48.43 2.05 0.76
OUTFLOW: ID= 1 (0302) ***** 88.02 31.75 48.43 2.03 0.76
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| ADD HYD (0303) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
ID1= 1 (0112): 1501.52 8.498 23.00 46.24
+ ID2= 2 (0302): 19971.09 88.024 31.75 48.43
=====
ID = 3 (0303): 21472.61 94.050 31.25 48.28
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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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| CALIB |
| NASHYD (0011) | Area (ha)=3099.17 Curve Number (CN)= 57.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10 |
| U.H. Tp(hrs)= 11.10 |
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Unit Hyd Qpeak (cms) = 10.949

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PEAK FLOW (cms) = 16.328 (1)
TIME TO PEAK (hrs) = 22.500
RUNOFF VOLUME (mm) = 41.798
TOTAL RAINFALL (mm) = 117.800
RUNOFF COEFFICIENT = 0.355

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(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| ROUTE CHN (0401) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)'= 15.00
-----

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-----
|<----- DATA FOR SECTION ( 1.1) ----->|
| Distance Elevation Manning |
| 0.00 236.00 0.0500 |
| 72.00 235.00 0.0500 |
| 492.00 230.00 0.0500 |
| 581.00 229.20 0.0500 /0.0350 Main Channel |
| 582.00 228.40 0.0350 Main Channel |
| 583.00 228.40 0.0350 Main Channel |
| 584.00 229.20 0.0350 /0.0500 Main Channel |
| 588.00 229.20 0.0500 |
| 607.00 230.00 0.0500 |
| 627.00 235.00 0.0500 |
-----

```

```

-----
|<----- TRAVEL TIME TABLE ----->|
| DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME |
| (m) (m) (cu.m.) (cms) (m/s) (min) |
| 0.27 228.67 .302E+04 0.1 0.21 666.69 |
| 0.53 228.93 .756E+04 0.3 0.30 465.92 |
| 0.80 229.20 .136E+05 0.6 0.37 377.99 |
| 1.16 229.56 .111E+06 3.0 0.23 616.66 |
| 1.52 229.92 .358E+06 11.5 0.27 519.91 |
| 1.89 230.28 .740E+06 30.1 0.35 410.09 |
| 2.25 230.65 .122E+07 59.5 0.41 342.49 |
| 2.61 231.01 .180E+07 100.6 0.47 298.58 |
| 2.97 231.37 .248E+07 154.6 0.53 267.22 |
| 3.34 231.74 .326E+07 222.9 0.58 243.40 |
| 3.70 232.10 .413E+07 306.6 0.63 224.51 |
| 4.06 232.46 .510E+07 406.9 0.68 209.05 |
| 4.42 232.82 .617E+07 524.8 0.72 196.10 |
| 4.79 233.19 .734E+07 661.4 0.77 185.06 |
| 5.15 233.55 .861E+07 817.9 0.81 175.49 |
| 5.51 233.91 .998E+07 995.2 0.85 167.10 |
| 5.87 234.27 .114E+08 1194.3 0.89 159.67 |
| 6.24 234.64 .130E+08 1416.3 0.93 153.03 |
| 6.60 235.00 .147E+08 1662.2 0.96 147.05 |
-----

```

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-----
|<----- hydrograph -----> |<-pipe / channel->|
| AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL |
| (ha) (cms) (hrs) (mm) (m) (m/s) |
INFLOW : ID= 2 (0011) 3099.17 16.33 22.50 41.80 1.62 0.29
OUTFLOW: ID= 1 (0401) 3099.17 12.26 29.25 41.80 1.54 0.27
-----

```

```

-----
| CALIB |
| NASHYD (0014) | Area (ha)=2163.09 Curve Number (CN)= 69.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10 |
| U.H. Tp(hrs)= 25.10 |
-----

```

Unit Hyd Qpeak (cms) = 3.379

```

PEAK FLOW (cms) = 7.314 (1)
TIME TO PEAK (hrs) = 36.500
RUNOFF VOLUME (mm) = 56.073
TOTAL RAINFALL (mm) = 117.800
RUNOFF COEFFICIENT = 0.476

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (0402) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
ID1= 1 (0014): 2163.09 7.314 36.50 56.07
+ ID2= 2 (0401): 3099.17 12.262 29.25 41.80
=====
ID = 3 (0402): 5262.26 19.035 31.00 47.66
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (0501) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
ID1= 1 (0303): 21472.61 94.050 31.25 48.28
+ ID2= 2 (0402): 5262.26 19.035 31.00 47.66
=====
ID = 3 (0501): 26734.87 113.072 31.25 48.16
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (0503) |
-----

```

IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

Table with 3 columns: Distance, Elevation, Manning. Rows show data for section 1.1 from 0.00 to 2645.00.

Table with 3 columns: 2540.00, 236.00, 0.0700; 2605.00, 238.00, 0.0700; 2645.00, 241.00, 0.0700.

TRAVEL TIME TABLE. Columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV.TIME. Rows show depth from 0.20 to 4.30 m.

TRAVEL TIME TABLE. Columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV.TIME. Rows show depth from 0.20 to 4.30 m.

hydrograph table. Columns: AREA, QPEAK, TPEAK, R.V., MAX DEPTH, MAX VEL. Rows show inflow and outflow for IDs 2 and 1.

ADD HYD (0504) table. Columns: AREA, QPEAK, TPEAK, R.V. Rows show IDs 1, 2, and 3.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0505) table. Columns: AREA, QPEAK, TPEAK, R.V. Rows show IDs 1, 2, and 3.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0506) | IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

Table with 3 columns: Distance, Elevation, Manning. Rows show data for section 1.1 from 0.00 to 2617.00.

TRAVEL TIME TABLE. Columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV.TIME. Rows show depth from 0.80 to 12.74 m.

RESERVOIR (0510) | IN= 2--> OUT= 1 | DT= 15.0 min

Table with 4 columns: OUTFLOW, STORAGE, OUTFLOW, STORAGE. Rows show values for 0.0000, 3.8000, 11.0800, 20.8900.

Table with 4 columns: AREA, QPEAK, TPEAK, R.V. Rows show values for IDs 2 and 1.

PEAK FLOW REDUCTION [Qout/Qin] (%) = 99.99
TIME SHIFT OF PEAK FLOW (min) = 0.00
MAXIMUM STORAGE USED (ha.m.) = 86.4330

ROUTE CHN (0513) | IN= 2--> OUT= 1 |

Table with 3 columns: Distance, Elevation, Manning. Rows show data for section 1.1 from 0.00 to 2535.00.

14.06	239.06	.697E+08	29005.2	3.03	40.02
15.39	240.39	.846E+08	37433.0	3.23	37.66
16.72	241.72	.101E+09	47908.6	3.47	34.97
18.04	243.04	.117E+09	59742.1	3.71	32.73
19.37	244.37	.135E+09	72949.0	3.94	30.83
20.69	245.69	.153E+09	87549.6	4.16	29.20
22.02	247.02	.173E+09	*****	4.37	27.79
23.35	248.35	.193E+09	*****	4.58	26.55
24.67	249.67	.214E+09	*****	4.77	25.45

ID = 3 (0507): 32516.83 136.048 34.50 50.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

** SIMULATION NUMBER: 6 **

<---- hydrograph ---->						<-pipe / channel-->	
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL		
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW : ID= 2 (0505) *****	124.71	31.25	48.79	2.59	0.88		
OUTFLOW: ID= 1 (0506) *****	121.52	33.75	48.79	2.56	0.88		

RESERVOIR (0512)					
IN= 2--> OUT= 1					
DT= 15.0 min					
OUTFLOW	STORAGE	OUTFLOW	STORAGE		
(cms)	(ha.m.)	(cms)	(ha.m.)		
0.0000	0.0000	*****	15.2850		
27.2000	3.7650	*****	20.9160		
76.8000	7.7410	*****	25.9120		
*****	11.0650	*****	36.6600		
*****	11.9150	*****	0.0000		

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0506) *****	121.516	33.75	48.79
OUTFLOW: ID= 1 (0512) *****	121.503	34.00	48.79

PEAK FLOW REDUCTION [Qout/Qin]{%}= 99.99
TIME SHIFT OF PEAK FLOW (min)= 15.00
MAXIMUM STORAGE USED (ha.m.)= 10.6839

ROUTE CHN (0514) |
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->			
Distance	Elevation	Manning	
0.00	230.00	0.0500	
400.00	229.00	0.0500	
545.00	228.00	0.0500	
602.00	226.00	0.0500	
604.00	225.80	0.0350 / 0.0350	Main Channel
605.00	225.00	0.0350	Main Channel
609.00	225.00	0.0350	Main Channel
610.00	225.80	0.0350 / 0.0500	Main Channel
614.00	226.00	0.0500	
650.00	227.00	0.0500	
810.00	229.50	0.0500	

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.20	225.20	.243E+04	0.3	0.39	121.91
0.40	225.40	.515E+04	1.1	0.59	80.58
0.60	225.60	.815E+04	2.1	0.74	64.02
0.80	225.80	.114E+05	3.5	0.87	54.69
1.05	226.05	.184E+05	6.1	0.94	50.48
1.29	226.29	.346E+05	10.3	0.85	55.99
1.54	226.54	.620E+05	17.4	0.80	59.38
1.79	226.79	.101E+06	28.3	0.81	59.18
2.03	227.03	.151E+06	43.8	0.83	57.35
2.28	227.28	.215E+06	63.9	0.85	55.99
2.53	227.53	.295E+06	91.4	0.89	53.82
2.77	227.77	.391E+06	127.0	0.93	51.35
3.02	228.02	.504E+06	170.6	0.97	49.24
3.27	228.27	.644E+06	215.5	0.96	49.84
3.51	228.51	.821E+06	279.7	0.97	48.92
3.76	228.76	.103E+07	363.1	1.00	47.47
4.01	229.01	.139E+07	465.2	1.04	45.89
4.25	229.25	.189E+07	572.0	1.03	46.41
4.50	229.50	.198E+07	719.1	1.04	45.95

<---- hydrograph ---->						<-pipe / channel-->	
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL		
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW : ID= 2 (0512) *****	121.50	34.00	48.79	2.73	0.92		
OUTFLOW: ID= 1 (0514) *****	121.11	34.75	48.79	2.73	0.92		

ADD HYD (0507)				
1 + 2 = 3				
ID#	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1 = 1 (0016):	3094.49	15.998	29.50	62.98
+ ID2 = 2 (0514):	29422.34	121.109	34.75	48.79

READ STORM | Filename: C:\Users\jscott\AppData
| | Local\Temp\
| | 88073fbd-731f-4992-8e5b-76831528f017\ef2ec18b
| | Ptotal=130.88 mm | Comments: 100yr/24hr

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	0.00	6.50	11.13	12.75	4.58	19.00	0.65
0.50	0.65	6.75	11.13	13.00	4.58	19.25	0.65
0.75	0.65	7.00	11.13	13.25	4.58	19.50	0.65
1.00	0.65	7.25	11.13	13.50	4.58	19.75	0.65
1.25	0.65	7.50	11.13	13.75	4.58	20.00	0.65
1.50	0.65	7.75	11.13	14.00	4.58	20.25	0.65
1.75	0.65	8.00	11.13	14.25	4.58	20.50	0.65
2.00	0.65	8.25	11.13	14.50	2.62	20.75	0.65
2.25	0.65	8.50	30.11	14.75	2.62	21.00	0.65
2.50	0.65	8.75	30.11	15.00	2.62	21.25	0.65
2.75	0.65	9.00	30.11	15.25	2.62	21.50	0.65
3.00	0.65	9.25	30.11	15.50	2.62	21.75	0.65
3.25	0.65	9.50	30.11	15.75	2.62	22.00	0.65
3.50	0.65	9.75	30.11	16.00	2.62	22.25	0.65
3.75	0.65	10.00	30.11	16.25	2.62	22.50	0.65
4.00	0.65	10.25	30.11	16.50	1.31	22.75	0.65
4.25	0.65	10.50	8.51	16.75	1.31	23.00	0.65
4.50	3.93	10.75	8.51	17.00	1.31	23.25	0.65
4.75	3.93	11.00	8.51	17.25	1.31	23.50	0.65
5.00	3.93	11.25	8.51	17.50	1.31	23.75	0.65
5.25	3.93	11.50	8.51	17.75	1.31	24.00	0.65
5.50	3.93	11.75	8.51	18.00	1.31	24.25	0.65
5.75	3.93	12.00	8.51	18.25	1.31		
6.00	3.93	12.25	8.51	18.50	0.65		
6.25	3.93	12.50	4.58	18.75	0.65		

CALIB |
NASHYD (0016) | Area (ha)=3094.49 Curve Number (CN)= 74.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
| U.H. Tp(hrs)= 18.10

Unit Hyd Qpeak (cms)= 6.704

PEAK FLOW (cms)= 18.718 (i)
TIME TO PEAK (hrs)= 29.500
RUNOFF VOLUME (mm)= 73.659
TOTAL RAINFALL (mm)= 130.880
RUNOFF COEFFICIENT = 0.563

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
NASHYD (0013) | Area (ha)=1627.59 Curve Number (CN)= 65.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
| U.H. Tp(hrs)= 15.50

Unit Hyd Qpeak (cms)= 4.118

PEAK FLOW (cms)= 9.275 (i)
TIME TO PEAK (hrs)= 27.000
RUNOFF VOLUME (mm)= 60.331
TOTAL RAINFALL (mm)= 130.880
RUNOFF COEFFICIENT = 0.461

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
NASHYD (0015) | Area (ha)=1059.88 Curve Number (CN)= 73.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
| U.H. Tp(hrs)= 9.30

Unit Hyd Qpeak (cms)= 4.469

PEAK FLOW (cms)= 11.257 (i)
TIME TO PEAK (hrs)= 20.250
RUNOFF VOLUME (mm)= 72.084
TOTAL RAINFALL (mm)= 130.880
RUNOFF COEFFICIENT = 0.551

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHVD (0012) Area (ha)=1501.52 Curve Number (CN)= 61.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
U.H. Tp(hrs)= 11.50

Unit Hyd Qpeak (cms) = 5.120

PEAK FLOW (cms) = 10.114 (1)
TIME TO PEAK (hrs) = 22.750
RUNOFF VOLUME (mm) = 54.968
TOTAL RAINFALL (mm) = 130.880
RUNOFF COEFFICIENT = 0.420

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHVD (0093) Area (ha)= 381.73 Curve Number (CN)= 69.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
U.H. Tp(hrs)= 14.90

Unit Hyd Qpeak (cms) = 1.005

PEAK FLOW (cms) = 2.468 (1)
TIME TO PEAK (hrs) = 26.250
RUNOFF VOLUME (mm) = 66.025
TOTAL RAINFALL (mm) = 130.880
RUNOFF COEFFICIENT = 0.504

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHVD (0010) Area (ha)=2842.76 Curve Number (CN)= 64.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
U.H. Tp(hrs)= 10.80

Unit Hyd Qpeak (cms) = 10.322

PEAK FLOW (cms) = 21.693 (1)
TIME TO PEAK (hrs) = 22.000
RUNOFF VOLUME (mm) = 58.960
TOTAL RAINFALL (mm) = 130.880
RUNOFF COEFFICIENT = 0.450

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHVD (0007) Area (ha)=1373.84 Curve Number (CN)= 73.0
ID= 1 DT=10 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
U.H. Tp(hrs)= 10.70

Unit Hyd Qpeak (cms) = 5.035

PEAK FLOW (cms) = 12.958 (1)
TIME TO PEAK (hrs) = 21.750
RUNOFF VOLUME (mm) = 72.084
TOTAL RAINFALL (mm) = 130.880
RUNOFF COEFFICIENT = 0.551

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHVD (0005) Area (ha)=1444.47 Curve Number (CN)= 60.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
U.H. Tp(hrs)= 8.00

Unit Hyd Qpeak (cms) = 7.080

PEAK FLOW (cms) = 12.846 (1)
TIME TO PEAK (hrs) = 19.000
RUNOFF VOLUME (mm) = 53.676
TOTAL RAINFALL (mm) = 130.880
RUNOFF COEFFICIENT = 0.410

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHVD (0006) Area (ha)=1504.44 Curve Number (CN)= 73.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
U.H. Tp(hrs)= 12.20

Unit Hyd Qpeak (cms) = 4.836

PEAK FLOW (cms)= 12.668 (1)
TIME TO PEAK (hrs)= 23.250
RUNOFF VOLUME (mm)= 72.084
TOTAL RAINFALL (mm)= 130.880
RUNOFF COEFFICIENT = 0.551

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHVD (0001) Area (ha)=2873.64 Curve Number (CN)= 61.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
U.H. Tp(hrs)= 11.00

Unit Hyd Qpeak (cms) = 10.244

PEAK FLOW (cms) = 20.108 (1)
TIME TO PEAK (hrs) = 22.250
RUNOFF VOLUME (mm) = 54.968
TOTAL RAINFALL (mm) = 130.880
RUNOFF COEFFICIENT = 0.420

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHVD (0002) Area (ha)= 988.72 Curve Number (CN)= 58.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
U.H. Tp(hrs)= 9.10

Unit Hyd Qpeak (cms) = 4.261

PEAK FLOW (cms) = 7.542 (1)
TIME TO PEAK (hrs) = 20.250
RUNOFF VOLUME (mm) = 51.147
TOTAL RAINFALL (mm) = 130.880
RUNOFF COEFFICIENT = 0.391

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0101) AREA QPEAK TPEAK R.V.
1 + 2 = 3 (ha) (cms) (hrs) (mm)
ID= 1 (0001): 2873.64 20.108 22.25 54.97
+ ID= 2 (0002): 988.72 7.542 20.25 51.15
=====

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0102) Routing time step (min)'= 15.00
IN= 2--> OUT= 1

DATA FOR SECTION (1.1) -----
Distance Elevation Manning
0.00 250.00 0.0700
43.00 240.00 0.0700 /0.0350 Main Channel
44.00 239.20 0.0350 Main Channel
45.00 239.20 0.0350 Main Channel
46.00 240.00 0.0350 /0.0700 Main Channel
63.50 241.00 0.0700
71.00 250.00 0.0700

TRAVEL TIME TABLE
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.40 239.60 .642E+04 0.5 0.80 221.73
0.80 240.00 .171E+05 1.8 1.15 155.20
1.39 240.59 .764E+05 8.0 1.12 159.62
1.98 241.18 .214E+06 23.7 1.19 150.23
2.56 241.76 .385E+06 51.8 1.44 123.91
3.15 242.35 .575E+06 90.1 1.68 106.38
3.74 242.94 .784E+06 138.5 1.89 94.40
4.33 243.53 .101E+07 196.9 2.08 85.71
4.92 244.12 .126E+07 265.6 2.26 79.08
5.51 244.71 .153E+07 344.7 2.42 73.81
6.09 245.29 .181E+07 434.5 2.57 69.50
6.68 245.88 .212E+07 535.2 2.71 65.89
7.27 246.47 .244E+07 647.3 2.84 62.81
7.86 247.06 .278E+07 771.1 2.97 60.13
8.45 247.65 .314E+07 906.8 3.09 57.77
9.04 248.24 .352E+07 1054.8 3.20 55.67
9.62 248.82 .392E+07 1215.5 3.32 53.79
10.21 249.41 .434E+07 1389.1 3.42 52.09
10.80 250.00 .478E+07 1576.1 3.53 50.53

<---- hydrograph ----> <-pipe / channel-->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0101)	3862.36	27.47	21.75	53.99	2.06	1.22
OUTFLOW: ID= 1 (0102)	3862.36	26.23	24.25	53.99	2.02	1.20

```

ADD HYD (0103)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0005): 1504.44 12.668 23.25 72.08
+ ID2= 2 (0102): 3862.36 26.232 24.25 53.99
=====
ID = 3 (0103): 5366.80 38.873 24.00 59.06

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (0104)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0005): 1444.47 12.846 19.00 53.68
+ ID2= 2 (0103): 5366.80 38.873 24.00 59.06
=====
ID = 3 (0104): 6811.27 49.757 22.75 57.92

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (0105)
IN= 2--> OUT= 1
Routing time step (min)'= 15.00

```

```

<----- DATA FOR SECTION ( 1.1) ----->
Distance Elevation Manning
0.00 237.00 0.1000
330.00 235.00 0.1000 /0.0350 Main Channel
331.00 234.20 0.0350 Main Channel
332.00 234.20 0.0350 Main Channel
333.00 235.00 0.0350 /0.1000 Main Channel
425.00 235.10 0.1000
698.00 236.00 0.1000
1101.00 239.00 0.1000

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELIV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.13 234.33 490E+03 0.1 0.64 82.66
0.27 234.47 112E+04 0.3 0.93 56.72
0.40 234.60 189E+04 0.7 1.14 45.94
0.53 234.73 280E+04 1.2 1.33 39.64
0.67 234.87 385E+04 1.8 1.49 35.34
0.80 235.00 504E+04 2.6 1.63 32.16
0.95 235.15 642E+04 3.3 0.45 116.42
1.11 235.31 128E+06 17.7 0.44 120.51
1.26 235.46 247E+06 38.2 0.49 107.48
1.42 235.62 400E+06 69.8 0.55 95.60
1.57 235.77 509E+06 114.0 0.61 86.08
1.72 235.92 812E+06 172.6 0.67 78.45
1.88 236.08 107E+07 250.3 0.74 71.19
2.03 236.23 135E+07 347.8 0.81 64.71
2.18 236.38 165E+07 461.9 0.88 59.65
2.34 236.54 198E+07 593.4 0.95 55.58
2.49 236.69 233E+07 742.8 1.01 52.21
2.65 236.85 270E+07 910.6 1.06 49.36
2.80 237.00 309E+07 1097.6 1.12 46.91

```

```

<----- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (0104) 6811.27 49.76 22.75 57.92 1.32 0.51
OUTFLOW: ID= 1 (0105) 6811.27 48.78 24.25 57.92 1.31 0.51

```

```

CALIB
NASHYD (0091) | Area (ha)= 281.97 Curve Number (CN)= 65.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
U.H. Tp(hrs)= 5.80

Unit Hyd Qpeak (cms)= 1.906

PEAK FLOW (cms)= 3.616 (i)
TIME TO PEAK (hrs)= 16.500
RUNOFF VOLUME (mm)= 60.331
TOTAL RAINFALL (mm)= 130.880
RUNOFF COEFFICIENT = 0.461

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (0106)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0091): 281.97 3.616 16.50 60.33
+ ID2= 2 (0105): 6811.27 48.777 24.25 57.92
=====
ID = 3 (0106): 7093.24 50.553 24.00 58.02

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (0107)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0007): 1373.84 12.958 21.75 72.08
+ ID2= 2 (0106): 7093.24 50.553 24.00 58.02
=====
ID = 3 (0107): 8467.08 63.176 23.50 60.30

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (0108)
IN= 2--> OUT= 1
Routing time step (min)'= 15.00

```

```

<----- DATA FOR SECTION ( 1.1) ----->
Distance Elevation Manning
0.00 234.00 0.1000
400.00 231.00 0.1000 /0.0350 Main Channel
401.00 229.70 0.0350 Main Channel
402.00 229.70 0.0350 Main Channel
404.00 230.50 0.0350 /0.1000 Main Channel
495.00 231.00 0.1000
1234.00 256.00 0.1000

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELIV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.20 229.90 151E+04 0.1 0.24 395.61
0.40 230.10 377E+04 0.2 0.35 274.62
0.60 230.30 677E+04 0.5 0.43 222.06
0.80 230.50 105E+05 0.9 0.50 190.52
1.03 230.73 437E+05 2.0 0.27 356.49
1.27 230.97 134E+06 5.0 0.21 448.69
1.50 231.20 278E+06 11.4 0.23 405.59
1.73 231.43 473E+06 21.6 0.26 364.80
1.97 231.67 718E+06 36.0 0.29 332.76
2.20 231.90 101E+07 55.1 0.31 306.87
2.43 232.13 136E+07 79.4 0.33 285.44
2.67 232.37 176E+07 109.6 0.36 267.34
2.90 232.60 221E+07 145.9 0.38 251.83
3.13 232.83 270E+07 189.0 0.40 238.35
3.37 233.07 325E+07 239.3 0.42 226.51
3.60 233.30 385E+07 297.1 0.44 216.02
3.83 233.53 450E+07 363.0 0.46 206.64
4.07 233.77 520E+07 437.3 0.48 198.20
4.30 234.00 595E+07 520.5 0.50 190.55

```

```

<----- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (0107) 8467.08 63.18 23.50 60.30 2.28 0.32
OUTFLOW: ID= 1 (0108) 8467.08 54.64 28.00 60.30 2.19 0.31

```

```

CALIB
NASHYD (0092) | Area (ha)=1102.32 Curve Number (CN)= 64.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
U.H. Tp(hrs)= 16.50

```

Unit Hyd Qpeak (cms)= 2.620

```

PEAK FLOW (cms)= 5.802 (i)
TIME TO PEAK (hrs)= 28.000
RUNOFF VOLUME (mm)= 58.960
TOTAL RAINFALL (mm)= 130.880
RUNOFF COEFFICIENT = 0.450

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (0109)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0092): 1102.32 5.802 28.00 58.96

```


+ ID2= 2 (0108): 8467.08 54.645 28.00 60.30
 ID = 3 (0109): 9569.40 60.447 28.00 60.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (0110) |
| 1 + 2 = 3 |
|-----|
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
+ ID1= 1 (0010): 2842.76 21.693 22.00 58.96
+ ID2= 2 (0109): 9569.40 60.447 28.00 60.14
=====
ID = 3 (0110): 12412.16 78.997 26.75 59.87
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (0111) |
| IN= 2--> OUT= 1 |
|-----|
| Routing time step (min)'= 15.00 |
  
```

```

<----- DATA FOR SECTION ( 1.1) ----->
| Distance | Elevation | Manning |
| 0.00 | 236.00 | 0.1000 |
| 275.00 | 232.00 | 0.1000 |
| 1039.00 | 230.00 | 0.1000 / 0.0350 | Main Channel
| 1040.00 | 229.20 | 0.0350 | Main Channel
| 1041.00 | 229.20 | 0.0350 | Main Channel
| 1043.00 | 230.00 | 0.0350 / 0.1000 | Main Channel
| 1117.00 | 230.60 | 0.1000 |
| 1325.00 | 234.00 | 0.1000 |
  
```

```

<----- TRAVEL TIME TABLE ----->
| DEPTH | ELEV | VOLUME | FLOW RATE | VELOCITY | TRAV.TIME |
| (m) | (m) | (cu.m.) | (cms) | (m/s) | (min) |
0.20 229.40 .985E+03 0.0 0.14 431.34
0.40 229.60 .251E+04 0.1 0.20 298.03
0.60 229.80 .457E+04 0.3 0.25 239.97
0.80 230.00 .717E+04 0.6 0.29 205.17
1.07 230.27 .754E+05 2.2 0.11 561.90
1.33 230.53 .272E+06 8.6 0.11 527.12
1.60 230.80 .594E+06 22.7 0.14 436.37
1.87 231.07 .103E+07 46.2 0.16 371.52
2.13 231.33 .158E+07 80.7 0.18 325.64
2.40 231.60 .224E+07 127.8 0.20 291.74
2.67 231.87 .301E+07 189.0 0.22 265.46
2.93 232.13 .389E+07 271.8 0.25 238.27
3.20 232.40 .481E+07 376.4 0.28 212.79
3.47 232.67 .576E+07 496.0 0.31 193.49
3.73 232.93 .674E+07 630.4 0.33 178.30
4.00 233.20 .775E+07 779.4 0.36 165.99
4.27 233.47 .881E+07 943.0 0.38 155.79
4.53 233.73 .990E+07 1121.1 0.41 147.16
4.80 234.00 .110E+08 1313.9 0.43 139.76
  
```

```

<---- hydrograph ----> <-pipe / channel->
| AREA | QPEAK | TPEAK | R.V. | MAX DEPTH | MAX VEL |
| (ha) | (cms) | (hrs) | (mm) | (m) | (m/s) |
INFLOW : ID= 2 (0110) ***** 79.00 26.75 59.87 2.12 0.18
OUTFLOW : ID= 1 (0111) ***** 67.66 30.75 59.87 2.03 0.17
  
```

```

-----
| ADD HYD (0112) |
| 1 + 2 = 3 |
|-----|
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
+ ID1= 1 (0093): 381.73 2.468 26.25 66.03
+ ID2= 2 (0111): 12412.16 67.663 30.75 59.87
=====
ID = 3 (0112): 12793.89 69.961 30.75 60.06
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0008) | Area (ha)=1549.97 Curve Number (CN)= 60.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
| U.H. Tp(hrs)= 9.10 |
  
```

Unit Hyd Qpeak (cms)= 6.679

```

PEAK FLOW (cms)= 12.417 (i)
TIME TO PEAK (hrs)= 20.250
RUNOFF VOLUME (mm)= 53.676
TOTAL RAINFALL (mm)= 130.880
RUNOFF COEFFICIENT = 0.410
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0003) | Area (ha)=2954.54 Curve Number (CN)= 64.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
| U.H. Tp(hrs)= 13.10 |
  
```

Unit Hyd Qpeak (cms)= 8.844

```

PEAK FLOW (cms)= 19.085 (i)
TIME TO PEAK (hrs)= 24.500
RUNOFF VOLUME (mm)= 58.960
TOTAL RAINFALL (mm)= 130.880
RUNOFF COEFFICIENT = 0.450
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0004) | Area (ha)=2672.69 Curve Number (CN)= 53.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
| U.H. Tp(hrs)= 10.70 |
  
```

Unit Hyd Qpeak (cms)= 9.795

```

PEAK FLOW (cms)= 15.685 (i)
TIME TO PEAK (hrs)= 22.000
RUNOFF VOLUME (mm)= 45.129
TOTAL RAINFALL (mm)= 130.880
RUNOFF COEFFICIENT = 0.345
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (0201) |
| 1 2 3 |
|-----|
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
+ ID1= 1 (0003): 2954.54 19.085 24.50 58.96
+ ID2= 2 (0004): 2672.69 15.685 22.00 45.13
=====
ID = 3 (0201): 5627.23 34.465 23.25 52.39
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (0202) |
| IN= 2--> OUT= 1 |
|-----|
| Routing time step (min)'= 15.00 |
  
```

```

<----- DATA FOR SECTION ( 1.1) ----->
| Distance | Elevation | Manning |
| 0.00 | 239.00 | 0.1000 |
| 690.00 | 230.00 | 0.1000 / 0.0350 | Main Channel
| 691.00 | 229.20 | 0.0350 | Main Channel
| 692.00 | 229.20 | 0.0350 | Main Channel
| 693.00 | 229.50 | 0.0350 / 0.1000 | Main Channel
| 1190.00 | 230.00 | 0.1000 |
| 1417.00 | 255.00 | 0.1000 |
  
```

```

<----- TRAVEL TIME TABLE ----->
| DEPTH | ELEV | VOLUME | FLOW RATE | VELOCITY | TRAV.TIME |
| (m) | (m) | (cu.m.) | (cms) | (m/s) | (min) |
0.30 229.50 .347E+04 0.4 0.84 136.57
0.80 230.00 .864E+06 45.2 0.36 318.78
1.30 230.50 .265E+07 274.1 0.71 161.20
1.80 231.00 .459E+07 651.8 0.97 117.24
2.30 231.50 .667E+07 1165.3 1.20 95.34
2.80 232.00 .889E+07 1810.2 1.40 81.89
3.30 232.50 .113E+08 2585.1 1.57 72.65
3.80 233.00 .138E+08 3490.4 1.73 65.95
4.30 233.50 .165E+08 4527.4 1.89 60.59
4.80 234.00 .193E+08 5697.8 2.03 56.38
5.30 234.50 .222E+08 7003.8 2.16 52.91
5.80 235.00 .253E+08 8447.7 2.28 50.00
6.30 235.50 .286E+08 10032.3 2.40 47.52
6.80 236.00 .320E+08 11760.1 2.52 45.36
7.30 236.50 .356E+08 13634.1 2.63 43.46
7.80 237.00 .393E+08 15657.0 2.73 41.78
8.30 237.50 .431E+08 17831.7 2.84 40.28
8.80 238.00 .471E+08 20161.3 2.93 38.92
9.30 238.50 .512E+08 22648.7 3.03 37.69
  
```

```

<---- hydrograph ----> <-pipe / channel->
| AREA | QPEAK | TPEAK | R.V. | MAX DEPTH | MAX VEL |
| (ha) | (cms) | (hrs) | (mm) | (m) | (m/s) |
INFLOW : ID= 2 (0201) 5627.23 34.47 23.25 52.39 0.68 0.42
OUTFLOW : ID= 1 (0202) 5627.23 30.95 28.75 52.39 0.64 0.44
  
```

```

ADD HYD (0303)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0008): 1549.97 12.417 20.25 53.68
+ ID2= 2 (0202): 5627.23 30.950 28.75 52.39
=====
ID = 3 (0203): 7177.20 39.511 26.25 52.67

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (0301)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0112): 13795.89 69.961 30.75 60.06
+ ID2= 2 (0203): 7177.20 39.511 26.25 52.67
=====
ID = 3 (0301): 19971.09 107.663 29.25 57.40

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (0302)
IN= 2--> OUT= 1
Routing time step (min)'= 15.00

```

```

----- DATA FOR SECTION ( 1.1) -----
Distance Elevation Manning
0.00 240.00 0.0700
110.00 239.00 0.0700
161.00 238.00 0.0700
250.00 237.00 0.0700
420.00 236.00 0.0700
560.00 235.00 0.0700
589.00 234.40 0.0700 /0.0350 Main Channel
589.10 233.60 0.0350 Main Channel
593.60 233.60 0.0350 Main Channel
594.10 234.40 0.0350 /0.0700 Main Channel
615.00 235.00 0.0700
740.00 236.00 0.0700
860.00 236.50 0.0700
1340.00 237.00 0.0700
1460.00 238.00 0.0700
1470.00 239.00 0.0700
1510.00 240.00 0.0700

```

```

----- TRAVEL TIME TABLE -----
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.27 233.87 .770E+04 1.0 0.74 132.49
0.53 234.13 .155E+05 2.9 1.10 88.48
0.80 234.40 .234E+05 5.5 1.37 71.19
1.15 234.75 .636E+05 11.6 1.07 91.11
1.50 235.10 .169E+06 24.5 0.85 114.97
1.85 235.45 .431E+06 54.5 0.74 131.62
2.20 235.80 .802E+06 117.3 0.78 125.39
2.55 236.15 .153E+07 218.3 0.83 117.06
2.90 236.50 .246E+07 375.3 0.89 109.30
3.25 236.85 .394E+07 579.0 0.86 113.45
3.60 237.20 .612E+07 1010.3 0.97 101.01
3.95 237.55 .851E+07 1631.3 1.12 86.99
4.30 237.90 .113E+08 2391.8 1.27 77.05
4.65 238.25 .137E+08 3337.5 1.42 68.52
5.00 238.60 .164E+08 4438.7 1.58 61.71
5.35 238.95 .192E+08 5671.0 1.73 56.40
5.70 239.30 .220E+08 6955.1 1.85 52.75
6.05 239.65 .249E+08 8355.2 1.96 49.76
6.40 240.00 .280E+08 9884.7 2.07 47.18

```

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (0301) ***** 107.66 29.25 57.40 2.15 0.77
OUTFLOW: ID= 1 (0302) ***** 105.59 31.50 57.40 2.13 0.77

```

```

ADD HYD (0303)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0012): 1501.52 10.114 22.75 54.97
+ ID2= 2 (0302): 19971.09 105.590 31.50 57.40
=====
ID = 3 (0303): 21472.61 112.839 31.00 57.23

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB
NASHYD (0011) | Area (ha)=3099.17 Curve Number (CN)= 57.0

```

```

ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
U.H. Tp(hrs)= 11.10

```

Unit Hyd Qpeak (cms)= 10.949

```

PEAK FLOW (cms)= 19.517 (i)
TIME TO PEAK (hrs)= 22.500
RUNOFF VOLUME (mm)= 49.909
TOTAL RAINFALL (mm)= 130.880
RUNOFF COEFFICIENT = 0.381

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ROUTE CHN (0401)
IN= 2--> OUT= 1
Routing time step (min)'= 15.00

```

```

----- DATA FOR SECTION ( 1.1) -----
Distance Elevation Manning
0.00 236.00 0.0500
72.00 235.00 0.0500
492.00 230.00 0.0500
581.00 229.20 0.0500 /0.0350 Main Channel
582.00 228.40 0.0350 Main Channel
583.00 228.40 0.0350 Main Channel
584.00 229.20 0.0350 /0.0500 Main Channel
588.00 229.20 0.0500
607.00 230.00 0.0500
627.00 235.00 0.0500

```

```

----- TRAVEL TIME TABLE -----
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.27 228.67 .302E+04 0.1 0.21 666.69
0.53 228.93 .756E+04 0.3 0.30 465.92
0.80 229.20 .136E+05 0.6 0.37 377.99
1.15 229.56 .111E+06 3.0 0.23 616.66
1.52 229.92 .358E+06 11.5 0.27 519.91
1.89 230.29 .740E+06 30.1 0.35 410.09
2.25 230.65 .122E+07 59.5 0.41 342.49
2.61 231.01 .180E+07 100.6 0.47 298.58
2.97 231.37 .248E+07 154.6 0.53 267.22
3.34 231.74 .326E+07 222.9 0.58 243.40
3.70 232.10 .413E+07 306.6 0.63 224.51
4.06 232.46 .510E+07 406.9 0.68 209.05
4.42 232.82 .617E+07 524.8 0.72 196.10
4.79 233.19 .734E+07 661.4 0.77 185.06
5.15 233.55 .861E+07 817.9 0.81 175.49
5.51 233.91 .998E+07 995.2 0.85 167.10
5.87 234.27 .114E+08 1194.3 0.89 159.67
6.24 234.64 .130E+08 1416.3 0.93 153.03
6.60 235.00 .147E+08 1662.2 0.96 147.05

```

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (0011) 3099.17 19.52 22.50 49.91 1.68 0.30
OUTFLOW: ID= 1 (0401) 3099.17 14.77 29.00 49.91 1.59 0.28

```

```

CALIB
NASHYD (0014) | Area (ha)=2163.09 Curve Number (CN)= 69.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
U.H. Tp(hrs)= 25.10

```

Unit Hyd Qpeak (cms)= 3.379

```

PEAK FLOW (cms)= 8.614 (i)
TIME TO PEAK (hrs)= 36.500
RUNOFF VOLUME (mm)= 66.025
TOTAL RAINFALL (mm)= 130.880
RUNOFF COEFFICIENT = 0.504

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (0402)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0014): 2163.09 8.614 36.50 66.03
+ ID2= 2 (0401): 3099.17 14.771 29.00 49.91
=====
ID = 3 (0402): 5262.26 22.690 30.75 56.53

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (0501)

```

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0303):	21472.61	112.839	31.00	57.23
+ ID2= 2 (0402):	5262.26	22.690	30.75	56.53
ID = 3 (0501):	26734.87	135.523	31.00	57.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0503) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning
0.00	237.00	0.0700
425.00	235.00	0.0700
445.00	234.00	0.0700
480.00	233.50	0.0700
1535.00	233.50	0.0700 / 0.0350
1536.50	232.70	0.0350
1537.50	232.70	0.0350
1540.00	233.50	0.0350 / 0.0700
1605.00	233.20	0.0700
1610.00	233.10	0.0700
2135.00	233.10	0.0700
2440.00	233.50	0.0700
2505.00	233.50	0.0700
2530.00	234.00	0.0700
2535.00	235.00	0.0700
2540.00	236.00	0.0700
2605.00	238.00	0.0700
2645.00	241.00	0.0700

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (cu.m.)	VOLUME (cms)	FLOW RATE (m/s)	VELOCITY (min)	TRAV.TIME (min)
0.20	232.90	.385E+03	0.2	0.55	28.92
0.40	233.10	.103E+04	0.6	0.80	26.75
0.60	233.30	.159E+06	39.5	0.32	67.02
0.80	233.50	.366E+06	133.3	0.47	45.74
1.03	233.73	.977E+06	428.7	0.56	37.97
1.27	233.97	.160E+07	914.3	0.74	29.09
1.50	234.20	.222E+07	1556.9	0.90	23.78
1.73	234.43	.285E+07	2336.2	1.05	20.32
1.97	234.67	.348E+07	3240.2	1.20	17.88
2.20	234.90	.411E+07	4260.9	1.33	16.07
2.43	235.13	.474E+07	5359.1	1.45	14.75
2.67	235.37	.539E+07	6537.6	1.56	13.74
2.90	235.60	.605E+07	7819.4	1.66	12.90
3.13	235.83	.673E+07	9203.8	1.76	12.19
3.37	236.07	.743E+07	10683.6	1.85	11.59
3.60	236.30	.814E+07	12247.8	1.93	11.07
3.83	236.53	.887E+07	13912.8	2.01	10.62
4.07	236.77	.961E+07	15679.0	2.09	10.22
4.30	237.00	.104E+08	17546.8	2.17	9.85

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (0501) ***** 135.52 31.00 57.09 0.80 0.47
OUTFLOW: ID= 1 (0503) ***** 135.33 31.50 57.09 0.80 0.47

RESERVOIR (0510)

DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	27.8300	83.9600
	3.8000	14.0500	*****	101.7400
	11.0800	33.6500	*****	145.2700
	20.8900	61.8900	0.0000	0.0000

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (0503) ***** 135.326 31.50 57.09
OUTFLOW: ID= 1 (0510) ***** 135.318 31.75 57.09

PEAK FLOW REDUCTION [Qout/qin] (%) = 99.99
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha.m.) = 87.0832

ROUTE CHN (0513)

| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning
0.00	237.00	0.0700
425.00	235.00	0.0700
445.00	234.00	0.0700

480.00	233.50	0.0700
1535.00	233.50	0.0700 / 0.0350
1536.50	232.70	0.0350
1537.50	232.70	0.0350
1540.00	233.50	0.0350 / 0.0700
1605.00	233.20	0.0700
1610.00	233.10	0.0700
2135.00	233.10	0.0700
2440.00	233.50	0.0700
2505.00	233.50	0.0700
2530.00	234.00	0.0700
2535.00	235.00	0.0700
2540.00	236.00	0.0700
2605.00	238.00	0.0700
2645.00	241.00	0.0700

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (cu.m.)	VOLUME (cms)	FLOW RATE (m/s)	VELOCITY (min)	TRAV.TIME (min)
0.20	232.90	.291E+03	0.2	0.55	29.43
0.40	233.10	.777E+03	0.6	0.80	20.23
0.60	233.30	.120E+06	39.5	0.32	50.68
0.80	233.50	.277E+06	133.3	0.47	34.59
1.03	233.73	.739E+06	428.7	0.56	28.72
1.27	233.97	.121E+07	914.3	0.74	22.00
1.50	234.20	.168E+07	1556.9	0.90	17.98
1.73	234.43	.215E+07	2336.2	1.05	15.37
1.97	234.67	.262E+07	3240.2	1.20	13.52
2.20	234.90	.311E+07	4260.9	1.33	12.15
2.43	235.13	.359E+07	5359.1	1.45	11.15
2.67	235.37	.408E+07	6537.6	1.56	10.39
2.90	235.60	.458E+07	7819.4	1.66	9.76
3.13	235.83	.509E+07	9203.8	1.76	9.22
3.37	236.07	.562E+07	10683.6	1.85	8.76
3.60	236.30	.615E+07	12247.8	1.93	8.37
3.83	236.53	.670E+07	13912.8	2.01	8.03
4.07	236.77	.727E+07	15679.0	2.09	7.73
4.30	237.00	.784E+07	17546.8	2.17	7.45

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (0510) ***** 135.32 31.75 57.09 0.80 0.47
OUTFLOW: ID= 1 (0513) ***** 135.19 32.25 57.09 0.80 0.47

ADD HYD (0504)

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0015):	1059.88	11.257	20.25	72.08
+ ID2= 2 (0513):	26734.87	135.188	32.25	57.09
ID = 3 (0504):	27794.75	140.420	31.25	57.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0505)

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0013):	1627.59	9.275	27.00	60.33
+ ID2= 2 (0504):	27794.75	140.420	31.25	57.67
ID = 3 (0505):	29422.34	149.210	30.75	57.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0506)

| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning
0.00	251.00	0.0500
323.00	238.00	0.0500
400.00	229.00	0.0500
545.00	228.00	0.0500
602.00	226.00	0.0500
604.00	225.80	0.0500 / 0.0350
605.00	225.00	0.0350
609.00	225.00	0.0350
610.00	225.80	0.0350 / 0.0500
614.00	226.00	0.0500
650.00	227.00	0.0500
810.00	229.50	0.0500
1855.00	240.00	0.0500
2617.00	259.00	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.80	225.80	.292E+05	3.5	0.87	139.36
2.13	227.13	.441E+06	50.6	0.84	145.20
3.45	228.45	.197E+07	262.2	0.97	125.42
4.78	229.78	.557E+07	971.2	1.27	95.57
6.11	231.11	.106E+08	2326.5	1.61	75.63
7.43	232.43	.169E+08	4425.6	1.90	63.77
8.76	233.76	.247E+08	7361.8	2.17	55.90
10.08	235.08	.338E+08	11229.7	2.42	50.22
11.41	236.41	.444E+08	16121.0	2.65	45.87
12.74	237.74	.563E+08	22124.8	2.86	42.40
14.06	239.06	.697E+08	29005.2	3.03	40.02
15.39	240.39	.846E+08	37433.0	3.23	37.66
16.72	241.72	.101E+09	47908.6	3.47	34.97
18.04	243.04	.117E+09	59742.1	3.71	32.73
19.37	244.37	.135E+09	72949.0	3.94	30.83
20.69	245.69	.153E+09	87549.6	4.16	29.20
22.02	247.02	.173E+09	*****	4.37	27.79
23.35	248.35	.193E+09	*****	4.58	26.55
24.67	249.67	.214E+09	*****	4.77	25.45

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0512)	*****	146.17	33.75	57.81	2.88	0.95
OUTFLOW: ID= 1 (0514)	*****	145.71	34.25	57.81	2.87	0.94

```

-----
| ADD HYD (0507) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
-----
ID= 1 (0016): 3094.49 18.718 29.50 73.66
+ ID= 2 (0514): 29422.34 145.709 34.25 57.81
=====
ID = 3 (0507): 32516.83 163.388 34.00 59.32
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

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-----
<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (0505) ***** 149.21 30.75 57.81 2.74 0.89
OUTFLOW: ID= 1 (0506) ***** 146.12 33.50 57.81 2.72 0.89
-----

```

```

-----
| RESERVOIR (0512) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----

```

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	*****	15.2850
27.2000	3.7650	*****	20.9160
76.8000	7.7410	*****	25.9120
*****	11.9650	*****	36.6600
*****	11.9150	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0506)	*****	146.118	33.50	57.81
OUTFLOW: ID= 1 (0512)	*****	146.170	33.75	57.81

PEAK FLOW REDUCTION [Qout/Qin](%)=100.04
TIME SHIFT OF PEAK FLOW (min)= 15.00
MAXIMUM STORAGE USED (ha.m.)= 11.1190

**** WARNING : HYDROGRAPH PEAK WAS NOT REDUCED.
CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.

```

-----
| ROUTE CHN (0514) |
| IN= 2--> OUT= 1 |
-----

```

Routing time step (min)'= 15.00

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-----
<----- DATA FOR SECTION ( 1.1) ----->
Distance Elevation Manning
0.00 230.00 0.0500
400.00 229.00 0.0500
545.00 228.00 0.0500
602.00 226.00 0.0500
604.00 225.80 0.0500 /0.0350 Main Channel
605.00 225.00 0.0350 Main Channel
609.00 225.00 0.0350 Main Channel
610.00 225.80 0.0350 /0.0500 Main Channel
614.00 226.00 0.0500
650.00 227.00 0.0500
810.00 229.50 0.0500
-----

```

```

-----
<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.20 225.20 .243E+04 0.3 0.39 121.91
0.40 225.40 .515E+04 1.1 0.59 80.58
0.60 225.60 .815E+04 2.1 0.74 64.02
0.80 225.80 .114E+05 3.5 0.87 54.69
1.05 226.05 .184E+05 6.1 0.94 50.48
1.29 226.29 .246E+05 10.3 0.85 55.99
1.54 226.54 .320E+05 17.4 0.80 59.38
1.79 226.79 .401E+06 28.3 0.81 59.18
2.03 227.03 .491E+06 43.8 0.83 57.35
2.28 227.28 .595E+06 63.9 0.85 55.99
2.53 227.53 .715E+06 91.4 0.89 53.82
2.77 227.77 .841E+06 127.0 0.93 51.35
3.02 228.02 .984E+06 170.6 0.97 49.24
3.27 228.27 .114E+07 215.5 0.96 49.84
3.51 228.51 .131E+07 279.7 0.97 48.92
3.76 228.76 .150E+07 363.1 1.00 47.47
4.01 229.01 .172E+07 466.2 1.04 45.89
4.25 229.25 .198E+07 573.0 1.03 46.41
4.50 229.50 .198E+07 719.1 1.04 45.95
-----

```

<---- hydrograph ----> <-pipe / channel->

EXISTING
BLACK RIVER REGIONAL STORM

```
V V I SSSS U U A L
V V I SS U U AAA L
V V I SS U U A A L
V V I SS U U A A L
V V I SSSS UUUU A A LLLL
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OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y M M O O O
O O T T H H Y Y M M O O O Company
OOO T T H H Y Y M M OOO Serial
```

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***** DETAILED OUTPUT *****

Input filename: C:\Program Files\Visual Otthymo 2.4r\VO2\voindat
Output filename: C:\Users\jscott\AppData\Local\Temp\adf6531f-2fd9-4ba7-8cd9-5205a923b97a\Scenario.out
Summary filename: C:\Users\jscott\AppData\Local\Temp\adf6531f-2fd9-4ba7-8cd9-5205a923b97a\Scenario.sum

DATE: 08/22/2012 TIME: 11:59:27

USER:

COMMENTS: _____

** SIMULATION NUMBER: 1 **

READ STORM File: C:\Users\jscott\AppData\Local\Temp\adf6531f-2fd9-4ba7-8cd9-5205a923b97a\d2aa967d
Ptotal=212.00 mm Comments: * REGIONAL DESIGN STORM

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.20	6.00	3.20	13.00	6.20	23.00	9.20	53.00
0.40	6.00	3.40	13.00	6.40	23.00	9.40	53.00
0.60	6.00	3.60	13.00	6.60	23.00	9.60	53.00
0.80	6.00	3.80	13.00	6.80	23.00	9.80	53.00
1.00	6.00	4.00	13.00	7.00	23.00	10.00	53.00
1.20	4.00	4.20	17.00	7.20	13.00	10.20	38.00
1.40	4.00	4.40	17.00	7.40	13.00	10.40	38.00
1.60	4.00	4.60	17.00	7.60	13.00	10.60	38.00
1.80	4.00	4.80	17.00	7.80	13.00	10.80	38.00
2.00	4.00	5.00	17.00	8.00	13.00	11.00	38.00
2.20	6.00	5.20	13.00	8.20	13.00	11.20	13.00
2.40	6.00	5.40	13.00	8.40	13.00	11.40	13.00
2.60	6.00	5.60	13.00	8.60	13.00	11.60	13.00
2.80	6.00	5.80	13.00	8.80	13.00	11.80	13.00
3.00	6.00	6.00	13.00	9.00	13.00	12.00	13.00

MODIFY STORM
CASE= 1
MODIFYING PARAMETERS
Multiplication Factor= 0.99
Time shift (min) = 0.00

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.200	5.95	3.200	12.90	6.200	22.82	9.20	52.58
0.400	5.95	3.400	12.90	6.400	22.82	9.40	52.58
0.600	5.95	3.600	12.90	6.600	22.82	9.60	52.58
0.800	5.95	3.800	12.90	6.800	22.82	9.80	52.58
1.000	5.95	4.000	12.90	7.000	22.82	10.00	52.58
1.200	3.97	4.200	16.86	7.200	12.90	10.20	37.70
1.400	3.97	4.400	16.86	7.400	12.90	10.40	37.70
1.600	3.97	4.600	16.86	7.600	12.90	10.60	37.70
1.800	3.97	4.800	16.86	7.800	12.90	10.80	37.70
2.000	3.97	5.000	16.86	8.000	12.90	11.00	37.70
2.200	5.95	5.200	12.90	8.200	12.90	11.20	12.90
2.400	5.95	5.400	12.90	8.400	12.90	11.40	12.90
2.600	5.95	5.600	12.90	8.600	12.90	11.60	12.90
2.800	5.95	5.800	12.90	8.800	12.90	11.80	12.90
3.000	5.95	6.000	12.90	9.000	12.90	12.00	12.90

CALIB
NASHYD (0016) Area (ha)=3094.49 Curve Number (CN)= 88.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
U.H. Tp(hrs)= 18.10

NOTE: RAINFALL WAS TRANSFORMED TO 15.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.250	5.95	3.250	12.90	6.250	22.82	9.25	52.58
0.500	5.95	3.500	12.90	6.500	22.82	9.50	52.58
0.750	5.95	3.750	12.90	6.750	22.82	9.75	52.58
1.000	5.95	4.000	12.90	7.000	22.82	10.00	52.58
1.250	3.97	4.250	16.86	7.250	12.90	10.25	37.70
1.500	3.97	4.500	16.86	7.500	12.90	10.50	37.70
1.750	3.97	4.750	16.86	7.750	12.90	10.75	37.70
2.000	3.97	5.000	16.86	8.000	12.90	11.00	37.70
2.250	5.95	5.250	12.90	8.250	12.90	11.25	12.90
2.500	5.95	5.500	12.90	8.500	12.90	11.50	12.90
2.750	5.95	5.750	12.90	8.750	12.90	11.75	12.90
3.000	5.95	6.000	12.90	9.000	12.90	12.00	12.90

Unit Hyd Qpeak (cms)= 6.704

PEAK FLOW (cms)= 45.605 (i)
TIME TO PEAK (hrs)= 26.750
RUNOFF VOLUME (mm)= 175.668
TOTAL RAINFALL (mm)= 210.304
RUNOFF COEFFICIENT = 0.835

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0013) Area (ha)=1627.59 Curve Number (CN)= 82.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
U.H. Tp(hrs)= 15.50

Unit Hyd Qpeak (cms)= 4.118

PEAK FLOW (cms)= 25.641 (i)
TIME TO PEAK (hrs)= 24.250
RUNOFF VOLUME (mm)= 161.456
TOTAL RAINFALL (mm)= 210.304
RUNOFF COEFFICIENT = 0.768

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0015) Area (ha)=1059.88 Curve Number (CN)= 87.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
U.H. Tp(hrs)= 9.30

Unit Hyd Qpeak (cms)= 4.469

PEAK FLOW (cms)= 28.804 (i)
TIME TO PEAK (hrs)= 18.250
RUNOFF VOLUME (mm)= 173.272
TOTAL RAINFALL (mm)= 210.304
RUNOFF COEFFICIENT = 0.824

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0012) Area (ha)=1501.52 Curve Number (CN)= 78.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
U.H. Tp(hrs)= 11.50

Unit Hyd Qpeak (cms)= 5.120

PEAK FLOW (cms)= 29.637 (i)
TIME TO PEAK (hrs)= 20.500
RUNOFF VOLUME (mm)= 152.196
TOTAL RAINFALL (mm)= 210.304
RUNOFF COEFFICIENT = 0.724

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0093) Area (ha)= 381.73 Curve Number (CN)= 84.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
U.H. Tp(hrs)= 14.90

Unit Hyd Qpeak (cms)= 1.005

PEAK FLOW (cms)= 6.424 (1)
 TIME TO PEAK (hrs)= 23.500
 RUNOFF VOLUME (mm)= 166.150
 TOTAL RAINFALL (mm)= 210.304
 RUNOFF COEFFICIENT = 0.790

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0010) | Area (ha)=2842.76 Curve Number (CN)= 81.0
 NASHVD (0002) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 10.80

Unit Hyd Qpeak (cms)= 10.322

PEAK FLOW (cms)= 62.126 (1)
 TIME TO PEAK (hrs)= 19.750
 RUNOFF VOLUME (mm)= 159.125
 TOTAL RAINFALL (mm)= 210.304
 RUNOFF COEFFICIENT = 0.757

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0007) | Area (ha)=1373.84 Curve Number (CN)= 87.0
 NASHVD (0007) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 10.70

Unit Hyd Qpeak (cms)= 5.035

PEAK FLOW (cms)= 32.872 (1)
 TIME TO PEAK (hrs)= 19.500
 RUNOFF VOLUME (mm)= 173.272
 TOTAL RAINFALL (mm)= 210.304
 RUNOFF COEFFICIENT = 0.824

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0005) | Area (ha)=1444.47 Curve Number (CN)= 78.0
 NASHVD (0005) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 8.00

Unit Hyd Qpeak (cms)= 7.080

PEAK FLOW (cms)= 39.691 (1)
 TIME TO PEAK (hrs)= 17.000
 RUNOFF VOLUME (mm)= 152.196
 TOTAL RAINFALL (mm)= 210.304
 RUNOFF COEFFICIENT = 0.724

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0006) | Area (ha)=1504.44 Curve Number (CN)= 87.0
 NASHVD (0006) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 12.20

Unit Hyd Qpeak (cms)= 4.836

PEAK FLOW (cms)= 31.870 (1)
 TIME TO PEAK (hrs)= 21.000
 RUNOFF VOLUME (mm)= 173.272
 TOTAL RAINFALL (mm)= 210.304
 RUNOFF COEFFICIENT = 0.824

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0001) | Area (ha)=2873.64 Curve Number (CN)= 78.0
 NASHVD (0001) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 11.00

Unit Hyd Qpeak (cms)= 10.244

PEAK FLOW (cms)= 59.124 (1)
 TIME TO PEAK (hrs)= 20.000
 RUNOFF VOLUME (mm)= 152.196
 TOTAL RAINFALL (mm)= 210.304
 RUNOFF COEFFICIENT = 0.724

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0002) | Area (ha)= 988.72 Curve Number (CN)= 76.0
 NASHVD (0002) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 9.10

Unit Hyd Qpeak (cms)= 4.261

PEAK FLOW (cms)= 23.509 (1)
 TIME TO PEAK (hrs)= 18.250
 RUNOFF VOLUME (mm)= 147.628
 TOTAL RAINFALL (mm)= 210.304
 RUNOFF COEFFICIENT = 0.702

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0101) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0001): 2873.64 59.124 20.00 152.20
 + ID2= 2 (0002): 988.72 23.509 18.25 147.63
 ID = 3 (0101): 3862.36 82.078 19.25 151.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0102) | IN= 2 -> OUT= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning	
0.00	250.00	0.0700	
43.00	240.00	0.0700 /0.0350	Main Channel
44.00	239.20	0.0350	Main Channel
45.00	239.20	0.0350	Main Channel
46.00	240.00	0.0350 /0.0700	Main Channel
63.50	241.00	0.0700	
71.00	250.00	0.0700	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.40	239.60	.642E+04	0.5	0.80	231.73
0.80	240.00	.171E+05	1.8	1.15	155.20
1.39	240.59	.764E+05	8.0	1.12	159.62
1.98	241.18	.214E+06	23.7	1.19	150.23
2.56	241.76	.385E+06	51.8	1.44	123.91
3.15	242.35	.575E+06	90.1	1.68	106.38
3.74	242.94	.784E+06	138.5	1.89	94.40
4.33	243.53	.101E+07	196.9	2.08	85.71
4.92	244.12	.126E+07	265.6	2.26	79.08
5.51	244.71	.153E+07	344.7	2.42	73.81
6.09	245.29	.181E+07	434.5	2.57	69.50
6.68	245.88	.212E+07	535.2	2.71	65.99
7.27	246.47	.244E+07	647.3	2.84	62.81
7.86	247.06	.278E+07	771.1	2.97	60.13
8.45	247.65	.314E+07	906.8	3.09	57.77
9.04	248.24	.352E+07	1054.8	3.20	55.67
9.62	248.82	.392E+07	1215.5	3.32	53.79
10.21	249.41	.434E+07	1389.1	3.42	52.09
10.80	250.00	.478E+07	1576.1	3.53	50.53

<---- hydrograph ----> <-pipe / channel-->

INFLOW: ID= 2 (0101)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
ID= 1 (0102)	3862.36	82.08	19.25	151.03	3.03	1.62
OUTFLOW: ID= 1 (0102)	3862.36	79.95	20.75	151.03	2.99	1.60

ADD HYD (0103) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0005): 1504.44 31.870 21.00 173.27
 + ID2= 2 (0102): 3862.36 79.949 20.75 151.03
 ID = 3 (0103): 5366.80 111.814 20.75 157.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0104) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0005): 1444.47 39.691 17.00 152.20
 + ID2= 2 (0103): 5366.80 111.814 20.75 157.26

ID = 3 (0104): 6811.27 146.613 19.75 156.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0105) | IN= 2 -> OUT= 1 | Routing time step (min)'= 15.00

Table with 4 columns: Distance, Elevation, Manning, and Channel Name. Rows show data for various distances from 0.00 to 1101.00.

TRAVEL TIME TABLE with 6 columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV.TIME. Rows show data for depths from 0.13 to 2.80.

hydrograph table with 6 columns: AREA, QPEAK, TPEAK, R.V., MAX DEPTH, MAX VEL. Rows show inflow and outflow data for ID=2 and ID=1.

CALIB | NASHVD (0091) | Area (ha)= 281.97 Curve Number (CN)= 62.0 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10 | U.H. Tp(hrs)= 5.80

Unit Hyd Qpeak (cms)= 1.906 | PEAK FLOW (cms)= 10.743 (i) | TIME TO PEAK (hrs)= 15.000 | RUNOFF VOLUME (mm)= 161.456 | TOTAL RAINFALL (mm)= 210.304 | RUNOFF COEFFICIENT = 0.768

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0106) | 1 + 2 = 3 | AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm) | ID1= 1 (0091): 281.97 10.743 15.00 161.46 | + ID2= 2 (0105): 6811.27 144.301 21.00 156.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0107) | 1 + 2 = 3 | AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm) | ID1= 1 (0007): 1373.84 32.872 19.50 173.27 | + ID2= 2 (0106): 7093.24 150.071 20.50 156.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0108) | IN= 2 -> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (1.1) table with 4 columns: Distance, Elevation, Manning, and Channel Name. Rows show data for various distances from 0.00 to 1234.00.

TRAVEL TIME TABLE with 6 columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV.TIME. Rows show data for depths from 0.20 to 4.30.

hydrograph table with 6 columns: AREA, QPEAK, TPEAK, R.V., MAX DEPTH, MAX VEL. Rows show inflow and outflow data for ID=2 and ID=1.

CALIB | NASHVD (0092) | Area (ha)=1102.32 Curve Number (CN)= 81.0 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10 | U.H. Tp(hrs)= 16.50

Unit Hyd Qpeak (cms)= 2.620 | PEAK FLOW (cms)= 16.118 (i) | TIME TO PEAK (hrs)= 25.250 | RUNOFF VOLUME (mm)= 159.125 | TOTAL RAINFALL (mm)= 210.304 | RUNOFF COEFFICIENT = 0.757

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0109) | 1 + 2 = 3 | AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm) | ID1= 1 (0092): 1102.32 16.118 25.25 159.13 | + ID2= 2 (0108): 8467.08 159.933 24.00 159.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0110) | 1 + 2 = 3 | AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm) | ID1= 1 (0010): 2842.76 62.116 19.75 159.13 | + ID2= 2 (0109): 9569.40 175.954 24.00 159.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0111) | IN= 2 -> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (1.1) table with 4 columns: Distance, Elevation, Manning, and Channel Name. Rows show data for various distances from 0.00 to 275.00.

1039.00 230.00 0.1000 / 0.0350 Main Channel
 1040.00 229.20 0.0350 Main Channel
 1041.00 229.20 0.0350 Main Channel
 1043.00 230.00 0.0350 / 0.1000 Main Channel
 1117.00 230.60 0.1000
 1325.00 234.00 0.1000

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.20	229.40	.985E+03	0.0	0.14	431.34
0.40	229.60	.251E+04	0.1	0.20	298.03
0.60	229.80	.457E+04	0.3	0.25	239.97
0.80	230.00	.717E+04	0.6	0.29	205.17
1.07	230.27	.754E+05	2.2	0.11	561.90
1.33	230.53	.272E+06	8.6	0.11	527.12
1.60	230.80	.594E+06	22.7	0.14	436.37
1.87	231.07	.103E+07	46.2	0.16	371.32
2.13	231.33	.158E+07	80.7	0.18	325.64
2.40	231.60	.224E+07	127.8	0.20	291.74
2.67	231.87	.301E+07	189.0	0.22	265.46
2.93	232.13	.389E+07	271.8	0.25	238.27
3.20	232.40	.481E+07	376.4	0.28	212.79
3.47	232.67	.576E+07	496.0	0.31	193.49
3.73	232.93	.674E+07	630.4	0.33	178.30
4.00	233.20	.776E+07	779.4	0.36	165.99
4.27	233.47	.881E+07	943.0	0.38	155.79
4.53	233.73	.990E+07	1121.1	0.41	147.16
4.80	234.00	.110E+08	1313.9	0.43	139.76

hydrograph <---> <--- pipe / channel --->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0110) ***** 231.78 23.00 159.13 2.80 0.24
 OUTFLOW : ID= 1 (0111) ***** 203.20 26.50 159.13 2.71 0.23

ADD HYD (0112) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0093): 381.73 6.424 23.50 166.15
 + ID2= 2 (0111): 12412.16 203.203 26.50 159.13
 ID = 3 (0112): 12793.89 209.409 26.50 159.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0008) | Area (ha)=1549.97 Curve Number (CN)= 78.0
 NASHVD (0003) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 9.10

Unit Hyd Qpeak (cms)= 6.679

PEAK FLOW (cms)= 37.954 (i)
 TIME TO PEAK (hrs)= 18.000
 RUNOFF VOLUME (mm)= 152.196
 TOTAL RAINFALL (mm)= 210.304
 RUNOFF COEFFICIENT = 0.724

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0003) | Area (ha)=2954.54 Curve Number (CN)= 81.0
 NASHVD (0003) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 13.10

Unit Hyd Qpeak (cms)= 8.844

PEAK FLOW (cms)= 53.877 (i)
 TIME TO PEAK (hrs)= 22.000
 RUNOFF VOLUME (mm)= 159.125
 TOTAL RAINFALL (mm)= 210.304
 RUNOFF COEFFICIENT = 0.757

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0004) | Area (ha)=2672.69 Curve Number (CN)= 72.0
 NASHVD (0004) | Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 10.70

Unit Hyd Qpeak (cms)= 9.795

PEAK FLOW (cms)= 51.500 (i)
 TIME TO PEAK (hrs)= 19.750

RUNOFF VOLUME (mm)= 138.613
 TOTAL RAINFALL (mm)= 210.304
 RUNOFF COEFFICIENT = 0.659

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0201) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0003): 2954.54 53.877 22.00 159.13
 + ID2= 2 (0004): 2672.69 51.500 19.75 138.61
 ID = 3 (0201): 5627.23 104.474 20.75 149.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0202) | Routing time step (min)'= 15.00
 IN= 2--> OUT= 1

DATA FOR SECTION (1.1) -----
 Distance Elevation Manning
 0.00 239.00 0.1000
 690.00 230.00 0.1000 / 0.0350 Main Channel
 691.00 229.20 0.0350 Main Channel
 692.00 229.20 0.0350 Main Channel
 693.00 229.50 0.0350 / 0.1000 Main Channel
 1190.00 230.00 0.1000
 1417.00 255.00 0.1000

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.30	229.50	.347E+04	0.4	0.84	136.57
0.80	230.00	.864E+06	45.2	0.36	318.78
1.30	230.50	.265E+07	274.1	0.71	161.20
1.80	231.00	.459E+07	651.8	0.97	117.24
2.30	231.50	.667E+07	1165.3	1.20	95.34
2.80	232.00	.899E+07	1810.2	1.40	81.89
3.30	232.50	.113E+08	2585.1	1.57	72.65
3.80	233.00	.138E+08	3490.4	1.73	65.85
4.30	233.50	.165E+08	4527.4	1.89	60.59
4.80	234.00	.193E+08	5697.8	2.03	56.38
5.30	234.50	.222E+08	7003.8	2.16	52.91
5.80	235.00	.253E+08	8447.7	2.28	50.00
6.30	235.50	.286E+08	10032.3	2.40	47.52
6.80	236.00	.320E+08	11760.1	2.52	45.36
7.30	236.50	.356E+08	13634.1	2.63	43.46
7.80	237.00	.393E+08	15657.0	2.73	41.78
8.30	237.50	.431E+08	17831.7	2.84	40.28
8.80	238.00	.471E+08	20161.3	2.93	38.92
9.30	238.50	.512E+08	22648.7	3.03	37.69

hydrograph <---> <--- pipe / channel --->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0201) 5627.23 104.47 20.75 149.38 0.93 0.41
 OUTFLOW : ID= 1 (0202) 5627.23 91.05 25.25 149.38 0.90 0.40

ADD HYD (0203) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0008): 1549.97 37.954 18.00 152.20
 + ID2= 2 (0202): 5627.23 91.050 25.25 149.38
 ID = 3 (0203): 7177.20 118.815 23.25 149.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0301) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0112): 12793.89 209.409 26.50 159.34
 + ID2= 2 (0203): 7177.20 118.815 23.25 149.99
 ID = 3 (0301): 19971.09 323.828 25.25 155.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0302) | Routing time step (min)'= 15.00
 IN= 2--> OUT= 1

----- DATA FOR SECTION (1.1) -----

Table with 3 columns: Distance, Elevation, Manning. Rows show data points from 0.00 to 1510.00.

607.00 230.00 0.0500
627.00 235.00 0.0500

----- TRAVEL TIME TABLE -----

Table with 6 columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV.TIME. Rows show hydrograph data points.

----- TRAVEL TIME TABLE -----

Table with 6 columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV.TIME. Rows show hydrograph data points.

----- hydrograph ----- <-pipe / channel-->

AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
INFLOW : ID= 2 (0011) 3099.17 60.45 20.00 145.36 2.26 0.41
OUTFLOW: ID= 1 (0401) 3099.17 49.03 24.50 145.36 2.12 0.39

CALIB NASHYD (0014) Area (ha)=2163.09 Curve Number (CN)= 84.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
U.N. Tp(hrs)= 25.10

Unit Hyd Qpeak (cms) = 3.379

PEAK FLOW (cms) = 21.922 (i)
TIME TO PEAK (hrs) = 33.750
RUNOFF VOLUME (mm) = 166.150
TOTAL RAINFALL (mm) = 210.304
RUNOFF COEFFICIENT = 0.790

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- hydrograph ----- <-pipe / channel-->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
INFLOW : ID= 2 (0301) ***** 323.83 25.25 155.98 2.79 0.87
OUTFLOW: ID= 1 (0302) ***** 317.46 27.00 155.98 2.77 0.87

ADD HYD (0303)
1 + 2 = 3
ID= 1 (0012): 1501.52 29.637 20.50 152.20
+ ID2= 2 (0302): 19971.09 317.457 27.00 155.98
ID = 3 (0303): 21472.61 340.661 26.75 155.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0011) Area (ha)=3099.17 Curve Number (CN)= 75.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 3.10
U.N. Tp(hrs)= 11.10

Unit Hyd Qpeak (cms) = 10.949

PEAK FLOW (cms) = 60.453 (i)
TIME TO PEAK (hrs) = 20.000
RUNOFF VOLUME (mm) = 145.359
TOTAL RAINFALL (mm) = 210.304
RUNOFF COEFFICIENT = 0.691

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (0401)
IR= 2--> OUT= 1
Routing time step (min)'= 15.00

----- DATA FOR SECTION (1.1) -----

Table with 3 columns: Distance, Elevation, Manning. Rows show data points from 0.00 to 588.00.

ROUTE CHN (0503)
IR= 2--> OUT= 1
Routing time step (min)'= 15.00

----- DATA FOR SECTION (1.1) -----

Table with 3 columns: Distance, Elevation, Manning. Rows show data points from 0.00 to 2440.00.

2505.00	233.50	0.0700
2530.00	234.00	0.0700
2535.00	235.00	0.0700
2540.00	236.00	0.0700
2605.00	238.00	0.0700
2645.00	241.00	0.0700

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----- TRAVEL TIME TABLE -----
DEPTH  ELEV  VOLUME  FLOW RATE  VELOCITY  TRAV.TIME
(m)      (m)  (cu.m.)  (cms)      (m/s)     (min)
0.20  232.90  .385E+03  0.2        0.55      38.92
0.40  233.10  1.03E+04  0.6        0.80      26.75
0.60  233.30  1.59E+06  39.5       0.32      67.02
0.80  233.50  3.66E+06  133.3      0.47      45.74
1.03  233.73  .977E+06  428.7     0.56      37.97
1.27  233.97  1.60E+07  914.3     0.74      29.09
1.50  234.20  2.22E+07  1556.9    0.90      23.78
1.73  234.43  2.85E+07  2336.2    1.05      20.32
1.97  234.67  3.48E+07  3240.2    1.20      17.88
2.20  234.90  4.11E+07  4260.9    1.33      16.07
2.43  235.13  4.74E+07  5350.1    1.45      14.75
2.67  235.37  5.39E+07  6537.6    1.56      13.74
2.90  235.60  6.05E+07  7819.4    1.66      12.90
3.13  235.83  6.73E+07  9203.8    1.76      12.19
3.37  236.07  7.43E+07  10683.6   1.85      11.59
3.60  236.30  8.14E+07  12247.8   1.93      11.07
3.83  236.53  8.87E+07  13912.8   2.01      10.62
4.07  236.77  9.61E+07  15679.0   2.09      10.22
4.30  237.00  1.04E+08  17546.8   2.17      9.85

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----- hydrograph ----- <-pipe / channel->
AREA  QPEAK  TPEAK  R.V.  MAX DEPTH  MAX VEL
(ha)  (cms)  (hrs)  (mm)  (m)        (m/s)
INFLOW : ID= 2 (0501) ***** 408.40  26.75  155.36  1.02  0.56
OUTFLOW : ID= 1 (0503) ***** 407.94  27.00  155.36  1.02  0.56

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| ROUTE CHN (0513) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

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----- DATA FOR SECTION ( 1.1) -----
Distance  Elevation  Manning
0.00      237.00    0.0700
425.00    235.00    0.0700
445.00    234.00    0.0700
480.00    233.50    0.0700
1535.00   233.50    0.0700 /0.0350 Main Channel
1536.50   232.70    0.0350 Main Channel
1537.50   232.70    0.0350 Main Channel
1540.00   233.50    0.0350 /0.0700 Main Channel
1605.00   233.20    0.0700
1610.00   233.10    0.0700
2135.00   233.10    0.0700
2440.00   233.50    0.0700
2505.00   233.50    0.0700
2530.00   234.00    0.0700
2535.00   235.00    0.0700
2540.00   236.00    0.0700
2605.00   238.00    0.0700
2645.00   241.00    0.0700

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----- TRAVEL TIME TABLE -----
DEPTH  ELEV  VOLUME  FLOW RATE  VELOCITY  TRAV.TIME
(m)      (m)  (cu.m.)  (cms)      (m/s)     (min)
0.20  232.90  .291E+03  0.2        0.55      29.43
0.40  233.10  .777E+03  0.6        0.80      20.23
0.60  233.30  1.20E+06  39.5       0.32      50.68
0.80  233.50  2.77E+06  133.3      0.47      34.59
1.03  233.73  7.39E+06  428.7     0.56      28.72
1.27  233.97  1.21E+07  914.3     0.74      22.00
1.50  234.20  1.68E+07  1556.9    0.90      17.98
1.73  234.43  2.15E+07  2336.2    1.05      15.37
1.97  234.67  2.63E+07  3240.2    1.20      13.52
2.20  234.90  3.11E+07  4260.9    1.33      12.15
2.43  235.13  3.59E+07  5350.1    1.45      11.15
2.67  235.37  4.08E+07  6537.6    1.56      10.39
2.90  235.60  4.58E+07  7819.4    1.66      9.76
3.13  235.83  5.09E+07  9203.8    1.76      9.22
3.37  236.07  5.62E+07  10683.6   1.85      8.76
3.60  236.30  6.15E+07  12247.8   1.93      8.37
3.83  236.53  6.70E+07  13912.8   2.01      8.03
4.07  236.77  7.27E+07  15679.0   2.09      7.73
4.30  237.00  7.84E+07  17546.8   2.17      7.45

```

```

----- hydrograph ----- <-pipe / channel->
AREA  QPEAK  TPEAK  R.V.  MAX DEPTH  MAX VEL
(ha)  (cms)  (hrs)  (mm)  (m)        (m/s)
INFLOW : ID= 2 (0503) ***** 407.94  27.00  155.36  1.02  0.56
OUTFLOW : ID= 1 (0513) ***** 407.62  27.50  155.36  1.02  0.56

```

```

----- ADD HYD (0504) -----
| 1 + 2 = 3 |
AREA  QPEAK  TPEAK  R.V.
(ha)  (cms)  (hrs)  (mm)
ID1= 1 (0015): 1059.88  26.804  18.25  173.27
+ ID2= 2 (0513): 26734.87  407.621  27.50  155.36
=====
ID = 3 (0504): 27794.75  423.238  27.25  156.04

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

----- ADD HYD (0505) -----
| 1 + 2 = 3 |
AREA  QPEAK  TPEAK  R.V.
(ha)  (cms)  (hrs)  (mm)
ID1= 1 (0013): 1627.59  25.641  24.25  161.46
+ ID2= 2 (0504): 27794.75  423.238  27.25  156.04
=====
ID = 3 (0505): 29422.34  448.004  27.25  156.34

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ROUTE CHN (0506) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

```

```

----- DATA FOR SECTION ( 1.1) -----
Distance  Elevation  Manning
0.00      251.00    0.0500
323.00    238.00    0.0500
400.00    229.00    0.0500
545.00    228.00    0.0500
602.00    226.00    0.0500
604.00    225.80    0.0500 /0.0350 Main Channel
605.00    225.00    0.0350 Main Channel
609.00    225.00    0.0350 Main Channel
610.00    225.80    0.0350 /0.0500 Main Channel
614.00    226.00    0.0500
650.00    227.00    0.0500
810.00    229.50    0.0500
1855.00   240.00    0.0500
2617.00   259.00    0.0500

```

```

----- TRAVEL TIME TABLE -----
DEPTH  ELEV  VOLUME  FLOW RATE  VELOCITY  TRAV.TIME
(m)      (m)  (cu.m.)  (cms)      (m/s)     (min)
0.80  225.80  .292E+05  3.5       0.87      139.36
2.13  227.13  .441E+06  50.6     0.84      145.20
3.45  228.45  .197E+07  262.2     0.97      125.42
4.78  229.78  .557E+07  971.2     1.27      95.57
6.11  231.11  1.05E+08  2326.5    1.61      75.63
7.43  232.43  1.69E+08  4425.6    1.90      63.77
8.76  233.76  2.47E+08  7361.8    2.17      55.90
10.08  235.08  3.38E+08  11229.7   2.42      50.22
11.41  236.41  4.44E+08  16123.0   2.65      45.87
12.74  237.74  5.63E+08  22124.8   2.86      42.40
14.06  239.06  6.97E+08  29005.2   3.03      40.02
15.39  240.39  8.46E+08  37433.0   3.23      37.66
16.72  241.72  1.01E+09  47908.6   3.47      34.97
18.04  243.04  1.17E+09  59742.1   3.71      32.73
19.37  244.37  1.35E+09  72949.0   3.94      30.83
20.69  245.69  1.53E+09  87549.6   4.16      29.20
22.02  247.02  1.73E+09  *****  4.37      27.79
23.35  248.35  1.93E+09  *****  4.58      26.55
24.67  249.67  2.14E+09  *****  4.77      25.45

```

```

----- hydrograph ----- <-pipe / channel->
AREA  QPEAK  TPEAK  R.V.  MAX DEPTH  MAX VEL
(ha)  (cms)  (hrs)  (mm)  (m)        (m/s)
INFLOW : ID= 2 (0505) ***** 448.00  27.25  156.34  3.80  1.03
OUTFLOW : ID= 1 (0506) ***** 438.81  29.00  156.34  3.78  1.03

```

```

| ROUTE CHN (0514) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

```

```

----- DATA FOR SECTION ( 1.1) -----
Distance  Elevation  Manning
0.00      230.00    0.0500
400.00    229.00    0.0500
545.00    228.00    0.0500
602.00    226.00    0.0500
604.00    225.80    0.0500 /0.0350 Main Channel
605.00    225.00    0.0350 Main Channel
609.00    225.00    0.0350 Main Channel
610.00    225.80    0.0350 /0.0500 Main Channel
614.00    226.00    0.0500
650.00    227.00    0.0500
810.00    229.50    0.0500

```

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.20	225.20	.243E+04	0.3	0.39	121.91
0.40	225.40	.515E+04	1.1	0.59	80.58
0.60	225.60	.815E+04	2.1	0.74	64.02
0.80	225.80	.114E+05	3.5	0.87	54.69
1.05	226.05	.184E+05	6.1	0.94	50.48
1.29	226.29	.346E+05	10.3	0.85	55.99
1.54	226.54	.620E+05	17.4	0.80	59.38
1.79	226.79	.101E+06	28.3	0.81	59.18
2.03	227.03	.151E+06	43.8	0.83	57.35
2.28	227.28	.215E+06	63.9	0.85	55.99
2.53	227.53	.295E+06	91.4	0.89	53.82
2.77	227.77	.391E+06	127.0	0.93	51.35
3.02	228.02	.504E+06	170.6	0.97	49.24
3.27	228.27	.644E+06	215.5	0.96	49.94
3.51	228.51	.821E+06	279.7	0.97	48.92
3.76	228.76	.103E+07	363.1	1.00	47.47
4.01	229.01	.128E+07	466.2	1.04	45.89
4.25	229.25	.159E+07	572.0	1.03	46.41
4.50	229.50	.198E+07	719.1	1.04	45.95

<---- hydrograph ----> <-pipe / channel->

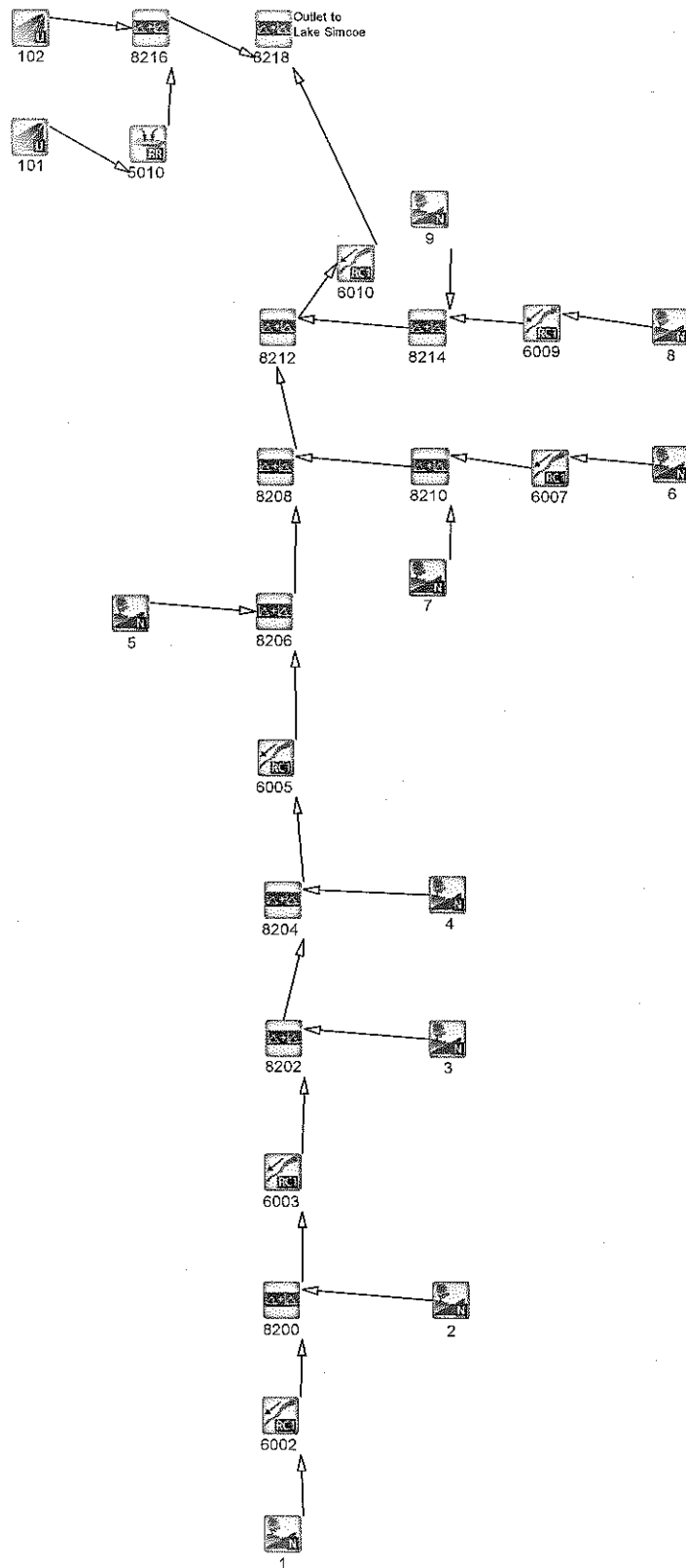
AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0506) *****	438.81	29.00	156.34	3.94	1.03
OUTFLOW : ID= 1 (0514) *****	437.39	29.50	156.34	3.93	1.03

ADD HYD (0507)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
1	2	3	1	2	3
ID1= 1 (0016):	3094.49	45.605	26.75	175.67	
+ ID2= 2 (0514):	29422.34	437.388	29.50	156.34	
ID = 3 (0507):	32516.83	481.948	29.50	158.18	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

MASKINONGE RIVER SUBWATERSHED



EXISTING
WEST HOLLAND RIVER 2-100 YEAR STORMS

V V I SSSS U U A L
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
WV I SSSS UUUU A A LLLL

OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y M M O O
O O T T H H Y M M O O Company
OOO T T H H Y M M OOO Serial

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***** DETAILED OUTPUT *****

Input filename: C:\Program Files\Visual Othymo 2.4r\VO2\vo1n.dat
Output filename: C:\Users\jscott\AppData\Local\Temp\3e280798-92ee-4282-809c-79f5caed0add\Scenario.out
Summary filename: C:\Users\jscott\AppData\Local\Temp\3e280798-92ee-4282-809c-79f5caed0add\Scenario.sum

DATE: 08/22/2012 TIME: 01:32:16

USER:

COMMENTS: _____

** SIMULATION NUMBER: 1 **

Filename: C:\Users\jscott\AppData\Local\Temp\3e280798-92ee-4282-809c-79f5caed0add\Scad6041
Comments: 2-Year 12-Hour SCS II Design Storm

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.05	3.25	1.68	6.25	7.56	9.25	1.47
0.50	1.05	3.50	1.68	6.50	7.56	9.50	1.47
0.75	1.05	3.75	1.68	6.75	3.36	9.75	1.47
1.00	1.05	4.00	1.68	7.00	3.36	10.00	1.47
1.25	1.05	4.25	2.52	7.25	2.52	10.25	0.84
1.50	1.05	4.50	2.52	7.50	2.52	10.50	0.84
1.75	1.05	4.75	3.36	7.75	2.52	10.75	0.84
2.00	1.05	5.00	3.36	8.00	2.52	11.00	0.84
2.25	1.26	5.25	5.04	8.25	1.47	11.25	0.84
2.50	1.26	5.50	5.04	8.50	1.47	11.50	0.84
2.75	1.26	5.75	20.16	8.75	1.47	11.75	0.84
3.00	1.26	6.00	55.44	9.00	1.47	12.00	0.84

CALIB (0356) Area (ha)=1015.22 Curve Number (CN)= 46.0
NASHYD (0356) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.37

Unit Hyd Qpeak (cms)= 12.651

PEAK FLOW (cms)= 1.461 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 4.058
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.097

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0354) Area (ha)= 262.68 Curve Number (CN)= 37.0
NASHYD (0354) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.38

Unit Hyd Qpeak (cms)= 3.252

PEAK FLOW (cms)= 0.268 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 2.897

TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.069

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0352) Area (ha)= 381.43 Curve Number (CN)= 54.0
NASHYD (0352) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.82

Unit Hyd Qpeak (cms)= 7.980

PEAK FLOW (cms)= 1.066 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 5.329
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.127

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0346) Area (ha)= 350.93 Curve Number (CN)= 70.0
NASHYD (0346) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 6.254

PEAK FLOW (cms)= 1.534 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 9.284
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.221

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0350) Area (ha)= 366.84 Curve Number (CN)= 48.0
NASHYD (0350) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 5.831

PEAK FLOW (cms)= 0.676 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 4.345
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.103

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0348) Area (ha)= 590.18 Curve Number (CN)= 48.0
NASHYD (0348) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.91

Unit Hyd Qpeak (cms)= 5.267

PEAK FLOW (cms)= 0.711 (i)
TIME TO PEAK (hrs)= 9.250
RUNOFF VOLUME (mm)= 4.368
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.104

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6350) Routing time step (min)= 15.00

<----- DATA FOR SECTION (3501.0) ----->

Distance	Elevation	Manning
0.00	287.00	0.0500
23.66	283.72	0.0500
70.98	280.94	0.0500
112.38	280.23	0.0500
171.52	276.80	0.0500
260.24	274.46	0.0500
266.16	274.26	0.0500
272.07	274.12	0.0500
276.49	274.12	0.0500 / 0.0300
276.99	273.82	0.0300
277.99	273.82	0.0300
279.49	273.82	0.0300
280.49	274.27	0.0300 / 0.0500
283.90	274.27	0.0500
289.81	274.57	0.0500

325.30	275.75	0.0500
396.28	278.98	0.0500
449.51	280.97	0.0500
496.83	283.90	0.0500
585.54	287.92	0.0500

2.55	241.36	.804E+05	101.0	1.96	13.27
3.53	242.34	.173E+06	275.6	2.48	10.46
4.50	243.31	.293E+06	553.9	2.95	8.80
5.48	244.29	.438E+06	961.8	3.42	7.60
6.45	245.26	.599E+06	1534.4	3.99	6.51
7.43	246.24	.770E+06	2228.4	4.51	5.76
8.41	247.22	.951E+06	3043.6	4.98	5.21
9.38	248.19	1.14E+07	3982.1	5.43	4.78
10.36	249.17	1.34E+07	5040.1	5.85	4.44
11.33	250.14	1.55E+07	6218.5	6.24	4.16
12.31	251.12	1.77E+07	7518.6	6.61	3.93
13.29	252.10	2.00E+07	8969.0	6.98	3.72
14.26	253.07	2.24E+07	10554.1	7.35	3.53
15.24	254.05	2.48E+07	12257.1	7.71	3.37
16.21	255.02	2.72E+07	14045.1	8.03	3.23
17.19	256.00	2.98E+07	15954.8	8.34	3.11
18.16	256.97	3.25E+07	17980.4	8.53	3.04

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.30	274.12	.273E+04	0.9	0.99	49.81
0.98	274.80	.583E+05	20.8	1.05	46.71
1.66	275.48	1.203E+06	95.2	1.38	35.53
2.33	276.15	2.438E+06	256.0	1.73	28.50
3.01	276.83	4.755E+06	524.8	2.05	23.97
3.69	277.51	8.114E+07	935.8	2.43	20.26
4.37	278.19	1.578E+07	1472.0	2.76	17.82
5.05	278.87	2.908E+07	2140.7	3.06	16.06
5.72	279.54	4.261E+07	2938.6	3.33	14.79
6.40	280.22	7.321E+07	3887.6	3.57	13.77
7.08	280.90	1.390E+07	4795.4	3.63	13.56
7.76	281.58	2.468E+07	6196.2	3.91	12.58
8.43	282.25	4.550E+07	7803.9	4.19	11.74
9.11	282.93	8.637E+07	9601.9	4.45	11.05
9.79	283.61	1.728E+07	11594.8	4.70	10.46
10.47	284.29	3.238E+07	13845.3	4.96	9.91
11.15	284.97	5.922E+07	16293.4	5.21	9.43
11.82	285.64	1.038E+08	18952.0	5.45	9.02
12.50	286.32	1.138E+08	21824.3	5.69	8.65

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLW : ID= 2 (8222)	1307.95	2.71	8.00	5.68	0.63
OUTFLOW : ID= 1 (6352)	1307.95	2.69	8.25	5.68	0.62

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLW : ID= 2 (0348)	590.18	0.71	9.25	4.37	0.23
OUTFLOW : ID= 1 (6350)	590.18	0.70	10.25	4.37	0.23

ADD HYD (8224) | 1 + 2 = 3 |

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0352):	381.43	1.066	7.00
+ ID2= 2 (6352):	1307.95	2.690	8.25
ID = 3 (8224):	1689.38	3.668	8.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8220) | 1 + 2 = 3 |

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0350):	366.84	0.676	7.75
+ ID2= 2 (6350):	590.18	0.695	10.25
ID = 3 (8220):	957.02	1.262	8.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8222) | 1 + 2 = 3 |

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0346):	350.93	1.534	7.25
+ ID2= 2 (8220):	957.02	1.262	8.75
ID = 3 (8222):	1307.95	2.712	8.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6352) | IN= 2--> OUT= 1 | Routing time step (min)= 15.00

ROUTE CHN (6354) | IN= 2--> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (3541.0) -----

Distance	Elevation	Manning
0.00	253.92	0.0500
7.95	251.83	0.0500
15.89	249.97	0.0500
19.87	249.05	0.0500
47.68	242.00	0.0500
67.55	237.10	0.0500
83.44	231.11	0.0500
85.43	230.75	0.0500
85.66	230.61	0.0500 / 0.0300
85.76	230.01	0.0300
87.41	230.01	0.0300
88.06	230.01	0.0300
88.16	230.61	0.0300 / 0.0500
89.40	230.72	0.0500
91.39	230.88	0.0500
93.37	231.03	0.0500
133.11	233.96	0.0500
150.99	235.39	0.0500
172.84	239.03	0.0500
196.68	250.00	0.0500

----- DATA FOR SECTION (3521.0) -----

Distance	Elevation	Manning
0.00	257.95	0.0500
7.83	257.34	0.0500
15.66	256.19	0.0500
21.53	254.01	0.0500
41.11	244.06	0.0500
76.35	241.38	0.0500
111.58	239.74	0.0500
113.54	239.61	0.0500
113.75	239.41	0.0500 / 0.0300
113.85	238.81	0.0300
115.50	238.81	0.0300
116.15	238.81	0.0300
116.25	239.41	0.0300
117.46	239.52	0.0500
119.41	239.72	0.0500
121.37	240.04	0.0500
131.16	241.84	0.0500
156.61	247.03	0.0500
176.19	251.46	0.0500
193.80	258.79	0.0500

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.60	239.41	.224E+04	2.2	1.52	17.12
1.58	240.39	.216E+05	22.3	1.61	16.16

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.60	230.61	.451E+04	1.5	1.06	49.40
1.62	231.63	.394E+05	14.6	1.16	45.13
2.64	232.65	1.288E+06	58.8	1.44	36.30
3.66	233.67	2.695E+06	149.7	1.74	30.00
4.68	234.69	4.463E+06	301.5	2.04	25.58
5.70	235.71	7.058E+06	530.5	2.36	22.14
6.72	236.73	9.808E+06	846.4	2.71	19.25
7.74	237.75	1.28E+07	1237.9	3.02	17.29
8.76	238.77	1.62E+07	1706.7	3.30	15.83
9.78	239.79	1.99E+07	2291.9	3.61	14.45
10.81	240.82	2.37E+07	2973.7	3.92	13.30
11.83	241.84	2.78E+07	3742.2	4.21	12.38
12.85	242.86	3.21E+07	4599.1	4.49	11.62
13.87	243.88	3.65E+07	5544.6	4.75	10.98
14.89	244.90	4.12E+07	6579.7	5.00	10.44
15.91	245.92	4.61E+07	7705.9	5.23	9.97
16.93	246.94	5.12E+07	8924.6	5.46	9.55
17.95	247.96	5.64E+07	10237.5	5.68	9.19
18.97	248.98	6.19E+07	11646.2	5.89	8.86

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLW : ID= 2 (8224)	1689.38	3.67	8.00	5.60	0.77
OUTFLOW : ID= 1 (6354)	1689.38	3.41	9.00	5.60	0.75

```

-----
| ADD HYD (8226) |
| 1 + 2 = 3 |
-----
| ID= 1 (0354): 262.68 0.268 8.25 2.90
+ ID= 2 (6354): 1689.38 3.413 9.00 5.60
=====
| ID = 3 (8226): 1952.06 3.673 9.00 5.24
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0344) | Area (ha)= 344.00 Curve Number (CN)= 59.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
-----
| U.H. Tp(hrs)= 0.87
-----

```

```

Unit Hyd Qpeak (cms)= 6.790

PEAK FLOW (cms)= 1.096 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 6.331
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.151

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANDHYD (3402) | Area (ha)= 234.60
| ID= 1 DT=15.0 min | Total Imp(%)= 28.00 Dir. Conn.(%)= 13.00
-----

```

```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 65.69 168.91
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1250.60 40.00
Mannings n = 0.013 0.250

```

```

Max.Eff.Inten.(mm/hr)= 55.44 4.42
over (min) 15.00 60.00
Storage Coeff. (min)= 18.13 (ii) 55.39 (ii)
Unit Hyd. Tpeak (min)= 15.00 60.00
Unit Hyd. peak (cms)= 0.06 0.02
**TOTALS*
PEAK FLOW (cms)= 3.14 1.13 3.461 (iii)
TIME TO PEAK (hrs)= 6.00 6.75 6.00
RUNOFF VOLUME (mm)= 40.00 5.40 9.90
TOTAL RAINFALL (mm)= 42.00 42.00 42.00
RUNOFF COEFFICIENT = 0.95 0.13 0.24

```

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 48.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANDHYD (3401) | Area (ha)= 51.20
| ID= 1 DT=15.0 min | Total Imp(%)= 46.00 Dir. Conn.(%)= 21.00
-----

```

```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 23.55 27.65
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 584.24 40.00
Mannings n = 0.013 0.250

```

```

Max.Eff.Inten.(mm/hr)= 55.44 8.48
over (min) 15.00 45.00
Storage Coeff. (min)= 11.48 (ii) 40.19 (ii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.08 0.03
**TOTALS*
PEAK FLOW (cms)= 1.34 0.35 1.474 (iii)
TIME TO PEAK (hrs)= 6.00 6.50 6.00
RUNOFF VOLUME (mm)= 40.00 6.57 13.59
TOTAL RAINFALL (mm)= 42.00 42.00 42.00
RUNOFF COEFFICIENT = 0.95 0.16 0.32

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 48.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| RESERVOIR (5340) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
| OUTFLOW STORAGE | OUTFLOW STORAGE |
| (cms) (ha.m.) | (cms) (ha.m.) |
| 0.0000 0.0000 | 5.3360 1.7877 |
| 0.1730 0.6348 | 7.2780 2.2185 |
| 1.8360 0.8250 | 9.1880 2.6486 |
| 2.1540 1.1051 | 9.5880 2.6586 |
| 3.3950 1.3506 | 0.0000 0.0000 |
-----

```

```

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (3401) 51.200 1.474 6.00 13.59
OUTFLOW : ID= 1 (5340) 51.200 0.126 8.75 13.57

```

```

PEAK FLOW REDUCTION [Qout/Qin](%)= 8.58
TIME SHIFT OF PEAK FLOW (min)=165.00
MAXIMUM STORAGE USED (ha.m.)= 0.4644

```

```

-----
| ADD HYD (8232) |
| 1 + 2 = 3 |
-----
| ID= 1 (3402): 234.60 3.461 6.00 9.90
+ ID= 2 (5340): 51.20 0.126 8.75 13.57
=====
| ID = 3 (8232): 285.80 3.513 6.00 10.55
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0342) | Area (ha)=1004.58 Curve Number (CN)= 54.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
-----
| U.H. Tp(hrs)= 2.57
-----

```

```

Unit Hyd Qpeak (cms)= 6.686

PEAK FLOW (cms)= 1.208 (i)
TIME TO PEAK (hrs)= 10.250
RUNOFF VOLUME (mm)= 5.390
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.128

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0422) | Area (ha)= 780.20 Curve Number (CN)= 54.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
-----
| U.H. Tp(hrs)= 1.95
-----

```

```

Unit Hyd Qpeak (cms)= 6.838

PEAK FLOW (cms)= 1.144 (i)
TIME TO PEAK (hrs)= 9.500
RUNOFF VOLUME (mm)= 5.383
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.128

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0410) | Area (ha)= 572.01 Curve Number (CN)= 48.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
-----
| U.H. Tp(hrs)= 1.46
-----

```

```

Unit Hyd Qpeak (cms)= 6.698

PEAK FLOW (cms)= 0.844 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 4.360
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.104

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0408) | Area (ha)= 231.62 Curve Number (CN)= 58.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
-----
| U.H. Tp(hrs)= 0.64
-----

```

Unit Hyd Qpeak (cms)= 6.198
PEAK FLOW (cms)= 0.897 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 6.074
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.145

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0376) Area (ha)= 463.85 Curve Number (CN)= 74.0
NASHYD (0376) ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 7.380

PEAK FLOW (cms)= 2.165 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 10.744
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.256

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0374) Area (ha)= 545.70 Curve Number (CN)= 61.0
NASHYD (0374) ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.51

Unit Hyd Qpeak (cms)= 6.158

PEAK FLOW (cms)= 1.234 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 6.828
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.163

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0372) Area (ha)= 110.42 Curve Number (CN)= 37.0
NASHYD (0372) ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 1.954

PEAK FLOW (cms)= 0.145 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 2.885
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.069

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0370) Area (ha)= 191.85 Curve Number (CN)= 63.0
NASHYD (0370) ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.67

Unit Hyd Qpeak (cms)= 4.860

PEAK FLOW (cms)= 0.850 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 7.219
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.172

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0368) Area (ha)= 159.48 Curve Number (CN)= 46.0
NASHYD (0368) ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.12

Unit Hyd Qpeak (cms)= 2.433

PEAK FLOW (cms)= 0.266 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 4.049
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.096

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0366) Area (ha)= 462.62 Curve Number (CN)= 62.0
NASHYD (0366) ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.06

Unit Hyd Qpeak (cms)= 7.451

PEAK FLOW (cms)= 1.407 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 7.039
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.168

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0364) Area (ha)= 155.27 Curve Number (CN)= 55.0
NASHYD (0364) ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.70

Unit Hyd Qpeak (cms)= 3.782

PEAK FLOW (cms)= 0.504 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 5.495
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.131

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8302)
1 2 3
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0364): 155.27 0.504 7.00 5.50
+ ID2= 2 (0366): 462.62 1.407 7.75 7.04

ID = 3 (8302): 617.89 1.886 7.25 6.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0362) Area (ha)= 118.78
STANDHYD (0362) ID= 1 DT=15.0 min Total Imp(%)= 22.00 Dir. Conn.(%)= 8.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 26.13 92.65
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 889.87 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 55.44 3.78
over (min)= 15.00 60.00
Storage Coeff. (min)= 14.78 (ii) 54.45 (ii)
Unit Hyd. Tpeak (min)= 15.00 60.00
Unit Hyd. peak (cms)= 0.07 0.02

TOTALS
PEAK FLOW (cms)= 1.07 0.54 1.224 (iii)
TIME TO PEAK (hrs)= 6.00 6.75 6.00
RUNOFF VOLUME (mm)= 40.00 4.74 7.56
TOTAL RAINFALL (mm)= 42.00 42.00 42.00
RUNOFF COEFFICIENT = 0.95 0.11 0.18

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 45.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0358) Area (ha)= 429.87 Curve Number (CN)= 35.0
NASHYD (0358) ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.03

Unit Hyd Qpeak (cms)= 7.091

PEAK FLOW (cms)= 0.496 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 2.665
TOTAL RAINFALL (mm)= 42.000

RUNOFF COEFFICIENT = 0.063

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0360) | Area (ha)= 138.37 Curve Number (CN)= 46.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.60

```

Unit Hyd Qpeak (cms)= 3.957

```

PEAK FLOW (cms)= 0.367 (1)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 3.995
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.095

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8306) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (0358): 429.87 0.496 7.75 2.66
| + ID2= 2 (0360): 138.37 0.367 6.75 4.00
|-----|
| ID = 3 (8306): 568.24 0.839 7.00 2.99

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6362) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00

```

<----- DATA FOR SECTION (3621.0) ----->

Distance	Elevation	Manning
0.00	261.46	0.0550
27.86	254.23	0.0550
51.07	251.96	0.0550
74.29	250.77	0.0550
97.50	249.91	0.0550
125.96	249.40	0.0550
150.93	247.40	0.0550
155.93	247.33	0.0550 / 0.0350
157.93	246.85	0.0350
159.18	246.65	0.0350
160.18	246.63	0.0350
160.93	246.85	0.0350
161.93	247.18	0.0350 / 0.0550
163.18	248.03	0.0550
168.18	248.58	0.0550
183.18	250.18	0.0550
201.97	252.59	0.0550
213.57	256.02	0.0550
225.18	260.31	0.0550
229.82	261.00	0.0550

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.55	247.18	.558E+04	1.9	1.03	49.95
1.28	247.91	.336E+05	16.7	1.53	33.61
2.00	248.63	.913E+05	53.6	1.81	28.40
2.73	249.36	.185E+06	122.3	2.05	25.18
3.46	250.09	.344E+06	224.6	2.02	25.52
4.19	250.82	.574E+06	417.9	2.25	22.91
4.91	251.54	.855E+06	699.1	2.53	20.38
5.64	252.27	.118E+07	1075.3	2.82	18.26
6.37	253.00	.153E+07	1559.9	3.15	16.38
7.10	253.73	.191E+07	2137.6	3.46	14.89
7.82	254.45	.231E+07	2815.2	3.77	13.67
8.55	255.18	.272E+07	3608.5	4.10	12.58
9.28	255.91	.315E+07	4488.2	4.41	11.69
10.01	256.64	.358E+07	5458.0	4.71	10.95
10.73	257.36	.403E+07	6512.4	4.99	10.32
11.46	258.09	.449E+07	7649.8	5.27	9.78
12.19	258.82	.496E+07	8869.5	5.53	9.32
12.92	259.55	.544E+07	10171.2	5.78	8.91
13.64	260.27	.593E+07	11554.8	6.02	8.55

<----- hydrograph -----> <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8306)	568.24	0.84	7.00	2.99	0.25
OUTFLOW: ID= 1 (6362)	568.24	0.75	8.25	2.99	0.22

```

-----
| ADD HYD (8304) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (0362): 118.78 1.224 6.00 7.56
| + ID2= 2 (6362): 568.24 0.746 8.25 2.99
|-----|
| ID = 3 (8304): 687.02 1.321 6.00 3.78

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8300) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (8302): 617.89 1.886 7.25 6.45
| + ID2= 2 (8304): 687.02 1.321 6.00 3.78
|-----|
| ID = 3 (8300): 1304.91 3.036 7.25 5.14

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6368) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00

```

<----- DATA FOR SECTION (3681.0) ----->

Distance	Elevation	Manning
0.00	230.00	0.0370
18.48	223.26	0.0370
36.96	223.05	0.0370
64.67	222.94	0.0370
110.87	222.86	0.0370
133.96	222.74	0.0370
147.82	222.65	0.0370
170.92	222.31	0.0370
174.79	222.26	0.0370 / 0.0300
174.89	221.86	0.0300
175.54	221.86	0.0300
176.19	221.86	0.0300
176.29	222.26	0.0300 / 0.0370
180.16	222.25	0.0370
184.78	222.28	0.0370
189.40	222.31	0.0370
332.60	222.37	0.0370
450.00	230.00	0.0370

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.40	222.26	.771E+03	0.3	0.52	41.74
0.81	222.67	.784E+05	30.3	0.51	43.17
1.21	223.07	.210E+06	123.2	0.77	28.42
1.62	223.48	.384E+06	301.0	1.03	21.25
2.03	223.89	.563E+06	550.2	1.28	17.06
2.44	224.30	.747E+06	860.7	1.51	14.46
2.84	224.70	.934E+06	1228.1	1.73	12.68
3.25	225.11	.113E+07	1649.3	1.92	11.37
3.66	225.52	.132E+07	2122.3	2.11	10.37
4.07	225.93	.152E+07	2645.5	2.29	9.58
4.47	226.33	.172E+07	3217.8	2.45	8.92
4.88	226.74	.193E+07	3838.3	2.61	8.38
5.29	227.15	.214E+07	4506.3	2.76	7.92
5.70	227.56	.236E+07	5221.3	2.91	7.52
6.10	227.96	.258E+07	5983.0	3.05	7.17
6.51	228.37	.280E+07	6790.9	3.19	6.87
6.92	228.78	.303E+07	7645.0	3.32	6.60
7.33	229.19	.326E+07	8545.1	3.45	6.35
7.73	229.59	.349E+07	9491.0	3.57	6.13

<----- hydrograph -----> <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8300)	1304.91	3.04	7.25	5.14	0.44
OUTFLOW: ID= 1 (6368)	1304.91	2.84	8.25	5.14	0.43

```

-----
| ADD HYD (8298) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (0368): 159.48 0.266 8.00 4.05
| + ID2= 2 (6368): 1304.91 2.837 8.25 5.14
|-----|
| ID = 3 (8298): 1464.39 3.099 8.25 5.02

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8296)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0370):	191.85	0.850	6.75	7.22
+ ID2= 2 (8296):	1464.39	3.099	8.25	5.02
=====				
ID = 3 (8296):	1656.24	3.807	8.00	5.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN= (5372) Routing time step (min)'= 15.00
IN= 2--> OUT= 1

<----- DATA FOR SECTION (3721.0) ----->

Distance	Elevation	Manning	
0.00	225.00	0.0390	
30.80	219.38	0.0390	
61.62	219.30	0.0390	
77.01	219.27	0.0390	
469.76	219.14	0.0390	
477.46	219.13	0.0390	
485.16	219.10	0.0390	
492.86	219.09	0.0390	
495.56	219.09	0.0390 / 0.0310	Main Channel
495.66	218.51	0.0310	Main Channel
500.56	218.51	0.0310	Main Channel
505.46	218.51	0.0310	Main Channel
505.56	219.09	0.0310 / 0.0390	Main Channel
508.26	219.09	0.0390	
515.96	219.10	0.0390	
523.67	219.21	0.0390	
562.17	219.32	0.0390	
654.58	219.43	0.0390	
731.59	219.46	0.0390	
762.39	225.00	0.0390	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.29	218.80	.431E+04	1.7	0.58	43.48
0.58	219.09	.867E+04	5.1	0.89	28.32
0.93	219.44	.200E+06	58.2	0.44	57.35
1.28	219.79	.568E+06	278.6	0.74	33.99
1.62	220.13	.939E+06	630.2	1.01	24.84
1.97	220.48	.131E+07	1090.4	1.25	20.05
2.32	220.83	.169E+07	1647.7	1.47	17.06
2.67	221.18	.206E+07	2294.6	1.68	14.99
3.01	221.52	.244E+07	3025.6	1.87	13.46
3.36	221.87	.282E+07	3836.5	2.05	12.27
3.71	222.22	.321E+07	4723.7	2.22	11.31
4.06	222.57	.359E+07	5684.6	2.39	10.53
4.40	222.91	.398E+07	6716.6	2.55	9.87
4.75	223.26	.437E+07	7817.7	2.70	9.31
5.10	223.61	.476E+07	8986.2	2.85	8.83
5.45	223.96	.515E+07	10220.4	2.99	8.40
5.79	224.30	.555E+07	11519.1	3.13	8.03
6.14	224.65	.594E+07	12880.8	3.27	7.69
6.49	225.00	.634E+07	14304.5	3.40	7.39

<---- hydrograph ----> <-pipe / channel-->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8296)	1656.24	3.81	8.00	5.27	0.47	0.74
OUTFLOW: ID= 1 (6372)	1656.24	3.70	8.50	5.27	0.46	0.73

ADD HYD (8294)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0372):	110.42	0.145	7.50	2.88
+ ID2= 2 (6372):	1656.24	3.698	8.50	5.27
=====				
ID = 3 (8294):	1766.66	3.833	8.50	5.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8292)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0374):	545.70	1.214	8.25	6.83
+ ID2= 2 (8294):	1766.66	3.833	8.50	5.13
=====				
ID = 3 (8292):	2312.36	5.066	8.50	5.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5505)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1	0.0000	0.0000	65.1290	345.3754
DT= 15.0 min	25.4850	24.6697	84.9510	456.3890
	31.1490	98.6787	*****	838.7689
	39.6440	*****	*****	838.7789
	48.1390	*****	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8292)	2312.360	5.066	8.50	5.53
OUTFLOW: ID= 1 (5505)	2312.360	3.659	11.25	5.53

PEAK FLOW REDUCTION [Qout/Qin](%)= 72.23
TIME SHIFT OF PEAK FLOW (min)=165.00
MAXIMUM STORAGE USED (ha.m.)= 3.5421

ADD HYD (8272)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0376):	463.85	2.165	7.50	10.74
+ ID2= 2 (5505):	2312.36	3.659	11.25	5.53
=====				
ID = 3 (8272):	2776.21	5.098	10.25	6.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0396)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	305.21	69.0
	U.H. Tp(hrs)=	1.08

Unit Hyd Qpeak (cms)= 4.811

PEAK FLOW (cms)= 1.174 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 8.977
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.214

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0394)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	325.45	53.0
	U.H. Tp(hrs)=	0.92

Unit Hyd Qpeak (cms)= 6.013

PEAK FLOW (cms)= 0.799 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 5.160
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.123

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0390)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	420.00	55.0
	U.H. Tp(hrs)=	1.07

Unit Hyd Qpeak (cms)= 6.683

PEAK FLOW (cms)= 0.992 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 5.541
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.132

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0388)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	220.77	58.0
	U.H. Tp(hrs)=	0.99

Unit Hyd Qpeak (cms)= 3.819

PEAK FLOW (cms)= 0.615 (i)
TIME TO PEAK (hrs)= 7.500

RUNOFF VOLUME (mm)= 6.132
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.146

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0386) | Area (ha)= 241.27 | Curve Number (CN)= 61.0
NASHYD (0386) | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.90

Unit Hyd Opeak (cms)= 4.562
PEAK FLOW (cms)= 0.799 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 6.784
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.162

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8286) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0386): 241.27 0.799 7.25 6.78
+ ID2= 2 (0386): 220.77 0.615 7.50 6.13
ID = 3 (8286): 462.04 1.411 7.25 6.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0384) | Area (ha)= 199.07 | Curve Number (CN)= 44.0
NASHYD (0384) | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.96

Unit Hyd Opeak (cms)= 3.537
PEAK FLOW (cms)= 0.343 (1)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 3.759
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.089

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0380) | Area (ha)= 182.01 | Curve Number (CN)= 40.0
NASHYD (0380) | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.55

Unit Hyd Opeak (cms)= 5.609
PEAK FLOW (cms)= 0.407 (1)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 3.195
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.076

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0382) | Area (ha)= 216.59 | Curve Number (CN)= 53.0
NASHYD (0382) | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.64

Unit Hyd Opeak (cms)= 5.733
PEAK FLOW (cms)= 0.697 (1)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 5.118
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.122

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8290) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0380): 182.01 0.407 6.75 3.20
+ ID2= 2 (0382): 216.59 0.697 6.75 5.12

ID = 3 (8290): 398.60 1.104 6.75 4.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6384) |
ID= 2 -> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (3841.0) -----

Distance	Elevation	Manning
0.00	294.40	0.0380
10.59	291.93	0.0380
21.17	289.19	0.0380
26.46	287.99	0.0380
31.76	286.79	0.0380
71.45	279.97	0.0380
74.10	279.79	0.0380
76.74	279.71	0.0380
78.99	279.64	0.0380 / 0.0300
79.14	279.30	0.0300
79.39	279.30	0.0300
79.64	279.30	0.0300
79.79	279.64	0.0300 / 0.0380
89.98	279.78	0.0380
119.09	281.86	0.0380
145.55	282.87	0.0380
198.48	284.85	0.0380
211.71	286.31	0.0380
230.23	287.59	0.0380
261.99	294.00	0.0380

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELUV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.34	279.64	.715E+03	0.2	0.85	63.42
1.10	280.40	.507E+05	21.9	1.40	38.54
1.85	281.15	.141E+06	91.6	2.10	25.69
2.61	281.91	.269E+06	218.9	2.63	20.46
3.36	282.66	.445E+06	407.1	2.96	18.23
4.12	283.42	.681E+06	698.3	3.31	16.26
4.87	284.17	.977E+06	1111.8	3.68	14.65
5.63	284.93	.133E+07	1674.3	4.06	13.27
6.39	285.69	.173E+07	2457.8	4.59	11.73
7.14	286.44	.215E+07	3378.8	5.07	10.63
7.90	287.20	.261E+07	4418.4	5.47	9.85
8.65	287.95	.310E+07	5676.3	5.92	9.11
9.41	288.71	.361E+07	7150.5	6.40	8.42
10.17	289.47	.414E+07	8783.1	6.86	7.86
10.92	290.22	.469E+07	10575.1	7.30	7.38
11.68	290.98	.525E+07	12520.4	7.72	6.98
12.43	291.73	.582E+07	14619.0	8.12	6.64
13.19	292.49	.642E+07	16863.2	8.50	6.34
13.94	293.24	.703E+07	19259.8	8.86	6.08

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8290) 398.60 1.10 6.75 4.24 0.37 0.86
OUTFLOW: ID= 1 (6384) 398.60 0.87 8.00 4.24 0.36 0.86

ADD HYD (8288) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0384): 199.07 0.343 7.50 3.76
+ ID2= 2 (6384): 398.60 0.866 8.00 4.24
ID = 3 (8288): 597.67 1.204 8.00 4.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8284) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8286): 462.04 1.411 7.25 6.47
+ ID2= 2 (8288): 597.67 1.204 8.00 4.08
ID = 3 (8284): 1059.71 2.590 7.75 5.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8280) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)

ID1= 1 (0390): 420.00 0.392 7.75 5.54
 + ID2= 2 (8284): 1059.71 2.590 7.75 5.12
 =====
 ID = 3 (8280): 1479.71 3.582 7.75 5.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0392) | Area (ha)= 167.22 Curve Number (CN)= 62.0
 NASHYD (0392) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.74

Unit Hyd Qpeak (cms)= 3.837
 PEAK FLOW (cms)= 0.666 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 6.993
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.166

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0378) | Area (ha)= 606.72 Curve Number (CN)= 55.0
 NASHYD (0378) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.18

Unit Hyd Qpeak (cms)= 8.771
 PEAK FLOW (cms)= 1.341 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 5.547
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.132

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8282) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0378): 606.72 1.341 8.00 5.55
 + ID2= 2 (0392): 167.22 0.666 7.00 6.99
 =====
 ID = 3 (8282): 773.94 1.958 7.50 5.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8278) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8280): 1479.71 3.582 7.75 5.24
 + ID2= 2 (8284): 773.94 1.958 7.50 5.86
 =====
 ID = 3 (8278): 2253.65 5.533 7.75 5.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6394) |
 IN= 2 -> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3941.0) ----->
 Distance Elevation Manning
 0.00 283.00 0.0380
 13.48 282.87 0.0380
 53.92 280.08 0.0380
 74.13 276.62 0.0380
 97.72 265.45 0.0380
 114.57 256.93 0.0380
 131.42 253.04 0.0380
 134.79 252.58 0.0380
 138.53 251.74 0.0380 / 0.0300 Main Channel
 139.03 251.20 0.0300 Main Channel
 141.53 251.20 0.0300 Main Channel
 144.03 251.20 0.0300 Main Channel
 144.53 251.74 0.0300 / 0.0380 Main Channel
 148.27 252.69 0.0380
 151.64 252.97 0.0380
 185.34 255.08 0.0380
 219.03 257.54 0.0380
 262.84 259.43 0.0380
 310.02 262.80 0.0380
 333.60 283.00 0.0380

<----- TRAVEL TIME TABLE ----->
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
 (m) (m) (cu.m.) (cms) (m/s) (min)
 0.54 251.74 .827E+04 4.5 1.51 30.71
 2.19 251.39 .758E+05 73.2 2.69 17.27
 3.83 255.03 .282E+06 331.8 3.27 14.17
 5.48 256.68 .633E+06 917.3 4.03 11.51
 7.12 258.32 .112E+07 1889.5 4.69 9.90
 8.77 259.97 .179E+07 3410.2 5.31 8.73
 10.41 261.61 .259E+07 5627.6 6.05 7.66
 12.06 263.26 .351E+07 8605.1 6.83 6.79
 13.70 264.90 .447E+07 12544.9 7.81 5.94
 15.35 266.55 .548E+07 17101.4 8.71 5.32
 16.99 268.19 .648E+07 22251.7 9.56 4.85
 18.64 269.84 .752E+07 27979.0 10.36 4.48
 20.28 271.48 .858E+07 34269.0 11.11 4.17
 21.93 273.13 .967E+07 41110.8 11.83 3.92
 23.57 274.77 .108E+08 48495.9 12.51 3.71
 25.22 276.42 .119E+08 56418.1 13.17 3.52
 26.86 278.06 .131E+08 64410.6 13.69 3.39
 28.51 279.71 .143E+08 73002.1 14.18 3.27
 30.15 281.35 .156E+08 81379.3 14.50 3.20

<---- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8278) 2253.65 5.53 7.75 5.45 0.57 1.52
 OUTFLOW : ID= 1 (6394) 2253.65 5.37 8.25 5.45 0.56 1.52

ADD HYD (8276) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0394): 325.45 0.799 7.25 5.16
 + ID2= 2 (6394): 2253.65 5.366 8.25 5.45
 =====
 ID = 3 (8276): 2579.10 6.118 8.25 5.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6396) |
 IN= 2 -> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3961.0) ----->
 Distance Elevation Manning
 0.00 263.00 0.0410
 11.75 257.14 0.0410
 23.50 253.97 0.0410
 41.13 247.83 0.0410
 76.38 232.09 0.0410
 135.13 229.07 0.0410
 149.82 228.97 0.0410
 152.75 228.96 0.0410 / 0.0300 Main Channel
 154.19 228.73 0.0300 Main Channel
 154.69 228.20 0.0300 Main Channel
 155.69 228.20 0.0300 Main Channel
 156.69 228.20 0.0300 Main Channel
 157.19 228.73 0.0300 Main Channel
 158.63 228.95 0.0300 / 0.0410 Main Channel
 161.57 228.96 0.0410
 164.51 229.71 0.0410
 196.82 241.70 0.0410
 223.26 249.21 0.0410
 246.76 255.13 0.0410
 290.82 263.51 0.0410

<----- TRAVEL TIME TABLE ----->
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
 (m) (m) (cu.m.) (cms) (m/s) (min)
 0.75 228.95 .780E+04 2.2 0.98 58.06
 2.54 230.74 .277E+06 145.1 1.78 31.76
 4.33 232.53 .786E+06 609.1 2.63 21.50
 6.13 234.33 .140E+07 1481.5 3.59 15.77
 7.92 236.12 .207E+07 2677.3 4.39 12.90
 9.71 237.91 .280E+07 4184.8 5.09 11.14
 11.50 239.70 .357E+07 6001.4 5.71 9.93
 13.29 241.49 .441E+07 8129.1 6.27 9.03
 15.09 243.29 .530E+07 10549.4 6.77 8.37
 16.88 245.08 .625E+07 13300.6 7.24 7.83
 18.67 246.87 .726E+07 16396.2 7.68 7.38
 20.46 248.66 .834E+07 19809.8 8.07 7.02
 22.26 250.46 .949E+07 23532.5 8.43 6.72
 24.05 252.25 .107E+08 27641.2 8.77 6.46
 25.84 254.04 .120E+08 32155.2 9.10 6.22
 27.63 255.83 .134E+08 36901.0 9.37 6.05
 29.42 257.62 .149E+08 42147.7 9.64 5.88
 31.22 259.42 .164E+08 48145.1 9.97 5.68
 33.01 261.21 .181E+08 54644.8 10.29 5.51

<---- hydrograph ----> <-pipe / channel->

AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8276) 2579.10 6.12 8.25 5.42 0.80 0.99
 OUTFLOW: ID= 1 (6396) 2579.10 5.54 9.25 5.42 0.79 0.99

 ADD HYD (8274) |
 1 + 2 = 3 |
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0396): 305.21 1.174 7.75 8.98
 + ID2= 2 (6396): 2579.10 5.542 9.25 5.42
 =====
 ID = 3 (8274): 2884.31 6.553 9.25 5.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8270) |
 1 + 2 = 3 |
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8272): 2776.21 5.098 10.25 6.40
 + ID2= 2 (8274): 2884.31 6.553 9.25 5.79
 =====
 ID = 3 (8270): 5660.52 11.498 9.50 6.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 RESERVOIR (5506) |
 IN= 2--> OUT= 1 |
 DT= 15.0 min |
 OUTFLOW STORAGE OUTFLOW STORAGE
 (cms) (ha.m.) (cms) (ha.m.)
 0.0000 0.0000 0.0000 135.6832
 31.1490 24.6697 96.2770 900.4431
 36.8120 37.0045 96.6770 900.4531
 45.3070 86.3439 0.0000 0.0000

AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (8270) 5660.520 11.498 9.50 6.09
 OUTFLOW: ID= 1 (5506) 5660.520 9.572 12.00 6.09

PEAK FLOW REDUCTION [Qout/Qin](%)= 83.25
 TIME SHIFT OF PEAK FLOW (min)=150.00
 MAXIMUM STORAGE USED (ha.m.)= 7.5866

 CALIB (0406) | Area (ha)= 142.65 Curve Number (CN)= 66.0
 NASHYD (0406) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.59

Unit Hyd Opeak (cms)= 4.135

PEAK FLOW (cms)= 0.781 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 7.974
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.190

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB (0402) | Area (ha)= 244.46 Curve Number (CN)= 47.0
 NASHYD (0402) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.98

Unit Hyd Opeak (cms)= 4.280

PEAK FLOW (cms)= 0.467 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 4.188
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.100

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB (0402) | Area (ha)= 244.00 Curve Number (CN)= 61.0
 NASHYD (0402) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.07

Unit Hyd Opeak (cms)= 3.879

PEAK FLOW (cms)= 0.710 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 6.803
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.162

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB (0400) | Area (ha)= 93.97 Curve Number (CN)= 52.0
 NASHYD (0400) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.44

Unit Hyd Opeak (cms)= 3.630

PEAK FLOW (cms)= 0.390 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 4.872
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.116

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB (0398) | Area (ha)= 328.19 Curve Number (CN)= 55.0
 NASHYD (0398) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.83

Unit Hyd Opeak (cms)= 6.759

PEAK FLOW (cms)= 0.938 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 5.517
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.131

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8268) |
 1 + 2 = 3 |
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0398): 328.19 0.938 7.00 5.52
 + ID2= 2 (0400): 93.97 0.390 6.50 4.87
 =====
 ID = 3 (8268): 422.16 1.286 7.00 5.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CN (5402) | Routing time step (min)'= 15.00
 IN= 2--> OUT= 1 |

<<----- DATA FOR SECTION (4021.0) ----->>

Distance	Elevation	Manning
0.00	238.50	0.0360
11.50	238.00	0.0360
23.00	237.93	0.0360
34.49	236.39	0.0360
63.24	233.98	0.0360
97.73	228.15	0.0360
123.60	227.08	0.0360
126.48	226.61	0.0360
127.60	226.47	0.0360 /0.0330 Main Channel
127.85	225.25	0.0330 Main Channel
129.35	225.25	0.0330 Main Channel
130.85	225.25	0.0330 Main Channel
131.10	226.47	0.0330 /0.0360 Main Channel
132.22	226.59	0.0360
143.72	227.42	0.0360
169.59	227.88	0.0360
192.59	231.19	0.0360
218.46	233.02	0.0360
241.45	235.50	0.0360
284.57	236.43	0.0360

----- TRAVEL TIME TABLE ----->>

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.41	225.66	.391E+04	1.7	1.36	38.31
0.81	226.06	.804E+04	5.0	1.93	27.02
1.22	226.47	.124E+05	9.1	2.30	22.61
1.84	227.09	.308E+05	23.3	2.36	22.03
2.46	227.71	.904E+05	62.0	2.14	24.31
3.09	228.34	.223E+06	188.3	2.63	19.75
3.71	228.96	.379E+06	410.2	3.38	15.39
4.33	229.58	.550E+06	711.5	4.04	12.89
4.95	230.20	.737E+06	1091.3	4.62	11.25

5.58	230.83	.939E+06	1550.6	5.15	10.10
6.20	231.45	.116E+07	2072.4	5.59	9.31
6.82	232.07	.140E+07	2659.2	5.93	8.77
7.44	232.69	.167E+07	3345.8	6.27	8.30
8.07	233.32	.195E+07	4159.0	6.64	7.83
8.69	233.94	.226E+07	5100.8	7.03	7.39
9.31	234.56	.259E+07	6081.3	7.32	7.11
9.93	235.18	.295E+07	7183.1	7.60	6.84
10.56	235.81	.334E+07	8345.2	7.62	6.83
11.18	236.43	.379E+07	9116.0	7.51	6.92

	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8268)	422.16	1.29	7.00	5.37	0.31	1.36
OUTFLOW : ID= 1 (6402)	422.16	1.15	7.75	5.37	0.28	1.36

ADD HYD (8266)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0402):	244.00	0.710	7.75	6.80
+ ID2= 2 (6402):	422.16	1.155	7.75	5.37
ID = 3 (8266):	666.16	1.865	7.75	5.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8264)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0404):	246.46	0.467	7.50	4.19
+ ID2= 2 (8266):	666.16	1.865	7.75	5.90
ID = 3 (8264):	912.62	2.329	7.75	5.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8262)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0406):	142.65	0.781	7.75	7.97
+ ID2= 2 (8264):	912.62	2.329	7.75	5.44
ID = 3 (8262):	1055.27	3.000	7.50	5.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8260)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (5506):	5660.52	9.572	12.00	6.09
+ ID2= 2 (8262):	1055.27	3.000	7.50	5.78
ID = 3 (8260):	6715.79	10.967	11.25	6.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8258)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0408):	231.62	0.897	6.75	6.07
+ ID2= 2 (8260):	6715.79	10.967	11.25	6.04
ID = 3 (8258):	6947.41	11.251	11.25	6.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8256)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0410):	572.01	0.844	8.25	4.36
+ ID2= 2 (8258):	6947.41	11.251	11.25	6.04
ID = 3 (8256):	7519.42	11.941	11.00	5.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5507)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IMS 2--> OUT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min	0.0000	0.0000	90.6140	67.8416
	39.6440	12.3348	*****	160.3529
	48.1390	18.5023	*****	160.3629
	67.9600	37.0045	0.0000	0.0000

INFLOW : ID= 2 (8256)	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
7519.421	11.941	11.00	11.00	5.91

OUTFLOW : ID= 1 (5507)	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
7519.421	11.702	12.00	12.00	5.91

PEAK FLOW REDUCTION [Qout/Qin](%)	TIME SHIFT OF PEAK FLOW (min)	MAXIMUM STORAGE USED (ha.m.)
98.00	60.00	3.6414

NASHYD (0420)	Area	(ha)	Curve Number (CN)
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)=	0.81	

Unit Hyd Qpeak	(cms)
3.692	

PEAK FLOW	(cms)
0.476 (1)	
TIME TO PEAK	(hrs)
7.000	
RUNOFF VOLUME	(mm)
5.148	
TOTAL RAINFALL	(mm)
42.000	
RUNOFF COEFFICIENT	=
0.123	

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number (CN)
NASHYD (0418)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min	U.H. Tp(hrs)=	1.05	

Unit Hyd Qpeak	(cms)
2.966	

PEAK FLOW	(cms)
0.600 (1)	
TIME TO PEAK	(hrs)
7.500	
RUNOFF VOLUME	(mm)
7.539	
TOTAL RAINFALL	(mm)
42.000	
RUNOFF COEFFICIENT	=
0.179	

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number (CN)
NASHYD (0416)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min	U.H. Tp(hrs)=	1.29	

Unit Hyd Qpeak	(cms)
5.832	

PEAK FLOW	(cms)
1.251 (1)	
TIME TO PEAK	(hrs)
8.000	
RUNOFF VOLUME	(mm)
7.557	
TOTAL RAINFALL	(mm)
42.000	
RUNOFF COEFFICIENT	=
0.180	

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number (CN)
NASHYD (0412)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min	U.H. Tp(hrs)=	0.80	

Unit Hyd Qpeak	(cms)
5.088	

PEAK FLOW	(cms)
0.677 (1)	
TIME TO PEAK	(hrs)
7.000	
RUNOFF VOLUME	(mm)
5.327	
TOTAL RAINFALL	(mm)
42.000	
RUNOFF COEFFICIENT	=
0.127	

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Dir. Conn.(%)
STANDHYD (4141)	Total Imp(%)	36.00	17.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 15.73 27.97
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 539.75 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 55.44 11.49
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 10.95 (ii) 36.37 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03
 TOTALS
 PEAK FLOW (cms)= 0.94 0.50 1.138 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 40.00 9.81 14.94
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.23 0.36

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5414)
 IN= 2 -> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.4720	1.9177
0.3820	0.3152	3.8720	1.9277
1.8280	0.4690	0.0000	0.0000

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (4141) 43.700 1.138 6.00 14.94
 OUTFLOW : ID= 1 (5414) 43.700 0.329 7.25 14.94

PEAK FLOW REDUCTION [Qout/qin](%)= 28.91
 TIME SHIFT OF PEAK FLOW (min)= 75.00
 MAXIMUM STORAGE USED (ha.m.)= 0.2718

CALIB STANDBYD (4142)
 ID= 1 DT=15.0 min
 Area (ha)= 144.30
 Total Imp(%)= 21.00 Dir. Conn.(%)= 10.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 30.30 114.00
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 980.82 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 55.44 8.83
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 15.67 (ii) 43.91 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.07 0.03
 TOTALS
 PEAK FLOW (cms)= 1.59 1.42 2.138 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 40.00 8.68 11.81
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.21 0.28

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8254)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 1 (4142):	144.30	2.138	6.00	11.81
+ ID2 = 2 (5414):	43.70	0.329	7.25	14.94

ID = 3 (8254):	188.00	2.278	6.00	12.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8252)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 1 (0412):	238.70	0.677	7.00	5.33
+ ID2 = 2 (8254):	188.00	2.278	6.00	12.54

ID = 3 (8252):	426.70	2.814	6.50	8.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6416)
 IN= 2 -> OUT= 1
 Routing time step (min)= 15.00

----- DATA FOR SECTION (4161.0) -----

Distance	Elevation	Manning
0.00	270.07	0.0340
20.67	267.91	0.0340
62.01	264.33	0.0340
113.69	259.75	0.0340
165.37	253.30	0.0340
227.38	246.29	0.0340
232.55	246.03	0.0340
237.72	246.16	0.0340
241.39	246.02	0.0340 / 0.0300
241.64	245.75	0.0300
242.89	245.75	0.0300
244.14	245.75	0.0300
244.39	246.02	0.0300 / 0.0340
248.06	246.20	0.0340
253.22	246.20	0.0340
259.39	246.63	0.0340
346.25	252.57	0.0340
413.43	257.77	0.0340
465.11	261.78	0.0340
511.62	270.00	0.0340

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.27	246.02	.295E+04	0.6	0.75	88.26
1.53	247.28	.168E+06	69.7	1.65	40.13
2.79	248.54	.490E+06	299.5	2.42	27.28
4.06	249.81	.962E+06	740.1	3.05	21.67
5.32	251.07	.158E+07	1440.0	3.61	18.33
6.58	252.33	.235E+07	2443.8	4.12	16.06
7.84	253.59	.327E+07	3811.2	4.63	14.30
9.10	254.85	.432E+07	5565.1	5.11	12.94
10.37	256.12	.550E+07	7721.3	5.57	11.88
11.63	257.38	.682E+07	10309.9	6.00	11.02
12.89	258.64	.827E+07	13360.6	6.42	10.31
14.15	259.90	.985E+07	16884.5	6.81	9.72
15.42	261.17	.116E+08	20818.4	7.14	9.26
16.68	262.43	.134E+08	25510.4	7.53	8.78
17.94	263.69	.154E+08	30958.0	7.97	8.30
19.20	264.95	.175E+08	36988.3	8.38	7.89
20.46	266.21	.197E+08	43617.1	8.78	7.53
21.73	267.48	.220E+08	50868.1	9.17	7.22
22.99	268.74	.244E+08	58854.0	9.56	6.92

hydrograph
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8252) 426.70 2.81 6.50 8.50 0.31 0.76
 OUTFLOW : ID= 1 (6416) 426.70 1.66 7.75 8.50 0.29 0.76

ADD HYD (8250)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 1 (0416):	439.30	1.251	8.00	7.56
+ ID2 = 2 (6416):	426.70	1.663	7.75	8.50

ID = 3 (8250):	866.00	2.906	8.00	8.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8248)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 1 (0418):	182.79	0.600	7.50	7.54
+ ID2 = 2 (8250):	866.00	2.906	8.00	8.02

ID = 3 (8248):	1048.79	3.503	8.00	7.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8246)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8242):	176.82	0.476	7.00	5.15
+ ID2= 2 (8248):	1048.79	3.503	8.00	7.94
=====				
ID = 3 (8246):	1224.61	3.958	7.75	7.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8244)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (5507):	7519.42	11.702	12.00	5.91
+ ID2= 2 (8246):	1224.61	3.958	7.75	7.54
=====				
ID = 3 (8244):	8744.03	13.941	11.25	6.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8242)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8242):	780.20	1.144	9.50	5.38
+ ID2= 2 (8244):	8744.03	13.941	11.25	6.14
=====				
ID = 3 (8242):	9524.23	15.007	11.00	6.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5508)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	*****	197.3574
	76.4550	30.8371	*****	394.7148
	*****	61.5742	*****	394.7248
	*****	*****	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8242)	9524.231	15.007	11.00	6.08
OUTFLOW: ID= 1 (5508)	9524.231	14.571	12.25	6.08

PEAK FLOW REDUCTION [Qout/Qin](%)= 97.09
 TIME SHIFT OF PEAK FLOW (min)= 75.00
 MAXIMUM STORAGE USED (ha.m.)= 5.8793

CALIB				
NASHYD (0336)				
ID= 1 DT=15.0 min	Area (ha)	(ha)=2785.00	Curve Number (CN)= 72.0	
	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50	
	U.H. Tp(hrs)=	15.39		

Unit Hyd Qpeak (cms)= 3.090

PEAK FLOW (cms)= 1.223 (i)
 TIME TO PEAK (hrs)= 22.500
 RINFLOW VOLUME (mm)= 10.068
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.240

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB				
STANDHYD (3382)				
ID= 1 DT=15.0 min	Area (ha)	(ha)= 525.90	Dir. Conn.(%)= 17.00	
	Total Imp(%)=	37.00		

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 194.58 331.32
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1872.43 40.00
 Mannings n = 0.013 0.250
 Max. Ref. Inten.(mm/hr)= 37.80 12.71
 over (min) 30.00 60.00
 Storage Coeff. (min)= 26.92 (ii) 51.33 (ii)
 Unit Hyd. Tpeak (min)= 30.00 60.00

Unit Hyd. peak (cms)= 0.04 0.02 *TOTALS*
 PEAK FLOW (cms)= 6.48 5.14 9.230 (iii)
 TIME TO PEAK (hrs)= 6.25 6.75 6.25
 RINFLOW VOLUME (mm)= 40.00 10.61 15.61
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.25 0.37

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB			
STANDHYD (3381)			
ID= 1 DT=15.0 min	Area (ha)	(ha)= 73.00	Dir. Conn.(%)= 23.00
	Total Imp(%)=	49.00	

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 35.77 37.23
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 697.61 40.00
 Mannings n = 0.013 0.250

Max. Ref. Inten.(mm/hr)= 55.44 16.56
 over (min) 15.00 45.00
 Storage Coeff. (min)= 12.77 (ii) 34.73 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03

TOTALS
 PEAK FLOW (cms)= 2.01 0.99 2.410 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RINFLOW VOLUME (mm)= 40.00 11.94 18.39
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.28 0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5338)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	3.3350	1.2400
	0.3230	0.3074	4.3560	1.3520
	0.9360	0.5374	5.3290	1.4590
	1.7590	0.8302	5.7290	1.4690
	2.4110	1.0550	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (3381)	73.000	2.410	6.00	18.39
OUTFLOW: ID= 1 (5338)	73.000	0.897	7.00	18.39

PEAK FLOW REDUCTION [Qout/Qin](%)= 37.23
 TIME SHIFT OF PEAK FLOW (min)= 60.00
 MAXIMUM STORAGE USED (ha.m.)= 0.5258

ADD HYD (8310)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (3382):	525.90	9.230	6.25	15.61
+ ID2= 2 (5338):	73.00	0.897	7.00	18.39
=====				
ID = 3 (8310):	598.90	9.798	6.25	15.95

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB				
NASHYD (3342)				
ID= 1 DT=15.0 min	Area (ha)	(ha)= 587.10	Curve Number (CN)= 56.0	
	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50	
	U.H. Tp(hrs)=	8.19		

Unit Hyd Qpeak (cms)= 1.224

PEAK FLOW (cms)= 0.275 (i)
 TIME TO PEAK (hrs)= 15.500

RUNOFF VOLUME (mm)= 5.784
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.138

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (3341)		Area (ha)= 33.00	Dir. Conn.(%)= 31.00	
STANDHYD (3341)		Area (ha)= 33.00	Dir. Conn.(%)= 31.00	
ID= 1 DT=15.0 min		Total Imp(%)= 51.00	Dir. Conn.(%)= 31.00	
		IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	16.83	16.17		
Dep. Storage (mm)=	2.00	5.00		
Average Slope (%)=	0.50	0.50		
Length (m)=	469.04	40.00		
Mannings n =	0.013	0.250		
Max.Eff.Inten.(mm/hr)=	55.44	9.00		
over (min)=	15.00	45.00		
Storage Coeff. (min)=	10.07 (ii)	38.09 (ii)		
Unit Hyd. Tpeak (min)=	15.00	45.00		
Unit Hyd. peak (cms)=	0.09	0.03		
TOTALS				
PEAK FLOW (cms)=	1.33	0.22	1.415 (iii)	
TIME TO PEAK (hrs)=	6.00	6.50	6.00	
RUNOFF VOLUME (mm)=	40.00	7.21	17.38	
TOTAL RAINFALL (mm)=	42.00	42.00	42.00	
RUNOFF COEFFICIENT =	0.95	0.17	0.41	

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 52.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5334)		OUTFLOW		STORAGE		OUTFLOW		STORAGE	
IIS 2--> OUT= 1		(cms)	(ha.m.)	(cms)	(ha.m.)	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min									
		0.0000	0.0000	0.6540	0.4604				
		0.0390	0.1699	0.8330	0.5249				
		0.1860	0.2610	1.0130	0.5896				
		0.3750	0.3534	1.4130	0.5996				
		0.4340	0.3813	0.0000	0.0000				
		AREA	QPEAK	TPEAK	R.V.				
		(ha)	(cms)	(hrs)	(mm)				
INFLOW : ID= 2 (3341)	33.000	1.415	6.00	17.38					
OUTFLOW: ID= 1 (5334)	33.000	0.280	7.00	17.34					
		PEAK FLOW REDUCTION [Qout/Qin](%)=	19.76						
		TIME SHIFT OF PEAK FLOW (min)=	60.00						
		MAXIMUM STORAGE USED (ha.m.)=	0.3077						

ADD HYD (8314)		AREA		QPEAK	TPEAK	R.V.
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (3342):	587.10	0.275	15.50	5.78		
+ ID2= 2 (5334):	33.00	0.280	7.00	17.34		
ID = 3 (8314):	620.10	0.389	7.25	6.40		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0306)		Area (ha)= 283.97	Curve Number (CN)= 52.0
NASHYD (0306)		Area (ha)= 283.97	Curve Number (CN)= 52.0
ID= 1 DT=15.0 min		Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
		U.H. Tp(hrs)= 6.44	
Unit Hyd Qpeak (cms)=	0.753		
PEAK FLOW (cms)=	0.146 (i)		
TIME TO PEAK (hrs)=	14.000		
RUNOFF VOLUME (mm)=	5.040		
TOTAL RAINFALL (mm)=	42.000		
RUNOFF COEFFICIENT =	0.120		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0286)		Area (ha)= 260.51	Curve Number (CN)= 84.0
NASHYD (0286)		Area (ha)= 260.51 <th>Curve Number (CN)= 84.0</th>	Curve Number (CN)= 84.0
ID= 1 DT=15.0 min		Ia (mm)= 5.00	# of Linear Res.(N)= 1.50

		U.H. Tp(hrs)= 1.16	
Unit Hyd Qpeak (cms)=	3.834		
PEAK FLOW (cms)=	1.732 (i)		
TIME TO PEAK (hrs)=	7.750		
RUNOFF VOLUME (mm)=	15.903		
TOTAL RAINFALL (mm)=	42.000		
RUNOFF COEFFICIENT =	0.379		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0282)		Area (ha)= 449.38	Curve Number (CN)= 77.0
NASHYD (0282)		Area (ha)= 449.38 <th>Curve Number (CN)= 77.0</th>	Curve Number (CN)= 77.0
ID= 1 DT=15.0 min		Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
		U.H. Tp(hrs)= 1.47	
Unit Hyd Qpeak (cms)=	5.226		
PEAK FLOW (cms)=	1.868 (i)		
TIME TO PEAK (hrs)=	8.250		
RUNOFF VOLUME (mm)=	12.059		
TOTAL RAINFALL (mm)=	42.000		
RUNOFF COEFFICIENT =	0.287		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0284)		Area (ha)= 78.93	Curve Number (CN)= 84.0
NASHYD (0284)		Area (ha)= 78.93 <th>Curve Number (CN)= 84.0</th>	Curve Number (CN)= 84.0
ID= 1 DT=15.0 min		Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
		U.H. Tp(hrs)= 0.57	
Unit Hyd Qpeak (cms)=	2.344		
PEAK FLOW (cms)=	0.907 (i)		
TIME TO PEAK (hrs)=	6.500		
RUNOFF VOLUME (mm)=	15.663		
TOTAL RAINFALL (mm)=	42.000		
RUNOFF COEFFICIENT =	0.373		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8388)		AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0282):	449.38	1.868	8.25	12.06	
+ ID2= 2 (0284):	78.93	0.907	6.50	15.66	
ID = 3 (8388):	528.31	2.560	7.50	12.60	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6286)		Routing time step (min)'= 15.00		
IN= 2--> OUT= 1				

DATA FOR SECTION (2861.0) ----->				
Distance	Elevation	Manning		
0.00	233.00	0.0450		
20.58	228.51	0.0450		
41.17	227.74	0.0450		
51.46	227.41	0.0450		
97.77	225.96	0.0450		
149.23	223.94	0.0450		
200.69	220.84	0.0450		
226.42	220.66	0.0450		
238.85	220.22	0.0450		
241.35	220.01	0.0450 / 0.0350	Main Channel	
241.85	219.70	0.0350	Main Channel	
245.85	219.72	0.0350	Main Channel	
246.35	220.06	0.0350	Main Channel	
248.85	220.23	0.0350	Main Channel	
303.60	221.64	0.0350	Main Channel	
380.79	224.98	0.0450		
432.25	229.54	0.0450		
457.98	233.33	0.0450		
483.71	234.27	0.0450		
509.44	233.81	0.0450		

TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.31	220.01	.382E+04	0.8	0.57	82.89
1.06	220.76	.533E+05	15.1	0.66	72.51
1.81	221.51	.240E+06	88.6	1.05	45.21
2.56	222.26	.510E+06	261.7	1.46	32.47
3.31	223.01	.843E+06	533.8	1.81	26.34

4.06	223.76	.124E+07	913.6	2.10	22.64
4.81	224.51	.171E+07	1399.0	2.34	20.33
5.56	225.26	.225E+07	2051.0	2.60	18.27
6.31	226.01	.285E+07	2907.0	2.91	16.35
7.06	226.76	.352E+07	3902.5	3.16	15.04
7.82	227.52	.426E+07	5059.9	3.39	14.01
8.57	228.27	.507E+07	6433.0	3.62	13.13
9.32	229.02	.593E+07	8069.2	3.89	12.24
10.07	229.77	.681E+07	9926.7	4.16	11.44
10.82	230.52	.772E+07	11962.8	4.42	10.75
11.57	231.27	.864E+07	14167.1	4.68	10.17
12.32	232.02	.959E+07	16537.1	4.92	9.66
13.07	232.77	.105E+08	19070.7	5.16	9.22
13.82	233.52	.115E+08	21790.4	5.39	8.82

```

<---- hydrograph ----> <-pipe / channel->
AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
(ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
INFLOW : ID= 2 (8388) 528.31  2.56  7.50 12.60  0.43  0.59
OUTFLOW: ID= 1 (6286) 528.31  2.18  9.25 12.60  0.40  0.58

```

```

ADD HYD (8386)
1 + 2 = 3
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0286): 260.51  1.732  7.75  15.90
+ ID2= 2 (6286): 528.31  2.176  9.25  12.60
=====
ID = 3 (8386): 788.82  3.749  8.50  13.69

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB
NASHYD (0302) Area (ha)= 473.90 Curve Number (CN)= 58.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.66

```

Unit Hyd Qpeak (cms)= 4.874

```

PEAK FLOW (cms)= 0.899 (i)
TIME TO PEAK (hrs)= 8.750
RUNOFF VOLUME (mm)= 6.167
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.147

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0300) Area (ha)= 258.93 Curve Number (CN)= 52.0
ID= 1 DT=15.0 min Ia (mm)= 4.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.03

```

Unit Hyd Qpeak (cms)= 4.292

```

PEAK FLOW (cms)= 0.599 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 5.248
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.125

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0298) Area (ha)= 330.51 Curve Number (CN)= 45.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.26

```

Unit Hyd Qpeak (cms)= 4.461

```

PEAK FLOW (cms)= 0.488 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 3.912
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.093

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8395)
1 + 2 = 3
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0298): 330.51  0.488  8.00  3.91
+ ID2= 2 (0300): 258.93  0.599  7.50  5.25
=====

```

ID = 3 (8395): 589.44 1.081 8.00 4.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB
NASHYD (0288) Area (ha)= 340.83 Curve Number (CN)= 78.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 2.21

```

Unit Hyd Qpeak (cms)= 2.629

PEAK FLOW (cms)= 1.075 (i)

TIME TO PEAK (hrs)= 9.750

RUNOFF VOLUME (mm)= 12.562

TOTAL RAINFALL (mm)= 42.000

RUNOFF COEFFICIENT = 0.299

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0290) Area (ha)= 269.18 Curve Number (CN)= 78.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.07

```

Unit Hyd Qpeak (cms)= 4.279

PEAK FLOW (cms)= 1.470 (i)

TIME TO PEAK (hrs)= 7.500

RUNOFF VOLUME (mm)= 12.485

TOTAL RAINFALL (mm)= 42.000

RUNOFF COEFFICIENT = 0.297

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8397)
1 + 2 = 3
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0288): 340.83  1.075  9.75  12.56
+ ID2= 2 (0290): 269.18  1.470  7.50  12.49
=====
ID = 3 (8397): 610.01  2.456  8.00  12.53

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB
NASHYD (0296) Area (ha)= 293.65 Curve Number (CN)= 76.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.13

```

Unit Hyd Qpeak (cms)= 4.437

PEAK FLOW (cms)= 1.427 (i)

TIME TO PEAK (hrs)= 7.750

RUNOFF VOLUME (mm)= 11.580

TOTAL RAINFALL (mm)= 42.000

RUNOFF COEFFICIENT = 0.276

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0292) Area (ha)= 738.49 Curve Number (CN)= 68.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.52

```

Unit Hyd Qpeak (cms)= 8.289

PEAK FLOW (cms)= 2.131 (i)

TIME TO PEAK (hrs)= 8.250

RUNOFF VOLUME (mm)= 8.698

TOTAL RAINFALL (mm)= 42.000

RUNOFF COEFFICIENT = 0.207

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0294) Area (ha)= 274.15 Curve Number (CN)= 57.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.87

```

Unit Hyd Qpeak (cms)= 5.367

PEAK FLOW (cms)= 0.809 (i)

TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 5.913
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.141

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8398)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0292):	738.49	2.131	8.25	8.70
+ ID2= 2 (0294):	274.15	0.809	7.25	5.91
=====				
ID = 3 (8398):	1012.64	2.892	8.00	7.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6296)
 IN= 2--> OUT= 1
 Routing time step (min)= 15.00

----- DATA FOR SECTION (2961.0) -----

Distance	Elevation	Manning	
0.00	243.98	0.0400	
42.59	243.18	0.0400	
85.17	241.81	0.0400	
120.66	240.50	0.0400	
156.15	239.56	0.0400	
198.74	236.15	0.0400	
237.78	234.01	0.0400	
241.33	233.82	0.0400	
248.77	233.12	0.0400 / 0.0400	Main Channel
249.87	232.32	0.0400	Main Channel
250.37	231.80	0.0400	Main Channel
250.87	232.23	0.0400	Main Channel
251.97	233.10	0.0400 / 0.0400	Main Channel
255.37	233.22	0.0400	
259.07	233.87	0.0400	
262.62	234.12	0.0400	
266.17	234.23	0.0400	
283.91	234.73	0.0400	
337.15	241.75	0.0400	
351.34	244.00	0.0400	

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.43	232.23	.510E+03	0.1	0.53	81.07
0.87	232.67	.213E+04	0.7	0.83	51.49
1.30	233.10	.501E+04	2.2	1.10	38.75
1.98	233.78	.242E+05	12.6	1.34	31.93
2.66	234.46	.738E+05	42.1	1.46	29.21
3.34	235.14	.177E+06	127.9	1.84	23.13
4.02	235.82	.314E+06	281.4	2.29	18.61
4.70	236.50	.481E+06	509.4	2.71	15.73
5.38	237.18	.672E+06	815.6	3.11	13.73
6.06	237.86	.886E+06	1200.7	3.46	12.30
6.74	238.54	.112E+07	1669.4	3.80	11.23
7.42	239.22	.139E+07	2226.9	4.11	10.38
8.10	239.90	.168E+07	2808.4	4.28	9.95
8.78	240.58	.202E+07	3470.4	4.40	9.68
9.46	241.26	.240E+07	4331.7	4.61	9.24
10.14	241.94	.283E+07	5335.2	4.83	8.84
10.82	242.62	.330E+07	6482.6	5.03	8.48
11.50	243.30	.381E+07	7764.1	5.21	8.18
12.18	243.98	.439E+07	9101.2	5.31	8.04

----- hydrograph -----> <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8398)	1012.64	2.89	8.00	7.94	1.35	1.11
OUTFLOW: ID= 1 (6296)	1012.64	2.79	8.75	7.94	1.34	1.11

ADD HYD (8396)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0296):	293.65	1.427	7.75	11.58
+ ID2= 2 (6296):	1012.64	2.793	8.75	7.94
=====				
ID = 3 (8396):	1306.29	4.146	8.50	8.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8394)

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8396):	1306.29	4.146	8.50	8.76
+ ID2= 2 (8397):	610.01	2.456	8.00	12.53
=====				
ID = 3 (8394):	1916.30	6.591	8.25	9.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8392)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8394):	1916.30	6.591	8.25	9.96
+ ID2= 2 (8393):	589.44	1.081	8.00	4.50
=====				
ID = 3 (8392):	2505.74	7.655	8.25	8.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6302)
 IN= 2--> OUT= 1
 Routing time step (min)= 15.00

----- DATA FOR SECTION (3021.0) ----->

Distance	Elevation	Manning	
0.00	228.10	0.0400	
18.47	227.12	0.0400	
36.95	226.12	0.0400	
46.18	225.84	0.0400	
55.42	225.58	0.0400	
272.47	222.88	0.0400	
277.09	222.76	0.0400	
281.71	222.58	0.0400	
288.54	222.18	0.0400 / 0.0300	Main Channel
288.64	221.00	0.0300	Main Channel
290.94	221.00	0.0300	Main Channel
291.04	221.00	0.0300	Main Channel
291.54	222.75	0.0300 / 0.0400	Main Channel
300.18	222.83	0.0400	
304.80	223.04	0.0400	
309.42	223.25	0.0400	
318.65	223.69	0.0400	
360.22	225.57	0.0400	
397.16	227.60	0.0400	
457.20	228.35	0.0400	

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.29	221.29	.228E+04	0.2	0.32	165.63
0.59	221.59	.465E+04	0.7	0.45	116.15
0.88	221.88	.713E+04	1.2	0.54	96.87
1.18	222.18	.971E+04	1.9	0.61	86.20
1.57	222.57	.175E+05	3.3	0.59	88.86
1.97	222.97	.427E+05	6.4	0.47	111.39
2.36	223.36	.115E+06	16.1	0.44	119.14
2.76	223.76	.237E+06	37.1	0.49	106.49
3.15	224.15	.409E+06	72.9	0.56	93.40
3.55	224.55	.631E+06	127.0	0.63	82.76
3.94	224.94	.903E+06	202.4	0.70	74.37
4.34	225.34	.123E+07	301.8	0.77	67.68
4.73	225.73	.160E+07	433.5	0.85	61.37
5.13	226.13	.200E+07	602.2	0.95	55.29
5.52	226.52	.242E+07	804.7	1.05	50.13
5.92	226.92	.288E+07	1034.2	1.14	46.11
6.31	227.31	.332E+07	1290.5	1.22	42.88
6.71	227.71	.380E+07	1560.9	1.29	40.55
7.10	228.10	.432E+07	1835.6	1.34	39.19

----- hydrograph -----> <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8392)	2505.74	7.66	8.25	8.68	2.02	0.47
OUTFLOW: ID= 1 (6302)	2505.74	6.18	10.50	8.68	1.94	0.48

ADD HYD (8390)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0302):	473.90	0.899	8.75	6.17
+ ID2= 2 (6302):	2505.74	6.182	10.50	8.68
=====				
ID = 3 (8390):	2979.64	7.014	10.50	8.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8348) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID= 1 (8345): 788.62 3.749 8.50 13.69 |
| + ID2= 2 (8390): 2979.64 7.014 10.50 8.28 |
|=====|
| ID = 3 (8348): 3768.46 10.463 9.75 9.41 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0304) | Area (ha)= 292.37 Curve Number (CN)= 63.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|=====|
| U.H. Tp(hrs)= 2.78 |
-----

```

Unit Hyd Qpeak (cms)= 1.793

```

PEAK FLOW (cms)= 0.450 (1)
TIME TO PEAK (hrs)= 10.500
RUNOFF VOLUME (mm)= 7.337
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.175

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0280) | Area (ha)= 299.86 Curve Number (CN)= 82.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|=====|
| U.H. Tp(hrs)= 0.85 |
-----

```

Unit Hyd Qpeak (cms)= 6.009

```

PEAK FLOW (cms)= 2.320 (1)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 14.568
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.347

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0278) | Area (ha)= 485.49 Curve Number (CN)= 82.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|=====|
| U.H. Tp(hrs)= 1.82 |
-----

```

Unit Hyd Qpeak (cms)= 5.453

```

PEAK FLOW (cms)= 2.409 (1)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 14.678
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.349

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0276) | Area (ha)= 90.89 Curve Number (CN)= 79.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|=====|
| U.H. Tp(hrs)= 0.67 |
-----

```

Unit Hyd Qpeak (cms)= 2.302

```

PEAK FLOW (cms)= 0.742 (1)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 12.858
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.306

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0274) | Area (ha)= 392.49 Curve Number (CN)= 75.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|=====|
| U.H. Tp(hrs)= 1.08 |
-----

```

Unit Hyd Qpeak (cms)= 6.182

```

PEAK FLOW (cms)= 1.889 (1)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 11.150
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.265

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8360) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0274): 392.49 1.889 7.50 11.15 |
| + ID2= 2 (0276): 90.89 0.742 6.75 12.86 |
|=====|
| ID = 3 (8360): 483.38 2.585 7.25 11.47 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8358) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0278): 485.49 2.409 8.25 14.68 |
| + ID2= 2 (8360): 483.38 2.585 7.25 11.47 |
|=====|
| ID = 3 (8358): 968.87 4.901 7.75 13.08 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6280) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00
-----

```

<----- DATA FOR SECTION (2801.0) ----->						
Distance	Elevation	Manning				
0.00	241.14	0.0500				
13.32	240.80	0.0500				
39.95	240.07	0.0500				
96.54	236.21	0.0500				
113.19	234.15	0.0500				
123.18	232.35	0.0500				
143.15	225.80	0.0500				
149.81	225.62	0.0500				
153.14	225.40	0.0500				
157.30	224.76	0.0500 / 0.0300	Main Channel			
159.80	224.26	0.0300	Main Channel			
162.30	224.85	0.0300	Main Channel			
162.85	224.97	0.0300 / 0.0500	Main Channel			
167.80	225.05	0.0500				
186.43	229.14	0.0500				
213.06	234.75	0.0500				
236.37	237.09	0.0500				
266.33	237.31	0.0500				
292.96	237.83	0.0500				
329.58	241.50	0.0500				

<----- TRAVEL TIME TABLE ----->						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.50	224.76	.263E+04	0.8	0.65	58.31	
1.36	225.62	.278E+05	13.4	1.10	34.54	
2.22	226.48	.855E+05	52.6	1.40	27.09	
3.09	227.35	.157E+06	120.4	1.74	21.78	
3.95	228.21	.242E+06	216.3	2.04	18.64	
4.81	229.07	.340E+06	341.8	2.29	16.56	
5.67	229.93	.450E+06	498.3	2.52	15.06	
6.53	230.79	.574E+06	688.3	2.73	13.90	
7.40	231.66	.711E+06	913.8	2.93	12.97	
8.26	232.52	.861E+06	1173.4	3.11	12.23	
9.12	233.38	.103E+07	1461.3	3.24	11.72	
9.98	234.24	.121E+07	1793.3	3.37	11.26	
10.85	235.11	.142E+07	2141.0	3.45	11.02	
11.71	235.97	.165E+07	2530.4	3.50	10.86	
12.57	236.83	.192E+07	2954.0	3.51	10.81	
13.43	237.69	.226E+07	3160.3	3.18	11.93	
14.29	238.55	.271E+07	3869.4	3.26	11.67	
15.16	239.42	.320E+07	4740.3	3.38	11.24	
16.02	240.28	.373E+07	5686.4	3.48	10.92	

```

-----
|<---- hydrograph ----> |<-pipe / channel->|
| AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL |
| (ha) (cms) (hrs) (mm) (m) (m/s) |
| INFLOW : ID= 2 (8358) 968.87 4.90 7.75 13.08 0.78 0.75 |
| OUTFLOW: ID= 1 (6280) 968.87 4.56 8.75 13.08 0.76 0.74 |
-----

```

```

-----
| ADD HYD (8354) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0280): 299.86 2.320 7.00 14.57 |
| + ID2= 2 (6280): 968.87 4.564 8.75 13.08 |
|=====|
| ID = 3 (8354): 1268.73 6.519 8.25 13.43 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHYD (0272) Area (ha)= 157.38 Curve Number (CN)= 75.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.09

Unit Hyd Qpeak (cms)= 2.456

PEAK FLOW (cms)= 0.753 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 11.152
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.266

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0270) Area (ha)= 243.61 Curve Number (CN)= 81.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
U.H. Tp(hrs)= 0.87

Unit Hyd Qpeak (cms)= 3.429

PEAK FLOW (cms)= 1.320 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 13.984
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.333

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0268) Area (ha)= 215.76 Curve Number (CN)= 75.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
U.H. Tp(hrs)= 0.69

Unit Hyd Qpeak (cms)= 3.807

PEAK FLOW (cms)= 1.091 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 11.050
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.263

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0264) Area (ha)= 353.96 Curve Number (CN)= 69.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
U.H. Tp(hrs)= 1.30

Unit Hyd Qpeak (cms)= 3.313

PEAK FLOW (cms)= 0.888 (i)
TIME TO PEAK (hrs)= 8.500
RUNOFF VOLUME (mm)= 8.987
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.214

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0266) Area (ha)= 508.09 Curve Number (CN)= 64.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
U.H. Tp(hrs)= 1.63

Unit Hyd Qpeak (cms)= 3.786

PEAK FLOW (cms)= 0.901 (i)
TIME TO PEAK (hrs)= 9.750
RUNOFF VOLUME (mm)= 7.566
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.180

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8380)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0264): 353.96 0.888 8.50 8.99

+ ID2= 2 (0266): 508.09 0.901 9.75 7.57
ID = 3 (8380): 862.05 1.777 9.00 8.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (0266) |
IN= 2---> OUT= 1 | Routing time step (min)'= 15.00

Table with 4 columns: Distance, Elevation, Manning, and Main Channel. Rows show data points for section 2681.0.

TRAVEL TIME TABLE

Table with 6 columns: DEPTH (m), ELEV (m), VOLUME (cu.m.), FLOW RATE (cms), VELOCITY (m/s), TRAV.TIME (min). Rows show data points for section 2681.0.

Table with 6 columns: AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm), MAX DEPTH (m), MAX VEL (m/s). Rows show INFLOW and OUTFLOW data.

ADD HYD (8382)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0268): 215.76 1.091 7.00 11.05
+ ID2= 2 (0268): 862.05 1.731 10.25 8.15
ID = 3 (8382): 1077.81 2.539 9.00 8.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (0270) |
IN= 2---> OUT= 1 | Routing time step (min)'= 15.00

Table with 4 columns: Distance, Elevation, Manning, and Main Channel. Rows show data points for section 2701.0.

69.68	233.95	0.0300	Main Channel
71.21	234.48	0.0300 / 0.0500	Main Channel
81.38	236.44	0.0500	
95.63	236.66	0.0500	
120.04	237.00	0.0500	
148.53	241.77	0.0500	
158.70	242.34	0.0500	
187.18	244.03	0.0500	
201.42	244.36	0.0500	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.45	234.34	.217E+04	0.7	0.99	52.83
0.98	234.87	.966E+04	4.6	1.49	35.20
1.50	235.39	.250E+05	13.7	1.73	30.41
2.03	235.92	.473E+05	29.5	1.95	26.87
2.56	236.45	.772E+05	52.5	2.14	24.50
3.09	236.98	.142E+06	82.4	1.83	28.73
3.61	237.50	.246E+06	156.1	2.03	25.91
4.14	238.03	.357E+06	262.0	2.31	22.73
4.67	238.56	.477E+06	392.1	2.59	20.27
5.20	239.09	.604E+06	547.8	2.86	18.38
5.72	239.61	.739E+06	729.2	3.11	16.90
6.25	240.14	.882E+06	936.5	3.34	15.70
6.78	240.67	.103E+07	1169.7	3.57	14.72
7.31	241.20	.119E+07	1425.9	3.77	13.94
7.83	241.72	.136E+07	1710.4	3.96	13.26
8.36	242.25	.154E+07	1967.9	4.02	13.07
8.89	242.78	.174E+07	2264.9	4.09	12.83
9.42	243.31	.196E+07	2589.3	4.15	12.65
9.94	243.83	.221E+07	2953.5	4.22	12.46

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8382) 1077.81	2.54	9.00	8.73	0.70	1.18
OUTFLOW : ID= 1 (6270) 1077.81	2.49	9.75	8.73	0.69	1.18

ADD HYD (8384)

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0270):	243.61	1.320	7.50	13.98
+ ID2= 2 (6270):	1077.81	2.492	9.75	8.73
ID = 3 (8384):	1321.42	3.620	9.25	9.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB

NASHYD (0262)	Area (ha)= 341.31	Curve Number (CN)= 82.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.30
	U.H. Tp(hrs)= 1.01	

Unit Hyd Qpeak (cms)= 4.094

PEAK FLOW (cms)= 1.720 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 14.597
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.348

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

NASHYD (0260)	Area (ha)= 476.24	Curve Number (CN)= 82.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.30
	U.H. Tp(hrs)= 1.33	

Unit Hyd Qpeak (cms)= 4.360

PEAK FLOW (cms)= 1.943 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 14.645
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.349

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

NASHYD (0258)	Area (ha)= 181.99	Curve Number (CN)= 79.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.30
	U.H. Tp(hrs)= 1.18	

Unit Hyd Qpeak (cms)= 1.881

PEAK FLOW (cms)= 0.722 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 12.980
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.309

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

NASHYD (0252)	Area (ha)= 319.99	Curve Number (CN)= 73.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.30
	U.H. Tp(hrs)= 1.04	

Unit Hyd Qpeak (cms)= 3.761

PEAK FLOW (cms)= 1.110 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 10.343
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.246

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

NASHYD (0256)	Area (ha)= 145.79	Curve Number (CN)= 67.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.30
	U.H. Tp(hrs)= 1.04	

Unit Hyd Qpeak (cms)= 1.707

PEAK FLOW (cms)= 0.405 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 8.355
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.199

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

NASHYD (0254)	Area (ha)= 403.00	Curve Number (CN)= 55.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.30
	U.H. Tp(hrs)= 1.22	

Unit Hyd Qpeak (cms)= 4.028

PEAK FLOW (cms)= 0.650 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 5.543
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.132

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6256)

IN= 2--> OUT= 1 Routing time step (min)= 15.00

----- DATA FOR SECTION (2561.0) -----

Distance	Elevation	Manning
0.00	276.07	0.0400
11.68	273.71	0.0400
23.36	271.35	0.0400
29.19	270.30	0.0400
35.03	269.44	0.0400
55.47	267.90	0.0400
78.82	266.24	0.0400
90.50	265.63	0.0400
102.18	265.40	0.0400
105.10	264.95	0.0400 / 0.0350
108.02	264.39	0.0350
110.94	264.72	0.0350
113.86	265.19	0.0350 / 0.0400
116.78	265.49	0.0400
143.05	268.24	0.0400
172.25	270.53	0.0400
207.28	271.95	0.0400
233.55	273.72	0.0400
256.91	274.98	0.0400
289.02	275.97	0.0400

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.56	264.95	.723E+04	1.9	0.90	62.51
1.14	265.53	.307E+05	13.5	1.49	37.92
1.72	266.11	.951E+05	48.6	1.73	32.62
2.30	266.69	.191E+06	117.8	2.09	27.03

2.88	267.27	.314E+06	225.2	2.43	23.24
3.46	267.85	.464E+06	375.5	2.74	20.59
4.04	268.43	.641E+06	573.2	3.03	18.62
4.62	269.01	.846E+06	821.9	3.29	17.15
5.20	269.59	.108E+07	1135.0	3.56	15.87
5.78	270.17	.134E+07	1525.1	3.86	14.64
6.36	270.75	.162E+07	1971.1	4.12	13.71
6.94	271.33	.194E+07	2474.0	4.33	13.04
7.52	271.91	.238E+07	3065.0	4.55	12.42
8.10	272.49	.286E+07	3773.4	4.80	11.75
8.68	273.07	.306E+07	4566.3	5.05	11.17
9.26	273.65	.348E+07	5443.7	5.30	10.66
9.84	274.23	.393E+07	6389.6	5.51	10.25
10.42	274.81	.440E+07	7426.2	5.72	9.88
11.00	275.39	.490E+07	8490.1	5.86	9.63

<---- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0254) 403.00 0.65 8.25 5.54 0.19 0.90
 OUTFLOW : ID= 1 (6256) 403.00 0.62 10.00 5.54 0.18 0.90

ADD HYD (8370)
 1 + 2 = 3
 ID1= 1 (0256): 145.79 0.405 8.00 8.36
 + ID2= 2 (6256): 403.00 0.617 10.00 5.54
 ID = 3 (8370): 548.79 0.986 9.50 6.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0250) Area (ha)= 192.88 Curve Number (CN)= 70.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.22

Unit Hyd Qpeak (cms)= 1.930

PEAK FLOW (cms)= 0.529 (1)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 9.304
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.222

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0246) Area (ha)= 759.61 Curve Number (CN)= 55.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.81

Unit Hyd Qpeak (cms)= 5.121

PEAK FLOW (cms)= 0.922 (1)
 TIME TO PEAK (hrs)= 10.000
 RUNOFF VOLUME (mm)= 5.563
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.132

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0248) Area (ha)= 146.04 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 0.78

Unit Hyd Qpeak (cms)= 2.271

PEAK FLOW (cms)= 0.446 (1)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 7.494
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.178

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8364)
 1 + 2 = 3
 ID1= 1 (0246): 759.61 0.922 10.00 5.56
 + ID2= 2 (0248): 146.04 0.446 7.50 7.49

ROUTE CHN (6250)
 ID = 3 (8364): 905.65 1.303 8.50 5.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6250)
 ID= 2 -> OUTF= 1 Routing time step (min)= 15.00

DATA FOR SECTION (2501.0)

Distance	Elevation	Manning
0.00	269.96	0.0500
8.56	268.55	0.0500
17.13	266.91	0.0500
21.41	266.13	0.0500
40.68	263.15	0.0500
62.09	260.75	0.0500
85.64	258.02	0.0500
88.20	257.69	0.0500
93.20	257.05	0.0500 / 0.0350
93.45	256.88	0.0350
94.20	256.56	0.0350
94.95	256.83	0.0350
95.20	257.08	0.0350 / 0.0500
100.62	257.45	0.0500
115.61	258.57	0.0500
139.16	260.43	0.0500
152.01	261.95	0.0500
171.27	264.63	0.0500
188.40	267.90	0.0500
211.95	274.18	0.0500

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.49	257.05	.205E+04	0.4	0.68	89.65
1.17	257.73	.248E+05	5.7	0.85	72.16
1.85	258.41	.841E+05	25.2	1.10	55.60
2.53	259.09	.180E+06	66.5	1.36	45.14
3.21	259.77	.312E+06	135.7	1.60	38.36
3.89	260.45	.481E+06	238.3	1.82	33.61
4.57	261.13	.681E+06	383.5	2.07	29.60
5.25	261.81	.911E+06	568.9	2.29	26.70
5.93	262.49	.117E+07	800.5	2.51	24.36
6.61	263.17	.146E+07	1079.0	2.72	22.49
7.28	263.84	.177E+07	1413.4	2.94	20.85
7.96	264.52	.210E+07	1797.0	3.14	19.50
8.64	265.20	.246E+07	2241.6	3.35	18.28
9.32	265.88	.283E+07	2739.9	3.55	17.25
10.00	266.56	.323E+07	3295.7	3.75	16.34
10.68	267.24	.364E+07	3908.9	3.94	15.54
11.36	267.92	.408E+07	4578.5	4.12	14.84
12.04	268.60	.453E+07	5320.1	4.32	14.18
12.72	269.28	.499E+07	6109.1	4.50	13.61

<---- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8364) 905.65 1.30 8.50 5.87 0.61 0.71
 OUTFLOW : ID= 1 (6250) 905.65 1.21 10.75 5.87 0.59 0.70

ADD HYD (8366)
 1 + 2 = 3
 ID1= 1 (0250): 192.88 0.529 8.25 9.30
 + ID2= 2 (6250): 905.65 1.210 10.75 5.87
 ID = 3 (8366): 1098.53 1.691 10.25 6.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8368)
 1 + 2 = 3
 ID1= 1 (8366): 1098.53 1.691 10.25 6.48
 + ID2= 2 (8370): 548.79 0.986 9.50 6.29
 ID = 3 (8368): 1647.32 2.669 10.00 6.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8372)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)

```

ID1= 1 (0252): 319.99 1.110 8.00 10.34
+ ID2= 2 (8368): 1647.32 2.669 10.00 6.41
=====
ID = 3 (8372): 1967.31 3.671 9.50 7.05

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6258) |
IN= 2--> OUT= 1 |

```

----- DATA FOR SECTION (2581.0) ----->

Distance	Elevation	Manning	
0.00	252.88	0.0500	
15.47	251.89	0.0500	
46.41	248.45	0.0500	
73.48	245.81	0.0500	
88.95	243.75	0.0500	
112.15	242.00	0.0500	
135.35	240.23	0.0500	
162.42	239.76	0.0500	
170.97	239.52	0.0500 / 0.0350	Main Channel
171.58	239.03	0.0350	Main Channel
174.02	239.03	0.0350	Main Channel
176.46	239.03	0.0350	Main Channel
177.07	239.52	0.0350 / 0.0500	Main Channel
185.63	239.67	0.0500	
208.83	239.87	0.0500	
239.77	240.14	0.0500	
274.57	244.93	0.0500	
336.45	249.51	0.0500	
363.52	249.77	0.0500	
382.85	251.78	0.0500	

----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.49	239.52	.135E+05	2.5	0.92	91.32
1.14	240.17	.175E+06	25.6	0.73	114.22
1.78	240.81	.532E+06	123.5	1.17	71.85
2.43	241.46	.932E+06	285.3	1.54	54.48
3.07	242.10	.138E+07	505.5	1.85	45.25
3.72	242.75	.186E+07	786.8	2.12	39.42
4.36	243.39	.239E+07	1127.3	2.37	35.33
5.01	244.04	.296E+07	1535.8	2.61	32.12
5.65	244.68	.358E+07	2014.3	2.84	29.48
6.30	245.33	.420E+07	2535.9	3.03	27.60
6.94	245.97	.488E+07	3108.4	3.20	26.16
7.59	246.62	.561E+07	3740.6	3.35	24.98
8.23	247.26	.638E+07	4446.7	3.50	23.93
8.88	247.91	.721E+07	5229.7	3.64	22.98
9.52	248.55	.809E+07	6094.3	3.78	22.12
10.17	249.20	.901E+07	7050.4	3.93	21.31
10.81	249.84	.100E+08	7773.0	3.90	21.46
11.46	250.49	.111E+08	8998.8	4.07	20.56
12.10	251.13	.122E+08	10318.2	4.24	19.75

----- hydrograph -----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8372) 1967.31	3.67	9.50	7.05	0.52	0.90
OUTFLOW : ID= 1 (6258) 1967.31	3.39	11.25	7.05	0.52	0.91

```

| ADD HYD (8374) |
| 1 + 2 = 3 |
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0258): 181.99 0.722 8.00 12.98
+ ID2= 2 (6258): 1967.31 3.386 11.25 7.05
=====
ID = 3 (8374): 2149.30 3.981 11.00 7.56

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ADD HYD (8376) |
| 1 + 2 = 3 |
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0260): 476.24 1.943 8.25 14.65
+ ID2= 2 (8374): 2149.30 3.981 11.00 7.56
=====
ID = 3 (8376): 2625.54 5.750 10.25 8.84

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ADD HYD (8378) |

```

```

| 1 + 2 = 3 |
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0262): 341.31 1.720 8.00 14.60
+ ID2= 2 (8376): 2625.54 5.750 10.25 8.84
=====
ID = 3 (8378): 2966.85 7.221 10.00 9.50

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ADD HYD (8362) |
| 1 + 2 = 3 |
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8378): 2966.85 7.221 10.00 9.50
+ ID2= 2 (8384): 1321.42 3.620 9.25 9.70
=====
ID = 3 (8362): 4288.27 10.785 9.75 9.56

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6272) |
IN= 2--> OUT= 1 |

```

----- DATA FOR SECTION (2721.0) ----->

Distance	Elevation	Manning	
0.00	231.01	0.0450	
23.01	223.65	0.0450	
34.81	222.46	0.0450	
51.77	222.11	0.0450	
69.02	221.87	0.0450	
161.06	221.92	0.0450	
166.81	221.91	0.0450	
172.56	221.89	0.0450	
180.57	221.40	0.0450 / 0.0300	Main Channel
181.57	220.60	0.0300	Main Channel
184.07	220.16	0.0300	Main Channel
195.57	221.85	0.0300 / 0.0450	Main Channel
201.32	221.82	0.0450	
207.07	221.77	0.0450	
212.83	221.72	0.0450	
253.09	222.52	0.0450	
408.40	222.65	0.0450	
460.17	223.20	0.0450	
517.69	224.84	0.0450	
569.46	232.57	0.0450	

----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.41	220.57	.189E+04	0.5	0.49	60.18
0.83	220.99	.697E+04	3.2	0.82	35.79
1.24	221.40	.145E+05	8.8	1.07	27.45
1.84	222.00	.643E+05	29.3	0.81	36.61
2.44	222.60	.202E+06	132.9	0.83	35.41
3.04	223.20	.707E+06	413.7	1.04	28.50
3.64	223.80	.118E+07	876.8	1.31	22.46
4.24	224.40	.168E+07	1494.8	1.57	18.75
4.84	225.00	.221E+07	2264.1	1.82	16.23
5.44	225.60	.274E+07	3196.3	2.06	14.29
6.04	226.20	.328E+07	4258.6	2.30	12.84
6.65	226.81	.383E+07	5444.7	2.52	11.72
7.25	227.41	.438E+07	6749.7	2.73	10.82
7.85	228.01	.494E+07	8169.4	2.93	10.08
8.45	228.61	.551E+07	9700.7	3.12	9.46
9.05	229.21	.608E+07	11340.8	3.30	8.94
9.65	229.81	.666E+07	13087.4	3.48	8.48
10.25	230.41	.724E+07	14938.5	3.65	8.08
10.85	231.01	.783E+07	16892.4	3.82	7.73

----- hydrograph -----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8362) 4288.27	10.79	9.75	9.56	1.30	1.04
OUTFLOW : ID= 1 (6272) 4288.27	10.70	10.25	9.56	1.30	1.04

```

| ADD HYD (8356) |
| 1 + 2 = 3 |
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0272): 157.38 0.753 7.75 11.15
+ ID2= 2 (6272): 4288.27 10.700 10.25 9.56
=====
ID = 3 (8356): 4445.65 11.269 10.00 9.62

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.


```

-----
| ADD HYD (8352) |
| 1 + 2 = 3 |
-----
| ID1= 1 (8354): 1268.73 6.519 8.25 13.43
+ ID2= 2 (8356): 4445.65 11.269 10.00 9.62
-----
| ID = 3 (8352): 5714.38 17.204 9.25 10.47
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6304) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)= 15.00
-----

```

```

-----
| <----- DATA FOR SECTION (3041.0) ----->
| Distance Elevation Manning
| 0.00 232.08 0.0500
| 19.00 231.87 0.0500
| 38.00 231.33 0.0500
| 66.51 230.44 0.0500
| 104.51 228.25 0.0500
| 118.76 225.17 0.0500
| 128.26 219.86 0.0500
| 175.77 219.17 0.0500
| 185.27 218.90 0.0500 / 0.0300 Main Channel
| 185.52 218.65 0.0300 Main Channel
| 190.02 218.37 0.0300 Main Channel
| 194.52 218.60 0.0300 Main Channel
| 194.77 218.85 0.0300 / 0.0500 Main Channel
| 204.27 219.60 0.0500
| 299.28 220.91 0.0500
| 327.78 222.36 0.0500
| 375.28 225.71 0.0500
| 403.79 229.37 0.0500
| 432.29 230.43 0.0500
| 470.29 232.00 0.0500
-----

```

```

-----
| <----- TRAVEL TIME TABLE ----->
| DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
| (m) (m) (cu.m.) (cms) (m/s) (min)
| 0.48 218.85 .121E+05 1.2 0.36 173.94
| 1.17 219.54 .839E+05 9.8 0.44 142.60
| 1.86 220.23 .323E+06 41.0 0.47 131.43
| 2.56 220.93 .708E+06 109.5 0.58 107.67
| 3.25 221.62 .118E+07 224.3 0.71 87.36
| 3.94 222.31 .168E+07 376.5 0.84 74.48
| 4.63 223.00 .222E+07 567.9 0.96 65.26
| 5.32 223.69 .279E+07 794.7 1.07 58.58
| 6.02 224.39 .339E+07 1056.2 1.17 53.52
| 6.71 225.08 .402E+07 1352.4 1.26 49.52
| 7.40 225.77 .468E+07 1677.3 1.34 46.46
| 8.09 226.46 .535E+07 2050.0 1.43 43.58
| 8.79 227.16 .607E+07 2457.9 1.52 41.14
| 9.48 227.85 .680E+07 2901.0 1.60 39.05
| 10.17 228.54 .755E+07 3352.7 1.66 37.53
| 10.86 229.23 .834E+07 3812.5 1.71 36.48
| 11.55 229.92 .919E+07 4241.8 1.73 36.13
| 12.25 230.62 .101E+08 4697.0 1.74 35.93
| 12.94 231.31 .111E+08 5186.3 1.74 35.83
-----

```

```

-----
| <----- hydrograph -----> <-pipe / channel->
| AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
| (ha) (cms) (hrs) (mm) (m) (m/s)
| INFLOW : ID= 2 (8352) 5714.38 17.20 9.25 10.47 1.34 0.45
| OUTFLOW: ID= 1 (6304) 5714.38 14.25 12.00 10.47 1.27 0.44
-----

```

```

-----
| ADD HYD (8350) |
| 1 + 2 = 3 |
-----
| ID1= 1 (0304): 292.37 0.450 10.50 7.34
+ ID2= 2 (6304): 5714.38 14.247 12.00 10.47
-----
| ID = 3 (8350): 6006.75 14.687 12.00 10.31
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8346) |
| 1 + 2 = 3 |
-----
| ID1= 1 (8348): 3768.46 10.463 9.75 9.41
+ ID2= 2 (8350): 6006.75 14.687 12.00 10.31
-----
| ID = 3 (8346): 9775.21 24.489 11.00 9.97
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8344) |
| 1 + 2 = 3 |
-----
| ID1= 1 (0306): 283.97 0.146 14.00 5.04
+ ID2= 2 (8346): 9775.21 24.489 11.00 9.97
-----
| ID = 3 (8344): 10059.18 24.623 11.00 9.83
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| RESERVOIR (5509) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
| OUTFLOW STORAGE OUTFLOW STORAGE
| (cms) (ha.m.) (cms) (ha.m.)
| 0.0000 0.0000 | ***** 370.0451
| 41.0590 56.7403 | ***** 863.4386
| 48.1390 86.3439 | *****
| 56.6340 ***** | *****
| 67.9600 ***** | 0.0000 0.0000
-----
| AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
| INFLOW : ID= 2 (8344) ***** 24.623 11.00 9.83
| OUTFLOW: ID= 1 (5509) ***** 19.107 14.50 9.83
-----

```

PEAK FLOW REDUCTION [Qout/Qin](%)= 77.60
TIME SHIFT OF PEAK FLOW (min)=210.00
MAXIMUM STORAGE USED (ha.m.)= 26.4092

```

-----
| CALIB |
| NASHYD (0332) | Area (ha)= 393.44 Curve Number (CN)= 75.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 2.32
-----

```

Unit Hyd Qpeak (cms)= 2.894
PEAK FLOW (cms)= 1.069 (i)
TIME TO PEAK (hrs)= 10.000
RUNOFF VOLUME (mm)= 11.219
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.267

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0330) | Area (ha)= 468.30 Curve Number (CN)= 80.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.09
-----

```

Unit Hyd Qpeak (cms)= 7.335
PEAK FLOW (cms)= 2.745 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 13.500
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.321

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ROUTE CHN (6332) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)= 15.00
-----

```

```

-----
| <----- DATA FOR SECTION (3321.0) ----->
| Distance Elevation Manning
| 0.00 234.00 0.0380
| 25.85 227.20 0.0380
| 96.94 226.44 0.0380
| 168.03 227.38 0.0380
| 219.73 225.62 0.0380
| 342.52 221.57 0.0380
| 368.37 221.42 0.0380
| 374.83 221.23 0.0380
| 379.79 220.98 0.0380 / 0.0300 Main Channel
| 380.29 220.47 0.0300 Main Channel
| 381.29 220.47 0.0300 Main Channel
| 382.79 220.47 0.0300 Main Channel
| 383.29 220.98 0.0300 / 0.0380 Main Channel
| 394.22 221.22 0.0380
| 400.68 221.33 0.0380
| 407.14 221.44 0.0380
| 491.16 225.70 0.0380
| 568.71 227.55 0.0380
| 607.49 230.14 0.0380
| 639.80 234.08 0.0380
-----

```

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.51	220.98	.554E+04	0.9	0.56	107.27
1.20	221.67	.885E+05	12.1	0.49	122.32
1.88	222.35	.309E+06	64.7	0.76	79.70
2.57	223.04	.615E+06	165.4	0.97	61.99
3.25	223.72	.101E+07	322.1	1.16	52.06
3.94	224.41	.148E+07	542.9	1.33	45.51
4.62	225.09	.204E+07	835.4	1.48	40.77
5.31	225.78	.269E+07	1202.8	1.62	37.27
5.99	226.46	.344E+07	1616.0	1.70	35.46
6.68	227.15	.447E+07	1932.0	1.56	38.56
7.36	227.83	.580E+07	2763.9	1.72	34.96
8.05	228.52	.718E+07	3861.9	1.95	30.97
8.73	229.20	.859E+07	5115.4	2.16	27.98
9.42	229.89	.100E+08	6519.2	2.35	25.64
10.10	230.57	.115E+08	8092.3	2.55	23.69
10.79	231.26	.130E+08	9821.5	2.73	22.05
11.47	231.94	.145E+08	11690.0	2.92	20.68
12.16	232.63	.160E+08	13694.9	3.09	19.52
12.84	233.31	.176E+08	15833.9	3.26	18.52

<---- hydrograph ----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (0330)	468.30	2.74	7.50	13.50	0.63
OUTFLOW: ID= 1 (6332)	468.30	2.16	9.75	13.50	0.59

ADD HYD (8322)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID= 1 (0332):	393.44	1.069	10.00	11.22
+ ID= 2 (6332):	468.30	2.156	9.75	13.50
ID = 3 (8322):	861.74	3.223	9.75	12.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)
NASHYD (0328)	492.92	77.0		
ID= 1 DT=15.0 min	5.00	1.50		
U.H. Tp(hrs)=	1.91			
Unit Hyd Qpeak	(cms)=	4.411		
PEAK FLOW	(cms)=	1.670 (i)		
TIME TO PEAK	(hrs)=	9.000		
RUNOFF VOLUME	(mm)=	12.082		
TOTAL RAINFALL	(mm)=	42.000		
RUNOFF COEFFICIENT	=	0.288		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)
NASHYD (0326)	678.91	80.0		
ID= 1 DT=15.0 min	5.00	1.50		
U.H. Tp(hrs)=	1.95			
Unit Hyd Qpeak	(cms)=	5.941		
PEAK FLOW	(cms)=	2.547 (i)		
TIME TO PEAK	(hrs)=	9.000		
RUNOFF VOLUME	(mm)=	13.570		
TOTAL RAINFALL	(mm)=	42.000		
RUNOFF COEFFICIENT	=	0.323		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)
NASHYD (0324)	615.64	79.0		
ID= 1 DT=15.0 min	5.00	1.50		
U.H. Tp(hrs)=	1.90			
Unit Hyd Qpeak	(cms)=	5.544		
PEAK FLOW	(cms)=	2.268 (i)		
TIME TO PEAK	(hrs)=	9.000		
RUNOFF VOLUME	(mm)=	13.047		
TOTAL RAINFALL	(mm)=	42.000		
RUNOFF COEFFICIENT	=	0.311		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)
NASHYD (0320)	278.74	81.0		
ID= 1 DT=15.0 min	5.00	1.50		
U.H. Tp(hrs)=	1.36			
Unit Hyd Qpeak	(cms)=	3.499		
PEAK FLOW	(cms)=	1.448 (i)		
TIME TO PEAK	(hrs)=	8.000		
RUNOFF VOLUME	(mm)=	14.083		
TOTAL RAINFALL	(mm)=	42.000		
RUNOFF COEFFICIENT	=	0.335		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)
NASHYD (3182)	457.40	78.0		
ID= 1 DT=15.0 min	5.00	1.50		
U.H. Tp(hrs)=	1.49			
Unit Hyd Qpeak	(cms)=	5.259		
PEAK FLOW	(cms)=	1.960 (i)		
TIME TO PEAK	(hrs)=	8.250		
RUNOFF VOLUME	(mm)=	12.530		
TOTAL RAINFALL	(mm)=	42.000		
RUNOFF COEFFICIENT	=	0.298		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Dir. Conn.(%)
STANDHYD (3181)	26.00	81.00	81.00
ID= 1 DT=15.0 min			

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	21.06
Dep. Storage (mm)=	2.00
Average Slope (%)=	0.50
Length (m)=	416.33
Mannings n =	0.013

Max. Eff. Inten. (mm/hr)=	55.44	11.13
over (min)	15.00	45.00
Storage Coeff. (min)=	9.37 (ii)	35.11 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.09	0.03
PEAK FLOW (cms)=	2.79	0.09
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	40.00	12.13
TOTAL RAINFALL (mm)=	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.29

TOTALS

2.825 (iii)

6.00

34.70

42.00

0.83

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 77.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5318)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min	0.0000	0.0000	3.3800	0.7654
	0.0510	0.4423	4.1500	0.8231
	0.8700	0.5378	4.8750	0.8778
	1.9590	0.6105	5.2750	0.8878
	2.5620	0.6796	0.0000	0.0000

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (3181)	26.000	2.825	6.00
OUTFLOW: ID= 1 (5318)	26.000	0.751	6.50
PEAK FLOW REDUCTION [Qout/Qin](%)=	26.57		
TIME SHIFT OF PEAK FLOW	(min)=	30.00	
MAXIMUM STORAGE USED	(ha.m.)=	0.5295	

ADD HYD (8334)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID= 1 (3182):	457.40	1.960	8.25	12.53

+ ID2= 2 (5318): 26.00 0.751 6.50 34.62
 ID = 3 (8334): 483.40 2.212 6.75 13.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | ROUTE CHN (5320) |
 | IN= 2----> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3201.0) ----->

Distance	Elevation	Manning
0.00	249.00	0.0380
22.92	245.86	0.0380
45.83	244.87	0.0380
91.66	243.13	0.0380
126.03	239.53	0.0380
160.41	237.17	0.0380
166.14	237.06	0.0380
177.59	237.13	0.0380
183.32	237.20	0.0380
189.05	236.70	0.0380 /0.0350 Main Channel
193.05	235.89	0.0350 Main Channel
197.05	236.64	0.0350 /0.0380 Main Channel
200.51	236.74	0.0380
206.24	237.03	0.0380
246.34	238.82	0.0380
263.53	243.87	0.0380
389.56	247.64	0.0380
452.58	247.74	0.0380
498.41	248.60	0.0380
567.16	249.84	0.0380

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.37	236.26	.270E+04	0.4	0.58	107.96
0.75	236.64	.108E+05	2.6	0.92	68.01
1.48	237.37	.877E+05	27.4	1.17	53.35
2.20	238.09	.277E+06	122.1	1.65	37.83
2.93	238.82	.540E+06	299.7	2.08	30.01
3.66	239.55	.857E+06	587.8	2.57	24.29
4.39	240.28	.120E+07	977.8	3.04	20.52
5.11	241.00	.158E+07	1458.1	3.46	18.02
5.84	241.73	.198E+07	2028.6	3.84	16.23
6.57	242.46	.240E+07	2690.5	4.19	14.86
7.29	243.18	.285E+07	3431.8	4.50	13.84
8.02	243.91	.335E+07	4149.1	4.64	13.44
8.75	244.64	.393E+07	4745.6	4.51	13.80
9.47	245.36	.463E+07	5618.6	4.83	13.75
10.20	246.09	.545E+07	6781.8	4.66	13.39
10.93	246.82	.635E+07	8274.5	4.87	12.79
11.66	247.55	.733E+07	9980.0	5.10	12.23
12.38	248.27	.853E+07	11422.0	5.01	12.45
13.11	249.00	.989E+07	13749.7	5.20	11.98

<---- hydrograph ----> <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8334)	483.40	2.21	6.75	13.72	0.68
OUTFLOW: ID= 1 (6320)	483.40	1.92	9.50	13.72	0.63

 | ADD HYD (8332) |
 | 1 + 2 = 3 |
 | ID1= 1 (0320): 278.74 1.448 8.00 14.08
 + ID2= 2 (6320): 483.40 1.921 9.50 13.72
 ID = 3 (8332): 762.14 3.286 8.75 13.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | CALIB (0314) | Area (ha)= 165.20 Curve Number (CN)= 78.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.71

Unit Hyd Qpeak (cms)= 3.978
 PEAK FLOW (cms)= 1.241 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 12.387
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.295

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | ROUTE CHN (6316) |
 | IN= 2----> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3161.0) ----->

Distance	Elevation	Manning
0.00	248.54	0.0350
27.93	248.34	0.0350
41.89	246.61	0.0350
62.83	243.09	0.0350
132.65	239.00	0.0350
188.50	236.74	0.0350
195.48	236.54	0.0350
202.46	236.32	0.0350
205.48	236.14	0.0350 /0.0310 Main Channel
205.98	235.61	0.0310 Main Channel
207.98	235.25	0.0310 Main Channel
209.98	235.53	0.0310 Main Channel
210.48	236.00	0.0310 /0.0350 Main Channel
216.42	236.73	0.0350
259.31	239.09	0.0350
328.12	239.84	0.0350
439.83	241.47	0.0350
530.58	242.08	0.0350
586.43	242.93	0.0350
691.16	248.00	0.0350

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.37	235.62	.215E+04	0.7	0.77	53.88
0.75	236.00	.634E+04	3.5	1.36	30.45
1.46	236.71	.297E+05	19.3	1.62	25.66
2.16	237.41	.103E+06	75.8	1.84	22.60
2.87	238.12	.239E+06	202.8	2.21	18.79
3.57	238.82	.407E+06	423.3	2.59	16.03
4.28	239.53	.654E+06	727.7	2.77	14.97
4.99	240.24	.103E+07	1242.0	3.01	13.76
5.69	240.94	.151E+07	2002.3	3.31	12.54
6.40	241.65	.210E+07	2987.9	3.55	11.70
7.10	242.35	.286E+07	4302.8	3.75	11.07
7.81	243.06	.374E+07	6165.3	4.10	10.11
8.51	243.76	.468E+07	8642.4	4.60	9.03
9.22	244.47	.565E+07	11496.3	5.06	8.20
9.93	245.18	.666E+07	14714.1	5.50	7.54
10.63	245.88	.770E+07	18290.3	5.91	7.02
11.34	246.59	.877E+07	22221.9	6.30	6.58
12.04	247.29	.988E+07	26464.4	6.67	6.22
12.75	248.00	.110E+08	31061.3	7.01	5.91

<---- hydrograph ----> <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0314)	165.20	1.24	7.00	12.39	0.45
OUTFLOW: ID= 1 (6316)	165.20	1.06	8.00	12.39	0.43

 | CALIB (0316) | Area (ha)= 232.34 Curve Number (CN)= 82.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.01

Unit Hyd Qpeak (cms)= 3.923
 PEAK FLOW (cms)= 1.569 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 14.611
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.348

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | ADD HYD (8338) |
 | 1 + 2 = 3 | Area (ha)= 359.44 Curve Number (CN)= 80.0
 | ID1= 1 (0316): 232.34 1.569 7.25 14.61
 + ID2= 2 (6316): 165.20 1.059 8.00 12.39
 ID = 3 (8338): 397.54 2.608 7.75 13.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | CALIB (0312) | Area (ha)= 359.44 Curve Number (CN)= 80.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.36

Unit Hyd Qpeak (cms)= 4.529

PEAK FLOW (cms)= 1.796 (1)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 13.533
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.322

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0308) | Area (ha)= 529.30 Curve Number (CN)= 62.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.62
  
```

Unit Hyd Qpeak (cms)= 5.575

PEAK FLOW (cms)= 1.176 (1)
 TIME TO PEAK (hrs)= 8.500
 RUNOFF VOLUME (mm)= 7.070
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.168

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0310) | Area (ha)= 138.28 Curve Number (CN)= 65.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.76
  
```

Unit Hyd Qpeak (cms)= 3.102

PEAK FLOW (cms)= 0.603 (1)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 7.758
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.185

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8342) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (0308): 529.30 1.176 8.50 7.07
| + ID2= 2 (0310): 138.28 0.603 7.00 7.76
|-----|
| ID = 3 (8342): 667.58 1.702 8.00 7.21
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6312) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
  
```

<----- DATA FOR SECTION (3121.0) ----->

Distance	Elevation	Manning
0.00	265.94	0.0360
38.07	265.43	0.0360
59.82	263.98	0.0360
103.32	254.59	0.0360
157.70	252.16	0.0360
217.52	250.45	0.0360
233.84	247.69	0.0360
234.71	247.27	0.0360
239.71	246.38	0.0360
244.71	246.12	0.0360 /0.0330 Main Channel
247.71	245.17	0.0330 Main Channel
249.71	245.19	0.0330 Main Channel
251.71	245.64	0.0330 /0.0360 Main Channel
259.71	246.47	0.0360
282.78	247.12	0.0360
315.41	251.60	0.0360
424.17	256.13	0.0360
478.55	257.04	0.0360
516.62	259.37	0.0360
538.37	266.00	0.0360

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.47	245.64	.111E+04	3.1	1.78	6.07
1.54	246.71	.103E+05	53.9	3.39	3.19
2.61	247.78	.414E+05	281.3	4.40	2.45
3.68	248.85	.839E+05	749.1	5.79	1.87
4.74	249.91	.136E+06	1463.2	6.97	1.55
5.81	250.98	.201E+06	2354.8	7.61	1.42
6.88	252.05	.295E+06	3533.0	7.76	1.39
7.95	253.12	.427E+06	5303.9	8.05	1.34

9.02	254.19	.594E+06	7856.5	8.58	1.26
10.09	255.26	.792E+06	11449.2	9.37	1.15
11.15	256.32	.101E+07	15862.4	10.15	1.06
12.22	257.39	.127E+07	21074.5	10.75	1.01
13.29	258.46	.155E+07	28184.1	11.78	0.92
14.36	259.53	.185E+07	36386.2	12.77	0.85
15.43	260.60	.215E+07	46110.6	13.89	0.78
16.50	261.67	.246E+07	58222.5	14.95	0.72
17.56	262.73	.278E+07	66498.1	15.97	0.68
18.63	263.80	.310E+07	81118.8	16.94	0.64
19.70	264.87	.344E+07	93603.2	17.66	0.61

```

-----
|----- hydrograph -----| <-pipe / channel->
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) (m) (m/s) |
|-----|
| INFLOW: ID= 2 (8342) 667.58 1.70 8.00 7.21 0.26 1.78 |
|-----|
| OUTFLOW: ID= 1 (6312) 667.58 1.70 8.00 7.21 0.26 1.78 |
  
```

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-----
| ADD HYD (8340) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (0312): 359.44 1.796 8.00 13.53
| + ID2= 2 (6312): 667.58 1.699 8.00 7.21
|-----|
| ID = 3 (8340): 1027.02 3.494 8.00 9.42
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8336) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (8338): 397.54 2.608 7.75 13.69
| + ID2= 2 (8340): 1027.02 3.494 8.00 9.42
|-----|
| ID = 3 (8336): 1424.56 6.086 8.00 10.61
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8330) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (8332): 762.14 3.286 8.75 13.85
| + ID2= 2 (8336): 1424.56 6.086 8.00 10.61
|-----|
| ID = 3 (8330): 2186.70 9.233 8.25 11.74
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| ROUTE CHN (6324) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
  
```

<----- DATA FOR SECTION (3241.0) ----->

Distance	Elevation	Manning
0.00	234.38	0.0360
33.57	232.48	0.0360
67.14	230.14	0.0360
83.93	228.80	0.0360
134.29	227.62	0.0360
209.82	225.10	0.0360
218.21	224.86	0.0360
226.61	224.47	0.0360
234.00	223.86	0.0360 /0.0300 Main Channel
234.10	223.66	0.0300 Main Channel
235.00	223.66	0.0300 Main Channel
235.90	223.66	0.0300 Main Channel
236.00	223.86	0.0300 /0.0360 Main Channel
243.39	224.92	0.0360
251.78	224.89	0.0360
335.71	225.64	0.0360
562.32	226.53	0.0360
637.85	228.36	0.0360
705.00	229.80	0.0360
830.89	234.00	0.0360

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.20	223.86	.145E+04	0.2	0.43	147.27
0.73	224.39	.159E+05	2.8	0.68	93.11
1.27	224.93	.561E+05	11.0	0.75	85.19
1.80	225.46	.217E+06	45.6	0.80	79.20
2.33	225.99	.566E+06	134.4	0.91	70.20
2.87	226.53	.122E+07	336.1	1.06	59.97

3.40	227.06	.208E+07	744.3	1.38	46.16
3.94	227.60	.298E+07	1295.9	1.66	38.38
4.47	228.13	.399E+07	1979.9	1.90	33.59
5.00	228.66	.509E+07	2807.5	2.11	30.21
5.54	229.20	.627E+07	3815.1	2.32	27.41
6.07	229.73	.753E+07	4984.1	2.53	25.17
6.60	230.26	.884E+07	6346.3	2.74	23.20
7.14	230.80	.102E+08	7861.2	2.95	21.61
7.67	231.33	.116E+08	9526.4	3.14	20.29
8.21	231.87	.131E+08	11342.2	3.32	19.18
8.74	232.40	.146E+08	13309.6	3.49	18.22
9.27	232.93	.161E+08	15414.0	3.66	17.41
9.81	233.47	.177E+08	17671.0	3.81	16.70

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8330) 2186.70 9.23 8.25 11.74 1.15 0.73
OUTFLOW : ID= 1 (6324) 2186.70 7.97 10.00 11.74 1.07 0.72

```

```

ADD HYD (8328)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8324): 615.64 2.268 9.00 13.05
+ ID2= 2 (6324): 2186.70 7.966 10.00 11.74
=====
ID = 3 (8328): 2802.34 10.191 9.75 12.03

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB
NASHYD (0322) | Area (ha)= 513.13 Curve Number (CN)= 80.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.75

```

Unit Hyd Qpeak (cms) = 5.020

```

PEAK FLOW (cms) = 2.099 (1)
TIME TO PEAK (hrs) = 8.500
RUNOFF VOLUME (mm) = 13.561
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.323

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8326)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8322): 513.13 2.099 8.50 13.56
+ ID2= 2 (8328): 2802.34 10.191 9.75 12.03
=====
ID = 3 (8326): 3315.47 12.230 9.75 12.27

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8324)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8326): 678.91 2.547 9.00 13.57
+ ID2= 2 (8326): 3315.47 12.230 9.75 12.27
=====
ID = 3 (8324): 3994.38 14.756 9.75 12.49

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CEN (8328)
IN= 2 ----> OUT= 1
Routing time step (min) = 15.00

```

<----- DATA FOR SECTION (3281.0) ----->

Distance	Elevation	Manning
0.00	228.00	0.0380
18.58	224.97	0.0380
78.98	223.52	0.0380
125.44	223.28	0.0380
171.90	221.71	0.0380
213.72	219.65	0.0380
218.36	219.40	0.0380
223.01	219.19	0.0380
225.95	219.14	0.0380 / 0.0320
226.45	218.14	0.0320
236.95	218.14	0.0320

245.85	218.14	0.0320	Main Channel
245.95	219.14	0.0320 / 0.0380	Main Channel
246.24	219.16	0.0380	
250.88	219.24	0.0380	
255.53	219.39	0.0380	
325.22	221.47	0.0380	
367.03	223.14	0.0380	
404.20	225.17	0.0380	
459.95	228.04	0.0380	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.50	218.64	.373E+05	3.2	0.33	192.37
1.00	219.14	.752E+05	10.1	0.51	124.71
1.52	219.66	.149E+06	22.2	0.57	112.06
2.04	220.18	.279E+06	43.4	0.59	107.06
2.56	220.70	.464E+06	77.0	0.63	100.52
3.08	221.22	.705E+06	125.7	0.68	93.56
3.61	221.75	.100E+07	192.7	0.73	86.62
4.13	222.27	.135E+07	278.1	0.79	80.92
4.65	222.79	.176E+07	385.9	0.84	75.86
5.17	223.31	.222E+07	514.9	0.89	71.82
5.69	223.83	.280E+07	662.7	0.90	70.40
6.21	224.35	.345E+07	867.7	0.96	66.35
6.73	224.87	.417E+07	1107.0	1.01	62.81
7.25	225.39	.494E+07	1400.0	1.08	58.80
7.78	225.92	.573E+07	1732.2	1.15	55.17
8.30	226.44	.655E+07	2099.9	1.22	52.02
8.82	226.96	.740E+07	2502.8	1.29	49.29
9.34	227.48	.828E+07	2940.9	1.36	46.90
9.86	228.00	.918E+07	3414.3	1.42	44.79

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8324) 3994.38 14.76 9.75 12.49 1.20 0.53
OUTFLOW : ID= 1 (6328) 3994.38 12.61 11.50 12.49 1.11 0.52

```

```

ADD HYD (8320)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8328): 492.92 1.670 9.00 12.08
+ ID2= 2 (6328): 3994.38 12.607 11.50 12.49
=====
ID = 3 (8320): 4487.30 14.097 11.25 12.44

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8318)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8320): 4487.30 14.097 11.25 12.44
+ ID2= 2 (8322): 861.74 3.223 9.75 12.46
=====
ID = 3 (8318): 5349.04 17.133 11.00 12.45

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8316)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8309): 10059.18 19.107 14.50 9.83
+ ID2= 2 (8318): 5349.04 17.133 11.00 12.45
=====
ID = 3 (8316): 15408.22 33.914 13.00 10.74

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8312)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8314): 620.10 0.389 7.25 6.40
+ ID2= 2 (8316): 15408.22 33.914 13.00 10.74
=====
ID = 3 (8312): 16028.32 34.228 13.00 10.57

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8308)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8310): 598.90 9.798 6.25 15.95
+ ID2= 2 (8312): 16028.32 34.228 13.00 10.57
=====
ID = 3 (8308): 16627.22 34.888 12.50 10.76

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

RESERVOIR (5510)
IN= 2--> OUT= 1
DT= 15.0 min
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
0.0000 0.0000 | ***** 74.0090
66.5450 18.5023 | *****
98.5430 37.0045 | *****

```

```

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (8308) ***** 34.888 12.50 10.76
OUTFLOW : ID= 1 (5510) ***** 34.412 13.25 10.76

```

```

PEAK FLOW REDUCTION [Qout/Qin](%)= 98.64
TIME SHIFT OF PEAK FLOW (min)= 45.00
MAXIMUM STORAGE USED (ha.m.)= 9.5730

```

```

ADD HYD (8240)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0336): 2785.00 1.223 22.50 10.07
+ ID2= 2 (5510): 16627.22 34.412 13.25 10.76
=====
ID = 3 (8240): 19412.22 35.468 13.50 10.66

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8238)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (5508): 9524.23 14.571 12.25 6.08
+ ID2= 2 (8240): 19412.22 35.468 13.50 10.66
=====
ID = 3 (8238): 28936.45 49.761 13.00 9.15

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8236)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0342): 1004.58 1.208 10.25 5.39
+ ID2= 2 (8238): 28936.45 49.761 13.00 9.15
=====
ID = 3 (8236): 29941.03 50.838 13.00 9.03

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8234)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8232): 285.80 3.513 6.00 10.55
+ ID2= 2 (8236): 29941.03 50.838 13.00 9.03
=====
ID = 3 (8234): 30226.83 51.024 13.00 9.04

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8230)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0344): 344.00 1.036 7.25 6.33
+ ID2= 2 (8234): 30226.83 51.024 13.00 9.04
=====
ID = 3 (8230): 30570.83 51.336 12.75 9.01

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8228)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8226): 1952.06 3.673 9.00 5.24
+ ID2= 2 (8230): 30570.83 51.336 12.75 9.01
=====
ID = 3 (8228): 32522.89 53.703 12.75 8.79

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8190)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0356): 1015.22 1.461 8.25 4.06
+ ID2= 2 (8228): 32522.89 53.703 12.75 8.79
=====
ID = 3 (8190): 33538.11 54.595 12.50 8.64

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

*****
** SIMULATION NUMBER: 2 **
*****

```

```

READ STORM
Filename: C:\Users\jscott\AppData
Local\Temp\
5a280798-92ee-4282-809c-79f5caed0add\5a76734a
Total= 54.40 mm
Comments: 5-Year 12-Hour SCS II Design Storm

```

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.36	3.25	2.18	6.25	9.79	9.25	1.90
0.50	1.36	3.50	2.18	6.50	9.79	9.50	1.90
0.75	1.36	3.75	2.18	6.75	4.35	9.75	1.90
1.00	1.36	4.00	2.18	7.00	4.35	10.00	1.90
1.25	1.36	4.25	3.26	7.25	3.26	10.25	1.09
1.50	1.36	4.50	3.26	7.50	3.26	10.50	1.09
1.75	1.36	4.75	4.35	7.75	3.26	10.75	1.09
2.00	1.36	5.00	4.35	8.00	3.26	11.00	1.09
2.25	1.63	5.25	6.53	8.25	1.90	11.25	1.09
2.50	1.63	5.50	6.53	8.50	1.90	11.50	1.09
2.75	1.63	5.75	26.11	8.75	1.90	11.75	1.09
3.00	1.63	6.00	71.83	9.00	1.90	12.00	1.09

```

CALIB
NASHYD (0356) Area (ha)=1015.22 Curve Number (CN)= 46.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.37

```

```

Unit Hyd Qpeak (cms)= 12.651
PEAK FLOW (cms)= 2.534 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 6.976
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.128

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0354) Area (ha)= 262.68 Curve Number (CN)= 37.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.38

```

```

Unit Hyd Qpeak (cms)= 3.252
PEAK FLOW (cms)= 0.468 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 5.032
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.093

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0352) Area (ha)= 381.43 Curve Number (CN)= 54.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.82

```

Unit Hyd Qpeak (cms)= 7.980

PEAK FLOW (cms)= 1.840 (1)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 9.056
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.166

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHYD (0346) | Area (ha)= 350.93 Curve Number (CN)= 70.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 6.254

PEAK FLOW (cms)= 2.564 (1)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 15.253
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.280

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHYD (0350) | Area (ha)= 366.84 Curve Number (CN)= 48.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 5.831

PEAK FLOW (cms)= 1.170 (1)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 7.450
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.137

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHYD (0348) | Area (ha)= 590.18 Curve Number (CN)= 48.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.91

Unit Hyd Qpeak (cms)= 5.267

PEAK FLOW (cms)= 1.224 (1)
 TIME TO PEAK (hrs)= 9.250
 RUNOFF VOLUME (mm)= 7.490
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.138

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6350) |
 IN= 2--> OUT= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (3501.0) ----->

Distance	Elevation	Manning
0.00	287.00	0.0500
23.66	283.72	0.0500
70.98	280.94	0.0500
112.38	280.23	0.0500
171.52	276.80	0.0500
260.24	274.46	0.0500
266.16	274.26	0.0500
272.07	274.12	0.0500
276.49	274.12	0.0500 / 0.0300 Main Channel
276.99	273.82	0.0300 Main Channel
277.99	273.82	0.0300 Main Channel
279.49	273.82	0.0300 Main Channel
280.49	274.27	0.0300 / 0.0500 Main Channel
283.00	274.27	0.0500
289.81	274.57	0.0500
325.30	275.75	0.0500
396.38	276.98	0.0500
449.51	280.97	0.0500
496.83	283.90	0.0500
585.54	287.92	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.30	274.12	273E+04	0.9	0.89	49.81
0.98	274.80	583E+05	20.8	1.05	46.71
1.66	275.48	203E+06	95.2	1.38	35.53
2.33	276.15	438E+06	256.0	1.73	28.50

3.01	276.83	755E+06	524.8	2.05	23.97
3.69	277.51	114E+07	935.8	2.43	20.26
4.37	278.19	157E+07	1472.0	2.76	17.82
5.05	278.87	206E+07	2140.7	3.06	16.06
5.72	279.54	263E+07	2938.6	3.33	14.79
6.40	280.22	321E+07	3887.6	3.57	13.77
7.08	280.90	390E+07	4795.4	3.63	13.56
7.76	281.58	468E+07	6196.2	3.91	12.58
8.43	282.25	550E+07	7803.9	4.19	11.74
9.11	282.93	637E+07	9601.9	4.45	11.05
9.79	283.61	728E+07	11594.8	4.70	10.46
10.47	284.29	823E+07	13845.3	4.96	9.91
11.15	284.97	922E+07	16293.4	5.21	9.43
11.82	285.64	103E+08	18952.0	5.45	9.02
12.50	286.32	113E+08	21824.3	5.69	8.65

<---- hydrograph ----> <-pipe / channel-->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0348) 590.18 1.22 9.25 7.49 0.31 0.99
 OUTFLOW: ID= 1 (6350) 590.18 1.20 10.25 7.49 0.31 0.99

ADD HYD (8220) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0350): 366.84 1.170 7.75 7.45
 + ID2= 2 (6350): 590.18 1.197 10.25 7.49
 ID = 3 (8220): 957.02 2.179 8.75 7.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8222) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0346): 350.93 2.564 7.25 15.25
 + ID2= 2 (8220): 957.02 2.179 8.75 7.47
 ID = 3 (8222): 1307.95 4.585 8.00 9.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6352) |
 IN= 2--> OUT= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (3521.0) ----->

Distance	Elevation	Manning
0.00	257.95	0.0500
7.83	257.34	0.0500
15.66	256.19	0.0500
21.53	254.01	0.0500
41.11	244.06	0.0500
76.38	241.38	0.0500
111.58	239.74	0.0500
113.54	239.61	0.0500
113.75	239.41	0.0500 / 0.0300 Main Channel
113.85	238.81	0.0300 Main Channel
115.50	238.81	0.0300 Main Channel
116.15	238.81	0.0300 Main Channel
116.25	239.41	0.0300 Main Channel
117.46	239.52	0.0500
119.41	239.72	0.0500
121.37	240.04	0.0500
131.16	241.84	0.0500
156.61	247.03	0.0500
176.19	251.46	0.0500
193.80	258.79	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.60	239.41	224E+04	2.2	1.52	17.12
1.58	240.39	216E+05	22.3	1.61	16.16
2.55	241.36	804E+05	101.0	1.96	13.27
3.53	242.34	173E+06	275.6	2.48	10.46
4.50	243.31	293E+06	553.9	2.95	8.80
5.48	244.29	438E+06	961.8	3.42	7.60
6.45	245.26	599E+06	1534.4	3.99	6.51
7.43	246.24	770E+06	2228.4	4.51	5.76
8.41	247.22	951E+06	3043.6	4.98	5.21
9.38	248.19	114E+07	3982.1	5.43	4.78
10.36	249.17	134E+07	5040.1	5.85	4.44
11.33	250.14	155E+07	6218.5	6.24	4.16
12.31	251.12	177E+07	7518.6	6.61	3.93
13.29	252.10	200E+07	8969.0	6.98	3.72
14.26	253.07	224E+07	10554.1	7.35	3.53

13.24 254.05 .248E+07 12257.1 7.71 3.37
 16.21 255.02 .272E+07 14045.1 8.03 3.23
 17.19 256.00 .298E+07 15954.8 8.34 3.11
 18.16 256.97 .325E+07 17780.4 8.53 3.04

<----- hydrograph -----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8222) 1307.95 4.55 8.00 9.56 0.72 1.53
 OUTFLOW: ID= 1 (6352) 1307.95 4.55 8.25 9.56 0.71 1.53

ADD HYD (8224)
 1 2 3
 ID1= 1 (0352): 381.43 1.840 7.00 9.06
 + ID2= 2 (6352): 1307.95 4.551 8.25 9.56
 ID = 3 (8224): 1689.38 6.228 8.00 9.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6354) Routing time step (min) = 15.00
 IN= 2 -> OUT= 1

DATA FOR SECTION (3541.0) ----->
 Distance Elevation Manning
 0.00 253.92 0.0500
 7.95 251.83 0.0500
 15.89 249.97 0.0500
 19.87 249.05 0.0500
 47.68 242.00 0.0500
 67.55 237.10 0.0500
 83.44 231.11 0.0500
 85.43 230.75 0.0500
 85.66 230.61 0.0500 / 0.0300 Main Channel
 85.76 230.01 0.0300 Main Channel
 87.41 230.01 0.0300 Main Channel
 88.06 230.01 0.0300 Main Channel
 88.16 230.61 0.0300 / 0.0500 Main Channel
 89.40 230.72 0.0500
 91.39 230.88 0.0500
 93.37 231.03 0.0500
 133.11 233.96 0.0500
 150.99 235.39 0.0500
 172.84 239.03 0.0500
 196.68 250.00 0.0500

TRAVEL TIME TABLE ----->
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
 (m) (m) (cu.m.) (cms) (m/s) (min)
 0.60 230.61 .451E+04 1.5 1.06 49.40
 1.62 231.63 .394E+05 14.6 3.16 45.13
 2.64 232.65 .128E+06 58.8 1.44 36.30
 3.66 233.67 .269E+06 149.7 1.74 30.00
 4.68 234.69 .463E+06 301.5 2.04 25.58
 5.70 235.71 .705E+06 530.5 2.36 22.14
 6.72 236.73 .980E+06 848.4 2.71 19.25
 7.74 237.75 .128E+07 1237.9 3.02 17.29
 8.76 238.77 .162E+07 1706.7 3.30 15.83
 9.78 239.79 .199E+07 2291.9 3.61 14.48
 10.81 240.82 .237E+07 2973.7 3.92 13.30
 11.83 241.84 .278E+07 3742.2 4.21 12.38
 12.85 242.86 .321E+07 4599.1 4.49 11.62
 13.87 243.88 .365E+07 5544.6 4.75 10.98
 14.89 244.90 .412E+07 6579.7 5.00 10.44
 15.91 245.92 .461E+07 7705.9 5.23 9.97
 16.93 246.94 .512E+07 8924.6 5.46 9.55
 17.95 247.96 .564E+07 10237.5 5.68 9.19
 18.97 248.98 .619E+07 11646.2 5.89 8.86

<----- hydrograph -----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8224) 1689.38 6.23 8.00 9.45 0.97 1.09
 OUTFLOW: ID= 1 (6354) 1689.38 5.80 9.00 9.45 0.93 1.09

ADD HYD (8226)
 1 2 3
 ID1= 1 (0354): 262.68 0.468 8.00 5.03
 + ID2= 2 (6354): 1689.38 5.795 9.00 9.45
 ID = 3 (8226): 1952.06 6.251 8.75 8.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHED (0344) Area (ha)= 344.00 Curve Number (CN)= 59.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.87

Unit Hyd Qpeak (cms)= 6.790

PEAK FLOW (cms)= 1.873 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 10.666
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.196

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3402) Area (ha)= 234.60
 ID= 1 DT=15.0 min Total Imp(%)= 28.00 Dir. Conn.(%)= 13.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 65.69 168.91
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1250.60 40.00
 Mannings n = 0.013 0.250

Max. Ref. Inten.(mm/hr)= 71.81 9.73
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 16.35 (ii) 43.52 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.07 0.03

PEAK FLOW (cms)= 4.27 2.34 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.50 5.191 (iii)
 RUNOFF VOLUME (mm)= 52.40 9.09 14.72
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.17 0.27

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 48.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3401) Area (ha)= 51.20
 ID= 1 DT=15.0 min Total Imp(%)= 46.00 Dir. Conn.(%)= 21.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 23.55 27.65
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 584.24 40.00
 Mannings n = 0.013 0.250

Max. Ref. Inten.(mm/hr)= 71.81 14.25
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 10.35 (ii) 33.67 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.09 0.03

PEAK FLOW (cms)= 1.79 0.64 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.50 2.052 (iii)
 RUNOFF VOLUME (mm)= 52.40 10.87 19.59
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.20 0.36

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 48.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5340)
 IN= 2 -> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	5.3860	1.7877
0.1730	0.6348	7.2780	2.2185
1.8360	0.8250	9.1880	2.6486
2.1540	1.1051	9.5880	2.6586

3.3950 1.3506 | 0.0000 0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (3401)	51.200	2.052	6.00	19.59
OUTFLOW: ID= 1 (5340)	51.200	0.284	8.00	19.57

PEAK FLOW REDUCTION [Qout/Qin](%)= 13.83
 TIME SHIFT OF PEAK FLOW (min)=120.00
 MAXIMUM STORAGE USED (ha.m.)= 0.6479

ADD HYD (8232)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (3402):	234.60	5.191	6.00	14.72
+ ID2= 2 (5340):	51.20	0.284	8.00	19.57

ID = 3 (8232):	285.80	5.262	6.00	15.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0342)	1004.58	54.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 2.57	

Unit Hyd Qpeak (cms)= 6.686

PEAK FLOW (cms)= 2.057 (i)
 TIME TO PEAK (hrs)= 10.250
 RUNOFF VOLUME (mm)= 9.159
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.168

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0422)	780.20	54.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 1.95	

Unit Hyd Qpeak (cms)= 6.838

PEAK FLOW (cms)= 1.954 (i)
 TIME TO PEAK (hrs)= 9.250
 RUNOFF VOLUME (mm)= 9.147
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.168

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0410)	572.01	48.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 1.46	

Unit Hyd Qpeak (cms)= 6.698

PEAK FLOW (cms)= 1.460 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 7.475
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.137

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0408)	231.62	58.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 0.64	

Unit Hyd Qpeak (cms)= 6.198

PEAK FLOW (cms)= 1.542 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 10.251
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.188

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

NASHYD (0376)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 1.07	

Unit Hyd Qpeak (cms)= 7.380

PEAK FLOW (cms)= 3.568 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 17.440
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.321

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0374)	545.70	61.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 1.51	

Unit Hyd Qpeak (cms)= 6.158

PEAK FLOW (cms)= 2.092 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 11.459
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.211

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0372)	110.42	37.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 0.96	

Unit Hyd Qpeak (cms)= 1.954

PEAK FLOW (cms)= 0.255 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 5.010
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.092

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0370)	191.85	63.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 0.67	

Unit Hyd Qpeak (cms)= 4.860

PEAK FLOW (cms)= 1.448 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 12.065
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.222

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0368)	159.48	46.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 1.12	

Unit Hyd Qpeak (cms)= 2.433

PEAK FLOW (cms)= 0.461 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 6.960
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.128

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0366)	462.62	62.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 1.06	

Unit Hyd Qpeak (cms)= 7.451

PEAK FLOW (cms)= 2.387 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 11.788
 TOTAL RAINFALL (mm)= 54.400

RUNOFF COEFFICIENT = 0.217

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0364) | Area (ha)= 155.27 Curve Number (CN)= 55.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.70

```

Unit Hyd Qpeak (cms)= 3.782

```

PEAK FLOW (cms)= 0.868 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 9.324
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.171

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8302) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (0364): | 155.27 0.868 7.00 9.32
| + ID2= 2 (0366): | 462.62 2.387 7.50 11.79
|-----|
| ID = 3 (8302): | 617.89 3.218 7.25 11.17

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| STANHYD (0362) | Area (ha)= 118.78
| ID= 1 DT=15.0 min | Total Imp(%)= 22.00 Dir. Conn.(%)= 8.00
|-----|
| IMPERVIOUS PERVIOUS (i)
| Surface Area (ha)= 26.13 92.65
| Dep. Storage (mm)= 2.00 5.00
| Average Slope (%)= 0.50 0.50
| Length (m)= 889.87 40.00
| Mannings n = 0.013 0.250
|
| Max.Eff.Inten.(mm/hr)= 71.81 8.34
| over (min) 15.00 45.00
| Storage Coeff. (min)= 13.33 (ii) 42.23 (ii)
| Unit Hyd. Tpeak (min)= 15.00 45.00
| Unit Hyd. peak (cms)= 0.08 0.03
|
|-----|
| PEAK FLOW (cms)= 1.45 1.12 *TOTALS* 1.889 (iii)
| TIME TO PEAK (hrs)= 6.00 6.50 6.00
| RUNOFF VOLUME (mm)= 52.40 8.03 11.58
| TOTAL RAINFALL (mm)= 54.40 54.40 54.40
| RUNOFF COEFFICIENT = 0.96 0.15 0.21

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 45.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0358) | Area (ha)= 429.87 Curve Number (CN)= 35.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.03

```

Unit Hyd Qpeak (cms)= 7.091

```

PEAK FLOW (cms)= 0.870 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 4.638
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.085

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0360) | Area (ha)= 138.37 Curve Number (CN)= 46.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.60

```

Unit Hyd Qpeak (cms)= 3.957

```

PEAK FLOW (cms)= 0.641 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 6.867
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.126

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8306) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (0358): | 429.87 0.870 7.50 4.64
| + ID2= 2 (0360): | 138.37 0.641 6.75 6.87
|-----|
| ID = 3 (8306): | 568.24 1.473 7.00 5.18

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CN (8362) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00
|-----|

```

----- DATA FOR SECTION (3621.0) -----

Distance	Elevation	Manning
0.00	261.46	0.0550
27.86	254.23	0.0550
51.07	251.96	0.0550
74.29	250.77	0.0550
97.50	249.91	0.0550
125.36	249.40	0.0550
150.93	247.40	0.0550
155.93	247.33	0.0550 / 0.0350
157.93	246.85	0.0350
159.18	246.65	0.0350
160.18	246.63	0.0350
160.93	246.85	0.0350
161.93	247.18	0.0350 / 0.0550
163.18	248.03	0.0550
168.18	248.58	0.0550
183.18	250.18	0.0550
201.97	252.59	0.0550
213.57	256.02	0.0550
225.18	260.31	0.0550
229.82	261.00	0.0550

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.55	247.18	.556E+04	1.9	1.03	49.95
1.28	247.91	.336E+05	16.7	1.53	33.61
2.00	248.63	.913E+05	53.6	1.81	28.40
2.73	249.36	.185E+06	122.3	2.05	25.18
3.46	250.09	.344E+06	224.6	2.02	25.52
4.19	250.82	.574E+06	417.9	2.25	22.91
4.91	251.54	.855E+06	699.1	2.53	20.38
5.64	252.27	.118E+07	1075.3	2.82	18.26
6.37	253.00	.153E+07	1559.9	3.15	16.38
7.10	253.73	.191E+07	2137.6	3.46	14.89
7.82	254.45	.231E+07	2815.2	3.77	13.67
8.55	255.18	.272E+07	3608.5	4.10	12.58
9.28	255.91	.315E+07	4488.2	4.41	11.69
10.01	256.64	.358E+07	5458.0	4.71	10.95
10.73	257.36	.403E+07	6512.4	4.99	10.32
11.46	258.09	.449E+07	7649.8	5.27	9.78
12.19	258.82	.495E+07	8869.5	5.53	9.32
12.92	259.55	.544E+07	10171.2	5.78	8.91
13.64	260.27	.593E+07	11554.8	6.02	8.55

```

----- hydrograph ----- <-pipe / channel->
|-----|
| AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
| (ha) (cms) (hrs) (mm) (m) (m/s)
| INFLOW: ID= 2 (8306) 568.24 1.47 7.00 5.18 0.44 1.03
|-----|
| OUTFLOW: ID= 1 (8362) 568.24 1.30 6.25 5.18 0.38 1.03

```

```

-----
| ADD HYD (8304) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (0362): | 118.78 1.889 6.00 11.58
| + ID2= 2 (8362): | 568.24 1.303 8.25 5.18
|-----|
| ID = 3 (8304): | 687.02 2.147 6.50 6.29

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8300)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8302):	617.89	3.218	7.25	11.17
+ ID2= 2 (8304):	687.02	2.147	6.50	6.29
ID = 3 (8300):	1304.91	5.155	7.00	8.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6368)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3681.0) ----->				
Distance	Elevation	Manning		
0.00	230.00	0.0370		
18.48	223.26	0.0370		
36.96	223.05	0.0370		
64.67	222.94	0.0370		
110.87	222.86	0.0370		
133.96	222.74	0.0370		
147.82	222.65	0.0370		
170.92	222.31	0.0370		
174.79	222.26	0.0370 / 0.0300	Main Channel	
174.89	221.86	0.0300	Main Channel	
175.54	221.86	0.0300	Main Channel	
176.19	221.86	0.0300	Main Channel	
176.29	222.26	0.0300 / 0.0370	Main Channel	
180.16	222.25	0.0370		
184.78	222.28	0.0370		
189.40	222.31	0.0370		
332.60	222.37	0.0370		
450.00	230.00	0.0370		

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.40	222.26	771E+03	0.3	0.52	41.74
0.81	222.67	784E+05	30.3	0.51	43.17
1.21	223.07	210E+06	123.2	0.77	28.42
1.62	223.48	384E+06	301.0	1.03	21.25
2.03	223.89	563E+06	550.2	1.28	17.06
2.44	224.30	747E+06	860.7	1.51	14.46
2.84	224.70	934E+06	1228.1	1.73	12.68
3.25	225.11	1135E+07	1649.3	1.92	11.37
3.66	225.52	1328E+07	2122.3	2.11	10.37
4.07	225.93	1528E+07	2645.5	2.29	9.58
4.47	226.33	1728E+07	3217.8	2.46	8.92
4.88	226.74	1938E+07	3838.3	2.61	8.38
5.29	227.15	2148E+07	4506.3	2.76	7.92
5.70	227.56	2368E+07	5221.3	2.91	7.52
6.10	227.96	2588E+07	5993.0	3.05	7.17
6.51	228.37	2808E+07	6790.9	3.19	6.87
6.92	228.78	3038E+07	7645.0	3.32	6.60
7.33	229.19	3268E+07	8545.1	3.45	6.35
7.73	229.59	3498E+07	9491.0	3.57	6.13

<---- hydrograph ---->						<-pipe / channel->	
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL		
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW : ID= 2 (8300)	1304.91	5.16	7.00	8.60	0.47	0.52	
OUTFLOW : ID= 1 (6368)	1304.91	4.77	8.25	8.60	0.46	0.52	

ADD HYD (8298)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0368):	159.48	0.461	7.75	6.96
+ ID2= 2 (6368):	1304.91	4.771	8.25	8.60
ID = 3 (8298):	1464.39	5.224	8.25	8.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8296)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0370):	191.85	1.448	6.75	12.06
+ ID2= 2 (8298):	1464.39	5.224	8.25	8.42
ID = 3 (8296):	1656.24	6.423	8.00	8.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6372)

IN= 2--> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3721.0) ----->			
Distance	Elevation	Manning	
0.00	225.00	0.0390	
30.80	219.38	0.0390	
61.61	219.30	0.0390	
77.01	219.27	0.0390	
469.76	219.14	0.0390	
477.46	219.13	0.0390	
485.16	219.10	0.0390	
492.86	219.09	0.0390	
495.56	219.09	0.0390 / 0.0310	Main Channel
495.66	218.51	0.0310	Main Channel
500.56	218.51	0.0310	Main Channel
505.46	218.51	0.0310	Main Channel
505.56	219.09	0.0310 / 0.0390	Main Channel
508.26	219.09	0.0390	
515.96	219.10	0.0390	
523.67	219.21	0.0390	
562.17	219.32	0.0390	
654.58	219.43	0.0390	
731.59	219.46	0.0390	
762.39	225.00	0.0390	

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.29	218.80	431E+04	1.7	0.58	43.48
0.58	219.09	867E+04	5.1	0.89	28.32
0.93	219.44	200E+06	58.2	0.44	57.35
1.28	219.79	568E+06	278.6	0.74	33.99
1.62	220.13	939E+06	630.2	1.01	24.84
1.97	220.48	131E+07	1090.4	1.25	20.05
2.32	220.83	169E+07	1647.7	1.47	17.06
2.67	221.18	206E+07	2294.6	1.68	14.99
3.01	221.52	244E+07	3025.6	1.87	13.46
3.36	221.87	282E+07	3836.5	2.05	12.27
3.71	222.22	321E+07	4723.7	2.22	11.31
4.06	222.57	359E+07	5684.6	2.39	10.53
4.40	222.91	398E+07	6716.6	2.55	9.87
4.75	223.26	437E+07	7817.7	2.70	9.31
5.10	223.61	476E+07	8986.2	2.85	8.83
5.45	223.96	515E+07	10220.4	2.99	8.40
5.79	224.30	555E+07	11519.1	3.13	8.03
6.14	224.65	594E+07	12880.8	3.27	7.69
6.49	225.00	634E+07	14304.5	3.40	7.39

<---- hydrograph ---->						<-pipe / channel->	
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL		
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW : ID= 2 (8296)	1656.24	6.42	8.00	8.84	0.59	0.87	
OUTFLOW : ID= 1 (6372)	1656.24	6.26	8.50	8.84	0.59	0.87	

ADD HYD (8294)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0372):	116.42	0.255	7.50	5.01
+ ID2= 2 (6372):	1656.24	6.262	8.50	8.84
ID = 3 (8294):	1766.66	6.496	8.50	8.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8292)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0374):	545.70	2.092	8.25	11.46
+ ID2= 2 (8294):	1766.66	6.496	8.50	8.60
ID = 3 (8292):	2312.36	8.581	8.50	9.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5505)			
IN= 2--> OUT= 1	DT= 15.0 min	OUTFLOW	STORAGE
(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	65.1290	345.3754
25.4850	24.6697	84.9510	456.3890
31.1490	98.6787	*****	838.7689
39.6440	*****	*****	838.7789
48.1390	*****	0.0000	0.0000

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)

INFLOW : ID= 2 (8292) 2312.360 8.581 8.50 9.28
OUTFLOW: ID= 1 (5505) 2312.360 6.211 11.00 9.28

PEAK FLOW REDUCTION [Qout/Qin](%)= 72.38
TIME SHIFT OF PEAK FLOW (min)=150.00
MAXIMUM STORAGE USED (ha.m.)= 6.0141

| ADD HYD (8272) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
ID1= 1 (0376): 463.85 3.568 7.50 17.44
+ ID2= 2 (5505): 2312.36 6.211 11.00 9.28
=====

ID = 3 (8272): 2776.21 8.592 10.25 10.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| CALIB |
| NASHYD (0396) | Area (ha)= 305.21 Curve Number (CN)= 69.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.08

Unit Hyd Qpeak (cms)= 4.811

PEAK FLOW (cms)= 1.961 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 14.789
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.272

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHYD (0394) | Area (ha)= 325.45 Curve Number (CN)= 53.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.92

Unit Hyd Qpeak (cms)= 6.013

PEAK FLOW (cms)= 1.379 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 8.784
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.161

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHYD (0390) | Area (ha)= 420.00 Curve Number (CN)= 55.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 6.683

PEAK FLOW (cms)= 1.700 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 9.400
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.173

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHYD (0388) | Area (ha)= 220.77 Curve Number (CN)= 58.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.99

Unit Hyd Qpeak (cms)= 3.819

PEAK FLOW (cms)= 1.051 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 10.350
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.190

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHYD (0386) | Area (ha)= 241.27 Curve Number (CN)= 61.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.90

Unit Hyd Qpeak (cms)= 4.562

PEAK FLOW (cms)= 1.360 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 11.386
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.209

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| ADD HYD (8286) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
ID1= 1 (0386): 241.27 1.360 7.25 11.39
+ ID2= 2 (0388): 220.77 1.051 7.50 10.35
=====

ID = 3 (8286): 462.04 2.411 7.25 10.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| CALIB |
| NASHYD (0384) | Area (ha)= 199.07 Curve Number (CN)= 44.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 3.537

PEAK FLOW (cms)= 0.598 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 6.477
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.119

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHYD (0380) | Area (ha)= 182.01 Curve Number (CN)= 40.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.55

Unit Hyd Qpeak (cms)= 5.609

PEAK FLOW (cms)= 0.715 (i)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 5.531
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.102

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHYD (0382) | Area (ha)= 216.59 Curve Number (CN)= 53.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.64

Unit Hyd Qpeak (cms)= 5.733

PEAK FLOW (cms)= 1.207 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 8.712
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.160

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| ADD HYD (8290) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
ID1= 1 (0380): 182.01 0.715 6.50 5.53
+ ID2= 2 (0382): 216.59 1.207 6.75 8.71
=====

ID = 3 (8290): 398.60 1.920 6.75 7.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ROUTE CHN (6384) |
| IN= 2 -> OUT= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (3841.0) ----->
Distance Elevation Manning
0.00 294.40 0.0360

10.59	291.93	0.0380	
21.17	289.19	0.0380	
26.46	287.99	0.0380	
31.76	286.79	0.0380	
71.45	279.97	0.0380	
74.10	279.79	0.0380	
76.74	279.71	0.0380	
78.99	279.64	0.0380 / 0.0300	Main Channel
79.14	279.30	0.0300	Main Channel
79.39	279.30	0.0300	Main Channel
79.64	279.30	0.0300	Main Channel
79.79	279.64	0.0300 / 0.0380	Main Channel
89.98	279.78	0.0380	
119.09	281.86	0.0380	
145.55	282.87	0.0380	
198.48	284.85	0.0380	
211.71	286.31	0.0380	
230.23	287.59	0.0380	
261.99	294.00	0.0380	

TRAVEL TIME TABLE						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.34	279.64	.715E+03	0.2	0.65	63.42	
1.10	280.40	.507E+05	21.9	1.40	38.54	
1.85	281.15	.141E+06	91.8	2.10	25.69	
2.61	281.91	.269E+06	218.9	2.63	20.46	
3.36	282.66	.445E+06	407.1	2.96	18.23	
4.12	283.42	.681E+06	698.3	3.31	16.26	
4.87	284.17	.977E+06	1111.8	3.68	14.65	
5.63	284.93	.133E+07	1674.3	4.06	13.27	
6.39	285.69	.173E+07	2457.8	4.59	11.73	
7.14	286.44	.215E+07	3378.8	5.07	10.63	
7.90	287.20	.261E+07	4418.4	5.47	9.85	
8.65	287.95	.310E+07	5676.3	5.92	9.11	
9.41	288.71	.361E+07	7150.5	6.40	8.42	
10.17	289.47	.414E+07	8783.1	6.86	7.86	
10.92	290.22	.469E+07	10575.1	7.30	7.38	
11.68	290.98	.525E+07	12520.4	7.72	6.98	
12.43	291.73	.582E+07	14619.0	8.12	6.64	
13.19	292.49	.642E+07	16863.2	8.50	6.34	
13.94	293.24	.703E+07	19259.8	8.86	6.08	

<---- hydrograph ----> <-pipe / channel->						
INFLW	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
ID=	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
ID= 2 (8290)	398.60	1.92	6.75	7.26	0.40	0.88
OUTFLOW: ID= 1 (6384)	398.60	1.50	8.00	7.26	0.39	0.87

ADD HYD (8288)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0384)	199.07	0.598	7.25	6.48
+ ID2= 2 (6384)	398.60	1.505	8.00	7.26
ID = 3 (8288)	597.67	2.089	8.00	7.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8284)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8286)	462.04	2.411	7.25	10.89
+ ID2= 2 (8288)	597.67	2.089	8.00	7.00
ID = 3 (8284)	1059.71	4.450	7.75	8.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8280)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0390)	420.00	1.700	7.50	9.40
+ ID2= 2 (8284)	1059.71	4.450	7.75	8.70
ID = 3 (8280)	1479.71	6.150	7.75	8.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB					
NASHYD (0392)	Area	(ha)= 167.22	Curve Number	(CN)= 62.0	
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50		
	U.H. Tp(hrs)=	0.74			

Unit Hyd Qpeak (cms)= 3.837

PEAK FLOW (cms)= 1.133 (1)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 11.712
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.215

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB					
NASHYD (0378)	Area	(ha)= 606.72	Curve Number	(CN)= 55.0	
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50		
	U.H. Tp(hrs)=	1.18			

Unit Hyd Qpeak (cms)= 8.771

PEAK FLOW (cms)= 2.295 (1)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 9.412
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.173

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8282)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0378)	606.72	2.295	8.00	9.41
+ ID2= 2 (0392)	167.22	1.133	7.00	11.71
ID = 3 (8282)	773.94	3.350	7.50	9.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8278)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8280)	1479.71	6.150	7.75	8.90
+ ID2= 2 (8282)	773.94	3.350	7.50	9.91
ID = 3 (8278)	2253.65	9.491	7.50	9.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6394)	
IR= 2--> ODF= 1	Routing time step (min)= 15.00

<----- DATA FOR SECTION (3941.0) ----->

Distance	Elevation	Manning
0.00	283.00	0.0380
13.48	282.87	0.0380
53.92	280.08	0.0380
74.13	276.62	0.0380
97.72	265.45	0.0380
114.57	256.93	0.0380
131.42	253.04	0.0380
134.79	252.58	0.0380
138.53	251.74	0.0380 / 0.0300
139.03	251.20	0.0300
141.53	251.20	0.0300
144.03	251.20	0.0300
144.53	251.74	0.0300 / 0.0380
148.27	252.69	0.0380
151.64	252.97	0.0380
165.34	255.08	0.0380
219.03	257.54	0.0380
262.84	259.43	0.0380
310.02	262.80	0.0380
333.60	283.00	0.0380

TRAVEL TIME TABLE						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.54	251.74	.827E+04	4.5	1.51	30.71	
2.19	253.39	.758E+05	73.2	2.69	17.27	
3.83	255.03	.282E+06	331.8	3.27	14.17	
5.48	256.68	.633E+06	917.3	4.03	11.51	
7.12	258.32	.112E+07	1889.5	4.69	9.90	
8.77	259.97	.179E+07	3410.2	5.21	8.73	
10.41	261.61	.259E+07	5927.6	6.05	7.66	
12.06	263.26	.351E+07	8605.1	6.83	6.79	
13.70	264.90	.447E+07	12544.9	7.81	5.94	
15.35	266.55	.546E+07	17101.4	8.71	5.32	

16.39	268.19	.648E+07	22251.7	9.56	4.85
18.64	269.84	.752E+07	27979.0	10.36	4.48
20.28	271.48	.858E+07	34269.0	11.11	4.17
21.93	273.13	.967E+07	41110.8	11.83	3.92
23.57	274.77	.108E+08	48495.9	12.51	3.71
25.22	276.42	.119E+08	56418.1	13.17	3.52
26.86	278.06	.131E+08	64410.6	13.69	3.39
28.51	279.71	.143E+08	73002.1	14.18	3.27
30.15	281.35	.156E+08	81379.3	14.50	3.20

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<---- hydrograph ----> <-pipe / channel->
      AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
      (ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
INFLOW : ID= 2 (8278) 2253.65  9.49  7.50  9.24  0.66  1.56
OUTFLOW: ID= 1 (6394) 2253.65  9.22  8.25  9.24  0.65  1.56

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ADD HYD (8276) |
1 + 2 = 3 |
      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)   (hrs)   (mm)
+ ID1= 1 (0394): 325.45  1.379  7.25  8.78
+ ID2= 2 (6394): 2253.65  9.215  8.25  9.24
ID = 3 (8276): 2579.10  10.506  8.00  9.19

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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ROUTE CHN (6396) |
IN= 2--> OUT= 1 |
Routing time step (min)= 15.00

```

<----- DATA FOR SECTION (3961.0) ----->

Distance	Elevation	Manning
0.00	263.00	0.0410
11.75	257.14	0.0410
23.50	253.97	0.0410
41.13	247.83	0.0410
76.38	232.09	0.0410
135.13	229.07	0.0410
149.82	228.97	0.0410
152.75	228.96	0.0410 / 0.0300
154.19	228.73	0.0300
154.69	228.20	0.0300
155.69	228.20	0.0300
156.69	228.20	0.0300
157.19	228.73	0.0300
158.63	228.95	0.0300 / 0.0410
161.57	228.96	0.0410
164.51	229.71	0.0410
196.82	241.70	0.0410
223.26	249.21	0.0410
246.76	255.13	0.0410
290.82	263.51	0.0410

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.75	228.95	.780E+04	2.2	0.98	58.06
2.54	230.74	.277E+06	145.1	1.78	31.76
4.33	232.53	.786E+06	609.1	2.63	21.50
6.13	234.33	.140E+07	1483.5	3.59	15.77
7.92	236.12	.207E+07	2677.8	4.39	12.90
9.71	237.91	.280E+07	4184.8	5.09	11.14
11.50	239.70	.357E+07	6001.4	5.71	9.93
13.29	241.49	.441E+07	8123.1	6.27	9.03
15.09	243.29	.530E+07	10549.4	6.77	8.37
16.88	245.08	.625E+07	13300.6	7.24	7.83
18.67	246.87	.726E+07	16396.2	7.68	7.38
20.46	248.66	.834E+07	19809.8	8.07	7.02
22.26	250.46	.949E+07	23532.5	8.43	6.72
24.05	252.25	.107E+08	27641.2	8.77	6.46
25.84	254.04	.120E+08	32155.2	9.10	6.22
27.63	255.83	.134E+08	36901.0	9.37	6.05
29.42	257.62	.149E+08	42147.7	9.64	5.88
31.22	259.42	.164E+08	48149.1	9.97	5.68
33.01	261.21	.181E+08	54644.8	10.29	5.51

```

<---- hydrograph ----> <-pipe / channel->
      AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
      (ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
INFLOW : ID= 2 (8276) 2579.10  10.51  8.00  9.19  0.85  1.00
OUTFLOW: ID= 1 (6396) 2579.10  9.52  9.25  9.19  0.84  1.00

```

```

ADD HYD (8274) |
1 + 2 = 3 |
      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)   (hrs)   (mm)
ID1= 1 (0396): 305.21  1.961  7.50  14.79

```

```

+ ID2= 2 (6396): 2579.10  9.515  9.25  9.19
ID = 3 (8274): 2884.31  11.197  9.00  9.78

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8270) |
1 + 2 = 3 |
      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)   (hrs)   (mm)
ID1= 1 (8272): 2776.21  8.592  10.25  10.64
+ ID2= 2 (8274): 2884.31  11.197  9.00  9.78
ID = 3 (8270): 5660.52  19.501  9.50  10.20

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

RESERVOIR (5506) |
IN= 2--> OUT= 1 |
DT= 15.0 min |
      OUTFLOW   STORAGE   OUTFLOW   STORAGE
      (cms)     (ha.m.)   (cms)     (ha.m.)
0.0000   0.0000   60.8810   135.6832
31.1490  24.6697   96.2770   900.4431
36.8120  37.0045   96.6770   900.4531
45.3070  86.3439   0.0000   0.0000

```

```

INFLOW : ID= 2 (8270) 5660.520  19.501  9.50  10.20
OUTFLOW: ID= 1 (5506) 5660.520  16.142  11.75  10.20

```

PEAK FLOW REDUCTION [Qout/Qin](%)= 82.78
TIME SHIFT OF PEAK FLOW (min)=135.00
MAXIMUM STORAGE USED (ha.m.)= 12.7854

```

CALIB (0406) |
NASHVD (0406) | Area (ha)= 142.65 Curve Number (CN)= 66.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.59

```

Unit Hyd Qpeak (cms)= 4.135

PEAK FLOW (cms)= 1.319 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 13.237
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.243

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB (0404) |
NASHVD (0404) | Area (ha)= 246.46 Curve Number (CN)= 47.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.98

```

Unit Hyd Qpeak (cms)= 4.280

PEAK FLOW (cms)= 0.810 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 7.191
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.132

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB (0402) |
NASHVD (0402) | Area (ha)= 244.00 Curve Number (CN)= 61.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.07

```

Unit Hyd Qpeak (cms)= 3.879

PEAK FLOW (cms)= 1.206 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 11.417
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.210

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB (0400) |
NASHVD (0400) | Area (ha)= 93.97 Curve Number (CN)= 52.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50

```

U.H. Tp(hrs)= 0.44
 Unit Hyd Qpeak (cms) = 3.630
 PEAK FLOW (cms) = 0.675 (1)
 TIME TO PEAK (hrs) = 6.500
 RUNOFF VOLUME (mm) = 8.305
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.153

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHYD (0398) | Area (ha) = 328.19 Curve Number (CN) = 55.0
 ID= 1 DT=15.0 min | Ia (mm) = 5.00 # of Linear Res.(N) = 1.50
 U.H. Tp(hrs) = 0.83

Unit Hyd Qpeak (cms) = 6.759
 PEAK FLOW (cms) = 1.617 (1)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 9.360
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.172

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8268) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0398): 328.19 1.617 7.00 9.36
 + ID2= 2 (0400): 93.97 0.675 6.50 8.30
 ID = 3 (8268): 422.16 2.224 6.75 9.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6402) |
 IN= 2--> OUT= 1 | Routing time step (min) = 15.00

<----- DATA FOR SECTION (4021.0) ----->

Distance	Elevation	Manning	
0.00	238.50	0.0360	
11.50	238.00	0.0360	
23.00	237.93	0.0360	
34.49	236.39	0.0360	
63.24	233.98	0.0360	
97.73	228.15	0.0360	
123.60	227.08	0.0360	
126.48	226.61	0.0360	
127.60	226.47	0.0360 / 0.0330	Main Channel
127.85	225.25	0.0330	Main Channel
129.35	225.25	0.0330	Main Channel
130.85	225.25	0.0330	Main Channel
131.10	226.47	0.0330 / 0.0360	Main Channel
132.22	226.59	0.0360	
143.72	227.42	0.0360	
169.59	227.88	0.0360	
192.59	231.19	0.0360	
218.46	233.02	0.0360	
241.45	235.50	0.0360	
284.57	236.43	0.0360	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.41	225.66	.391E+04	1.7	1.36	38.31
0.81	226.06	.804E+04	5.0	1.93	27.02
1.22	226.47	.124E+05	9.1	2.30	22.61
1.84	227.09	.309E+05	23.3	2.36	22.03
2.46	227.71	.904E+05	62.0	2.14	24.31
3.09	228.34	.223E+06	188.3	2.63	19.75
3.71	228.96	.379E+06	410.2	3.38	15.39
4.33	229.58	.550E+06	711.5	4.04	12.89
4.95	230.20	.737E+06	1091.3	4.62	11.25
5.58	230.83	.939E+06	1550.6	5.15	10.10
6.20	231.45	.116E+07	2072.4	5.59	9.31
6.82	232.07	.140E+07	2655.2	5.93	8.77
7.44	232.69	.167E+07	3345.8	6.27	8.30
8.07	233.32	.195E+07	4159.0	6.64	7.83
8.69	233.94	.226E+07	5100.8	7.03	7.39
9.31	234.56	.259E+07	6081.3	7.32	7.11
9.93	235.18	.295E+07	7183.1	7.60	6.84
10.56	235.81	.334E+07	8145.2	7.62	6.83
11.18	236.43	.379E+07	9116.0	7.51	6.92

<---- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL

INFLOW : ID= 2 (8268) 422.16 (ha) (cms) (hrs) (mm) (m) (m/s)
 2.22 6.75 9.12 0.47 1.43
 OUTFLOW: ID= 1 (6402) 422.16 2.00 7.75 9.12 0.44 1.39

ADD HYD (8266) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0402): 244.00 1.206 7.50 11.42
 + ID2= 2 (6402): 422.16 1.996 7.75 9.12
 ID = 3 (8266): 666.16 3.200 7.75 9.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8264) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0404): 246.46 0.810 7.50 7.19
 + ID2= 2 (8266): 666.16 3.200 7.75 9.96
 ID = 3 (8264): 912.62 4.005 7.75 9.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8262) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0406): 142.65 1.319 6.75 13.24
 + ID2= 2 (8264): 912.62 4.005 7.75 9.12
 ID = 3 (8262): 1055.27 5.146 7.25 9.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8260) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (5506): 5660.52 16.142 11.75 10.20
 + ID2= 2 (8262): 1055.27 5.146 7.25 9.76
 ID = 3 (8260): 6715.79 18.516 11.25 10.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8258) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0408): 231.62 1.542 6.75 10.25
 + ID2= 2 (8260): 6715.79 18.516 11.25 10.13
 ID = 3 (8258): 6947.41 18.994 11.00 10.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8256) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0410): 572.01 1.460 8.25 7.48
 + ID2= 2 (8258): 6947.41 18.994 11.00 10.14
 ID = 3 (8256): 7519.42 20.184 11.00 9.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5507) |
 IN= 2--> OUT= 1 | DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	90.6140	67.8416
39.6440	12.3348	*****	160.3529
48.1300	18.5023	*****	160.3529
67.9600	37.0045	0.0000	0.0000

AREA QPEAK TPEAK R.V.

INFLW : ID= 2 (8256) (ha) (cms) (hrs) (mm)
 7519.421 20.184 11.00 9.93
 OUTFLOW: ID= 1 (5507) 7519.421 19.753 12.00 9.93

PEAK FLOW REDUCTION [Qout/Qin](%)= 97.87
 TIME SHIFT OF PEAK FLOW (min)= 60.00
 MAXIMUM STORAGE USED (ha.m.)= 6.1500

CALIB
 NASHYD (0420) Area (ha)= 175.82 Curve Number (CN)= 53.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.81

Unit Hyd Qpeak (cms)= 3.692
 PEAK FLOW (cms)= 0.822 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 8.762
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.161

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0418) Area (ha)= 182.79 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.05

Unit Hyd Qpeak (cms)= 2.966
 PEAK FLOW (cms)= 1.014 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 12.572
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.231

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0416) Area (ha)= 439.20 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.29

Unit Hyd Qpeak (cms)= 5.832
 PEAK FLOW (cms)= 2.108 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 12.603
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.232

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0412) Area (ha)= 238.70 Curve Number (CN)= 54.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.80

Unit Hyd Qpeak (cms)= 5.088
 PEAK FLOW (cms)= 1.169 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 9.052
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.166

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANSHYD (4141) Area (ha)= 43.70
 ID= 1 DT=15.0 min Total Imp(%)= 36.00 Dir. Conn.(%)= 17.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	15.73	27.97
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	539.75	40.00
Mannings n	=	0.013	0.250
Max. Eff. Inten. (mm/hr)=		71.81	18.99
over (min)		15.00	45.00
Storage Coeff. (min)=		9.87 (ii)	30.67 (iii)
Unit Hyd. Tpeak (min)=		15.00	45.00
Unit Hyd. peak (cms)=		0.09	0.03

TOTALS

PEAK FLOW (cms)= 1.26 0.90 1.628 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 52.40 15.90 22.10
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.29 0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 64.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5414)
 IM= 2--> OUF= 1
 DT= 15.0 min

		OUTFLOW	STORAGE	OUTFLOW	STORAGE
		(cms)	(ha.m.)	(cms)	(ha.m.)
		0.0000	0.0000	3.4720	1.9177
		0.3820	0.3152	3.4720	1.9277
		1.8280	0.4690	0.0000	0.0000

		AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
INFLW : ID= 2 (4141)		43.700	1.628	6.00	22.10
OUTFLOW: ID= 1 (5414)		43.700	0.795	6.75	22.10

PEAK FLOW REDUCTION [Qout/Qin](%)= 48.83
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 0.3604

CALIB
 STANSHYD (4142) Area (ha)= 144.30
 ID= 1 DT=15.0 min Total Imp(%)= 21.00 Dir. Conn.(%)= 10.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	30.30	114.00
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	980.82	40.00
Mannings n	=	0.013	0.250
Max. Eff. Inten. (mm/hr)=		71.81	14.82
over (min)		15.00	45.00
Storage Coeff. (min)=		14.13 (ii)	37.09 (iii)
Unit Hyd. Tpeak (min)=		15.00	45.00
Unit Hyd. peak (cms)=		0.07	0.03

		PEAK FLOW	TPEAK	R.V.
		(cms)	(hrs)	(mm)
		2.15	2.62	3.220 (iii)
		6.00	6.50	6.50
		52.40	14.26	18.07
		54.40	54.40	54.40
		0.96	0.26	0.33

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 64.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8254)
 1 + 2 = 3

		AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (4142):		144.30	3.220	6.50	18.07
+ ID2= 2 (5414):		43.70	0.795	6.75	22.10
=====					
ID = 3 (8254):		188.00	3.727	6.50	19.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8252)
 1 + 2 = 3

		AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0412):		238.70	1.169	7.00	9.05
+ ID2= 2 (8254):		188.00	3.727	6.50	19.01
=====					
ID = 3 (8252):		426.70	4.816	6.50	13.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.


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-----
| ROUTE CHN (6416) |
| IN= 2--> OUT= 1 |
-----
| Routing time step (min)= 15.00
-----
|<----- DATA FOR SECTION (4161.0) ----->
| Distance      Elevation      Manning
| 0.00          270.07          0.0340
| 20.67         267.91          0.0340
| 62.01         264.33          0.0340
| 113.69        259.75          0.0340
| 165.37        253.30          0.0340
| 227.38        246.29          0.0340
| 232.55        246.03          0.0340
| 237.72        246.16          0.0340
| 241.39        246.02          0.0340 / 0.0300 Main Channel
| 241.64        245.75          0.0300 Main Channel
| 242.89        245.75          0.0300 Main Channel
| 244.14        245.75          0.0300 Main Channel
| 244.39        246.02          0.0300 / 0.0340 Main Channel
| 248.06        246.20          0.0340
| 253.22        246.28          0.0340
| 258.39        246.63          0.0340
| 346.25        252.57          0.0340
| 413.43        257.77          0.0340
| 465.11        261.78          0.0340
| 511.62        270.00          0.0340
-----

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-----
|<----- TRAVEL TIME TABLE ----->
| DEPTH  ELVY  VOLUME  FLOW RATE  VELOCITY  TRAV.TIME
| (m)    (m)    (cu.m.) (cms)      (m/s)     (min)
| 0.27   246.02  .295E+04  0.6        0.75      88.26
| 1.53   247.28  .168E+06  69.7       1.65      40.13
| 2.79   246.54  .490E+06  299.5      2.42      27.28
| 4.06   249.81  .962E+06  740.1      3.05      21.67
| 5.32   251.07  .158E+07  1440.0     3.61      18.33
| 6.58   252.33  .235E+07  2443.8     4.12      16.06
| 7.84   253.59  .327E+07  3811.2     4.63      14.30
| 9.10   254.85  .432E+07  5565.1     5.11      12.94
| 10.37  256.12  .550E+07  7721.3     5.57      11.88
| 11.63  257.38  .682E+07  10309.9    6.00      11.02
| 12.89  258.64  .827E+07  13360.6    6.42      10.31
| 14.15  259.90  .985E+07  16884.5    6.81      9.72
| 15.42  261.17  .116E+08  20818.4    7.14      9.26
| 16.68  262.43  .134E+08  25510.4    7.53      8.78
| 17.94  263.69  .154E+08  30958.0    7.97      8.30
| 19.20  264.95  .175E+08  36988.3    8.38      7.89
| 20.46  266.21  .197E+08  43617.1    8.78      7.53
| 21.73  267.48  .220E+08  50865.1    9.17      7.22
| 22.99  268.74  .244E+08  58854.0    9.56      6.92
-----

```

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-----
|<---- hydrograph ----> <-pipe / channel->
| AREA  QPEAK  TPEAK  R.V.  MAX DEPTH  MAX VEL
| (ha)  (cms)  (hrs)  (mm)  (m)         (m/s)
| INFLOW : ID= 2 (8252) 426.70  4.82  6.50  13.44  0.35  0.78
| OUTFLOW: ID= 1 (6416) 426.70  2.78  7.50  13.44  0.31  0.76
-----

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-----
| ADD HYD (8250) |
| 1 + 2 = 3 |
| AREA  QPEAK  TPEAK  R.V.
| (ha)  (cms)  (hrs)  (mm)
| ID1= 1 (0416): 439.30  2.108  8.00  12.60
| + ID2= 2 (6416): 426.70  2.775  7.50  13.44
|=====
| ID = 3 (8250): 866.00  4.861  7.75  13.01
-----
| NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
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-----
| ADD HYD (8248) |
| 1 + 2 = 3 |
| AREA  QPEAK  TPEAK  R.V.
| (ha)  (cms)  (hrs)  (mm)
| ID1= 1 (0418): 182.79  1.014  7.50  12.57
| + ID2= 2 (8250): 866.00  4.861  7.75  13.01
|=====
| ID = 3 (8248): 1048.79  5.871  7.75  12.94
-----
| NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
-----

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-----
| ADD HYD (8246) |
| 1 + 2 = 3 |
| AREA  QPEAK  TPEAK  R.V.
| (ha)  (cms)  (hrs)  (mm)
| ID1= 1 (0420): 175.82  0.822  7.00  8.76
| + ID2= 2 (8248): 1048.79  5.871  7.75  12.94
|=====
| ID = 3 (8246): 1224.61  6.658  7.50  12.34
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8244) |
| 1 + 2 = 3 |
| AREA  QPEAK  TPEAK  R.V.
| (ha)  (cms)  (hrs)  (mm)
| ID1= 1 (5507): 7519.42  19.753  12.00  9.93
| + ID2= 2 (8246): 1224.61  6.658  7.50  12.34
|=====
| ID = 3 (8244): 8744.03  23.417  11.25  10.27
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8242) |
| 1 + 2 = 3 |
| AREA  QPEAK  TPEAK  R.V.
| (ha)  (cms)  (hrs)  (mm)
| ID1= 1 (0422): 780.20  1.954  9.25  9.15
| + ID2= 2 (8244): 8744.03  23.417  11.25  10.27
|=====
| ID = 3 (8242): 9524.23  25.245  11.00  10.18
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| RESERVOIR (5508) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
|-----|-----|-----|-----|
| OUTFLOW  STORAGE  OUTFLOW  STORAGE
| (cms)    (ha.m.)  (cms)    (ha.m.)
| 0.0000   0.0000  0.0000   197.3574
| 76.4550  30.8371  0.0000   394.7148
| *****  61.6742  0.0000   394.7248
| *****  0.0000  0.0000   0.0000
|-----|-----|-----|-----|
| AREA  QPEAK  TPEAK  R.V.
| (ha)  (cms)  (hrs)  (mm)
| INFLOW : ID= 2 (8242) 9524.231  25.245  11.00  10.18
| OUTFLOW: ID= 1 (5508) 9524.231  24.490  12.25  10.18
-----

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```

-----
| PEAK FLOW REDUCTION [Qout/Qin](%)= 97.01
| TIME SPLIT OF PEAK FLOW (min)= 75.00
| MAXIMUM STORAGE USED (ha.m.)= 9.8788
-----

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-----
| CALIB |
| NASHYD (0336) |
| ID= 1 DT=15.0 min |
| Area (ha)=2785.00
| U.H. Tp(hrs)= 15.39
| Curve Number (CN)= 72.0
| # of Linear Res.(N)= 1.50
-----

```

```

-----
| Unit Hyd Qpeak (cms)= 3.090
| PEAK FLOW (cms)= 1.997 (i)
| TIME TO PEAK (hrs)= 22.250
| RUNOFF VOLUME (mm)= 16.446
| TOTAL RAINFALL (mm)= 54.400
| RUNOFF COEFFICIENT = 0.302
-----

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANDHYD (3382) |
| ID= 1 DT=15.0 min |
| Area (ha)= 525.90
| Total Imp(%)= 37.00
| Dir. Conn.(%)= 17.00
|-----|-----|-----|
| Surface Area (ha)= 194.58  331.32
| Dep. Storage (mm)= 2.00  5.00
| Average Slope (%)= 0.50  0.50
| Length (m)= 1872.43  40.00
| Mannings n = 0.013  0.250
|-----|-----|-----|
| Max. Eff. Inten.(mm/hr)= 48.96  20.86
| over (min)= 30.00  45.00
| Storage Coeff. (min)= 24.27 (ii)  44.30 (ii)
| Unit Hyd. Tpeak (min)= 30.00  45.00
| Unit Hyd. peak (cms)= 0.04  0.03
|-----|-----|-----|
| PEAK FLOW (cms)= 8.78  9.65  15.834 (iii)
| TIME TO PEAK (hrs)= 6.25  6.50  6.25
| RUNOFF VOLUME (mm)= 52.40  17.08  23.08
| TOTAL RAINFALL (mm)= 54.40  54.40  54.40
| RUNOFF COEFFICIENT = 0.96  0.31  0.42
|-----|-----|-----|
| ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
| YOU SHOULD CONSIDER SPLITTING THE AREA.
|-----|-----|-----|
| (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
|-----|-----|-----|

```

CN* = 66.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (3381) Area (ha)= 73.00
ID= 1 DT=15.0 min Total Imp(%)= 49.00 Dir. Conn.(%)= 23.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	35.77	37.23	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	697.61	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	71.81	43.61	
over (min)=	15.00	30.00	
Storage Coeff. (min)=	11.52 (ii)	26.43 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.08	0.04	
		TOTALS	
PEAK FLOW (cms)=	2.70	1.99	3.937 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	52.40	18.95	26.64
TOTAL RAINFALL (mm)=	54.40	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.35	0.49

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 66.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.3350	1.2400
0.3230	0.3074	4.3560	1.3520
0.9360	0.5374	5.3290	1.4590
1.7590	0.8302	5.7290	1.4690
2.4110	1.0550	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
INFLOW : ID= 2 (3381)	73.000	3.937	6.00	26.64
OUTFLOW: ID= 1 (5338)	73.000	1.561	6.75	26.64

PEAK FLOW REDUCTION [Qout/Qin](%)= 39.64
TIME SHIFT OF PEAK FLOW (min)= 45.00
MAXIMUM STORAGE USED (ha.m.)= 0.7662

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
INFLOW : ID= 2 (3382)	525.90	15.834	6.25	23.09
+ ID2= 2 (5338)	73.00	1.561	6.75	26.64
=====				
ID= 3 (8310)	598.90	17.031	6.25	23.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHYD (3342) Area (ha)= 587.10 Curve Number (CN)= 56.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 8.19

Unit Hyd Qpeak (cms)= 1.224
PEAK FLOW (cms)= 0.467 (i)
TIME TO PEAK (hrs)= 15.500
RUNOFF VOLUME (mm)= 9.797
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.180

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (3341) Area (ha)= 33.00
ID= 1 DT=15.0 min Total Imp(%)= 51.00 Dir. Conn.(%)= 31.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	16.83	16.17
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	469.04	40.00
Mannings n =	0.013	0.250

Max.Eff.Inten.(mm/hr)=	71.81	15.09
over (min)=	15.00	45.00
Storage Coeff. (min)=	9.08 (ii)	31.87 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.09	0.03

PEAK FLOW (cms)=	1.77	0.41	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.50	1.938 (iii)
RUNOFF VOLUME (mm)=	52.40	11.90	6.00
TOTAL RAINFALL (mm)=	54.40	54.40	24.45
RUNOFF COEFFICIENT =	0.96	0.22	54.40
			0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 52.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.6540	0.4604
0.0390	0.1699	0.8330	0.5249
0.1860	0.2610	1.0130	0.5896
0.3750	0.3534	1.4130	0.5996
0.4340	0.3813	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
INFLOW : ID= 2 (3341)	33.000	1.938	6.00	24.45
OUTFLOW: ID= 1 (5334)	33.000	0.481	6.75	24.42

PEAK FLOW REDUCTION [Qout/Qin](%)= 24.84
TIME SHIFT OF PEAK FLOW (min)= 45.00
MAXIMUM STORAGE USED (ha.m.)= 0.3985

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
INFLOW : ID= 2 (3342)	587.10	0.467	15.50	9.80
+ ID2= 2 (5334)	33.00	0.481	6.75	24.42
=====				
ID= 3 (8314)	620.10	0.655	7.00	10.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHYD (0306) Area (ha)= 283.97 Curve Number (CN)= 52.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 6.44

Unit Hyd Qpeak (cms)= 0.753
PEAK FLOW (cms)= 0.250 (i)
TIME TO PEAK (hrs)= 13.750
RUNOFF VOLUME (mm)= 8.592
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.158

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0286) Area (ha)= 260.51 Curve Number (CN)= 84.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.16

Unit Hyd Qpeak (cms)= 3.834
PEAK FLOW (cms)= 2.734 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 24.753
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.455

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| CALIB |
| NASHYD (0282) | Area (ha)= 449.38 Curve Number (CN)= 77.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 3.47
-----
Unit Hyd Qpeak (cms)= 5.226

PEAK FLOW (cms)= 3.035 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 19.368
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.356

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| CALIB |
| NASHYD (0284) | Area (ha)= 78.93 Curve Number (CN)= 84.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.57
-----
Unit Hyd Qpeak (cms)= 2.344

PEAK FLOW (cms)= 1.445 (1)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 24.380
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.448

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| ADD HYD (8388) |
| 1 + 2 = 3 |
| (ha) (cms) (hrs) (mm)
ID1= 1 (0282): 449.38 3.035 8.00 19.37
+ ID2= 2 (0284): 78.93 1.445 6.50 24.38
-----
ID = 3 (8388): 528.31 4.143 7.25 20.12
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| ROUTE CHN (6286) |
| IN= 2---> OUT= 1 | Routing time step (min)= 15.00
-----
<----- DATA FOR SECTION (2861.0) ----->
Distance Elevation Manning
0.00 233.00 0.0450
20.58 228.51 0.0450
41.17 227.74 0.0450
51.46 227.41 0.0450
97.77 225.96 0.0450
149.23 223.94 0.0450
200.69 220.84 0.0450
226.42 220.66 0.0450
238.85 220.22 0.0450
241.35 220.01 0.0450 / 0.0350 Main Channel
241.85 219.70 0.0350 Main Channel
245.85 219.72 0.0350 Main Channel
246.35 220.06 0.0350 Main Channel
248.85 220.23 0.0350 Main Channel
303.60 221.64 0.0350 Main Channel
380.79 224.98 0.0450
432.25 229.54 0.0450
457.98 233.33 0.0450
483.71 234.27 0.0450
509.44 233.81 0.0450

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-----
<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.31 220.01 .382E+04 0.8 0.57 82.89
1.06 220.76 .525E+05 12.1 0.66 72.51
1.81 221.51 .240E+06 88.6 1.05 45.21
2.56 222.26 .510E+06 261.7 1.46 32.47
3.31 223.01 .843E+06 533.8 1.81 26.34
4.06 223.76 .124E+07 913.6 2.10 22.64
4.81 224.51 .171E+07 1399.0 2.34 20.33
5.56 225.26 .225E+07 2051.0 2.60 18.27
6.31 226.01 .285E+07 2907.0 2.91 16.35
7.06 226.76 .352E+07 3902.5 3.16 15.04
7.82 227.52 .426E+07 5069.9 3.39 14.01
8.57 228.27 .507E+07 6433.0 3.62 13.13
9.32 229.02 .593E+07 8069.2 3.89 12.24
10.07 229.77 .681E+07 9925.7 4.16 11.44
10.82 230.52 .772E+07 11962.8 4.42 10.75
11.57 231.27 .864E+07 14167.1 4.68 10.17
12.32 232.02 .959E+07 16537.1 4.92 9.66

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13.07 232.77 .105E+08 19070.7 5.16 9.22
13.82 233.52 .115E+08 21790.4 5.39 8.82
-----
<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8388) 528.31 4.14 7.25 20.12 0.53 0.60
OUTFLOW: ID= 1 (6286) 528.31 3.51 9.25 20.12 0.49 0.59

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-----
| ADD HYD (8386) |
| 1 + 2 = 3 |
| (ha) (cms) (hrs) (mm)
ID1= 1 (0286): 260.51 2.734 7.50 24.75
+ ID2= 2 (6286): 528.31 3.506 9.25 20.12
-----
ID = 3 (8386): 788.82 5.992 8.50 21.65
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| CALIB |
| NASHYD (0302) | Area (ha)= 473.90 Curve Number (CN)= 58.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.66
-----
Unit Hyd Qpeak (cms)= 4.874

PEAK FLOW (cms)= 1.530 (1)
TIME TO PEAK (hrs)= 8.500
RUNOFF VOLUME (mm)= 10.409
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.191

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| CALIB |
| NASHYD (0300) | Area (ha)= 258.93 Curve Number (CN)= 52.0
| ID= 1 DT=15.0 min | Ia (mm)= 4.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.03
-----
Unit Hyd Qpeak (cms)= 4.292

PEAK FLOW (cms)= 1.017 (1)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 8.830
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.162

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| CALIB |
| NASHYD (0298) | Area (ha)= 330.51 Curve Number (CN)= 45.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.26
-----
Unit Hyd Qpeak (cms)= 4.461

PEAK FLOW (cms)= 0.846 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 6.733
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.124

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| ADD HYD (8395) |
| 1 + 2 = 3 |
| (ha) (cms) (hrs) (mm)
ID1= 1 (0298): 330.51 0.846 8.00 6.73
+ ID2= 2 (0300): 258.93 1.017 7.50 8.83
-----
ID = 3 (8395): 589.44 1.852 7.75 7.65
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| CALIB |
| NASHYD (0288) | Area (ha)= 340.83 Curve Number (CN)= 78.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 2.21
-----
Unit Hyd Qpeak (cms)= 2.629

```

PEAK FLOW (cms)= 1.727 (i)
 TIME TO PEAK (hrs)= 9.500
 RUNOFF VOLUME (mm)= 20.098
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.369

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHYD (0290) | Area (ha)= 269.18 Curve Number (CN)= 78.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 4.279

PEAK FLOW (cms)= 2.388 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 19.976
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.367

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8397) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0288): 340.83 1.727 9.50 20.10
 + ID2= 2 (0290): 269.18 2.388 7.50 19.98
 ID = 3 (8397): 610.01 3.977 8.00 20.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB |
 NASHYD (0296) | Area (ha)= 293.65 Curve Number (CN)= 76.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.13

Unit Hyd Qpeak (cms)= 4.437

PEAK FLOW (cms)= 2.330 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 18.668
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.343

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHYD (0292) | Area (ha)= 738.49 Curve Number (CN)= 68.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.52

Unit Hyd Qpeak (cms)= 8.289

PEAK FLOW (cms)= 3.557 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 14.367
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.264

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHYD (0294) | Area (ha)= 274.15 Curve Number (CN)= 57.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.87

Unit Hyd Qpeak (cms)= 5.367

PEAK FLOW (cms)= 1.387 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 9.999
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.184

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8398) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)

(ha) (cms) (hrs) (mm)
 ID1= 1 (0292): 738.49 3.557 8.25 14.37
 + ID2= 2 (0294): 274.15 1.387 7.25 10.00
 ID = 3 (8398): 1012.64 4.858 8.00 13.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6296) |
 IN= 2--> OUT= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (2961.0) ----->

Distance	Elevation	Manning
0.00	243.98	0.0400
42.89	243.18	0.0400
85.17	241.81	0.0400
120.66	240.50	0.0400
156.15	239.56	0.0400
198.74	236.15	0.0400
237.78	234.01	0.0400
241.33	233.82	0.0400
248.77	233.12	0.0400 / 0.0400
249.87	232.32	0.0400
250.37	231.80	0.0400
250.87	232.23	0.0400
251.97	233.10	0.0400 / 0.0400
255.37	233.22	0.0400
259.07	233.87	0.0400
262.62	234.12	0.0400
266.17	234.23	0.0400
283.91	234.73	0.0400
337.15	241.75	0.0400
351.34	244.00	0.0400

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.43	232.23	.510E+03	0.1	0.53	81.07
0.87	232.67	.213E+04	0.7	0.83	51.49
1.30	233.10	.501E+04	2.2	1.10	38.75
1.98	233.78	.242E+05	12.6	1.34	31.93
2.66	234.46	.738E+05	42.1	1.46	29.21
3.34	235.14	.177E+06	127.9	1.84	23.13
4.02	235.82	.314E+06	281.4	2.29	18.61
4.70	236.50	.481E+06	509.4	2.71	15.73
5.38	237.18	.672E+06	815.6	3.11	13.73
6.06	237.86	.886E+06	1200.7	3.46	12.30
6.74	238.54	.112E+07	1669.4	3.80	11.23
7.42	239.22	.139E+07	2226.9	4.11	10.38
8.10	239.90	.168E+07	2808.4	4.28	9.95
8.78	240.58	.202E+07	3470.4	4.40	9.68
9.46	241.26	.240E+07	4331.7	4.61	9.24
10.14	241.94	.283E+07	5335.2	4.83	8.84
10.82	242.62	.330E+07	6482.6	5.03	8.48
11.50	243.30	.381E+07	7764.1	5.21	8.18
12.18	243.98	.439E+07	9101.2	5.31	8.04

<---- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8398) 1012.64 4.86 8.00 13.18 1.48 1.15
 OUTFLOW: ID= 1 (6296) 1012.64 4.69 8.75 13.18 1.46 1.15

ADD HYD (8396) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0296): 293.65 2.330 7.50 18.67
 + ID2= 2 (6296): 1012.64 4.688 8.75 13.18
 ID = 3 (8396): 1306.29 6.905 8.25 14.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8394) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8396): 1306.29 6.905 8.25 14.42
 + ID2= 2 (8397): 610.01 3.977 8.00 20.04
 ID = 3 (8394): 1916.30 10.857 8.25 16.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8392)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8394):	1916.30	10.857	8.25	16.21
+ ID2= 2 (8395):	589.44	1.852	7.75	7.65
ID = 3 (8392):	2505.74	12.674	8.25	14.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CWN (8302)
IN# 2--> OUT# 1

Routing time step (min) = 15.00

Distance	Elevation	Manning	
0.00	228.10	0.0400	
18.47	227.12	0.0400	
36.95	226.12	0.0400	
46.18	225.84	0.0400	
55.42	225.58	0.0400	
272.47	222.88	0.0400	
277.09	222.76	0.0400	
281.71	222.58	0.0400	
288.54	222.18	0.0400 / 0.0300	Main Channel
288.64	221.00	0.0300	Main Channel
290.94	221.00	0.0300	Main Channel
291.04	221.00	0.0300	Main Channel
291.54	222.75	0.0300 / 0.0400	Main Channel
300.18	222.83	0.0400	
304.80	223.04	0.0400	
309.42	223.25	0.0400	
318.65	223.69	0.0400	
360.22	225.57	0.0400	
397.16	227.60	0.0400	
457.20	228.35	0.0400	

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.29	221.29	.228E+04	0.2	0.32	165.63
0.59	221.59	.455E+04	0.7	0.45	116.15
0.88	221.88	.713E+04	1.2	0.54	96.87
1.18	222.18	.971E+04	1.9	0.61	86.20
1.57	222.57	.175E+05	3.3	0.59	88.86
1.97	222.97	.427E+05	6.4	0.47	111.39
2.36	223.36	.115E+06	16.1	0.44	119.14
2.76	223.76	.237E+06	37.1	0.49	106.49
3.15	224.15	.409E+06	73.9	0.56	93.40
3.55	224.55	.631E+06	127.0	0.63	82.76
3.94	224.94	.903E+06	202.4	0.70	74.37
4.34	225.34	.123E+07	301.8	0.77	67.68
4.73	225.73	.160E+07	433.5	0.85	61.37
5.13	226.13	.200E+07	602.2	0.95	55.29
5.52	226.52	.242E+07	804.7	1.05	50.13
5.92	226.92	.286E+07	1034.2	1.14	46.11
6.31	227.31	.332E+07	1290.5	1.22	42.88
6.71	227.71	.380E+07	1560.9	1.29	40.55
7.10	228.10	.432E+07	1835.6	1.34	39.19

INFLOW : ID= 2 (8392)	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
OUTFLOW: ID= 1 (8302)	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
ID= 2 (8392)	2505.74	12.67	8.25	14.20	2.23	0.45
ID= 1 (8302)	2505.74	10.20	10.50	14.20	2.12	0.46

ADD HYD (8390)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0302):	473.90	1.530	8.50	10.41
+ ID2= 2 (8302):	2505.74	10.198	10.50	14.20
ID = 3 (8390):	2979.64	11.602	10.25	13.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8348)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8386):	788.82	5.992	8.50	21.65
+ ID2= 2 (8390):	2979.64	11.602	10.25	13.59
ID = 3 (8348):	3768.46	17.080	9.75	15.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)=
NASHYD (0304)	Ia	(mm)	# of Linear Res.(N)=	1.50
ID= 1 DT=15.0 min	292.37	5.00	2.78	
U.H. Tp(hrs)=	2.78			

Unit Hyd Qpeak (cms) = 1.793

PEAK FLOW (cms) = 0.753 (i)
TIME TO PEAK (hrs) = 10.500
RUNOFF VOLUME (mm) = 12.262
TOTAL RAINFALL (mm) = 54.400
RUNOFF COEFFICIENT = 0.225

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)=
NASHYD (0280)	Ia	(mm)	# of Linear Res.(N)=	1.50
ID= 1 DT=15.0 min	299.86	5.00	0.85	
U.H. Tp(hrs)=	0.85			

Unit Hyd Qpeak (cms) = 6.009

PEAK FLOW (cms) = 3.720 (i)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 22.907
TOTAL RAINFALL (mm) = 54.400
RUNOFF COEFFICIENT = 0.421

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)=
NASHYD (0278)	Ia	(mm)	# of Linear Res.(N)=	1.50
ID= 1 DT=15.0 min	485.49	5.00	1.52	
U.H. Tp(hrs)=	1.52			

Unit Hyd Qpeak (cms) = 5.453

PEAK FLOW (cms) = 3.831 (i)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 23.080
TOTAL RAINFALL (mm) = 54.400
RUNOFF COEFFICIENT = 0.424

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)=
NASHYD (0276)	Ia	(mm)	# of Linear Res.(N)=	1.50
ID= 1 DT=15.0 min	90.89	5.00	0.67	
U.H. Tp(hrs)=	0.67			

Unit Hyd Qpeak (cms) = 2.302

PEAK FLOW (cms) = 1.207 (i)
TIME TO PEAK (hrs) = 6.750
RUNOFF VOLUME (mm) = 20.490
TOTAL RAINFALL (mm) = 54.400
RUNOFF COEFFICIENT = 0.377

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)=
NASHYD (0274)	Ia	(mm)	# of Linear Res.(N)=	75.0
ID= 1 DT=15.0 min	392.49	5.00	1.08	
U.H. Tp(hrs)=	1.08			

Unit Hyd Qpeak (cms) = 6.182

PEAK FLOW (cms) = 3.104 (i)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 18.038
TOTAL RAINFALL (mm) = 54.400
RUNOFF COEFFICIENT = 0.332

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8360)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0274):	392.49	3.104	7.50	18.04
+ ID2= 2 (0274):	90.89	1.207	6.75	20.49
ID = 3 (8360):	483.38	4.235	7.25	18.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8358)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0278):	485.49	3.831	8.00	23.08
+ ID2= 2 (8360):	483.38	4.235	7.25	18.50
=====				
ID = 3 (8358):	968.87	7.908	7.75	20.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6280) |
 IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2801.0) ----->

Distance	Elevation	Manning
0.00	241.14	0.0500
13.32	240.80	0.0500
39.95	240.07	0.0500
96.54	236.21	0.0500
113.19	234.15	0.0500
123.18	232.35	0.0500
143.15	225.80	0.0500
149.81	225.62	0.0500
153.14	225.40	0.0500
157.30	224.76	0.0500 / 0.0300
159.80	224.26	0.0300
162.30	224.85	0.0300
162.55	224.97	0.0300 / 0.0500
167.80	225.05	0.0500
186.43	229.14	0.0500
213.06	234.75	0.0500
236.37	237.09	0.0500
266.33	237.31	0.0500
292.96	237.83	0.0500
329.58	241.50	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.50	224.76	.263E+04	0.8	0.65	58.31
1.36	225.62	.278E+05	13.4	1.10	34.54
2.22	226.48	.855E+05	52.6	1.40	27.09
3.09	227.35	.157E+06	120.4	1.74	21.78
3.95	228.21	.242E+06	216.3	2.04	18.64
4.81	229.07	.340E+06	341.8	2.29	16.56
5.67	229.93	.450E+06	498.3	2.52	15.06
6.53	230.79	.574E+06	688.3	2.73	13.90
7.40	231.66	.711E+06	913.8	2.93	12.97
8.26	232.52	.861E+06	1173.4	3.11	12.23
9.12	233.38	.103E+07	1461.3	3.24	11.72
9.98	234.24	.121E+07	1793.3	3.37	11.26
10.85	235.11	.142E+07	2141.0	3.45	11.02
11.71	235.97	.165E+07	2530.4	3.50	10.86
12.57	236.83	.192E+07	2954.0	3.51	10.81
13.43	237.69	.226E+07	3160.3	3.18	11.93
14.29	238.55	.271E+07	3869.4	3.26	11.67
15.16	239.42	.320E+07	4740.3	3.38	11.24
16.02	240.28	.373E+07	5688.4	3.48	10.92

<----- hydrograph -----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW: ID= 2 (8358)	968.87	7.91	7.75	20.79	0.99
OUTFLOW: ID= 1 (6280)	968.87	7.48	8.50	20.79	0.95

ADD HYD (8354)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0260):	299.86	3.720	7.00	22.91
+ ID2= 2 (6280):	968.87	7.477	8.50	20.79
=====				
ID = 3 (8354):	1268.73	10.661	8.00	21.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	NASHYD	Area	(ha)	QPEAK	(cms)	TPEAK	(hrs)	R.V.	(mm)
ID= 1 DT=15.0 min		157.38		5.00		7.50		14.81	
				5.00		1.50		9.50	

Unit Hyd Qpeak (cms)= 2.456

PEAK FLOW (cms)=	1.236 (i)
TIME TO PEAK (hrs)=	7.500
RUNOFF VOLUME (mm)=	18.040
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.332

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD	Area	(ha)	QPEAK	(cms)	TPEAK	(hrs)	R.V.	(mm)
ID= 1 DT=15.0 min		243.61		5.00		8.00		23.08	
				5.00		0.87		1.30	

Unit Hyd Qpeak (cms)= 3.429

PEAK FLOW (cms)=	2.117 (i)
TIME TO PEAK (hrs)=	7.500
RUNOFF VOLUME (mm)=	22.091
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.406

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD	Area	(ha)	QPEAK	(cms)	TPEAK	(hrs)	R.V.	(mm)
ID= 1 DT=15.0 min		215.76		5.00		7.50		14.81	
				5.00		0.69		1.30	

Unit Hyd Qpeak (cms)= 3.807

PEAK FLOW (cms)=	1.800 (i)
TIME TO PEAK (hrs)=	7.000
RUNOFF VOLUME (mm)=	17.875
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.329

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD	Area	(ha)	QPEAK	(cms)	TPEAK	(hrs)	R.V.	(mm)
ID= 1 DT=15.0 min		353.96		5.00		8.00		23.08	
				5.00		1.30		1.30	

Unit Hyd Qpeak (cms)= 3.313

PEAK FLOW (cms)=	1.477 (i)
TIME TO PEAK (hrs)=	8.250
RUNOFF VOLUME (mm)=	14.806
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.272

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD	Area	(ha)	QPEAK	(cms)	TPEAK	(hrs)	R.V.	(mm)
ID= 1 DT=15.0 min		508.09		5.00		1.63		1.30	

Unit Hyd Qpeak (cms)= 3.786

PEAK FLOW (cms)=	1.507 (i)
TIME TO PEAK (hrs)=	9.500
RUNOFF VOLUME (mm)=	12.617
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.232

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8380)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0264):	353.96	1.477	8.25	14.81
+ ID2= 2 (0265):	508.09	1.507	9.50	12.62
=====				
ID = 3 (8380):	862.05	2.967	8.75	13.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6268) |
 IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2681.0) ----->

Distance	Elevation	Manning	
0.00	274.03	0.0500	
10.55	272.73	0.0500	
21.10	271.28	0.0500	
30.12	266.83	0.0500	
37.04	260.36	0.0500	
42.32	260.06	0.0500	
49.95	259.93	0.0500	
101.00	256.87	0.0500	
103.00	256.58	0.0500 / 0.0350	Main Channel
104.00	256.32	0.0350	Main Channel
106.00	256.05	0.0350	Main Channel
108.00	256.25	0.0350	Main Channel
110.78	256.65	0.0350 / 0.0500	Main Channel
113.42	256.81	0.0500	
116.06	257.00	0.0500	
155.42	260.24	0.0500	
187.27	263.35	0.0500	
211.01	267.22	0.0500	
224.20	269.60	0.0500	
261.13	272.68	0.0500	

(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.45	234.34	.227E+04	0.7	0.89	52.83
0.98	234.87	.966E+04	4.6	1.49	35.20
1.50	235.39	.250E+05	13.7	1.73	30.41
2.03	235.92	.475E+05	25.5	1.95	26.87
2.56	236.45	.772E+05	52.5	2.14	24.50
3.09	236.98	.142E+06	82.4	1.83	28.73
3.61	237.50	.246E+06	158.1	2.03	25.91
4.14	238.03	.357E+06	267.0	2.31	22.73
4.67	238.56	.477E+06	392.1	2.59	20.27
5.20	239.09	.604E+06	547.8	2.86	18.38
5.72	239.61	.739E+06	729.2	3.11	16.90
6.25	240.14	.882E+06	936.5	3.34	15.70
6.78	240.67	.103E+07	1169.7	3.57	14.72
7.31	241.20	.119E+07	1425.9	3.77	13.94
7.83	241.72	.136E+07	1710.4	3.96	13.26
8.36	242.25	.154E+07	1967.9	4.02	13.07
8.89	242.78	.174E+07	2264.9	4.09	12.83
9.42	243.31	.196E+07	2589.3	4.15	12.65
9.94	243.83	.221E+07	2953.5	4.22	12.46

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.53	256.58	.697E+04	2.1	0.97	55.64
1.38	257.43	.477E+05	25.9	1.76	30.66
2.22	258.27	.123E+06	84.3	2.22	24.26
3.07	259.12	.231E+06	185.4	2.60	20.72
3.92	259.97	.371E+06	335.8	2.92	18.42
4.77	260.82	.560E+06	547.6	3.15	17.06
5.61	261.66	.790E+06	845.6	3.46	15.37
6.46	262.51	.106E+07	1225.0	3.75	14.37
7.31	263.36	.136E+07	1693.3	4.02	13.37
8.16	264.21	.169E+07	2283.2	4.35	12.36
9.00	265.05	.206E+07	2969.1	4.66	11.54
9.85	265.90	.245E+07	3754.4	4.96	10.86
10.70	266.75	.286E+07	4643.0	5.24	10.28
11.55	267.60	.331E+07	5636.1	5.50	9.78
12.39	268.44	.378E+07	6746.2	5.76	9.34
13.24	269.29	.428E+07	7972.2	6.01	8.96
14.09	270.14	.482E+07	9218.9	6.18	8.71
14.94	270.99	.539E+07	10550.5	6.32	8.52
15.78	271.83	.601E+07	12024.7	6.46	8.33

INFLOW	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
ID= 2 (8380)	862.05	2.97	8.75	13.52	0.56	0.98	
OUTFLOW	ID= 1 (6268)	862.05	2.89	10.25	13.52	0.56	0.98

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8382)	1077.81	4.22	8.75	14.39	0.93
OUTFLOW : ID= 1 (6270)	1077.81	4.19	9.25	14.39	0.92

ADD HYD	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0267)	243.61	2.117	7.50	22.09
+ ID2= 2 (6270)	1077.81	4.186	9.25	14.39
ID = 3 (8384)	1321.42	6.059	8.75	15.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	Curve Number
NASHYD (0262)	(ha) = 341.31	(CN) = 82.0
ID= 1 DT=15.0 min	Ia (mm) = 5.00	# of Linear Res.(N) = 1.30
	U.H. Tp(hrs) = 1.01	

Unit Hyd Qpeak (cms) = 4.094

PEAK FLOW (cms) = 2.732 (i)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 22.953
TOTAL RAINFALL (mm) = 54.400
RUNOFF COEFFICIENT = 0.422

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	Curve Number
NASHYD (0260)	(ha) = 476.24	(CN) = 82.0
ID= 1 DT=15.0 min	Ia (mm) = 5.00	# of Linear Res.(N) = 1.30
	U.H. Tp(hrs) = 1.33	

Unit Hyd Qpeak (cms) = 4.360

PEAK FLOW (cms) = 3.089 (i)
TIME TO PEAK (hrs) = 8.250
RUNOFF VOLUME (mm) = 23.028
TOTAL RAINFALL (mm) = 54.400
RUNOFF COEFFICIENT = 0.423

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	Curve Number
NASHYD (0258)	(ha) = 181.99	(CN) = 79.0
ID= 1 DT=15.0 min	Ia (mm) = 5.00	# of Linear Res.(N) = 1.30
	U.H. Tp(hrs) = 1.18	

Unit Hyd Qpeak (cms) = 1.881

PEAK FLOW (cms) = 1.165 (i)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 20.683
TOTAL RAINFALL (mm) = 54.400
RUNOFF COEFFICIENT = 0.380

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Distance	Elevation	Manning	
0.00	245.98	0.0500	
8.14	245.66	0.0500	
16.28	245.16	0.0500	
20.35	244.84	0.0500	
38.66	242.98	0.0500	
48.83	240.65	0.0500	
63.07	235.91	0.0500	
65.11	235.18	0.0500	
68.43	234.34	0.0500 / 0.0300	Main Channel
68.68	233.95	0.0300	Main Channel
69.18	233.89	0.0300	Main Channel
69.68	233.95	0.0300	Main Channel
71.21	234.48	0.0300 / 0.0500	Main Channel
81.38	236.44	0.0500	
95.63	236.66	0.0500	
120.04	237.00	0.0500	
148.53	241.77	0.0500	
158.70	242.34	0.0500	
187.18	244.03	0.0500	
201.42	246.36	0.0500	

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
-------	------	--------	-----------	----------	-----------

CALIB

NASHYD (0252) Area (ha)= 319.99 Curve Number (CN)= 73.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.04

Unit Hyd Qpeak (cms)= 3.761

PEAK FLOW (cms)= 1.827 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 16.843
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.310

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0256) Area (ha)= 145.79 Curve Number (CN)= 67.0
 NASHYD (0256) Area (ha)= 145.79 Curve Number (CN)= 67.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.04

Unit Hyd Qpeak (cms)= 1.707

PEAK FLOW (cms)= 0.678 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 13.836
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.254

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0254) Area (ha)= 403.00 Curve Number (CN)= 55.0
 NASHYD (0254) Area (ha)= 403.00 Curve Number (CN)= 55.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.22

Unit Hyd Qpeak (cms)= 4.028

PEAK FLOW (cms)= 1.113 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 9.405
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.173

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6256) Routing time step (min)'= 15.00
 IN= 2--> OUT= 1

<----- DATA FOR SECTION (2561.0) ----->

Distance	Elevation	Manning	
0.00	276.07	0.0400	
11.68	273.71	0.0400	
23.36	271.35	0.0400	
29.19	270.30	0.0400	
35.03	269.44	0.0400	
55.47	267.90	0.0400	
78.82	266.24	0.0400	
90.50	265.63	0.0400	
102.18	265.40	0.0400	
105.10	264.95	0.0400 / 0.0350	Main Channel
108.02	264.39	0.0350	Main Channel
110.94	264.72	0.0350	Main Channel
113.86	265.19	0.0350 / 0.0400	Main Channel
116.78	265.49	0.0400	
143.05	268.24	0.0400	
172.25	270.53	0.0400	
207.28	271.95	0.0400	
233.55	273.72	0.0400	
256.91	274.98	0.0400	
289.02	275.97	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.56	264.95	.723E+04	1.9	0.90	62.51
1.14	265.53	.307E+05	13.5	1.49	37.92
1.72	266.11	.951E+05	48.6	1.73	32.62
2.30	266.69	.191E+06	117.8	2.09	27.03
2.88	267.27	.314E+06	225.2	2.43	23.24
3.46	267.85	.464E+06	375.5	2.74	20.59
4.04	268.43	.641E+06	573.2	3.03	18.62
4.62	269.01	.846E+06	821.9	3.29	17.15
5.20	269.59	.108E+07	1135.0	3.56	15.87
5.78	270.17	.134E+07	1525.1	3.86	14.64
6.36	270.75	.162E+07	1971.1	4.12	13.71
6.94	271.33	.194E+07	2474.0	4.33	13.04
7.52	271.91	.229E+07	3065.0	4.55	12.42
8.10	272.49	.266E+07	3773.4	4.80	11.75
8.68	273.07	.306E+07	4566.3	5.05	11.17
9.26	273.65	.348E+07	5443.7	5.30	10.66

9.84 274.23 .392E+07 6389.6 5.51 10.25
 10.42 274.81 .440E+07 7426.2 5.72 9.88
 11.00 275.39 .490E+07 8490.1 5.86 9.63

<----- hydrograph -----> <-pipe / channel->

AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0254) 403.00 1.11 8.25 9.41 0.32 0.90
 OUTFLOW: ID= 1 (6256) 403.00 1.05 10.00 9.41 0.31 0.90

ADD HYD (8370)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0256): 145.79 0.678 8.00 13.84
 + ID2= 2 (6256): 403.00 1.052 10.00 9.41
 ID = 3 (8370): 548.79 1.667 9.25 10.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0250) Area (ha)= 192.88 Curve Number (CN)= 70.0
 NASHYD (0250) Area (ha)= 192.88 Curve Number (CN)= 70.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.22

Unit Hyd Qpeak (cms)= 1.930

PEAK FLOW (cms)= 0.878 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 15.287
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.281

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0246) Area (ha)= 759.61 Curve Number (CN)= 55.0
 NASHYD (0246) Area (ha)= 759.61 Curve Number (CN)= 55.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.81

Unit Hyd Qpeak (cms)= 5.121

PEAK FLOW (cms)= 1.566 (i)
 TIME TO PEAK (hrs)= 10.000
 RUNOFF VOLUME (mm)= 9.438
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.173

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0248) Area (ha)= 146.04 Curve Number (CN)= 64.0
 NASHYD (0248) Area (ha)= 146.04 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 0.78

Unit Hyd Qpeak (cms)= 2.271

PEAK FLOW (cms)= 0.754 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 12.498
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.230

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8364)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0246): 759.61 1.566 10.00 9.44
 + ID2= 2 (0248): 146.04 0.754 7.25 12.50
 ID = 3 (8364): 905.65 2.217 8.25 9.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6250) Routing time step (min)'= 15.00
 IN= 2--> OUT= 1

<----- DATA FOR SECTION (2501.0) ----->

Distance Elevation Manning

0.00	269.96	0.0500
8.56	268.25	0.0500
17.13	266.91	0.0500
21.41	266.13	0.0500
40.68	263.15	0.0500
62.09	260.75	0.0500
85.64	258.02	0.0500
88.20	257.69	0.0500
93.20	257.05	0.0500 / 0.0350
93.45	256.88	0.0350
94.20	256.56	0.0350
94.95	256.83	0.0350
95.20	257.08	0.0350 / 0.0500
100.62	257.45	0.0500
115.61	258.57	0.0500
139.16	260.43	0.0500
152.01	261.95	0.0500
171.27	264.63	0.0500
188.40	267.90	0.0500
211.95	274.18	0.0500

----- DATA FOR SECTION (2581.0) -----

Distance	Elevation	Manning
0.00	252.88	0.0500
15.47	251.89	0.0500
46.41	248.45	0.0500
73.48	245.81	0.0500
88.95	243.75	0.0500
112.15	242.00	0.0500
135.35	240.23	0.0500
162.42	239.76	0.0500
170.97	239.52	0.0500 / 0.0350
171.58	239.03	0.0350
174.02	239.03	0.0350
176.46	239.03	0.0350
177.07	239.52	0.0350 / 0.0500
185.63	239.67	0.0500
208.83	239.87	0.0500
239.77	240.14	0.0500
274.57	244.93	0.0500
336.45	249.51	0.0500
363.52	249.77	0.0500
382.85	251.78	0.0500

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.49	257.05	.205E+04	0.4	0.68	89.65
1.17	257.73	.248E+05	5.7	0.85	72.16
1.85	258.41	.841E+05	25.2	1.10	55.60
2.53	259.09	.190E+06	66.5	1.36	45.14
3.21	259.77	.312E+06	135.7	1.60	38.36
3.89	260.45	.481E+06	238.3	1.82	33.61
4.57	261.13	.681E+06	383.5	2.07	29.60
5.25	261.81	.911E+06	568.9	2.29	26.70
5.93	262.49	.117E+07	800.5	2.51	24.36
6.61	263.17	.146E+07	1079.0	2.72	22.49
7.28	263.84	.177E+07	1413.4	2.94	20.85
7.96	264.52	.210E+07	1797.0	3.14	19.50
8.64	265.20	.246E+07	2241.6	3.35	18.28
9.32	265.88	.283E+07	2739.9	3.55	17.25
10.00	266.56	.323E+07	3295.7	3.75	16.34
10.68	267.24	.364E+07	3908.9	3.94	15.54
11.36	267.92	.408E+07	4578.5	4.12	14.84
12.04	268.60	.453E+07	5320.1	4.32	14.18
12.72	269.28	.499E+07	6109.1	4.50	13.61

----- hydrograph ----- <-pipe / channel-->

INFLOW : ID=	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
2 (8364)	905.65	2.22	8.25	9.93	0.72	0.73	
OUTFLOW : ID=	1 (6250)	905.65	2.06	10.50	9.93	0.70	0.73

ADD HYD (8366)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0250):	192.88	0.878	8.00	15.29
+ ID2= 2 (6250):	905.65	2.056	10.50	9.93
ID = 3 (8366):	1098.53	2.855	10.00	10.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8368)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8366):	1098.53	2.855	10.00	10.87
+ ID2= 2 (8370):	548.79	1.667	9.25	10.58
ID = 3 (8368):	1647.32	4.503	10.00	10.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8372)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0252):	319.99	1.827	8.00	16.84
+ ID2= 2 (8368):	1647.32	4.503	10.00	10.78
ID = 3 (8372):	1967.31	6.156	9.25	11.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6258) |
IN= 2--> OUT= 1 | Routing time step (min)= 15.00

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.49	239.52	.135E+05	2.5	0.92	91.32
1.14	240.17	.175E+06	25.6	0.73	114.22
1.78	240.81	.532E+06	123.5	1.17	71.85
2.43	241.46	.932E+06	285.3	1.54	54.48
3.07	242.10	.138E+07	506.5	1.85	45.25
3.72	242.75	.186E+07	786.8	2.12	39.42
4.36	243.39	.239E+07	1127.3	2.37	35.33
5.01	244.04	.296E+07	1535.8	2.61	32.12
5.65	244.68	.356E+07	2014.3	2.84	29.48
6.30	245.33	.420E+07	2535.9	3.03	27.60
6.94	245.97	.488E+07	3108.4	3.20	26.16
7.59	246.62	.561E+07	3740.6	3.35	24.98
8.23	247.26	.638E+07	4446.7	3.50	23.93
8.88	247.91	.721E+07	5229.7	3.64	22.98
9.52	248.55	.809E+07	6094.3	3.78	22.12
10.17	249.20	.901E+07	7050.4	3.93	21.31
10.81	249.84	.100E+08	7773.0	3.90	21.46
11.46	250.49	.111E+08	8998.8	4.07	20.56
12.10	251.13	.122E+08	10318.2	4.24	19.75

----- hydrograph ----- <-pipe / channel-->

INFLOW : ID=	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
2 (8372)	1967.31	6.16	9.25	11.76	0.59	0.88	
OUTFLOW : ID=	1 (6258)	1967.31	5.64	11.25	11.76	0.58	0.89

ADD HYD (8374)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0258):	181.99	1.165	8.00	20.68
+ ID2= 2 (6258):	1967.31	5.637	11.25	11.76
ID = 3 (8374):	2149.30	6.579	11.00	12.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8376)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0260):	476.24	3.089	8.25	23.03
+ ID2= 2 (8374):	2149.30	6.579	11.00	12.52
ID = 3 (8376):	2625.54	9.354	10.25	14.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8378)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0262):	341.31	2.732	8.00	22.95
+ ID2= 2 (8376):	2625.54	9.354	10.25	14.42
ID = 3 (8378):	2966.85	11.655	10.00	15.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8362)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8378): 2966.85 11.655 10.00 15.41
+ ID2= 2 (8384): 1321.42 6.059 8.75 15.81
=====
ID = 3 (8362): 4288.27 17.537 9.50 15.53

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (8272)
IN= 2--> OUT= 1
Routing time step (min)'= 15.00

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```

<----- DATA FOR SECTION (2721.0) ----->
Distance Elevation Manning
0.00 231.01 0.0450
23.01 223.65 0.0450
34.51 222.46 0.0450
51.77 222.13 0.0450
69.02 221.87 0.0450
161.06 221.92 0.0450
166.81 221.91 0.0450
172.56 221.89 0.0450
180.57 221.40 0.0450 /0.0300 Main Channel
181.57 220.60 0.0300 Main Channel
184.07 220.16 0.0300 Main Channel
195.57 221.85 0.0300 /0.0450 Main Channel
201.32 221.82 0.0450
207.07 221.77 0.0450
212.83 221.72 0.0450
253.09 222.52 0.0450
408.40 222.65 0.0450
460.17 223.20 0.0450
517.69 224.84 0.0450
569.46 232.57 0.0450

```

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<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.41 220.57 .189E+04 0.5 0.49 60.18
0.83 220.99 .697E+04 3.2 0.82 35.79
1.24 221.40 .145E+05 8.8 1.07 27.45
1.84 222.00 .643E+05 29.3 0.81 36.61
2.44 222.60 .282E+06 132.9 0.83 35.41
3.04 223.20 .707E+06 413.7 1.04 28.50
3.64 223.80 .118E+07 876.8 1.31 22.46
4.24 224.40 .168E+07 1494.8 1.57 18.75
4.84 225.00 .221E+07 2264.1 1.82 16.23
5.44 225.60 .274E+07 3195.3 2.06 14.29
6.04 226.20 .328E+07 4258.6 2.30 12.84
6.65 226.81 .383E+07 5444.7 2.52 11.72
7.25 227.41 .438E+07 6749.7 2.73 10.82
7.85 228.01 .494E+07 8169.4 2.93 10.08
8.45 228.61 .551E+07 9700.7 3.12 9.46
9.05 229.21 .608E+07 11340.8 3.30 8.94
9.65 229.81 .666E+07 13087.4 3.48 8.48
10.25 230.41 .724E+07 14938.5 3.65 8.08
10.85 231.01 .783E+07 16892.4 3.82 7.73

```

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8362) 4288.27 17.54 9.50 15.53 1.50 0.94
OUTFLOW: ID= 1 (8272) 4288.27 17.32 10.25 15.53 1.49 0.94

```

```

ADD HYD (8356)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0272): 157.38 1.236 7.50 18.04
+ ID2= 2 (8272): 4288.27 17.324 10.25 15.53
=====
ID = 3 (8356): 4445.65 18.249 10.00 15.62

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8352)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8354): 1266.73 10.681 8.00 21.29
+ ID2= 2 (8356): 4445.65 18.249 10.00 15.62
=====
ID = 3 (8352): 5714.38 27.725 9.00 16.88

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6304)
IN= 2--> OUT= 1
Routing time step (min)'= 15.00

```

```

<----- DATA FOR SECTION (3041.0) ----->
Distance Elevation Manning
0.00 232.08 0.0500
19.00 231.87 0.0500
38.00 231.33 0.0500
66.51 230.44 0.0500
104.51 228.25 0.0500
118.76 225.17 0.0500
128.26 219.86 0.0500
175.77 219.17 0.0500
185.27 218.90 0.0500 /0.0300 Main Channel
185.52 218.65 0.0300 Main Channel
190.02 218.37 0.0300 Main Channel
194.52 218.60 0.0300 Main Channel
194.77 218.85 0.0300 /0.0500 Main Channel
204.27 219.60 0.0500
299.28 220.91 0.0500
327.78 222.36 0.0500
375.28 225.71 0.0500
403.79 229.37 0.0500
432.29 230.43 0.0500
470.29 232.00 0.0500

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.48 218.85 .121E+05 1.2 0.36 173.94
1.17 219.54 .839E+05 9.8 0.44 142.60
1.86 220.23 .323E+06 41.0 0.47 131.43
2.56 220.93 .708E+06 109.5 0.58 107.67
3.25 221.62 .118E+07 224.3 0.74 87.36
3.94 222.31 .168E+07 376.5 0.84 74.48
4.63 223.00 .222E+07 567.9 0.96 65.26
5.32 223.69 .279E+07 794.7 1.07 58.58
6.02 224.39 .339E+07 1056.2 1.17 53.52
6.71 225.08 .402E+07 1352.4 1.26 49.52
7.40 225.77 .468E+07 1677.3 1.34 46.46
8.09 226.46 .536E+07 2050.0 1.43 43.58
8.79 227.16 .607E+07 2457.9 1.52 41.14
9.48 227.85 .680E+07 2901.0 1.60 39.05
10.17 228.54 .755E+07 3352.7 1.66 37.53
10.86 229.23 .834E+07 3812.5 1.71 36.48
11.55 229.92 .919E+07 4241.8 1.73 36.13
12.25 230.62 .101E+08 4697.0 1.74 35.93
12.94 231.31 .111E+08 5186.3 1.74 35.83

```

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8352) 5714.38 27.72 9.00 16.88 1.57 0.46
OUTFLOW: ID= 1 (6304) 5714.38 22.85 11.75 16.88 1.46 0.45

```

```

ADD HYD (8350)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0304): 292.37 0.753 10.50 12.26
+ ID2= 2 (6304): 5714.38 22.850 11.75 16.88
=====
ID = 3 (8350): 6006.75 23.587 11.75 16.65

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8346)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8348): 3768.46 17.080 9.75 15.28
+ ID2= 2 (8350): 6006.75 23.587 11.75 16.65
=====
ID = 3 (8346): 9775.21 39.706 10.75 16.12

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8344)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0306): 283.97 0.250 13.75 8.59
+ ID2= 2 (8346): 9775.21 39.706 10.75 16.12
=====
ID = 3 (8344): 10059.18 39.930 10.75 15.91

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5509)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	*****	370.0451
41.0590	56.7403	*****	863.4386
46.1390	86.3439	*****	*****
56.6340	*****	*****	*****
67.9600	*****	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
*****	39.930	10.75	15.91
*****	30.768	14.50	15.91

PEAK FLOW REDUCTION [Qout/Qin](%)= 77.05
 TIME SHIFT OF PEAK FLOW (min)=225.00
 MAXIMUM STORAGE USED (ha.m.)= 42.5230

CALIB (0332)
 NASHYD (0332) Area (ha)= 393.44 Curve Number (CN)= 75.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 2.32

Unit Hyd Qpeak (cms)	Value
Unit Hyd Qpeak	2.894
PEAK FLOW (cms)	1.734 (i)
TIME TO PEAK (hrs)	10.000
RUNOFF VOLUME (mm)	18.150
TOTAL RAINFALL (mm)	54.400
RUNOFF COEFFICIENT	0.334

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0330)
 NASHYD (0330) Area (ha)= 468.30 Curve Number (CN)= 80.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.09

Unit Hyd Qpeak (cms)	Value
Unit Hyd Qpeak	7.335
PEAK FLOW (cms)	4.424 (i)
TIME TO PEAK (hrs)	7.500
RUNOFF VOLUME (mm)	21.421
TOTAL RAINFALL (mm)	54.400
RUNOFF COEFFICIENT	0.394

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6332)
 IN= 2--> OUT= 1 Routing time step (min)= 15.00

<----- DATA FOR SECTION (3321.0) ----->

Distance	Elevation	Manning
0.00	234.00	0.0380
25.85	227.20	0.0380
96.94	226.44	0.0380
168.03	227.38	0.0380
219.73	225.62	0.0380
342.52	221.57	0.0380
368.37	221.42	0.0380
374.83	221.23	0.0380
379.79	220.98	0.0380 /0.0300 Main Channel
380.29	220.47	0.0300 Main Channel
381.29	220.47	0.0300 Main Channel
382.79	220.47	0.0300 Main Channel
383.29	220.98	0.0300 /0.0380 Main Channel
394.22	221.22	0.0380
400.68	221.33	0.0380
407.14	221.44	0.0380
491.16	225.70	0.0380
568.71	227.55	0.0380
607.49	230.14	0.0380
639.80	234.08	0.0380

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.51	220.98	.554E+04	0.9	0.56	107.27
1.20	221.67	.885E+05	12.1	0.49	122.32
1.88	222.35	.309E+06	64.7	0.76	79.70
2.57	223.04	.615E+06	165.4	0.97	61.99
3.25	223.72	.101E+07	323.1	1.16	52.06
3.94	224.41	.148E+07	542.9	1.33	45.51
4.62	225.09	.204E+07	835.4	1.48	40.77
5.31	225.78	.269E+07	1202.8	1.62	37.27

5.99	226.46	.344E+07	1618.0	1.70	35.46
6.68	227.15	.447E+07	1932.0	1.56	38.56
7.36	227.83	.580E+07	2763.9	1.72	34.96
8.05	228.52	.718E+07	3861.9	1.95	30.97
8.73	229.20	.859E+07	5115.4	2.16	27.98
9.42	229.89	1.00E+08	6519.2	2.35	25.64
10.10	230.57	1.15E+08	8092.3	2.55	23.69
10.79	231.26	1.30E+08	9821.5	2.73	22.05
11.47	231.94	1.45E+08	11690.0	2.92	20.68
12.16	232.63	1.60E+08	13694.9	3.09	19.52
12.84	233.31	1.76E+08	15833.9	3.26	18.52

<---- hydrograph ----> <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
*****	468.30	4.42	7.50	21.42	0.73
*****	468.30	3.43	9.75	21.42	0.67

ADD HYD (8322)
 1 + 2 = 3
 ID1= 1 (0332): 393.44 1.734 10.00 18.15
 + ID2= 2 (6332): 468.30 3.427 9.75 21.42
 ID= 3 (8322): 861.74 5.160 9.75 19.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0328)
 NASHYD (0328) Area (ha)= 492.92 Curve Number (CN)= 77.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.91

Unit Hyd Qpeak (cms)	Value
Unit Hyd Qpeak	4.411
PEAK FLOW (cms)	2.703 (i)
TIME TO PEAK (hrs)	8.750
RUNOFF VOLUME (mm)	19.405
TOTAL RAINFALL (mm)	54.400
RUNOFF COEFFICIENT	0.357

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0326)
 NASHYD (0326) Area (ha)= 678.91 Curve Number (CN)= 80.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.95

Unit Hyd Qpeak (cms)	Value
Unit Hyd Qpeak	5.941
PEAK FLOW (cms)	4.073 (i)
TIME TO PEAK (hrs)	9.000
RUNOFF VOLUME (mm)	21.534
TOTAL RAINFALL (mm)	54.400
RUNOFF COEFFICIENT	0.396

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0324)
 NASHYD (0324) Area (ha)= 615.64 Curve Number (CN)= 79.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.90

Unit Hyd Qpeak (cms)	Value
Unit Hyd Qpeak	5.544
PEAK FLOW (cms)	3.645 (i)
TIME TO PEAK (hrs)	8.750
RUNOFF VOLUME (mm)	20.790
TOTAL RAINFALL (mm)	54.400
RUNOFF COEFFICIENT	0.382

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0320)
 NASHYD (0320) Area (ha)= 278.74 Curve Number (CN)= 81.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.36

Unit Hyd Qpeak (cms)	Value
Unit Hyd Qpeak	3.499
PEAK FLOW (cms)	2.314 (i)
TIME TO PEAK (hrs)	8.000
RUNOFF VOLUME (mm)	22.248

TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.409

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (3182) Area (ha)= 457.40 Curve Number (CN)= 78.0
 NASHED (1) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.49

Unit Hyd Qpeak (cms)= 5.259

PEAK FLOW (cms)= 3.172 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 20.047
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.369

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (3181) Area (ha)= 26.00
 STANDHYD (1) Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00
 ID= 1 DT=15.0 min

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 21.06 4.94
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 416.33 40.00
 Mannings n = 0.013 0.250

Max. Ref. Inten.(mm/hr)= 71.81 18.34
 over (min) 15.00 30.00
 Storage Coeff. (min)= 8.45 (ii) 29.53 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.04

TOTALS

PEAK FLOW (cms)= 3.71 0.17 3.819 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 52.40 19.48 46.14
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.36 0.85

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 77.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5318)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.3800	0.7654
0.0510	0.4423	4.1500	0.8231
0.8700	0.5378	4.8750	0.8778
1.9590	0.6105	5.2750	0.8878
2.5620	0.6796	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (3181)	26.000	3.819	6.00	46.14
OUTFLOW: ID= 1 (5318)	26.000	1.977	6.25	46.06

PEAK FLOW REDUCTION [Qout/Qin](%)= 51.76
 TIME SHIFT OF PEAK FLOW (min)= 15.00
 MAXIMUM STORAGE USED (ha.m.)= 0.6441

ADD HYD (8334)
 1 + 2 = 3
 ID1= 1 (3182): 457.40 3.172 8.00 20.05
 + ID2= 2 (5318): 26.00 1.977 6.25 46.06
 ID = 3 (8334): 483.40 3.965 6.25 21.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6320)
 IN= 2--> OUT= 1

Routing time step (min)'= 15.00

----- DATA FOR SECTION (3201.0) -----

Distance	Elevation	Manning
0.00	249.00	0.0380
22.92	245.86	0.0380
45.83	244.87	0.0380
91.66	243.11	0.0380
126.03	239.53	0.0380
160.41	237.17	0.0380
166.14	237.06	0.0380
177.59	237.13	0.0380
183.32	237.20	0.0380
189.05	236.70	0.0380 / 0.0350
193.05	235.89	0.0350
197.05	236.64	0.0350 / 0.0380
200.51	236.74	0.0380
206.24	237.03	0.0380
246.34	238.82	0.0380
263.83	243.87	0.0380
389.56	247.64	0.0380
452.58	247.74	0.0380
498.41	248.60	0.0380
567.16	249.84	0.0380

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.37	236.26	.270E+04	0.4	0.58	107.96
0.75	236.64	.108E+05	2.6	0.92	68.01
1.48	237.37	.877E+05	27.4	1.17	53.35
2.20	238.09	.277E+06	122.1	1.65	37.83
2.93	238.82	.540E+06	299.7	2.08	30.01
3.66	239.55	.857E+06	587.8	2.57	24.29
4.39	240.28	.120E+07	977.8	3.04	20.52
5.11	241.00	.158E+07	1458.1	3.46	18.02
5.84	241.73	.198E+07	2028.6	3.84	16.23
6.57	242.46	.240E+07	2690.5	4.19	14.86
7.29	243.18	.285E+07	3431.8	4.50	13.84
8.02	243.91	.335E+07	4149.1	4.64	13.44
8.75	244.64	.393E+07	4746.6	4.51	13.80
9.47	245.36	.463E+07	5618.6	4.53	13.75
10.20	246.09	.545E+07	6781.8	4.66	13.39
10.93	246.82	.635E+07	8274.5	4.87	12.79
11.66	247.55	.733E+07	9988.0	5.10	12.23
12.38	248.27	.853E+07	11422.0	5.01	12.45
13.11	249.00	.989E+07	13749.7	5.20	11.98

----- hydrograph ----- <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW: ID= 2 (8334) 483.40 3.97 6.25 21.45 0.79 0.93
 OUTFLOW: ID= 1 (6320) 483.40 3.17 9.00 21.45 0.77 0.92

ADD HYD (8332)
 1 + 2 = 3
 ID1= 1 (0320): 278.74 2.314 8.00 22.25
 + ID2= 2 (6320): 483.40 3.174 9.00 21.45
 ID = 3 (8332): 762.14 5.391 8.50 21.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0314) Area (ha)= 165.20 Curve Number (CN)= 78.0
 NASHED (1) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.71

Unit Hyd Qpeak (cms)= 3.978

PEAK FLOW (cms)= 2.028 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 19.819
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.364

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6316)
 IN= 2--> OUT= 1

Routing time step (min)'= 15.00

----- DATA FOR SECTION (3161.0) -----

Distance	Elevation	Manning
0.00	248.54	0.0350
27.93	248.34	0.0350
41.89	246.61	0.0350
62.83	243.09	0.0350
132.65	239.00	0.0350
188.50	236.74	0.0350

195.48	236.54	0.0350	
202.46	236.32	0.0350	
205.48	236.14	0.0350 /0.0310	Main Channel
205.98	235.61	0.0310	Main Channel
207.98	235.25	0.0310	Main Channel
209.98	235.53	0.0310	Main Channel
210.48	236.00	0.0310 /0.0350	Main Channel
216.42	236.73	0.0350	
258.31	239.09	0.0350	
328.12	239.84	0.0350	
439.83	241.47	0.0350	
530.58	242.08	0.0350	
586.43	242.93	0.0350	
691.16	248.00	0.0350	

----->>>> TRAVEL TIME TABLE <<<<<<

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.37	235.62	.215E+04	0.7	0.77	53.88
0.75	236.00	.634E+04	3.5	1.36	30.45
1.46	236.71	.1297E+05	19.3	1.62	25.66
2.16	237.41	.103E+06	75.8	1.84	22.60
2.87	238.12	.229E+06	202.8	2.21	18.79
3.57	238.82	.407E+06	423.3	2.59	16.03
4.28	239.53	.654E+06	727.7	2.77	14.97
4.99	240.24	.103E+07	1242.0	3.01	13.76
5.69	240.94	.151E+07	2002.3	3.31	12.54
6.40	241.65	.210E+07	2987.9	3.55	11.70
7.10	242.35	.286E+07	4302.8	3.75	11.07
7.81	243.06	.374E+07	6165.3	4.10	10.11
8.51	243.76	.468E+07	8642.4	4.60	9.03
9.22	244.47	.563E+07	11496.3	5.06	8.20
9.93	245.18	.666E+07	14714.1	5.50	7.54
10.63	245.88	.770E+07	18290.3	5.91	7.02
11.34	246.59	.877E+07	22221.9	6.30	6.58
12.04	247.29	.988E+07	26464.4	6.67	6.22
12.75	248.00	.110E+08	31061.3	7.01	5.91

----->>>> hydrograph <<<<<< <<-pipe / channel->>

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (0314)	165.20	2.03	6.75	19.82	0.56
OUTFLOW : ID= 1 (6316)	165.20	1.77	7.75	19.82	0.52

NASHYD (0308) | Area (ha)= 529.30 | Curve Number (CN)= 62.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.62

Unit Hyd Qpeak (cms)= 5.575

PEAK FLOW (cms)= 1.987 (l)
 TIME TO PEAK (hrs)= 8.500
 RUNOFF VOLUME (mm)= 11.840
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.218

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----->>>> CALIB | NASHYD (0310) | Area (ha)= 138.28 | Curve Number (CN)= 65.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.76

Unit Hyd Qpeak (cms)= 3.102

PEAK FLOW (cms)= 1.019 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 12.908
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.237

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----->>>> ADD HYD (8342) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID= 1 (0308): 529.30 1.987 8.50 11.84
 + ID= 2 (0310): 138.28 1.019 7.00 12.91
 ID = 3 (8342): 667.58 2.871 8.00 12.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----->>>> ROUTE CHN (6312) | Routing time step (min)'= 15.00
 IN= 2--> OUT= 1 |

----->>>> DATA FOR SECTION (3121.0) <<<<<<

Distance	Elevation	Manning
0.00	265.94	0.0360
38.07	265.43	0.0360
59.82	263.98	0.0360
103.32	254.59	0.0360
157.70	252.16	0.0360
217.52	250.45	0.0360
233.84	247.69	0.0360
234.71	247.27	0.0360
239.71	246.38	0.0360
244.71	246.12	0.0360 /0.0330
259.71	246.67	0.0360
249.71	245.19	0.0330
251.71	245.64	0.0330 /0.0360
282.78	247.12	0.0360
315.41	251.60	0.0360
424.17	256.13	0.0360
478.55	257.04	0.0360
516.62	259.37	0.0360
538.37	266.00	0.0360

----->>>> CALIB | NASHYD (0316) | Area (ha)= 232.34 | Curve Number (CN)= 82.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.01

Unit Hyd Qpeak (cms)= 3.923

PEAK FLOW (cms)= 2.513 (l)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 22.974
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.422

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----->>>> ADD HYD (8338) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID= 1 (0316): 232.34 2.513 7.25 22.97
 + ID= 2 (6316): 165.20 1.774 7.75 19.82
 ID = 3 (8338): 397.54 4.262 7.50 21.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----->>>> CALIB | NASHYD (0312) | Area (ha)= 359.44 | Curve Number (CN)= 80.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.36

Unit Hyd Qpeak (cms)= 4.529

PEAK FLOW (cms)= 2.884 (l)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 21.475
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.395

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----->>>> CALIB |

----->>>> TRAVEL TIME TABLE <<<<<<

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.47	245.64	.111E+04	3.1	1.78	6.07
1.54	246.71	.103E+05	53.9	3.39	3.19
2.61	247.78	.414E+05	281.3	4.40	2.45
3.68	248.85	.839E+05	749.1	5.79	1.87
4.74	249.91	.135E+06	1463.2	6.97	1.55
5.81	250.98	.201E+06	2354.8	7.61	1.42
6.88	252.05	.295E+06	3533.0	7.76	1.39
7.95	253.12	.427E+06	5303.9	8.05	1.34
9.02	254.19	.594E+06	7856.5	8.58	1.26
10.09	255.26	.792E+06	11449.2	9.37	1.15
11.15	256.32	.101E+07	15862.4	10.15	1.06
12.22	257.39	.127E+07	21074.5	10.75	1.01
13.29	258.46	.155E+07	28184.1	11.78	0.92
14.36	259.53	.185E+07	36386.2	12.77	0.85
15.43	260.60	.215E+07	46110.6	13.89	0.78
16.50	261.67	.246E+07	56822.5	14.95	0.72
17.56	262.73	.278E+07	68498.1	15.97	0.68
18.63	263.80	.310E+07	81118.8	16.94	0.64
19.70	264.87	.344E+07	93603.2	17.66	0.61

```

<---- hydrograph ----> <-pipe / channel->
AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
(ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
INFLOW : ID= 2 (8342) 667.58  2.87  8.00  12.06  0.44  1.78
OUTFLOW: ID= 1 (6312) 667.58  2.87  8.00  12.06  0.44  1.78

```

```

| ADD HYD (8340) |
| 1 + 2 = 3 |
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0312): 359.44  2.884  8.00  21.47
+ ID2= 2 (6312): 667.58  2.869  8.00  12.06
=====
ID = 3 (8340): 1027.02  5.753  8.00  15.36

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ADD HYD (8336) |
| 1 + 2 = 3 |
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (8338): 397.54  4.262  7.50  21.66
+ ID2= 2 (8340): 1027.02  5.753  8.00  15.36
=====
ID = 3 (8336): 1424.56  9.969  7.75  17.12

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ADD HYD (8330) |
| 1 + 2 = 3 |
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (8332): 762.14  5.391  8.50  21.74
+ ID2= 2 (8336): 1424.56  9.969  7.75  17.12
=====
ID = 3 (8330): 2186.70  15.231  8.00  18.73

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ROUTE CHN (6324) |
| IN# 2--> OUT# 1 | Routing time step (min)= 15.00

```

```

<----- DATA FOR SECTION (3241.0) ----->
Distance   Elevation   Manning
0.00       234.38      0.0360
33.57     232.48      0.0360
67.14     230.14      0.0360
83.93     228.80      0.0360
134.29    227.62      0.0360
209.82    225.10      0.0360
218.21    224.86      0.0360
226.61    224.47      0.0360
234.00    223.86      0.0360 / 0.0300 Main Channel
234.10    223.66      0.0300 Main Channel
235.00    223.66      0.0300 Main Channel
235.90    223.66      0.0300 Main Channel
236.00    223.86      0.0300 / 0.0360 Main Channel
243.39    224.92      0.0360
251.78    224.89      0.0360
335.71    225.64      0.0360
562.32    226.53      0.0360
637.85    228.36      0.0360
705.00    229.80      0.0360
830.89    234.00      0.0360

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH   ELEV   VOLUME   FLOW RATE   VELOCITY   TRAV.TIME
(m)     (m)    (cu.m.)  (cms)        (m/s)      (min)
0.20    223.86  .145E+04  0.2          0.43       147.27
0.73    224.39  .159E+05  2.8         0.68       93.11
1.27    224.93  .561E+05  11.0        0.75       85.19
1.80    225.46  .217E+06  45.6        0.80       79.20
2.33    225.99  .566E+06  134.4       0.91       70.20
2.87    226.53  .122E+07  338.1       1.06       59.97
3.40    227.06  .206E+07  744.3       1.38       46.16
3.94    227.60  .298E+07  1295.9      1.66       38.38
4.47    228.13  .399E+07  1979.9      1.90       33.59
5.00    228.66  .509E+07  2807.5      2.11       30.21
5.54    229.20  .627E+07  3815.1      2.32       27.41
6.07    229.73  .753E+07  4984.1      2.53       25.17
6.60    230.26  .884E+07  6346.3      2.74       23.20
7.14    230.80  .102E+08  7861.2      2.95       21.61
7.67    231.33  .118E+08  9526.4      3.14       20.29
8.21    231.87  .131E+08  11342.2     3.32       19.18
8.74    232.40  .146E+08  13309.6    3.49       18.22
9.27    232.93  .161E+08  15414.0    3.66       17.41

```

```

9.81  233.47  .177E+08  17671.0  3.81  16.70
<---- hydrograph ----> <-pipe / channel->
AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
(ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
INFLOW : ID= 2 (8330) 2186.70  15.23  8.00  18.73  1.33  0.75
OUTFLOW: ID= 1 (6324) 2186.70  13.09  9.75  18.73  1.30  0.75

```

```

| ADD HYD (8328) |
| 1 + 2 = 3 |
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0324): 615.64  3.645  8.75  20.79
+ ID2= 2 (6324): 2186.70  13.089  9.75  18.73
=====
ID = 3 (8328): 2802.34  16.666  9.75  19.18

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| CALIB NASHYD (0322) |
| ID= 1 DT=15.0 min | Area (ha)= 513.13 Curve Number (CN)= 80.0
| U.H. Tp(hrs)= 1.75 | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50

```

Unit Hyd Qpeak (cms)= 5.020

```

PEAK FLOW (cms)= 3.366 (i)
TIME TO PEAK (hrs)= 8.500
RUNOFF VOLUME (mm)= 21.519
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.396

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

| ADD HYD (8326) |
| 1 + 2 = 3 |
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0322): 513.13  3.366  8.50  21.52
+ ID2= 2 (8328): 2802.34  16.666  9.75  19.18
=====
ID = 3 (8326): 3315.47  19.923  9.50  19.54

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ADD HYD (8324) |
| 1 + 2 = 3 |
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0326): 678.91  4.073  9.00  21.53
+ ID2= 2 (8326): 3315.47  19.923  9.50  19.54
=====
ID = 3 (8324): 3994.38  23.966  9.50  19.88

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ROUTE CHN (6328) |
| IN# 2--> OUT# 1 | Routing time step (min)= 15.00

```

```

<----- DATA FOR SECTION (3281.0) ----->
Distance   Elevation   Manning
0.00       228.00      0.0380
18.58     224.97      0.0380
78.98     223.52      0.0380
125.44    223.28      0.0380
171.90    221.71      0.0380
213.72    219.65      0.0380
218.36    219.40      0.0380
223.01    219.19      0.0380
225.95    219.14      0.0380 / 0.0320 Main Channel
226.45    218.14      0.0320 Main Channel
236.95    218.14      0.0320 Main Channel
245.95    218.14      0.0320 Main Channel
245.95    219.14      0.0320 / 0.0380 Main Channel
246.24    219.16      0.0380
250.88    219.24      0.0380
255.53    219.39      0.0380
325.22    221.47      0.0380
367.03    223.14      0.0380
404.20    225.17      0.0380
459.95    228.04      0.0380

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH   ELEV   VOLUME   FLOW RATE   VELOCITY   TRAV.TIME
(m)     (m)    (cu.m.)  (cms)        (m/s)      (min)

```

(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.50	218.64	.373E+05	3.2	0.33	192.37
1.00	219.14	.752E+05	10.1	0.51	124.71
1.52	219.66	.149E+06	22.2	0.57	113.06
2.04	220.18	.279E+06	43.4	0.59	107.06
2.56	220.70	.464E+06	77.0	0.63	100.52
3.08	221.22	.705E+06	125.7	0.68	93.56
3.61	221.75	.100E+07	192.7	0.73	86.62
4.13	222.27	.135E+07	276.1	0.79	80.92
4.65	222.79	.176E+07	385.9	0.84	75.86
5.17	223.31	.222E+07	514.9	0.89	71.82
5.69	223.83	.280E+07	662.7	0.90	70.40
6.21	224.35	.345E+07	867.7	0.96	66.35
6.73	224.87	.417E+07	1107.0	1.01	62.81
7.25	225.39	.494E+07	1400.0	1.08	58.80
7.78	225.92	.573E+07	1732.2	1.15	55.17
8.30	226.44	.655E+07	2099.9	1.22	52.02
8.82	226.96	.740E+07	2502.8	1.29	49.29
9.34	227.48	.828E+07	2940.9	1.36	46.90
9.86	228.00	.918E+07	3414.3	1.42	44.79

<---- hydrograph ----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8324) 3994.38	23.97	9.50	19.88	1.57	0.57
OUTFLOW: ID= 1 (6328) 3994.38	20.57	11.25	19.88	1.45	0.56

ADD HYD (8320)	1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0328):		492.92	2.703	8.75	19.40
+ ID2= 2 (6328):		3994.38	20.574	11.25	19.88
ID = 3 (8320):		4487.30	22.991	11.00	19.83

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8318)	1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8320):		4487.30	22.991	11.00	19.83
+ ID2= 2 (8322):		861.74	5.160	9.75	19.93
ID = 3 (8318):		5349.04	27.883	11.00	19.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8316)	1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (5509):		10059.18	30.768	14.50	15.51
+ ID2= 2 (8316):		5349.04	27.883	11.00	19.84
ID = 3 (8316):		15408.22	54.401	12.50	17.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8312)	1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8314):		620.10	0.655	7.00	10.58
+ ID2= 2 (8316):		15408.22	54.401	12.50	17.28
ID = 3 (8312):		16028.32	54.930	12.50	17.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8308)	1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8310):		598.90	17.031	6.25	23.52
+ ID2= 2 (8312):		16028.32	54.930	12.50	17.02
ID = 3 (8308):		16627.22	55.984	12.25	17.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5510)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUF= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min	0.0000	0.0000	*****	74.0090
	66.5450	18.5023	*****	*****
	98.5430	37.0045	*****	*****

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8308)	*****	55.984	12.25
OUTFLOW: ID= 1 (5510)	*****	55.092	13.25

PEAK FLOW REDUCTION [Qout/Qin](%) = 98.41
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 15.3262

ADD HYD (8240)	1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0336):		2785.00	1.997	22.25	16.45
+ ID2= 2 (5510):		16627.22	55.092	13.25	17.25
ID = 3 (8240):		19412.22	56.803	13.25	17.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8238)	1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (5508):		9524.23	24.490	12.25	10.18
+ ID2= 2 (8240):		19412.22	56.803	13.25	17.14
ID = 3 (8238):		28936.45	80.945	12.75	14.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8236)	1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0344):		344.00	1.873	7.00	10.67
+ ID2= 2 (8238):		28936.45	80.945	12.75	14.85
ID = 3 (8236):		29941.03	82.805	12.75	14.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8234)	1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8232):		285.80	5.262	6.00	15.59
+ ID2= 2 (8236):		29941.03	82.805	12.75	14.66
ID = 3 (8234):		30226.83	83.076	12.75	14.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8230)	1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0344):		344.00	1.873	7.00	10.67
+ ID2= 2 (8234):		30226.83	83.076	12.75	14.67
ID = 3 (8230):		30570.83	83.630	12.75	14.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8228)	1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8226):		1952.06	6.251	8.75	8.85
+ ID2= 2 (8230):		30570.83	83.630	12.75	14.62
ID = 3 (8228):		32522.89	87.695	12.50	14.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8238)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (5508): 9524.23 42.683 12.25 17.78
+ ID2= 2 (8240): 19412.22 86.298 13.50 28.47
=====
ID = 3 (8238): 28936.45 130.148 12.75 24.95

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8236)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0342): 1004.58 3.660 10.00 16.26
+ ID2= 2 (8238): 28936.45 130.148 12.75 24.95
=====
ID = 3 (8236): 29941.03 133.430 12.75 24.66

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8234)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8232): 285.80 8.040 6.00 24.37
+ ID2= 2 (8236): 29941.03 133.430 12.75 24.66
=====
ID = 3 (8234): 30226.83 133.766 12.75 24.65

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8230)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0344): 344.00 3.342 7.00 18.72
+ ID2= 2 (8234): 30226.83 133.766 12.75 24.65
=====
ID = 3 (8230): 30570.83 134.707 12.75 24.59

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8228)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8226): 1952.06 11.133 8.75 15.62
+ ID2= 2 (8230): 30570.83 134.707 12.75 24.59
=====
ID = 3 (8228): 32522.89 141.797 12.50 24.05

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8190)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0356): 1015.22 4.610 8.00 12.58
+ ID2= 2 (8228): 32522.89 141.797 12.50 24.05
=====
ID = 3 (8190): 33538.11 144.663 12.25 23.70

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

** SIMULATION NUMBER: 13 **

```

READ STORM
Filename: C:\Users\jascott\AppData
Local\Temp\
3e280798-92ee-4282-809c-79f5caed0add\fs74c8d3
Ptotal= 62.70 mm
Comments: 10-Year 12-Hour SCS II Design Storm

```

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.57	3.25	2.51	6.25	11.29	9.25	2.19
0.50	1.57	3.50	2.51	6.50	11.29	9.50	2.19
0.75	1.57	3.75	2.51	6.75	5.02	9.75	2.19
1.00	1.57	4.00	2.51	7.00	5.02	10.00	2.19
1.25	1.57	4.25	3.76	7.25	3.76	10.25	1.25
1.50	1.57	4.50	3.76	7.50	3.76	10.50	1.25

1.75	1.57	4.75	5.02	7.75	3.76	10.75	1.25
2.00	1.57	5.00	5.02	8.00	3.76	11.00	1.25
2.25	1.88	5.25	7.52	8.25	2.19	11.25	1.25
2.50	1.88	5.50	7.52	8.50	2.19	11.50	1.25
2.75	1.98	5.75	30.10	8.75	2.19	11.75	1.25
3.00	1.88	6.00	82.76	9.00	2.19	12.00	1.25

```

CALIB
NASHYD (0356) Area (ha)=1015.22 Curve Number (CN)= 46.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.37

```

Unit Hyd Qpeak (cms)= 12.651

PEAK FLOW (cms)= 3.392 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 9.295
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.148

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0354) Area (ha)= 262.68 Curve Number (CN)= 37.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.38

```

Unit Hyd Qpeak (cms)= 3.252

PEAK FLOW (cms)= 0.631 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 6.749
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.108

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0352) Area (ha)= 381.43 Curve Number (CN)= 54.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.82

```

Unit Hyd Qpeak (cms)= 7.980

PEAK FLOW (cms)= 2.453 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 11.980
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.191

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0346) Area (ha)= 350.93 Curve Number (CN)= 70.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.96

```

Unit Hyd Qpeak (cms)= 6.254

PEAK FLOW (cms)= 3.349 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 19.771
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.315

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0350) Area (ha)= 366.84 Curve Number (CN)= 48.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.07

```

Unit Hyd Qpeak (cms)= 5.831

PEAK FLOW (cms)= 1.564 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 9.910
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.158

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0348) Area (ha)= 590.18 Curve Number (CN)= 48.0
 NASHVD (0348) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.91

Unit Hyd Qpeak (cms)= 5.267
 PEAK FLOW (cms)= 1.632 (1)
 TIME TO PEAK (hrs)= 9.000
 RUNOFF VOLUME (mm)= 9.963
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.159

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6350) |
 IN= 2--> OUT= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (3501.0) ----->

Distance	Elevation	Manning	
0.00	287.00	0.0500	
23.66	283.72	0.0500	
70.98	280.94	0.0500	
112.38	280.23	0.0500	
171.52	276.80	0.0500	
260.24	274.46	0.0500	
266.16	274.26	0.0500	
272.07	274.12	0.0500	
276.49	274.12	0.0500 / 0.0300	Main Channel
276.99	273.82	0.0300	Main Channel
277.99	273.82	0.0300	Main Channel
279.49	273.82	0.0300	Main Channel
280.49	274.27	0.0300 / 0.0500	Main Channel
283.90	274.27	0.0500	
289.81	274.57	0.0500	
325.30	275.75	0.0500	
396.38	278.98	0.0500	
449.51	280.97	0.0500	
496.83	283.90	0.0500	
585.54	287.92	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.30	274.12	273E+04	0.9	0.89	49.81
0.98	274.80	583E+05	20.8	1.05	46.71
1.66	275.48	203E+06	95.2	1.38	35.53
2.33	276.15	438E+06	256.0	1.73	28.50
3.01	276.83	755E+06	524.8	2.05	23.97
3.69	277.51	114E+07	935.8	2.43	20.26
4.37	278.19	157E+07	1472.0	2.76	17.82
5.05	278.87	206E+07	2140.7	3.06	16.06
5.72	279.54	261E+07	2938.6	3.33	14.79
6.40	280.22	321E+07	3887.6	3.57	13.77
7.08	280.90	390E+07	4795.4	3.63	13.56
7.76	281.58	468E+07	6196.2	3.91	12.58
8.43	282.25	550E+07	7803.9	4.19	11.74
9.11	282.93	637E+07	9601.9	4.45	11.05
9.79	283.61	728E+07	11594.8	4.70	10.46
10.47	284.29	823E+07	13845.3	4.96	9.91
11.15	284.97	922E+07	16293.4	5.21	9.43
11.82	285.64	103E+08	18952.0	5.45	9.02
12.50	286.32	113E+08	21824.3	5.69	8.65

<---- hydrograph ----> <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0348)	590.18	1.63	9.00	9.96	0.32	0.99
OUTFLOW : ID= 1 (6350)	590.18	1.59	10.25	9.96	0.32	0.99

ADD HYD (8220) |
 1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (0350): 366.84 1.564 7.50 9.96
 + ID2= 2 (6350): 590.18 1.595 10.25 9.96
 ID = 3 (8220): 957.02 2.909 8.75 9.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8222) |
 1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (0346): 350.93 3.349 7.25 19.77
 + ID2= 2 (8220): 957.02 2.909 8.75 9.94

ID = 3 (8222): 1307.95 6.043 8.00 12.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6352) |
 IN= 2--> OUT= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (3521.0) ----->

Distance	Elevation	Manning	
0.00	257.95	0.0500	
7.83	257.34	0.0500	
15.66	256.19	0.0500	
21.53	254.01	0.0500	
41.11	244.06	0.0500	
76.35	241.38	0.0500	
111.58	239.74	0.0500	
113.54	239.61	0.0500	
113.75	239.41	0.0500 / 0.0300	Main Channel
113.85	238.81	0.0300	Main Channel
115.50	238.81	0.0300	Main Channel
116.15	238.81	0.0300	Main Channel
116.25	239.41	0.0300	Main Channel
117.46	239.52	0.0500	
119.41	239.72	0.0500	
121.37	240.04	0.0500	
131.16	241.84	0.0500	
156.61	247.03	0.0500	
176.19	251.46	0.0500	
193.80	258.79	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.60	239.41	224E+04	2.2	1.52	17.12
1.58	240.39	216E+05	22.3	1.61	16.16
2.55	241.36	804E+05	101.0	1.96	13.27
3.53	242.34	173E+06	275.6	2.48	10.46
4.50	243.31	293E+06	553.9	2.95	8.80
5.48	244.29	438E+06	961.8	3.42	7.60
6.45	245.26	599E+06	1534.4	3.99	6.51
7.43	246.24	770E+06	2228.4	4.51	5.76
8.41	247.22	951E+06	3043.6	4.98	5.21
9.38	248.19	114E+07	3982.1	5.43	4.78
10.36	249.17	134E+07	5040.1	5.85	4.44
11.33	250.14	155E+07	6218.5	6.24	4.16
12.31	251.12	177E+07	7518.6	6.61	3.93
13.29	252.10	200E+07	8969.0	6.98	3.72
14.26	253.07	224E+07	10554.1	7.35	3.53
15.24	254.05	248E+07	12257.1	7.71	3.37
16.21	255.02	272E+07	14045.1	8.03	3.23
17.19	256.00	298E+07	15954.8	8.34	3.11
18.16	256.97	325E+07	17780.4	8.53	3.04

<---- hydrograph ----> <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8222)	1307.95	6.04	8.00	12.58	0.79	1.53
OUTFLOW : ID= 1 (6352)	1307.95	6.00	8.25	12.58	0.78	1.53

ADD HYD (8224) |
 1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (0352): 381.43 2.453 7.00 11.98
 + ID2= 2 (6352): 1307.95 5.998 8.25 12.58
 ID = 3 (8224): 1689.38 8.227 8.00 12.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6354) |
 IN= 2--> OUT= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (3541.0) ----->

Distance	Elevation	Manning	
0.00	253.92	0.0500	
7.95	251.83	0.0500	
15.89	249.97	0.0500	
19.87	249.05	0.0500	
47.68	242.00	0.0500	
67.55	237.10	0.0500	
83.44	231.11	0.0500	
85.43	230.75	0.0500	
85.66	230.61	0.0500 / 0.0300	Main Channel
85.76	230.01	0.0300	Main Channel
87.41	230.01	0.0300	Main Channel
88.06	230.01	0.0300	Main Channel
88.16	230.61	0.0300 / 0.0500	Main Channel

89.40	230.72	0.0500
91.39	230.88	0.0500
93.37	231.03	0.0500
133.11	233.96	0.0500
150.39	235.39	0.0500
172.84	239.03	0.0500
196.68	250.00	0.0500

-----> TRAVEL TIME TABLE <-----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.60	230.61	.451E+04	1.5	1.06	49.40
1.62	231.63	.394E+05	14.6	1.16	45.13
2.64	232.65	.128E+06	58.8	1.44	36.30
3.66	233.67	.269E+06	149.7	1.74	30.00
4.68	234.69	.463E+06	301.5	2.04	25.58
5.70	235.71	.708E+06	530.5	2.36	22.14
6.72	236.73	.980E+06	848.4	2.71	19.25
7.74	237.75	.128E+07	1237.9	3.02	17.29
8.76	238.77	.162E+07	1705.7	3.30	15.83
9.78	239.79	.199E+07	2291.9	3.61	14.45
10.81	240.82	.237E+07	2973.7	3.92	13.30
11.83	241.84	.278E+07	3742.2	4.21	12.38
12.85	242.86	.321E+07	4599.1	4.49	11.62
13.87	243.88	.365E+07	5544.6	4.75	10.98
14.89	244.90	.412E+07	6579.7	5.00	10.44
15.91	245.92	.461E+07	7705.9	5.23	9.97
16.93	246.94	.512E+07	8924.6	5.46	9.55
17.95	247.96	.564E+07	10237.5	5.68	9.19
18.97	248.98	.619E+07	11646.2	5.89	8.86

<---- hydrograph ----> <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8224) 1689.38	8.23	8.00	12.44	1.12	1.11
OUTFLOW: ID= 1 (6354) 1689.38	7.67	8.75	12.44	1.08	1.10

ADD HYD (8226)

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0354):	262.68	0.631	8.00	6.75
+ ID2= 2 (6354):	1689.38	7.666	8.75	12.44
ID= 3 (8226):	1952.06	8.283	8.75	11.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (3402)

Area (ha)	(ha)= 344.00	Curve Number (CN)= 59.0
Ia (mm)	= 5.00	# of Linear Res.(N)= 1.50
U.H. Tp(hrs)	= 0.87	

Unit Hyd Qpeak (cms)= 6.790

PEAK FLOW (cms)	= 2.485 (i)
TIME TO PEAK (hrs)	= 7.000
RUNOFF VOLUME (mm)	= 14.035
TOTAL RAINFALL (mm)	= 62.700
RUNOFF COEFFICIENT	= 0.224

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (3402)

Area (ha)	(ha)= 234.60	Dir. Conn.(%)= 13.00
Total Imp(%)	= 28.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	65.69	168.91
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1250.60	40.00
Mannings n	0.013	0.250

Max.Eff.Inten.(mm/hr)	= 82.76	12.93
over (min)	15.00	45.00
Storage Coeff. (min)	15.44 (ii)	39.69 (ii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.07	0.03
PEAK FLOW (cms)	5.04	3.27
TIME TO PEAK (hrs)	6.00	6.50
RUNOFF VOLUME (mm)	60.70	11.98
TOTAL RAINFALL (mm)	62.70	62.70
RUNOFF COEFFICIENT	0.97	0.19

TOTALS

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3401)	Area (ha)= 51.20	Dir. Conn.(%)= 21.00
ID= 1 DT=15.0 min	Total Imp(%)= 46.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	23.55	27.65
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	584.24	40.00
Mannings n	0.013	0.250

Max.Eff.Inten.(mm/hr)	= 82.76	18.77
over (min)	15.00	45.00
Storage Coeff. (min)	9.78 (ii)	30.67 (ii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.09	0.03

PEAK FLOW (cms)	= 2.10	0.89
TIME TO PEAK (hrs)	= 6.00	6.50
RUNOFF VOLUME (mm)	= 60.70	14.21
TOTAL RAINFALL (mm)	= 62.70	62.70
RUNOFF COEFFICIENT	= 0.97	0.23

TOTALS

(iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5340)

IN=	OUT=	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
DT= 15.0 min	1	0.0000	0.0000	5.3360	1.7877
		0.1730	0.6348	7.2780	2.2185
		1.8360	0.8250	9.1880	2.6486
		2.1540	1.1051	9.5880	2.6586
		3.3950	1.3506	0.0000	0.0000

INFLOW : ID=	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
2 (3401)	51.200	2.465	6.00	23.97
OUTFLOW: ID= 1 (5340)	51.200	0.624	7.00	23.95

PEAK FLOW REDUCTION [Qout/Qin](%)	= 25.34
TIME SHIFT OF PEAK FLOW (min)	= 60.00
MAXIMUM STORAGE USED (ha.m.)	= 0.6899

ADD HYD (8232)

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (3402):	234.60	6.356	6.00	18.31
+ ID2= 2 (5340):	51.20	0.624	7.00	23.95
ID= 3 (8232):	285.80	6.442	6.00	19.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0342)

Area (ha)	(ha)=1004.58	Curve Number (CN)= 54.0
Ia (mm)	= 5.00	# of Linear Res.(N)= 1.50
U.H. Tp(hrs)	= 2.57	

Unit Hyd Qpeak (cms)= 6.686

PEAK FLOW (cms)	= 2.724 (i)
TIME TO PEAK (hrs)	= 10.250
RUNOFF VOLUME (mm)	= 12.117
TOTAL RAINFALL (mm)	= 62.700
RUNOFF COEFFICIENT	= 0.193

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0422)

Area (ha)	(ha)= 780.20	Curve Number (CN)= 54.0
-----------	--------------	-------------------------

|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.95

Unit Hyd Qpeak (cms)= 6.838

PEAK FLOW (cms)= 2.591 (i)
TIME TO PEAK (hrs)= 9.250
RUNOFF VOLUME (mm)= 12.102
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.193

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHVD (0410) | Area (ha)= 572.01 Curve Number (CN)= 48.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.46

Unit Hyd Qpeak (cms)= 6.698

PEAK FLOW (cms)= 1.949 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 9.944
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.159

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHVD (0408) | Area (ha)= 231.62 Curve Number (CN)= 58.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.64

Unit Hyd Qpeak (cms)= 6.198

PEAK FLOW (cms)= 2.048 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 13.505
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.215

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHVD (0376) | Area (ha)= 463.85 Curve Number (CN)= 74.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 7.380

PEAK FLOW (cms)= 4.625 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 22.449
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.358

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHVD (0374) | Area (ha)= 545.70 Curve Number (CN)= 61.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.51

Unit Hyd Qpeak (cms)= 6.158

PEAK FLOW (cms)= 2.759 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 15.043
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.240

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHVD (0372) | Area (ha)= 110.42 Curve Number (CN)= 37.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 1.954

PEAK FLOW (cms)= 0.343 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 6.719
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.107

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHVD (0370) | Area (ha)= 191.85 Curve Number (CN)= 63.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.67

Unit Hyd Qpeak (cms)= 4.860

PEAK FLOW (cms)= 1.913 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 15.799
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.252

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHVD (0369) | Area (ha)= 159.48 Curve Number (CN)= 46.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.12

Unit Hyd Qpeak (cms)= 2.433

PEAK FLOW (cms)= 0.617 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 9.274
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.148

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHVD (0366) | Area (ha)= 462.62 Curve Number (CN)= 62.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.06

Unit Hyd Qpeak (cms)= 7.451

PEAK FLOW (cms)= 3.151 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 15.457
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.247

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHVD (0364) | Area (ha)= 155.27 Curve Number (CN)= 55.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.70

Unit Hyd Qpeak (cms)= 3.782

PEAK FLOW (cms)= 1.155 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 12.322
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.197

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| ADD HYD (8302) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
ID1= 1 (0364): 155.27 1.155 6.75 12.32
+ ID2= 2 (0366): 462.62 3.151 7.50 15.46
=====

ID = 3 (8302): 617.89 4.258 7.25 14.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| CALIB |
| STANSHVD (0362) | Area (ha)= 118.78
|ID= 1 DT=15.0 min | Total Imp(%)= 22.00 Dir. Conn.(%)= 8.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 26.13 92.65
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 889.87 40.00

Mannings n = 0.013 0.250
 Max.Eff.Inten.(mm/hr)= 82.76 11.12
 over (min) 15.00 45.00
 Storage Coeff. (min)= 12.59 (ii) 38.34 (iii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03

TOTALS
 PEAK FLOW (cms)= 1.71 1.58 2.335 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 60.70 10.62 14.63
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.17 0.23

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0358) Area (ha)= 429.87 Curve Number (CN)= 35.0
 NASHYD (0358) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.03

Unit Hyd Qpeak (cms)= 7.091

PEAK FLOW (cms)= 1.174 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 6.228
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.099

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0360) Area (ha)= 138.37 Curve Number (CN)= 46.0
 NASHYD (0360) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.60

Unit Hyd Qpeak (cms)= 3.957

PEAK FLOW (cms)= 0.860 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 9.151
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.146

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8306) AREA QPEAK TPEAK R.V.
 1 + 2 = 3 (ha) (cms) (hrs) (mm)
 ID= 1 (0358): 429.87 1.174 7.50 6.23
 + ID2= 2 (0360): 138.37 0.860 6.75 9.15
 ID = 3 (8306): 568.24 1.984 7.00 6.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6362) Routing time step (min)= 15.00
 IN= 2--> OUT= 1

----- DATA FOR SECTION (3621.0) -----
 Distance Elevation Manning
 0.00 261.46 0.0550
 27.86 254.23 0.0550
 51.07 251.96 0.0550
 74.29 250.77 0.0550
 97.50 249.91 0.0550
 125.36 249.40 0.0550
 150.93 247.40 0.0550
 155.93 247.33 0.0550 /0.0350 Main Channel
 157.93 246.85 0.0350 Main Channel
 159.18 246.65 0.0350 Main Channel
 160.18 246.63 0.0350 Main Channel
 160.93 246.85 0.0350 Main Channel
 161.93 247.18 0.0350 /0.0550 Main Channel
 163.18 248.03 0.0550
 168.18 248.58 0.0550
 183.18 250.18 0.0550
 201.97 252.59 0.0550

213.57 256.02 0.0550
 225.18 260.31 0.0550
 229.82 261.00 0.0550

----- TRAVEL TIME TABLE -----
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
 (m) (m) (cu.m.) (cms) (m/s) (min)
 0.55 247.18 .556E+04 1.9 1.03 49.95
 1.28 247.91 .338E+05 16.7 1.53 23.61
 2.00 248.63 .913E+05 53.6 1.81 28.40
 2.73 249.36 .185E+06 122.3 2.05 25.18
 3.46 250.09 .344E+06 224.6 2.02 25.52
 4.19 250.82 .574E+06 417.9 2.25 22.91
 4.91 251.54 .855E+06 699.1 2.53 20.38
 5.64 252.27 .118E+07 1075.3 2.82 18.26
 6.37 253.00 .153E+07 1559.9 3.15 16.38
 7.10 253.73 .191E+07 2137.6 3.46 14.89
 7.82 254.45 .231E+07 2815.2 3.77 13.67
 8.55 255.18 .272E+07 3608.5 4.10 12.58
 9.28 255.91 .315E+07 4488.2 4.41 11.69
 10.01 256.64 .388E+07 5458.0 4.71 10.95
 10.73 257.36 .403E+07 6512.4 4.99 10.32
 11.46 258.09 .449E+07 7649.8 5.27 9.78
 12.19 258.82 .495E+07 8869.5 5.53 9.32
 12.92 259.55 .544E+07 10171.2 5.78 8.91
 13.64 260.27 .593E+07 11554.8 6.02 8.55

AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8306) 568.24 1.98 7.00 6.94 0.56 1.03
 OUTFLOW: ID= 1 (6362) 568.24 1.75 6.25 6.94 0.52 1.03

ADD HYD (8304) AREA QPEAK TPEAK R.V.
 1 + 2 = 3 (ha) (cms) (hrs) (mm)
 ID1= 1 (0362): 118.78 2.335 6.00 14.63
 + ID2= 2 (6362): 568.24 1.750 8.25 6.94
 ID = 3 (8304): 687.02 2.882 6.50 8.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8300) AREA QPEAK TPEAK R.V.
 1 + 2 = 3 (ha) (cms) (hrs) (mm)
 ID1= 1 (8302): 617.89 4.258 7.25 14.67
 + ID2= 2 (8304): 687.02 2.882 6.50 8.27
 ID = 3 (8300): 1304.91 6.844 7.00 11.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6368) Routing time step (min)= 15.00
 IN= 2--> OUT= 1

----- DATA FOR SECTION (3681.0) -----
 Distance Elevation Manning
 0.00 230.00 0.0370
 18.48 223.26 0.0370
 36.96 223.05 0.0370
 64.67 222.94 0.0370
 110.87 222.86 0.0370
 133.96 222.74 0.0370
 147.82 222.65 0.0370
 170.92 222.31 0.0370
 174.79 222.26 0.0370 /0.0300 Main Channel
 174.89 221.86 0.0300 Main Channel
 175.54 221.86 0.0300 Main Channel
 176.19 221.86 0.0300 Main Channel
 176.29 222.26 0.0300 /0.0370 Main Channel
 180.16 222.25 0.0370
 184.78 222.28 0.0370
 189.40 222.31 0.0370
 332.60 222.37 0.0370
 450.00 230.00 0.0370

----- TRAVEL TIME TABLE -----
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
 (m) (m) (cu.m.) (cms) (m/s) (min)
 0.40 222.26 .771E+03 0.3 0.52 41.74
 0.81 222.67 .784E+05 30.3 0.51 43.17
 1.21 223.07 .210E+06 123.2 0.77 28.42
 1.62 223.48 .384E+06 301.0 1.03 21.25
 2.03 223.89 .563E+06 550.2 1.28 17.06
 2.44 224.30 .747E+06 860.7 1.51 14.46

2.84	224.70	.934E+06	1228.1	1.73	12.68
3.25	225.11	.113E+07	1649.3	1.92	11.37
3.66	225.52	.132E+07	2122.3	2.11	10.37
4.07	225.93	.152E+07	2645.5	2.29	9.58
4.47	226.33	.172E+07	3217.8	2.45	8.92
4.88	226.74	.193E+07	3836.3	2.61	8.38
5.29	227.15	.214E+07	4506.3	2.76	7.92
5.70	227.56	.236E+07	5221.3	2.91	7.52
6.10	227.96	.258E+07	5993.0	3.05	7.17
6.51	228.37	.280E+07	6790.9	3.19	6.87
6.92	228.78	.303E+07	7645.0	3.32	6.60
7.33	229.19	.326E+07	8545.1	3.45	6.35
7.73	229.59	.349E+07	9491.0	3.57	6.13

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8300) 1304.91 6.84 7.00 11.30 0.49 0.52
OUTFLOW : ID= 1 (6368) 1304.91 6.29 8.25 11.30 0.48 0.52

```

```

ADD HYD (8298)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0368): 159.48 0.617 7.75 9.27
+ ID2= 2 (6368): 1304.91 6.294 8.25 11.30
ID = 3 (8298): 1464.39 6.900 8.00 11.08
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

```

ADD HYD (8296)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0370): 191.85 1.913 6.75 15.80
+ ID2= 2 (8298): 1464.39 6.900 8.00 11.08
ID = 3 (8296): 1656.24 8.479 7.75 11.63
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

```

ROUTE CHN (6372)
IN= 2----> OUT= 1 1
Routing time step (min)= 15.00

```

----- DATA FOR SECTION (3721.0) -----

Distance	Elevation	Manning
0.00	225.00	0.0390
30.80	219.38	0.0390
61.61	219.30	0.0390
77.01	219.27	0.0390
469.76	219.14	0.0390
477.46	219.13	0.0390
485.16	219.10	0.0390
492.86	219.09	0.0390
495.56	219.09	0.0390 / 0.0310 Main Channel
495.66	218.51	0.0310 Main Channel
500.56	218.51	0.0310 Main Channel
505.46	218.51	0.0310 Main Channel
505.56	219.09	0.0310 / 0.0390 Main Channel
508.26	219.09	0.0390
515.96	219.10	0.0390
523.67	219.21	0.0390
562.17	219.32	0.0390
654.58	219.43	0.0390
731.59	219.46	0.0390
762.39	225.00	0.0390

```

TRAVEL TIME TABLE
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.29 218.80 .431E+04 1.7 0.58 43.48
0.58 219.09 .867E+04 5.1 0.89 28.32
0.93 219.44 .200E+06 58.2 0.44 57.35
1.28 219.79 .568E+06 278.6 0.74 33.99
1.62 220.13 .939E+06 630.2 1.01 24.84
1.97 220.48 .131E+07 1090.4 1.25 20.05
2.32 220.83 .169E+07 1647.7 1.47 17.06
2.67 221.18 .208E+07 2294.6 1.68 14.99
3.01 221.52 .244E+07 3025.6 1.87 13.46
3.36 221.87 .282E+07 3836.5 2.05 12.27
3.71 222.22 .321E+07 4723.7 2.22 11.31
4.06 222.57 .359E+07 5684.6 2.39 10.53
4.40 222.91 .398E+07 6716.6 2.55 9.87
4.75 223.26 .437E+07 7817.7 2.70 9.31
5.10 223.61 .476E+07 8986.2 2.85 8.83
5.45 223.96 .515E+07 10220.4 2.99 8.40
5.79 224.30 .555E+07 11519.1 3.13 8.03

```

6.14	224.65	.594E+07	12880.8	3.27	7.69
6.49	225.00	.634E+07	14304.5	3.40	7.39

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8296) 1656.24 8.48 7.75 11.63 0.60 0.83
OUTFLOW : ID= 1 (6372) 1656.24 8.21 8.50 11.63 0.60 0.84

```

```

ADD HYD (8294)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0372): 110.42 0.343 7.25 6.72
+ ID2= 2 (6372): 1656.24 8.215 8.50 11.63
ID = 3 (8294): 1766.66 8.529 8.50 11.32
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

```

ADD HYD (8292)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0374): 545.70 2.759 8.25 15.04
+ ID2= 2 (8294): 1766.66 8.529 8.50 11.32
ID = 3 (8292): 2312.36 11.277 8.50 12.20
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

```

RESERVOIR (5505)
IN= 2----> OUT= 1
DT= 15.0 min
OUTFLOW STORAGE OUTFLOW STORAGE
(cms) (ha.m.) (cms) (ha.m.)
0.0000 0.0000 65.1290 345.3754
25.4850 24.6697 84.9510 456.3890
31.1490 98.6787 838.7689
39.6440 838.7789
48.1390 0.0000

```

```

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (8292) 2312.360 11.277 8.50 12.20
OUTFLOW : ID= 1 (5505) 2312.360 8.244 11.25 12.20

```

```

PEAK FLOW REDUCTION [Qout/Qin]= 73.11
TIME SHIFT OF PEAK FLOW (min)=165.00
MAXIMUM STORAGE USED (ha.m.)= 7.9877

```

```

ADD HYD (8272)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0376): 463.85 4.625 7.50 22.45
+ ID2= 2 (5505): 2312.36 8.244 11.25 12.20
ID = 3 (8272): 2776.21 11.221 10.25 13.91
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

```

CALIB (0396) Area (ha)= 305.21 Curve Number (CN)= 69.0
NASHYD (0396) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.08
Unit Hyd Qpeak (cms)= 4.811
PEAK FLOW (cms)= 2.564 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 19.201
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.306
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

```

CALIB (0394) Area (ha)= 325.45 Curve Number (CN)= 53.0
NASHYD (0394) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.92
Unit Hyd Qpeak (cms)= 6.013

```

PEAK FLOW (cms) = 1.838 (1)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 11.632
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.186

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----  
| CALIB |  
| NASHYD (0390) | Area (ha) = 420.00 Curve Number (CN) = 55.0  
| ID= 1 DT=15.0 min | Ia (mm) = 5.00 # of Linear Res.(N) = 1.50  
| U.H. Tp(hrs) = 1.07
```

Unit Hyd Qpeak (cms) = 6.683

PEAK FLOW (cms) = 2.260 (1)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 12.424
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.198

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----  
| CALIB |  
| NASHYD (0388) | Area (ha) = 220.77 Curve Number (CN) = 58.0  
| ID= 1 DT=15.0 min | Ia (mm) = 5.00 # of Linear Res.(N) = 1.50  
| U.H. Tp(hrs) = 0.99
```

Unit Hyd Qpeak (cms) = 3.819

PEAK FLOW (cms) = 1.394 (1)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 13.635
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.217

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----  
| CALIB |  
| NASHYD (0386) | Area (ha) = 241.27 Curve Number (CN) = 61.0  
| ID= 1 DT=15.0 min | Ia (mm) = 5.00 # of Linear Res.(N) = 1.50  
| U.H. Tp(hrs) = 0.90
```

Unit Hyd Qpeak (cms) = 4.562

PEAK FLOW (cms) = 1.798 (1)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 14.947
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.238

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----  
| ADD HYD (8286) |  
| 1 + 2 = 3 | AREA OPEAK TPEAK R.V.  
| (ha) (cms) (hrs) (mm)  
+ ID1= 1 (0386): 241.27 1.798 7.25 14.95  
+ ID2= 2 (0388): 220.77 1.394 7.25 13.64  
=====   
ID = 3 (8286): 462.04 3.192 7.25 14.32
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
-----  
| CALIB |  
| NASHYD (0384) | Area (ha) = 199.07 Curve Number (CN) = 44.0  
| ID= 1 DT=15.0 min | Ia (mm) = 5.00 # of Linear Res.(N) = 1.50  
| U.H. Tp(hrs) = 0.96
```

Unit Hyd Qpeak (cms) = 3.537

PEAK FLOW (cms) = 0.803 (1)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 8.644
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.138

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----  
| CALIB |  
| NASHYD (0380) | Area (ha) = 182.01 Curve Number (CN) = 40.0  
| ID= 1 DT=15.0 min | Ia (mm) = 5.00 # of Linear Res.(N) = 1.50
```

U.H. Tp(hrs) = 0.55

Unit Hyd Qpeak (cms) = 5.609

PEAK FLOW (cms) = 0.964 (1)
TIME TO PEAK (hrs) = 6.500
RUNOFF VOLUME (mm) = 7.404
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.118

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----  
| CALIB |  
| NASHYD (0382) | Area (ha) = 216.59 Curve Number (CN) = 53.0  
| ID= 1 DT=15.0 min | Ia (mm) = 5.00 # of Linear Res.(N) = 1.50  
| U.H. Tp(hrs) = 0.64
```

Unit Hyd Qpeak (cms) = 5.733

PEAK FLOW (cms) = 1.611 (1)
TIME TO PEAK (hrs) = 6.750
RUNOFF VOLUME (mm) = 11.537
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.184

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----  
| ADD HYD (8290) |  
| 1 + 2 = 3 | AREA OPEAK TPEAK R.V.  
| (ha) (cms) (hrs) (mm)  
+ ID1= 1 (0380): 182.01 0.964 6.50 7.40  
+ ID2= 2 (0382): 216.59 1.611 6.75 11.54  
=====   
ID = 3 (8290): 398.60 2.571 6.75 9.65
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
-----  
| ROUTE CHN (6384) |  
| IN= 2---> OUT= 1 | Routing time step (min) = 15.00
```

```
-----  
<----- DATA FOR SECTION (3841.0) ----->  
Distance Elevation Manning  
0.00 294.40 0.0380  
10.59 291.93 0.0380  
21.17 289.19 0.0380  
26.46 287.99 0.0380  
31.76 286.79 0.0380  
37.45 279.97 0.0380  
44.10 279.79 0.0380  
50.74 279.71 0.0380  
57.39 279.64 0.0380 / 0.0300 Main Channel  
64.03 279.30 0.0300 Main Channel  
70.68 279.30 0.0300 Main Channel  
77.32 279.30 0.0300 Main Channel  
83.97 279.64 0.0300 / 0.0380 Main Channel  
90.61 279.78 0.0380  
97.26 281.86 0.0380  
103.90 282.87 0.0380  
110.54 284.85 0.0380  
117.18 286.31 0.0380  
123.82 287.59 0.0380  
130.46 294.00 0.0380
```

```
<----- TRAVEL TIME TABLE ----->
```

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.34	279.64	.715E+03	0.2	0.85	63.42
1.10	280.40	.507E+05	21.9	1.40	38.54
1.85	281.15	.141E+06	91.8	2.10	25.69
2.61	281.91	.269E+06	218.9	2.63	20.46
3.36	282.66	.445E+06	407.1	2.96	18.23
4.12	283.42	.681E+06	698.3	3.31	16.26
4.87	284.17	.977E+06	1114.8	3.68	14.65
5.63	284.93	.133E+07	1674.3	4.06	13.07
6.39	285.69	.173E+07	2457.8	4.59	11.73
7.14	286.44	.219E+07	3378.8	5.07	10.63
7.90	287.20	.261E+07	4418.4	5.47	9.85
8.65	287.95	.310E+07	5676.3	5.92	9.11
9.41	288.71	.361E+07	7150.5	6.40	8.42
10.17	289.47	.414E+07	8783.1	6.86	7.86
10.92	290.22	.469E+07	10575.1	7.30	7.38
11.68	290.98	.525E+07	12520.4	7.72	6.98
12.43	291.73	.582E+07	14619.0	8.12	6.64
13.19	292.49	.642E+07	16863.2	8.50	6.34
13.94	293.24	.703E+07	19259.8	8.86	6.08

```
<----- hydrograph -----> <-pipe / channel->  
AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
```

INFLOW : ID= 2 (8290) 398.60 2.57 6.75 9.65 0.42 0.89
 OUTFLOW : ID= 1 (6384) 398.60 2.01 8.00 9.65 0.40 0.88

ADD HYD (8288)
 1 + 2 = 3
 AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (0384): 199.07 0.803 7.25 8.64
 + ID2= 2 (6384): 398.60 2.015 8.00 9.65
 ID = 3 (8288): 597.67 2.797 8.00 9.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8284)
 1 + 2 = 3
 AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (8286): 462.04 3.192 7.25 14.32
 + ID2= 2 (8288): 597.67 2.797 8.00 9.31
 ID = 3 (8284): 1059.71 5.920 7.75 11.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8280)
 1 + 2 = 3
 AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (0390): 420.00 2.260 7.50 12.42
 + ID2= 2 (8284): 1059.71 5.920 7.75 11.50
 ID = 3 (8280): 1479.71 8.177 7.50 11.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0392) Area (ha)= 167.22 Curve Number (CN)= 62.0
 NASHYD (0392) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.74
 Unit Hyd Qpeak (cms)= 3.837
 PEAK FLOW (cms)= 1.497 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 15.356
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.245

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0378) Area (ha)= 606.72 Curve Number (CN)= 55.0
 NASHYD (0378) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.18

Unit Hyd Qpeak (cms)= 8.771
 PEAK FLOW (cms)= 3.046 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 12.439
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.198

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8282)
 1 + 2 = 3
 AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (0378): 606.72 3.046 8.00 12.44
 + ID2= 2 (0392): 167.22 1.497 7.00 15.36
 ID = 3 (8282): 773.94 4.443 7.25 13.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8278)

1 + 2 = 3 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8280): 1479.71 8.177 7.50 11.76
 + ID2= 2 (8282): 773.94 4.443 7.25 13.07
 ID = 3 (8278): 2253.65 12.619 7.50 12.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6394)
 ID= 2 -> OUT= 1 Routing time step (min)'= 15.00

DATA FOR SECTION (3941.0)

Distance	Elevation	Manning
0.00	283.00	0.0380
13.48	282.87	0.0380
53.92	280.08	0.0380
74.13	276.62	0.0380
97.72	265.45	0.0380
114.57	256.93	0.0380
131.42	253.04	0.0380
134.79	252.58	0.0380
138.53	251.74	0.0380 /0.0300 Main Channel
139.03	251.20	0.0300 Main Channel
141.53	251.20	0.0300 Main Channel
144.03	251.20	0.0300 Main Channel
144.53	251.74	0.0300 /0.0380 Main Channel
148.27	252.69	0.0380
151.64	252.97	0.0380
185.34	255.08	0.0380
219.03	257.54	0.0380
262.84	259.43	0.0380
310.02	262.80	0.0380
333.60	283.00	0.0380

TRAVEL TIME TABLE

DEPTH (m)	SECT (cu.m.)	VOLUME (cms)	FLOW RATE (m/s)	VELOCITY (min)	TRAV. TIME (min)
0.54	251.74	.827E+04	4.5	1.51	30.71
2.19	253.39	.758E+05	73.2	2.69	17.27
3.83	255.03	.282E+06	331.8	3.27	14.17
5.48	256.68	.633E+06	917.3	4.03	11.51
7.12	258.32	.112E+07	1889.5	4.69	9.90
8.77	259.97	.179E+07	3410.2	5.21	8.73
10.41	261.61	.259E+07	5927.6	6.05	7.66
12.06	263.26	.351E+07	8605.1	6.83	6.79
13.70	264.90	.447E+07	12544.9	7.81	5.94
15.35	266.55	.546E+07	17103.4	8.71	5.32
16.99	268.19	.648E+07	22253.7	9.56	4.85
18.64	269.84	.752E+07	27979.0	10.36	4.48
20.28	271.48	.858E+07	34269.0	11.11	4.17
21.93	273.13	.967E+07	41110.6	11.83	3.92
23.57	274.77	.108E+08	48495.9	12.51	3.71
25.22	276.42	.119E+08	56418.1	13.17	3.52
26.86	278.06	.131E+08	64410.6	13.69	3.39
28.51	279.71	.143E+08	73003.1	14.18	3.27
30.15	281.35	.156E+08	81379.3	14.50	3.20

hydrograph

INFLOW : ID= 2 (8278)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW : ID= 1 (6394)	2253.65	12.62	7.50	12.21	0.73	1.59
		12.25	8.25	12.21	0.73	1.59

ADD HYD (8276)
 1 + 2 = 3
 AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (0394): 325.45 1.838 7.25 11.63
 + ID2= 2 (6394): 2253.65 12.251 8.25 12.21
 ID = 3 (8276): 2579.10 13.996 8.00 12.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6396)
 ID= 2 -> OUT= 1 Routing time step (min)'= 15.00

DATA FOR SECTION (3961.0)

Distance	Elevation	Manning
0.00	263.00	0.0410
11.75	257.14	0.0410
23.50	253.97	0.0410
41.13	247.83	0.0410
146.38	232.09	0.0410
135.13	229.07	0.0410
149.82	228.97	0.0410
152.75	228.96	0.0410 /0.0300 Main Channel

154.19	228.73	0.0300	Main Channel
154.69	228.20	0.0300	Main Channel
155.69	228.20	0.0300	Main Channel
156.69	228.20	0.0300	Main Channel
157.19	228.73	0.0300	Main Channel
159.63	228.95	0.0300 / 0.0410	Main Channel
161.57	228.96	0.0410	
164.51	229.71	0.0410	
196.82	241.70	0.0410	
223.26	249.21	0.0410	
246.76	255.13	0.0410	
290.82	263.51	0.0410	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (cu.m.)	VOLUME (cms)	FLOW RATE (m/s)	VELOCITY (m/s)	TRAV.TIME (min)
0.75	228.95	.780E+04	2.2	0.98	58.06
2.54	230.74	.277E+06	145.1	1.78	31.76
4.33	232.53	.786E+06	609.1	2.63	21.50
6.13	234.33	.140E+07	1481.5	3.59	15.77
7.92	236.12	.207E+07	2677.3	4.39	12.90
9.71	237.91	.280E+07	4184.8	5.09	11.14
11.50	239.70	.357E+07	6001.4	5.71	9.93
13.29	241.49	.441E+07	8123.1	6.27	9.03
15.09	243.29	.530E+07	10549.4	6.77	8.37
16.88	245.08	.625E+07	13300.6	7.24	7.83
18.67	246.87	.726E+07	16396.2	7.68	7.38
20.46	248.66	.834E+07	19809.8	8.07	7.02
22.26	250.46	.949E+07	23532.5	8.43	6.72
24.05	252.25	.107E+08	27641.2	8.77	6.46
25.84	254.04	.120E+08	32155.2	9.10	6.22
27.63	255.83	.134E+08	36901.0	9.37	6.05
29.42	257.62	.149E+08	42147.7	9.64	5.88
31.22	259.42	.164E+08	48149.1	9.97	5.68
33.01	261.21	.181E+08	54644.8	10.29	5.51

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8276)	2579.10	14.00	8.00	12.14	0.90
OUTFLOW : ID= 1 (6396)	2579.10	12.65	9.25	12.14	0.88

ADD HYD (8274)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0396):	305.21	2.564	7.50	19.20
+ ID2= 2 (6396):	2579.10	12.651	9.25	12.14
ID = 3 (8274):	2884.31	14.864	9.00	12.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8270)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8272):	2776.21	11.221	10.25	13.91
+ ID2= 2 (8274):	2884.31	14.864	9.00	12.88
ID = 3 (8270):	5660.52	25.641	9.25	13.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5506)	DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	60.8820	135.6832		
31.1490	24.6597	96.2770	900.4531		
36.8120	37.0045	96.6770	900.4531		
45.3070	86.3439	0.0000	0.0000		

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8270)	5660.520	25.641	9.25
OUTFLOW : ID= 1 (5506)	5660.520	21.225	11.75

PEAK FLOW REDUCTION [Qout/Qin](%) = 82.78
 TIME SHFT OF PEAK FLOW (min)=150.00
 MAXIMUM STORAGE USED (ha.m.)= 16.8111

CALIB	NASHYD (0406)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	Ia (mm)	142.65	66.0
U.H. Tp(hrs)=		5.00	# of Linear Res.(N)= 1.50
		0.59	

Unit Hyd Qpeak (cms)= 4.135

PEAK FLOW (cms)= 1.735 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 17.263
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.275

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD (0404)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	Ia (mm)	246.46	47.0
U.H. Tp(hrs)=		5.00	# of Linear Res.(N)= 1.50
		0.98	

Unit Hyd Qpeak (cms)= 4.280

PEAK FLOW (cms)= 1.084 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 9.573
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.153

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD (0402)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	Ia (mm)	244.00	61.0
U.H. Tp(hrs)=		5.00	# of Linear Res.(N)= 1.50
		1.07	

Unit Hyd Qpeak (cms)= 3.879

PEAK FLOW (cms)= 1.593 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 14.988
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.239

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD (0400)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	Ia (mm)	93.97	52.0
U.H. Tp(hrs)=		5.00	# of Linear Res.(N)= 1.50
		0.44	

Unit Hyd Qpeak (cms)= 3.630

PEAK FLOW (cms)= 0.902 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 11.008
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.176

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD (0398)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	Ia (mm)	328.19	55.0
U.H. Tp(hrs)=		5.00	# of Linear Res.(N)= 1.50
		0.83	

Unit Hyd Qpeak (cms)= 6.759

PEAK FLOW (cms)= 2.154 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 12.370
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.197

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8268)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0398):	328.19	2.154	7.00	12.37
+ ID2= 2 (0400):	93.97	0.902	6.50	11.01
ID = 3 (8268):	422.16	2.966	6.75	12.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6402)

| IN= 2--> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (4021.0) -----					
Distance	Elevation	Manning			
0.00	238.50	0.0360			
11.50	238.00	0.0360			
23.00	237.93	0.0360			
34.49	236.39	0.0360			
63.24	233.98	0.0360			
97.73	228.15	0.0360			
123.60	227.08	0.0360			
126.48	226.61	0.0360			
127.60	226.47	0.0360 / 0.0330	Main Channel		
127.85	225.25	0.0330	Main Channel		
129.35	225.25	0.0330	Main Channel		
130.85	225.25	0.0330	Main Channel		
131.10	226.47	0.0330 / 0.0360	Main Channel		
132.22	226.59	0.0360			
143.72	227.42	0.0360			
169.59	227.88	0.0360			
192.59	231.19	0.0360			
218.46	233.02	0.0360			
241.45	235.50	0.0360			
284.57	236.43	0.0360			

----- TRAVEL TIME TABLE -----					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.41	225.66	.391E+04	1.7	1.36	38.31
0.81	226.06	.804E+04	5.0	1.93	27.02
1.22	226.47	.124E+05	9.1	2.30	22.61
1.84	227.09	.308E+05	23.3	2.36	22.03
2.46	227.71	.904E+05	62.0	2.14	24.31
3.09	228.34	.223E+06	188.3	2.63	19.75
3.71	228.96	.379E+06	410.2	3.38	15.39
4.33	229.58	.550E+06	711.5	4.04	12.89
4.95	230.20	.737E+06	1091.3	4.62	11.25
5.58	230.83	.939E+06	1550.6	5.15	10.10
6.20	231.45	.115E+07	2072.4	5.59	9.31
6.82	232.07	.140E+07	2659.2	5.93	8.77
7.44	232.69	.167E+07	3345.8	6.27	8.30
8.07	233.32	.195E+07	4159.0	6.64	7.83
8.69	233.94	.226E+07	5100.8	7.03	7.39
9.31	234.56	.259E+07	6081.3	7.32	7.11
9.93	235.18	.295E+07	7183.1	7.60	6.84
10.56	235.81	.334E+07	8145.2	7.62	6.83
11.18	236.43	.379E+07	9116.0	7.51	6.92

----- hydrograph -----						<-pipe / channel-->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)		
INFLOW : ID= 2 (8268)	422.16	2.97	6.75	12.07	0.56	1.53	
OUTFLOW : ID= 1 (6402)	422.16	2.70	7.50	12.07	0.53	1.49	

ADD HYD (8266)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0402):	244.00	1.593	7.50	14.99
+ ID2= 2 (6402):	422.16	2.701	7.50	12.07
ID = 3 (8266):	666.16	4.294	7.50	13.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8264)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0404):	246.46	1.084	7.50	9.57
+ ID2= 2 (8266):	666.16	4.294	7.50	13.14
ID = 3 (8264):	912.62	5.378	7.50	12.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8262)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0406):	142.65	1.735	6.75	17.26
+ ID2= 2 (8264):	912.62	5.378	7.50	12.17
ID = 3 (8262):	1055.27	6.905	7.25	12.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8260)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (5506):	5660.52	21.225	11.75	13.39
+ ID2= 2 (8262):	1055.27	6.905	7.25	12.86
ID = 3 (8260):	6715.79	24.324	11.25	13.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8258)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0408):	231.62	2.048	6.75	13.51
+ ID2= 2 (8260):	6715.79	24.324	11.25	13.31
ID = 3 (8258):	6947.41	24.951	11.00	13.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8256)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0410):	572.01	1.949	8.25	9.94
+ ID2= 2 (8258):	6947.41	24.951	11.00	13.31
ID = 3 (8256):	7519.42	26.526	11.00	13.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5507)					
IN= 2--> OUT= 1	DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
		0.0000	0.0000	90.6140	67.8416
		39.6440	12.3348	*****	160.3529
		48.1390	18.5023	*****	160.3529
		67.9600	37.0045	0.0000	0.0000

INFLOW : ID= 2 (8256)	7519.421	26.526	11.00	13.06
OUTFLOW : ID= 1 (5507)	7519.421	25.968	11.75	13.06

PEAK FLOW REDUCTION [Qout/Qin](%)= 97.89
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 8.0848

CALIB (0420)			
NASHYD	Area (ha)	Curve Number (CN)	# of Linear Res.(N)
ID= 1 DT=15.0 min	175.82	53.0	1.50
	Ia (mm)= 5.00		
	U.H. Tp(hrs)= 0.81		

Unit Hyd Opeak (cms)= 3.692

PEAK FLOW (cms)= 1.097 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 11.603
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.185

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0418)			
NASHYD	Area (ha)	Curve Number (CN)	# of Linear Res.(N)
ID= 1 DT=15.0 min	182.79	53.0	1.50
	Ia (mm)= 5.00		
	U.H. Tp(hrs)= 1.05		

Unit Hyd Opeak (cms)= 2.966

PEAK FLOW (cms)= 1.335 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 16.442
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.262

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB			
-------	--	--	--

NASHYD (0416) Area (ha)= 439.30 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.29

Unit Hyd Qpeak (cms)= 5.832
 PEAK FLOW (cms)= 2.770 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 16.482
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.263

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0412) Area (ha)= 238.70 Curve Number (CN)= 54.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.80

Unit Hyd Qpeak (cms)= 5.088
 PEAK FLOW (cms)= 1.557 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 11.975
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.191

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (4141) Area (ha)= 43.70
 ID= 1 DT=15.0 min Total Imp(%)= 36.00 Dir. Conn.(%)= 17.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	15.73	27.97
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	539.75	40.00
Mannings n	0.013	0.250
Max.Eff.Inten.(mm/hr)	82.76	24.71
over (min)	15.00	30.00
Storage Coeff. (min)	9.33 (ii)	28.04 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.04
		TOTALS
PEAK FLOW (cms)	1.47	1.34
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	60.70	20.49
TOTAL RAINFALL (mm)	62.70	62.70
RUNOFF COEFFICIENT	0.97	0.33

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 64.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5414)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.4720	1.9277
0.3820	0.3152	3.8720	1.9277
1.8280	0.4690	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (4141)	43.700	2.298	6.00	27.32
OUTFLOW: ID= 1 (5414)	43.700	1.248	6.50	27.32

PEAK FLOW REDUCTION [Qout/Qin](%)= 54.31
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 0.4093

CALIB
 STANDHYD (4142) Area (ha)= 144.30
 ID= 1 DT=15.0 min Total Imp(%)= 21.00 Dir. Conn.(%)= 10.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	30.30	114.00
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50

Length (m)= 980.82 40.00
 Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 82.76 19.43
 over (min) 15.00 45.00
 Storage Coeff. (min)= 13.35 (ii) 33.95 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03

TOTALS

	(cms)	(hrs)	(mm)
PEAK FLOW	2.53	3.58	4.256 (iii)
TIME TO PEAK	6.00	6.50	6.50
RUNOFF VOLUME	60.70	18.51	22.73
TOTAL RAINFALL	62.70	62.70	62.70
RUNOFF COEFFICIENT	0.97	0.30	0.36

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 64.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8254)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (4142):	144.30	4.256	6.50	22.73
+ ID2= 2 (5414):	43.70	1.248	6.50	27.32

ID = 3 (8254):	188.00	5.504	6.50	23.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8252)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0412):	238.70	1.557	7.00	11.98
+ ID2= 2 (8254):	188.00	5.504	6.50	23.79

ID = 3 (8252):	426.70	6.962	6.50	17.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6416) | Routing time step (min)'= 15.00

----- DATA FOR SECTION (4161.0) ----->

Distance	Elevation	Manning
0.00	270.07	0.0340
20.67	267.91	0.0340
62.01	264.33	0.0340
113.69	259.75	0.0340
165.37	253.30	0.0340
227.38	246.29	0.0340
232.55	246.03	0.0340
237.72	246.16	0.0340
241.39	246.02	0.0340 /0.0300
241.64	245.75	0.0300
242.89	245.75	0.0300
244.14	245.75	0.0300
244.39	246.02	0.0300 /0.0340
248.06	246.20	0.0340
253.22	246.28	0.0340
258.39	246.63	0.0340
346.25	252.57	0.0340
413.43	257.77	0.0340
465.11	261.78	0.0340
511.62	270.00	0.0340

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.27	246.02	.295E+04	0.6	0.75	88.26
1.53	247.28	.168E+06	69.7	1.65	40.13
2.79	248.54	.490E+06	299.5	2.42	27.28
4.06	249.81	.962E+06	740.1	3.05	21.67
5.32	251.07	.158E+07	1440.0	3.61	18.33
6.58	252.33	.235E+07	2443.8	4.12	16.06
7.84	253.59	.327E+07	3811.2	4.63	14.30
9.10	254.85	.432E+07	5565.1	5.11	12.94
10.37	256.12	.550E+07	7721.3	5.57	11.88
11.63	257.38	.682E+07	10309.9	6.00	11.02
12.89	258.64	.827E+07	13360.6	6.42	10.31
14.15	259.90	.985E+07	16884.5	6.81	9.72
15.42	261.17	.116E+08	20810.4	7.14	9.26

16.68	262.43	.134E+08	25510.4	7.53	8.78
17.94	263.69	.154E+08	30958.0	7.97	8.30
19.20	264.95	.175E+08	36988.3	8.38	7.89
20.46	266.21	.197E+08	43617.1	8.78	7.53
21.73	267.48	.220E+08	50865.1	9.17	7.22
22.99	268.74	.244E+08	58854.0	9.56	6.92

		<--- hydrograph --->				<-pipe / channel->	
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (8252)	426.70	6.96	6.50	17.18	0.39	0.79	
OUTFLOW: ID= 1 (6416)	426.70	3.67	7.50	17.18	0.33	0.77	

ADD HYD (8250)		AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0416):	439.30	2.770	8.00	16.48	
+ ID2= 2 (6416):	426.70	3.675	7.50	17.18	
ID = 3 (8250):	866.00	6.400	7.50	16.83	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8248)		AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0418):	382.79	1.335	7.50	16.44	
+ ID2= 2 (8250):	866.00	6.400	7.50	16.83	
ID = 3 (8248):	1048.79	7.735	7.50	16.76	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8246)		AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0420):	175.82	1.097	7.00	11.60	
+ ID2= 2 (8248):	1048.79	7.735	7.50	16.76	
ID = 3 (8246):	1224.61	8.801	7.50	16.02	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8244)		AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (5507):	7519.42	25.968	11.75	13.06	
+ ID2= 2 (8246):	1224.61	8.801	7.50	16.02	
ID = 3 (8244):	8744.03	30.680	11.00	13.47	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8242)		AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0422):	780.20	2.591	9.25	12.30	
+ ID2= 2 (8244):	8744.03	30.680	11.00	13.47	
ID = 3 (8242):	9524.23	33.097	11.00	13.36	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5508)		OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1	DT= 15.0 min	(cms)	(ha.m.)	(cms)	(ha.m.)
		0.0000	0.0000	*****	197.3574
		76.4550	30.8371	*****	394.7148
		*****	61.6742	*****	394.7248
		*****	*****	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8242)	9524.231	33.097	11.00	13.36
OUTFLOW: ID= 1 (5508)	9524.231	32.116	12.25	13.36

PEAK FLOW REDUCTION [Qout/Qin]= 97.04
 TIME SHIFT OF PEAK FLOW (min)= 75.00
 MAXIMUM STORAGE USED (ha.m.)= 12.9573

CALIB	Area	(ha)=	Curve Number	(CN)=
NASHVD (0336)	2785.00			72.0
ID= 1 DT=15.0 min	5.00			(mm)= 5.00
	15.39			# of Linear Res.(N)= 1.50
U.H. Tp(hrs)=				

Unit Hyd Qpeak (cms)= 3.090

PEAK FLOW (cms)= 2.580 (i)
 TIME TO PEAK (hrs)= 22.250
 RUNOFF VOLUME (mm)= 21.246
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.339

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)=	Dir. Conn.(%)=
STANDHYD (3382)	525.90		17.00
ID= 1 DT=15.0 min	37.00		

Surface Area (ha)= 194.58 331.32
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1872.43 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 56.43 44.05
 over (min) 30.00 45.00
 Storage Coeff. (min)= 22.93 (ii) 37.78 (ii)
 Unit Hyd. Tpeak (min)= 30.00 45.00
 Unit Hyd. peak (cms)= 0.04 0.03

TOTALS
 20.457 (iii)

PEAK FLOW (cms)= 10.37 13.64
 TIME TO PEAK (hrs)= 6.25 6.50 6.25
 RUNOFF VOLUME (mm)= 60.70 21.93 28.52
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.35 0.45

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)=	Dir. Conn.(%)=
STANDHYD (3381)	73.00		23.00
ID= 1 DT=15.0 min	49.00		

Surface Area (ha)= 35.77 37.23
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 697.61 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 82.76 55.73
 over (min) 15.00 30.00
 Storage Coeff. (min)= 10.88 (ii) 24.40 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

TOTALS
 4.845 (iii)

PEAK FLOW (cms)= 3.17 2.65 4.845 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 60.70 24.15 32.56
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.39 0.52

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5338)		OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1	DT= 15.0 min	(cms)	(ha.m.)	(cms)	(ha.m.)
		0.0000	0.0000	3.3350	1.2400

0.3230	0.3074	4.3560	1.3520
0.9360	0.5374	5.3290	1.4590
1.7590	0.8302	5.7290	1.4690
2.4110	1.0550	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (3381)	73.000	4.845	6.00	32.56
OUTFLOW: ID= 1 (5338)	73.000	1.996	6.75	32.55

PEAK FLOW REDUCTION [Qout/Qin]= 41.19
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 0.9240

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ADD HYD (8310)				
1 + 2 = 3				
ID1= 1 (3382):	529.90	20.457	6.25	28.52
+ ID2= 2 (5338):	73.00	1.996	6.75	32.55
ID = 3 (8310):	598.90	22.328	6.50	29.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

	Area (ha)	Curve Number (CN)
CALIB (3342)	587.10	56.0
NASHYD (0306)	5.00	1.50
ID= 1 DT=15.0 min		
U.H. Tp(hrs)=	8.19	

Unit Hyd Qpeak (cms)= 1.224

PEAK FLOW (cms)= 0.616 (i)
 TIME TO PEAK (hrs)= 15.250
 RUNOFF VOLUME (mm)= 12.935
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.206

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

	Area (ha)	Dir. Conn. (%)
CALIB STANDHYD (3341)	33.00	31.00
ID= 1 DT=15.0 min		
Total Imp(%)=	51.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	16.83	16.17
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	469.04	40.00
Mannings n =	0.013	0.250

Max. Ref. Inten. (mm/hr)=	82.76	19.84
over (min)=	15.00	30.00
Storage Coeff. (min)=	8.58 (ii)	29.01 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.04

TOTALS

PEAK FLOW (cms)=	2.07	0.61	2.446 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	60.70	15.50	29.51
TOTAL RAINFALL (mm)=	62.70	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.25	0.47

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 52.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
RESERVOIR (5334)				
IN= 2--> OUT= 1				
DT= 15.0 min				
0.0000	0.0000	0.4540	0.4604	
0.0390	0.1699	0.8330	0.5249	
0.1860	0.2610	1.0130	0.5896	
0.3750	0.3534	1.4130	0.5996	
0.4340	0.3813	0.0000	0.0000	

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (3341)	33.000	2.446	6.00	29.51
OUTFLOW: ID= 1 (5334)	33.000	0.677	6.75	29.48

PEAK FLOW REDUCTION [Qout/Qin]= 27.69

TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 0.4746

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ADD HYD (8314)				
1 + 2 = 3				
ID1= 1 (3342):	587.10	0.616	15.25	12.94
+ ID2= 2 (5334):	33.00	0.677	6.75	29.48
ID = 3 (8314):	620.10	0.893	6.75	13.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

	Area (ha)	Curve Number (CN)
CALIB (0306)	283.97	52.0
NASHYD (0306)	5.00	1.50
ID= 1 DT=15.0 min		
U.H. Tp(hrs)=	6.44	

Unit Hyd Qpeak (cms)= 0.753

PEAK FLOW (cms)= 0.331 (i)
 TIME TO PEAK (hrs)= 13.750
 RUNOFF VOLUME (mm)= 11.388
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.182

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

	Area (ha)	Curve Number (CN)
CALIB NASHYD (0286)	260.51	84.0
ID= 1 DT=15.0 min		
U.H. Tp(hrs)=	1.16	

Unit Hyd Qpeak (cms)= 3.834

PEAK FLOW (cms)= 3.462 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 31.127
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.496

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

	Area (ha)	Curve Number (CN)
CALIB NASHYD (0282)	449.38	77.0
ID= 1 DT=15.0 min		
U.H. Tp(hrs)=	1.47	

Unit Hyd Qpeak (cms)= 5.226

PEAK FLOW (cms)= 3.907 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 24.782
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.395

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

	Area (ha)	Curve Number (CN)
CALIB NASHYD (0284)	78.93	84.0
ID= 1 DT=15.0 min		
U.H. Tp(hrs)=	0.57	

Unit Hyd Qpeak (cms)= 2.344

PEAK FLOW (cms)= 1.836 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 30.659
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.489

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ADD HYD (8388)				
1 + 2 = 3				
ID1= 1 (0282):	449.38	3.907	8.00	24.78
+ ID2= 2 (0284):	78.93	1.836	6.50	30.66
ID = 3 (8388):	528.31	5.316	7.25	25.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6286) |
IN= 2--> OUF= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (2861.0) -----

Distance	Elevation	Manning	
0.00	233.00	0.0450	
20.58	228.51	0.0450	
41.17	227.74	0.0450	
61.46	227.41	0.0450	
97.77	225.96	0.0450	
149.23	223.94	0.0450	
200.69	220.84	0.0450	
226.42	220.66	0.0450	
238.85	220.22	0.0450	
241.35	220.01	0.0450 / 0.0350	Main Channel
241.85	219.70	0.0350	Main Channel
245.85	219.72	0.0350	Main Channel
246.35	220.06	0.0350	Main Channel
248.85	220.23	0.0350	Main Channel
303.60	221.64	0.0350	Main Channel
380.79	224.98	0.0450	
432.25	229.54	0.0450	
457.98	233.33	0.0450	
483.71	234.27	0.0450	
509.44	233.81	0.0450	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.31	220.01	.382E+04	0.8	0.57	82.89
1.06	220.76	.525E+05	12.1	0.66	72.51
1.81	221.51	.240E+06	88.6	1.05	45.21
2.56	222.26	.510E+06	261.7	1.46	32.47
3.31	223.01	.843E+06	533.8	1.81	26.34
4.06	223.76	.124E+07	913.6	2.10	22.64
4.81	224.51	.171E+07	1399.0	2.34	20.33
5.56	225.26	.225E+07	2051.0	2.60	18.27
6.31	226.01	.285E+07	2907.0	2.91	16.35
7.06	226.76	.352E+07	3902.5	3.16	15.04
7.82	227.52	.426E+07	5069.9	3.39	14.01
8.57	228.27	.507E+07	6433.0	3.62	13.13
9.32	229.02	.593E+07	8069.2	3.89	12.24
10.07	229.77	.681E+07	9924.7	4.16	11.44
10.82	230.52	.772E+07	11962.8	4.42	10.75
11.57	231.27	.864E+07	14167.1	4.68	10.17
12.32	232.02	.959E+07	16537.1	4.92	9.66
13.07	232.77	.105E+08	19070.7	5.16	9.22
13.82	233.52	.115E+08	21790.4	5.39	8.82

----- hydrograph ----- <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8388)	528.31	5.32	7.25	25.66	0.61	0.60
OUTFLOW : ID= 1 (6286)	528.31	4.49	9.00	25.66	0.56	0.60

ADD HYD (8386) |
1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 1 (0286): 260.51 3.462 7.50 31.13
+ ID2= 2 (6286): 528.31 4.493 9.00 25.66
ID = 3 (8386): 788.82 7.643 8.50 27.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0302) | Area (ha)= 473.90 Curve Number (CN)= 58.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.66

Unit Hyd Qpeak (cms)= 4.874

PEAK FLOW (cms)= 2.024 (i)
TIME TO PEAK (hrs)= 9.500
RUNOFF VOLUME (mm)= 13.712
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.219

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0300) | Area (ha)= 258.93 Curve Number (CN)= 52.0
ID= 1 DT=15.0 min | Ia (mm)= 4.00 # of Linear Res.(N)= 1.50

U.H. Tp(hrs)= 1.03

Unit Hyd Qpeak (cms)= 4.292

PEAK FLOW (cms)= 1.347 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 11.639
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.186

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0298) | Area (ha)= 330.51 Curve Number (CN)= 45.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.26

Unit Hyd Qpeak (cms)= 4.461

PEAK FLOW (cms)= 1.133 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 8.978
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.143

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8395) | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
1 + 2 = 3 | ID1= 1 (0298): 330.51 1.133 8.00 8.98
+ ID2= 2 (0300): 258.93 1.347 7.50 11.64
ID = 3 (8395): 589.44 2.465 7.75 10.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0288) | Area (ha)= 340.83 Curve Number (CN)= 78.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 2.21

Unit Hyd Qpeak (cms)= 2.629

PEAK FLOW (cms)= 2.210 (i)
TIME TO PEAK (hrs)= 9.500
RUNOFF VOLUME (mm)= 25.660
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.409

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0290) | Area (ha)= 269.18 Curve Number (CN)= 78.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 4.279

PEAK FLOW (cms)= 3.070 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 25.504
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.407

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8397) | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
1 + 2 = 3 | ID1= 1 (0288): 340.83 2.210 9.50 25.66
+ ID2= 2 (0290): 269.18 3.070 7.50 25.50
ID = 3 (8397): 610.01 5.105 8.00 25.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0296) | Area (ha)= 293.65 Curve Number (CN)= 76.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.13

Unit Hyd Qpeak (cms)= 4.437
 PEAK FLOW (cms)= 3.009 (1)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 23.935
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.382

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB | Area (ha)= 738.49 | Curve Number (CN)= 68.0
 NASHYD (0292) | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.52

Unit Hyd Qpeak (cms)= 8.289
 PEAK FLOW (cms)= 4.648 (1)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 18.682
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.298

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB | Area (ha)= 274.15 | Curve Number (CN)= 57.0
 NASHYD (0294) | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.87

Unit Hyd Qpeak (cms)= 5.367
 PEAK FLOW (cms)= 1.843 (1)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 13.187
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.210

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8398) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0292): 738.49 4.648 8.25 18.68
 + ID2= 2 (0294): 274.15 1.843 7.00 13.19
 ID = 3 (8398): 1012.64 6.373 8.00 17.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6296) |
 IN= 2--> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (2961.0) -----

Distance	Elevation	Manning
0.00	243.98	0.0400
42.59	243.18	0.0400
85.17	241.81	0.0400
120.66	240.50	0.0400
156.15	239.56	0.0400
198.74	236.15	0.0400
237.78	234.01	0.0400
241.33	233.82	0.0400
248.77	233.12	0.0400 / 0.0400 Main Channel
249.87	232.32	0.0400 Main Channel
250.37	231.80	0.0400 Main Channel
250.87	232.23	0.0400 Main Channel
251.97	233.10	0.0400 / 0.0400 Main Channel
255.37	233.22	0.0400
259.87	233.87	0.0400
262.62	234.12	0.0400
266.17	234.23	0.0400
283.91	234.73	0.0400
337.15	241.75	0.0400
351.34	244.00	0.0400

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.43	232.23	.510E+03	0.1	0.53	81.07
0.87	232.67	.213E+04	0.7	0.83	51.49
1.30	233.10	.501E+04	2.2	1.10	38.75
1.98	233.78	.242E+05	12.6	1.34	31.93
2.66	234.46	.738E+05	42.1	1.46	29.21
3.34	235.14	.177E+06	127.9	1.64	23.13

4.02	235.82	.314E+06	281.4	2.29	18.61
4.70	236.50	.481E+06	509.4	2.71	15.73
5.38	237.18	.672E+06	815.6	3.11	13.73
6.06	237.86	.886E+06	1200.7	3.46	12.30
6.74	238.54	.112E+07	1659.4	3.60	11.23
7.42	239.22	.139E+07	2226.9	4.11	10.38
8.10	239.90	.168E+07	2808.4	4.28	9.95
8.78	240.58	.202E+07	3470.4	4.40	9.68
9.46	241.26	.240E+07	4331.7	4.61	9.24
10.14	241.94	.283E+07	5355.2	4.83	8.84
10.82	242.62	.330E+07	6482.6	5.03	8.48
11.50	243.30	.381E+07	7764.1	5.21	8.18
12.18	243.98	.439E+07	9101.2	5.31	8.04

----- hydrograph -----<-pipe / channel-->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8398) 1012.64 6.37 8.00 17.19 1.57 1.18
 OUTFLOW : ID= 1 (6296) 1012.64 6.16 8.50 17.19 1.56 1.18

 ADD HYD (8396) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0296): 293.65 3.009 7.50 23.94
 + ID2= 2 (6296): 1012.64 6.164 8.50 17.19
 ID = 3 (8396): 1306.29 9.032 8.25 18.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8394) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (8396): 1306.29 9.032 8.25 18.71
 + ID2= 2 (8397): 610.01 5.105 8.00 25.59
 ID = 3 (8394): 1916.30 14.100 8.25 20.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8392) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (8394): 1916.30 14.100 8.25 20.90
 + ID2= 2 (8395): 589.44 2.465 7.75 10.15
 ID = 3 (8392): 2505.74 16.538 8.00 18.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6302) |
 IN= 2--> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (3021.0) -----

Distance	Elevation	Manning
0.00	228.10	0.0400
18.47	227.12	0.0400
36.95	226.12	0.0400
46.18	225.84	0.0400
55.42	225.58	0.0400
272.47	222.88	0.0400
277.09	222.76	0.0400
281.71	222.58	0.0400
288.54	222.18	0.0400 / 0.0300 Main Channel
288.64	221.00	0.0300 Main Channel
290.94	221.00	0.0300 Main Channel
291.04	221.00	0.0300 Main Channel
291.54	222.75	0.0300 / 0.0400 Main Channel
300.18	222.83	0.0400
304.80	223.04	0.0400
309.42	223.25	0.0400
318.65	223.69	0.0400
360.22	225.57	0.0400
397.16	227.60	0.0400
457.20	228.35	0.0400

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.29	221.29	.228E+04	0.2	0.32	165.63
0.59	221.59	.465E+04	0.7	0.45	116.15
0.88	221.88	.713E+04	1.2	0.54	96.87
1.18	222.18	.971E+04	1.9	0.61	86.20

1.57	222.57	.175E+05	3.3	0.59	88.86
1.97	222.97	.427E+05	6.4	0.47	111.39
2.36	223.36	.115E+06	16.1	0.44	119.14
2.76	223.76	.237E+06	37.1	0.49	106.49
3.15	224.15	.409E+06	72.9	0.56	93.40
3.55	224.55	.631E+06	127.0	0.63	82.76
3.94	224.94	.903E+06	202.4	0.70	74.37
4.34	225.34	1.23E+07	301.8	0.77	67.68
4.73	225.73	1.60E+07	433.5	0.85	61.37
5.13	226.13	2.00E+07	602.2	0.95	55.29
5.52	226.52	2.42E+07	804.7	1.05	50.13
5.92	226.92	2.86E+07	1034.2	1.14	46.11
6.31	227.31	3.32E+07	1290.5	1.22	42.88
6.71	227.71	3.80E+07	1560.9	1.29	40.55
7.10	228.10	4.32E+07	1835.6	1.34	39.19

<--- hydrograph ---> <-pipe / channel-->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8392) 2505.74	16.54	8.00	18.37	2.37	0.44
OUTFLOW: ID= 1 (8302) 2505.74	13.15	10.50	18.37	2.24	0.45

ADD HYD (8390)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0302):	473.90	2.024	8.50	13.71
+ ID2= 2 (8302):	2505.74	13.148	10.50	18.37
=====				
ID = 3 (8390):	2979.64	15.009	10.25	17.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8348)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8386):	788.82	7.643	8.50	27.47
+ ID2= 2 (8390):	2979.64	15.009	10.25	17.63
=====				
ID = 3 (8348):	3768.46	21.986	9.75	19.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	QPEAK	TPEAK	R.V.
NASHYD (0304)	(ha)	(cms)	(hrs)	(mm)	
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50		
	U.H. Tp(hrs)=	2.78			
Unit Hyd Opeak	(cms)=	1.793			
PEAK FLOW	(cms)=	0.987 (1)			
TIME TO PEAK	(hrs)=	10.500			
RUNOFF VOLUME	(mm)=	16.057			
TOTAL RAINFALL	(mm)=	62.700			
RUNOFF COEFFICIENT	=	0.256			

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	QPEAK	TPEAK	R.V.
NASHYD (0280)	(ha)	(cms)	(hrs)	(mm)	
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50		
	U.H. Tp(hrs)=	0.85			
Unit Hyd Opeak	(cms)=	6.009			
PEAK FLOW	(cms)=	4.745 (1)			
TIME TO PEAK	(hrs)=	7.000			
RUNOFF VOLUME	(mm)=	28.965			
TOTAL RAINFALL	(mm)=	62.700			
RUNOFF COEFFICIENT	=	0.462			

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	QPEAK	TPEAK	R.V.
NASHYD (0278)	(ha)	(cms)	(hrs)	(mm)	
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50		
	U.H. Tp(hrs)=	1.52			
Unit Hyd Opeak	(cms)=	5.453			
PEAK FLOW	(cms)=	4.873 (1)			
TIME TO PEAK	(hrs)=	8.000			
RUNOFF VOLUME	(mm)=	29.184			

TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.465

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	QPEAK	TPEAK	R.V.
NASHYD (0276)	(ha)	(cms)	(hrs)	(mm)	
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50		
	U.H. Tp(hrs)=	0.67			
Unit Hyd Opeak	(cms)=	2.302			
PEAK FLOW	(cms)=	1.552 (1)			
TIME TO PEAK	(hrs)=	6.750			
RUNOFF VOLUME	(mm)=	26.101			
TOTAL RAINFALL	(mm)=	62.700			
RUNOFF COEFFICIENT	=	0.416			

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	QPEAK	TPEAK	R.V.
NASHYD (0274)	(ha)	(cms)	(hrs)	(mm)	
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50		
	U.H. Tp(hrs)=	1.08			
Unit Hyd Opeak	(cms)=	6.182			
PEAK FLOW	(cms)=	4.016 (1)			
TIME TO PEAK	(hrs)=	7.500			
RUNOFF VOLUME	(mm)=	23.174			
TOTAL RAINFALL	(mm)=	62.700			
RUNOFF COEFFICIENT	=	0.370			

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8360)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0274):	392.49	4.016	7.50	23.17
+ ID2= 2 (0276):	90.89	1.552	6.75	26.10
=====				
ID = 3 (8360):	483.38	5.470	7.25	23.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8358)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0278):	485.49	4.873	8.00	29.18
+ ID2= 2 (8360):	483.38	5.470	7.25	23.72
=====				
ID = 3 (8358):	968.87	10.143	7.50	26.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6280) Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2801.0) ----->

Distance	Elevation	Manning
0.00	241.14	0.0500
13.32	240.80	0.0500
39.95	240.07	0.0500
96.54	236.21	0.0500
113.19	234.15	0.0500
123.18	232.35	0.0500
143.15	225.80	0.0500
149.81	225.62	0.0500
153.14	225.40	0.0500
157.30	224.76	0.0500 /0.0300 Main Channel
159.80	224.26	0.0300 Main Channel
162.30	224.85	0.0300 Main Channel
162.55	224.97	0.0300 /0.0500 Main Channel
167.80	225.05	0.0500
186.43	229.14	0.0500
213.06	234.75	0.0500
236.37	237.09	0.0500
266.33	237.31	0.0500
292.96	237.83	0.0500
329.58	241.50	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
-------	------	--------	-----------	----------	-----------

(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.50	224.76	.263E+04	0.8	0.65	58.31
1.36	225.62	.278E+05	13.4	1.10	34.54
2.22	226.48	.895E+05	52.6	1.40	27.09
3.09	227.35	.157E+06	120.4	1.74	21.78
3.95	228.21	.242E+06	216.3	2.04	18.64
4.81	229.07	.340E+06	341.8	2.29	16.56
5.67	229.93	.450E+06	498.3	2.52	15.06
6.53	230.79	.574E+06	686.3	2.73	13.90
7.40	231.66	.711E+06	913.8	2.93	12.97
8.26	232.52	.861E+06	1173.4	3.11	12.23
9.12	233.38	1.03E+07	1461.3	3.24	11.72
9.98	234.24	1.21E+07	1793.3	3.37	11.26
10.85	235.11	1.42E+07	2141.0	3.45	11.02
11.71	235.97	1.65E+07	2530.4	3.50	10.86
12.57	236.83	1.92E+07	2954.0	3.51	10.81
13.43	237.69	2.22E+07	3160.3	3.18	11.93
14.29	238.55	2.71E+07	3869.4	3.26	11.67
15.16	239.42	3.20E+07	4740.3	3.38	11.24
16.02	240.28	3.73E+07	5688.4	3.48	10.92

<--- hydrograph ---> <-pipe / channel-->

	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8358)	968.87	10.14	7.50	26.46	1.14	0.93
OUTFLOW: ID= 1 (6280)	968.87	9.72	8.25	26.46	1.11	0.92

ADD HYD (8354)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID= 1 (0280):	299.86	4.745	7.00	28.96
+ ID= 2 (6280):	968.87	9.721	8.25	26.46
ID = 3 (8354):	1268.73	13.898	8.00	27.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 75.0
NASHVD (0272)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50	
ID= 1 DT=15.0 min	U.H. Tp(hrs)= 1.09			

Unit Hyd Qpeak (cms)= 2.456

PEAK FLOW (cms)= 1.599 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 23.176
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.370

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 81.0
NASHVD (0270)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.30	
ID= 1 DT=15.0 min	U.H. Tp(hrs)= 0.87			

Unit Hyd Qpeak (cms)= 3.429

PEAK FLOW (cms)= 2.706 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 28.005
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.447

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 75.0
NASHVD (0268)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.30	
ID= 1 DT=15.0 min	U.H. Tp(hrs)= 0.69			

Unit Hyd Qpeak (cms)= 3.807

PEAK FLOW (cms)= 2.333 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 22.965
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.366

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

NASHVD (0264)	Area	(ha)= 353.96	Curve Number	(CN)= 69.0
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.30	
	U.H. Tp(hrs)= 1.30			

Unit Hyd Qpeak (cms)= 3.313

PEAK FLOW (cms)= 1.927 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 19.223
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.307

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)= 508.09	Curve Number	(CN)= 64.0
NASHVD (0266)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.30	
ID= 1 DT=15.0 min	U.H. Tp(hrs)= 1.63			

Unit Hyd Qpeak (cms)= 3.786

PEAK FLOW (cms)= 1.975 (i)
 TIME TO PEAK (hrs)= 9.250
 RUNOFF VOLUME (mm)= 16.500
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.263

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8380)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID= 1 (0264):	353.96	1.927	8.25	19.22
+ ID= 2 (0266):	508.09	1.975	9.25	16.50
ID = 3 (8380):	862.05	3.882	8.50	17.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6268)	Routing time step (min)= 15.00
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<----- DATA FOR SECTION (2681.0) ----->				
Distance	Elevation	Manning		
0.00	274.03	0.0500		
10.55	272.73	0.0500		
21.10	271.28	0.0500		
50.12	266.83	0.0500		
87.04	260.36	0.0500		
92.32	260.06	0.0500		
94.95	259.93	0.0500		
101.00	256.87	0.0500		
103.00	256.58	0.0500 / 0.0350	Main Channel	
104.00	256.32	0.0350	Main Channel	
106.00	256.05	0.0350	Main Channel	
108.00	256.25	0.0350	Main Channel	
110.78	256.65	0.0350 / 0.0500	Main Channel	
113.42	256.81	0.0500		
116.06	257.00	0.0500		
155.62	260.24	0.0500		
187.27	263.35	0.0500		
211.01	267.22	0.0500		
224.20	269.60	0.0500		
261.13	272.68	0.0500		

<----- TRAVEL TIME TABLE ----->						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.53	256.58	.697E+04	2.1	0.97	55.64	
1.38	257.43	.477E+05	25.9	1.76	30.66	
2.22	258.27	.123E+06	84.3	2.22	24.26	
3.07	259.12	.231E+06	185.4	2.60	20.72	
3.92	259.97	.371E+06	335.8	2.92	18.42	
4.77	260.82	.560E+06	547.6	3.15	17.06	
5.61	261.66	.790E+06	845.6	3.46	15.57	
6.46	262.51	1.06E+07	1225.0	3.75	14.37	
7.31	263.36	1.36E+07	1693.3	4.02	13.37	
8.16	264.21	1.69E+07	2283.2	4.35	12.36	
9.00	265.05	2.06E+07	2969.1	4.66	11.54	
9.85	265.90	2.43E+07	3754.4	4.96	10.86	
10.70	266.75	2.86E+07	4643.0	5.24	10.28	
11.55	267.60	3.31E+07	5636.1	5.50	9.78	
12.39	268.44	3.78E+07	6746.2	5.76	9.34	
13.24	269.29	4.28E+07	7972.2	6.01	8.96	
14.09	270.14	4.82E+07	9218.9	6.18	8.71	
14.94	270.99	5.39E+07	10550.5	6.32	8.52	
15.78	271.83	6.01E+07	12024.7	6.46	8.33	


```

<----- hydrograph ----> <-pipe / channel->
AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
(ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
INFLOW : ID= 2 (8380) 862.05   3.88   8.50 17.62   0.59   1.00
OUTFLOW: ID= 1 (6268) 862.05   3.78  10.00 17.62   0.59   1.00

```

```

ADD HYD (8382)
1 + 2 = 3
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0268): 215.76  2.333  7.00  22.96
+ ID2= 2 (6268): 862.05  3.775  10.00  17.62
=====
ID = 3 (8382): 1077.81  5.511  8.75  18.69

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6270)
IR= 2--> OUT= 1
Routing time step (min)= 15.00

```

<----- DATA FOR SECTION (2701.0) ----->

Distance	Elevation	Manning	
0.00	245.98	0.0500	
8.14	245.66	0.0500	
16.28	245.16	0.0500	
20.35	244.84	0.0500	
38.66	242.98	0.0500	
48.83	240.65	0.0500	
63.07	235.91	0.0500	
65.11	235.18	0.0500	
68.43	234.34	0.0500 / 0.0300	Main Channel
68.68	233.95	0.0300	Main Channel
69.18	233.89	0.0300	Main Channel
69.68	233.95	0.0300	Main Channel
71.21	234.48	0.0300 / 0.0500	Main Channel
81.38	236.44	0.0500	
95.63	236.66	0.0500	
120.04	237.00	0.0500	
148.53	241.77	0.0500	
158.70	242.34	0.0500	
187.18	244.03	0.0500	
201.42	244.36	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELRV (m)	VOLUME (cu.m)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.45	234.34	21.7E+04	0.7	0.99	52.83
0.98	234.87	966E+04	4.6	1.49	35.20
1.50	235.39	250E+05	13.7	1.73	30.41
2.03	235.92	475E+05	29.5	1.95	26.87
2.56	236.45	772E+05	52.5	2.14	24.50
3.09	236.98	1142E+06	82.4	1.83	28.73
3.61	237.50	246E+06	150.1	2.03	25.91
4.14	238.03	357E+06	262.0	2.31	22.73
4.67	238.56	477E+06	392.1	2.59	20.27
5.20	239.09	604E+06	547.8	2.86	18.38
5.72	239.61	739E+06	725.2	3.11	16.90
6.25	240.14	882E+06	936.5	3.34	15.70
6.78	240.67	103E+07	1169.7	3.57	14.72
7.31	241.20	119E+07	1425.9	3.77	13.94
7.83	241.72	136E+07	1710.4	3.96	13.26
8.36	242.25	154E+07	1967.9	4.02	13.07
8.89	242.78	174E+07	2264.9	4.09	12.83
9.42	243.31	196E+07	2589.3	4.15	12.65
9.94	243.83	221E+07	2953.5	4.22	12.46

```

<----- hydrograph ----> <-pipe / channel->
AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
(ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
INFLOW : ID= 2 (8382) 1077.81  5.51  8.75 18.69   1.03  1.51
OUTFLOW: ID= 1 (6270) 1077.81  5.44  9.50 18.69   1.03  1.51

```

```

ADD HYD (8384)
1 + 2 = 3
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0270): 243.61  2.706  7.25  28.01
+ ID2= 2 (6270): 1077.81  5.436  9.50  18.69
=====
ID = 3 (8384): 1321.42  7.795  8.75  20.41

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB (0262) | Area (ha)= 341.31 Curve Number (CN)= 82.0
NASHVD (0262) |

```

```

ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
                   | U.H. Tp(hrs)= 1.01

```

Unit Hyd Qpeak (cms)= 4.094

```

PEAK FLOW (cms)= 3.472 (1)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 29.023
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.463

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB (0260) | Area (ha)= 476.24 Curve Number (CN)= 82.0
NASHVD (0260) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.33

```

Unit Hyd Qpeak (cms)= 4.360

```

PEAK FLOW (cms)= 3.926 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 29.117
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.464

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB (0258) | Area (ha)= 181.99 Curve Number (CN)= 79.0
NASHVD (0258) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.18

```

Unit Hyd Qpeak (cms)= 1.881

```

PEAK FLOW (cms)= 1.492 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 26.347
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.420

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB (0252) | Area (ha)= 319.99 Curve Number (CN)= 73.0
NASHVD (0252) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.04

```

Unit Hyd Qpeak (cms)= 3.761

```

PEAK FLOW (cms)= 2.368 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 21.720
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.346

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB (0256) | Area (ha)= 145.79 Curve Number (CN)= 67.0
NASHVD (0256) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.04

```

Unit Hyd Qpeak (cms)= 1.707

```

PEAK FLOW (cms)= 0.887 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 18.019
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.287

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB (0254) | Area (ha)= 403.00 Curve Number (CN)= 55.0
NASHVD (0254) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.22

```

Unit Hyd Qpeak (cms)= 4.028

```

PEAK FLOW (cms)= 1.477 (1)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 12.430
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.198

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6256) | Routing time step (min)'= 15.00
IN= 2--> OUT= 1 |

DATA FOR SECTION (2561.0) ----->

Distance	Elevation	Manning
0.00	276.07	0.0400
11.68	273.71	0.0400
23.36	271.35	0.0400
29.19	270.30	0.0400
35.03	269.44	0.0400
55.47	267.90	0.0400
78.82	266.24	0.0400
90.50	265.63	0.0400
102.18	265.40	0.0400
105.10	264.95	0.0400 / 0.0350
108.02	264.39	0.0350
110.94	264.72	0.0350
113.86	265.19	0.0350 / 0.0400
116.78	265.49	0.0400
143.05	268.24	0.0400
172.25	270.53	0.0400
207.28	271.95	0.0400
233.55	273.72	0.0400
256.91	274.98	0.0400
289.02	275.97	0.0400

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.56	264.95	723E+04	1.9	0.90	62.51
1.14	265.53	307E+05	13.5	1.49	37.92
1.72	266.11	951E+05	48.6	1.73	32.62
2.30	266.69	191E+06	117.8	2.09	27.03
2.88	267.27	314E+06	225.2	2.43	23.24
3.46	267.85	464E+06	375.5	2.74	20.59
4.04	268.43	641E+06	573.2	3.03	18.62
4.62	269.01	846E+06	821.9	3.29	17.15
5.20	269.59	108E+07	1135.0	3.56	15.87
5.78	270.17	134E+07	1525.1	3.86	14.64
6.36	270.75	162E+07	1971.1	4.12	13.71
6.94	271.33	194E+07	2474.0	4.33	13.04
7.52	271.91	228E+07	3065.0	4.55	12.42
8.10	272.49	266E+07	3773.4	4.80	11.75
8.68	273.07	306E+07	4566.3	5.05	11.17
9.26	273.65	348E+07	5443.7	5.30	10.66
9.84	274.23	393E+07	6389.6	5.51	10.25
10.42	274.81	440E+07	7426.2	5.72	9.88
11.00	275.39	490E+07	8490.1	5.86	9.63

AREA hydrograph ----->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0254)	403.00	1.48	8.25	12.43	0.43
OUTFLOW : ID= 1 (6256)	403.00	1.39	10.00	12.43	0.40
				0.40	0.90

ADD HYD (8370) |
1 * 2 = 3 |

ID#	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID# 1 (0256)	145.79	0.887	8.00	18.02
+ ID# 2 (6256)	403.00	1.392	10.00	12.43

ID = 3 (8370)	548.79	2.197	9.25	13.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB | Area (ha)= 192.88 | Curve Number (CN)= 70.0
NASHYD (0250) | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.22

Unit Hyd Qpeak (cms) = 1.930

PEAK FLOW (cms) = 1.144 (i)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 19.816
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.316

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB | Area (ha)= 759.61 | Curve Number (CN)= 55.0
NASHYD (0246) |

ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
U.H. Tp(hrs)= 1.61

Unit Hyd Qpeak (cms) = 5.121

PEAK FLOW (cms) = 2.071 (i)
TIME TO PEAK (hrs) = 10.000
RUNOFF VOLUME (mm) = 12.474
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.199

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB | Area (ha)= 146.04 | Curve Number (CN)= 64.0
NASHYD (0248) | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.78

Unit Hyd Qpeak (cms) = 2.271

PEAK FLOW (cms) = 0.994 (i)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 16.345
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.261

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8364) |
1 * 2 = 3 |

ID#	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID# 1 (0246)	759.61	2.071	10.00	12.47
+ ID# 2 (0248)	146.04	0.994	7.25	16.34

ID = 3 (8364)	905.65	2.933	8.25	13.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6250) | Routing time step (min)'= 15.00
IN= 2--> OUT= 1 |

DATA FOR SECTION (2501.0) ----->

Distance	Elevation	Manning
0.00	269.96	0.0500
8.56	268.55	0.0500
17.13	266.91	0.0500
21.41	266.13	0.0500
40.68	263.15	0.0500
62.09	260.75	0.0500
85.64	258.02	0.0500
88.20	257.69	0.0500
93.20	257.05	0.0500 / 0.0350
93.45	256.88	0.0350
94.20	256.56	0.0350
94.95	256.83	0.0350
95.20	257.08	0.0350 / 0.0500
100.62	257.45	0.0500
115.61	258.57	0.0500
139.16	260.43	0.0500
152.01	261.95	0.0500
171.27	264.63	0.0500
188.40	267.90	0.0500
211.95	274.18	0.0500

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.49	257.05	205E+04	0.4	0.68	89.65
1.17	257.73	248E+05	5.7	0.85	72.16
1.85	258.41	841E+05	25.2	1.10	55.60
2.53	259.09	180E+06	66.5	1.36	45.14
3.21	259.77	312E+06	135.7	1.60	38.36
3.89	260.45	461E+06	230.3	1.82	33.61
4.57	261.13	681E+06	383.5	2.07	29.60
5.25	261.81	911E+06	568.9	2.29	26.70
5.93	262.49	117E+07	800.5	2.51	24.36
6.61	263.17	146E+07	1079.0	2.72	22.49
7.28	263.84	177E+07	1413.4	2.94	20.85
7.96	264.52	210E+07	1797.0	3.14	19.50
8.64	265.20	246E+07	2241.6	3.35	18.28
9.32	265.88	283E+07	2739.9	3.55	17.25
10.00	266.56	323E+07	3295.7	3.75	16.34
10.68	267.24	364E+07	3908.9	3.94	15.54
11.36	267.92	408E+07	4578.5	4.12	14.84
12.04	268.60	453E+07	5320.1	4.32	14.18
12.72	269.28	499E+07	6109.1	4.50	13.61

AREA hydrograph ----->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8364)	905.65	2.93	8.25	13.10	0.81	0.75
OUTFLOW: ID= 1 (6250)	905.65	2.72	10.50	13.10	0.79	0.75

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8372)	1967.31	8.08	9.25	15.39	0.65	0.86
OUTFLOW: ID= 1 (6258)	1967.31	7.36	11.25	15.39	0.63	0.87

```

ADD HYD (8366)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0250): 192.88 1.144 8.00 19.82
+ ID2= 2 (6250): 905.65 2.724 10.50 13.10
=====
ID = 3 (8366): 1098.53 3.765 10.00 14.28

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8374)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0258): 181.99 1.492 8.00 26.35
+ ID2= 2 (6258): 1967.31 7.363 11.25 15.39
=====
ID = 3 (8374): 2149.30 8.558 11.00 16.31

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8368)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8366): 1098.53 3.765 10.00 14.28
+ ID2= 2 (8370): 548.79 2.197 9.25 13.91
=====
ID = 3 (8368): 1647.32 5.937 9.75 14.16

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8376)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0260): 476.24 3.926 8.00 29.12
+ ID2= 2 (8374): 2149.30 8.558 11.00 16.31
=====
ID = 3 (8376): 2625.54 12.057 10.25 18.64

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8372)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0252): 319.99 2.368 8.00 21.72
+ ID2= 2 (8368): 1647.32 5.937 9.75 14.16
=====
ID = 3 (8372): 1967.31 8.081 9.25 15.39

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8378)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0262): 341.31 3.472 7.75 29.02
+ ID2= 2 (8376): 2625.54 12.057 10.25 18.64
=====
ID = 3 (8378): 2966.85 14.956 10.00 19.83

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6258)
IN= 2--> OUT= 1
Routing time step (min)= 15.00

```

```

<----- DATA FOR SECTION (2581.0) ----->
Distance Elevation Manning
0.00 252.88 0.0500
15.47 251.89 0.0500
46.41 248.45 0.0500
73.48 245.81 0.0500
88.95 243.75 0.0500
112.15 242.00 0.0500
135.35 240.23 0.0500
162.42 239.76 0.0500
170.97 239.52 0.0500 / 0.0350 Main Channel
171.58 239.03 0.0350 Main Channel
174.02 239.03 0.0350 Main Channel
176.46 239.03 0.0350 Main Channel
177.07 239.52 0.0350 / 0.0500 Main Channel
185.63 239.67 0.0500
208.83 239.87 0.0500
239.77 240.14 0.0500
274.57 244.93 0.0500
336.45 249.51 0.0500
363.52 249.77 0.0500
382.85 251.78 0.0500

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.49 239.52 135E+05 2.5 0.92 91.32
1.14 240.17 175E+06 25.6 0.73 114.22
1.78 240.81 532E+06 123.5 1.17 71.85
2.43 241.46 932E+06 285.3 1.54 54.48
3.07 242.10 138E+07 506.5 1.85 45.25
3.72 242.75 186E+07 786.8 2.12 39.42
4.36 243.39 239E+07 1127.3 2.37 35.33
5.01 244.04 296E+07 1535.8 2.61 32.12
5.65 244.68 356E+07 2014.3 2.84 29.48
6.30 245.33 420E+07 2535.9 3.03 27.60
6.94 245.97 488E+07 3108.4 3.20 26.16
7.59 246.62 561E+07 3740.6 3.35 24.98
8.23 247.26 638E+07 4446.7 3.50 23.93
8.88 247.91 721E+07 5229.7 3.64 22.98
9.52 248.55 809E+07 6094.3 3.78 22.12
10.17 249.20 901E+07 7050.4 3.93 21.31
10.81 249.84 1008E+08 7773.0 3.90 21.46
11.46 250.49 1111E+08 8998.8 4.07 20.56
12.10 251.13 1228E+08 10316.2 4.24 19.75

```

```

ADD HYD (8362)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8378): 2966.85 14.956 10.00 19.83
+ ID2= 2 (8384): 1321.42 7.795 8.75 20.41
=====
ID = 3 (8362): 4288.27 22.550 9.50 20.01

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6272)
IN= 2--> OUT= 1
Routing time step (min)= 15.00

```

```

<----- DATA FOR SECTION (2721.0) ----->
Distance Elevation Manning
0.00 231.01 0.0450
23.01 223.65 0.0450
34.51 222.46 0.0450
51.77 222.11 0.0450
69.02 221.87 0.0450
161.06 221.92 0.0450
166.81 221.91 0.0450
172.56 221.89 0.0450
180.57 221.40 0.0450 / 0.0300 Main Channel
181.57 220.60 0.0300 Main Channel
184.07 220.16 0.0300 Main Channel
195.57 221.85 0.0300 / 0.0450 Main Channel
201.32 221.82 0.0450
207.07 221.77 0.0450
212.83 221.72 0.0450
253.09 222.52 0.0450
408.40 222.65 0.0450
460.17 223.20 0.0450
517.69 224.84 0.0450
569.46 232.57 0.0450

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.41 220.57 189E+04 0.5 0.49 60.18
0.83 220.99 697E+04 3.2 0.82 35.79
1.24 221.40 145E+05 8.8 1.07 27.45
1.84 222.00 643E+05 29.3 0.61 36.61

```

2.44	222.60	.282E+06	132.9	0.83	35.41
3.04	223.20	.707E+06	413.7	1.04	28.50
3.64	223.80	.118E+07	876.8	1.31	22.46
4.24	224.40	.168E+07	1494.8	1.57	18.75
4.84	225.00	.223E+07	2264.1	1.82	16.23
5.44	225.60	.274E+07	3196.3	2.06	14.29
6.04	226.20	.328E+07	4258.6	2.30	12.84
6.65	226.81	.383E+07	5444.7	2.52	11.72
7.25	227.41	.438E+07	6749.7	2.73	10.82
7.85	228.01	.494E+07	8169.4	2.93	10.08
8.45	228.61	.551E+07	9700.7	3.12	9.46
9.05	229.21	.608E+07	11340.8	3.30	8.94
9.65	229.81	.666E+07	13087.4	3.48	8.48
10.25	230.41	.724E+07	14938.5	3.65	8.08
10.85	231.01	.783E+07	16892.4	3.82	7.73

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8352) 4288.27 22.55 9.50 20.01 1.64 0.88
OUTFLOW : ID= 1 (6272) 4288.27 22.22 10.25 20.01 1.63 0.88

```

```

ADD HYD (8356)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0272): 157.38 1.599 7.50 23.18
+ ID2= 2 (6272): 4288.27 22.221 10.25 20.01
=====
ID = 3 (8356): 4445.65 23.386 10.00 20.12

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8352)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8354): 1268.73 13.898 8.00 27.05
+ ID2= 2 (8356): 4445.65 23.386 10.00 20.12
=====
ID = 3 (8352): 5714.38 35.368 9.00 21.66

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6304)
IN= 2--> OUT= 1
Routing time step (min)= 15.00

```

```

<----- DATA FOR SECTION (3041.0) ----->
Distance Elevation Manning
0.00 232.08 0.0500
19.00 231.87 0.0500
38.00 231.33 0.0500
66.51 230.44 0.0500
104.51 228.25 0.0500
118.76 225.17 0.0500
128.26 219.86 0.0500
175.77 219.17 0.0500
185.27 218.90 0.0500 /0.0300 Main Channel
185.52 218.65 0.0300 Main Channel
190.02 218.37 0.0300 Main Channel
194.52 218.60 0.0300 Main Channel
194.77 218.85 0.0300 /0.0500 Main Channel
204.27 219.60 0.0500
299.28 220.91 0.0500
327.78 222.36 0.0500
375.28 225.71 0.0500
403.79 229.37 0.0500
432.29 230.43 0.0500
470.29 232.00 0.0500

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.48 218.85 121E+05 1.2 0.36 173.94
1.17 219.54 839E+05 9.8 0.44 142.60
1.86 220.23 323E+06 41.0 0.47 131.43
2.56 220.93 708E+06 109.5 0.58 107.67
3.25 221.62 118E+07 224.3 0.71 87.36
3.94 222.31 168E+07 376.5 0.84 74.48
4.63 223.00 222E+07 567.9 0.96 65.26
5.32 223.69 279E+07 794.7 1.07 58.58
6.02 224.39 339E+07 1056.2 1.17 53.52
6.71 225.08 402E+07 1352.4 1.26 49.52
7.40 225.77 468E+07 1677.2 1.34 46.46
8.09 226.46 536E+07 2050.0 1.43 43.58
8.79 227.16 607E+07 2457.9 1.52 41.14
9.48 227.85 680E+07 2901.0 1.60 39.05
10.17 228.54 755E+07 3352.7 1.66 37.53

```

10.86	229.23	.834E+07	3812.5	1.71	36.48
11.55	229.92	.919E+07	4241.8	1.73	36.13
12.25	230.62	.101E+08	4697.0	1.74	35.93
12.94	231.31	.111E+08	5186.3	1.74	35.83

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8352) 5714.38 35.37 9.00 21.66 1.74 0.47
OUTFLOW : ID= 1 (6304) 5714.38 29.22 11.50 21.66 1.60 0.46

```

```

ADD HYD (8350)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0304): 292.37 0.987 10.50 16.06
+ ID2= 2 (6304): 5714.38 29.219 11.50 21.66
=====
ID = 3 (8350): 6006.75 30.190 11.50 21.39

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8346)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8348): 3768.46 21.986 9.75 19.69
+ ID2= 2 (8350): 6006.75 30.190 11.50 21.39
=====
ID = 3 (8346): 9775.21 50.996 10.75 20.73

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8344)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0306): 283.97 0.331 13.75 11.39
+ ID2= 2 (8346): 9775.21 50.996 10.75 20.73
=====
ID = 3 (8344): 10059.18 51.294 10.75 20.47

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

RESERVOIR (5509)
IN= 2--> OUT= 1
DT= 15.0 min
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
0.0000 0.0000 | ***** 370.0451
41.0590 56.7403 | ***** 863.4386
48.1390 86.3439 | *****
56.6340 ***** | *****
67.9600 ***** | 0.0000 0.0000

```

```

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (8344) ***** 51.294 10.75 20.47
OUTFLOW : ID= 1 (5509) ***** 39.479 14.50 20.47

```

PEAK FLOW REDUCTION [Qout/Qin](%)= 76.97
TIME SHIFT OF PEAK FLOW (min)=225.00
MAXIMUM STORAGE USED (ha.m.)= 54.5767

```

CALIB NASHYD (0332)
Area (ha)= 393.44 Curve Number (CN)= 75.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 2.32

```

Unit Hyd Qpeak (cms) = 2.894
PEAK FLOW (cms) = 2.232 (1)
TIME TO PEAK (hrs) = 9.750
RUNOFF VOLUME (mm) = 23.317
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.372

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB NASHYD (0330)
Area (ha)= 468.30 Curve Number (CN)= 80.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.09

```

Unit Hyd Qpeak (cms)= 7.335
 PEAK FLOW (cms)= 5.661 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 27.223
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.434

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6332) | Routing time step (min)= 15.00
 IN= 2--> OUT= 1 |

Distance	Elevation	Manning
0.00	234.00	0.0380
25.85	227.20	0.0380
56.94	226.44	0.0380
168.03	227.38	0.0380
219.73	225.62	0.0380
342.52	221.57	0.0380
368.37	221.42	0.0380
374.83	221.23	0.0380
379.79	220.98	0.0380 / 0.0300
380.29	220.47	0.0300
381.29	220.47	0.0300
382.79	220.47	0.0300
383.29	220.98	0.0300 / 0.0380
384.22	221.22	0.0380
400.68	221.33	0.0380
407.14	221.44	0.0380
491.16	225.70	0.0380
569.71	227.55	0.0380
607.49	230.14	0.0380
639.80	234.08	0.0380

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.51	220.98	.554E+04	0.9	0.56	107.27
1.20	221.67	.885E+05	12.1	0.49	122.32
1.88	222.35	.309E+06	64.7	0.76	79.70
2.57	223.04	.615E+06	165.4	0.97	61.99
3.25	223.72	.101E+07	323.1	1.16	52.06
3.94	224.41	.148E+07	542.9	1.33	45.51
4.62	225.09	.204E+07	835.4	1.48	40.77
5.31	225.78	.269E+07	1202.8	1.62	37.27
5.99	226.46	.344E+07	1618.0	1.70	35.46
6.68	227.15	.447E+07	1932.0	1.56	38.56
7.36	227.83	.580E+07	2763.9	1.72	34.96
8.05	228.52	.718E+07	3861.9	1.95	30.97
8.73	229.20	.859E+07	5115.4	2.16	27.98
9.42	229.89	.100E+08	6519.2	2.35	25.64
10.10	230.57	.115E+08	8092.3	2.55	23.69
10.79	231.26	.130E+08	9821.5	2.73	22.05
11.47	231.94	.145E+08	11690.0	2.92	20.68
12.16	232.63	.160E+08	13694.9	3.09	19.52
12.84	233.31	.176E+08	15833.9	3.26	18.52

<---- hydrograph ----> <-pipe / channel-->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW: ID= 2 (0330)	468.30	5.66	7.50	27.22	0.80
OUTFLOW: ID= 1 (6332)	468.30	4.35	9.75	27.22	0.72

ADD HYD (8322)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID= 1 (0332):	393.44	2.232	9.75	23.32
+ ID= 2 (6332):	468.30	4.351	9.75	27.22
ID = 3 (8322):	861.74	6.582	9.75	25.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	(ha)	Curve Number	(CN)=
NASHYD (0328)	492.92	492.92	77.0	77.0	77.0
ID= 1 DT=15.0 min	5.00	5.00	# of Linear Res.(N)=	1.50	1.50
U.H. Tp(hrs)=	1.91				

Unit Hyd Qpeak (cms)= 4.411

PEAK FLOW (cms)= 3.474 (i)
 TIME TO PEAK (hrs)= 8.750
 RUNOFF VOLUME (mm)= 24.828
 TOTAL RAINFALL (mm)= 62.700

RUNOFF COEFFICIENT = 0.396

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	(ha)	Curve Number	(CN)=
NASHYD (0326)	678.91	678.91	80.0	80.0	80.0
ID= 1 DT=15.0 min	5.00	5.00	# of Linear Res.(N)=	1.50	1.50
U.H. Tp(hrs)=	1.95				

Unit Hyd Qpeak (cms)= 5.941
 PEAK FLOW (cms)= 5.199 (i)
 TIME TO PEAK (hrs)= 8.750
 RUNOFF VOLUME (mm)= 27.366
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.436

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	(ha)	Curve Number	(CN)=
NASHYD (0324)	615.64	615.64	79.0	79.0	79.0
ID= 1 DT=15.0 min	5.00	5.00	# of Linear Res.(N)=	1.50	1.50
U.H. Tp(hrs)=	1.90				

Unit Hyd Qpeak (cms)= 5.544
 PEAK FLOW (cms)= 4.663 (i)
 TIME TO PEAK (hrs)= 8.750
 RUNOFF VOLUME (mm)= 26.483
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.422

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	(ha)	Curve Number	(CN)=
NASHYD (0320)	278.74	278.74	81.0	81.0	81.0
ID= 1 DT=15.0 min	5.00	5.00	# of Linear Res.(N)=	1.50	1.50
U.H. Tp(hrs)=	1.36				

Unit Hyd Qpeak (cms)= 3.499
 PEAK FLOW (cms)= 2.949 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 28.204
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.450

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	(ha)	Curve Number	(CN)=
NASHYD (3182)	457.40	457.40	78.0	78.0	78.0
ID= 1 DT=15.0 min	5.00	5.00	# of Linear Res.(N)=	1.50	1.50
U.H. Tp(hrs)=	1.49				

Unit Hyd Qpeak (cms)= 5.259
 PEAK FLOW (cms)= 4.075 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 25.594
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.408

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	(ha)	Dir. Conn.(%)
STANDHYD (3181)	26.00	26.00	81.00	81.00
ID= 1 DT=15.0 min				

Surface Area	(ha)	IMPERVIOUS	PERVIOUS (i)
Dep. Storage	21.06	2.00	5.00
Average Slope	(%)	0.50	0.50
Length	(m)	416.33	40.00
Mannings n		0.013	0.250

Max. Eff. Inten.(mm/hr)=	82.76	23.72	
Storage Coeff. (min)=	15.00	30.00	
Unit Hyd. Tpeak (min)=	7.98 (ii)	27.00 (ii)	
Unit Hyd. peak (cms)=	0.10	0.04	
PEAK FLOW (cms)=	4.34	0.23	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	4.484 (iii)
RUNOFF VOLUME (mm)=	60.70	24.93	53.90
TOTAL RAINFALL (mm)=	62.70	62.70	62.70

RUNOFF COEFFICIENT = 0.97 0.40 0.86

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 77.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: RESERVOIR (5318), IN= 2--> OUT= 1, DT= 15.0 min, OUTFLOW (cms), STORAGE (ha.m.), OUTFLOW (cms), STORAGE (ha.m.).

INFLOW : ID= 2 (3181) 26.000 4.484 6.00 53.90
OUTFLOW: ID= 1 (5318) 26.000 2.420 6.25 53.82

PEAK FLOW REDUCTION [Qout/Qin](%)= 53.97
TIME SHIFT OF PEAK FLOW (min)= 15.00
MAXIMUM STORAGE USED (ha.m.)= 0.7068

Table with columns: ADD HYD (8334), 1 + 2 = 3, AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm).

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6320) Routing time step (min)'= 15.00
IN= 2--> OUT= 1

Table with columns: DISTANCE, ELEVATION, MANNING, MAIN CHANNEL.

Table with columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV.TIME.

hydrograph table with columns: AREA, QPEAK, TPEAK, R.V., MAX DEPTH, MAX VEL.

Table with columns: ADD HYD (8332), 1 + 2 = 3, AREA, QPEAK, TPEAK, R.V.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0314) Area (ha)= 165.20 Curve Number (CN)= 78.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.71

Unit Hyd Qpeak (cms)= 3.978

PEAK FLOW (cms)= 2.616 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 25.303
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.404

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6316) Routing time step (min)'= 15.00
IN= 2--> OUT= 1

Table with columns: DISTANCE, ELEVATION, MANNING, MAIN CHANNEL.

Table with columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV.TIME.

hydrograph table with columns: AREA, QPEAK, TPEAK, R.V., MAX DEPTH, MAX VEL.

```

-----
| CALIB |
| NASHVD (0316) | Area (ha)= 232.34 Curve Number (CN)= 82.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.01

```

```

Unit Hyd Qpeak (cms) = 3.923

PEAK FLOW (cms) = 3.203 (1)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 29.050
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.463

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8338) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (0316): | 232.34 3.203 7.25 29.05
| + ID2= 2 (6316): | 165.20 2.347 7.50 25.30
|-----|
| ID = 3 (8338): | 397.54 5.531 7.50 27.49

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHVD (0312) | Area (ha)= 359.44 Curve Number (CN)= 80.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.36

```

```

Unit Hyd Qpeak (cms) = 4.529

PEAK FLOW (cms) = 3.684 (1)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 27.291
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.435

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHVD (0308) | Area (ha)= 529.30 Curve Number (CN)= 62.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.62

```

```

Unit Hyd Qpeak (cms) = 5.575

PEAK FLOW (cms) = 2.615 (1)
TIME TO PEAK (hrs) = 8.500
RUNOFF VOLUME (mm) = 15.525
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.248

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHVD (0310) | Area (ha)= 138.28 Curve Number (CN)= 65.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.76

```

```

Unit Hyd Qpeak (cms) = 3.102

PEAK FLOW (cms) = 1.342 (1)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 16.858
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.269

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8342) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (0308): | 529.30 2.615 8.50 15.53
| + ID2= 2 (0310): | 138.28 1.342 7.00 16.86
|-----|
| ID = 3 (8342): | 667.58 3.777 8.00 15.80

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6312) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00
|-----|

```

----- DATA FOR SECTION (3121.0) -----				
Distance	Elevation	Manning		
0.00	265.94	0.0360		
38.07	265.43	0.0360		
59.82	263.98	0.0360		
103.32	254.59	0.0360		
157.70	252.16	0.0360		
217.52	250.45	0.0360		
233.84	247.69	0.0360		
234.71	247.27	0.0360		
239.71	246.38	0.0360		
244.71	246.12	0.0360 /0.0330	Main Channel	
247.71	245.17	0.0330	Main Channel	
249.71	245.19	0.0330	Main Channel	
251.71	245.64	0.0330 /0.0360	Main Channel	
259.71	246.67	0.0360		
282.78	247.12	0.0360		
315.41	251.60	0.0360		
424.17	256.13	0.0360		
478.55	257.04	0.0360		
516.62	259.37	0.0360		
538.37	266.00	0.0360		

```

----- TRAVEL TIME TABLE -----

```

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.47	245.64	.111E+04	3.1	1.78	6.07
1.54	246.71	.103E+05	53.9	3.39	3.19
2.61	247.78	.414E+05	281.3	4.40	2.45
3.68	248.85	.839E+05	749.1	5.79	1.87
4.74	249.91	.135E+06	1463.2	6.97	1.55
5.81	250.98	.201E+06	2354.8	7.61	1.42
6.88	252.05	.295E+06	3533.0	7.76	1.39
7.95	253.12	.427E+06	5303.9	8.05	1.34
9.02	254.19	.594E+06	7856.5	8.58	1.26
10.09	255.26	.792E+06	11449.2	9.37	1.15
11.15	256.32	.101E+07	15862.4	10.15	1.06
12.22	257.39	.127E+07	21074.5	10.75	1.01
13.29	258.46	.155E+07	28184.1	11.78	0.92
14.36	259.53	.185E+07	36386.2	12.77	0.85
15.43	260.60	.215E+07	46110.6	13.89	0.78
16.50	261.67	.246E+07	56822.5	14.95	0.72
17.56	262.73	.278E+07	68498.1	15.97	0.68
18.63	263.80	.310E+07	81118.8	16.94	0.64
19.70	264.87	.344E+07	93603.2	17.66	0.61

```

----- hydrograph ----- <-pipe / channel->
|-----|
| AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL |
| (ha) (cms) (hrs) (mm) (m) (m/s) |
INFLOW : ID= 2 (8342) 667.58 3.78 8.00 15.80 0.49 1.79
OUTFLOW : ID= 1 (6312) 667.58 3.78 8.00 15.80 0.49 1.79

```

```

-----
| ADD HYD (8340) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (0312): | 359.44 3.684 8.00 27.29
| + ID2= 2 (6312): | 667.58 3.776 8.00 15.80
|-----|
| ID = 3 (8340): | 1027.02 7.460 8.00 19.82

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8336) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (8338): | 397.54 5.531 7.50 27.49
| + ID2= 2 (8340): | 1027.02 7.460 8.00 19.82
|-----|
| ID = 3 (8336): | 1424.56 12.900 7.75 21.96

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8330) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (8332): | 762.14 6.855 8.50 27.51
| + ID2= 2 (8336): | 1424.56 12.900 7.75 21.96
|-----|
| ID = 3 (8330): | 2186.70 19.539 8.00 23.90

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ID1= 1 (8322): 513.13 4.298 8.50 27.35
 + ID2= 2 (8328): 2802.34 21.348 9.50 24.46
 ID = 3 (8326): 3315.47 25.514 9.50 24.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6324)			
IN= 2--> OUTF= 1			
Routing time step (min)= 15.00			
----- DATA FOR SECTION (3241.0) -----			
Distance	Elevation	Manning	
0.00	234.38	0.0360	
33.57	232.48	0.0360	
67.14	230.14	0.0360	
83.93	228.80	0.0360	
134.29	227.62	0.0360	
209.82	225.10	0.0360	
218.21	224.86	0.0360	
226.61	224.47	0.0360	
234.00	223.86	0.0360 / 0.0300	Main Channel
234.10	223.66	0.0300	Main Channel
235.00	223.66	0.0300	Main Channel
235.90	223.66	0.0300	Main Channel
236.00	223.86	0.0300 / 0.0360	Main Channel
243.39	224.92	0.0360	
251.78	224.89	0.0360	
335.71	225.64	0.0360	
562.32	226.53	0.0360	
637.85	228.36	0.0360	
705.00	229.80	0.0360	
830.89	234.00	0.0360	

----- TRAVEL TIME TABLE -----						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.20	223.86	.145E+04	0.2	0.43	147.27	
0.73	224.39	.159E+05	2.8	0.68	93.11	
1.27	224.93	.561E+05	11.0	0.75	85.19	
1.80	225.46	.217E+06	45.6	0.80	79.20	
2.33	225.99	.566E+06	134.4	0.91	70.20	
2.87	226.53	.122E+07	338.1	1.06	59.97	
3.40	227.06	.205E+07	744.3	1.38	46.16	
3.94	227.60	.298E+07	1295.9	1.66	38.38	
4.47	228.13	.399E+07	1979.9	1.90	33.59	
5.00	228.66	.509E+07	2807.5	2.11	30.21	
5.54	229.20	.627E+07	3815.1	2.32	27.41	
6.07	229.73	.753E+07	4984.1	2.53	25.17	
6.60	230.26	.884E+07	6346.3	2.74	23.20	
7.14	230.80	.102E+08	7861.2	2.95	21.61	
7.67	231.33	.118E+08	9526.4	3.14	20.29	
8.21	231.87	.131E+08	11342.2	3.32	19.18	
8.74	232.40	.146E+08	13309.6	3.49	18.22	
9.27	232.93	.161E+08	15414.0	3.66	17.41	
9.81	233.47	.177E+08	17671.0	3.81	16.70	

----- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8330) 2186.70 19.54 8.00 23.90 1.40 0.76
 OUTFLOW: ID= 1 (6324) 2186.70 16.78 9.75 23.90 1.35 0.76

ADD HYD (8328)				
1 + 2 = 3				
AREA	QPEAK	TPEAK	R.V.	
(ha)	(cms)	(hrs)	(mm)	(mm)
ID1= 1 (0324):	615.64	4.663	8.75	26.48
+ ID2= 2 (6324):	2186.70	16.778	9.75	23.90
=====				
ID = 3 (8328):	2802.34	21.348	9.50	24.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0322)				
NASHYD (0322)				
Area	(ha)	513.13	Curve Number (CN)=	80.0
IN= 1 DT=15.0 min	Is	(mm)	=	5.00
U.H. Tp(hrs)= 1.75				

Unit Hyd Qpeak (cms)= 5.020

PEAK FLOW (cms)= 4.298 (l)
 TIME TO PEAK (hrs)= 8.500
 RUNOFF VOLUME (mm)= 27.347
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.436

(l) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8326)				
1 + 2 = 3				
AREA	QPEAK	TPEAK	R.V.	
(ha)	(cms)	(hrs)	(mm)	(mm)

ADD HYD (8324)				
1 + 2 = 3				
AREA	QPEAK	TPEAK	R.V.	
(ha)	(cms)	(hrs)	(mm)	(mm)
ID1= 1 (0326):	678.91	5.199	8.75	27.37
+ ID2= 2 (8326):	3315.47	25.514	9.50	24.91
=====				
ID = 3 (8324):	3994.38	30.661	9.50	25.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6328)			
IN= 2--> OUTF= 1			
Routing time step (min)= 15.00			
----- DATA FOR SECTION (3281.0) -----			
Distance	Elevation	Manning	
0.00	228.00	0.0380	
18.58	224.97	0.0380	
78.98	223.52	0.0380	
125.44	223.28	0.0380	
171.90	221.71	0.0380	
213.72	219.65	0.0380	
218.36	219.40	0.0380	
223.01	219.19	0.0380	
225.95	219.14	0.0380 / 0.0320	Main Channel
226.45	218.14	0.0320	Main Channel
236.95	218.14	0.0320	Main Channel
245.85	218.14	0.0320	Main Channel
245.95	219.14	0.0320 / 0.0380	Main Channel
246.24	219.16	0.0380	
250.88	219.24	0.0380	
255.53	219.39	0.0380	
325.22	221.47	0.0380	
367.03	223.14	0.0380	
404.20	225.17	0.0380	
459.95	228.04	0.0380	

----- TRAVEL TIME TABLE -----						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.50	218.64	.373E+05	3.2	0.33	192.37	
1.00	219.14	.752E+05	10.1	0.51	124.71	
1.52	219.66	.149E+06	22.2	0.57	112.06	
2.04	220.18	.279E+06	43.4	0.59	107.06	
2.56	220.70	.464E+06	77.0	0.63	100.52	
3.08	221.22	.705E+06	125.7	0.68	93.56	
3.61	221.75	.100E+07	192.7	0.73	86.62	
4.13	222.27	.135E+07	278.1	0.79	80.92	
4.65	222.79	.176E+07	385.9	0.84	75.86	
5.17	223.31	.222E+07	514.9	0.89	71.82	
5.69	223.83	.280E+07	662.7	0.90	70.40	
6.21	224.35	.345E+07	867.7	0.96	66.35	
6.73	224.87	.417E+07	1107.0	1.01	62.81	
7.25	225.39	.494E+07	1400.0	1.08	58.80	
7.78	225.92	.573E+07	1732.2	1.15	55.17	
8.30	226.44	.655E+07	2099.9	1.22	52.02	
8.82	226.96	.740E+07	2502.8	1.29	49.29	
9.34	227.48	.828E+07	2940.9	1.36	46.90	
9.86	228.00	.918E+07	3414.3	1.42	44.79	

----- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8324) 3994.38 30.66 9.50 25.33 1.73 0.58
 OUTFLOW: ID= 1 (6328) 3994.38 26.24 11.25 25.33 1.62 0.57

ADD HYD (8320)				
1 + 2 = 3				
AREA	QPEAK	TPEAK	R.V.	
(ha)	(cms)	(hrs)	(mm)	(mm)
ID1= 1 (0328):	492.92	3.474	8.75	24.83
+ ID2= 2 (6328):	3994.38	26.237	11.25	25.33
=====				
ID = 3 (8320):	4487.30	29.749	11.00	25.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8316)				
----------------	--	--	--	--

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8320):	4487.30	29.349	11.00	25.27
+ ID2= 2 (8322):	861.74	6.582	9.75	25.44
ID = 3 (8318):	5349.04	35.607	10.75	25.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (5509):	10059.18	39.479	14.50	20.47
+ ID2= 2 (8318):	5349.04	35.607	10.75	25.30
ID = 3 (8316):	15408.22	70.146	12.75	22.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8314):	620.10	0.893	6.75	13.82
+ ID2= 2 (8316):	15408.22	70.146	12.75	22.15
ID = 3 (8312):	16028.32	70.826	12.75	21.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8310):	598.90	22.328	6.50	29.01
+ ID2= 2 (8312):	16028.32	70.826	12.75	21.82
ID = 3 (8308):	16627.22	71.998	12.25	22.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5510)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1				
DT= 15.0 min				
	0.0000	0.0000	*****	74.0090
	66.5450	18.5023	*****	*****
	98.5430	37.0045	*****	*****

INFLOW : ID= 2 (8308)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
*****		71.998	12.25	22.08
OUTFLOW: ID= 1 (5510)	*****	69.834	13.50	22.08

PEAK FLOW REDUCTION [Qout/qin](%)= 96.99
 TIME SHIFT OF PEAK FLOW (min)= 75.00
 MAXIMUM STORAGE USED (ha.m.)= 20.4048

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0336):	2785.00	2.580	22.25	21.25
+ ID2= 2 (5510):	16627.22	69.834	13.50	22.08
ID = 3 (8240):	19412.22	72.075	13.50	21.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (5508):	9524.23	32.116	12.25	13.36
+ ID2= 2 (8240):	19412.22	72.075	13.50	21.97
ID = 3 (8238):	28936.45	103.371	13.00	19.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0342):	1004.58	2.724	10.25	12.12
+ ID2= 2 (8238):	28936.45	103.371	13.00	19.13
ID = 3 (8236):	29941.03	105.769	13.00	18.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8232):	285.80	6.442	6.00	19.32
+ ID2= 2 (8236):	29941.03	105.769	13.00	18.90
ID = 3 (8234):	30226.83	106.051	12.75	18.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0344):	344.00	2.485	7.00	14.04
+ ID2= 2 (8234):	30226.83	106.051	12.75	18.90
ID = 3 (8230):	30570.83	106.769	12.75	18.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8226):	1952.06	8.283	8.75	11.68
+ ID2= 2 (8230):	30570.83	106.769	12.75	18.85
ID = 3 (8228):	32522.89	112.061	12.50	18.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0356):	1015.22	3.392	8.00	9.30
+ ID2= 2 (8228):	32522.89	112.061	12.50	18.42
ID = 3 (8190):	33538.11	114.186	12.25	18.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

3e280798-92ee-4282-809c-79ef5caed0add\73alf747
 Ptotal= 73.10 mm
 Comments: 25-Year 12-Hour SCS II Design Storm

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.83	3.25	2.92	6.25	13.16	9.25	2.56
0.50	1.83	3.50	2.92	6.50	13.16	9.50	2.56
0.75	1.83	3.75	2.92	6.75	5.85	9.75	2.56
1.00	1.83	4.00	2.92	7.00	5.85	10.00	2.56
1.25	1.83	4.25	4.39	7.25	4.39	10.25	1.46
1.50	1.83	4.50	4.39	7.50	4.39	10.50	1.46
1.75	1.83	4.75	5.85	7.75	4.39	10.75	1.46
2.00	1.83	5.00	5.85	8.00	4.39	11.00	1.46
2.25	2.19	5.25	8.77	8.25	2.56	11.25	1.46
2.50	2.19	5.50	8.77	8.50	2.56	11.50	1.46
2.75	2.19	5.75	35.09	8.75	2.56	11.75	1.46
3.00	2.19	6.00	36.49	9.00	2.56	12.00	1.46

CALIB (0356) Area (ha)=1015.22 Curve Number (CN)= 46.0
 NASHYD (0356) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.37

Unit Hyd Qpeak (cms)= 12.651
 PEAK FLOW (cms)= 4.610 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 12.581
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.172

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0354) Area (ha)= 262.68 Curve Number (CN)= 37.0
 NASHYD (0354) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.38

Unit Hyd Qpeak (cms)= 3.252
 PEAK FLOW (cms)= 0.864 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 9.206
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.126

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0352) Area (ha)= 381.43 Curve Number (CN)= 54.0
 NASHYD (0352) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.82

Unit Hyd Qpeak (cms)= 7.980
 PEAK FLOW (cms)= 3.316 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 16.078
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.220

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0346) Area (ha)= 350.93 Curve Number (CN)= 70.0
 NASHYD (0346) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 6.254
 PEAK FLOW (cms)= 4.425 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 25.923
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.355

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0350) Area (ha)= 366.84 Curve Number (CN)= 48.0
 NASHYD (0350) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 5.831

PEAK FLOW (cms)= 2.124 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 13.386
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.183

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0348) Area (ha)= 590.18 Curve Number (CN)= 48.0
 NASHYD (0348) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.91

Unit Hyd Qpeak (cms)= 5.267

PEAK FLOW (cms)= 2.211 (i)
 TIME TO PEAK (hrs)= 9.000
 RUNOFF VOLUME (mm)= 13.458
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.184

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6350) Routing time step (min)= 15.00
 IN= 2--> OUT= 1

----- DATA FOR SECTION (3501.0) -----

Distance	Elevation	Manning
0.00	287.00	0.0500
23.66	283.72	0.0500
70.98	280.94	0.0500
112.38	280.23	0.0500
171.52	276.80	0.0500
260.24	274.46	0.0500
266.16	274.26	0.0500
272.07	274.12	0.0500
276.49	274.12	0.0500 / 0.0300
276.99	273.82	0.0300
277.99	273.82	0.0300
279.49	273.82	0.0300
280.49	274.27	0.0300 / 0.0500
283.90	274.27	0.0500
289.81	274.57	0.0500
325.30	275.75	0.0500
396.28	278.98	0.0500
449.51	280.97	0.0500
496.83	283.90	0.0500
585.54	287.92	0.0500

----- TRAVEL TIME TABLE -----

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.30	274.12	273E+04	0.9	0.99	49.81
0.98	274.80	583E+05	20.8	1.05	46.71
1.66	275.48	203E+06	95.2	1.38	35.53
2.33	276.15	438E+06	256.0	1.73	28.50
3.01	276.83	753E+06	524.8	2.05	23.97
3.69	277.51	114E+07	935.8	2.43	20.26
4.37	278.19	157E+07	1472.0	2.76	17.82
5.05	278.87	205E+07	2140.7	3.06	16.06
5.72	279.54	261E+07	2938.6	3.33	14.79
6.40	280.22	321E+07	3887.6	3.57	13.77
7.08	280.90	390E+07	4795.4	3.63	13.56
7.76	281.58	468E+07	6196.2	3.91	12.58
8.43	282.25	550E+07	7803.9	4.19	11.74
9.11	282.93	637E+07	9601.9	4.45	11.05
9.79	283.61	728E+07	11594.8	4.70	10.46
10.47	284.29	823E+07	13845.3	4.96	9.91
11.15	284.97	922E+07	16293.4	5.21	9.43
11.82	285.64	103E+08	18952.0	5.45	9.02
12.50	286.32	113E+08	21824.3	5.69	8.65

----- hydrograph ----- <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0348) 590.18 2.21 9.00 13.46 0.34 0.99
 OUTFLOW : ID= 1 (6350) 590.18 2.16 10.25 13.46 0.34 0.99

ADD HYD (8220)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID= 1 (0350): 366.84 2.124 7.50 13.39
 + ID= 2 (6350): 590.18 2.158 10.25 13.46
 ID= 3 (8220): 957.02 3.943 8.75 13.43

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with 5 columns: ADD HYD (8222), AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID1=1 (0345), ID2=2 (8220), and ID=3 (8222).

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6352) Routing time step (min) = 15.00

DATA FOR SECTION (3521.0) table with columns: Distance, Elevation, Manning. Lists data points from 0.00 to 193.80.

TRAVEL TIME TABLE

TRAVEL TIME TABLE with columns: DEPTH (m), ELEV (m), VOLUME (cu.m.), FLOW RATE (cms), VELOCITY (m/s), TRAV.TIME (min). Lists data points from 0.60 to 18.16.

hydrograph table header

hydrograph table with columns: AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm), MAX DEPTH (m), MAX VEL (m/s). Includes INFLOW and OUTFLOW data.

Table with 5 columns: ADD HYD (8224), AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID1=1 (0352), ID2=2 (6352), and ID=3 (8224).

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6354) Routing time step (min) = 15.00

DATA FOR SECTION (3541.0) table with columns: Distance, Elevation, Manning. Lists data points from 0.00 to 7.95.

Table with 5 columns: AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm), MAX DEPTH (m), MAX VEL (m/s). Lists data points from 15.89 to 196.68.

TRAVEL TIME TABLE

TRAVEL TIME TABLE with columns: DEPTH (m), ELEV (m), VOLUME (cu.m.), FLOW RATE (cms), VELOCITY (m/s), TRAV.TIME (min). Lists data points from 0.60 to 18.97.

hydrograph table header

hydrograph table with columns: AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm), MAX DEPTH (m), MAX VEL (m/s). Includes INFLOW and OUTFLOW data.

Table with 5 columns: ADD HYD (8226), AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes data for ID1=1 (0354), ID2=2 (6354), and ID=3 (8226).

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHVD (0344) Area (ha) = 344.00 Curve Number (CN) = 59.0. Includes DT, Ia, and U.H. Tp values.

Unit Hyd Qpeak (cms) = 6.790. PEAK FLOW (cms) = 3.342 (i). TIME TO PEAK (hrs) = 7.000. RUNOFF VOLUME (mm) = 18.719. TOTAL RAINFALL (mm) = 73.100. RUNOFF COEFFICIENT = 0.256.

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (3402) Area (ha) = 234.60. Includes DT, Total Imp, and Dir. Conn. values.

IMPERVIOUS PERVIOUS (i) table with columns: Surface Area (ha), Dep. Storage (mm), Average Slope (%), Length (m), Mannings n, Max.Eff.Inten.(mm/hr) over (min), Storage Coeff. (min). Lists values for both impervious and pervious areas.

Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.07 0.03

TOTALS
 PEAK FLOW (cms)= 6.04 4.65 7.929 (iii)
 TIME TO PEAK (hrs)= 6.00 6.00 6.00
 RUNOFF VOLUME (mm)= 73.10 16.33 23.19
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.22 0.32

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (3401) Area (ha)= 51.20
 ID= 1 DT=15.0 min Total Imp(%)= 46.00 Dir. Conn.(%)= 21.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	23.55	27.65
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	584.24	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	96.49	25.11
over (min)	15.00	30.00
Storage Coeff. (min)	9.20 (ii)	27.79 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.04

TOTALS
 PEAK FLOW (cms)= 2.49 1.35 3.325 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 71.10 18.84 29.81
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.26 0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5340)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1 DT= 15.0 min	0.0000	0.0000	5.3360	1.7877
	0.1730	0.6348	7.2780	2.2185
	1.8360	0.8250	9.1880	2.6486
	2.1540	1.1051	9.5880	2.6586
	3.3950	1.3506	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (3401)	51.200	3.325	6.00	29.81
OUTFLOW: ID= 1 (5340)	51.200	1.201	6.75	29.79

PEAK FLOW REDUCTION [Qout/Qin](%)= 36.13
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 0.7607

ADD HYD (8232)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID= 1 (3402):	234.60	7.929	6.00	23.19
+ ID2= 2 (5340):	51.20	1.201	6.75	29.79
=====				
ID = 3 (8232):	285.80	8.040	6.00	24.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0342) Area (ha)=1004.58 Curve Number (CN)= 54.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 2.57

Unit Hyd Qpeak (cms)= 6.686

PEAK FLOW (cms)= 3.660 (i)
 TIME TO PEAK (hrs)= 10.000
 RUNOFF VOLUME (mm)= 16.262
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.222

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0422) Area (ha)= 780.20 Curve Number (CN)= 54.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.95

Unit Hyd Qpeak (cms)= 6.838

PEAK FLOW (cms)= 3.488 (i)
 TIME TO PEAK (hrs)= 9.000
 RUNOFF VOLUME (mm)= 16.241
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.222

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0410) Area (ha)= 572.01 Curve Number (CN)= 48.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.46

Unit Hyd Qpeak (cms)= 6.698

PEAK FLOW (cms)= 2.643 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 13.432
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.184

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0408) Area (ha)= 231.62 Curve Number (CN)= 58.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.64

Unit Hyd Qpeak (cms)= 6.198

PEAK FLOW (cms)= 2.757 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 18.036
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.247

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0376) Area (ha)= 463.85 Curve Number (CN)= 74.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 7.380

PEAK FLOW (cms)= 6.058 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 29.204
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.400

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0374) Area (ha)= 545.70 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.51

Unit Hyd Qpeak (cms)= 6.158

PEAK FLOW (cms)= 3.687 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 20.009
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.274

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0372) Area (ha)= 110.42 Curve Number (CN)= 37.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 1.954
 PEAK FLOW (cms)= 0.471 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 9.165
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.125

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0370) Area (ha)= 191.85 Curve Number (CN)= 63.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.67

Unit Hyd Qpeak (cms)= 4.860
 PEAK FLOW (cms)= 2.560 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 20.954
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.287

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0368) Area (ha)= 159.48 Curve Number (CN)= 46.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.12

Unit Hyd Qpeak (cms)= 2.433
 PEAK FLOW (cms)= 0.838 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 12.552
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.172

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0366) Area (ha)= 462.62 Curve Number (CN)= 62.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.06

Unit Hyd Qpeak (cms)= 7.451
 PEAK FLOW (cms)= 4.212 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 20.530
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.281

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0364) Area (ha)= 155.27 Curve Number (CN)= 55.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.70

Unit Hyd Qpeak (cms)= 3.782
 PEAK FLOW (cms)= 1.561 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 16.518
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.226

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8302)
 1 + 2 = 3
 ID1= 1 (0364): 155.27 1.561 6.75 16.52
 + ID2= 2 (0366): 462.62 4.212 7.50 20.53
 ID = 3 (8302): 617.89 5.710 7.25 19.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANBYD (0362) Area (ha)= 118.78
 ID= 1 DT=15.0 min Total Imp(%)= 22.00 Dir. Conn.(%)= 8.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	26.13	92.65
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	889.87	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten.(mm/hr)=	96.49	15.08
Storage Coeff. (min)=	11.84 (ii)	34.64 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03

		TOTALS
PEAK FLOW (cms)=	2.03	2.25
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	71.10	14.28
TOTAL RAINFALL (mm)=	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.20

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES;
 CN* = 45.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0358) Area (ha)= 429.87 Curve Number (CN)= 35.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.03

Unit Hyd Qpeak (cms)= 7.091
 PEAK FLOW (cms)= 1.611 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 8.508
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.116

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0360) Area (ha)= 138.37 Curve Number (CN)= 46.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.60

Unit Hyd Qpeak (cms)= 3.957
 PEAK FLOW (cms)= 1.171 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 12.385
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.169

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8306)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0358):	429.87	1.611	7.50	8.51
+ ID2= 2 (0360):	138.37	1.171	6.75	12.38
ID = 3 (8306):	568.24	2.716	7.00	9.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6362) | Routing time step (min)*= 15.00
 | IN= 2--> OUT= 1 |

Distance	Elevation	Manning
0.00	261.46	0.0550
27.86	254.23	0.0550
51.07	251.96	0.0550
74.29	250.77	0.0550
97.50	249.91	0.0550

125.36	249.40	0.0550	
150.93	247.40	0.0550	
155.93	247.33	0.0550 / 0.0350	Main Channel
157.93	246.85	0.0350	Main Channel
159.18	246.65	0.0350	Main Channel
160.18	246.63	0.0350	Main Channel
160.93	246.85	0.0350	Main Channel
161.93	247.18	0.0350 / 0.0550	Main Channel
163.18	248.03	0.0550	
168.18	248.58	0.0550	
183.18	250.18	0.0550	
201.97	252.59	0.0550	
213.57	256.02	0.0550	
225.18	260.31	0.0550	
229.82	261.00	0.0550	

```

----- TRAVEL TIME TABLE -----
DEPTH    ELEV    VOLUME    FLOW RATE    VELOCITY    TRAV.TIME
(m)      (m)      (cu.m.)   (cms)         (m/s)       (min)
-----
0.55     247.18    556E+04   1.9           1.03        49.95
1.28     247.91    1336E+05  16.7         1.53        33.61
2.00     248.63    913E+05   53.6         1.81        28.40
2.73     249.36    185E+06   122.3        2.05        25.18
3.46     250.09    344E+06   224.6        2.02        25.52
4.19     250.82    574E+06   417.9        2.25        22.91
4.91     251.54    855E+06   699.1        2.53        20.38
5.64     252.27    118E+07   1075.3       2.82        18.26
6.37     253.00    153E+07   1559.9       3.15        16.38
7.10     253.73    191E+07   2137.6       3.46        14.89
7.82     254.45    231E+07   2815.2       3.77        13.67
8.55     255.18    272E+07   3608.5       4.10        12.58
9.28     255.91    313E+07   4488.2       4.41        11.69
10.01    256.64    358E+07   5458.0       4.71        10.95
10.73    257.36    403E+07   6512.4       4.99        10.32
11.46    258.09    449E+07   7649.8       5.27        9.78
12.19    258.82    496E+07   8869.5       5.53        9.32
12.92    259.55    544E+07   10171.2      5.78        8.91
13.64    260.27    593E+07   11554.8      6.02        8.55

```

```

----- hydrograph ----- <-pipe / channel->
AREA      QPEAK    TPEAK    R.V.    MAX DEPTH    MAX VEL
(ha)      (cms)    (hrs)    (mm)    (m)           (m/s)
-----
INFLOW : ID= 2 (8306) 568.24  2.72  7.00  9.45  0.59  1.05
OUTFLOW: ID= 1 (6362) 568.24  2.40  8.25  9.45  0.58  1.04

```

```

----- ADD HYD (8304) -----
| 1 + 2 = 3 |
AREA      QPEAK    TPEAK    R.V.
(ha)      (cms)    (hrs)    (mm)
-----
ID1= 1 (0362): 118.78  2.946  6.00  18.83
+ ID2= 2 (6362): 568.24  2.400  8.25  9.45
-----
ID = 3 (8304): 687.02  3.953  6.50  11.07

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

----- ADD HYD (8300) -----
| 1 + 2 = 3 |
AREA      QPEAK    TPEAK    R.V.
(ha)      (cms)    (hrs)    (mm)
-----
ID1= 1 (8302): 617.89  5.710  7.25  19.52
+ ID2= 2 (8304): 687.02  3.953  6.50  11.07
-----
ID = 3 (8300): 1304.91  9.215  7.00  15.07

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

----- ROUTE CHN (6368) -----
| IN= 2--> OUT= 1 |
Routing time step (min)'= 15.00

```

```

----- DATA FOR SECTION (3681.0) -----
Distance    Elevation    Manning
-----
0.00        230.00        0.0370
18.48       223.26        0.0370
36.96       223.05        0.0370
64.67       222.94        0.0370
110.87      222.86        0.0370
133.96      222.74        0.0370
147.82      222.65        0.0370
170.92      222.31        0.0370
174.79      222.26        0.0370 / 0.0300 Main Channel
174.89      221.86        0.0300 Main Channel
175.54      221.86        0.0300 Main Channel
176.19      221.86        0.0300 Main Channel
176.29      222.26        0.0300 / 0.0370 Main Channel
180.16      222.25        0.0370
184.78      222.28        0.0370
189.40      222.31        0.0370

```

332.60	222.37	0.0370
450.00	230.00	0.0370

```

----- TRAVEL TIME TABLE -----
DEPTH    ELEV    VOLUME    FLOW RATE    VELOCITY    TRAV.TIME
(m)      (m)      (cu.m.)   (cms)         (m/s)       (min)
-----
0.40     222.26    771E+03   0.3           0.52        41.74
0.81     222.67    784E+05   30.3          0.51        43.17
1.21     223.07    210E+06   122.2         0.77        28.42
1.62     223.48    384E+06   301.0         1.03        21.25
2.03     223.89    563E+06   550.2         1.28        17.06
2.44     224.30    747E+06   860.7         1.51        14.46
2.84     224.70    934E+06   1228.1        1.73        12.68
3.25     225.11    113E+07   1649.3        1.92        11.37
3.66     225.52    132E+07   2122.3        2.11        10.37
4.07     225.93    152E+07   2645.5        2.29        9.58
4.47     226.33    172E+07   3217.8        2.48        8.92
4.88     226.74    193E+07   3838.3        2.61        8.38
5.29     227.15    214E+07   4506.3        2.76        7.92
5.70     227.56    236E+07   5221.3        2.91        7.52
6.10     227.96    258E+07   5983.0        3.05        7.17
6.51     228.37    280E+07   6790.9        3.19        6.87
6.92     228.78    303E+07   7645.0        3.32        6.60
7.33     229.19    326E+07   8545.1        3.45        6.35
7.73     229.59    349E+07   9491.0        3.57        6.13

```

```

----- hydrograph ----- <-pipe / channel->
AREA      QPEAK    TPEAK    R.V.    MAX DEPTH    MAX VEL
(ha)      (cms)    (hrs)    (mm)    (m)           (m/s)
-----
INFLOW : ID= 2 (8300) 1304.91  9.22  7.00  15.07  0.52  0.52
OUTFLOW: ID= 1 (6368) 1304.91  8.43  8.25  15.07  0.51  0.52

```

```

----- ADD HYD (8298) -----
| 1 + 2 = 3 |
AREA      QPEAK    TPEAK    R.V.
(ha)      (cms)    (hrs)    (mm)
-----
ID1= 1 (0368): 159.48  0.838  7.75  12.55
+ ID2= 2 (6368): 1304.91  8.430  8.25  15.07
-----
ID = 3 (8298): 1464.39  9.262  8.00  14.80

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

----- ADD HYD (8296) -----
| 1 + 2 = 3 |
AREA      QPEAK    TPEAK    R.V.
(ha)      (cms)    (hrs)    (mm)
-----
ID1= 1 (0370): 191.85  2.560  6.75  20.95
+ ID2= 2 (8298): 1464.39  9.262  8.00  14.80
-----
ID = 3 (8296): 1656.24  11.367  7.75  15.51

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

----- ROUTE CHN (6372) -----
| IN= 2--> OUT= 1 |
Routing time step (min)'= 15.00

```

```

----- DATA FOR SECTION (3721.0) -----
Distance    Elevation    Manning
-----
0.00        225.00        0.0390
30.80       219.38        0.0390
61.61       219.30        0.0390
77.01       219.27        0.0390
469.76      219.14        0.0390
477.46      219.13        0.0390
485.16      219.10        0.0390
492.86      219.09        0.0390
495.56      219.09        0.0390 / 0.0310 Main Channel
495.66      218.51        0.0310 Main Channel
500.56      218.51        0.0310 Main Channel
505.46      218.51        0.0310 Main Channel
505.56      219.09        0.0310 / 0.0390 Main Channel
508.26      219.09        0.0390
515.96      219.10        0.0390
523.67      219.21        0.0390
562.17      219.32        0.0390
654.58      219.43        0.0390
731.59      219.46        0.0390
762.39      225.00        0.0390

```

```

----- TRAVEL TIME TABLE -----
DEPTH    ELEV    VOLUME    FLOW RATE    VELOCITY    TRAV.TIME
(m)      (m)      (cu.m.)   (cms)         (m/s)       (min)
-----
0.29     218.80    431E+04   1.7           0.58        43.48
0.58     219.09    867E+04   5.1           0.89        28.32
0.93     219.44    200E+06   58.2          0.44        57.35
1.28     219.79    568E+06   278.6         0.74        33.99
1.62     220.13    939E+06   630.2         1.01        24.84

```

1.97	220.48	.131E+07	1090.4	1.25	20.05
2.32	220.83	.169E+07	1647.7	1.47	17.06
2.67	221.18	.206E+07	2294.6	1.68	14.99
3.01	221.52	.244E+07	3025.6	1.87	13.46
3.36	221.87	.282E+07	3836.5	2.05	12.27
3.71	222.22	.321E+07	4723.7	2.22	11.31
4.06	222.57	.359E+07	5684.6	2.39	10.53
4.40	222.91	.398E+07	6716.6	2.55	9.87
4.75	223.26	.437E+07	7817.7	2.70	9.31
5.10	223.61	.476E+07	8986.2	2.85	8.83
5.45	223.96	.515E+07	10220.4	2.99	8.40
5.79	224.30	.553E+07	11519.1	3.13	8.03
6.14	224.65	.594E+07	12880.8	3.27	7.69
6.49	225.00	.634E+07	14304.5	3.40	7.39

```

----- hydrograph ----- <-pipe / channel->
AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
(ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
INFLOW : ID= 2 (8296) 1656.24  11.37  7.75  15.51  0.62  0.79
OUTFLOW: ID= 1 (6372) 1656.24  10.94  8.50  15.51  0.62  0.80

```

```

-----
| ADD HYD (8294) |
| 1 + 2 = 3 |
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0372): 110.42  0.471  7.25  9.16
+ ID2= 2 (6372): 1656.24  10.944  8.50  15.51
=====
ID = 3 (8294): 1766.66  11.372  8.50  15.12

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8292) |
| 1 + 2 = 3 |
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0374): 545.70  3.687  8.25  20.01
+ ID2= 2 (8294): 1766.66  11.372  8.50  15.12
=====
ID = 3 (8292): 2312.36  15.040  8.50  16.27

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| RESERVOIR (5505) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
OUTFLOW   STORAGE   |   OUTFLOW   STORAGE
(cms)     (ha.m.)    | (cms)     (ha.m.)
0.0000    0.0000    | 65.1290   345.3754
25.4850   24.6697   | 84.9510   456.3890
31.1490   98.6787   | *****   838.7689
39.6440   *****   | *****   838.7789
48.1390   *****   | 0.0000    0.0000

```

```

AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
INFLOW : ID= 2 (8292) 2312.360  15.040  8.50  16.27
OUTFLOW: ID= 1 (5505) 2312.360  11.011  11.25  16.27

```

```

PEAK FLOW REDUCTION [Qout/Qin](%)= 73.21
TIME SHIFT OF PEAK FLOW (min)=165.00
MAXIMUM STORAGE USED (ha.m.)= 10.6689

```

```

-----
| ADD HYD (8272) |
| 1 + 2 = 3 |
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0376): 483.85  6.058  7.50  29.20
+ ID2= 2 (5505): 2312.36  11.011  11.25  16.27
=====
ID = 3 (8272): 2776.21  14.844  10.25  18.43

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB   |
| NASHYD (0396) | Area (ha)= 305.21 Curve Number (CN)= 69.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.08
-----

```

```

Unit Hyd Qpeak (cms)= 4.811
PEAK FLOW (cms)= 3.390 (1)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 25.220
TOTAL RAINFALL (mm)= 73.100

```

RUNOFF COEFFICIENT = 0.345
(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB   |
| NASHYD (0394) | Area (ha)= 325.45 Curve Number (CN)= 53.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.92
-----

```

```

Unit Hyd Qpeak (cms)= 6.013
PEAK FLOW (cms)= 2.485 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 15.628
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.214

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB   |
| NASHYD (0390) | Area (ha)= 420.00 Curve Number (CN)= 55.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.07
-----

```

```

Unit Hyd Qpeak (cms)= 6.683
PEAK FLOW (cms)= 3.048 (1)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 16.653
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.228

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB   |
| NASHYD (0388) | Area (ha)= 220.77 Curve Number (CN)= 58.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.99
-----

```

```

Unit Hyd Qpeak (cms)= 3.819
PEAK FLOW (cms)= 1.875 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 18.209
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.249

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB   |
| NASHYD (0386) | Area (ha)= 241.27 Curve Number (CN)= 61.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.90
-----

```

```

Unit Hyd Qpeak (cms)= 4.562
PEAK FLOW (cms)= 2.408 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 19.881
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.272

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8286) |
| 1 + 2 = 3 |
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0386): 241.27  2.408  7.25  19.88
+ ID2= 2 (0388): 220.77  1.875  7.25  18.21
=====
ID = 3 (8286): 462.04  4.284  7.25  19.08

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB   |
| NASHYD (0384) | Area (ha)= 199.07 Curve Number (CN)= 44.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.96
-----

```

```

Unit Hyd Qpeak (cms)= 3.537
PEAK FLOW (cms)= 1.095 (1)

```

TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 11.721
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.160

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0380) | Area (ha)= 182.01 Curve Number (CN)= 40.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.55

Unit Hyd Qpeak (cms)= 5.609

PEAK FLOW (cms)= 1.322 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 10.074
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.138

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0382) | Area (ha)= 216.59 Curve Number (CN)= 53.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.64

Unit Hyd Qpeak (cms)= 5.733

PEAK FLOW (cms)= 2.181 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 15.501
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.212

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8290) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0380): 182.01 1.322 6.50 10.07
 + ID2= 2 (0382): 216.59 2.181 6.75 15.50
 ID = 3 (8290): 398.60 3.495 6.75 13.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6384) |
 IH= 2--> ODT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3841.0) ----->

Distance	Elevation	Manning	
0.00	294.40	0.0380	
10.59	291.93	0.0380	
21.17	289.19	0.0380	
26.46	287.99	0.0380	
31.76	286.79	0.0380	
37.06	285.59	0.0380	
42.35	284.39	0.0380	
47.65	283.19	0.0380	
52.94	281.99	0.0380	
58.24	280.79	0.0380	
63.53	279.59	0.0380	
68.83	278.39	0.0380	
74.12	277.19	0.0380	
79.42	275.99	0.0380	
84.71	274.79	0.0380	
90.01	273.59	0.0380	
95.30	272.39	0.0380	
100.60	271.19	0.0380	
105.89	269.99	0.0380	
111.19	268.79	0.0380	
116.48	267.59	0.0380	
121.78	266.39	0.0380	
127.07	265.19	0.0380	
132.37	263.99	0.0380	
137.66	262.79	0.0380	
142.96	261.59	0.0380	

DEPTH (m)	RELV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.34	279.64	.715E+03	0.2	0.85	63.42
1.10	280.40	.507E+05	21.9	1.40	38.54
1.85	281.15	.143E+06	91.8	2.10	25.69
2.61	281.91	.269E+06	218.9	2.63	20.46
3.36	282.66	.445E+06	407.1	2.96	18.23
4.12	283.42	.681E+06	698.3	3.21	16.26
4.87	284.17	.978E+06	1111.8	3.68	14.65
5.63	284.93	.133E+07	1674.3	4.06	13.27
6.39	285.69	.173E+07	2457.8	4.59	11.73
7.14	286.44	.215E+07	3378.8	5.07	10.63

7.90 287.20 .261E+07 4418.4 5.47 9.85
 8.45 287.95 .330E+07 5676.3 5.92 9.11
 9.41 288.71 .361E+07 7150.5 6.40 8.42
 10.17 289.47 .414E+07 8783.1 6.86 7.86
 10.92 290.22 .469E+07 10575.1 7.30 7.38
 11.68 290.98 .525E+07 12520.4 7.72 6.98
 12.43 291.73 .582E+07 14619.0 8.12 6.64
 13.19 292.49 .642E+07 16863.2 8.50 6.34
 13.94 293.24 .703E+07 19259.8 8.86 6.08

<----- hydrograph -----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW: ID= 2 (8290) 398.60 3.50 6.75 13.02 0.45 0.90
 OUTFLOW: ID= 1 (6384) 398.60 2.74 8.00 13.02 0.43 0.89

ADD HYD (8288) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0384): 199.07 1.095 7.25 11.72
 + ID2= 2 (6384): 398.60 2.742 8.00 13.02
 ID = 3 (8288): 597.67 3.810 7.75 12.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8284) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8286): 462.04 4.284 7.25 19.08
 + ID2= 2 (8288): 597.67 3.810 7.75 12.59
 ID = 3 (8284): 1059.71 8.003 7.50 15.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8280) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0380): 420.00 3.048 7.50 16.65
 + ID2= 2 (8284): 1059.71 8.003 7.50 15.42
 ID = 3 (8280): 1479.71 11.051 7.50 15.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0392) | Area (ha)= 167.22 Curve Number (CN)= 62.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.74

Unit Hyd Qpeak (cms)= 3.837

PEAK FLOW (cms)= 2.002 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 20.396
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.279

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0378) | Area (ha)= 606.72 Curve Number (CN)= 55.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.18

Unit Hyd Qpeak (cms)= 8.771

PEAK FLOW (cms)= 4.102 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 16.674
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.228

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8282) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)


```

-----
      (ha)   (cms)   (hrs)   (mm)
ID1= 1 (0378): 606.72 4.102 7.75 16.67
+ ID2= 2 (0392): 167.22 2.002 7.00 20.40
-----
ID = 3 (8282): 773.94 5.981 7.25 17.48

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8278) |
| 1 + 2 = 3 |
      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)   (hrs)   (mm)
-----
ID1= 1 (8280): 1479.71 11.051 7.50 15.77
+ ID2= 2 (8282): 773.94 5.981 7.25 17.48
-----
ID = 3 (8278): 2253.65 17.022 7.50 16.36

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6394) |
| IN= 2---> OUT= 1 | Routing time step (min)'= 15.00
-----

```

```

-----
<----- DATA FOR SECTION (3941.0) ----->
Distance      Elevation      Manning
-----
0.00          283.00         0.0380
13.48        282.87         0.0380
53.92        280.08         0.0380
74.13        276.42         0.0380
97.72        265.45         0.0380
114.57       256.93         0.0380
131.42       253.04         0.0380
134.79       252.58         0.0380
138.53       251.74         0.0380 /0.0300 Main Channel
139.03       251.20         0.0300 Main Channel
141.53       251.20         0.0300 Main Channel
144.03       251.20         0.0300 Main Channel
144.53       251.74         0.0300 /0.0380 Main Channel
148.27       252.69         0.0380
151.64       252.97         0.0380
185.34       255.08         0.0380
219.03       257.54         0.0380
262.84       259.43         0.0380
310.02       262.80         0.0380
333.60       283.00         0.0380

```

```

-----
<----- TRAVEL TIME TABLE ----->
DEPTH      ELEV      VOLUME      FLOW RATE      VELOCITY      TRAV.TIME
(m)        (m)        (cu.m.)     (cms)          (m/s)        (min)
-----
0.54      251.74     .827E+04     4.5            1.51         30.71
2.19      253.39     .758E+05     73.2          2.69         17.27
3.83      255.03     .282E+06     331.8         3.27         14.17
5.48      256.68     .633E+06     917.3         4.03         11.51
7.12      258.32     .112E+07     1889.5        4.69         9.90
8.77      259.97     .179E+07     3410.2        5.31         8.73
10.41     261.61     .259E+07     5627.6        6.05         7.66
12.06     263.26     .351E+07     8605.1        6.83         6.79
13.70     264.90     .447E+07     12544.9       7.81         5.94
15.35     266.55     .548E+07     17103.4       8.71         5.32
16.99     268.19     .648E+07     22251.7       9.56         4.85
18.64     269.84     .752E+07     27979.0       10.36        4.48
20.28     271.48     .858E+07     34269.0       11.11        4.17
21.93     273.13     .967E+07     41110.8       11.83        3.92
23.57     274.77     .108E+08     48495.9       12.51        3.71
25.22     276.42     .119E+08     56418.1       13.17        3.52
26.86     278.06     .131E+08     64410.6       13.69        3.39
28.51     279.71     .143E+08     73002.1       14.18        3.27
30.15     281.35     .156E+08     81379.3       14.50        3.20

```

```

-----
<---- hydrograph ----> <-pipe / channel->
AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
(ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
-----
INFLOW : ID= 2 (8278) 2253.65 17.02 7.50 16.36 0.84 1.64
OUTFLOW: ID= 1 (6394) 2253.65 16.55 8.00 16.36 0.83 1.64

```

```

-----
| ADD HYD (8276) |
| 1 + 2 = 3 |
      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)   (hrs)   (mm)
-----
ID1= 1 (0394): 325.45 2.485 7.25 15.63
+ ID2= 2 (6394): 2253.65 16.553 8.00 16.36
-----
ID = 3 (8276): 2579.10 18.926 8.00 16.26

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6396) |

```

```

| IN= 2---> OUT= 1 | Routing time step (min)'= 15.00

```

```

-----
<----- DATA FOR SECTION (3961.0) ----->
Distance      Elevation      Manning
-----
0.00          263.00         0.0410
11.75        257.14         0.0410
23.50        253.97         0.0410
41.13        247.83         0.0410
76.38        232.09         0.0410
135.13       229.07         0.0410
149.82       228.97         0.0410
152.75       228.96         0.0410 /0.0300 Main Channel
154.19       228.73         0.0300 Main Channel
154.69       228.20         0.0300 Main Channel
155.69       228.20         0.0300 Main Channel
156.69       228.20         0.0300 Main Channel
157.19       228.73         0.0300 Main Channel
158.63       228.95         0.0300 /0.0410 Main Channel
161.57       228.96         0.0410
164.51       229.71         0.0410
196.82       241.70         0.0410
223.26       249.21         0.0410
246.76       255.13         0.0410
290.82       263.51         0.0410

```

```

-----
<----- TRAVEL TIME TABLE ----->
DEPTH      ELEV      VOLUME      FLOW RATE      VELOCITY      TRAV.TIME
(m)        (m)        (cu.m.)     (cms)          (m/s)        (min)
-----
0.75      228.95     .780E+04     2.2            0.98         58.06
2.54      230.74     .277E+06     145.1          1.78         31.76
4.33      232.53     .786E+06     609.1          2.63         21.50
6.13      234.33     .140E+07     1481.5         3.59         15.77
7.92      236.12     .207E+07     2677.3         4.39         12.90
9.71      237.91     .280E+07     4184.8         5.09         11.14
11.50     239.70     .357E+07     6001.4         5.71         9.93
13.29     241.49     .441E+07     8129.1         6.27         9.03
15.09     243.29     .530E+07     10549.4        6.77         8.37
16.88     245.08     .625E+07     13300.6        7.24         7.83
18.67     246.87     .725E+07     16396.2        7.68         7.38
20.46     248.66     .834E+07     19809.8        8.07         7.02
22.26     250.46     .949E+07     23532.5        8.43         6.72
24.05     252.25     .107E+08     27641.2        8.77         6.46
25.84     254.04     .120E+08     32155.2        9.10         6.22
27.63     255.83     .134E+08     36901.0        9.37         6.05
29.42     257.62     .149E+08     42147.7        9.64         5.88
31.22     259.42     .164E+08     48149.1        9.97         5.68
33.01     261.21     .181E+08     54644.8        10.29         5.51

```

```

-----
<---- hydrograph ----> <-pipe / channel->
AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
(ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
-----
INFLOW : ID= 2 (8276) 2579.10 18.93 8.00 16.26 0.96 1.03
OUTFLOW: ID= 1 (6396) 2579.10 17.09 9.00 16.26 0.93 1.02

```

```

-----
| ADD HYD (8274) |
| 1 + 2 = 3 |
      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)   (hrs)   (mm)
-----
ID1= 1 (0396): 305.21 3.390 7.50 25.22
+ ID2= 2 (6396): 2579.10 17.092 9.00 16.26
-----
ID = 3 (8274): 2884.31 20.021 9.00 17.21

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8270) |
| 1 + 2 = 3 |
      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)   (hrs)   (mm)
-----
ID1= 1 (8272): 2776.21 14.844 10.25 18.43
+ ID2= 2 (8274): 2884.31 20.021 9.00 17.21
-----
ID = 3 (8270): 5660.52 34.206 9.25 17.81

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| RESERVOIR (5506) |
| IN= 2---> OUT= 1 |
| DT= 15.0 min |
      OUTFLOW STORAGE | OUTFLOW STORAGE
      (cms) (ha.m.) | (cms) (ha.m.)
-----
0.0000 0.0000 | 60.8810 135.6832
31.1490 24.6697 | 96.2770 900.4431
36.8120 37.0045 | 96.6770 900.4531
45.3070 86.3439 | 0.0000 0.0000

```

```

-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
-----
INFLOW : ID= 2 (8270) 5660.52 34.206 9.25 17.81

```

OUTFLOW: ID= 1 (5506) 5660.520 28.236 11.75 17.81

PEAK FLOW REDUCTION [Qout/Qin](%)= 82.54
TIME SHIFT OF PEAK FLOW (min)=150.00
MAXIMUM STORAGE USED (ha.m.)= 22.3717

CALIB
NASHYD (0406) Area (ha)= 142.65 Curve Number (CN)= 66.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.59

Unit Hyd Qpeak (cms)= 4.135

PEAK FLOW (cms)= 2.315 (I)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 22.790
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.312

(I) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0404) Area (ha)= 246.46 Curve Number (CN)= 47.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.98

Unit Hyd Qpeak (cms)= 4.280

PEAK FLOW (cms)= 1.475 (I)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 12.944
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.177

(I) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0402) Area (ha)= 244.00 Curve Number (CN)= 61.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 3.879

PEAK FLOW (cms)= 2.133 (I)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 19.936
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.273

(I) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0400) Area (ha)= 93.97 Curve Number (CN)= 52.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.44

Unit Hyd Qpeak (cms)= 3.630

PEAK FLOW (cms)= 1.222 (I)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 14.807
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.203

(I) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0398) Area (ha)= 328.19 Curve Number (CN)= 55.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.83

Unit Hyd Qpeak (cms)= 6.759

PEAK FLOW (cms)= 2.909 (I)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 16.581
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.227

(I) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8268)

1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0398): 328.19 2.909 7.00 16.58
+ ID2= 2 (0400): 93.97 1.222 6.50 14.81
ID = 3 (8268): 422.16 4.012 6.75 16.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6402) |
ID= 2 -> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (4021.0) ----->

Distance	Elevation	Manning
0.00	218.50	0.0360
11.50	238.00	0.0360
23.00	237.93	0.0360
34.49	236.39	0.0360
63.24	233.98	0.0360
97.73	228.15	0.0360
123.60	227.08	0.0360
126.48	226.61	0.0360
127.60	226.47	0.0360 / 0.0330
127.85	225.25	0.0330
129.35	225.25	0.0330
130.85	225.25	0.0330
131.10	226.47	0.0330 / 0.0360
132.22	226.59	0.0360
143.72	227.42	0.0360
169.59	227.88	0.0360
192.59	231.19	0.0360
218.46	233.02	0.0360
241.45	235.50	0.0360
284.57	236.43	0.0360

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.41	225.66	.391E+04	1.7	1.36	38.31
0.81	226.06	.804E+04	5.0	1.93	27.02
1.22	226.47	1.24E+05	9.1	2.30	22.61
1.84	227.09	3.08E+05	23.3	2.36	22.03
2.46	227.71	9.04E+05	62.0	2.14	24.31
3.09	228.34	2.23E+06	188.2	2.63	19.75
3.71	228.96	3.79E+06	410.2	3.38	15.39
4.33	229.58	5.50E+06	711.5	4.04	12.89
4.95	230.20	7.37E+06	1091.3	4.62	11.25
5.58	230.83	9.39E+06	1550.6	5.15	10.10
6.20	231.45	1.16E+07	2072.4	5.59	9.31
6.82	232.07	1.40E+07	2659.2	5.93	8.77
7.44	232.69	1.67E+07	3345.8	6.27	8.30
8.07	233.32	1.95E+07	4159.0	6.64	7.83
8.69	233.94	2.26E+07	5100.8	7.03	7.39
9.31	234.56	2.59E+07	6081.3	7.32	7.11
9.93	235.18	2.95E+07	7183.1	7.60	6.84
10.56	235.81	3.34E+07	8345.2	7.62	6.83
11.18	236.43	3.79E+07	9116.0	7.51	6.92

<----- hydrograph -----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8268) 422.16 4.01 6.75 16.19 0.70 1.72
OUTFLOW: ID= 1 (6402) 422.16 3.72 7.50 16.19 0.66 1.66

ADD HYD (8266) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0402): 244.00 2.133 7.50 19.94
+ ID2= 2 (6402): 422.16 3.724 7.50 16.19
ID = 3 (8266): 666.16 5.857 7.50 17.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8264) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0404): 246.46 1.475 7.25 12.94
+ ID2= 2 (8266): 666.16 5.857 7.50 17.56
ID = 3 (8264): 912.62 7.330 7.50 16.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8262) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0406): 142.65 2.315 6.50 22.79
+ ID2= 2 (8264): 812.62 7.330 7.50 16.31
=====
ID = 3 (8262): 1055.27 9.418 7.25 17.19

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8260) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (5506): 5660.52 28.236 11.75 17.81
+ ID2= 2 (8262): 1055.27 9.418 7.25 17.19
=====
ID = 3 (8260): 6715.79 32.331 11.25 17.71

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8258) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0408): 231.62 2.757 6.75 18.04
+ ID2= 2 (8260): 6715.79 32.331 11.25 17.71
=====
ID = 3 (8258): 6947.41 33.171 11.00 17.72

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8256) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0410): 572.01 2.643 8.25 13.43
+ ID2= 2 (8258): 6947.41 33.171 11.00 17.72
=====
ID = 3 (8256): 7519.42 35.308 10.75 17.40

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| RESERVOIR (5507) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
0.0000 0.0000 | 90.6140 67.8416
39.6440 12.3348 | ***** 160.3529
48.1390 18.5023 | ***** 160.3629
67.9600 37.0045 | 0.0000 0.0000
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (8256) 7519.421 35.308 10.75 17.40
OUTFLOW: ID= 1 (5507) 7519.421 34.570 11.75 17.40
-----
PEAK FLOW REDUCTION [Qout/Qin](%)= 97.91
TIME SHIFT OF PEAK FLOW (min)= 60.00
MAXIMUM STORAGE USED (ha.m.)= 10.7597

```

```

-----
| CALIB (0420) |
| NASHYD (0420) | Area (ha)= 175.82 Curve Number (CN)= 53.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.81
-----
Unit Hyd Qpeak (cms)= 3.692
PEAK FLOW (cms)= 1.485 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 15.590
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.213

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0418) |
| NASHYD (0418) | Area (ha)= 182.79 Curve Number (CN)= 64.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.05
-----
Unit Hyd Qpeak (cms)= 2.966

```

```

PEAK FLOW (cms)= 1.780 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 21.774
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.298

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0416) |
| NASHYD (0416) | Area (ha)= 439.30 Curve Number (CN)= 64.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.29
-----
Unit Hyd Qpeak (cms)= 5.832

```

```

PEAK FLOW (cms)= 3.686 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 21.827
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.299

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0412) |
| NASHYD (0412) | Area (ha)= 238.70 Curve Number (CN)= 54.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.80
-----
Unit Hyd Qpeak (cms)= 5.088

```

```

PEAK FLOW (cms)= 2.105 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 16.071
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.220

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (4141) |
| STANDHYD (4141) | Area (ha)= 43.70
| ID= 1 DT=15.0 min | Total Imp(%)= 36.00 Dir. Conn.(%)= 17.00
-----

```

```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 15.73 27.97
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 539.75 40.00
Mannings n = 0.013 0.250
Max. Eff. Inten.(mm/hr)= 96.49 52.82
over (min) 15.00 30.00
Storage Coeff. (min)= 8.77 (ii) 22.58 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.09 0.04
*TOTALS*
PEAK FLOW (cms)= 1.74 1.96 2.978 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 71.10 26.72 34.27
TOTAL RAINFALL (mm)= 73.10 73.10 73.10
RUNOFF COEFFICIENT = 0.97 0.37 0.47

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| RESERVOIR (5414) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
0.0000 0.0000 | 3.4720 1.9177
0.3820 0.3152 | 3.8720 1.9277
1.8280 0.4690 | 0.0000 0.0000
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (4141) 43.700 2.978 6.00 34.27
OUTFLOW: ID= 1 (5414) 43.700 1.837 6.50 34.26

```

PEAK FLOW REDUCTION [Qout/Qin](%)= 61.71
TIME SHIFT OF PEAK FLOW (min)= 30.00

MAXIMUM STORAGE USED (ha.m.)= 0.4849

CALLS STANHYD (4142) Area (ha)= 144.30 Dir. Conn.(%)= 10.00

Surface Area (ha)= 30.30 IMPERVIOUS 114.00
Dep. Storage (mm)= 2.00 PERVIOUS (i) 5.00
Average Slope (%)= 0.50

Unit Hyd. Tpeak (min)= 15.00 (ii) 45.00
Unit Hyd. Tpeak (cms)= 0.08 0.03

PEAK FLOW (cms)= 3.02 4.96 5.721 (iii)
TIME TO PEAK (hrs)= 6.00 6.50 6.50
RUNOFF VOLUME (mm)= 73.10 24.32 29.00

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8254) AREA QPEAK TPEAK R.V.
ID1= 1 (4142): 144.30 5.721 6.50 29.00
+ ID2= 2 (5414): 43.70 1.837 6.50 34.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8252) AREA QPEAK TPEAK R.V.
ID1= 1 (0412): 238.70 2.105 7.00 16.07
+ ID2= 2 (8254): 188.00 7.558 6.50 30.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6416) Routing time step (min)= 15.00

Table with columns: Distance, Elevation, Manning. Rows show data for various points along the channel, including elevations and Manning coefficients.

TRAVEL TIME TABLE with columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV. TIME. Shows values for depth, elevation, volume, flow rate, velocity, and travel time.

Table with 6 columns of numerical data, likely representing flow characteristics at different stages or locations.

hydrograph <----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
INFLOW: ID= 2 (8252) 426.70 9.54 6.50 22.31 0.43 0.81

ADD HYD (8250) AREA QPEAK TPEAK R.V.
ID1= 1 (0416): 439.30 3.686 8.00 21.83
+ ID2= 2 (6416): 426.70 4.904 7.50 22.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8248) AREA QPEAK TPEAK R.V.
ID1= 1 (0418): 182.79 1.780 7.50 21.77
+ ID2= 2 (8250): 866.00 8.541 7.50 22.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8246) AREA QPEAK TPEAK R.V.
ID1= 1 (0420): 175.82 1.485 7.00 15.59
+ ID2= 2 (8248): 1048.79 10.321 7.50 22.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8244) AREA QPEAK TPEAK R.V.
ID1= 1 (5507): 7519.42 34.570 11.75 17.40
+ ID2= 2 (8246): 1224.61 11.759 7.50 21.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8242) AREA QPEAK TPEAK R.V.
ID1= 1 (0422): 780.20 3.488 9.00 16.24
+ ID2= 2 (8244): 8744.03 40.750 11.00 17.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5508)
IN= 2--> OUT= 1

DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	*****	197.3574
	76.4550	30.8371	*****	394.7148
	*****	61.6742	*****	394.7248
	*****	*****	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8242)	9524.231	43.985	11.00	17.78
OUTFLOW: ID= 1 (5508)	9524.231	42.683	12.25	17.78

PEAK FLOW REDUCTION [Qout/Qin]= 97.04
 TIME SHIFT OF PEAK FLOW (min)= 75.00
 MAXIMUM STORAGE USED (ha.m.)= 17.2250

CALIB (0336)	Area (ha)	Curve Number (CN)	R.V. (mm)
NASHYD	2785.00	72.0	6.00
ID= 1 DT=15.0 min	IA (mm)= 5.00	# of Linear Res.(N)= 1.50	
	U.H. Tp(hrs)= 15.39		

Unit Hyd Qpeak (cms)= 3.090
 PEAK FLOW (cms)= 3.370 (i)
 TIME TO PEAK (hrs)= 22.250
 RUNOFF VOLUME (mm)= 27.751
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.380

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3382)	Area (ha)	Total Imp(%)	Dir. Conn.(%)
ID= 1 DT=15.0 min	525.90	37.00	17.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	194.58	331.32
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1872.43	40.00
Mannings n	0.013	0.250
Max. Eff. Inten.(mm/hr)	96.49	57.38
over (min)	15.00	45.00
Storage Coeff. (min)	18.51 (ii)	31.86 (ii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.06	0.03
	TOTALS	
PEAK FLOW (cms)	15.86	19.45
TIME TO PEAK (hrs)	6.00	6.50
RUNOFF VOLUME (mm)	71.10	28.48
TOTAL RAINFALL (mm)	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.39

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3381)	Area (ha)	Total Imp(%)	Dir. Conn.(%)
ID= 1 DT=15.0 min	73.00	49.00	23.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	35.77	37.23
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	697.61	40.00
Mannings n	0.013	0.250
Max. Eff. Inten.(mm/hr)	96.49	72.01
over (min)	15.00	30.00
Storage Coeff. (min)	10.23 (ii)	22.43 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.04
	TOTALS	
PEAK FLOW (cms)	3.77	3.58
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	71.10	31.13
TOTAL RAINFALL (mm)	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

CN* = 66.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5338)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1	0.0000	0.0000	3.3350	1.2400
DT= 15.0 min	0.3230	0.3074	4.3560	1.3520
	0.9360	0.5374	5.3290	1.4590
	1.7590	0.8302	5.7290	1.4690
	2.4110	1.0550	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (3381)	73.000	6.061	6.00	40.32
OUTFLOW: ID= 1 (5338)	73.000	2.647	6.75	40.32

PEAK FLOW REDUCTION [Qout/Qin]= 43.67
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 1.1258

ADD HYD (8310)	Area (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID= 1 (3382):	525.90	25.210	6.50	35.73
+ ID2= 2 (5338):	73.00	2.647	6.75	40.32
ID = 3 (8310):	598.90	27.843	6.50	36.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (3342)	Area (ha)	Curve Number (CN)	R.V. (mm)
ID= 1 DT=15.0 min	587.10	56.0	6.00
	IA (mm)= 5.00	# of Linear Res.(N)= 1.50	
	U.H. Tp(hrs)= 8.19		

Unit Hyd Qpeak (cms)= 1.224

PEAK FLOW (cms)= 0.825 (i)
 TIME TO PEAK (hrs)= 15.250
 RUNOFF VOLUME (mm)= 17.318
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.237

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3341)	Area (ha)	Total Imp(%)	Dir. Conn.(%)
ID= 1 DT=15.0 min	33.00	51.00	31.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	16.83	16.17
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	469.04	40.00
Mannings n	0.013	0.250
Max. Eff. Inten.(mm/hr)	96.49	43.28
over (min)	15.00	30.00
Storage Coeff. (min)	8.06 (ii)	23.02 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.10	0.04
	TOTALS	
PEAK FLOW (cms)	2.45	0.92
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	71.10	20.49
TOTAL RAINFALL (mm)	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.28

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 52.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5334)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1				
DT= 15.0 min				

0.0000	0.0000	0.6540	0.4604
0.0390	0.1699	0.8330	0.5249
0.1860	0.2610	1.0130	0.5896
0.3750	0.3534	1.4130	0.5996
0.4340	0.3813	0.0000	0.0000

INFLW : ID= 2 (3341) 33.000 3.022 6.00 36.18
 OUTFLOW: ID= 1 (5334) 33.000 0.928 6.50 36.15

PEAK FLOW REDUCTION [Qout/Qin](%)= 30.69
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 0.5651

ADD HYD (8314)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (3342): 587.10 0.825 15.25 17.32
 + ID2= 2 (5334): 33.00 0.928 6.50 36.15
 ID = 3 (8314): 620.10 1.209 6.75 18.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0306)
 NASHYD (0306) Area (ha)= 283.97 Curve Number (CN)= 52.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 6.44

Unit Hyd Qpeak (cms)= 0.753

PEAK FLOW (cms)= 0.445 (1)
 TIME TO PEAK (hrs)= 13.750
 RUNOFF VOLUME (mm)= 15.318
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.210

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0286)
 NASHYD (0286) Area (ha)= 260.51 Curve Number (CN)= 84.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.16

Unit Hyd Qpeak (cms)= 3.834

PEAK FLOW (cms)= 4.421 (1)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 39.488
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.540

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0282)
 NASHYD (0282) Area (ha)= 449.38 Curve Number (CN)= 77.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.47

Unit Hyd Qpeak (cms)= 5.226

PEAK FLOW (cms)= 5.078 (1)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 32.027
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.438

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0284)
 NASHYD (0284) Area (ha)= 78.93 Curve Number (CN)= 84.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.57

Unit Hyd Qpeak (cms)= 2.344

PEAK FLOW (cms)= 2.351 (1)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 38.894
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.532

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8388)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0282): 449.38 5.078 8.00 32.03
 + ID2= 2 (0284): 78.93 2.351 6.50 38.89
 ID = 3 (8388): 528.31 6.886 7.25 33.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6286)
 IN= 2--> OUT= 1 Routing time step (min)= 15.00

DATA FOR SECTION (2861.0)			
Distance	Elevation	Manning	
0.00	233.00	0.0450	
20.58	228.51	0.0450	
41.17	227.74	0.0450	
51.46	227.41	0.0450	
97.77	225.96	0.0450	
149.23	223.94	0.0450	
200.69	220.84	0.0450	
226.42	220.66	0.0450	
238.85	220.22	0.0450	
241.35	220.01	0.0450 / 0.0350	Main Channel
241.85	219.70	0.0350	Main Channel
245.85	219.72	0.0350	Main Channel
246.35	220.06	0.0350	Main Channel
248.85	220.23	0.0350	Main Channel
303.60	221.64	0.0350	Main Channel
380.79	224.98	0.0450	
432.25	229.54	0.0450	
457.98	233.33	0.0450	
483.71	234.27	0.0450	
509.44	233.81	0.0450	

TRAVEL TIME TABLE					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.31	220.01	.382E+04	0.8	0.57	82.89
1.06	220.76	.523E+05	15.1	0.66	72.51
1.81	221.51	.240E+06	88.6	1.05	45.21
2.56	222.26	.510E+06	261.7	1.46	32.47
3.31	223.01	.843E+06	533.8	1.81	26.34
4.06	223.76	.124E+07	913.6	2.10	22.64
4.81	224.51	.171E+07	1399.0	2.34	20.33
5.56	225.26	.223E+07	2051.0	2.60	18.27
6.31	226.01	.285E+07	2907.0	2.91	16.35
7.06	226.76	.352E+07	3902.5	3.16	15.04
7.82	227.52	.426E+07	5069.9	3.39	14.01
8.57	228.27	.507E+07	6433.0	3.62	13.13
9.32	229.02	.593E+07	8069.2	3.89	12.24
10.07	229.77	.681E+07	9926.7	4.16	11.44
10.82	230.52	.772E+07	11962.8	4.42	10.75
11.57	231.27	.864E+07	14167.1	4.68	10.17
12.32	232.02	.959E+07	16537.1	4.92	9.66
13.07	232.77	.105E+08	19070.7	5.16	9.22
13.82	233.52	.115E+08	21790.4	5.39	8.82

INFLW : ID= 2 (8388) 528.31 6.89 7.25 33.05 0.72 0.62
 OUTFLOW: ID= 1 (6286) 528.31 5.82 9.00 33.05 0.64 0.61

ADD HYD (8386)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0286): 260.51 4.421 7.50 39.49
 + ID2= 2 (6286): 528.31 5.824 9.00 33.05
 ID = 3 (8386): 788.82 9.844 8.50 35.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0302)
 NASHYD (0302) Area (ha)= 473.90 Curve Number (CN)= 58.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.66

Unit Hyd Qpeak (cms)= 4.874

PEAK FLOW (cms)= 2.715 (1)

TIME TO PEAK (hrs)= 8.500
 RUNOFF VOLUME (mm)= 18.313
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.251

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0300) Area (ha)= 258.93 Curve Number (CN)= 52.0
 ID= 1 DT=15.0 min Ia (mm)= 4.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.03

Unit Hyd Qpeak (cms)= 4.292
 PEAK FLOW (cms)= 1.811 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 15.576
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.213

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0298) Area (ha)= 330.51 Curve Number (CN)= 45.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.26

Unit Hyd Qpeak (cms)= 4.461
 PEAK FLOW (cms)= 1.541 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 12.163
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.166

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8395)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0298):	330.51	1.541	8.00	12.16
+ ID2= 2 (0300):	258.93	1.811	7.50	15.58
ID = 3 (8395):	589.44	3.331	7.75	13.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0288) Area (ha)= 340.83 Curve Number (CN)= 78.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 2.21

Unit Hyd Qpeak (cms)= 2.629
 PEAK FLOW (cms)= 2.858 (i)
 TIME TO PEAK (hrs)= 9.250
 RUNOFF VOLUME (mm)= 33.083
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.453

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0290) Area (ha)= 269.18 Curve Number (CN)= 78.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 4.279
 PEAK FLOW (cms)= 3.986 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 32.883
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.450

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8397)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0288):	340.83	2.858	9.25	33.08

+ ID2= 2 (0290): 269.18 3.986 7.25 32.88
 ID = 3 (8397): 610.01 6.617 8.00 32.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0296) Area (ha)= 293.65 Curve Number (CN)= 76.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.13

Unit Hyd Qpeak (cms)= 4.437
 PEAK FLOW (cms)= 3.926 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 31.003
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.424

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0292) Area (ha)= 738.49 Curve Number (CN)= 68.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.52

Unit Hyd Qpeak (cms)= 8.289
 PEAK FLOW (cms)= 6.145 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 24.582
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.336

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0294) Area (ha)= 274.15 Curve Number (CN)= 57.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.87

Unit Hyd Qpeak (cms)= 5.367
 PEAK FLOW (cms)= 2.485 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 17.633
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.241

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8398)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0292):	738.49	6.145	8.25	24.58
+ ID2= 2 (0294):	274.15	2.485	7.00	17.63
ID = 3 (8398):	1012.64	8.463	8.00	22.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (8296) Routing time step (min)= 15.00

Distance	Elevation	Manning
0.00	243.98	0.0400
42.59	243.18	0.0400
85.17	241.81	0.0400
127.76	240.50	0.0400
170.34	239.56	0.0400
212.93	236.15	0.0400
255.51	234.01	0.0400
298.10	233.82	0.0400
340.68	233.12	0.0400 / 0.0400
383.27	232.32	0.0400
425.85	231.80	0.0400
468.44	232.23	0.0400
511.02	233.10	0.0400 / 0.0400
553.61	233.22	0.0400
596.19	233.87	0.0400
638.78	234.12	0.0400
681.36	234.23	0.0400
723.95	234.73	0.0400

337.15 241.75 0.0400
351.34 244.00 0.0400

318.65 223.69 0.0400
360.22 225.57 0.0400
397.16 227.60 0.0400
457.20 228.35 0.0400

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.43	232.23	.510E+03	0.1	0.53	81.07
0.87	232.67	.213E+04	0.7	0.83	51.49
1.30	233.10	.501E+04	2.2	1.10	38.75
1.98	233.78	.242E+05	12.6	1.34	31.93
2.66	234.46	.738E+05	42.1	1.46	29.21
3.34	235.14	.177E+06	127.9	1.84	23.13
4.02	235.82	.314E+06	281.4	2.22	18.61
4.70	236.50	.481E+06	509.4	2.71	15.73
5.38	237.18	.672E+06	815.6	3.11	13.73
6.06	237.86	.886E+06	1200.7	3.46	12.30
6.74	238.54	.112E+07	1669.4	3.80	11.23
7.42	239.22	.139E+07	2226.9	4.11	10.38
8.10	239.90	.168E+07	2808.4	4.28	9.95
8.78	240.58	.202E+07	3470.4	4.40	9.68
9.46	241.26	.240E+07	4331.7	4.61	9.24
10.14	241.94	.283E+07	5335.2	4.83	8.84
10.82	242.62	.330E+07	6482.6	5.03	8.48
11.50	243.30	.381E+07	7764.1	5.21	8.18
12.18	243.98	.439E+07	9101.2	5.31	8.04

----- hydrograph ----- <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8398) 1012.64	8.46	8.00	22.70	1.71	1.23
OUTFLOW : ID= 1 (6296) 1012.64	8.22	8.50	22.70	1.69	1.22

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.29	221.29	.228E+04	0.2	0.32	165.63
0.59	221.59	.465E+04	0.7	0.45	116.15
0.88	221.88	.713E+04	1.2	0.54	96.87
1.18	222.18	.971E+04	1.9	0.61	86.20
1.57	222.57	.157E+05	3.3	0.59	88.86
1.97	222.97	.427E+05	6.4	0.47	111.39
2.36	223.36	.115E+06	16.1	0.44	119.14
2.76	223.76	.237E+06	37.1	0.49	106.49
3.15	224.15	.409E+06	72.9	0.56	93.40
3.55	224.55	.631E+06	127.0	0.63	82.76
3.94	224.94	.903E+06	202.4	0.70	74.37
4.34	225.34	.123E+07	301.8	0.77	67.68
4.73	225.73	.160E+07	433.5	0.85	61.37
5.13	226.13	.200E+07	602.2	0.95	55.29
5.52	226.52	.242E+07	804.7	1.05	50.13
5.92	226.92	.286E+07	1034.2	1.14	46.11
6.31	227.31	.332E+07	1290.5	1.22	42.88
6.71	227.71	.380E+07	1560.9	1.29	40.55
7.10	228.10	.432E+07	1835.6	1.34	39.19

----- hydrograph ----- <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8392) 2505.74	21.86	8.00	24.05	2.47	0.45
OUTFLOW : ID= 1 (6302) 2505.74	17.19	10.25	24.05	2.38	0.44

----- ADD HYD (8396) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0296):	293.65	3.926	7.50	31.00
+ ID2= 2 (6296):	1012.64	8.218	8.50	22.70
ID = 3 (8396):	1306.29	11.968	8.25	24.57

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ADD HYD (8394) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8396):	1306.29	11.968	8.25	24.57
+ ID2= 2 (8397):	610.01	6.617	8.00	32.99
ID = 3 (8394):	1916.30	18.538	8.00	27.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ADD HYD (8392) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8394):	1916.30	18.538	8.00	27.25
+ ID2= 2 (8395):	589.44	3.331	7.75	13.66
ID = 3 (8392):	2505.74	21.856	8.00	24.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ROUTE CHN (6302) -----

IN= 2--> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (3021.0) -----

Distance	Elevation	Manning
0.00	228.10	0.0400
18.47	227.12	0.0400
36.95	226.12	0.0400
46.18	225.84	0.0400
55.42	225.58	0.0400
272.47	222.88	0.0400
277.09	222.76	0.0400
281.71	222.58	0.0400
288.54	222.18	0.0400 / 0.0300
288.64	221.00	0.0300
290.94	221.00	0.0300
291.04	221.00	0.0300
291.54	222.75	0.0300 / 0.0400
300.18	222.83	0.0400
304.80	223.04	0.0400
309.42	223.25	0.0400

----- ADD HYD (8390) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0302):	473.90	2.715	8.50	18.31
+ ID2= 2 (6302):	2505.74	17.159	10.25	24.05
ID = 3 (8390):	2979.64	19.699	10.25	23.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ADD HYD (8348) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8386):	788.82	9.844	8.50	35.18
+ ID2= 2 (8390):	2979.64	19.699	10.25	23.14
ID = 3 (8348):	3768.46	28.649	9.75	25.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- CALIB -----

NASHYD (0304)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	5.00	63.0
U.H. Tp(hrs)	2.78	# of Linear Res.(N)= 1.50

Unit Hyd Qpeak (cms) = 1.793

PEAK FLOW (cms) = 1.311 (i)
TIME TO PEAK (hrs) = 10.250
RUNOFF VOLUME (mm) = 21.297
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.291

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB -----

NASHYD (0280)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	5.00	82.0
U.H. Tp(hrs)	0.65	# of Linear Res.(N)= 1.50

Unit Hyd Qpeak (cms) = 6.009

PEAK FLOW (cms) = 6.103 (i)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 36.959
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.506

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.


```

-----
| CALIB |
| NASHYD (0278) | Area (ha)= 485.49 Curve Number (CN)= 82.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.52

```

```

Unit Hyd Qpeak (cms)= 5.453

PEAK FLOW (cms)= 6.254 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 37.239
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.509

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0276) | Area (ha)= 90.89 Curve Number (CN)= 79.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.67

```

```

Unit Hyd Qpeak (cms)= 2.302

PEAK FLOW (cms)= 2.014 (1)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 33.570
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.459

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0274) | Area (ha)= 392.49 Curve Number (CN)= 75.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.08

```

```

Unit Hyd Qpeak (cms)= 6.182

PEAK FLOW (cms)= 5.249 (1)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 30.083
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.412

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8360) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| (ha) (cms) (hrs) (mm)
| ID1= 1 (0274): 392.49 5.249 7.50 30.08
| + ID2= 2 (0276): 90.89 2.014 6.75 33.57
|-----|
| ID = 3 (8360): 483.38 7.141 7.00 30.74

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8358) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| (ha) (cms) (hrs) (mm)
| ID1= 1 (0278): 485.49 6.254 8.00 37.24
| + ID2= 2 (8360): 483.38 7.141 7.00 30.74
|-----|
| ID = 3 (8358): 968.87 13.139 7.50 34.00

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CIB (6280) |
| IN= 2---> OUT= 1 | Routing time step (min)'= 15.00
|-----|

```

```

<----- DATA FOR SECTION (2801.0) ----->
Distance Elevation Manning
0.00 241.14 0.0500
13.32 240.80 0.0500
39.95 240.07 0.0500
96.54 236.23 0.0500
113.19 234.15 0.0500
123.18 232.35 0.0500
143.15 225.80 0.0500
149.81 225.62 0.0500
153.14 225.40 0.0500
157.30 224.76 0.0500 /0.0300 Main Channel
159.80 224.26 0.0300 Main Channel

```

```

162.30 224.85 0.0300 Main Channel
162.55 224.97 0.0300 /0.0500 Main Channel
167.80 225.05 0.0500
186.43 229.14 0.0500
213.06 234.75 0.0500
236.37 237.09 0.0500
266.33 237.31 0.0500
292.96 237.83 0.0500
329.58 241.50 0.0500

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu. m) (cms) (m/s) (min)
0.50 224.76 .263E+04 0.8 0.65 58.31
1.36 225.62 .278E+05 13.4 1.10 34.54
2.22 226.48 .855E+05 52.6 1.40 27.09
3.09 227.35 .157E+06 120.4 1.74 21.78
3.95 228.21 .242E+06 216.3 2.04 18.64
4.81 229.07 .340E+06 341.8 2.29 16.56
5.67 229.93 .450E+06 498.3 2.52 15.06
6.53 230.79 .574E+06 688.3 2.73 13.90
7.40 231.66 .711E+06 913.8 2.93 12.97
8.26 232.52 .861E+06 1173.4 3.11 12.23
9.12 233.38 .103E+07 1463.3 3.24 11.72
9.98 234.24 .121E+07 1793.3 3.37 11.26
10.85 235.11 .142E+07 2141.0 3.45 11.02
11.71 235.97 .165E+07 2530.4 3.50 10.86
12.57 236.83 .192E+07 2954.0 3.51 10.81
13.43 237.69 .226E+07 3160.3 3.18 11.93
14.29 238.55 .271E+07 3869.4 3.26 11.67
15.16 239.42 .320E+07 4740.3 3.38 11.24
16.02 240.28 .373E+07 5688.4 3.48 10.92

```

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8358) 968.87 13.14 7.50 34.00 1.34 1.08
OUTFLOW : ID= 1 (6280) 968.87 12.85 8.00 34.00 1.32 1.06

```

```

-----
| ADD HYD (8354) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| (ha) (cms) (hrs) (mm)
| ID1= 1 (0280): 299.86 6.103 7.00 36.96
| + ID2= 2 (6280): 968.87 12.849 8.00 34.00
|-----|
| ID = 3 (8354): 1268.73 18.341 7.75 34.70

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0272) | Area (ha)= 157.38 Curve Number (CN)= 75.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.09

```

```

Unit Hyd Qpeak (cms)= 2.456

PEAK FLOW (cms)= 2.090 (1)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 30.086
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.412

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0270) | Area (ha)= 243.61 Curve Number (CN)= 81.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
|-----|
| U.H. Tp(hrs)= 0.87

```

```

Unit Hyd Qpeak (cms)= 3.429

PEAK FLOW (cms)= 3.490 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 35.833
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.490

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0268) | Area (ha)= 215.76 Curve Number (CN)= 75.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
|-----|
| U.H. Tp(hrs)= 0.69

```

Unit Hyd Qpeak (cms)= 3.807

PEAK FLOW (cms)= 3.054 (1)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 29.811
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.408

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHYD (0264) | Area (ha)= 353.96 Curve Number (CN)= 69.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.30

Unit Hyd Qpeak (cms)= 3.313

PEAK FLOW (cms)= 2.544 (1)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 25.249
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.345

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHYD (0266) | Area (ha)= 508.09 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.63

Unit Hyd Qpeak (cms)= 3.786

PEAK FLOW (cms)= 2.622 (1)
 TIME TO PEAK (hrs)= 9.250
 RUNOFF VOLUME (mm)= 21.852
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.299

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8380)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID= 1 (0264):	353.96	2.544	8.25	25.25
+ ID2= 2 (0266):	508.09	2.622	9.25	21.85
=====				
ID = 3 (8380):	862.05	5.142	8.50	23.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6268) |
 IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (2681.0) -----

Distance	Elevation	Manning
0.00	274.03	0.0500
10.55	272.73	0.0500
21.10	271.28	0.0500
50.12	266.83	0.0500
87.04	260.36	0.0500
92.32	260.06	0.0500
94.95	259.93	0.0500
101.00	256.87	0.0500
103.00	256.58	0.0500 / 0.0350 Main Channel
104.00	256.32	0.0350 Main Channel
106.00	256.05	0.0350 Main Channel
108.00	256.25	0.0350 Main Channel
110.78	256.65	0.0350 / 0.0500 Main Channel
113.42	256.81	0.0500
116.06	257.00	0.0500
155.62	260.24	0.0500
187.27	263.35	0.0500
211.01	267.22	0.0500
224.20	269.60	0.0500
261.13	272.68	0.0500

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.53	256.58	.697E+04	2.1	0.97	55.64
1.38	257.43	.477E+05	25.9	1.76	30.66
2.22	258.27	.123E+06	84.3	2.22	24.26
3.07	259.12	.231E+06	185.4	2.60	20.72
3.92	259.97	.371E+06	335.8	2.92	18.42
4.77	260.82	.560E+06	547.6	3.15	17.06
5.61	261.66	.790E+06	845.6	3.46	15.57
6.46	262.51	.106E+07	1225.0	3.75	14.37

7.31	263.36	.136E+07	1693.3	4.02	13.37
8.16	264.21	.169E+07	2283.2	4.35	12.36
9.00	265.05	.206E+07	2969.1	4.66	11.54
9.85	265.90	.245E+07	3754.4	4.96	10.86
10.70	266.75	.286E+07	4643.0	5.24	10.28
11.55	267.60	.331E+07	5636.1	5.50	9.78
12.39	268.44	.378E+07	6746.2	5.76	9.34
13.24	269.29	.428E+07	7972.2	6.01	8.96
14.09	270.14	.482E+07	9318.9	6.18	8.71
14.94	270.99	.539E+07	10750.5	6.32	8.52
15.78	271.83	.601E+07	12204.7	6.46	8.33

----- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8380) 862.05 5.14 8.50 23.25 0.64 1.03
 OUTFLOW: ID= 1 (6268) 862.05 5.00 10.00 23.25 0.63 1.02

ADD HYD (8382) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID= 1 (0268): 215.76 3.054 7.00 29.81
 + ID2= 2 (6268): 862.05 5.000 10.00 23.25
 =====
 ID = 3 (8382): 1077.81 7.285 8.75 24.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6270) |
 IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (2701.0) -----

Distance	Elevation	Manning
0.00	245.98	0.0500
8.14	245.66	0.0500
16.28	245.16	0.0500
20.35	244.84	0.0500
38.66	242.98	0.0500
48.83	240.65	0.0500
63.07	235.91	0.0500
65.11	235.18	0.0500
68.43	234.34	0.0500 / 0.0300 Main Channel
68.68	233.95	0.0300 Main Channel
69.18	233.89	0.0300 Main Channel
69.68	233.95	0.0300 Main Channel
71.21	234.48	0.0300 / 0.0500 Main Channel
81.38	236.44	0.0500
95.63	236.66	0.0500
120.04	237.00	0.0500
148.53	241.77	0.0500
158.70	242.34	0.0500
187.18	244.03	0.0500
201.42	244.36	0.0500

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.45	234.34	.217E+04	0.7	0.99	52.83
0.98	234.87	.966E+04	4.6	1.49	35.20
1.50	235.39	.250E+05	13.7	1.73	30.41
2.03	235.92	.475E+05	29.5	1.95	26.87
2.56	236.45	.772E+05	52.5	2.14	24.50
3.09	236.98	.142E+06	82.4	1.83	28.73
3.61	237.50	.246E+06	158.1	2.03	25.91
4.14	238.03	.357E+06	262.0	2.31	22.73
4.67	238.56	.477E+06	392.1	2.59	20.27
6.78	240.67	.103E+07	1169.7	2.86	18.38
7.31	241.20	.119E+07	1425.9	3.77	13.94
7.83	241.72	.136E+07	1710.4	3.96	13.26
8.36	242.25	.154E+07	1967.9	4.02	13.07
8.89	242.78	.174E+07	2264.9	4.09	12.83
9.42	243.31	.196E+07	2589.3	4.15	12.65
9.94	243.83	.221E+07	2953.5	4.22	12.46

----- hydrograph ----> <-pipe / channel->

AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8382) 1077.81 7.29 8.75 24.56 1.13 1.56
 OUTFLOW: ID= 1 (6270) 1077.81 7.18 9.25 24.56 1.13 1.55

ADD HYD (8384) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.

```

-----
      (ha)  (cms)  (hrs)  (mm)
ID1= 1 (0270): 243.61 3.490 7.25 35.83
+ ID2= 2 (6270): 1077.81 7.180 9.25 24.56
=====
ID = 3 (8384): 1321.42 10.209 8.75 26.64

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0262) | Area (ha)= 341.31 Curve Number (CN)= 82.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
| U.H. Tp(hrs)= 1.01

```

Unit Hyd Qpeak (cms)= 4.094

```

PEAK FLOW (cms)= 4.455 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 37.033
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.507

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0260) | Area (ha)= 476.24 Curve Number (CN)= 82.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
| U.H. Tp(hrs)= 1.33

```

Unit Hyd Qpeak (cms)= 4.360

```

PEAK FLOW (cms)= 5.038 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 37.154
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.508

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0258) | Area (ha)= 181.99 Curve Number (CN)= 79.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
| U.H. Tp(hrs)= 1.18

```

Unit Hyd Qpeak (cms)= 1.881

```

PEAK FLOW (cms)= 1.930 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 33.887
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.464

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0252) | Area (ha)= 319.99 Curve Number (CN)= 73.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
| U.H. Tp(hrs)= 1.04

```

Unit Hyd Qpeak (cms)= 3.761

```

PEAK FLOW (cms)= 3.102 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 28.314
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.387

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0256) | Area (ha)= 145.79 Curve Number (CN)= 67.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
| U.H. Tp(hrs)= 1.04

```

Unit Hyd Qpeak (cms)= 1.707

```

PEAK FLOW (cms)= 1.174 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 23.749
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.325

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0254) | Area (ha)= 403.00 Curve Number (CN)= 55.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
| U.H. Tp(hrs)= 1.22

```

Unit Hyd Qpeak (cms)= 4.028

```

PEAK FLOW (cms)= 1.990 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 16.662
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.228

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ROUTE CHN (6256) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00

```

<----- DATA FOR SECTION (2561.0) ----->

Distance	Elevation	Manning	
0.00	276.07	0.0400	
11.68	273.71	0.0400	
23.36	271.35	0.0400	
29.19	270.30	0.0400	
35.03	269.44	0.0400	
55.47	267.90	0.0400	
78.82	266.24	0.0400	
90.50	265.63	0.0400	
102.18	265.40	0.0400	
105.10	264.95	0.0400 / 0.0350	Main Channel
108.02	264.39	0.0350	Main Channel
110.94	264.72	0.0350	Main Channel
113.86	265.19	0.0350 / 0.0400	Main Channel
116.78	265.49	0.0400	
143.05	268.24	0.0400	
172.25	270.53	0.0400	
207.28	271.95	0.0400	
233.55	273.72	0.0400	
256.91	274.98	0.0400	
289.02	275.97	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.56	264.95	.723E+04	1.9	0.90	62.51
1.14	265.53	.307E+05	13.5	1.49	37.92
1.72	266.11	.951E+05	48.6	1.73	32.62
2.30	266.69	.191E+06	117.8	2.09	27.03
2.88	267.27	.334E+06	225.2	2.43	23.24
3.46	267.85	.464E+06	375.5	2.74	20.59
4.04	268.43	.641E+06	573.2	3.03	18.62
4.62	269.01	.846E+06	821.9	3.29	17.15
5.20	269.59	.108E+07	1135.0	3.56	15.87
5.78	270.17	.134E+07	1525.1	3.86	14.64
6.36	270.75	.162E+07	1971.1	4.12	13.71
6.94	271.33	.194E+07	2474.0	4.33	13.04
7.52	271.91	.228E+07	3065.0	4.55	12.42
8.10	272.49	.266E+07	3773.4	4.80	11.75
8.68	273.07	.308E+07	4566.2	5.05	11.17
9.26	273.65	.348E+07	5443.7	5.30	10.66
9.84	274.23	.393E+07	6389.6	5.51	10.25
10.42	274.81	.440E+07	7426.2	5.72	9.88
11.00	275.39	.490E+07	8490.1	5.86	9.63

```

-----
| hydrograph |
| AREA | QPEAK | TPEAK | R.V. | MAX DEPTH | MAX VEL |
| (ha) | (cms) | (hrs) | (mm) | (m) | (m/s) |
INFLOW: ID= 2 (0254) 403.00 1.99 8.00 16.66 0.56 0.90
OUTFLOW: ID= 1 (6256) 403.00 1.87 9.75 16.66 0.54 0.90

```

```

-----
| ADD HYD (8370) |
| 1 + 2 = 3 |
| ID= 1 (0256): 145.79 1.174 8.00 23.75
| + ID2= 2 (6256): 403.00 1.869 9.75 16.66
| ID = 3 (8370): 548.79 2.934 9.25 18.54

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0250) | Area (ha)= 192.88 Curve Number (CN)= 70.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
| U.H. Tp(hrs)= 1.22

```

Unit Hyd Qpeak (cms)= 1.930

PEAK FLOW (cms)= 1.508 (1)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 25.980
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.355

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0246) | Area (ha)= 759.61 | Curve Number (CN)= 55.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
 | U.N. Tp(hrs)= 1.81

Unit Hyd Opeak (cms)= 5.121

PEAK FLOW (cms)= 2.778 (1)
 TIME TO PEAK (hrs)= 10.000
 RUNOFF VOLUME (mm)= 16.720
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.229

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0248) | Area (ha)= 146.04 | Curve Number (CN)= 64.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
 | U.N. Tp(hrs)= 0.78

Unit Hyd Opeak (cms)= 2.271

PEAK FLOW (cms)= 1.326 (1)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 21.646
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.296

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8364) |
 | 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0246): 759.61 2.778 10.00 16.72
 + ID2= 2 (0248): 146.04 1.326 7.25 21.65

 ID = 3 (8364): 905.65 3.933 8.25 17.51

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6250) | Routing time step (min)'= 15.00
 | IN= 2--> OUT= 1 |

<----- DATA FOR SECTION (2501.0) ----->

Distance	Elevation	Manning	
0.00	269.96	0.0500	
8.56	268.55	0.0500	
17.13	266.91	0.0500	
21.41	266.13	0.0500	
40.68	263.15	0.0500	
62.09	260.75	0.0500	
85.64	258.02	0.0500	
88.20	257.69	0.0500	
93.20	257.05	0.0500 /0.0350	Main Channel
93.45	256.88	0.0350	Main Channel
94.20	256.56	0.0350	Main Channel
94.95	256.83	0.0350	Main Channel
95.20	257.08	0.0350 /0.0500	Main Channel
100.62	257.45	0.0500	
115.61	258.57	0.0500	
139.16	260.43	0.0500	
152.01	261.95	0.0500	
171.27	264.63	0.0500	
188.40	267.90	0.0500	
211.95	274.18	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.49	257.05	.203E+04	0.4	0.68	89.65
1.17	257.73	.248E+05	5.7	0.85	72.16
1.85	258.41	.841E+05	25.2	1.10	55.60
2.53	259.09	.180E+06	66.5	1.36	45.14
3.21	259.77	.312E+06	135.7	1.60	38.36
3.89	260.45	.481E+06	238.3	1.82	33.61
4.57	261.13	.681E+06	383.5	2.07	29.60
5.25	261.81	.911E+06	568.9	2.29	26.70
5.93	262.49	.117E+07	800.5	2.51	24.36

6.61	263.17	.148E+07	1079.0	2.72	22.49
7.28	263.84	.177E+07	1413.4	2.94	20.85
7.96	264.52	.210E+07	1797.0	3.14	19.50
8.64	265.20	.246E+07	2241.6	3.35	18.28
9.32	265.88	.283E+07	2739.9	3.55	17.25
10.00	266.56	.323E+07	3295.7	3.75	16.34
10.68	267.24	.364E+07	3908.9	3.94	15.54
11.36	267.92	.408E+07	4578.5	4.12	14.84
12.04	268.60	.453E+07	5320.1	4.32	14.18
12.72	269.28	.499E+07	6109.1	4.50	13.61

<----- hydrograph ----> <-pipe / channel-->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8364) 905.65 3.93 8.25 17.51 0.94 0.78
 OUTFLOW : ID= 1 (6250) 905.65 3.66 10.25 17.51 0.90 0.77

ADD HYD (8366) |
 | 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0250): 192.88 1.508 8.00 25.98
 + ID2= 2 (6250): 905.65 3.662 10.25 17.51

 ID = 3 (8366): 1098.53 5.036 10.00 19.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8368) |
 | 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8366): 1098.53 5.036 10.00 19.00
 + ID2= 2 (8370): 548.79 2.934 9.25 18.54

 ID = 3 (8368): 1647.32 7.940 9.75 18.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8372) |
 | 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0250): 319.99 3.102 8.00 28.31
 + ID2= 2 (8368): 1647.32 7.940 9.75 18.85

 ID = 3 (8372): 1967.31 10.760 9.00 20.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6258) | Routing time step (min)'= 15.00
 | IN= 2--> OUT= 1 |

<----- DATA FOR SECTION (2581.0) ----->

Distance	Elevation	Manning	
0.00	252.88	0.0500	
15.47	251.89	0.0500	
46.41	248.45	0.0500	
73.48	245.81	0.0500	
88.95	243.75	0.0500	
112.15	242.00	0.0500	
135.35	240.23	0.0500	
162.42	239.76	0.0500	
170.97	239.52	0.0500 /0.0350	Main Channel
171.58	239.03	0.0350	Main Channel
174.02	239.03	0.0350	Main Channel
176.46	239.03	0.0350	Main Channel
177.07	239.52	0.0350 /0.0500	Main Channel
185.63	239.67	0.0500	
208.83	239.87	0.0500	
239.77	240.14	0.0500	
274.57	244.93	0.0500	
336.45	249.51	0.0500	
363.52	249.77	0.0500	
382.85	251.78	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.49	239.52	.135E+05	2.5	0.92	91.32
1.14	240.17	.175E+06	25.6	0.73	114.22
1.78	240.81	.532E+06	123.5	1.17	71.85
2.43	241.46	.832E+06	285.3	1.54	54.48
3.07	242.10	.138E+07	506.5	1.85	45.25
3.72	242.75	.186E+07	786.8	2.12	39.42
4.36	243.39	.239E+07	1127.3	2.37	35.33

5.01	244.04	.298E+07	1535.8	2.61	32.12
5.65	244.68	.358E+07	2014.3	2.84	29.48
6.30	245.33	.420E+07	2535.9	3.03	27.60
6.94	245.97	.488E+07	3108.4	3.20	26.16
7.59	246.62	.563E+07	3745.6	3.35	24.98
8.23	247.26	.638E+07	4446.7	3.50	23.93
8.88	247.91	.721E+07	5229.7	3.64	22.98
9.52	248.55	.809E+07	6094.3	3.78	22.12
10.17	249.20	.903E+07	7050.4	3.93	21.31
10.81	249.84	.100E+08	7773.0	3.90	21.46
11.46	250.49	.111E+08	8998.8	4.07	20.56
12.10	251.13	.122E+08	10316.2	4.24	19.75

408.40	222.65	0.0450
460.17	223.20	0.0450
517.69	224.84	0.0450
569.46	232.57	0.0450

```

----- TRAVEL TIME TABLE -----
DEPTH  ELEV  VOLUME  FLOW RATE  VELOCITY  TRAV.TIME
(m)      (m)      (cu.m.)  (cms)      (m/s)     (min)
0.41  220.57  .189E+04  0.5        0.49      60.18
0.83  220.99  .697E+04  3.2        0.82      35.79
1.24  221.40  .145E+05  8.8        1.07      27.45
1.84  222.00  .643E+05  29.3       0.81      36.61
2.44  222.60  .282E+06  132.9      0.83      35.41
3.04  223.20  .707E+06  413.7      1.04      28.50
3.64  223.80  .118E+07  876.8      1.31      22.46
4.24  224.40  .168E+07  1494.8     1.57      18.75
4.84  225.00  .221E+07  2264.1     1.82      16.23
5.44  225.60  .274E+07  3196.3     2.06      14.29
6.04  226.20  .328E+07  4258.6     2.30      12.84
6.65  226.81  .383E+07  5444.7     2.52      11.72
7.25  227.41  .438E+07  6749.7     2.73      10.82
7.85  228.01  .494E+07  8169.4     2.93      10.08
8.45  228.61  .551E+07  9700.7     3.12      9.46
9.05  229.21  .608E+07  11345.8    3.30      8.94
9.65  229.81  .666E+07  13087.4    3.48      8.48
10.25  230.41  .724E+07  14938.5    3.65      8.08
10.85  231.01  .783E+07  16892.4    3.82      7.73

```

```

<---- hydrograph ----> <-pipe / channel->
AREA  QPEAK  TPEAK  R.V.  MAX DEPTH  MAX VEL
(ha)  (cms)  (hrs)  (mm)  (m)        (m/s)
INFLOW : ID= 2 (8372) 1967.31  10.76  9.00  20.39  0.72  0.84
OUTFLOW : ID= 1 (6258) 1967.31  9.73  11.25  20.39  0.69  0.85

```

```

-----
| ADD HYD (8374) |
| 1 + 2 = 3 |
AREA  QPEAK  TPEAK  R.V.
(ha)  (cms)  (hrs)  (mm)
ID1= 1 (0258): 181.99  1.930  8.00  33.89
+ ID2= 2 (6258): 1967.31  9.734  11.25  20.39
=====
ID = 3 (8374): 2149.30  11.262  11.00  21.53

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

----- TRAVEL TIME TABLE -----
AREA  QPEAK  TPEAK  R.V.  MAX DEPTH  MAX VEL
(ha)  (cms)  (hrs)  (mm)  (m)        (m/s)
INFLOW : ID= 2 (8362) 4288.27  29.33  9.50  26.07  1.84  0.81
OUTFLOW : ID= 1 (6272) 4288.27  28.76  10.25  26.07  1.83  0.81

```

```

-----
| ADD HYD (8376) |
| 1 + 2 = 3 |
AREA  QPEAK  TPEAK  R.V.
(ha)  (cms)  (hrs)  (mm)
ID1= 1 (0260): 476.24  5.038  8.00  37.15
+ ID2= 2 (8374): 2149.30  11.262  11.00  21.53
=====
ID = 3 (8376): 2625.54  15.711  10.25  24.37

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8356) |
| 1 + 2 = 3 |
AREA  QPEAK  TPEAK  R.V.
(ha)  (cms)  (hrs)  (mm)
ID1= 1 (0272): 157.38  2.090  7.50  30.09
+ ID2= 2 (6272): 4288.27  28.759  10.25  26.07
=====
ID = 3 (8356): 4445.65  30.243  10.00  26.22

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8378) |
| 1 + 2 = 3 |
AREA  QPEAK  TPEAK  R.V.
(ha)  (cms)  (hrs)  (mm)
ID1= 1 (0262): 341.31  4.455  7.75  37.03
+ ID2= 2 (8376): 2625.54  15.711  10.25  24.37
=====
ID = 3 (8378): 2966.85  19.393  10.00  25.82

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8352) |
| 1 + 2 = 3 |
AREA  QPEAK  TPEAK  R.V.
(ha)  (cms)  (hrs)  (mm)
ID1= 1 (8354): 1268.73  18.341  7.75  34.70
+ ID2= 2 (8356): 4445.65  30.243  10.00  26.22
=====
ID = 3 (8352): 5714.38  45.383  8.75  28.10

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8362) |
| 1 + 2 = 3 |
AREA  QPEAK  TPEAK  R.V.
(ha)  (cms)  (hrs)  (mm)
ID1= 1 (8378): 2966.85  19.393  10.00  25.82
+ ID2= 2 (8384): 1321.42  10.209  8.75  26.64
=====
ID = 3 (8362): 4288.27  29.326  9.50  26.07

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6304) |
| IN= 2--> OUT= 1 |
Routing time step (min)= 15.00

```

----- DATA FOR SECTION (3041.0) -----

Distance	Elevation	Manning
0.00	232.08	0.0500
19.00	231.87	0.0500
38.00	231.33	0.0500
66.51	230.44	0.0500
104.51	228.25	0.0500
118.76	225.17	0.0500
128.26	219.86	0.0500
175.77	219.17	0.0500
185.27	218.90	0.0500 / 0.0300
185.52	218.65	0.0300
190.02	218.37	0.0300
194.52	218.60	0.0300
194.77	218.85	0.0300 / 0.0500
204.27	219.60	0.0500
299.28	220.91	0.0500
327.78	222.36	0.0500
375.28	225.71	0.0500
403.79	229.37	0.0500
432.29	230.43	0.0500
470.29	232.00	0.0500

```

-----
| ROUTE CHN (6272) |
| IN= 2--> OUT= 1 |
Routing time step (min)= 15.00

```

----- DATA FOR SECTION (2721.0) -----

Distance	Elevation	Manning
0.00	231.01	0.0450
23.01	223.65	0.0450
34.51	222.46	0.0450
51.77	222.11	0.0450
69.02	221.87	0.0450
161.06	221.92	0.0450
166.81	221.91	0.0450
172.56	221.89	0.0450
180.57	221.40	0.0450 / 0.0300
181.57	220.60	0.0300
184.07	220.16	0.0300
195.57	221.85	0.0300 / 0.0450
201.32	221.82	0.0450
207.07	221.77	0.0450
212.83	221.72	0.0450
253.09	222.52	0.0450

```

----- TRAVEL TIME TABLE -----
DEPTH  ELEV  VOLUME  FLOW RATE  VELOCITY  TRAV.TIME
(m)      (m)      (cu.m.)  (cms)      (m/s)     (min)
0.48  218.85  .121E+05  1.2        0.36      173.94
1.17  219.54  .839E+05  9.8        0.44      142.60
1.86  220.23  .323E+06  41.0       0.47      131.43

```

2.56	220.93	.708E+06	109.5	0.58	107.67
3.25	221.62	.118E+07	224.3	0.71	87.36
3.94	222.31	.168E+07	376.5	0.84	74.48
4.63	223.00	.222E+07	567.9	0.96	65.26
5.32	223.69	.279E+07	794.7	1.07	58.58
6.02	224.39	.339E+07	1056.2	1.17	53.52
6.71	225.08	.402E+07	1352.4	1.26	49.52
7.40	225.77	.468E+07	1677.3	1.34	46.46
8.09	226.46	.536E+07	2050.0	1.43	43.58
8.79	227.16	.607E+07	2457.9	1.52	41.14
9.48	227.85	.680E+07	2901.0	1.60	39.05
10.17	228.54	.755E+07	3352.7	1.66	37.53
10.86	229.23	.834E+07	3812.5	1.71	36.48
11.55	229.92	.919E+07	4241.8	1.73	36.13
12.25	230.62	.101E+08	4697.0	1.74	35.93
12.94	231.31	.111E+08	5186.3	1.74	35.83

<---- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8352) 5714.38 45.38 8.75 28.10 1.31 0.48
 OUTFLOW: ID= 1 (6304) 5714.38 37.83 11.50 28.10 1.80 0.47

ADD HYD (8350)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0304): 292.37 1.311 10.25 21.30
 + ID2= 2 (6304): 5714.38 37.830 11.50 28.10
 ID = 3 (8350): 6006.75 39.122 11.25 27.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8346)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8348): 3768.46 28.649 9.75 25.66
 + ID2= 2 (8350): 6006.75 39.122 11.25 27.77
 ID = 3 (8346): 9775.21 66.196 10.75 26.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8344)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0306): 283.97 0.445 13.75 15.32
 + ID2= 2 (8346): 9775.21 66.196 10.75 26.96
 ID = 3 (8344): 10059.18 66.598 10.75 26.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5509)
 IN= 2 --> OUT= 1
 DT= 15.0 min
 OUTFLOW STORAGE OUTFLOW STORAGE
 (cms) (ha.m.) (cms) (ha.m.)
 0.0000 0.0000 ***** 370.0451
 41.0590 56.7403 ***** 863.4386
 48.1390 86.3439 ***** *****
 56.6340 ***** *****
 67.9600 ***** 0.0000 0.0000

AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8344) ***** 66.598 10.75 26.63
 OUTFLOW: ID= 1 (5509) ***** 45.744 15.25 26.63

PEAK FLOW REDUCTION [Qout/Qin](%)= 68.69
 TIME SHIFT OF PEAK FLOW (min)=270.00
 MAXIMUM STORAGE USED (ha.m.)= 76.3485

CALIB (0332) Area (ha)= 393.44 Curve Number (CN)= 75.0
 NASHYD (0332) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 2.32

Unit Hyd Qpeak (cms)= 2.894

PEAK FLOW (cms)= 2.902 (1)
 TIME TO PEAK (hrs)= 9.750

RUNOFF VOLUME (mm)= 30.269
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.414

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0330) Area (ha)= 468.30 Curve Number (CN)= 80.0
 NASHYD (0330) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.09

Unit Hyd Qpeak (cms)= 7.335

PEAK FLOW (cms)= 7.311 (1)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 34.924
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.478

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6332) Routing time step (min)= 15.00
 IN= 2 --> OUT= 1

----- DATA FOR SECTION (3321.0) -----

Distance	Elevation	Manning
0.00	234.00	0.0380
25.85	227.20	0.0380
96.94	226.44	0.0380
168.03	227.38	0.0380
219.73	225.62	0.0380
342.52	221.57	0.0380
368.37	221.42	0.0380
374.83	221.23	0.0380
379.79	220.98	0.0380 / 0.0300
380.29	220.47	0.0300
381.29	220.47	0.0300
382.79	220.47	0.0300
383.29	220.98	0.0300 / 0.0380
394.22	221.22	0.0380
400.68	221.33	0.0380
407.14	221.44	0.0380
491.16	225.70	0.0380
568.71	227.55	0.0380
607.49	230.14	0.0380
639.80	234.08	0.0380

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.51	220.98	.554E+04	0.9	0.56	107.27
1.20	221.67	.885E+05	12.1	0.49	122.32
1.88	222.35	.309E+06	64.7	0.76	79.70
2.57	223.04	.615E+06	165.4	0.97	61.99
3.25	223.72	.101E+07	322.1	1.16	52.06
3.94	224.41	.148E+07	542.9	1.33	45.51
4.62	225.09	.204E+07	835.4	1.48	40.77
5.31	225.78	.269E+07	1202.8	1.62	37.27
5.99	226.46	.344E+07	1618.0	1.70	35.46
6.68	227.15	.447E+07	1932.0	1.56	38.56
7.36	227.83	.580E+07	2763.9	1.72	34.96
8.05	228.52	.718E+07	3861.9	1.95	30.97
8.73	229.20	.859E+07	5115.4	2.16	27.98
9.42	229.89	.100E+08	6519.2	2.35	25.64
10.10	230.57	.115E+08	8092.3	2.55	23.69
10.79	231.26	.130E+08	9821.5	2.73	22.05
11.47	231.94	.145E+08	11690.0	2.92	20.68
12.16	232.63	.160E+08	13694.9	3.09	19.52
12.84	233.31	.176E+08	15833.9	3.26	18.52

<---- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0330) 468.30 7.31 7.50 34.92 0.90 0.52
 OUTFLOW: ID= 1 (6332) 468.30 5.57 9.75 34.92 0.80 0.53

ADD HYD (8322) AREA QPEAK TPEAK R.V.
 1 + 2 = 3 (ha) (cms) (hrs) (mm)
 ID1= 1 (0332): 393.44 2.902 9.75 30.27
 + ID2= 2 (6332): 468.30 5.567 9.75 34.92
 ID = 3 (8322): 861.74 8.469 9.75 32.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0328) | Area (ha)= 492.92 Curve Number (CN)= 77.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.91
|
Unit Hyd Qpeak (cms)= 4.411
|
PEAK FLOW (cms)= 4.508 (i)
TIME TO PEAK (hrs)= 8.750
RUNOFF VOLUME (mm)= 32.087
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.439

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0326) | Area (ha)= 679.91 Curve Number (CN)= 80.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.95
|
Unit Hyd Qpeak (cms)= 5.941
|
PEAK FLOW (cms)= 6.697 (i)
TIME TO PEAK (hrs)= 8.750
RUNOFF VOLUME (mm)= 35.107
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.480

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0324) | Area (ha)= 615.64 Curve Number (CN)= 79.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.90
|
Unit Hyd Qpeak (cms)= 5.544
|
PEAK FLOW (cms)= 6.021 (i)
TIME TO PEAK (hrs)= 8.750
RUNOFF VOLUME (mm)= 34.061
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.466

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0320) | Area (ha)= 278.74 Curve Number (CN)= 81.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.36
|
Unit Hyd Qpeak (cms)= 3.499
|
PEAK FLOW (cms)= 3.792 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 36.087
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.494

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (3182) | Area (ha)= 457.40 Curve Number (CN)= 78.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.49
|
Unit Hyd Qpeak (cms)= 5.259
|
PEAK FLOW (cms)= 5.284 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 32.999
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.451

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANBYD (3181) | Area (ha)= 26.00
| ID= 1 DT=15.0 min | Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00
|-----|
|
Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 2.00 4.94
Average Slope (%)= 0.50 0.50
Length (m)= 416.33 40.00

```

```

Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 96.49 50.01
over (min) 15.00 30.00
Storage Coeff (min)= 7.51 (ii) 21.62 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.10 0.05
*TOTALS*
PEAK FLOW (cms)= 5.13 0.33 5.342 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 71.10 32.21 63.71
TOTAL RAINFALL (mm)= 73.10 73.10 73.10
RUNOFF COEFFICIENT = 0.97 0.44 0.87

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
- CN* = 77.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| RESERVOIR (5318) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
|-----|
| OUTFLOW STORAGE | OUTFLOW STORAGE |
| (cms) (ha.m.) | (cms) (ha.m.) |
| 0.0000 0.0000 | 3.3800 0.7654 |
| 0.0510 0.4423 | 4.1500 0.8231 |
| 0.8700 0.5378 | 4.8750 0.8778 |
| 1.9590 0.6105 | 5.2750 0.8878 |
| 2.5620 0.6796 | 0.0000 0.0000 |

```

```

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID= 2 (3181) 26.000 5.342 6.00 63.71
OUTFLOW: ID= 1 (5318) 26.000 2.943 6.25 63.63

```

```

PEAK FLOW REDUCTION [Qout/ Qin](%)= 55.08
TIME SHIFT OF PEAK FLOW (min)= 15.00
MAXIMUM STORAGE USED (ha.m.)= 0.7756

```

```

-----
| ADD HYD (8334) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (3182): 457.40 5.284 8.00 33.00 |
| + ID2= 2 (5318): 26.00 2.943 6.25 63.63 |
|=====|
| ID = 3 (8334): 483.40 6.378 6.25 34.65 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6320) |
| IN= 2--> OUT= 1 |
|-----|
Routing time step (min)= 15.00

```

```

-----
|-----> DATA FOR SECTION (3201.0) <-----|
Distance Elevation Manning
0.00 249.00 0.0380
22.92 245.86 0.0380
45.83 244.87 0.0380
91.66 243.11 0.0380
126.03 239.53 0.0380
160.41 237.17 0.0380
166.14 237.06 0.0380
177.59 237.13 0.0380
183.32 237.20 0.0380
189.05 236.70 0.0380 /0.0350 Main Channel
193.05 235.89 0.0350 Main Channel
197.05 236.64 0.0350 /0.0380 Main Channel
200.51 236.74 0.0380
206.24 237.03 0.0380
246.34 238.82 0.0380
263.53 243.87 0.0380
389.56 247.64 0.0380
452.58 247.74 0.0380
498.41 248.60 0.0380
567.16 249.84 0.0380

```

```

-----
|-----> TRAVEL TIME TABLE <-----|
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.37 236.26 .270E+04 0.4 0.58 107.96
0.75 236.64 .108E+05 2.6 0.92 68.01
1.48 237.37 .877E+05 27.4 1.17 53.35
2.20 238.09 .277E+06 122.1 1.65 37.83
2.93 238.82 .540E+06 295.7 2.08 30.01
3.66 239.55 .857E+06 587.8 2.57 24.29
4.39 240.28 .120E+07 977.8 3.04 20.52
5.11 241.00 .158E+07 1456.1 3.46 18.02

```

5.84	241.73	.198E+07	2028.6	3.84	16.23
6.57	242.46	.240E+07	6990.5	4.19	14.86
7.29	243.18	.285E+07	3431.8	4.50	13.84
8.02	243.91	.335E+07	4149.1	4.64	13.44
8.75	244.64	.393E+07	4745.6	4.51	13.80
9.47	245.36	.463E+07	5618.6	4.53	13.75
10.20	246.09	.545E+07	6781.8	4.66	13.39
10.93	246.82	.635E+07	8274.5	4.87	12.79
11.66	247.55	.733E+07	9989.0	5.10	12.23
12.38	248.27	.853E+07	11422.0	5.01	12.45
13.11	249.00	.989E+07	13749.7	5.20	11.98

```

<--- hydrograph ---> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW: ID= 2 (8334) 483.40 6.38 6.25 34.65 0.86 0.95
OUTFLOW: ID= 1 (8320) 483.40 5.21 9.00 34.65 0.82 0.94

```

```

ADD HYD (8332)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0320): 278.74 3.792 8.00 36.09
+ ID2= 2 (6320): 483.40 5.207 9.00 34.65
=====
ID = 3 (8332): 762.14 8.824 8.50 35.17

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB (0314)
NASHYD (0314) Area (ha)= 165.20 Curve Number (CN)= 78.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.71

```

```

Unit Hyd Qpeak (cms)= 3.978
PEAK FLOW (cms)= 3.404 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 32.624
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.446

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ROUTE CHN (6316)
IN= 2---> OUT= 1 Routing time step (min)"= 15.00

```

```

<----- DATA FOR SECTION (3161.0) ----->
Distance Elevation Manning
0.00 248.54 0.0350
205.48 236.14 0.0350 /0.0310 Main Channel
205.98 235.61 0.0310 Main Channel
207.98 235.25 0.0310 Main Channel
209.98 235.53 0.0310 Main Channel
210.48 236.00 0.0310 /0.0350 Main Channel
216.42 236.73 0.0350
258.31 239.09 0.0350
328.12 239.84 0.0350
439.83 241.47 0.0350
530.58 242.08 0.0350
586.43 242.93 0.0350
691.16 248.00 0.0350

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.37 235.62 .215E+04 0.7 0.77 53.88
0.75 236.00 .634E+04 3.5 1.36 30.45
1.46 236.71 .297E+05 19.3 1.62 25.66
2.16 237.41 .103E+06 75.8 1.84 22.60
2.87 238.12 .229E+06 202.8 2.21 18.79
3.57 238.82 .407E+06 423.3 2.59 16.03
4.28 239.53 .654E+06 727.7 2.77 14.97
4.99 240.24 .103E+07 1242.0 3.01 13.76
5.69 240.94 .151E+07 2002.3 3.31 12.54
6.40 241.65 .210E+07 2987.9 3.55 11.70
7.10 242.35 .286E+07 4302.8 3.75 11.07
7.81 243.06 .374E+07 6165.3 4.10 10.11
8.51 243.76 .468E+07 8642.4 4.60 9.03
9.22 244.47 .565E+07 11496.3 5.06 8.20
9.93 245.18 .666E+07 14714.1 5.50 7.54

```

10.63	245.88	.770E+07	18290.3	5.91	7.02
11.34	246.59	.877E+07	22221.9	6.30	6.58
12.04	247.29	.988E+07	26464.4	6.67	6.22
12.75	248.00	.110E+08	31061.3	7.01	5.91

```

<--- hydrograph ---> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW: ID= 2 (0314) 165.20 3.40 6.75 32.62 0.74 1.34
OUTFLOW: ID= 1 (6316) 165.20 3.22 7.25 32.62 0.71 1.27

```

```

CALIB (0316)
NASHYD (0316) Area (ha)= 232.34 Curve Number (CN)= 82.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.01

```

```

Unit Hyd Qpeak (cms)= 3.923
PEAK FLOW (cms)= 4.118 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 37.068
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.507

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8338)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0316): 232.34 4.118 7.25 37.07
+ ID2= 2 (6316): 165.20 3.215 7.25 32.62
=====
ID = 3 (8338): 397.54 7.333 7.25 35.22

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB (0312)
NASHYD (0312) Area (ha)= 359.44 Curve Number (CN)= 80.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.36

```

```

Unit Hyd Qpeak (cms)= 4.529
PEAK FLOW (cms)= 4.750 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 35.011
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.479

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB (0308)
NASHYD (0308) Area (ha)= 529.30 Curve Number (CN)= 62.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.62

```

```

Unit Hyd Qpeak (cms)= 5.575
PEAK FLOW (cms)= 3.491 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 20.621
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.282

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB (0310)
NASHYD (0310) Area (ha)= 138.28 Curve Number (CN)= 65.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.76

```

```

Unit Hyd Qpeak (cms)= 3.102
PEAK FLOW (cms)= 1.788 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 22.291
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.305

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8342)					
1 + 2 = 3					
	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	(mm)
ID1= 1 (0308):	529.30	3.451	8.25	20.62	
+ ID2= 2 (0310):	138.28	1.788	7.00	22.29	
ID = 3 (8342):	667.58	5.034	7.75	20.97	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6312) |
IN= 2 -> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (3121.0)					
Distance	Elevation	Manning			
0.00	265.94	0.0360			
30.07	265.43	0.0360			
59.82	263.98	0.0360			
103.32	254.59	0.0360			
157.70	252.16	0.0360			
217.52	250.45	0.0360			
233.84	247.69	0.0360			
234.71	247.27	0.0360			
239.71	246.38	0.0360			
244.71	246.12	0.0360 / 0.0330	Main Channel		
247.71	245.17	0.0330	Main Channel		
249.71	245.19	0.0330	Main Channel		
251.71	245.64	0.0330 / 0.0360	Main Channel		
259.71	246.47	0.0360			
282.78	247.12	0.0360			
315.41	251.60	0.0360			
424.17	256.13	0.0360			
478.55	257.04	0.0360			
516.62	259.37	0.0360			
538.37	266.00	0.0360			

TRAVEL TIME TABLE						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.47	245.64	1.11E+04	3.1	1.78	6.07	
1.54	246.71	1.03E+05	53.9	3.39	3.19	
2.61	247.78	4.14E+05	281.3	4.40	2.45	
3.68	248.85	8.39E+05	749.1	5.79	1.87	
4.74	249.91	1.38E+06	1463.2	6.97	1.55	
5.81	250.98	2.01E+06	2354.8	7.61	1.42	
6.88	252.05	2.95E+06	3533.0	7.76	1.39	
7.95	253.12	4.07E+06	5303.9	8.05	1.34	
9.02	254.19	5.49E+06	7856.5	8.58	1.26	
10.09	255.26	7.92E+06	11449.2	9.37	1.15	
11.15	256.32	1.01E+07	15862.4	10.15	1.06	
12.22	257.39	1.27E+07	21074.5	10.75	1.01	
13.29	258.46	1.55E+07	28184.1	11.78	0.92	
14.36	259.53	1.85E+07	36386.2	12.77	0.85	
15.43	260.60	2.15E+07	46110.6	13.89	0.78	
16.50	261.67	2.46E+07	56922.5	14.95	0.72	
17.56	262.73	2.78E+07	68498.1	15.97	0.68	
18.63	263.80	3.10E+07	81118.8	16.94	0.64	
19.70	264.87	3.44E+07	93603.2	17.66	0.61	

<---- hydrograph ----> <-pipe / channel->						
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8342)	667.58	5.03	7.75	20.97	0.51	1.81
OUTFLOW : ID= 1 (6312)	667.58	5.03	8.00	20.97	0.51	1.81

ADD HYD (8340)					
1 + 2 = 3					
	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	(mm)
ID1= 1 (0312):	359.44	4.750	8.00	35.01	
+ ID2= 2 (6313):	667.58	5.033	8.00	20.97	
ID = 3 (8340):	1027.02	9.783	8.00	25.88	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8336)					
1 + 2 = 3					
	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	(mm)
ID1= 1 (8338):	397.54	7.333	7.25	35.22	
+ ID2= 2 (8340):	1027.02	9.783	8.00	25.88	
ID = 3 (8336):	1424.56	16.950	7.50	28.49	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8330)					
1 + 2 = 3					
	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	(mm)
ID1= 1 (8332):	762.14	8.824	8.50	35.17	
+ ID2= 2 (8336):	1424.56	16.950	7.50	28.49	
ID = 3 (8330):	2186.70	25.333	7.75	30.82	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6324) |
IN= 2 -> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (3241.0)					
Distance	Elevation	Manning			
0.00	234.38	0.0360			
33.57	232.48	0.0360			
67.14	230.14	0.0360			
83.93	228.80	0.0360			
134.29	227.62	0.0360			
209.82	225.10	0.0360			
218.21	224.86	0.0360			
226.61	224.47	0.0360			
234.00	223.86	0.0360 / 0.0300	Main Channel		
234.10	223.66	0.0300	Main Channel		
235.00	223.66	0.0300	Main Channel		
235.90	223.66	0.0300	Main Channel		
236.00	223.86	0.0300 / 0.0360	Main Channel		
243.39	224.92	0.0360			
251.78	224.89	0.0360			
335.71	225.69	0.0360			
562.32	226.53	0.0360			
637.85	228.36	0.0360			
705.00	229.80	0.0360			
830.89	234.00	0.0360			

TRAVEL TIME TABLE						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.20	223.86	1.45E+04	0.2	0.43	147.27	
0.73	224.39	1.59E+05	2.8	0.68	93.11	
1.27	224.93	5.61E+05	11.0	0.75	85.19	
1.80	225.46	2.17E+06	45.6	0.80	79.20	
2.33	225.99	5.66E+06	134.4	0.91	70.20	
2.87	226.53	1.22E+07	338.1	1.06	59.97	
3.40	227.06	2.06E+07	744.3	1.38	46.16	
3.94	227.60	2.98E+07	1295.9	1.66	38.38	
4.47	228.13	3.99E+07	1979.9	1.90	33.59	
5.00	228.66	5.09E+07	2807.5	2.11	30.21	
5.54	229.20	6.27E+07	3815.1	2.32	27.41	
6.07	229.73	7.53E+07	4984.1	2.53	25.17	
6.60	230.26	8.84E+07	6346.3	2.74	23.20	
7.14	230.80	1.02E+08	7863.2	2.95	21.61	
7.67	231.33	1.16E+08	9526.4	3.14	20.29	
8.21	231.87	1.31E+08	11342.2	3.32	19.18	
8.74	232.40	1.46E+08	13309.6	3.49	18.22	
9.27	232.93	1.61E+08	15414.0	3.66	17.41	
9.81	233.47	1.77E+08	17671.0	3.81	16.70	

<---- hydrograph ----> <-pipe / channel->						
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8330)	2186.70	25.33	7.75	30.82	1.49	0.77
OUTFLOW : ID= 1 (6324)	2186.70	21.73	9.50	30.82	1.43	0.76

ADD HYD (8328)					
1 + 2 = 3					
	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	(mm)
ID1= 1 (0324):	615.64	6.021	8.75	34.06	
+ ID2= 2 (6324):	2186.70	21.726	9.50	30.82	
ID = 3 (8328):	2802.34	27.648	9.50	31.53	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB |
NASHYD (0322) | Area (ha)= 513.13 Curve Number (CN)= 80.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.75

Unit Hyd Qpeak (cms)= 5.020
PEAK FLOW (cms)= 5.538 (1)
TIME TO PEAK (hrs)= 8.500

RUNOFF VOLUME (mm)= 35.083
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.480

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----
| ADD HYD (8326) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0326): 513.13 5.538 8.50 35.08 |
| + ID2= 2 (8328): 2802.34 27.648 9.50 31.53 |
|-----|
| ID = 3 (8326): 3315.47 32.995 9.50 32.08 |
-----
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
-----
| ADD HYD (8324) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0326): 678.91 6.697 8.75 35.11 |
| + ID2= 2 (8326): 3315.47 32.995 9.50 32.08 |
|-----|
| ID = 3 (8324): 3994.38 39.647 9.25 32.59 |
-----
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
-----
| ROUTE CHN (8328) |
| IN= 2--> OUT= 1 |
|-----|
| Routing time step (min)= 15.00 |
-----
```

```
-----
|----- DATA FOR SECTION (3281.0) -----|
| Distance Elevation Manning |
| 0.00 228.00 0.0380 |
| 18.58 224.97 0.0380 |
| 78.98 223.52 0.0380 |
| 125.44 223.28 0.0380 |
| 171.90 221.71 0.0380 |
| 213.72 219.65 0.0380 |
| 218.36 219.40 0.0380 |
| 223.01 219.19 0.0380 |
| 225.95 219.14 0.0380 /0.0320 Main Channel |
| 226.45 218.14 0.0320 Main Channel |
| 236.95 218.14 0.0320 Main Channel |
| 245.85 218.14 0.0320 Main Channel |
| 245.95 219.14 0.0320 /0.0380 Main Channel |
| 246.24 219.16 0.0380 |
| 250.88 219.24 0.0380 |
| 255.53 219.39 0.0380 |
| 325.22 221.47 0.0380 |
| 367.03 223.14 0.0380 |
| 404.20 225.17 0.0380 |
| 459.95 228.04 0.0380 |
-----
```

```
-----
|----- TRAVEL TIME TABLE -----|
| DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME |
| (m) (m) (cu.m.) (cms) (m/s) (min) |
| 0.50 218.64 .373E+05 3.2 0.33 192.37 |
| 1.00 219.14 .752E+05 10.1 0.51 124.71 |
| 1.52 219.66 .149E+06 22.2 0.57 112.06 |
| 2.04 220.18 .279E+06 43.4 0.59 107.06 |
| 2.56 220.70 .464E+06 77.0 0.63 100.52 |
| 3.08 221.22 .705E+06 125.7 0.68 93.56 |
| 3.61 221.75 .100E+07 192.7 0.73 86.62 |
| 4.13 222.27 .135E+07 278.1 0.79 80.92 |
| 4.65 222.79 .176E+07 385.9 0.84 75.86 |
| 5.17 223.31 .222E+07 514.9 0.89 71.82 |
| 5.69 223.83 .280E+07 662.7 0.90 70.40 |
| 6.21 224.35 .345E+07 867.7 0.96 66.35 |
| 6.73 224.87 .417E+07 1107.0 1.01 62.81 |
| 7.25 225.39 .494E+07 1400.0 1.08 58.80 |
| 7.78 225.92 .573E+07 1732.2 1.15 55.17 |
| 8.30 226.44 .655E+07 2099.9 1.22 52.02 |
| 8.82 226.96 .740E+07 2502.8 1.29 49.29 |
| 9.34 227.48 .828E+07 2940.9 1.36 46.90 |
| 9.86 228.00 .918E+07 3414.3 1.42 44.79 |
-----
```

```
-----
|----- hydrograph -----> <-pipe / channel->
| AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL |
| (ha) (cms) (hrs) (mm) (m) (m/s) |
| INFLOW : ID= 2 (8324) 3994.38 39.65 9.25 32.59 1.95 0.59 |
| OUTFLOW : ID= 1 (8328) 3994.38 33.85 11.00 32.59 1.81 0.58 |
-----
```

```
-----
| ADD HYD (8320) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0326): 513.13 5.538 8.50 35.08 |
| + ID2= 2 (8328): 2802.34 27.648 9.50 31.53 |
|-----|
| ID = 3 (8320): 3315.47 32.995 9.50 32.08 |
-----
```

```
-----
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0328): 492.92 4.508 8.75 32.09 |
| + ID2= 2 (8328): 3994.38 33.850 11.00 32.59 |
|-----|
| ID = 3 (8320): 4487.30 37.891 11.00 32.54 |
-----
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
-----
| ADD HYD (8318) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (8320): 4487.30 37.891 11.00 32.54 |
| + ID2= 2 (8322): 861.74 8.469 9.75 32.80 |
|-----|
| ID = 3 (8318): 5349.04 46.022 10.75 32.58 |
-----
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
-----
| ADD HYD (8316) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (8309): 10059.18 45.744 15.25 26.63 |
| + ID2= 2 (8318): 5349.04 46.022 10.75 32.58 |
|-----|
| ID = 3 (8316): 15408.22 86.971 11.75 28.69 |
-----
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
-----
| ADD HYD (8312) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (8314): 620.10 1.209 6.75 18.32 |
| + ID2= 2 (8316): 15408.22 86.971 11.75 28.69 |
|-----|
| ID = 3 (8312): 16028.32 87.875 11.75 28.29 |
-----
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
-----
| ADD HYD (8308) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (8310): 598.90 27.843 6.50 36.29 |
| + ID2= 2 (8312): 16028.32 87.875 11.75 28.29 |
|-----|
| ID = 3 (8308): 16627.22 89.765 11.50 28.58 |
-----
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
-----
| RESERVOIR (5510) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
|-----|
| OUTFLOW STORAGE | OUTFLOW STORAGE |
| (cms) (ha.m.) | (cms) (ha.m.) |
| 0.0000 0.0000 | ***** 74.0090 |
| 66.5450 18.5023 | ***** |
| 98.5430 37.0045 | ***** |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| INFLOW : ID= 2 (8308) ***** 89.765 11.50 28.58 |
| OUTFLOW : ID= 1 (5510) ***** 85.396 13.25 28.58 |
-----
```

PEAK FLOW REDUCTION [Qout/Qin] (%)= 95.13
 TIME SHIFT OF PEAK FLOW (min)=105.00
 MAXIMUM STORAGE USED (ha.m.)= 29.4137

```
-----
| ADD HYD (8240) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0336): 2785.00 3.370 22.25 27.75 |
| + ID2= 2 (5510): 16627.22 85.396 13.25 28.58 |
|-----|
| ID = 3 (8240): 19412.22 88.298 13.50 28.47 |
-----
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8238)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (5508): 9524.23 42.683 12.25 17.78
+ ID2= 2 (8240): 19412.22 86.298 13.50 28.47
=====
ID = 3 (8238): 28936.45 130.148 12.75 24.95

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8236)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0342): 1004.58 3.660 10.00 16.26
+ ID2= 2 (8238): 28936.45 130.148 12.75 24.95
=====
ID = 3 (8236): 29941.03 133.430 12.75 24.66

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8234)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8232): 285.80 8.040 6.00 24.37
+ ID2= 2 (8236): 29941.03 133.430 12.75 24.66
=====
ID = 3 (8234): 30226.83 133.766 12.75 24.65

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8230)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0344): 344.00 3.342 7.00 18.72
+ ID2= 2 (8234): 30226.83 133.766 12.75 24.65
=====
ID = 3 (8230): 30570.83 134.707 12.75 24.59

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8228)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8226): 1952.06 11.133 8.75 15.62
+ ID2= 2 (8230): 30570.83 134.707 12.75 24.59
=====
ID = 3 (8228): 32522.89 141.797 12.50 24.05

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8190)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0356): 1015.22 4.610 8.00 12.58
+ ID2= 2 (8228): 32522.89 141.797 12.50 24.05
=====
ID = 3 (8190): 33538.11 144.663 12.25 23.70

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

** SIMULATION NUMBER: 13 **

```

READ STORM
Filename: C:\Users\jascott\AppData
Local\Temp\
3e280798-92ee-4282-809c-79f5caed0add\fs74c8d3
Ptotal= 62.70 mm
Comments: 10-Year 12-Hour SCS II Design Storm

```

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.57	3.25	2.51	6.25	11.29	9.25	2.19
0.50	1.57	3.50	2.51	6.50	11.29	9.50	2.19
0.75	1.57	3.75	2.51	6.75	5.02	9.75	2.19
1.00	1.57	4.00	2.51	7.00	5.02	10.00	2.19
1.25	1.57	4.25	3.76	7.25	3.76	10.25	1.25
1.50	1.57	4.50	3.76	7.50	3.76	10.50	1.25

1.75	1.57	4.75	5.02	7.75	3.76	10.75	1.25
2.00	1.57	5.00	5.02	8.00	3.76	11.00	1.25
2.25	1.88	5.25	7.52	8.25	2.19	11.25	1.25
2.50	1.88	5.50	7.52	8.50	2.19	11.50	1.25
2.75	1.98	5.75	30.10	8.75	2.19	11.75	1.25
3.00	1.88	6.00	82.76	9.00	2.19	12.00	1.25

```

CALIB
NASHYD (0356) Area (ha)=1015.22 Curve Number (CN)= 46.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.37

```

Unit Hyd Qpeak (cms)= 12.651

PEAK FLOW (cms)= 3.392 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 9.295
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.148

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0354) Area (ha)= 262.68 Curve Number (CN)= 37.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.38

```

Unit Hyd Qpeak (cms)= 3.252

PEAK FLOW (cms)= 0.631 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 6.749
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.108

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0352) Area (ha)= 381.43 Curve Number (CN)= 54.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.82

```

Unit Hyd Qpeak (cms)= 7.980

PEAK FLOW (cms)= 2.453 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 11.980
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.191

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0346) Area (ha)= 350.93 Curve Number (CN)= 70.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.96

```

Unit Hyd Qpeak (cms)= 6.254

PEAK FLOW (cms)= 3.349 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 19.771
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.315

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0350) Area (ha)= 366.84 Curve Number (CN)= 48.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.07

```

Unit Hyd Qpeak (cms)= 5.831

PEAK FLOW (cms)= 1.564 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 9.910
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.158

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

OUTFLOW: ID= 1 (5510) ***** 105.320 13.75 38.94

PEAK FLOW REDUCTION [Qout/Qin](%)= 92.90
TIME SHIFT OF PEAK FLOW (min)=120.00
MAXIMUM STORAGE USED (ha.m.)= 44.9262

ADD HYD (8240)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0336):	2785.00	4.639	22.25	38.20
+ ID2= 2 (5510):	16627.22	105.320	13.75	38.94
ID = 3 (8240):	19412.22	109.428	14.00	38.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8238)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (5508):	9524.23	56.167	11.75	25.09
+ ID2= 2 (8240):	19412.22	109.428	14.00	38.84
ID = 3 (8238):	28936.45	163.719	13.00	34.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8236)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0344):	1004.58	4.230	10.00	23.19
+ ID2= 2 (8238):	28936.45	163.719	13.00	34.32
ID = 3 (8236):	29941.03	168.365	12.75	33.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8234)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8232):	285.80	10.972	6.50	32.52
+ ID2= 2 (8236):	29941.03	168.365	12.75	33.94
ID = 3 (8234):	30226.83	168.774	12.50	33.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8230)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0344):	344.00	4.771	7.00	26.48
+ ID2= 2 (8234):	30226.83	168.774	12.50	33.93
ID = 3 (8230):	30570.83	170.215	12.50	33.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8228)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8226):	1952.06	15.927	8.75	22.21
+ ID2= 2 (8230):	30570.83	170.215	12.50	33.84
ID = 3 (8228):	32522.89	180.663	12.00	33.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8190)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0356):	1015.22	6.685	8.00	18.15
+ ID2= 2 (8228):	32522.89	180.663	12.00	33.15
ID = 3 (8190):	33538.11	185.041	12.00	32.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

** SIMULATION NUMBER: 11 **

READ STORM		Filename: C:\Users\jsscott\AppData\Local\Temp\3e280798-92ee-4282-809c-79f5caed0add\9255d239
Total= 80.80 mm		Comments: 50-Year 12-Hour SCS II Design Storm

TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)
0.25	2.02	3.25	3.23	6.25	14.54	9.25	2.83
0.50	2.02	3.50	3.23	6.50	14.54	9.50	2.83
0.75	2.02	3.75	3.23	6.75	6.46	9.75	2.83
1.00	2.02	4.00	3.23	7.00	6.46	10.00	2.83
1.25	2.02	4.25	4.85	7.25	4.85	10.25	1.62
1.50	2.02	4.50	4.85	7.50	4.85	10.50	1.62
1.75	2.02	4.75	6.46	7.75	4.85	10.75	1.62
2.00	2.02	5.00	6.46	8.00	4.85	11.00	1.62
2.25	2.42	5.25	9.70	8.25	2.83	11.25	1.62
2.50	2.42	5.50	9.70	8.50	2.83	11.50	1.62
2.75	2.42	5.75	38.78	8.75	2.83	11.75	1.62
3.00	2.42	6.00	106.66	9.00	2.83	12.00	1.62

CALIB (0356)			
NASHYD (0356)	Area (ha)	Curve Number (CN)	# of Linear Res.(N)
ID= 1 DT=15.0 min	1015.22	46.0	1.50
U.H. Tp(hrs)= 1.37			

Unit Hyd Qpeak (cms)= 12.651
PEAK FLOW (cms)= 5.609 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 15.266
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.189

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0354)			
NASHYD (0354)	Area (ha)	Curve Number (CN)	# of Linear Res.(N)
ID= 1 DT=15.0 min	262.68	37.0	1.50
U.H. Tp(hrs)= 1.38			

Unit Hyd Qpeak (cms)= 3.252
PEAK FLOW (cms)= 1.056 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 11.232
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.139

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0352)			
NASHYD (0352)	Area (ha)	Curve Number (CN)	# of Linear Res.(N)
ID= 1 DT=15.0 min	381.43	54.0	1.50
U.H. Tp(hrs)= 0.82			

Unit Hyd Qpeak (cms)= 7.980
PEAK FLOW (cms)= 4.018 (1)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 19.394
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.240

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0346)			
NASHYD (0346)	Area (ha)	Curve Number (CN)	# of Linear Res.(N)
ID= 1 DT=15.0 min	350.93	70.0	1.50
U.H. Tp(hrs)= 0.96			

Unit Hyd Qpeak (cms)= 6.254
PEAK FLOW (cms)= 5.278 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 30.777
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.381

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHVD (0350) Area (ha)= 366.84 Curve Number (CN)= 48.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.07

Unit Hyd Opeak (cms)= 5.831

PEAK FLOW (cms)= 2.583 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 16.221
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.201

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHVD (0348) Area (ha)= 590.18 Curve Number (CN)= 48.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.91

Unit Hyd Opeak (cms)= 5.267

PEAK FLOW (cms)= 2.683 (i)
TIME TO PEAK (hrs)= 9.000
RUNOFF VOLUME (mm)= 16.307
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.202

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ROUTE CHN (6350) Routing time step (min)'= 15.00
IN= 2--> OUT= 1

```

DATA FOR SECTION (3501.0) ----->					
Distance	Elevation	Manning			
0.00	287.00	0.0500			
23.66	283.72	0.0500			
70.98	280.94	0.0500			
112.38	280.23	0.0500			
171.52	276.80	0.0500			
260.24	274.46	0.0500			
266.16	274.26	0.0500			
272.07	274.12	0.0500			
276.49	274.12	0.0500 / 0.0300	Main Channel		
276.99	273.82	0.0300	Main Channel		
277.99	273.82	0.0300	Main Channel		
279.49	273.82	0.0300	Main Channel		
280.49	274.27	0.0300 / 0.0500	Main Channel		
283.90	274.27	0.0500			
289.81	274.57	0.0500			
325.30	275.75	0.0500			
396.28	278.98	0.0500			
449.51	280.97	0.0500			
496.83	283.90	0.0500			
585.54	287.92	0.0500			

TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.30	274.12	.273E+04	0.9	0.99	49.81
0.98	274.80	.583E+05	20.8	1.05	46.71
1.66	275.48	.203E+06	95.2	1.38	35.53
2.33	276.15	.438E+06	256.0	1.73	28.50
3.01	276.83	.755E+06	524.8	2.05	23.97
3.69	277.51	.114E+07	935.8	2.43	20.26
4.37	278.19	.157E+07	1472.0	2.76	17.82
5.05	278.87	.206E+07	2140.7	3.06	16.06
5.72	279.54	.261E+07	2936.6	3.33	14.79
6.40	280.22	.321E+07	3887.6	3.57	13.77
7.08	280.90	.390E+07	4795.4	3.63	13.56
7.76	281.58	.468E+07	6196.2	3.91	12.56
8.43	282.25	.550E+07	7803.9	4.19	11.74
9.11	282.93	.637E+07	9601.9	4.45	11.05
9.79	283.61	.728E+07	11594.8	4.70	10.46
10.47	284.29	.823E+07	13845.3	4.96	9.91
11.15	284.97	.922E+07	16293.4	5.21	9.43
11.82	285.64	.103E+08	18952.0	5.45	9.02
12.50	286.32	.113E+08	21824.3	5.69	8.65

```

<---- hydrograph ----> <-pipe / channel->
AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (0348) 590.18 2.68 9.00 16.31 0.36 0.99
OUTFLOW : ID= 1 (6350) 590.18 2.62 10.25 16.31 0.36 0.99

```

```

ADD HYD (8220)
1 + 2 = 3 AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0350): 366.84 2.583 7.50 16.22
+ ID2= 2 (6350): 590.18 2.617 10.25 16.31
-----
ID = 3 (8220): 957.02 4.788 8.75 16.27

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8222)
1 + 2 = 3 AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0348): 350.93 5.278 7.25 30.78
+ ID2= 2 (8220): 957.02 4.788 8.75 16.27
-----
ID = 3 (8222): 1307.95 9.701 8.00 20.17

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6352) Routing time step (min)'= 15.00
IN= 2--> OUT= 1

```

DATA FOR SECTION (3521.0) ----->					
Distance	Elevation	Manning			
0.00	257.95	0.0500			
7.63	257.34	0.0500			
15.66	256.19	0.0500			
21.53	254.01	0.0500			
41.11	244.06	0.0500			
76.35	241.38	0.0500			
111.58	239.74	0.0500			
113.54	239.61	0.0500			
113.75	239.41	0.0500 / 0.0300	Main Channel		
113.85	238.81	0.0300	Main Channel		
115.50	238.81	0.0300	Main Channel		
116.15	238.81	0.0300	Main Channel		
116.25	239.41	0.0300	Main Channel		
117.46	239.52	0.0500			
119.41	239.72	0.0500			
121.37	240.04	0.0500			
131.16	241.84	0.0500			
156.61	247.03	0.0500			
176.19	251.46	0.0500			
193.80	258.79	0.0500			

TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.60	239.41	.224E+04	2.2	1.52	17.12
1.58	240.39	.216E+05	22.3	1.61	16.16
2.55	241.36	.804E+05	101.0	1.96	13.27
3.53	242.34	.173E+06	275.6	2.48	10.46
4.50	243.31	.293E+06	553.9	2.95	8.80
5.48	244.29	.438E+06	961.8	3.42	7.60
6.45	245.26	.599E+06	1534.4	3.99	6.51
7.43	246.24	.770E+06	2228.4	4.51	5.76
8.41	247.22	.951E+06	3043.6	4.98	5.21
9.38	248.19	.114E+07	3982.1	5.43	4.78
10.36	249.17	.134E+07	5040.1	5.85	4.44
11.33	250.14	.155E+07	6218.5	6.24	4.16
12.31	251.12	.177E+07	7518.6	6.61	3.93
13.29	252.10	.200E+07	8969.0	6.98	3.72
14.26	253.07	.224E+07	10554.1	7.35	3.53
15.24	254.05	.248E+07	12257.1	7.71	3.37
16.21	255.02	.272E+07	14045.1	8.03	3.23
17.19	256.00	.298E+07	15954.8	8.34	3.11
18.16	256.97	.325E+07	17780.4	8.53	3.04

```

<---- hydrograph ----> <-pipe / channel->
AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8222) 1307.95 9.70 8.00 20.17 0.37 1.55
OUTFLOW : ID= 1 (6352) 1307.95 9.63 8.25 20.17 0.36 1.55

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ADD HYD (8224)
1 + 2 = 3 AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0352): 381.43 4.018 7.00 19.39
+ ID2= 2 (6353): 1307.95 9.633 8.25 20.17
-----
ID = 3 (8224): 1689.38 13.278 7.75 19.99

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTER CN (6354) |
 IN= 2--> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (3541.0) -----

Distance	Elevation	Manning	
0.00	253.92	0.0500	
7.95	251.83	0.0500	
15.89	249.97	0.0500	
19.87	249.05	0.0500	
47.68	242.00	0.0500	
67.55	237.10	0.0500	
83.44	231.11	0.0500	
85.43	230.75	0.0500	
85.66	230.61	0.0500 / 0.0300	Main Channel
85.76	230.01	0.0300	Main Channel
87.41	230.01	0.0300	Main Channel
88.06	230.01	0.0300	Main Channel
88.16	230.61	0.0300 / 0.0500	Main Channel
89.40	230.72	0.0500	
91.39	230.88	0.0500	
93.37	231.03	0.0500	
133.11	233.96	0.0500	
150.99	235.39	0.0500	
172.84	239.03	0.0500	
196.68	250.00	0.0500	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.60	230.61	.451E+04	1.5	1.06	49.40
1.62	231.63	.394E+05	14.6	1.16	45.13
2.64	232.65	.128E+06	58.8	1.44	36.30
3.66	233.67	.269E+06	149.7	1.74	30.00
4.68	234.69	.463E+06	301.5	2.04	25.58
5.70	235.71	.705E+06	530.5	2.36	22.14
6.72	236.73	.980E+06	848.4	2.71	19.25
7.74	237.75	.128E+07	1237.9	3.02	17.29
8.76	238.77	.162E+07	1706.7	3.30	15.83
9.78	239.79	.199E+07	2291.9	3.61	14.45
10.81	240.82	.237E+07	2973.7	3.92	13.30
11.83	241.84	.278E+07	3742.2	4.21	12.38
12.85	242.86	.321E+07	4599.1	4.49	11.62
13.87	243.88	.365E+07	5544.6	4.75	10.98
14.89	244.90	.412E+07	6579.7	5.00	10.44
15.91	245.92	.461E+07	7705.9	5.23	9.97
16.93	246.94	.512E+07	8924.6	5.46	9.55
17.95	247.96	.564E+07	10237.5	5.68	9.19
18.97	248.98	.619E+07	11646.2	5.89	8.86

----- hydrograph ----- <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8224) 1689.38	13.28	7.75	19.99	1.52	1.15
OUTFLOW : ID= 1 (6354) 1689.38	12.41	8.75	19.99	1.45	1.14

ADD HYD (8226) |
 1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)

ID1= 1 (0354):	262.68	1.056	8.00	11.23
+ ID2= 2 (6354):	1689.38	12.412	8.75	19.99
ID = 3 (8226):	1952.06	13.442	8.75	18.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0344) | Area (ha)= 344.00 Curve Number (CN)= 59.0
 NASHVD (0344) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.87

Unit Hyd Qpeak (cms)=	6.790
PEAK FLOW (cms)=	4.034 (i)
TIME TO PEAK (hrs)=	7.000
RUNOFF VOLUME (mm)=	22.484
TOTAL RAINFALL (mm)=	80.800
RUNOFF COEFFICIENT =	0.278

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB | Area (ha)= 234.60
 STANDHYD (3402) | Total Imp(%)= 28.00 Dir. Conn.(%)= 13.00
 ID= 1 DT=15.0 min

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)=	65.69	168.91	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	1250.60	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	106.66	21.15	
over (min)=	15.00	45.00	
Storage Coeff. (min)=	13.95 (ii)	33.87 (ii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.07	0.03	
PEAK FLOW (cms)=	6.78	5.80	9.169 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	78.80	19.31	27.04
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.24	0.33

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB | Area (ha)= 51.20
 STANDHYD (3401) | Total Imp(%)= 46.00 Dir. Conn.(%)= 21.00
 ID= 1 DT=15.0 min

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)=	23.55	27.65	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	584.24	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	106.66	49.39	
over (min)=	15.00	30.00	
Storage Coeff. (min)=	8.84 (ii)	23.02 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.09	0.04	
PEAK FLOW (cms)=	2.78	1.79	3.900 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	78.80	22.56	34.37
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.28	0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5340) |
 IN= 2--> OUT= 1 |

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	5.3360	1.7877
0.1730	0.6348	7.2780	2.2185
1.8360	0.8250	9.1880	2.6486
2.1540	1.3051	9.5880	2.6586
3.3950	1.3506	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (3401)	51.200	3.900	6.00
OUTFLOW : ID= 1 (5340)	51.200	1.657	6.50

PEAK FLOW REDUCTION [Qout/Qin](%)=	42.48
TIME SHIFT OF PEAK FLOW (min)=	30.00
MAXIMUM STORAGE USED (ha.m.)=	0.8094

ADD HYD (8232) |
 1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)

ID1= 1 (3402):	234.60	9.169	6.00	27.04
+ ID2= 2 (5340):	51.20	1.657	6.50	34.35
ID = 3 (8232):	285.80	9.337	6.50	28.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHVD (0342) Area (ha)=1004.58 Curve Number (CN)= 54.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 2.57

Unit Hyd Qpeak (cms)= 6.686
PEAK FLOW (cms)= 4.420 (i)
TIME TO PEAK (hrs)= 10.000
RUNOFF VOLUME (mm)= 19.616
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.243

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0422) Area (ha)= 780.20 Curve Number (CN)= 54.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.95

Unit Hyd Qpeak (cms)= 6.838
PEAK FLOW (cms)= 4.215 (i)
TIME TO PEAK (hrs)= 9.000
RUNOFF VOLUME (mm)= 19.591
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.242

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0410) Area (ha)= 572.01 Curve Number (CN)= 48.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.46

Unit Hyd Qpeak (cms)= 6.698
PEAK FLOW (cms)= 3.210 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 16.276
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.201

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0408) Area (ha)= 231.62 Curve Number (CN)= 58.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.64

Unit Hyd Qpeak (cms)= 6.198
PEAK FLOW (cms)= 3.331 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 21.683
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.268

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0376) Area (ha)= 463.85 Curve Number (CN)= 74.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 7.380
PEAK FLOW (cms)= 7.184 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 34.493
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.427

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0374) Area (ha)= 545.70 Curve Number (CN)= 61.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.51

Unit Hyd Qpeak (cms)= 6.158

PEAK FLOW (cms)= 4.432 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 23.989
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.297

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0372) Area (ha)= 110.42 Curve Number (CN)= 37.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 1.954
PEAK FLOW (cms)= 0.576 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 11.182
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.138

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0370) Area (ha)= 191.85 Curve Number (CN)= 63.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.67

Unit Hyd Qpeak (cms)= 4.860
PEAK FLOW (cms)= 3.080 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 25.072
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.310

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0368) Area (ha)= 159.48 Curve Number (CN)= 46.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.12

Unit Hyd Qpeak (cms)= 2.433
PEAK FLOW (cms)= 1.020 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 15.231
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.189

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0366) Area (ha)= 462.62 Curve Number (CN)= 62.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.06

Unit Hyd Qpeak (cms)= 7.451
PEAK FLOW (cms)= 5.064 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 24.589
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.304

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0364) Area (ha)= 155.27 Curve Number (CN)= 55.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.70

Unit Hyd Qpeak (cms)= 3.782
PEAK FLOW (cms)= 1.891 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 19.909
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.246

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| ADD HYD (8302) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
ID1= 1 (0364): 155.27 1.891 6.75 19.91
+ ID2= 2 (0366): 462.62 5.064 7.50 24.59
-----
| ID = 3 (8302): 617.89 6.881 7.25 23.41
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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| CALIB |
| STANHYD (0362) |
| ID= 1 DT=15.0 min |
-----
| Area (ha)= 118.78 |
| Total Imp(%)= 22.00 |
| Dir. Conn.(%)= 8.00 |
-----

```

```

-----
| IMPERVIOUS | PERVIOUS (i) |
| (ha) | (ha) |
Surface Area (ha)= 26.13 92.65
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 889.87 40.00
Mannings n = 0.013 0.250
-----
| Max. Eff. Inten. (mm/hr)= 106.66 18.31 |
| cover (min)= 15.00 45.00 |
| Storage Coeff. (min)= 11.38 (ii) 32.47 (ii) |
| Unit Hyd. Tpeak (min)= 15.00 45.00 |
| Unit Hyd. peak (cms)= 0.08 0.03 |
-----
| PEAK FLOW (cms)= 2.28 2.82 |
| TIME TO PEAK (hrs)= 6.00 6.50 |
| RUNOFF VOLUME (mm)= 78.80 17.25 22.18 |
| TOTAL RAINFALL (mm)= 80.80 80.80 |
| RUNOFF COEFFICIENT = 0.98 0.21 |
-----

```

TOTALS

3.434 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES;
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0358) |
| ID= 1 DT=15.0 min |
-----
| Area (ha)= 429.87 |
| Ia (mm)= 5.00 |
| U.H. Tp(hrs)= 1.03 |
| Curve Number (CN)= 35.0 |
| # of Linear Res.(N)= 1.50 |
-----

```

Unit Hyd Qpeak (cms)= 7.091

```

PEAK FLOW (cms)= 1.973 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 10.392
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.129
-----

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0360) |
| ID= 1 DT=10 min |
-----
| Area (ha)= 138.37 |
| Ia (mm)= 5.00 |
| U.H. Tp(hrs)= 0.60 |
| Curve Number (CN)= 46.0 |
| # of Linear Res.(N)= 1.50 |
-----

```

Unit Hyd Qpeak (cms)= 3.957

```

PEAK FLOW (cms)= 1.427 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 15.028
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.186
-----

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8306) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
ID1= 1 (0358): 429.87 1.973 7.50 10.39
+ ID2= 2 (0360): 138.37 1.427 6.75 15.03
-----
| ID = 3 (8306): 568.24 3.321 7.00 11.52
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6362) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)= 15.00
-----

```

```

-----
| DATA FOR SECTION (3621.0) ----->
| Distance | Elevation | Manning |
|-----|-----|-----|
| 0.00 | 261.46 | 0.0550 |
| 27.86 | 254.23 | 0.0550 |
| 51.07 | 251.96 | 0.0550 |
| 74.29 | 250.77 | 0.0550 |
| 97.50 | 249.91 | 0.0550 |
| 125.36 | 249.40 | 0.0550 |
| 150.93 | 247.40 | 0.0550 |
| 155.93 | 247.33 | 0.0550 / 0.0350 |
| 157.93 | 246.85 | 0.0350 |
| 159.18 | 246.65 | 0.0350 |
| 160.18 | 246.63 | 0.0350 |
| 160.93 | 246.85 | 0.0350 |
| 161.93 | 247.18 | 0.0350 / 0.0550 |
| 163.18 | 248.03 | 0.0550 |
| 168.18 | 248.58 | 0.0550 |
| 183.18 | 250.18 | 0.0550 |
| 201.97 | 252.59 | 0.0550 |
| 213.57 | 256.02 | 0.0550 |
| 225.18 | 260.31 | 0.0550 |
| 229.82 | 261.00 | 0.0550 |
-----

```

```

-----
| TRAVEL TIME TABLE ----->
| DEPTH | ELEV | VOLUME | FLOW RATE | VELOCITY | TRAV.TIME |
| (m) | (m) | (cu.m.) | (cms) | (m/s) | (min) |
|-----|-----|-----|-----|-----|-----|
| 0.55 | 247.18 | .558E+04 | 1.9 | 1.03 | 49.95 |
| 1.28 | 247.91 | .338E+05 | 16.7 | 1.53 | 33.61 |
| 2.00 | 248.63 | .913E+05 | 53.6 | 1.81 | 28.40 |
| 2.73 | 249.36 | .185E+06 | 122.3 | 2.05 | 25.18 |
| 3.46 | 250.09 | .348E+06 | 224.6 | 2.02 | 25.52 |
| 4.19 | 250.82 | .574E+06 | 417.9 | 2.25 | 22.91 |
| 4.91 | 251.54 | .855E+06 | 699.1 | 2.53 | 20.38 |
| 5.64 | 252.27 | 1.188E+07 | 1075.3 | 2.82 | 18.26 |
| 6.37 | 253.00 | 1.53E+07 | 1559.9 | 3.15 | 16.38 |
| 7.10 | 253.73 | 1.91E+07 | 2137.6 | 3.46 | 14.89 |
| 7.82 | 254.45 | 2.31E+07 | 2815.2 | 3.77 | 13.67 |
| 8.55 | 255.18 | 2.72E+07 | 3608.5 | 4.10 | 12.58 |
| 9.28 | 255.91 | 3.15E+07 | 4488.2 | 4.41 | 11.69 |
| 10.01 | 256.64 | 3.58E+07 | 5458.0 | 4.71 | 10.95 |
| 10.73 | 257.36 | 4.03E+07 | 6512.4 | 4.99 | 10.32 |
| 11.46 | 258.09 | 4.49E+07 | 7649.8 | 5.27 | 9.78 |
| 12.19 | 258.82 | 4.96E+07 | 8869.5 | 5.53 | 9.32 |
| 12.92 | 259.55 | 5.44E+07 | 10171.2 | 5.78 | 8.91 |
| 13.64 | 260.27 | 5.93E+07 | 11554.8 | 6.02 | 8.55 |
-----

```

```

-----
| hydrograph ----->
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
INFLOW : ID= 2 (8306) 568.24 3.32 7.00 11.52 0.62 1.07
OUTFLOW: ID= 1 (6362) 568.24 2.94 8.25 11.52 0.60 1.06
-----

```

```

-----
| ADD HYD (8304) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
ID1= 1 (0362): 118.78 3.434 6.00 22.18
+ ID2= 2 (6362): 568.24 2.938 8.25 11.52
-----
| ID = 3 (8304): 687.02 4.850 6.50 13.36
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8300) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
ID1= 1 (8302): 617.89 6.881 7.25 23.41
+ ID2= 2 (8304): 687.02 4.850 6.50 13.36
-----
| ID = 3 (8300): 1304.91 11.147 7.00 18.12
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6368) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)= 15.00
-----

```

```

-----
| DATA FOR SECTION (3681.0) ----->
| Distance | Elevation | Manning |
|-----|-----|-----|
| 0.00 | 230.00 | 0.0370 |
| 18.48 | 223.26 | 0.0370 |
| 36.96 | 223.05 | 0.0370 |
| 55.44 | 222.94 | 0.0370 |
| 73.92 | 222.86 | 0.0370 |
-----

```


133.96	222.74	0.0370	
147.82	222.65	0.0370	
170.92	222.31	0.0370	
174.79	222.26	0.0370 / 0.0300	Main Channel
174.89	221.86	0.0300	Main Channel
175.54	221.86	0.0300	Main Channel
176.19	221.86	0.0300	Main Channel
176.29	222.26	0.0300 / 0.0370	Main Channel
180.16	222.25	0.0370	
184.78	222.28	0.0370	
189.40	222.31	0.0370	
332.60	222.37	0.0370	
450.00	230.00	0.0370	

731.59	219.46	0.0390
762.39	225.00	0.0390

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.40	222.26	.771E+03	0.3	0.52	41.74
0.81	222.67	.784E+05	30.3	0.51	43.17
1.21	223.07	.210E+06	123.2	0.77	28.42
1.62	223.48	.384E+06	301.0	1.03	21.25
2.03	223.89	.563E+06	550.2	1.28	17.06
2.44	224.30	.747E+06	860.7	1.51	14.46
2.84	224.70	.934E+06	1228.1	1.73	12.68
3.25	225.11	.113E+07	1649.3	1.92	11.37
3.66	225.52	.132E+07	2122.3	2.11	10.37
4.07	225.93	.152E+07	2645.5	2.29	9.58
4.47	226.33	.172E+07	3217.8	2.45	8.92
4.88	226.74	.193E+07	3838.3	2.61	8.38
5.29	227.15	.214E+07	4506.3	2.76	7.92
5.70	227.56	.236E+07	5221.3	2.91	7.52
6.10	227.96	.258E+07	5983.0	3.05	7.17
6.51	228.37	.280E+07	6790.9	3.19	6.87
6.92	228.78	.303E+07	7645.0	3.32	6.60
7.33	229.19	.326E+07	8545.1	3.45	6.35
7.73	229.59	.349E+07	9491.0	3.57	6.13

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8300)	1304.91	11.15	7.00	18.12	0.55
OUTFLOW : ID= 1 (6368)	1304.91	10.16	8.00	18.12	0.53

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0368)	159.48	1.020	7.75
ID2= 2 (6368)	1304.91	10.162	8.00
ID = 3 (8298)	1464.39	11.178	8.00

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0370)	191.85	3.080	6.75
ID2= 2 (8298)	1464.39	11.178	8.00
ID = 3 (8296)	1656.24	13.707	7.75

ROUTE CHN (6372) | Routing time step (min)'= 15.00 | IN= 2--> OUT= 1 |

Distance	Elevation	Manning
0.00	225.00	0.0390
30.80	219.38	0.0390
61.61	219.30	0.0390
77.01	219.27	0.0390
469.76	219.14	0.0390
477.46	219.13	0.0390
485.16	219.10	0.0390
492.86	219.09	0.0390
495.56	219.09	0.0390 / 0.0310
495.66	218.51	0.0310
500.56	218.51	0.0310
505.46	218.51	0.0310
505.56	219.09	0.0310 / 0.0390
508.26	219.09	0.0390
515.96	219.10	0.0390
523.67	219.21	0.0390
562.17	219.32	0.0390
654.58	219.43	0.0390

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.29	218.80	.431E+04	1.7	0.58	43.48
0.58	219.09	.867E+04	5.1	0.89	28.32
0.93	219.44	.200E+05	58.2	0.44	57.35
1.28	219.79	.568E+05	278.6	0.74	33.99
1.62	220.13	.939E+05	630.2	1.01	24.84
1.97	220.48	1.31E+06	1090.4	1.25	20.05
2.32	220.83	1.69E+06	1647.7	1.47	17.06
2.67	221.18	2.06E+06	2294.6	1.68	14.99
3.01	221.52	2.44E+06	3025.6	1.87	13.46
3.36	221.87	2.82E+06	3836.5	2.05	12.27
3.71	222.22	3.21E+06	4723.7	2.22	11.31
4.06	222.57	3.59E+06	5684.6	2.39	10.53
4.40	222.91	3.98E+06	6716.6	2.55	9.87
4.75	223.26	4.37E+06	7817.7	2.70	9.31
5.10	223.61	4.76E+06	8986.2	2.85	8.83
5.45	223.96	5.15E+06	10220.4	2.99	8.40
5.79	224.30	5.55E+06	11519.1	3.13	8.03
6.14	224.65	5.94E+06	12880.8	3.27	7.69
6.49	225.00	6.34E+06	14304.5	3.40	7.39

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8296)	1656.24	13.71	7.75	18.65	0.64
OUTFLOW : ID= 1 (6372)	1656.24	13.13	8.50	18.65	0.63

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0372)	110.42	0.576	7.25
ID2= 2 (6372)	1656.24	13.134	8.50
ID = 3 (8294)	1766.66	13.657	8.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0374)	545.70	4.432	8.25
ID2= 2 (8294)	1766.66	13.657	8.50
ID = 3 (8292)	2312.36	18.064	8.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	65.1290	345.3784
25.4850	24.6697	84.9510	456.3890
31.1490	98.6787	*****	838.7689
39.6440	*****	*****	838.7789
48.1390	*****	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8292)	2312.360	18.064	8.50
OUTFLOW : ID= 1 (5505)	2312.360	13.246	11.25

PEAK FLOW REDUCTION [Qout/Qin](%)= 73.32
TIME SHIFT OF PEAK FLOW (min)=165.00
MAXIMUM STORAGE USED (ha.m.)= 12.8339

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0376)	463.85	7.184	7.25
ID2= 2 (5505)	2312.36	13.246	11.25
ID = 3 (8272)	2776.21	17.737	10.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB
NASHYD (0396) Area (ha)= 305.21 Curve Number (CN)= 69.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.08

```

```

Unit Hyd Qpeak (cms)= 4.811
PEAK FLOW (cms)= 4.047 (1)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 29.979
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.371

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0394) Area (ha)= 325.45 Curve Number (CN)= 53.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.92

```

```

Unit Hyd Qpeak (cms)= 6.013
PEAK FLOW (cms)= 3.012 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 18.867
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.233

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0390) Area (ha)= 420.00 Curve Number (CN)= 55.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.07

```

```

Unit Hyd Qpeak (cms)= 6.683
PEAK FLOW (cms)= 3.688 (1)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 20.072
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.248

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0388) Area (ha)= 220.77 Curve Number (CN)= 58.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.99

```

```

Unit Hyd Qpeak (cms)= 3.819
PEAK FLOW (cms)= 2.265 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 21.891
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.271

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0386) Area (ha)= 241.27 Curve Number (CN)= 61.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.90

```

```

Unit Hyd Qpeak (cms)= 4.562
PEAK FLOW (cms)= 2.901 (1)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 23.835
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.295

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8286)
1 2 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0386): 241.27 2.901 7.00 23.84
+ ID2= 2 (0388): 220.77 2.265 7.25 21.89
=====
ID = 3 (8286): 462.04 5.164 7.25 22.91

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB
NASHYD (0384) Area (ha)= 199.07 Curve Number (CN)= 44.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.96

```

```

Unit Hyd Qpeak (cms)= 3.537
PEAK FLOW (cms)= 1.336 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 14.242
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.176

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0380) Area (ha)= 182.01 Curve Number (CN)= 40.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.55

```

```

Unit Hyd Qpeak (cms)= 5.609
PEAK FLOW (cms)= 1.618 (1)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 12.271
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.152

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0382) Area (ha)= 216.59 Curve Number (CN)= 53.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.64

```

```

Unit Hyd Qpeak (cms)= 5.733
PEAK FLOW (cms)= 2.645 (1)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 18.713
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.232

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8290)
1 + 2 = 3 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0380): 182.01 1.618 6.50 12.27
+ ID2= 2 (0382): 216.59 2.645 6.75 18.71
=====
ID = 3 (8290): 398.60 4.252 6.75 15.77

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6384)
IN= 2--> OUT= 1 Routing time step (min)= 15.00

```

```

<----- DATA FOR SECTION (3841.0) ----->
Distance Elevation Manning
0.00 294.40 0.0380
10.59 291.93 0.0380
21.17 289.19 0.0380
26.46 287.99 0.0380
31.76 286.79 0.0380
71.45 279.97 0.0380
74.10 279.79 0.0380
76.74 279.71 0.0380
78.99 279.64 0.0380 / 0.0300 Main Channel
79.14 279.30 0.0300 Main Channel
79.39 279.30 0.0300 Main Channel
79.64 279.30 0.0300 Main Channel
79.79 279.64 0.0300 / 0.0380 Main Channel
89.98 279.78 0.0380
119.09 281.86 0.0380
145.55 282.87 0.0380
198.48 284.85 0.0380
211.71 286.31 0.0380
230.23 287.59 0.0380
261.99 294.00 0.0380

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME

```

(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.34	279.64	.715E+01	0.2	0.85	63.42
1.10	280.40	.507E+05	21.9	1.40	38.54
1.85	281.15	.141E+06	91.8	2.10	25.69
2.61	281.91	.269E+06	215.9	2.63	20.46
3.36	282.66	.445E+06	407.1	2.96	18.23
4.12	283.42	.681E+06	698.3	3.31	16.26
4.87	284.17	.977E+06	1111.8	3.68	14.65
5.63	284.93	.133E+07	1674.3	4.06	13.27
6.39	285.69	.173E+07	2457.8	4.59	11.73
7.14	286.44	.215E+07	3378.8	5.07	10.63
7.90	287.20	.261E+07	4416.4	5.47	9.85
8.65	287.95	.310E+07	5676.3	5.92	9.11
9.41	288.71	.361E+07	7150.5	6.40	8.42
10.17	289.47	.414E+07	8783.1	6.86	7.86
10.92	290.22	.469E+07	10575.1	7.30	7.38
11.68	290.98	.528E+07	12520.4	7.72	6.98
12.43	291.73	.582E+07	14619.0	8.12	6.64
13.19	292.49	.642E+07	16863.2	8.50	6.34
13.94	293.24	.703E+07	19259.8	8.86	6.08

```

<---- hydrograph ----> <-pipe / channel->
      AREA   QPEAK  TPEAK  R.V.  MAX DEPTH  MAX VEL
      (ha)   (cms) (hrs) (mm)   (m)         (m/s)
INFLOW : ID= 2 (8290) 398.60 4.25 6.75 15.77 0.48 0.92
OUTFLOW: ID= 1 (6384) 398.60 3.34 8.00 15.77 0.45 0.90

```

```

ADD HYD (8288)
  1 + 2 = 3
      AREA   QPEAK  TPEAK  R.V.
      (ha)   (cms) (hrs) (mm)
ID1= 1 (0384): 199.07 1.336 7.25 14.24
+ ID2= 2 (6384): 398.60 3.339 8.00 15.77
=====
ID = 3 (8288): 597.67 4.648 7.75 15.26

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8284)
  1 + 2 = 3
      AREA   QPEAK  TPEAK  R.V.
      (ha)   (cms) (hrs) (mm)
ID1= 1 (8286): 462.04 5.164 7.25 22.91
+ ID2= 2 (8288): 597.67 4.648 7.75 15.26
=====
ID = 3 (8284): 1059.71 9.705 7.50 18.59

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8280)
  1 + 2 = 3
      AREA   QPEAK  TPEAK  R.V.
      (ha)   (cms) (hrs) (mm)
ID1= 1 (0390): 420.00 3.688 7.50 20.07
+ ID2= 2 (8284): 1059.71 9.705 7.50 18.59
=====
ID = 3 (8280): 1479.71 13.393 7.50 19.01

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB (0392) | Area (ha)= 167.22 Curve Number (CN)= 62.0
NASHYD | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.74

```

Unit Hyd Qpeak (cms) = 3.837

PEAK FLOW (cms) = 2.409 (i)

TIME TO PEAK (hrs) = 7.000

RUNOFF VOLUME (mm) = 24.429

TOTAL RAINFALL (mm) = 80.800

RUNOFF COEFFICIENT = 0.302

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB (0378) | Area (ha)= 606.72 Curve Number (CN)= 55.0
NASHYD | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.18

```

Unit Hyd Qpeak (cms) = 8.771

PEAK FLOW (cms) = 4.960 (i)

TIME TO PEAK (hrs) = 7.750

RUNOFF VOLUME (mm) = 20.097
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.249

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8282)
  1 + 2 = 3
      AREA   QPEAK  TPEAK  R.V.
      (ha)   (cms) (hrs) (mm)
ID1= 1 (0378): 606.72 4.960 7.75 20.10
+ ID2= 2 (0392): 167.22 2.409 7.00 24.43
=====
ID = 3 (8282): 773.94 7.226 7.25 21.03

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8278)
  1 + 2 = 3
      AREA   QPEAK  TPEAK  R.V.
      (ha)   (cms) (hrs) (mm)
ID1= 1 (8280): 1479.71 13.393 7.50 19.01
+ ID2= 2 (8282): 773.94 7.226 7.25 21.03
=====
ID = 3 (8278): 2253.65 20.600 7.50 19.71

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CN (6394) |
IN= 2----> OUT= 1 | Routing time step (min) = 15.00

```

```

----- DATA FOR SECTION (3941.0) -----
Distance Elevation Manning
0.00 283.00 0.0380
13.48 282.87 0.0380
53.92 280.08 0.0380
74.13 276.62 0.0380
97.72 265.45 0.0380
114.57 256.93 0.0380
131.42 253.04 0.0380
134.79 252.58 0.0380
138.53 251.74 0.0380 /0.0300 Main Channel
139.03 251.20 0.0300 Main Channel
141.53 251.20 0.0300 Main Channel
144.03 251.20 0.0300 Main Channel
144.53 251.74 0.0300 /0.0380 Main Channel
148.27 252.69 0.0380
151.64 252.97 0.0380
185.34 255.08 0.0380
219.03 257.54 0.0380
262.84 259.43 0.0380
310.02 262.80 0.0380
333.60 283.00 0.0380

```

```

----- TRAVEL TIME TABLE -----
DEPTH ELUV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.54 251.74 .827E+04 4.5 1.51 30.71
2.19 251.39 .758E+05 73.2 2.69 17.27
3.83 255.03 .282E+06 331.8 3.27 14.17
5.48 256.68 .633E+06 917.3 4.03 11.51
7.12 258.32 .112E+07 1889.5 4.69 9.90
8.77 259.97 .179E+07 3410.2 5.31 8.73
10.41 261.61 .259E+07 5627.6 6.05 7.66
12.06 263.26 .351E+07 8605.1 6.83 6.79
13.70 264.90 .447E+07 12544.9 7.81 5.94
15.35 266.55 .548E+07 17103.4 8.71 5.32
16.99 268.19 .648E+07 22251.7 9.56 4.85
18.64 269.84 .752E+07 27979.0 10.36 4.48
20.28 271.48 .858E+07 34269.0 11.11 4.17
21.93 273.13 .967E+07 41110.8 11.83 3.92
23.57 274.77 .108E+08 48495.9 12.51 3.71
25.22 276.42 .119E+08 56418.1 13.17 3.52
26.86 278.06 .131E+08 64410.6 13.69 3.39
28.51 279.71 .143E+08 73002.1 14.18 3.27
30.15 281.35 .156E+08 81379.3 14.50 3.20

```

```

<---- hydrograph ----> <-pipe / channel->
      AREA   QPEAK  TPEAK  R.V.  MAX DEPTH  MAX VEL
      (ha)   (cms) (hrs) (mm)   (m)         (m/s)
INFLOW : ID= 2 (8278) 2253.65 20.60 7.50 19.71 0.93 1.68
OUTFLOW: ID= 1 (6394) 2253.65 20.08 8.00 19.71 0.91 1.68

```

```

ADD HYD (8276)
  1 + 2 = 3
      AREA   QPEAK  TPEAK  R.V.

```

```

-----
      (ha)  (cms)  (hrs)  (mm)
ID1= 1 (0394): 325.45 3.012 7.25 18.87
+ ID2= 2 (6394): 2253.65 20.077 8.00 19.71
-----
ID = 3 (8276): 2579.10 22.946 8.00 19.60

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6396) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00
-----

```

```

<----- DATA FOR SECTION (3961.0) ----->
Distance  Elevation  Manning
0.00      263.00      0.0410
11.75     257.14      0.0410
23.50     253.97      0.0410
41.13     247.83      0.0410
76.38     232.09      0.0410
135.13    229.07      0.0410
149.82    228.97      0.0410
152.75    228.96      0.0410 /0.0300 Main Channel
154.19    228.73      0.0300 Main Channel
154.69    228.20      0.0300 Main Channel
155.69    228.20      0.0300 Main Channel
156.69    228.20      0.0300 Main Channel
157.19    228.73      0.0300 Main Channel
158.63    228.95      0.0300 /0.0410 Main Channel
161.57    228.96      0.0410
164.51    229.71      0.0410
196.82    241.70      0.0410
223.26    249.21      0.0410
246.76    255.13      0.0410
290.82    263.51      0.0410

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH  ELEV  VOLUME  FLOW RATE  VELOCITY  TRAV.TIME
(m)    (m)    (cu.m.) (cms)      (m/s)     (min)
0.75   228.95  780E+04  2.2        0.98      58.06
2.54   230.74  277E+06  145.1      1.78      31.76
4.33   232.53  786E+06  609.1      2.63      21.50
6.13   234.33  140E+07  1481.5     3.59      15.77
7.92   236.12  207E+07  2677.3     4.39      12.90
9.71   237.91  280E+07  4184.8     5.09      11.14
11.50  239.70  357E+07  6001.4     5.71      9.93
13.29  241.49  441E+07  8123.1     6.27      9.03
15.09  243.29  530E+07  10549.4    6.77      8.37
16.88  245.08  625E+07  13300.6    7.24      7.83
18.67  246.87  726E+07  16396.2    7.68      7.38
20.46  248.66  834E+07  19809.8    8.07      7.02
22.26  250.46  949E+07  23532.5    8.43      6.72
24.05  252.25  107E+08  27641.2    8.77      6.46
25.84  254.04  120E+08  32155.2    9.10      6.22
27.63  255.83  134E+08  36901.0    9.37      6.05
29.42  257.62  149E+08  42147.7    9.64      5.88
31.22  259.42  164E+08  48149.1    9.97      5.68
33.01  261.21  181E+08  54644.8    10.29     5.51

```

```

<---- hydrograph ----> <-pipe / channel->
      AREA  QPEAK  TPEAK  R.V.  MAX DEPTH  MAX VEL
      (ha)  (cms)  (hrs)  (mm)  (m)        (m/s)
INFLOW : ID= 2 (8276) 2579.10 22.95 8.00 19.60 1.01 1.04
OUTFLOW: ID= 1 (6396) 2579.10 20.73 9.00 19.60 0.98 1.04

```

```

-----
| ADD HYD (8274) |
| 1 + 2 = 3 |
-----
      AREA  QPEAK  TPEAK  R.V.
      (ha)  (cms)  (hrs)  (mm)
ID1= 1 (0396): 305.21 4.047 7.50 29.98
+ ID2= 2 (6396): 2579.10 20.733 9.00 19.60
-----
ID = 3 (8274): 2884.31 24.211 9.00 20.70

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8270) |
| 1 + 2 = 3 |
-----
      AREA  QPEAK  TPEAK  R.V.
      (ha)  (cms)  (hrs)  (mm)
ID1= 1 (8272): 2776.21 17.737 10.25 22.05
+ ID2= 2 (8274): 2884.31 24.211 9.00 20.70
-----
ID = 3 (8270): 5660.52 41.092 9.25 21.36

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| RESERVOIR (5506) |
-----

```

```

-----
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
      OUTFLOW  STORAGE  OUTFLOW  STORAGE
      (cms)   (ha.m.)  (cms)   (ha.m.)
0.0000     0.0000    60.8810  135.6832
31.1490    24.6597    96.2770  900.4431
36.8120    37.0045    96.6770  900.4531
45.3070    86.3439    0.0000   0.0000

```

```

      AREA  QPEAK  TPEAK  R.V.
      (ha)  (cms)  (hrs)  (mm)
INFLOW : ID= 2 (8270) 5660.520 41.092 9.25 21.36
OUTFLOW: ID= 1 (5506) 5660.520 32.437 12.00 21.36

```

```

      PEAK FLOW REDUCTION [Qout/Qin](%)= 78.94
      TIME SHIFT OF PEAK FLOW (min)=165.00
      MAXIMUM STORAGE USED (ha.m.)= 27.4808

```

```

-----
| CALIB |
| NASHYD (0406) | Area (ha)= 142.65 Curve Number (CN)= 66.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.59
-----

```

Unit Hyd Qpeak (cms)= 4.135

```

PEAK FLOW (cms)= 2.779 (1)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 27.183
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.336

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0404) | Area (ha)= 246.46 Curve Number (CN)= 47.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.98
-----

```

Unit Hyd Qpeak (cms)= 4.280

```

PEAK FLOW (cms)= 1.795 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 15.696
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.194

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0402) | Area (ha)= 244.00 Curve Number (CN)= 61.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.07
-----

```

Unit Hyd Qpeak (cms)= 3.879

```

PEAK FLOW (cms)= 2.567 (1)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 23.900
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.296

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0400) | Area (ha)= 93.97 Curve Number (CN)= 52.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.44
-----

```

Unit Hyd Qpeak (cms)= 3.630

```

PEAK FLOW (cms)= 1.483 (1)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 17.989
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.221

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0398) | Area (ha)= 328.19 Curve Number (CN)= 55.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.83
-----

```

Unit Hyd Qpeak (cms)= 6.759

PEAK FLOW (cms)= 3.522 (1)

TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 19.985
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.247

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8268) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0398): 328.19 3.522 7.00 19.99
+ ID2= 2 (0400): 93.97 1.483 6.50 17.89
=====
ID = 3 (8268): 422.16 4.864 6.75 19.52
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6402) |
| IM= 2--> OUT= 1 |
-----
Routing time step (min)= 15.00
  
```

<----- DATA FOR SECTION (4021.0) ----->

Distance	Elevation	Manning	
0.00	238.50	0.0360	
11.50	238.00	0.0360	
23.00	237.93	0.0360	
34.49	236.39	0.0360	
63.24	233.98	0.0360	
97.73	228.15	0.0360	
123.60	227.08	0.0360	
126.48	226.61	0.0360	
127.60	226.47	0.0360 / 0.0330	Main Channel
127.85	225.25	0.0330	Main Channel
129.35	225.25	0.0330	Main Channel
130.85	225.25	0.0330	Main Channel
131.10	226.47	0.0330 / 0.0360	Main Channel
132.22	226.59	0.0360	
143.72	227.42	0.0360	
169.59	227.88	0.0360	
192.59	231.19	0.0360	
218.46	233.02	0.0360	
241.45	235.50	0.0360	
264.57	236.43	0.0360	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELRV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.41	225.66	.391E+04	1.7	1.36	38.31
0.81	226.06	.804E+04	5.0	1.93	27.02
1.22	226.47	1.24E+05	9.1	2.30	22.61
1.84	227.09	3.08E+05	23.3	2.36	22.03
2.46	227.71	9.04E+05	62.0	2.14	24.31
3.09	228.34	2.23E+06	188.3	2.63	19.75
3.71	228.96	3.79E+06	410.2	3.38	15.39
4.33	229.58	5.50E+06	711.5	4.04	12.89
4.95	230.20	7.37E+06	1091.3	4.62	11.25
5.58	230.83	9.39E+06	1550.6	5.15	10.10
6.20	231.45	1.18E+07	2072.4	5.59	9.31
6.82	232.07	1.40E+07	2659.2	5.93	8.77
7.44	232.69	1.67E+07	3345.8	6.27	8.30
8.07	233.32	1.95E+07	4159.0	6.64	7.83
8.69	233.94	2.26E+07	5100.8	7.03	7.39
9.31	234.56	2.59E+07	6081.3	7.32	7.11
9.93	235.18	2.95E+07	7183.1	7.60	6.84
10.56	235.81	3.34E+07	8345.2	7.62	6.83
11.18	236.43	3.79E+07	9116.0	7.51	6.92

<----- hydrograph -----> <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8268)	422.16	4.86	6.75	19.52	0.80	1.90
OUTFLOW: ID= 1 (6402)	422.16	4.63	7.25	19.52	0.77	1.84

```

-----
| ADD HYD (8266) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0402): 244.00 2.567 7.50 23.90
+ ID2= 2 (6402): 422.16 4.628 7.25 19.52
=====
ID = 3 (8266): 666.16 7.186 7.25 21.12
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8264) |
  
```

```

-----
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0404): 246.46 1.795 7.25 15.70
+ ID2= 2 (8266): 666.16 7.186 7.25 21.12
=====
ID = 3 (8264): 912.62 8.981 7.25 19.66
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8262) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0406): 342.65 2.779 6.50 27.18
+ ID2= 2 (8264): 912.62 8.981 7.25 19.66
=====
ID = 3 (8262): 1055.27 11.503 7.25 20.68
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8260) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (5506): 5660.52 32.437 12.00 21.36
+ ID2= 2 (8262): 1055.27 11.503 7.25 20.68
=====
ID = 3 (8260): 6715.79 37.738 10.25 21.25
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8258) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0408): 231.62 3.331 6.75 21.68
+ ID2= 2 (8260): 6715.79 37.738 10.25 21.25
=====
ID = 3 (8258): 6947.41 39.031 10.25 21.27
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8256) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0410): 572.01 3.210 8.25 16.28
+ ID2= 2 (8258): 6947.41 39.031 10.25 21.27
=====
ID = 3 (8256): 7519.42 41.844 10.25 20.89
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| RESERVOIR (5507) |
| IM= 2--> OUT= 1 |
| DT= 15.0 min |
-----
          OUTFLOW STORAGE | OUTFLOW STORAGE
          (cms) (ha.m.) | (cms) (ha.m.)
-----
          0.0000 0.0000 | 90.6140 67.8416
          39.6440 12.3348 | ***** 160.3529
          48.1390 18.5023 | ***** 160.3629
          67.9600 37.0045 | 0.0000 0.0000
  
```

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8256)	7519.421	41.844	10.25	20.89
OUTFLOW: ID= 1 (5507)	7519.421	40.140	11.75	20.89

PEAK FLOW REDUCTION [Qout/qin](%)= 95.93
 TIME SHIFT OF PEAK FLOW (min)= 90.00
 MAXIMUM STORAGE USED (ha.m.)= 12.6987

```

-----
| CALIB (0420) |
| ID= 1 DT=15.0 min |
-----
Area (ha)= 175.82 Curve Number (CN)= 53.0
Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.81
  
```

Unit Hyd Qpeak (cms)= 3.692

PEAK FLOW (cms)= 1.800 (l)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 18.821
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.233

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

0.0000 0.0000 | 3.4720 1.9177
0.3820 0.3152 | 3.8720 1.9277
1.8280 0.4690 | 0.0000 0.0000

CALIB
NASHYD (0418) Area (ha)= 182.79 Curve Number (CN)= 64.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.05

Unit Hyd Opeak (cms)= 2.966

PEAK FLOW (cms)= 2.136 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 26.026
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.322

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0416) Area (ha)= 439.30 Curve Number (CN)= 64.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.29

Unit Hyd Opeak (cms)= 5.832

PEAK FLOW (cms)= 4.419 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 26.090
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.323

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0412) Area (ha)= 238.70 Curve Number (CN)= 54.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.80

Unit Hyd Opeak (cms)= 5.088

PEAK FLOW (cms)= 2.551 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 19.387
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.240

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (4141) Area (ha)= 43.70 Dir. Conn.(%)= 17.00
ID= 1 DT=15.0 min Total Imp(%)= 36.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 15.73 27.97
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 539.75 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 106.66 62.65
over (min) 15.00 30.00
Storage Coeff. (min)= 8.43 (ii) 21.33 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.09 0.05

TOTALS

PEAK FLOW (cms)= 1.95 2.39 3.467 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 78.80 31.64 39.66
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.39 0.49

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5414)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (4141) 43.700 3.467 6.00 39.66
OUTFLOW: ID= 1 (5414) 43.700 1.932 6.50 39.66

PEAK FLOW REDUCTION [Qout/Qin](%)= 55.72
TIME SHIFT OF PEAK FLOW (min)= 30.00
MAXIMUM STORAGE USED (ha.m.)= 0.5610

CALIB
STANDHYD (4142) Area (ha)= 144.30 Dir. Conn.(%)= 10.00
ID= 1 DT=15.0 min Total Imp(%)= 21.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 30.30 114.00
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 980.82 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 106.66 50.20
over (min) 15.00 30.00
Storage Coeff. (min)= 12.06 (ii) 26.15 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04

TOTALS

PEAK FLOW (cms)= 3.39 7.05 8.447 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.25
RUNOFF VOLUME (mm)= 78.80 25.93 33.92
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.36 0.42

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8254)
1 + 2 = 3 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (4142): 144.30 8.447 6.25 33.92
+ ID2= 1 (5414): 43.70 1.932 6.50 39.65
=====

ID = 3 (8254): 188.00 10.322 6.25 35.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8252)
1 + 2 = 3 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0412): 238.70 2.551 7.00 19.39
+ ID2= 2 (8254): 188.00 10.322 6.25 35.25
=====

ID = 3 (8252): 426.70 12.418 6.25 26.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6416) Routing time step (min)= 15.00
IN= 2--> OUT= 1

<----- DATA FOR SECTION (4161.0) ----->

Distance Elevation Manning
0.00 270.07 0.0340
20.67 267.91 0.0340
62.01 264.33 0.0340
113.69 259.75 0.0340
165.37 253.30 0.0340
227.38 246.29 0.0340
232.55 246.03 0.0340
237.72 246.16 0.0340
241.39 246.02 0.0340 / 0.0300 Main Channel
241.64 245.75 0.0300 Main Channel
242.89 245.75 0.0300 Main Channel
244.14 245.75 0.0300 Main Channel
244.39 246.02 0.0300 / 0.0340 Main Channel
248.06 246.20 0.0340

253.22	246.28	0.0340
258.39	246.63	0.0340
346.25	252.57	0.0340
413.43	257.77	0.0340
465.11	261.78	0.0340
511.62	270.00	0.0340

TRAVEL TIME TABLE						
DEPTH	SEGV	VOLUME	FLOW RATE	VELOCITY	TRAV TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.27	246.02	.295E+04	0.6	0.75	88.26	
1.53	247.28	.168E+06	69.7	1.65	40.13	
2.79	248.54	.490E+06	299.5	2.42	27.28	
4.06	249.81	.962E+06	740.1	3.05	21.67	
5.32	251.07	.158E+07	1440.0	3.61	18.33	
6.58	252.33	.235E+07	2443.8	4.12	16.06	
7.84	253.59	.327E+07	3811.2	4.63	14.30	
9.10	254.85	.432E+07	5565.1	5.11	12.94	
10.37	256.12	.550E+07	7721.3	5.57	11.88	
11.63	257.38	.682E+07	10309.9	6.00	11.02	
12.89	258.64	.827E+07	13360.6	6.42	10.31	
14.15	259.90	.985E+07	16884.5	6.81	9.72	
15.42	261.17	.116E+08	20816.4	7.14	9.26	
16.68	262.43	.134E+08	25510.4	7.53	8.78	
17.94	263.69	.154E+08	30958.0	7.97	8.30	
19.20	264.95	.175E+08	36988.3	8.38	7.89	
20.46	266.21	.197E+08	43617.1	8.78	7.53	
21.73	267.48	.220E+08	50868.1	9.17	7.22	
22.99	268.74	.244E+08	58854.0	9.56	6.92	

<---- hydrograph ----> <-pipe / channel->						
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8252)	426.70	12.42	6.25	26.38	0.49	0.83
OUTFLOW : ID= 1 (6416)	426.70	5.93	7.25	26.38	0.37	0.78

ADD HYD (8250)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0416):	439.30	4.419	8.00	26.09	
+ ID2= 2 (6416):	426.70	5.929	7.25	26.38	
ID = 3 (8250):	866.00	10.218	7.25	26.23	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8248)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0418):	182.79	2.136	7.50	26.03	
+ ID2= 2 (8250):	866.00	10.218	7.25	26.23	
ID = 3 (8248):	1048.79	12.352	7.25	26.20	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8246)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0420):	175.82	1.800	7.00	18.82	
+ ID2= 2 (8248):	1048.79	12.352	7.25	26.20	
ID = 3 (8246):	1224.61	14.132	7.25	25.14	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8244)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (5507):	7519.42	40.140	11.75	20.89	
+ ID2= 2 (8246):	1224.61	14.132	7.25	25.14	
ID = 3 (8244):	8744.03	48.352	10.50	21.48	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8242)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	

ID1= 1 (0422): 780.20 4.215 9.00 19.59
+ ID2= 2 (8244): 8744.03 48.352 10.50 21.48
=====

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5508)				
IN= 2-->	OUT= 1			
DT= 15.0 min				
OUTFLOW	STORAGE	OUTFLOW	STORAGE	
(cms)	(ha.m.)	(cms)	(mm)	
0.0000	0.0000	*****	197.3574	
76.4550	30.8371	*****	394.7148	
*****	61.6742	*****	394.7248	
*****	*****	0.0000	0.0000	

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8242)	9524.231	52.389	10.50	21.33
OUTFLOW : ID= 1 (5508)	9524.231	50.303	12.00	21.33

PEAK FLOW REDUCTION [Qout/Qin](%)= 96.02			
TIME SHIFT OF PEAK FLOW (min)= 90.00			
MAXIMUM STORAGE USED (ha.m.)= 20.2951			

CALIB (0336)			
NASHYD	Area	Curve Number	(CN)= 72.0
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 15.39			

Unit Hyd Qpeak (cms)= 3.090

PEAK FLOW (cms)= 3.992 (i)
TIME TO PEAK (hrs)= 22.250
RUNOFF VOLUME (mm)= 32.865
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.407

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (3382)			
ID= 1 DT=15.0 min	Area	Total Imp(%)	Dir. Conn.(%)= 17.00
(ha)= 525.90			

IMPERVIOUS PERVIOUS (i)		
Surface Area (ha)=	194.58	331.32
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1872.43	40.00
Mannings n =	0.013	0.250

Max. Ref. Inten.(mm/hr)=	106.66	67.85
over (min)	15.00	45.00
Storage Coeff. (min)=	17.78 (ii)	30.27 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.06	0.03

TOTALS		
PEAK FLOW (cms)=	17.87	23.62
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	78.80	33.63
TOTAL RAINFALL (mm)=	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.42
		0.51

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 66.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (3381)			
ID= 1 DT=15.0 min	Area	Total Imp(%)	Dir. Conn.(%)= 23.00
(ha)= 73.00			

IMPERVIOUS PERVIOUS (i)		
Surface Area (ha)=	35.77	37.23
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	697.61	40.00
Mannings n =	0.013	0.250

Max. Ref. Inten.(mm/hr)=	106.66	84.70
over (min)	15.00	30.00
Storage Coeff. (min)=	9.83 (ii)	21.26 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00

Unit Hyd. peak (cms)= 0.09 0.05 *TOTALS*
 PEAK FLOW (cms)= 4.22 4.33 7.010 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 78.80 36.58 46.29
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.45 0.57

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5338)
 ID= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.3350	1.2400
0.3230	0.3074	4.3560	1.3520
0.9360	0.5374	5.3290	1.4590
1.7590	0.8302	5.7290	1.4690
2.4110	1.0550	0.0000	0.0000

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (3381) 73.000 7.010 6.00 46.29
 OUTFLOW: ID= 1 (5338) 73.000 3.356 6.50 46.29

PEAK FLOW REDUCTION [Qout/Qin]= 47.87
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 1.2583

ADD HYD (8310)
 1 + 2 = 3

Area (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
525.90	29.867	6.50	41.31
73.00	3.356	6.50	46.29

ID= 1 (3382): 525.90 29.867 6.50 41.31
 + ID2= 2 (5338): 73.00 3.356 6.50 46.29
 ID = 3 (8310): 598.90 33.223 6.50 41.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (3342) Area (ha)= 587.10 Curve Number (CN)= 56.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 8.19

Unit Hyd Qpeak (cms)= 1.224

PEAK FLOW (cms)= 0.993 (i)
 TIME TO PEAK (hrs)= 15.250
 RUNOFF VOLUME (mm)= 20.856
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.258

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (3341) Area (ha)= 33.00 Dir. Conn.(%)= 31.00
 ID= 1 DT=15.0 min Total Imp(%)= 51.00

Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
16.83	16.17	
2.00	5.00	
0.50	0.50	
469.04	40.00	
0.013	0.250	

Max. Eff. Inten.(mm/hr)= 106.66 51.83
 over (min) 15.00 30.00
 Storage Coeff. (min)= 7.75 (ii) 21.66 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.10 0.05

PEAK FLOW (cms)= 2.74 1.13 3.444 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 78.80 24.48 41.32
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.30 0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

CN* = 52.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5334)
 ID= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.6540	0.4604
0.0390	0.1659	0.8330	0.5249
0.1860	0.2610	1.0130	0.5896
0.3750	0.3534	1.4130	0.5996
0.4340	0.3813	0.0000	0.0000

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (3341) 33.000 3.444 6.00 41.32
 OUTFLOW: ID= 1 (5334) 33.000 1.579 6.50 41.29

PEAK FLOW REDUCTION [Qout/Qin]= 45.84
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 0.6194

ADD HYD (8314)
 1 + 2 = 3

Area (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
587.10	0.993	15.25	20.86
33.00	1.579	6.50	41.29

ID= 1 (3342): 587.10 0.993 15.25 20.86
 + ID2= 2 (5334): 33.00 1.579 6.50 41.29
 ID = 3 (8314): 620.10 1.884 6.50 21.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0306) Area (ha)= 283.97 Curve Number (CN)= 52.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 6.44

Unit Hyd Qpeak (cms)= 0.753

PEAK FLOW (cms)= 0.538 (i)
 TIME TO PEAK (hrs)= 13.750
 RUNOFF VOLUME (mm)= 18.507
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.229

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0286) Area (ha)= 260.51 Curve Number (CN)= 84.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.16

Unit Hyd Qpeak (cms)= 3.834

PEAK FLOW (cms)= 5.156 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 45.890
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.568

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0282) Area (ha)= 449.38 Curve Number (CN)= 77.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.47

Unit Hyd Qpeak (cms)= 5.226

PEAK FLOW (cms)= 5.992 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 37.664
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.466

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0284) Area (ha)= 78.93 Curve Number (CN)= 84.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50

U.H. Tp(hrs)= 0.57

Unit Hyd Qpeak (cms)= 2.344

PEAK FLOW (cms)= 2.746 (i)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 45.199
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.559

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 5 columns: ADD HYD (8388), AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Rows include ID1=1 (0282), ID2=2 (0284), and ID=3 (8388).

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CIN (6286) Routing time step (min)'= 15.00
IN= 2--> OUT= 1

Table with 4 columns: Distance, Elevation, Manning, Routing time step (min)'= 15.00. Lists distance and elevation points along a channel.

Table with 6 columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV.TIME. Shows hydrograph data for various depths and elevations.

Table with 6 columns: AREA, QPEAK, TPEAK, R.V., MAX DEPTH, MAX VEL. Includes inflow and outflow data for ID=2 (8388) and ID=1 (6286).

Table with 5 columns: ADD HYD (8386), AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Rows include ID1=1 (0286), ID2=2 (6286), and ID=3 (8386).

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with 4 columns: CALIB, NASHYD (0302), Area (ha), Curve Number (CN)= 58.0. Includes ID=1 DT=15.0 min and U.H. Tp(hrs)= 1.66.

Unit Hyd Qpeak (cms)= 4.874

PEAK FLOW (cms)= 3.272 (i)
TIME TO PEAK (hrs)= 8.500
RUNOFF VOLUME (mm)= 22.015
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.272

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 4 columns: CALIB, NASHYD (0300), Area (ha), Curve Number (CN)= 52.0. Includes ID=1 DT=15.0 min and U.H. Tp(hrs)= 1.03.

Unit Hyd Qpeak (cms)= 4.292

PEAK FLOW (cms)= 2.189 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 18.764
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.232

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 4 columns: CALIB, NASHYD (0298), Area (ha), Curve Number (CN)= 45.0. Includes ID=1 DT=15.0 min and U.H. Tp(hrs)= 1.26.

Unit Hyd Qpeak (cms)= 4.461

PEAK FLOW (cms)= 1.875 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 14.769
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.183

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 5 columns: ADD HYD (8395), AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Rows include ID1=1 (0298), ID2=2 (0300), and ID=3 (8395).

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with 4 columns: CALIB, NASHYD (0288), Area (ha), Curve Number (CN)= 78.0. Includes ID=1 DT=15.0 min and U.H. Tp(hrs)= 2.21.

Unit Hyd Qpeak (cms)= 2.629

PEAK FLOW (cms)= 3.362 (i)
TIME TO PEAK (hrs)= 9.250
RUNOFF VOLUME (mm)= 38.847
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.481

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 4 columns: CALIB, NASHYD (0290), Area (ha), Curve Number (CN)= 78.0. Includes ID=1 DT=15.0 min and U.H. Tp(hrs)= 1.07.

Unit Hyd Qpeak (cms)= 4.279

PEAK FLOW (cms)= 4.704 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 38.611
TOTAL RAINFALL (mm)= 80.800

RUNOFF COEFFICIENT = 0.478

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8397)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0288): 340.83 3.362 9.25 38.85
+ ID2= 2 (0290): 269.18 4.704 7.25 38.61
=====
ID = 3 (8397): 610.01 7.793 8.00 38.74

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB (0296) Area (ha)= 293.65 Curve Number (CN)= 76.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.13

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```

Unit Hyd Qpeak (cms)= 4.437
PEAK FLOW (cms)= 4.643 (1)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 36.515
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.452

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB (0292) Area (ha)= 738.49 Curve Number (CN)= 68.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.52

```

```

Unit Hyd Qpeak (cms)= 8.289
PEAK FLOW (cms)= 7.336 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 29.254
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.362

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB (0294) Area (ha)= 274.15 Curve Number (CN)= 57.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.87

```

```

Unit Hyd Qpeak (cms)= 5.367
PEAK FLOW (cms)= 3.004 (1)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 21.217
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.263

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8398)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0292): 738.49 7.336 8.00 29.25
+ ID2= 2 (0294): 274.15 3.004 7.00 21.22
=====
ID = 3 (8398): 1012.64 10.132 8.00 27.08

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6296)
IN= 2--> OUT= 1 Routing time step (min)= 15.00

```

```

<----- DATA FOR SECTION (2961.0) ----->
Distance Elevation Manning
0.00 243.98 0.0400
42.59 243.18 0.0400
85.17 241.81 0.0400
120.66 240.50 0.0400
156.15 239.56 0.0400
198.74 236.15 0.0400
237.78 234.01 0.0400

```

```

241.33 233.82 0.0400
248.77 233.12 0.0400 /0.0400 Main Channel
249.87 232.32 0.0400 Main Channel
250.37 231.80 0.0400 Main Channel
250.87 232.23 0.0400 Main Channel
251.97 233.10 0.0400 /0.0400 Main Channel
255.37 233.22 0.0400
259.07 233.87 0.0400
262.62 234.12 0.0400
266.17 234.23 0.0400
283.91 234.73 0.0400
337.15 241.75 0.0400
351.34 244.00 0.0400

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.43 232.23 .510E+03 0.1 0.53 81.07
0.87 232.67 .213E+04 0.7 0.83 51.49
1.30 233.10 .501E+04 2.2 1.10 38.75
1.98 233.78 .242E+05 12.6 1.34 31.93
2.66 234.46 .738E+05 42.1 1.46 29.21
3.34 235.14 .177E+06 127.9 1.84 23.13
4.02 235.82 .314E+06 281.4 2.29 18.61
4.70 236.50 .481E+06 509.4 2.71 15.73
5.38 237.18 .672E+06 815.6 3.11 13.73
6.06 237.86 .886E+06 1200.7 3.46 12.30
6.74 238.54 .112E+07 1669.4 3.80 11.23
7.42 239.22 .139E+07 2226.9 4.11 10.38
8.10 239.90 .168E+07 2808.4 4.28 9.95
8.78 240.58 .202E+07 3470.4 4.40 9.68
9.46 241.26 .240E+07 4331.7 4.61 9.24
10.14 241.94 .283E+07 5335.2 4.83 8.84
10.82 242.62 .330E+07 6482.6 5.03 8.48
11.50 243.30 .381E+07 7764.1 5.21 8.18
12.18 243.98 .439E+07 9101.2 5.31 8.04

```

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8398) 1012.64 10.13 8.00 27.08 1.82 1.27
OUTFLOW : ID= 1 (6296) 1012.64 9.86 8.50 27.08 1.80 1.26

```

```

ADD HYD (8396)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0296): 293.65 4.643 7.50 36.52
+ ID2= 2 (6296): 1012.64 9.865 8.50 27.08
=====
ID = 3 (8396): 1306.29 14.310 8.25 29.20

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8394)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8396): 1306.29 14.310 8.25 29.20
+ ID2= 2 (8397): 610.01 7.793 8.00 38.74
=====
ID = 3 (8394): 1916.30 22.076 8.00 32.24

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8392)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8394): 1916.30 22.076 8.00 32.24
+ ID2= 2 (8395): 589.44 4.038 7.75 16.52
=====
ID = 3 (8392): 2505.74 26.095 8.00 28.54

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6302)
IN= 2--> OUT= 1 Routing time step (min)= 15.00

```

```

<----- DATA FOR SECTION (3021.0) ----->
Distance Elevation Manning
0.00 228.10 0.0400
18.47 227.12 0.0400
36.95 226.12 0.0400
46.18 225.84 0.0400
55.42 225.58 0.0400

```

272.47	222.88	0.0400	
277.09	222.76	0.0400	
281.71	222.58	0.0400	
288.54	222.18	0.0400 / 0.0300	Main Channel
288.64	221.00	0.0300	Main Channel
290.94	221.00	0.0300	Main Channel
291.04	221.00	0.0300	Main Channel
291.54	222.75	0.0300 / 0.0400	Main Channel
300.18	222.83	0.0400	
304.80	223.04	0.0400	
309.42	223.25	0.0400	
318.65	223.69	0.0400	
360.22	225.57	0.0400	
397.16	227.60	0.0400	
457.20	228.35	0.0400	

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.29	221.29	228E+04	0.2	0.32	165.63
0.59	221.59	465E+04	0.7	0.45	116.15
0.88	221.88	713E+04	1.2	0.54	96.87
1.18	222.18	971E+04	1.9	0.61	86.20
1.57	222.57	115E+05	3.3	0.59	88.86
1.97	222.97	427E+05	6.4	0.47	111.39
2.36	223.36	115E+06	16.1	0.44	119.14
2.76	223.76	237E+06	37.1	0.49	106.49
3.15	224.15	409E+06	72.9	0.56	93.40
3.55	224.55	631E+06	127.0	0.63	82.76
3.94	224.94	903E+06	202.4	0.70	74.37
4.34	225.34	123E+07	301.8	0.77	67.68
4.73	225.73	160E+07	433.5	0.85	61.37
5.13	226.13	200E+07	602.2	0.95	55.29
5.52	226.52	242E+07	804.7	1.05	50.13
5.92	226.92	286E+07	1034.2	1.14	46.11
6.31	227.31	332E+07	1290.5	1.22	42.88
6.71	227.71	380E+07	1560.9	1.29	40.55
7.10	228.10	432E+07	1835.6	1.34	39.19

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8392) 2505.74	26.10	8.00	28.54	2.55	0.46
OUTFLOW: ID= 1 (6302) 2505.74	20.61	10.25	28.54	2.45	0.45

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0302):	473.90	3.272	8.50	22.02
+ ID2= 2 (6302):	2505.74	20.608	10.25	28.54
ID = 3 (8390):	2979.64	23.639	10.00	27.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8386):	788.82	11.552	8.50	41.13
+ ID2= 2 (8390):	2979.64	23.639	10.00	27.50
ID = 3 (8348):	3768.46	34.216	9.50	30.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	NASHYD (0204)	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res.(N)
		292.27	5.00	63.0	1.50
			2.78		

Unit Hyd Qpeak (cms)= 1.793
 PEAK FLOW (cms)= 1.571 (i)
 TIME TO PEAK (hrs)= 10.250
 RUNOFF VOLUME (mm)= 25.482
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.315

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD (0280)	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res.(N)
		299.86	5.00	82.0	1.50
			0.85		

Unit Hyd Qpeak (cms)= 6.009

PEAK FLOW (cms)= 7.151 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 43.109
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.534

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD (0278)	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res.(N)
		485.49	5.00	82.0	1.50
			1.52		

Unit Hyd Qpeak (cms)= 5.453

PEAK FLOW (cms)= 7.318 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 43.435
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.538

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD (0276)	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res.(N)
		90.89	5.00	79.0	1.50
			0.67		

Unit Hyd Qpeak (cms)= 2.302

PEAK FLOW (cms)= 2.373 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 39.356
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.487

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD (0274)	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res.(N)
		392.49	5.00	79.0	1.50
			1.08		

Unit Hyd Qpeak (cms)= 6.182

PEAK FLOW (cms)= 6.216 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 35.482
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.439

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0274):	392.49	6.216	7.50	35.48
+ ID2= 2 (0276):	90.89	2.373	6.75	39.36
ID = 3 (8360):	483.38	8.455	7.00	36.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0278):	485.49	7.318	8.00	43.44
+ ID2= 2 (8360):	483.38	8.455	7.00	36.21
ID = 3 (8358):	968.87	15.469	7.50	39.83

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6280)	Routing time step (min)
	15.00

----- DATA FOR SECTION (2801.0) -----
 Distance Elevation Manning

0.00	241.14	0.0500	
13.32	240.80	0.0500	
39.95	240.07	0.0500	
96.54	236.21	0.0500	
113.19	234.15	0.0500	
123.18	232.35	0.0500	
143.15	225.80	0.0500	
149.81	225.62	0.0500	
153.14	225.40	0.0500	
157.30	224.76	0.0500 / 0.0300	Main Channel
159.80	224.26	0.0300	Main Channel
162.30	224.85	0.0300	Main Channel
162.55	224.97	0.0300 / 0.0500	Main Channel
167.80	225.05	0.0500	
186.43	229.14	0.0500	
213.06	234.75	0.0500	
236.37	237.09	0.0500	
266.33	237.31	0.0500	
292.96	237.83	0.0500	
329.58	241.50	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.50	224.76	.263E+04	0.8	0.65	58.31
1.36	225.62	.278E+05	13.4	1.10	34.54
2.22	226.48	.855E+05	52.6	1.40	27.09
3.09	227.35	.157E+06	120.4	1.74	21.78
3.95	228.21	.242E+06	216.3	2.04	18.64
4.81	229.07	.340E+06	341.8	2.29	16.56
5.67	229.93	.450E+06	498.3	2.52	15.06
6.53	230.79	.574E+06	688.3	2.73	13.90
7.40	231.66	.711E+06	913.8	2.93	12.97
8.26	232.52	.861E+06	1173.4	3.11	12.23
9.12	233.38	.103E+07	1461.3	3.24	11.72
9.98	234.24	.121E+07	1793.3	3.37	11.26
10.85	235.11	.142E+07	2141.0	3.45	11.02
11.71	235.97	.165E+07	2530.4	3.50	10.86
12.57	236.83	.192E+07	2954.0	3.51	10.81
13.43	237.69	.226E+07	3160.3	3.18	11.93
14.29	238.55	.271E+07	3869.4	3.26	11.67
15.16	239.42	.320E+07	4740.3	3.38	11.24
16.02	240.28	.373E+07	5688.4	3.48	10.92

<---- hydrograph ----> <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8358)	968.87	15.47	7.50	39.83	1.41
OUTFLOW : ID= 1 (6280)	968.87	15.03	8.25	39.83	1.40

ADD HYD (8354)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0280):	299.86	7.151	7.00	43.11
+ ID2= 2 (6280):	968.87	15.029	8.25	39.83
ID = 3 (8354):	1268.73	21.461	7.75	40.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	NASHVD (0272)	Area (ha)	(ha)	Curve Number (CN)
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50		
	U.H. Tp(hrs)= 1.09			
Unit Hyd Opeak (cms)=	2.456			
PEAK FLOW (cms)=	2.475 (1)			
TIME TO PEAK (hrs)=	7.500			
RUNOFF VOLUME (mm)=	35.486			
TOTAL RAINFALL (mm)=	80.800			
RUNOFF COEFFICIENT =	0.439			

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHVD (0270)	Area (ha)	(ha)	Curve Number (CN)
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.30		
	U.H. Tp(hrs)= 0.87			
Unit Hyd Opeak (cms)=	3.429			
PEAK FLOW (cms)=	4.097 (1)			
TIME TO PEAK (hrs)=	7.250			
RUNOFF VOLUME (mm)=	41.870			
TOTAL RAINFALL (mm)=	80.800			
RUNOFF COEFFICIENT =	0.518			

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHVD (0268)	Area (ha)	(ha)	Curve Number (CN)
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.30		
	U.H. Tp(hrs)= 0.69			

Unit Hyd Opeak (cms)= 3.807

PEAK FLOW (cms)=	3.621 (1)
TIME TO PEAK (hrs)=	7.000
RUNOFF VOLUME (mm)=	35.162
TOTAL RAINFALL (mm)=	80.800
RUNOFF COEFFICIENT =	0.435

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHVD (0264)	Area (ha)	(ha)	Curve Number (CN)
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.30		
	U.H. Tp(hrs)= 1.30			

Unit Hyd Opeak (cms)= 3.313

PEAK FLOW (cms)=	3.033 (1)
TIME TO PEAK (hrs)=	8.000
RUNOFF VOLUME (mm)=	30.013
TOTAL RAINFALL (mm)=	80.800
RUNOFF COEFFICIENT =	0.371

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHVD (0266)	Area (ha)	(ha)	Curve Number (CN)
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.30		
	U.H. Tp(hrs)= 1.63			

Unit Hyd Opeak (cms)= 3.786

PEAK FLOW (cms)=	3.140 (1)
TIME TO PEAK (hrs)=	9.000
RUNOFF VOLUME (mm)=	26.119
TOTAL RAINFALL (mm)=	80.800
RUNOFF COEFFICIENT =	0.323

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8380)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0264):	353.96	3.033	8.00	30.01
+ ID2= 2 (0266):	508.09	3.140	9.00	26.12
ID = 3 (8380):	862.05	6.146	8.50	27.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6268)	Routing time step (min)=
IN= 2--> OUT= 1	15.00

<----- DATA FOR SECTION (2681.0) ----->

Distance	Elevation	Manning
0.00	274.03	0.0500
10.55	272.73	0.0500
21.10	271.28	0.0500
50.12	266.83	0.0500
87.04	260.36	0.0500
92.32	260.06	0.0500
94.95	259.93	0.0500
101.00	256.87	0.0500
103.00	256.58	0.0500 / 0.0350
104.00	256.32	0.0350
106.00	256.05	0.0350
108.00	256.25	0.0350
110.78	256.65	0.0350 / 0.0500
113.42	256.81	0.0500
116.06	257.00	0.0500
155.62	260.24	0.0500
187.27	263.35	0.0500
211.01	267.22	0.0500
224.20	269.60	0.0500
261.13	272.68	0.0500

----- TRAVEL TIME TABLE ----->

Table with 7 columns: DEPTH (m), ELEV (m), VOLUME (cu.m.), FLOW RATE (cms), VELOCITY (m/s), TRAV.TIME (min). Rows show data for various depths from 0.53 to 15.78 meters.

<---- hydrograph ----> <-pipe / channel-->

Summary hydrograph table with columns: AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm), MAX DEPTH (m), MAX VEL (m/s). Includes INFLOW and OUTFLOW data for ID=2 and ID=1.

ADD HYD (8382) summary table showing area, peak, and time for multiple IDs (1, 2, 3) and their combined total.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6270) Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2701.0) ----->

Table with 5 columns: Distance, Elevation, Manning, Channel. Lists data for various distances from 0.00 to 201.42, including channel types like 'Main Channel'.

----- TRAVEL TIME TABLE ----->

Table with 7 columns: DEPTH (m), ELEV (m), VOLUME (cu.m.), FLOW RATE (cms), VELOCITY (m/s), TRAV.TIME (min). Rows show data for various depths from 0.45 to 9.94 meters.

<---- hydrograph ----> <-pipe / channel-->

Summary hydrograph table with columns: AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm), MAX DEPTH (m), MAX VEL (m/s). Includes INFLOW and OUTFLOW data for ID=2 and ID=1.

Summary hydrograph table for the top section, including INFLOW and OUTFLOW data.

ADD HYD (8384) summary table showing area, peak, and time for multiple IDs (1, 2, 3) and their combined total.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0262) summary table showing area, peak, and time for ID=1.

Unit Hyd Qpeak (cms)= 4.094

Summary table for ID=1: PEAK FLOW, TIME TO PEAK, RUNOFF VOLUME, TOTAL RAINFALL, RUNOFF COEFFICIENT.

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0260) summary table showing area, peak, and time for ID=1.

Unit Hyd Qpeak (cms)= 4.360

Summary table for ID=1: PEAK FLOW, TIME TO PEAK, RUNOFF VOLUME, TOTAL RAINFALL, RUNOFF COEFFICIENT.

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0258) summary table showing area, peak, and time for ID=1.

Unit Hyd Qpeak (cms)= 1.881

Summary table for ID=1: PEAK FLOW, TIME TO PEAK, RUNOFF VOLUME, TOTAL RAINFALL, RUNOFF COEFFICIENT.

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0252) summary table showing area, peak, and time for ID=1.

Unit Hyd Qpeak (cms)= 3.761

Summary table for ID=1: PEAK FLOW, TIME TO PEAK, RUNOFF VOLUME, TOTAL RAINFALL, RUNOFF COEFFICIENT.

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0256) summary table showing area, peak, and time for ID=1.

Unit Hyd Qpeak (cms)= 1.707

PEAK FLOW (cms)= 1.403 (1)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 28.295
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.350

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----
| CALIB |
| NASHVD (0254) | Area (ha)= 403.00 Curve Number (CN)= 55.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
|-----|
| U.H. Tp(hrs)= 1.22
```

Unit Hyd Qpeak (cms)= 4.028

PEAK FLOW (cms)= 2.406 (1)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 20.083
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.249

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----
| ROUTE CHN (6256) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
```

----- DATA FOR SECTION (2561.0) ----->

Distance	Elevation	Manning
0.00	276.07	0.0400
11.68	273.71	0.0400
23.36	271.35	0.0400
29.19	270.30	0.0400
35.03	269.44	0.0400
45.47	267.90	0.0400
78.82	266.24	0.0400
90.50	265.63	0.0400
102.18	265.40	0.0400
105.10	264.95	0.0400 /0.0350 Main Channel
108.02	264.39	0.0350 Main Channel
110.94	264.72	0.0350 Main Channel
113.86	265.19	0.0350 /0.0400 Main Channel
116.78	265.49	0.0400
143.05	268.24	0.0400
172.25	270.53	0.0400
207.28	271.95	0.0400
233.55	273.72	0.0400
256.91	274.98	0.0400
289.02	275.97	0.0400

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.56	264.95	.723E+04	1.9	0.90	62.51
1.14	265.53	.307E+05	13.5	1.49	37.92
1.72	266.11	.951E+05	48.6	1.73	32.62
2.30	266.69	.191E+06	117.8	2.09	27.03
2.88	267.27	.314E+06	225.2	2.43	23.24
3.46	267.85	.464E+06	375.5	2.74	20.59
4.04	268.43	.641E+06	573.2	3.03	18.62
4.62	269.01	.846E+06	821.9	3.29	17.15
5.20	269.59	.108E+07	1135.0	3.56	15.87
5.78	270.17	.134E+07	1525.1	3.86	14.64
6.36	270.75	.162E+07	1971.1	4.12	13.71
6.94	271.33	.194E+07	2474.0	4.33	13.04
7.52	271.91	.228E+07	3065.0	4.55	12.42
8.10	272.49	.266E+07	3773.4	4.80	11.75
8.68	273.07	.308E+07	4566.3	5.05	11.17
9.26	273.65	.348E+07	5443.7	5.30	10.66
9.84	274.23	.393E+07	6389.6	5.51	10.25
10.42	274.81	.440E+07	7426.2	5.72	9.88
11.00	275.39	.490E+07	8490.1	5.86	9.63

<----- hydrograph -----> <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0254)	403.00	2.41	8.00	20.08	0.58
OUTFLOW : ID= 1 (6256)	403.00	2.26	9.75	20.08	0.58

```
-----
| ADD HYD (8370) |
| 1 + 2 = 3 |
|-----|
| ID1= 1 (0256): | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
| + ID2= 2 (6256): | 403.00 2.263 9.75 20.08
|-----|
| ID = 3 (8370): | 548.79 3.538 9.00 22.26
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
-----
| CALIB |
| NASHVD (0250) | Area (ha)= 192.88 Curve Number (CN)= 70.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
|-----|
| U.H. Tp(hrs)= 1.22
```

Unit Hyd Qpeak (cms)= 1.930

PEAK FLOW (cms)= 1.797 (1)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 30.845
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.382

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----
| CALIB |
| NASHVD (0246) | Area (ha)= 759.61 Curve Number (CN)= 55.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
|-----|
| U.H. Tp(hrs)= 1.81
```

Unit Hyd Qpeak (cms)= 5.121

PEAK FLOW (cms)= 3.349 (1)
 TIME TO PEAK (hrs)= 10.000
 RUNOFF VOLUME (mm)= 20.153
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.249

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----
| CALIB |
| NASHVD (0248) | Area (ha)= 146.04 Curve Number (CN)= 64.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
|-----|
| U.H. Tp(hrs)= 0.78
```

Unit Hyd Qpeak (cms)= 2.271

PEAK FLOW (cms)= 1.592 (1)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 25.873
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.320

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----
| ADD HYD (8364) |
| 1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
|-----|
| ID1= 1 (0246): | 759.61 3.349 10.00 20.15
| + ID2= 2 (0248): | 146.04 1.592 7.25 25.87
|-----|
| ID = 3 (8364): | 905.65 4.741 8.25 21.08
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
-----
| ROUTE CHN (6250) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
```

<----- DATA FOR SECTION (2501.0) ----->

Distance	Elevation	Manning
0.00	269.96	0.0500
8.56	268.55	0.0500
17.13	266.91	0.0500
21.41	266.13	0.0500
40.68	263.15	0.0500
62.09	260.75	0.0500
85.64	258.02	0.0500
88.20	257.69	0.0500
93.20	257.05	0.0500 /0.0350 Main Channel
93.45	256.88	0.0350 Main Channel
94.20	256.56	0.0350 Main Channel
94.95	256.83	0.0350 Main Channel
95.20	257.08	0.0350 /0.0500 Main Channel
100.62	257.45	0.0500
115.61	258.57	0.0500
139.16	260.43	0.0500
152.01	261.95	0.0500
171.27	264.63	0.0500
188.40	267.90	0.0500
211.95	274.18	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.49	257.05	.205E+04	0.4	0.68	89.65
1.17	257.73	.248E+05	5.7	0.85	72.16
1.85	258.41	.841E+05	25.2	1.10	55.60
2.53	259.09	.180E+06	66.5	1.36	45.14
3.21	259.77	.312E+06	135.7	1.60	38.36
3.89	260.45	.481E+06	238.3	1.82	33.61
4.57	261.13	.693E+06	383.5	2.07	29.60
5.25	261.81	.911E+06	568.9	2.29	26.70
5.93	262.49	.117E+07	800.5	2.51	24.36
6.61	263.17	.146E+07	1079.0	2.72	22.49
7.29	263.84	.177E+07	1413.4	2.94	20.85
7.96	264.52	.210E+07	1797.0	3.14	19.50
8.64	265.20	.246E+07	2241.6	3.35	18.28
9.32	265.88	.283E+07	2739.9	3.55	17.25
10.00	266.56	.323E+07	3295.7	3.75	16.34
10.68	267.24	.364E+07	3908.9	3.94	15.54
11.36	267.92	.408E+07	4578.5	4.12	14.84
12.04	268.60	.453E+07	5320.1	4.32	14.18
12.72	269.28	.499E+07	6109.1	4.50	13.61

<---- hydrograph ---->				<-pipe / channel->			
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (8364)	905.65	4.74	8.25	21.08	1.04	0.81	
OUTFLOW : ID= 1 (6250)	905.65	4.43	10.25	21.08	1.00	0.80	

ADD HYD (8366)						
1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0250):			192.88	1.797	8.00	30.85
+ ID2= 2 (6250):			905.65	4.429	10.25	21.08
=====						
ID = 3 (8366):			1098.53	6.064	9.75	22.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8368)						
1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8366):			1098.53	6.064	9.75	22.79
+ ID2= 2 (8370):			548.79	3.538	9.00	22.26
=====						
ID = 3 (8368):			1647.32	9.574	9.50	22.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8372)						
1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0252):			318.99	3.678	8.00	33.49
+ ID2= 2 (8368):			1647.32	9.574	9.50	22.62
=====						
ID = 3 (8372):			1967.31	12.931	9.00	24.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6258)	
IN= 2-->	OUT= 1
Routing time step (min)= 15.00	

<----- DATA FOR SECTION (2581.0) ----->				
Distance	Elevation	Manning		
0.00	252.88	0.0500		
15.47	251.89	0.0500		
46.41	248.45	0.0500		
73.48	245.81	0.0500		
88.95	243.75	0.0500		
112.15	242.00	0.0500		
135.35	240.23	0.0500		
162.42	239.76	0.0500		
170.97	239.52	0.0500 / 0.0350	Main Channel	
171.58	239.03	0.0350	Main Channel	
174.02	239.03	0.0350	Main Channel	
176.46	239.03	0.0350	Main Channel	
177.07	239.52	0.0350 / 0.0500	Main Channel	
185.63	239.67	0.0500		
208.83	239.87	0.0500		
239.77	240.14	0.0500		
274.57	244.83	0.0500		
336.45	249.51	0.0500		
363.52	249.77	0.0500		
362.85	251.78	0.0500		

<----- TRAVEL TIME TABLE ----->						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.49	257.05	.135E+05	2.5	0.92	91.32	
1.14	240.17	.175E+06	25.6	0.73	114.22	
1.78	240.81	.532E+06	123.5	1.17	71.85	
2.43	241.46	.932E+06	285.3	1.54	54.48	
3.07	242.10	.138E+07	506.5	1.85	45.25	
3.72	242.75	.186E+07	786.8	2.12	39.42	
4.36	243.39	.239E+07	1127.3	2.37	35.33	
5.01	244.04	.296E+07	1535.8	2.61	32.12	
5.65	244.68	.358E+07	2014.3	2.84	29.48	
6.30	245.33	.420E+07	2535.9	3.03	27.60	
6.94	245.97	.488E+07	3108.4	3.20	26.16	
7.59	246.62	.561E+07	3740.6	3.35	24.98	
8.23	247.26	.638E+07	4446.7	3.50	23.93	
8.88	247.91	.721E+07	5229.7	3.64	22.98	
9.52	248.55	.809E+07	6094.3	3.78	22.12	
10.17	249.20	.901E+07	7050.4	3.93	21.31	
10.81	249.84	.100E+08	7773.0	3.90	21.46	
11.46	250.49	.111E+08	8998.8	4.07	20.56	
12.10	251.13	.122E+08	10318.2	4.24	19.75	

<---- hydrograph ---->				<-pipe / channel->			
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (8372)	1967.31	12.93	9.00	24.38	0.78	0.82	
OUTFLOW : ID= 1 (6258)	1967.31	11.62	11.25	24.38	0.75	0.83	

ADD HYD (8374)						
1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0258):			381.99	2.269	8.00	39.73
+ ID2= 2 (6258):			1967.31	11.623	11.25	24.38
=====						
ID = 3 (8374):			2149.30	13.409	11.00	25.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8376)						
1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0260):			476.24	5.896	8.00	43.34
+ ID2= 2 (8374):			2149.30	13.409	11.00	25.68
=====						
ID = 3 (8376):			2625.54	18.584	10.25	28.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8378)						
1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0262):			341.31	5.213	7.75	43.20
+ ID2= 2 (8376):			2625.54	18.584	10.25	28.89
=====						
ID = 3 (8378):			2966.85	22.865	10.00	30.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8362)						
1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8378):			2966.85	22.865	10.00	30.53
+ ID2= 2 (8384):			1321.42	12.139	8.75	31.54
=====						
ID = 3 (8362):			4288.27	34.648	9.50	30.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6272)	
IN= 2-->	OUT= 1
Routing time step (min)= 15.00	

<----- DATA FOR SECTION (2721.0) ----->			
Distance	Elevation	Manning	
0.00	231.01	0.0450	
23.01	223.65	0.0450	
34.51	222.46	0.0450	
51.77	222.11	0.0450	
69.02	221.87	0.0450	

161.06	221.92	0.0450	
166.81	221.31	0.0450	
172.56	221.89	0.0450	
180.57	221.40	0.0450 / 0.0300	Main Channel
181.57	220.60	0.0300	Main Channel
184.07	220.16	0.0300	Main Channel
195.57	221.85	0.0300 / 0.0450	Main Channel
201.32	221.82	0.0450	
207.07	221.77	0.0450	
212.83	221.72	0.0450	
253.09	222.52	0.0450	
408.40	222.65	0.0450	
460.17	223.20	0.0450	
517.69	224.84	0.0450	
569.46	232.57	0.0450	

375.38	225.71	0.0500
403.79	229.37	0.0500
432.29	230.43	0.0500
470.29	232.00	0.0500

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-----
<<----- TRAVEL TIME TABLE ----->>
DEPTH   ELEV   VOLUME   FLOW RATE   VELOCITY   TRAV.TIME
(m)     (m)     (cu.m.)   (cms)       (m/s)      (min)
-----
0.41    220.57   .189E+04   0.5         0.49       60.18
0.83    220.99   .697E+04   3.2         0.82       35.79
1.24    221.40   .145E+05   8.8         1.07       27.45
1.84    222.00   .643E+05   29.3        0.81       36.61
2.44    222.60   .282E+06   132.9       0.83       35.41
3.04    223.20   .707E+06   413.7       1.04       28.50
3.64    223.80   .118E+07   876.8       1.31       22.46
4.24    224.40   .168E+07   1494.8      1.57       18.75
4.84    225.00   .221E+07   2264.1      1.82       16.23
5.44    225.60   .274E+07   3196.3      2.06       14.29
6.04    226.20   .328E+07   4258.6      2.30       12.84
6.65    226.81   .383E+07   5444.7      2.52       11.72
7.25    227.41   .438E+07   6749.7      2.73       10.82
7.85    228.01   .494E+07   8169.4      2.93       10.08
8.45    228.61   .551E+07   9700.7      3.12       9.46
9.05    229.21   .608E+07   11340.8     3.30       8.94
9.65    229.81   .666E+07   13087.4     3.48       8.48
10.25   230.41   .724E+07   14938.5     3.65       8.08
10.85   231.01   .783E+07   16892.4     3.82       7.73
-----

```

```

-----
<<----- hydrograph ----->> <-pipe / channel->
AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
(ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
-----
INFLOW : ID= 2 (8362) 4288.27 34.18 10.25 30.84 1.87 0.81
OUTFLOW: ID= 1 (6272) 4288.27 34.18 10.25 30.84 1.87 0.81
-----

```

```

-----
| ADD HYD (8356) |
| 1 + 2 = 3 |
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
-----
ID1= 1 (0272): 157.38 2.475 7.50 35.49
+ ID2= 2 (6272): 4288.27 34.180 10.25 30.84
=====
ID = 3 (8356): 4445.65 35.965 10.00 31.01
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
-----

```

```

-----
| ADD HYD (8352) |
| 1 + 2 = 3 |
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
-----
ID1= 1 (8354): 1268.73 21.461 7.75 40.61
+ ID2= 2 (8356): 4445.65 35.965 10.00 31.01
=====
ID = 3 (8352): 5714.38 54.080 9.00 33.14
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
-----

```

```

-----
| ROUTE CHN (6304) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)= 15.00
-----

```

```

-----
<<----- DATA FOR SECTION (3041.0) ----->>
Distance   Elevation   Manning
-----
0.00       232.08      0.0500
19.00      231.87      0.0500
38.00      231.33      0.0500
66.51      230.44      0.0500
104.51     228.25      0.0500
118.76     225.17      0.0500
128.26     219.86      0.0500
175.77     219.17      0.0500
185.27     218.90      0.0500 / 0.0300 Main Channel
185.52     218.65      0.0300 Main Channel
190.02     218.37      0.0300 Main Channel
194.52     218.60      0.0300 Main Channel
194.77     218.85      0.0300 / 0.0500 Main Channel
204.27     219.60      0.0500
299.28     220.91      0.0500
327.78     222.36      0.0500
-----

```

```

-----
<<----- TRAVEL TIME TABLE ----->>
DEPTH   ELEV   VOLUME   FLOW RATE   VELOCITY   TRAV.TIME
(m)     (m)     (cu.m.)   (cms)       (m/s)      (min)
-----
0.48    218.85   .121E+05   1.2         0.36       173.94
1.17    219.54   .839E+05   9.8         0.44       142.60
1.86    220.23   .323E+06   41.0        0.47       131.43
2.56    220.93   .708E+06   109.5       0.58       107.67
3.25    221.62   .118E+07   224.3       0.71       87.36
3.94    222.31   .168E+07   376.5       0.84       74.48
4.63    223.00   .222E+07   567.9       0.96       65.26
5.32    223.69   .279E+07   794.7       1.07       58.58
6.02    224.39   .339E+07   1054.2      1.17       53.52
6.71    225.08   .402E+07   1352.4      1.26       49.52
7.40    225.77   .468E+07   1677.3      1.34       46.46
8.09    226.46   .538E+07   2050.0      1.43       43.58
8.79    227.16   .607E+07   2457.9      1.52       41.14
9.48    227.85   .680E+07   2901.0      1.60       39.05
10.17   228.54   .755E+07   3352.7      1.66       37.53
10.86   229.23   .834E+07   3813.5      1.71       36.48
11.55   229.92   .919E+07   4241.8      1.73       36.13
12.25   230.62   .101E+08  4697.0      1.74       35.93
12.94   231.31   .111E+08  5186.3      1.74       35.83
-----

```

```

-----
<<----- hydrograph ----->> <-pipe / channel->
AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
(ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
-----
INFLOW : ID= 2 (8352) 5714.38 54.08 9.00 33.14 2.00 0.49
OUTFLOW: ID= 1 (6304) 5714.38 44.75 11.50 33.14 1.90 0.48
-----

```

```

-----
| ADD HYD (8350) |
| 1 + 2 = 3 |
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
-----
ID1= 1 (0304): 292.37 1.571 10.25 25.48
+ ID2= 2 (6304): 5714.38 54.08 9.00 33.14
=====
ID = 3 (8350): 6006.75 46.284 11.50 32.77
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
-----

```

```

-----
| ADD HYD (8346) |
| 1 + 2 = 3 |
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
-----
ID1= 1 (8348): 3768.46 34.216 9.50 30.36
+ ID2= 2 (8350): 6006.75 46.284 11.50 32.77
=====
ID = 3 (8346): 9775.21 78.525 10.50 31.84
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
-----

```

```

-----
| ADD HYD (8344) |
| 1 + 2 = 3 |
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
-----
ID1= 1 (0306): 283.97 0.538 13.75 18.51
+ ID2= 2 (8346): 9775.21 78.525 10.50 31.84
=====
ID = 3 (8344): 10059.18 79.003 10.50 31.46
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
-----

```

```

-----
| RESERVOIR (5509) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
OUTFLOW   STORAGE   |   OUTFLOW   STORAGE
(cms)     (ha.m.)   |   (cms)     (ha.m.)
-----
0.0000    0.0000   |   0.0000    370.0451
41.0590   56.7403   |   0.0000    863.4386
48.1390   86.3439   |   0.0000    0.0000
56.6340    0.0000   |   0.0000    0.0000
67.9600    0.0000   |   0.0000    0.0000
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
-----
INFLOW : ID= 2 (8344) 0.0000 79.003 10.50 31.46
OUTFLOW: ID= 1 (5509) 0.0000 50.565 15.50 31.46
-----

```

```

-----
PEAK FLOW REDUCTION [Qout/Qin](%)= 64.00
TIME SHIFT OF PEAK FLOW (min)=300.00
MAXIMUM STORAGE USED (ha.m.)= 96.9336
-----

```



```

-----
| CALIB |
| NASHYD (0332) | Area (ha)= 393.44 Curve Number (CN)= 75.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.32
-----

```

```

Unit Hyd Qpeak (cms)= 2.894

PEAK FLOW (cms)= 3.428 (1)
TIME TO PEAK (hrs)= 9.500
RUNOFF VOLUME (mm)= 35.702
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.442

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0330) | Area (ha)= 468.30 Curve Number (CN)= 80.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.09
-----

```

```

Unit Hyd Qpeak (cms)= 7.335

PEAK FLOW (cms)= 8.597 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 40.876
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.506

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ROUTE CHN (6332) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
-----

```

----- DATA FOR SECTION (3321.0) -----			
Distance	Elevation	Manning	
0.00	234.00	0.0380	
25.85	227.20	0.0380	
96.94	226.44	0.0380	
168.03	227.38	0.0380	
219.73	225.62	0.0380	
342.52	221.57	0.0380	
368.37	221.42	0.0380	
374.83	221.23	0.0380	
379.79	220.98	0.0380 / 0.0300	Main Channel
380.29	220.47	0.0300	Main Channel
381.29	220.47	0.0300	Main Channel
382.79	220.47	0.0300	Main Channel
383.29	220.98	0.0300 / 0.0380	Main Channel
394.22	221.22	0.0380	
400.68	221.33	0.0380	
407.14	221.44	0.0380	
491.16	225.70	0.0380	
568.71	227.55	0.0380	
607.49	230.14	0.0380	
639.80	234.08	0.0380	

----- TRAVEL TIME TABLE -----					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.51	220.98	.554E+04	0.9	0.56	107.27
1.20	221.67	.885E+05	12.1	0.49	122.32
1.88	222.35	.309E+06	64.7	0.76	79.70
2.57	223.04	.615E+06	165.4	0.97	61.99
3.25	223.72	.101E+07	322.1	1.16	52.06
3.94	224.41	.148E+07	542.9	1.33	45.51
4.62	225.09	.204E+07	835.4	1.48	40.77
5.31	225.78	.269E+07	1202.8	1.62	37.27
5.99	226.46	.344E+07	1618.0	1.70	35.46
6.68	227.15	.447E+07	1932.0	1.56	38.56
7.36	227.83	.580E+07	2763.9	1.72	34.96
8.05	228.52	.718E+07	3861.9	1.95	30.97
8.73	229.20	.859E+07	5115.4	2.16	27.98
9.42	229.89	.100E+08	6519.2	2.35	25.64
10.10	230.57	.115E+08	8092.3	2.55	23.69
10.79	231.26	.130E+08	9821.5	2.73	22.05
11.47	231.94	.145E+08	11690.0	2.92	20.68
12.16	232.63	.160E+08	13694.9	3.09	19.52
12.84	233.31	.176E+08	15833.9	3.26	18.52

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (0330) 468.30 8.60 7.25 40.88 0.98 0.51
OUTFLOW: ID= 1 (6332) 468.30 6.50 9.50 40.88 0.85 0.53
-----

```

```

-----
| ADD HYD (8322) |
| 1 + 2 = 3 |
-----
| Area (ha) QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID= 1 (0332): 393.44 3.428 9.50 35.70 |
| + ID= 2 (6332): 468.30 6.501 9.50 40.88 |
| ===== |
| ID = 3 (8322): 861.74 9.928 9.50 38.51 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0328) | Area (ha)= 492.92 Curve Number (CN)= 77.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.91
-----

```

```

Unit Hyd Qpeak (cms)= 4.411

PEAK FLOW (cms)= 5.314 (1)
TIME TO PEAK (hrs)= 8.750
RUNOFF VOLUME (mm)= 37.735
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.467

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0326) | Area (ha)= 678.91 Curve Number (CN)= 80.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.95
-----

```

```

Unit Hyd Qpeak (cms)= 5.941

PEAK FLOW (cms)= 7.858 (1)
TIME TO PEAK (hrs)= 8.750
RUNOFF VOLUME (mm)= 41.091
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.509

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0324) | Area (ha)= 615.64 Curve Number (CN)= 79.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.90
-----

```

```

Unit Hyd Qpeak (cms)= 5.544

PEAK FLOW (cms)= 7.077 (1)
TIME TO PEAK (hrs)= 8.500
RUNOFF VOLUME (mm)= 39.932
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.494

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0320) | Area (ha)= 278.74 Curve Number (CN)= 81.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.36
-----

```

```

Unit Hyd Qpeak (cms)= 3.499

PEAK FLOW (cms)= 4.443 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 42.167
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.522

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (3182) | Area (ha)= 457.40 Curve Number (CN)= 78.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.49
-----

```

```

Unit Hyd Qpeak (cms)= 5.259

PEAK FLOW (cms)= 6.226 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 38.748
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.480

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANHYD (3181) | Area (ha)= 26.00
| ID= 1 DT=15.0 min | Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00
-----
| IMPERVIOUS | PERVIOUS (I) |
| Surface Area (ha)= 21.06 | 4.94 |
| Dep. Storage (mm)= 2.00 | 5.00 |
| Average Slope (%)= 0.50 | 0.50 |
| Length (m)= 416.33 | 40.00 |
| Mannings n = 0.013 | 0.250 |
|
| Max.Eff.Inten.(mm/hr)= 106.66 | 58.84 |
| over (min) 15.00 | 30.00 |
| Storage Coeff. (min)= 7.21 (ii) | 20.44 (iii) |
| Unit Hyd. Tpeak (min)= 15.00 | 30.00 |
| Unit Hyd. peak (cms)= 0.10 | 0.05 |
|
| PEAK FLOW (cms)= 5.72 | 0.41 | *TOTALS*
| TIME TO PEAK (hrs)= 6.00 | 6.25 | 5.977 (iii)
| RUNOFF VOLUME (mm)= 78.80 | 37.88 | 71.03
| TOTAL RAINFALL (mm)= 80.80 | 80.80 | 80.80
| RUNOFF COEFFICIENT = 0.98 | 0.47 | 0.68

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 77.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| RESERVOIR (5318) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
| OUTFLOW | STORAGE | OUTFLOW | STORAGE |
| (cms) | (ha.m.) | (cms) | (ha.m.) | |
|---|---|---|---|---|
| 0.0000 | 0.0000 | 3.3800 | 0.7654 |
| 0.0510 | 0.4423 | 4.1500 | 0.8231 |
| 0.8700 | 0.5378 | 4.8750 | 0.8778 |
| 1.9590 | 0.5105 | 5.2750 | 0.8878 |
| 2.5620 | 0.6796 | 0.0000 | 0.0000 |
|-----|-----|-----|-----|
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
| INFLOW : ID= 2 (3181) | 26.000 | 5.977 | 6.00 | 71.03 |
| OUTFLOW : ID= 1 (5318) | 26.000 | 3.426 | 6.25 | 70.94 |
|-----|-----|-----|-----|
| PEAK FLOW REDUCTION [Qout/Qin](%)= 57.33 |
| TIME SHIFT OF PEAK FLOW (min)= 15.00 |
| MAXIMUM STORAGE USED (ha.m.)= 0.8392 |

```

```

-----
| ADD HYD (8334) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) |
| ID= 1 (3182): | 457.40 | 6.226 | 8.00 | 38.75 |
| + ID= 2 (5318): | 26.00 | 3.426 | 6.25 | 70.94 |
|-----|-----|-----|-----|
| ID = 3 (8334): | 483.40 | 7.520 | 6.25 | 40.48 |
|-----|-----|-----|-----|
| NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY. |

```

```

-----
| ROUTE CHN (6320) |
| IN= 2--> OUT= 1 |
-----
| ROUTING time step (min)= 15.00 |
-----
| <----- DATA FOR SECTION (3201.0) -----> |
| Distance | Elevation | Manning | |
|---|---|---|---|
| 0.00 | 249.00 | 0.0380 |
| 22.92 | 245.86 | 0.0380 |
| 45.83 | 244.87 | 0.0380 |
| 91.66 | 243.11 | 0.0380 |
| 126.03 | 239.53 | 0.0380 |
| 160.41 | 237.17 | 0.0380 |
| 166.14 | 237.06 | 0.0380 |
| 177.59 | 237.13 | 0.0380 |
| 183.32 | 237.20 | 0.0380 |
| 189.05 | 236.70 | 0.0380 / 0.0350 | Main Channel |
| 193.05 | 235.89 | 0.0350 | Main Channel |
| 197.05 | 236.64 | 0.0350 / 0.0380 | Main Channel |
| 200.51 | 236.74 | 0.0380 |
| 206.24 | 237.03 | 0.0380 |
| 246.34 | 238.82 | 0.0380 |
| 263.53 | 243.87 | 0.0380 |
| 389.56 | 247.64 | 0.0380 |
| 452.58 | 247.74 | 0.0380 |
| 498.41 | 248.60 | 0.0380 |
| 567.16 | 249.84 | 0.0380 |

```

```

-----
| <----- TRAVEL TIME TABLE -----> |
| DEPTH | ELEV | VOLUME | FLOW RATE | VELOCITY | TRAV.TIME |
| (m) | (m) | (cu.m.) | (cms) | (m/s) | (min) |
|-----|-----|-----|-----|-----|-----|
| 0.37 | 236.26 | .270E+04 | 0.4 | 0.58 | 107.96 |
| 0.75 | 236.64 | .108E+05 | 2.6 | 0.92 | 68.01 |
| 1.48 | 237.37 | .877E+05 | 27.4 | 1.17 | 53.35 |
| 2.20 | 238.09 | .277E+06 | 122.1 | 1.65 | 37.83 |
| 2.93 | 238.82 | .540E+06 | 299.7 | 2.08 | 30.01 |
| 3.66 | 239.55 | .807E+06 | 587.8 | 2.57 | 24.29 |
| 4.39 | 240.28 | .120E+07 | 977.8 | 3.04 | 20.52 |
| 5.11 | 241.00 | .158E+07 | 1458.1 | 3.46 | 18.02 |
| 5.84 | 241.73 | .198E+07 | 2028.6 | 3.84 | 16.23 |
| 6.57 | 242.46 | .240E+07 | 2690.5 | 4.19 | 14.86 |
| 7.29 | 243.18 | .285E+07 | 3431.8 | 4.50 | 13.84 |
| 8.02 | 243.91 | .335E+07 | 4149.1 | 4.64 | 13.44 |
| 8.75 | 244.64 | .393E+07 | 4746.6 | 4.51 | 13.80 |
| 9.47 | 245.36 | .463E+07 | 5518.6 | 4.53 | 13.75 |
| 10.20 | 246.09 | .545E+07 | 6781.8 | 4.66 | 13.39 |
| 10.93 | 246.82 | .635E+07 | 8274.5 | 4.87 | 12.79 |
| 11.66 | 247.55 | .733E+07 | 9988.0 | 5.10 | 12.23 |
| 12.38 | 248.27 | .853E+07 | 11422.0 | 5.01 | 12.45 |
| 13.11 | 249.00 | .989E+07 | 13749.7 | 5.20 | 11.98 |

```

```

-----
| <---- hydrograph ----> <-pipe / channel-> |
| AREA | QPEAK | TPEAK | R.V. | MAX DEPTH | MAX VEL | |
| (ha) | (cms) | (hrs) | (mm) | (m) | (m/s) |
| INFLOW : ID= 2 (8334) | 483.40 | 7.52 | 6.25 | 40.48 | 0.89 | 0.96 |
| OUTFLOW : ID= 1 (6320) | 483.40 | 6.12 | 9.00 | 40.48 | 0.85 | 0.95 |

```

```

-----
| ADD HYD (8332) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) |
| ID= 1 (0320): | 278.74 | 4.443 | 8.00 | 42.17 |
| + ID= 2 (6320): | 483.40 | 6.122 | 9.00 | 40.48 |
|-----|-----|-----|-----|
| ID = 3 (8332): | 762.14 | 10.364 | 8.50 | 41.10 |
|-----|-----|-----|-----|
| NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY. |

```

```

-----
| CALIB (0314) |
| NASHYD (0314) | Area (ha)= 165.20 Curve Number (CN)= 78.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.71 |
-----
| Unit Hyd Qpeak (cms)= 3.978 |
| PEAK FLOW (cms)= 4.019 (i) |
| TIME TO PEAK (hrs)= 6.750 |
| RUNOFF VOLUME (mm)= 38.307 |
| TOTAL RAINFALL (mm)= 80.800 |
| RUNOFF COEFFICIENT = 0.474 |
| (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY. |

```

```

-----
| ROUTE CHN (6316) |
| IN= 2--> OUT= 1 |
-----
| Routing time step (min)= 15.00 |
-----
| <----- DATA FOR SECTION (3161.0) -----> |
| Distance | Elevation | Manning | |
|---|---|---|---|
| 0.00 | 248.54 | 0.0350 |
| 27.93 | 248.34 | 0.0350 |
| 41.89 | 246.61 | 0.0350 |
| 62.83 | 243.09 | 0.0350 |
| 132.65 | 239.00 | 0.0350 |
| 188.50 | 236.74 | 0.0350 |
| 195.48 | 236.54 | 0.0350 |
| 202.46 | 236.32 | 0.0350 |
| 205.48 | 236.14 | 0.0350 / 0.0310 | Main Channel |
| 205.98 | 235.61 | 0.0310 | Main Channel |
| 207.98 | 235.25 | 0.0310 | Main Channel |
| 209.98 | 235.53 | 0.0310 | Main Channel |
| 210.48 | 236.00 | 0.0310 / 0.0350 | Main Channel |
| 216.42 | 236.73 | 0.0350 |
| 258.31 | 239.09 | 0.0350 |
| 328.12 | 239.84 | 0.0350 |
| 439.83 | 241.47 | 0.0350 |
| 530.58 | 242.08 | 0.0350 |
| 586.43 | 242.93 | 0.0350 |
| 691.16 | 248.00 | 0.0350 |

```

```

-----
| <----- TRAVEL TIME TABLE -----> |
| DEPTH | ELEV | VOLUME | FLOW RATE | VELOCITY | TRAV.TIME |
| (m) | (m) | (cu.m.) | (cms) | (m/s) | (min) |
|-----|-----|-----|-----|-----|-----|
| 0.37 | 236.62 | .215E+04 | 0.7 | 0.77 | 53.88 |
| 0.75 | 236.00 | .634E+04 | 3.5 | 1.36 | 30.45 |
| 1.46 | 236.71 | .297E+05 | 19.3 | 1.62 | 25.66 |
| 2.16 | 237.41 | .103E+06 | 75.8 | 1.84 | 22.60 |

```

2.87	238.12	.228E+06	202.8	2.21	18.79
3.57	238.82	.407E+06	423.3	2.59	16.03
4.28	239.53	.654E+06	727.7	2.77	14.97
4.99	240.24	.103E+07	1242.0	3.01	13.76
5.59	240.94	.153E+07	2002.3	3.31	12.54
6.40	241.65	.210E+07	2987.9	3.55	11.70
7.10	242.35	.286E+07	4302.8	3.75	11.07
7.81	243.06	.374E+07	6165.3	4.10	10.11
8.51	243.76	.468E+07	8642.4	4.60	9.03
9.22	244.47	.565E+07	11496.3	5.06	8.20
9.93	245.18	.666E+07	14714.1	5.50	7.54
10.63	245.88	.770E+07	18290.3	5.91	7.02
11.34	246.59	.877E+07	22221.9	6.30	6.58
12.04	247.29	.988E+07	26464.4	6.67	6.22
12.75	248.00	.110E+08	31061.3	7.01	5.91

<---- hydrograph ----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (0314)	165.20	4.02	6.75	38.31	0.77	1.37
OUTFLOW: ID= 1 (6316)	165.20	3.84	7.25	38.31	0.77	1.37

```

-----
| CALIB |
| NASHYD (0316) | Area (ha)= 232.34 Curve Number (CN)= 82.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.01 |
|-----|
Unit Hyd Qpeak (cms) = 3.923
PEAK FLOW (cms) = 4.823 (i)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 43.236
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.535
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

```

-----
| ADD HYD (8338) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
| ID= 1 (0316): 232.34 4.823 7.25 43.24
| + ID= 2 (6316): 165.20 3.837 7.25 38.31
|-----|
| ID = 3 (8338): 397.54 8.660 7.25 41.19
|-----|
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

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-----
| CALIB |
| NASHYD (0312) | Area (ha)= 359.44 Curve Number (CN)= 80.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.36 |
|-----|
Unit Hyd Qpeak (cms) = 4.529
PEAK FLOW (cms) = 5.575 (i)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 40.979
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.507
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| CALIB |
| NASHYD (0308) | Area (ha)= 529.30 Curve Number (CN)= 62.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.62 |
|-----|
Unit Hyd Qpeak (cms) = 5.575
PEAK FLOW (cms) = 4.194 (i)
TIME TO PEAK (hrs) = 8.250
RUNOFF VOLUME (mm) = 24.698
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.306
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| CALIB |
| NASHYD (0310) | Area (ha)= 138.28 Curve Number (CN)= 65.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.76 |
|-----|
Unit Hyd Qpeak (cms) = 3.102

```

```

PEAK FLOW (cms) = 2.144 (i)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 26.616
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.329

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8342) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
| ID= 1 (0308): 529.30 4.194 8.25 24.70
| + ID= 2 (0310): 138.28 2.144 7.00 26.62
|-----|
| ID = 3 (8342): 667.58 6.044 7.75 25.10
|-----|

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6312) | Routing time step (min)'= 15.00
| IN= 2--> OUT= 1 |
|-----|

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-----
| DATA FOR SECTION (3121.0) ----->
| Distance Elevation Manning |
| 0.00 265.94 0.0360 |
| 38.07 265.43 0.0360 |
| 59.82 263.98 0.0360 |
| 103.32 254.59 0.0360 |
| 157.70 252.16 0.0360 |
| 217.52 250.45 0.0360 |
| 233.84 247.69 0.0360 |
| 234.71 247.27 0.0360 |
| 239.71 246.38 0.0360 |
| 244.71 246.12 0.0360 / 0.0330 Main Channel |
| 247.71 245.17 0.0330 Main Channel |
| 249.71 245.19 0.0330 Main Channel |
| 251.71 245.64 0.0330 / 0.0360 Main Channel |
| 259.71 246.67 0.0360 |
| 282.78 247.12 0.0360 |
| 315.41 251.60 0.0360 |
| 424.17 256.13 0.0360 |
| 478.55 257.04 0.0360 |
| 516.62 259.37 0.0360 |
| 538.37 266.00 0.0360 |

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-----
| TRAVEL TIME TABLE ----->
| DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME |
| (m) (m) (cu.m.) (cms) (m/s) (min) |
| 0.47 245.64 .111E+04 3.1 1.78 6.07 |
| 1.54 246.71 .103E+05 53.9 3.39 3.19 |
| 2.61 247.78 .414E+05 281.3 4.40 2.45 |
| 3.68 248.85 .839E+05 749.1 5.79 1.87 |
| 4.74 249.91 .136E+06 1463.2 6.97 1.55 |
| 5.81 250.98 .201E+06 2354.8 7.61 1.42 |
| 6.88 252.05 .295E+06 3533.0 7.76 1.39 |
| 7.95 253.12 .427E+06 5303.9 8.05 1.34 |
| 9.02 254.19 .594E+06 7854.5 8.58 1.26 |
| 10.09 255.26 .792E+06 11449.2 9.37 1.15 |
| 11.15 256.32 .101E+07 15862.4 10.15 1.06 |
| 12.22 257.39 .127E+07 21074.5 10.75 1.01 |
| 13.29 258.46 .155E+07 28184.1 11.78 0.92 |
| 14.36 259.53 .185E+07 36386.2 12.77 0.85 |
| 15.43 260.60 .213E+07 46110.6 13.89 0.78 |
| 16.50 261.67 .248E+07 56822.5 14.95 0.72 |
| 17.56 262.73 .278E+07 68498.1 15.97 0.68 |
| 18.63 263.80 .310E+07 81118.8 16.94 0.64 |
| 19.70 264.87 .344E+07 93603.2 17.66 0.61 |

```

```

-----
| <---- hydrograph ----> <-pipe / channel->
| AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
| (ha) (cms) (hrs) (mm) (m) (m/s)
| INFLOW : ID= 2 (8342) 667.58 6.04 7.75 25.10 0.53 1.83
| OUTFLOW: ID= 1 (6312) 667.58 6.04 8.00 25.10 0.53 1.83

```

```

-----
| ADD HYD (8340) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
| ID= 1 (0312): 359.44 5.575 8.00 40.98
| + ID= 2 (6312): 667.58 6.040 8.00 25.10
|-----|
| ID = 3 (8340): 1027.02 11.615 8.00 30.65
|-----|

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8336)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8336):	397.54	8.660	7.25	41.19
+ ID2= 2 (8340):	1027.02	11.615	8.00	30.65
=====				
ID = 3 (8336):	1424.56	20.099	7.50	33.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8330)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8332):	762.14	10.364	8.50	41.10
+ ID2= 2 (8336):	1424.56	20.099	7.50	33.59
=====				
ID = 3 (8330):	2186.70	29.999	7.75	36.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6324) |
| IIS= 2 -> OPT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3241.0) ----->

Distance	Elevation	Manning	
0.00	234.38	0.0360	
33.57	232.48	0.0360	
67.14	230.14	0.0360	
83.93	228.80	0.0360	
134.29	227.62	0.0360	
209.82	225.10	0.0360	
218.21	224.86	0.0360	
226.61	224.47	0.0360	
234.00	223.86	0.0360 / 0.0300	Main Channel
234.10	223.66	0.0300	Main Channel
235.00	223.66	0.0300	Main Channel
235.90	223.66	0.0300	Main Channel
236.00	223.86	0.0300 / 0.0360	Main Channel
243.39	224.92	0.0360	
251.78	224.89	0.0360	
335.71	225.64	0.0360	
562.32	226.53	0.0360	
637.85	228.36	0.0360	
705.00	229.80	0.0360	
830.89	234.00	0.0360	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.20	223.86	.145E+04	0.2	0.43	147.27
0.73	224.39	.159E+05	2.8	0.68	93.11
1.27	224.93	.561E+05	11.0	0.75	85.19
1.80	225.46	.217E+06	45.6	0.80	79.20
2.33	225.99	.566E+06	134.4	0.91	70.20
2.87	226.53	.122E+07	336.1	1.06	59.97
3.40	227.06	.208E+07	744.3	1.38	46.16
3.94	227.60	.298E+07	1295.9	1.66	38.38
4.47	228.13	.399E+07	1979.9	1.90	33.59
5.00	228.66	.509E+07	2807.5	2.11	30.21
5.54	229.20	.627E+07	3815.1	2.32	27.41
6.07	229.73	.753E+07	4984.1	2.53	25.17
6.60	230.26	.884E+07	6346.3	2.74	23.20
7.14	230.80	.102E+08	7861.2	2.95	21.61
7.67	231.33	.116E+08	9526.4	3.14	20.29
8.21	231.87	.131E+08	11342.2	3.32	19.18
8.74	232.40	.146E+08	13309.6	3.49	18.22
9.27	232.93	.161E+08	15414.0	3.66	17.41
9.81	233.47	.177E+08	17671.0	3.81	16.70

<----- hydrograph -----> <-pipe / channel->

INFLOW : ID= 2 (8330)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
ID= 1 (6324)	2186.70	30.00	7.75	36.21	1.56	0.78
OUTFLOW : ID= 1 (6324)	2186.70	25.63	9.50	36.21	1.49	0.77

ADD HYD (8328)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0324):	615.64	7.077	8.50	39.93
+ ID2= 2 (6324):	2186.70	25.633	9.50	36.21
=====				
ID = 3 (8328):	2802.34	32.579	9.50	37.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB			
NASHVD (0322)	Area (ha)	Curve Number (CN)	# of Linear Res.(N)
ID= 1 DT=15.0 min	513.13	5.00	1.50
U.H. Tp(hrs)= 1.75			

Unit Hyd Qpeak (cms)= 5.020

PEAK FLOW (cms)= 6.502 (1)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 41.063
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.508

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8326)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0322):	513.13	6.502	8.25	41.06
+ ID2= 2 (8328):	2802.34	32.579	9.50	37.03
=====				
ID = 3 (8326):	3315.47	38.885	9.25	37.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8324)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0326):	678.91	7.858	8.75	41.09
+ ID2= 2 (8326):	3315.47	38.885	9.25	37.65
=====				
ID = 3 (8324):	3994.38	46.684	9.25	38.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6328) |
| IIS= 2 -> OPT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3281.0) ----->

Distance	Elevation	Manning	
0.00	228.00	0.0380	
18.58	224.97	0.0380	
78.98	223.52	0.0380	
125.44	223.28	0.0380	
171.90	221.71	0.0380	
213.72	219.65	0.0380	
218.36	219.40	0.0380	
223.01	219.19	0.0380	
225.95	219.14	0.0380 / 0.0320	Main Channel
226.45	218.14	0.0320	Main Channel
236.95	218.14	0.0320	Main Channel
245.85	218.14	0.0320	Main Channel
245.95	219.14	0.0320 / 0.0380	Main Channel
246.24	219.16	0.0380	
250.88	219.24	0.0380	
255.53	219.39	0.0380	
325.22	221.47	0.0380	
367.03	223.14	0.0380	
404.20	225.17	0.0380	
459.95	228.04	0.0380	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.50	218.64	.373E+05	3.2	0.33	192.37
1.00	219.14	.752E+05	10.1	0.51	124.71
1.52	219.66	.149E+06	22.2	0.57	112.06
2.04	220.18	.279E+06	43.4	0.59	107.06
2.56	220.70	.464E+06	77.0	0.63	100.52
3.08	221.22	.705E+06	125.7	0.68	93.56
3.61	221.75	.100E+07	192.7	0.73	86.62
4.13	222.27	.135E+07	278.1	0.79	80.92
4.65	222.79	.176E+07	385.9	0.84	75.86
5.17	223.31	.222E+07	514.9	0.89	71.82
5.69	223.83	.280E+07	662.7	0.90	70.40
6.21	224.35	.345E+07	867.7	0.96	66.35
6.73	224.87	.417E+07	1107.0	1.01	62.81
7.25	225.39	.494E+07	1400.0	1.08	58.80
7.78	225.92	.573E+07	1732.2	1.15	55.17
8.30	226.44	.655E+07	2099.9	1.22	52.02
8.82	226.96	.740E+07	2502.8	1.29	49.29
9.34	227.48	.828E+07	2940.9	1.36	46.90
9.86	228.00	.918E+07	3414.3	1.42	44.79

<----- hydrograph -----> <-pipe / channel->

	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8324)	3994.38	46.68	9.25	38.24	2.09	0.60
OUTFLOW: ID= 1 (6328)	3994.38	39.91	11.00	38.24	1.96	0.59

ADD HYD (8320)						
1 + 2 = 3						
	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (8328):	492.92	5.314	8.75	37.73		
+ ID2= 2 (6328):	3994.38	39.913	11.00	38.24		
ID = 3 (8320):	4487.30	44.685	10.75	38.18		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8318)						
1 + 2 = 3						
	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (8320):	4487.30	44.685	10.75	38.18		
+ ID2= 2 (8322):	861.74	9.928	9.50	38.51		
ID = 3 (8318):	5349.04	54.210	10.75	38.23		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8316)						
1 + 2 = 3						
	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (8509):	10059.30	50.223	6.50	41.92		
+ ID2= 2 (8318):	5349.04	54.210	10.75	38.23		
ID = 3 (8316):	15408.22	97.899	11.75	33.81		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8312)						
1 + 2 = 3						
	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (8314):	620.10	1.884	6.50	21.94		
+ ID2= 2 (8316):	15408.22	97.899	11.75	33.81		
ID = 3 (8312):	16028.32	98.974	11.75	33.35		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8308)						
1 + 2 = 3						
	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (8310):	598.90	33.223	6.50	41.92		
+ ID2= 2 (8312):	16028.32	98.974	11.75	33.35		
ID = 3 (8308):	16627.22	101.128	11.50	33.66		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5510)						
IN= 2--> OUT= 1						
DT= 15.0 min						
	OUTFLOW	STORAGE	OUTFLOW	STORAGE		
	(cms)	(ha.m.)	(cms)	(ha.m.)		
	0.0000	0.0000	*****	74.0090		
	66.5450	18.5023	*****	*****		
	98.5430	37.0045	*****	*****		

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8308)	*****	101.128	11.50	33.66
OUTFLOW: ID= 1 (5510)	*****	96.575	13.25	33.66

PEAK FLOW REDUCTION [Qout/Qin] (%) = 95.50
 TIME SHIFT OF PEAK FLOW (min) = 105.00
 MAXIMUM STORAGE USED (ha.m.) = 35.8687

ADD HYD (8240)						
1 + 2 = 3						
	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0336):	2785.00	3.992	22.25	32.86
+ ID2= 2 (5510):	16627.22	96.575	13.25	33.66
ID = 3 (8240):	19412.22	100.014	13.25	33.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8238)						
1 + 2 = 3						
	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (5508):	9524.23	50.303	12.00	21.33		
+ ID2= 2 (8240):	19412.22	100.014	13.25	33.55		
ID = 3 (8238):	28936.45	149.326	12.75	29.53		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8236)						
1 + 2 = 3						
	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (0342):	1004.58	4.420	10.00	19.62		
+ ID2= 2 (8238):	28936.45	149.326	12.75	29.53		
ID = 3 (8236):	29941.03	153.277	12.75	29.20		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8234)						
1 + 2 = 3						
	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (8232):	285.80	9.337	6.50	28.35		
+ ID2= 2 (8236):	29941.03	153.277	12.75	29.20		
ID = 3 (8234):	30226.83	153.639	12.75	29.19		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8230)						
1 + 2 = 3						
	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (0344):	344.00	4.034	7.00	22.48		
+ ID2= 2 (8234):	30226.83	153.639	12.75	29.19		
ID = 3 (8230):	30570.83	154.850	12.50	29.11		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8228)						
1 + 2 = 3						
	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (8226):	1952.06	13.442	8.75	18.81		
+ ID2= 2 (8230):	30570.83	154.850	12.50	29.11		
ID = 3 (8228):	32522.89	163.469	12.25	28.49		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8190)						
1 + 2 = 3						
	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (0356):	1015.22	5.609	8.00	15.27		
+ ID2= 2 (8228):	32522.89	163.469	12.25	28.49		
ID = 3 (8190):	33538.11	166.987	12.25	28.09		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ** SIMULATION NUMBER: 12 **

READ STORM	Filename:
	C:\Users\jascott\AppData\Local\Temp\

3e280798-92ee-4282-809c-79ef5caed0add\73alf747
 Ptotal= 73.10 mm
 Comments: 25-Year 12-Hour SCS II Design Storm

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.83	3.25	2.92	6.25	13.16	9.25	2.56
0.50	1.83	3.50	2.92	6.50	13.16	9.50	2.56
0.75	1.83	3.75	2.92	6.75	5.85	9.75	2.56
1.00	1.83	4.00	2.92	7.00	5.85	10.00	2.56
1.25	1.83	4.25	4.39	7.25	4.39	10.25	1.46
1.50	1.83	4.50	4.39	7.50	4.39	10.50	1.46
1.75	1.83	4.75	5.85	7.75	4.39	10.75	1.46
2.00	1.83	5.00	5.85	8.00	4.39	11.00	1.46
2.25	2.19	5.25	8.77	8.25	2.56	11.25	1.46
2.50	2.19	5.50	8.77	8.50	2.56	11.50	1.46
2.75	2.19	5.75	35.09	8.75	2.56	11.75	1.46
3.00	2.19	6.00	36.49	9.00	2.56	12.00	1.46

CALIB (0356) Area (ha)=1015.22 Curve Number (CN)= 46.0
 NASHYD (0356) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.37

Unit Hyd Qpeak (cms)= 12.651
 PEAK FLOW (cms)= 4.610 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 12.581
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.172

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0354) Area (ha)= 262.68 Curve Number (CN)= 37.0
 NASHYD (0354) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.38

Unit Hyd Qpeak (cms)= 3.252
 PEAK FLOW (cms)= 0.864 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 9.206
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.126

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0352) Area (ha)= 381.43 Curve Number (CN)= 54.0
 NASHYD (0352) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.82

Unit Hyd Qpeak (cms)= 7.980
 PEAK FLOW (cms)= 3.316 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 16.078
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.220

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0346) Area (ha)= 350.93 Curve Number (CN)= 70.0
 NASHYD (0346) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 6.254
 PEAK FLOW (cms)= 4.425 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 25.923
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.355

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0350) Area (ha)= 366.84 Curve Number (CN)= 48.0
 NASHYD (0350) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 5.831

PEAK FLOW (cms)= 2.124 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 13.386
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.183

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0348) Area (ha)= 590.18 Curve Number (CN)= 48.0
 NASHYD (0348) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.91

Unit Hyd Qpeak (cms)= 5.267

PEAK FLOW (cms)= 2.211 (i)
 TIME TO PEAK (hrs)= 9.000
 RUNOFF VOLUME (mm)= 13.458
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.184

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6350) Routing time step (min)= 15.00
 IN= 2--> OUT= 1

Distance	Elevation	Manning
0.00	287.00	0.0500
23.66	283.72	0.0500
70.98	280.94	0.0500
112.38	280.23	0.0500
171.52	276.80	0.0500
260.24	274.46	0.0500
266.16	274.26	0.0500
272.07	274.12	0.0500
276.49	274.12	0.0500 / 0.0300
276.99	273.82	0.0300
277.99	273.82	0.0300
279.49	273.82	0.0300
280.49	274.27	0.0300 / 0.0500
283.90	274.27	0.0500
289.81	274.57	0.0500
325.30	275.75	0.0500
396.28	278.98	0.0500
449.51	280.97	0.0500
496.83	283.90	0.0500
585.54	287.92	0.0500

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.30	274.12	273E+04	0.9	0.99	49.81
0.98	274.80	583E+05	20.8	1.05	46.71
1.66	275.48	203E+06	95.2	1.38	35.53
2.33	276.15	438E+06	256.0	1.73	28.50
3.01	276.83	753E+06	524.8	2.05	23.97
3.69	277.51	114E+07	935.8	2.43	20.26
4.37	278.19	157E+07	1472.0	2.76	17.82
5.05	278.87	205E+07	2140.7	3.06	16.06
5.72	279.54	261E+07	2938.6	3.33	14.79
6.40	280.22	321E+07	3887.6	3.57	13.77
7.08	280.90	390E+07	4795.4	3.63	13.56
7.76	281.58	468E+07	6196.2	3.91	12.58
8.43	282.25	550E+07	7803.9	4.19	11.74
9.11	282.93	637E+07	9601.9	4.45	11.05
9.79	283.61	728E+07	11594.8	4.70	10.46
10.47	284.29	823E+07	13845.3	4.96	9.91
11.15	284.97	922E+07	16293.4	5.21	9.43
11.82	285.64	103E+08	18952.0	5.45	9.02
12.50	286.32	113E+08	21824.3	5.69	8.65

hydrograph <--> <-pipe / channel-->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0348) 590.18 2.21 9.00 13.46 0.34 0.99
 OUTFLOW : ID= 1 (6350) 590.18 2.16 10.25 13.46 0.34 0.99

ADD HYD (8220)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID= 1 (0350): 366.84 2.124 7.50 13.39
 + ID= 2 (6350): 590.18 2.158 10.25 13.46
 ID= 3 (8220): 957.02 3.943 8.75 13.43

```

-----
| ADD HYD (8190) |
| 1 + 2 3 |
|-----|
| AREA OPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0356): 1015.22 2.534 8.00 6.98 |
| + ID2= 2 (8228): 32522.89 87.695 12.50 14.27 |
|-----|
| ID = 3 (8190): 33538.11 89.249 12.50 14.05 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

*****
** SIMULATION NUMBER: 6 **
*****

```

```

-----
| READ STORM |
|-----|
| Filename: C:\Users\jscott\AppData |
| Local\Temp\ |
| 3e280798-92ee-4282-809c-79f5caed0add\2687e965 |
| Comments: 100-Year 12-Hour SCS II Design Storm |
-----

```

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	2.21	3.25	3.54	6.25	15.93	9.25	3.10
0.50	2.21	3.50	3.54	6.50	15.93	9.50	3.10
0.75	2.21	3.75	3.54	6.75	7.08	9.75	3.10
1.00	2.21	4.00	3.54	7.00	7.08	10.00	3.10
1.25	2.21	4.25	5.31	7.25	5.31	10.25	1.77
1.50	2.21	4.50	5.31	7.50	5.31	10.50	1.77
1.75	2.21	4.75	7.08	7.75	5.31	10.75	1.77
2.00	2.21	5.00	7.08	8.00	5.31	11.00	1.77
2.25	2.65	5.25	10.62	8.25	3.10	11.25	1.77
2.50	2.65	5.50	10.62	8.50	3.10	11.50	1.77
2.75	2.65	5.75	42.48	8.75	3.10	11.75	1.77
3.00	2.65	6.00	116.82	9.00	3.10	12.00	1.77

```

-----
| CALIB |
| NASHYD (0356) | Area (ha)=1015.22 Curve Number (CN)= 46.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 1.37 |
-----

```

```

Unit Hyd Qpeak (cms)= 12.651

PEAK FLOW (cms)= 6.685 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 18.151
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.205

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0354) | Area (ha)= 262.68 Curve Number (CN)= 37.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 1.38 |
-----

```

```

Unit Hyd Qpeak (cms)= 3.252

PEAK FLOW (cms)= 1.265 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 13.427
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.152

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0352) | Area (ha)= 381.43 Curve Number (CN)= 54.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 0.82 |
-----

```

```

Unit Hyd Qpeak (cms)= 7.980

PEAK FLOW (cms)= 4.770 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 22.930
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.259

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
|-----|

```

```

-----
| NASHYD (0346) | Area (ha)= 350.93 Curve Number (CN)= 70.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 0.96 |
-----

```

```
Unit Hyd Qpeak (cms)= 6.254
```

```

PEAK FLOW (cms)= 6.173 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 35.852
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.405

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0350) | Area (ha)= 366.84 Curve Number (CN)= 48.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 1.07 |
-----

```

```
Unit Hyd Qpeak (cms)= 5.831
```

```

PEAK FLOW (cms)= 3.076 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 19.261
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.218

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0348) | Area (ha)= 590.18 Curve Number (CN)= 48.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 1.91 |
-----

```

```
Unit Hyd Qpeak (cms)= 5.267
```

```

PEAK FLOW (cms)= 3.191 (i)
TIME TO PEAK (hrs)= 9.000
RUNOFF VOLUME (mm)= 19.364
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.219

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ROUTE CHN (6350) |
| IN= 2--> OUT= 1 |
|-----|

```

<----- DATA FOR SECTION (3501.0) ----->

Distance	Elevation	Manning
0.00	287.00	0.0500
23.66	283.72	0.0500
70.98	280.94	0.0500
112.38	280.23	0.0500
171.52	276.80	0.0500
260.24	274.46	0.0500
266.16	274.26	0.0500
272.07	274.12	0.0500
276.49	274.12	0.0500 / 0.0300
276.99	273.82	0.0300
277.99	273.82	0.0300
279.49	273.82	0.0300
280.49	274.27	0.0300 / 0.0500
283.90	274.27	0.0500
289.81	274.57	0.0500
325.30	275.75	0.0500
396.28	278.98	0.0500
449.51	280.97	0.0500
496.83	283.90	0.0500
585.54	287.92	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.30	274.12	273E+04	0.9	0.99	49.81
0.98	274.80	583E+05	20.8	1.05	46.71
1.66	275.48	203E+06	95.2	1.38	35.53
2.33	276.15	438E+06	256.0	1.73	28.50
3.01	276.83	755E+06	524.8	2.05	23.97
3.69	277.51	114E+07	935.8	2.43	20.26
4.37	278.19	157E+07	1472.0	2.76	17.82
5.05	278.87	206E+07	2140.7	3.06	16.06
5.72	279.54	261E+07	2938.6	3.33	14.79
6.40	280.22	321E+07	3887.6	3.57	13.77
7.08	280.90	390E+07	4795.4	3.63	13.56
7.76	281.58	468E+07	6196.2	3.91	12.58
8.43	282.25	550E+07	7801.9	4.19	11.74
9.11	282.93	637E+07	9601.9	4.45	11.05
9.79	283.61	728E+07	11594.8	4.70	10.46
10.47	284.29	823E+07	13845.3	4.96	9.91

11.15 284.97 .922E+07 16293.4 5.21 9.43
 11.82 285.64 .103E+08 18952.0 5.45 9.02
 12.50 286.32 .113E+08 21824.3 5.69 8.65

```

<----- hydrograph -----> <-pipe / channel->
AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (0348) 590.18 3.19 9.00 19.36 0.38 0.99
OUTFLOW: ID= 1 (6350) 590.18 3.11 10.25 19.36 0.37 0.99
  
```

```

| ADD HYD (8220) |
| 1 + 2 = 3 |
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0350): 366.84 3.076 7.50 19.26
+ ID2= 2 (6350): 590.18 3.111 10.25 19.36
=====
ID = 3 (8220): 957.02 5.697 8.75 19.32
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ADD HYD (8222) |
| 1 + 2 = 3 |
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0346): 350.93 6.173 7.25 35.85
+ ID2= 2 (8220): 957.02 5.697 8.75 19.32
=====
ID = 3 (8222): 1307.95 11.432 8.00 23.76
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ROUTE CHN (6352) |
| IN= 2--> OUT= 1 |
Routing time step (min)'= 15.00
  
```

```

<----- DATA FOR SECTION (3521.0) ----->
Distance Elevation Manning
0.00 257.95 0.0500
7.63 257.34 0.0500
15.66 256.19 0.0500
21.53 254.01 0.0500
41.11 244.06 0.0500
76.35 241.38 0.0500
111.58 239.74 0.0500
113.54 239.61 0.0500
113.75 239.41 0.0500 /0.0300 Main Channel
113.85 238.81 0.0300 Main Channel
115.50 238.81 0.0300 Main Channel
116.15 238.81 0.0300 Main Channel
116.25 239.41 0.0300 Main Channel
117.46 239.52 0.0500
119.41 239.72 0.0500
121.37 240.04 0.0500
131.16 241.84 0.0500
156.61 247.03 0.0500
176.19 251.46 0.0500
193.80 258.79 0.0500
  
```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.60 239.41 2.24E+04 2.2 1.52 17.12
1.58 240.39 2.16E+05 22.3 1.61 16.16
2.55 241.36 8.04E+05 101.0 1.96 13.27
3.53 242.34 1.73E+06 275.6 2.48 10.46
4.50 243.31 2.93E+06 553.9 2.95 8.90
5.48 244.29 4.38E+06 961.8 3.42 7.60
6.45 245.26 5.99E+06 1534.4 3.99 6.51
7.43 246.24 7.70E+06 2228.4 4.51 5.76
8.41 247.22 9.51E+06 3043.6 4.98 5.21
9.38 248.19 1.14E+07 3982.1 5.43 4.78
10.36 249.17 1.34E+07 5040.1 5.85 4.44
11.33 250.14 1.55E+07 6210.5 6.24 4.16
12.31 251.12 1.77E+07 7518.6 6.61 3.93
13.29 252.10 2.00E+07 8969.0 6.98 3.72
14.26 253.07 2.24E+07 10554.1 7.35 3.53
15.24 254.05 2.48E+07 12257.1 7.71 3.37
16.21 255.02 2.72E+07 14045.1 8.03 3.23
17.19 256.00 2.98E+07 15954.8 8.34 3.11
18.16 256.97 3.25E+07 17780.4 8.53 3.04
  
```

```

<----- hydrograph -----> <-pipe / channel->
AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8222) 1307.95 11.43 8.00 23.76 1.05 1.56
OUTFLOW: ID= 1 (6352) 1307.95 11.35 8.25 23.76 1.05 1.56
  
```

```

| ADD HYD (8224) |
| 1 + 2 = 3 |
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0352): 381.43 4.770 7.00 22.93
+ ID2= 2 (6352): 1307.95 11.354 8.25 23.76
=====
ID = 3 (8224): 1689.38 15.685 7.75 23.57
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ROUTE CHN (6354) |
| IN= 2--> OUT= 1 |
Routing time step (min)'= 15.00
  
```

```

<----- DATA FOR SECTION (3541.0) ----->
Distance Elevation Manning
0.00 253.92 0.0500
7.95 251.83 0.0500
15.89 249.97 0.0500
19.87 249.05 0.0500
47.68 242.00 0.0500
67.55 237.10 0.0500
83.44 231.11 0.0500
85.43 230.75 0.0500
85.66 230.61 0.0500 /0.0300 Main Channel
85.76 230.01 0.0300 Main Channel
87.41 230.01 0.0300 Main Channel
88.06 230.01 0.0300 Main Channel
88.16 230.61 0.0300 /0.0500 Main Channel
89.40 230.72 0.0500
91.39 230.88 0.0500
93.37 231.03 0.0500
133.11 233.96 0.0500
150.99 235.39 0.0500
172.84 239.03 0.0500
196.68 250.00 0.0500
  
```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.60 230.61 4.51E+04 1.5 1.06 49.40
1.62 231.63 3.94E+05 14.6 1.16 45.13
2.64 232.65 1.28E+06 58.8 1.44 36.30
3.66 233.67 2.69E+06 149.7 1.74 30.00
4.68 234.69 4.63E+06 301.5 2.04 25.58
5.70 235.71 7.05E+06 530.5 2.36 22.14
6.72 236.73 9.80E+06 848.4 2.71 19.25
7.74 237.75 1.28E+07 1237.9 3.02 17.29
8.76 238.77 1.62E+07 1706.7 3.30 15.83
9.78 239.79 1.99E+07 2291.9 3.61 14.45
10.81 240.82 2.37E+07 2973.7 3.92 13.30
11.83 241.84 2.78E+07 3742.2 4.21 12.38
12.85 242.86 3.21E+07 4599.1 4.49 11.62
13.87 243.88 3.65E+07 5544.6 4.75 10.98
14.89 244.90 4.12E+07 6579.7 5.00 10.44
15.91 245.92 4.61E+07 7705.9 5.23 9.97
16.93 246.94 5.12E+07 8924.6 5.46 9.55
17.95 247.96 5.64E+07 10237.5 5.68 9.19
18.97 248.98 6.19E+07 11646.2 5.89 8.86
  
```

```

<----- hydrograph -----> <-pipe / channel->
AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8224) 1689.38 15.68 7.75 23.57 1.65 1.16
OUTFLOW: ID= 1 (6354) 1689.38 14.70 8.75 23.57 1.62 1.16
  
```

```

| ADD HYD (8226) |
| 1 + 2 = 3 |
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0354): 262.68 1.265 8.00 13.43
+ ID2= 2 (6354): 1689.38 14.695 8.75 23.57
=====
ID = 3 (8226): 1952.06 15.927 8.75 22.21
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| CALIB |
| NASHYD (0344) |
| ID= 1 DT=15.0 min |
Area (ha)= 344.00 Curve Number (CN)= 59.0
Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.87
  
```

```

Unit Hyd Qpeak (cms)= 6.790
PEAK FLOW (cms)= 4.771 (i)
TIME TO PEAK (hrs)= 7.000
  
```


RUNOFF VOLUME (mm)= 26.476
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.299

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANHYD (3402) | Area (ha)= 234.60
| ID= 1 DT=15.0 min | Total Imp(%)= 28.00 Dir. Conn.(%)= 13.00
-----
| IMPERVIOUS | PERVIOUS (i)
| Surface Area (ha)= 65.69 | 168.91
| Dep. Storage (mm)= 2.00 | 5.00
| Average Slope (%)= 0.50 | 0.50
| Length (m)= 1250.60 | 40.00
| Mannings n = 0.013 | 0.250
-----
| Max.Eff.Inten.(mm/hr)= 116.82 | 25.11
| over (min) | 15.00 | 45.00
| Storage Coeff. (min)= 13.46 (ii) | 32.05 (ii)
| Unit Hyd. Tpeak (min)= 15.00 | 45.00
| Unit Hyd. peak (cms)= 0.08 | 0.03
-----
| *TOTALS*
| PEAK FLOW (cms)= 7.54 | 7.07 | 10.471 (iii)
| TIME TO PEAK (hrs)= 6.00 | 6.50 | 6.00
| RUNOFF VOLUME (mm)= 86.50 | 22.80 | 31.08
| TOTAL RAINFALL (mm)= 88.50 | 88.50 | 88.50
| RUNOFF COEFFICIENT = 0.98 | 0.26 | 0.35
  
```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANHYD (3401) | Area (ha)= 51.20
| ID= 1 DT=15.0 min | Total Imp(%)= 46.00 Dir. Conn.(%)= 21.00
-----
| IMPERVIOUS | PERVIOUS (i)
| Surface Area (ha)= 23.55 | 27.65
| Dep. Storage (mm)= 2.00 | 5.00
| Average Slope (%)= 0.50 | 0.50
| Length (m)= 584.24 | 40.00
| Mannings n = 0.013 | 0.250
-----
| Max.Eff.Inten.(mm/hr)= 116.82 | 58.14
| over (min) | 15.00 | 30.00
| Storage Coeff. (min)= 8.52 (ii) | 21.81 (ii)
| Unit Hyd. Tpeak (min)= 15.00 | 30.00
| Unit Hyd. peak (cms)= 0.09 | 0.05
-----
| *TOTALS*
| PEAK FLOW (cms)= 3.08 | 2.17 | 4.437 (iii)
| TIME TO PEAK (hrs)= 6.00 | 6.25 | 6.00
| RUNOFF VOLUME (mm)= 86.50 | 26.50 | 39.10
| TOTAL RAINFALL (mm)= 88.50 | 88.50 | 88.50
| RUNOFF COEFFICIENT = 0.98 | 0.30 | 0.44
  
```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| RESERVOIR (5340) |
| IN= 2 -> OUT= 1 |
| DT= 15.0 min |
-----
| OUTFLOW | STORAGE | OUTFLOW | STORAGE
| (cms) | (ha.m.) | (cms) | (ha.m.)
| 0.0000 | 0.0000 | 5.3360 | 1.7877
| 0.1730 | 0.6348 | 7.2780 | 2.2185
| 1.8360 | 0.8250 | 9.1880 | 2.6486
| 2.1540 | 1.3051 | 9.5880 | 2.6586
| 3.3950 | 1.3506 | 0.0000 | 0.0000
-----
| AREA | QPEAK | TPEAK | R.V.
| (ha) | (cms) | (hrs) | (mm)
INFLOW : ID= 2 (3401) | 51.200 | 4.437 | 6.00 | 39.10
OUTFLOW: ID= 1 (5340) | 51.200 | 1.889 | 6.50 | 39.08
-----
| PEAK FLOW REDUCTION [Qout/Qin](%)= 42.58
| TIME SHIFT OF PEAK FLOW (min)= 30.00
| MAXIMUM STORAGE USED (ha.m.)= 0.8780
  
```

```

-----
| ADD HYD (8232) |
| 1 + 2 + 3 |
-----
| AREA | QPEAK | TPEAK | R.V.
| (ha) | (cms) | (hrs) | (mm)
ID= 1 (3402): | 234.60 | 10.471 | 6.00 | 31.08
+ ID= 2 (5340): | 51.20 | 1.889 | 6.50 | 39.08
-----
| CALIB |
| ID= 3 (8232): | 285.80 | 10.972 | 6.50 | 32.52
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0342) | Area (ha)=1004.58 Curve Number (CN)= 54.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 2.57
-----
| Unit Hyd Qpeak (cms)= 6.686
| PEAK FLOW (cms)= 5.230 (i)
| TIME TO PEAK (hrs)= 10.000
| RUNOFF VOLUME (mm)= 23.193
| TOTAL RAINFALL (mm)= 88.500
| RUNOFF COEFFICIENT = 0.262
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0422) | Area (ha)= 780.20 Curve Number (CN)= 54.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.95
-----
| Unit Hyd Qpeak (cms)= 6.838
| PEAK FLOW (cms)= 4.992 (i)
| TIME TO PEAK (hrs)= 9.000
| RUNOFF VOLUME (mm)= 23.163
| TOTAL RAINFALL (mm)= 88.500
| RUNOFF COEFFICIENT = 0.262
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0410) | Area (ha)= 572.01 Curve Number (CN)= 48.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.46
-----
| Unit Hyd Qpeak (cms)= 6.698
| PEAK FLOW (cms)= 3.821 (i)
| TIME TO PEAK (hrs)= 8.000
| RUNOFF VOLUME (mm)= 19.326
| TOTAL RAINFALL (mm)= 88.500
| RUNOFF COEFFICIENT = 0.218
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0408) | Area (ha)= 231.62 Curve Number (CN)= 58.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.64
-----
| Unit Hyd Qpeak (cms)= 6.198
| PEAK FLOW (cms)= 3.942 (i)
| TIME TO PEAK (hrs)= 6.750
| RUNOFF VOLUME (mm)= 25.554
| TOTAL RAINFALL (mm)= 88.500
| RUNOFF COEFFICIENT = 0.289
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0376) | Area (ha)= 463.85 Curve Number (CN)= 74.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.07
-----
| Unit Hyd Qpeak (cms)= 7.380
| PEAK FLOW (cms)= 8.365 (i)
| TIME TO PEAK (hrs)= 7.250
| RUNOFF VOLUME (mm)= 39.991
| TOTAL RAINFALL (mm)= 88.500
| RUNOFF COEFFICIENT = 0.452
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0374) Area (ha)= 545.70 Curve Number (CN)= 61.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.51

Unit Hyd Qpeak (cms)= 6.158

PEAK FLOW (cms)= 5.223 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 28.198
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.319

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0372) Area (ha)= 110.42 Curve Number (CN)= 37.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 1.954

PEAK FLOW (cms)= 0.691 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 13.367
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.151

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0370) Area (ha)= 191.85 Curve Number (CN)= 63.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.67

Unit Hyd Qpeak (cms)= 4.860

PEAK FLOW (cms)= 3.631 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 29.417
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.332

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0368) Area (ha)= 159.48 Curve Number (CN)= 46.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.12

Unit Hyd Qpeak (cms)= 2.433

PEAK FLOW (cms)= 1.216 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 18.110
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.205

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0366) Area (ha)= 462.62 Curve Number (CN)= 62.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.06

Unit Hyd Qpeak (cms)= 7.451

PEAK FLOW (cms)= 5.968 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 28.878
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.326

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0364) Area (ha)= 155.27 Curve Number (CN)= 55.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.70

Unit Hyd Qpeak (cms)= 3.782

PEAK FLOW (cms)= 2.245 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 23.520
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.266

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8302)
1 + 2 = 3 AREA QPEAK TPEAK R.V.
ID= 1 (0364): 155.27 2.245 6.75 23.52
+ ID2= 2 (0366): 462.62 5.968 7.50 28.88
=====

ID = 3 (8302): 617.89 8.124 7.25 27.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (0362) Area (ha)= 118.78
ID= 1 DT=15.0 min Total Imp(%)= 22.00 Dir. Conn.(%)= 8.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 26.13 92.65
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 889.87 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.82 21.80
over (min)= 15.00 45.00
Storage Coeff. (min)= 10.97 (ii) 30.65 (ii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.08 0.03

PEAK FLOW (cms)= 2.53 3.45 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.50 4.005 (iii)
RUNOFF VOLUME (mm)= 86.50 20.43 25.72
TOTAL RAINFALL (mm)= 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.23 0.29

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 45.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0358) Area (ha)= 429.87 Curve Number (CN)= 35.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.03

Unit Hyd Qpeak (cms)= 7.091

PEAK FLOW (cms)= 2.367 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 12.436
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.141

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0360) Area (ha)= 138.37 Curve Number (CN)= 46.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.60

Unit Hyd Qpeak (cms)= 3.957

PEAK FLOW (cms)= 1.703 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 17.868
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.202

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8306)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0358): 429.87 2.367 7.50 12.44
+ ID2= 2 (0360): 138.37 1.703 6.75 17.87
=====
ID = 3 (8306): 568.24 3.976 7.00 13.76

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (8362)
IN= 2--> OUT= 1
Routing time step (min)'= 15.00

```

```

----- DATA FOR SECTION (3621.0) -----
Distance Elevation Manning
0.00 261.46 0.0550
27.86 254.23 0.0550
51.07 251.96 0.0550
74.29 250.77 0.0550
97.50 249.91 0.0550
125.36 249.40 0.0550
150.93 247.40 0.0550
155.93 247.33 0.0550 /0.0350 Main Channel
157.93 246.85 0.0350 Main Channel
159.18 246.65 0.0350 Main Channel
160.18 246.63 0.0350 Main Channel
160.93 246.85 0.0350 Main Channel
161.93 247.18 0.0350 /0.0550 Main Channel
163.18 248.03 0.0550
168.18 248.58 0.0550
183.18 250.18 0.0550
201.97 252.59 0.0550
213.57 256.02 0.0550
225.18 280.31 0.0550
229.82 261.00 0.0550

```

```

----- TRAVEL TIME TABLE -----
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.55 247.18 .550E+04 1.9 1.03 49.95
1.28 247.91 .336E+05 16.7 1.53 33.61
2.00 248.63 .913E+05 53.6 1.81 28.40
2.73 249.36 .185E+06 122.3 2.05 25.18
3.46 250.09 .344E+06 224.6 2.02 25.52
4.19 250.82 .574E+06 417.9 2.25 22.91
4.91 251.54 .855E+06 699.1 2.53 20.38
5.64 252.27 .118E+07 1075.3 2.82 18.26
6.37 253.00 .153E+07 1559.9 3.15 16.38
7.10 253.73 .191E+07 2137.6 3.46 14.89
7.82 254.45 .231E+07 2815.2 3.77 13.67
8.55 255.18 .272E+07 3608.5 4.10 12.58
9.28 255.91 .315E+07 4488.2 4.41 11.69
10.01 256.64 .358E+07 5458.0 4.71 10.95
10.73 257.36 .403E+07 6512.4 4.99 10.32
11.46 258.09 .449E+07 7649.8 5.27 9.78
12.19 258.82 .496E+07 8869.5 5.53 9.32
12.92 259.55 .544E+07 10171.2 5.78 8.91
13.64 260.27 .593E+07 11554.8 6.02 8.55

```

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8306) 568.24 3.98 7.00 13.76 0.65 1.08
OUTFLOW: ID= 1 (8362) 568.24 3.52 8.25 13.76 0.63 1.07

```

```

ADD HYD (8304)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0362): 118.78 4.005 6.50 25.72
+ ID2= 2 (8362): 568.24 3.522 8.25 13.76
=====
ID = 3 (8304): 687.02 5.831 6.50 15.83

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8300)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8302): 617.89 8.124 7.25 27.53
+ ID2= 2 (8304): 687.02 5.831 6.50 15.83
=====
ID = 3 (8300): 1304.91 13.263 6.50 21.37

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (8368)
IN= 2--> OUT= 1
Routing time step (min)'= 15.00

```

```

----- DATA FOR SECTION (3681.0) -----
Distance Elevation Manning
0.00 230.00 0.0370
18.48 223.26 0.0370
36.96 223.05 0.0370
64.67 222.94 0.0370
110.87 222.86 0.0370
133.96 222.74 0.0370
147.82 222.65 0.0370
170.92 222.31 0.0370
174.79 222.26 0.0370 /0.0300 Main Channel
174.89 221.86 0.0300 Main Channel
175.84 221.86 0.0300 Main Channel
176.19 221.86 0.0300 Main Channel
176.29 222.26 0.0300 /0.0370 Main Channel
180.16 222.25 0.0370
184.78 222.28 0.0370
189.40 222.31 0.0370
332.60 222.37 0.0370
450.00 230.00 0.0370

```

```

----- TRAVEL TIME TABLE -----
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.40 222.26 .771E+03 0.3 0.52 41.74
0.81 222.67 .784E+05 30.3 0.51 43.17
1.21 223.07 .210E+06 123.2 0.77 28.42
1.62 223.48 .384E+06 301.0 1.03 21.25
2.03 223.89 .563E+06 550.2 1.28 17.06
2.44 224.30 .747E+06 860.7 1.51 14.46
2.84 224.70 .934E+06 1228.1 1.73 12.68
3.25 225.11 .113E+07 1649.3 1.92 11.37
3.66 225.52 .132E+07 2122.3 2.11 10.37
4.07 225.93 .152E+07 2645.5 2.29 9.58
4.47 226.33 .172E+07 3217.8 2.45 8.92
4.88 226.74 .193E+07 3838.3 2.61 8.38
5.29 227.15 .214E+07 4506.3 2.76 7.92
5.70 227.56 .236E+07 5221.3 2.91 7.52
6.10 227.96 .258E+07 5983.0 3.05 7.17
6.51 228.37 .280E+07 6790.9 3.19 6.87
6.92 228.78 .303E+07 7645.0 3.32 6.60
7.33 229.19 .326E+07 8545.1 3.45 6.35
7.73 229.59 .349E+07 9491.0 3.57 6.13

```

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8300) 1304.91 13.26 6.50 21.37 0.58 0.52
OUTFLOW: ID= 1 (8368) 1304.91 12.01 8.00 21.37 0.56 0.52

```

```

ADD HYD (8298)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0368): 159.48 1.216 7.75 18.11
+ ID2= 2 (8368): 1304.91 12.014 8.00 21.37
=====
ID = 3 (8298): 1464.39 13.223 8.00 21.01

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8296)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0370): 191.85 3.631 6.75 29.42
+ ID2= 2 (8298): 1464.39 13.223 8.00 21.01
=====
ID = 3 (8296): 1656.24 16.201 7.75 21.99

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (8372)
IN= 2--> OUT= 1
Routing time step (min)'= 15.00

```

```

----- DATA FOR SECTION (3721.0) -----
Distance Elevation Manning
0.00 225.00 0.0390
30.80 219.38 0.0390
61.61 219.30 0.0390
77.01 219.27 0.0390
469.76 219.14 0.0390
477.46 219.13 0.0390
485.16 219.10 0.0390

```

492.86 219.09 0.0390
 495.56 219.09 0.0390 / 0.0310 Main Channel
 495.66 218.51 0.0310 Main Channel
 500.56 218.51 0.0310 Main Channel
 505.46 218.51 0.0310 Main Channel
 505.56 219.09 0.0310 / 0.0390 Main Channel
 508.26 219.09 0.0390
 515.96 219.10 0.0390
 523.67 219.21 0.0390
 562.17 219.32 0.0390
 654.58 219.43 0.0390
 731.59 219.46 0.0390
 762.39 225.00 0.0390

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.29	218.80	.431E+04	1.7	0.58	43.48
0.58	219.09	.867E+04	5.1	0.89	28.32
0.93	219.44	.200E+06	58.2	0.44	57.35
1.28	219.79	.568E+06	278.6	0.74	33.99
1.62	220.13	.939E+06	630.2	1.01	24.84
1.97	220.48	.131E+07	1090.4	1.25	20.05
2.32	220.83	.169E+07	1647.7	1.47	17.06
2.67	221.18	.206E+07	2294.6	1.68	14.99
3.01	221.52	.244E+07	3025.6	1.87	13.46
3.36	221.87	.282E+07	3836.5	2.05	12.27
3.71	222.22	.321E+07	4723.7	2.22	11.31
4.06	222.57	.359E+07	5684.6	2.39	10.53
4.40	222.91	.398E+07	6716.6	2.55	9.87
4.75	223.26	.437E+07	7817.7	2.70	9.31
5.10	223.61	.476E+07	8986.2	2.85	8.83
5.45	223.96	.515E+07	10220.4	2.99	8.40
5.79	224.30	.555E+07	11519.1	3.13	8.03
6.14	224.65	.594E+07	12880.8	3.27	7.69
6.49	225.00	.634E+07	14304.5	3.40	7.39

<---- hydrograph ----> <-pipe / channel-->
 AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8296) 1656.24 16.20 7.75 21.99 0.65 0.73
 OUTFLOW : ID= 1 (6372) 1656.24 15.45 8.50 21.99 0.65 0.74

----- ADD HYD (8294) -----
 1 + 2 = 3 | AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0372): 110.42 0.691 7.25 13.37
 + ID2= 2 (6372): 1656.24 15.450 8.50 21.99
 =====
 ID = 3 (8294): 1766.66 16.075 8.50 21.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ADD HYD (8292) -----
 1 + 2 = 3 | AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0374): 545.70 5.223 8.25 28.20
 + ID2= 2 (8294): 1766.66 16.075 8.50 21.45
 =====
 ID = 3 (8292): 2312.36 21.266 8.50 23.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- RESERVOIR (5505) -----
 IN= 2 ---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	65.1290	345.3754
25.4850	24.6697	84.9510	456.3890
31.1490	98.6787	*****	838.7689
39.6440	*****	*****	838.7789
48.1390	*****	0.0000	0.0000

AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (8292) 2312.360 21.266 8.50 23.04
 OUTFLOW : ID= 1 (5505) 2312.360 15.623 11.25 23.04

PEAK FLOW REDUCTION [Qout/Qin](%)= 73.47
 TIME SHIFT OF PEAK FLOW (min)=165.00
 MAXIMUM STORAGE USED (ha.m.)= 15.1370

----- ADD HYD (8272) -----

----- 1 + 2 = 3 -----
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0376): 463.85 8.365 7.25 39.99
 + ID2= 2 (5505): 2312.36 15.623 11.25 23.04
 =====
 ID = 3 (8272): 2776.21 20.788 10.25 25.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- CALIB -----
 NASHVD (0396) | Area (ha)= 305.21 Curve Number (CN)= 69.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.08

Unit Hyd Opeak (cms)= 4.811

PEAK FLOW (cms)= 4.736 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 34.962
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.395

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB -----
 NASHVD (0394) | Area (ha)= 325.45 Curve Number (CN)= 53.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.92

Unit Hyd Opeak (cms)= 6.013

PEAK FLOW (cms)= 3.577 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 22.323
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.252

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB -----
 NASHVD (0390) | Area (ha)= 420.00 Curve Number (CN)= 55.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.07

Unit Hyd Opeak (cms)= 6.683

PEAK FLOW (cms)= 4.371 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 23.713
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.268

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB -----
 NASHVD (0388) | Area (ha)= 220.77 Curve Number (CN)= 58.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.99

Unit Hyd Opeak (cms)= 3.819

PEAK FLOW (cms)= 2.679 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 25.800
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.292

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB -----
 NASHVD (0386) | Area (ha)= 241.27 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.90

Unit Hyd Opeak (cms)= 4.562

PEAK FLOW (cms)= 3.426 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 28.018
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.317

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8286) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
ID1= 1 (0386): 241.27 3.426 7.00 28.02
+ ID2= 2 (0388): 220.77 2.679 7.25 25.80
=====
ID = 3 (8286): 462.04 6.100 7.25 26.96
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0384) | Area (ha)= 199.07 Curve Number (CN)= 44.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.96
-----

```

```

Unit Hyd Qpeak (cms)= 3.537
PEAK FLOW (cms)= 1.596 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 16.955
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.192

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0380) | Area (ha)= 182.01 Curve Number (CN)= 40.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.55
-----

```

```

Unit Hyd Qpeak (cms)= 5.609
PEAK FLOW (cms)= 1.939 (i)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 14.644
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.165

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0382) | Area (ha)= 216.59 Curve Number (CN)= 53.0
| ID= 1 DT=15.0 min | Ia (mm)= 6.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.64
-----

```

```

Unit Hyd Qpeak (cms)= 5.733
PEAK FLOW (cms)= 3.143 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 22.142
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.250

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8290) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
ID1= 1 (0380): 182.01 1.939 6.50 14.64
+ ID2= 2 (0382): 216.59 3.143 6.75 22.14
=====
ID = 3 (8290): 398.60 5.066 6.75 18.72
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6384) |
| IN= 2--> OUF= 1 | Routing time step (min)= 15.00
-----

```

```

<----- DATA FOR SECTION (3841.0) ----->
Distance Elevation Manning
0.00 294.40 0.0380
10.59 291.93 0.0380
21.17 289.19 0.0380
26.46 287.99 0.0380
31.76 286.79 0.0380
71.45 279.97 0.0380
74.10 279.79 0.0380
76.74 279.71 0.0380
78.99 279.64 0.0380 / 0.0300 Main Channel
79.14 279.30 0.0300 Main Channel
79.39 279.30 0.0300 Main Channel
79.64 279.30 0.0300 Main Channel

```

```

79.79 279.64 0.0300 / 0.0380 Main Channel
89.98 279.78 0.0380
119.09 281.86 0.0380
145.55 282.87 0.0380
198.48 284.85 0.0380
211.71 286.31 0.0380
230.23 287.59 0.0380
261.99 294.00 0.0380

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.34 279.64 .715E+03 0.2 0.85 63.42
1.10 280.40 .507E+05 21.9 1.40 38.54
1.85 281.15 .141E+06 91.8 2.10 25.69
2.61 281.91 .269E+06 218.9 2.63 20.46
3.36 282.66 .448E+06 407.1 2.96 18.23
4.12 283.42 .681E+06 698.3 3.31 16.26
4.87 284.17 .977E+06 1111.8 3.68 14.65
5.63 284.93 .133E+07 1674.3 4.06 13.27
6.39 285.69 .173E+07 2457.8 4.59 11.73
7.14 286.44 .215E+07 3378.8 5.07 10.63
7.90 287.20 .261E+07 4418.4 5.47 9.85
8.65 287.95 .310E+07 5676.3 5.92 9.11
9.41 288.71 .361E+07 7150.5 6.40 8.42
10.17 289.47 .414E+07 8783.1 6.86 7.86
10.92 290.22 .469E+07 10575.1 7.30 7.38
11.68 290.98 .525E+07 12520.4 7.72 6.98
12.43 291.73 .582E+07 14619.0 8.12 6.64
13.19 292.49 .642E+07 16863.2 8.50 6.34
13.94 293.24 .703E+07 19259.8 8.86 6.08

```

```

<---- hydrograph ----> <-pipe / channel-->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8290) 398.60 5.07 6.75 18.72 0.51 0.93
OUTFLOW : ID= 1 (6384) 398.60 3.99 7.75 18.72 0.47 0.91

```

```

-----
| ADD HYD (8288) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
ID1= 1 (0384): 199.07 1.596 7.25 16.95
+ ID2= 2 (6384): 398.60 3.985 7.75 18.92
=====
ID = 3 (8288): 597.67 5.554 7.75 18.13
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8284) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
ID1= 1 (8286): 462.04 6.100 7.25 26.96
+ ID2= 2 (8288): 597.67 5.554 7.75 18.13
=====
ID = 3 (8284): 1059.71 11.532 7.50 21.98
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8280) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
ID1= 1 (0390): 420.00 4.371 7.50 23.71
+ ID2= 2 (8284): 1059.71 11.532 7.50 21.98
=====
ID = 3 (8280): 1479.71 15.903 7.50 22.47
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0392) | Area (ha)= 167.22 Curve Number (CN)= 62.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.74
-----

```

```

Unit Hyd Qpeak (cms)= 3.837
PEAK FLOW (cms)= 2.840 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 28.690
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.324

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0378) | Area (ha)= 606.72 Curve Number (CN)= 55.0
| ID= 1 DT=15.0 min | Is (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.18

```

```

Unit Hyd Qpeak (cms)= 8.771

PEAK FLOW (cms)= 5.877 (1)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 23.743
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.268

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8282) |
| 1 + 2 = 3 |
|-----|
| ID1= 1 (0378): 606.72 5.877 7.75 23.74
+ ID2= 2 (0392): 167.22 2.840 7.00 28.69
|-----|
| ID = 3 (8282): 773.94 8.554 7.25 24.81

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8278) |
| 1 + 2 = 3 |
|-----|
| ID1= 1 (8280): 1479.71 15.903 7.50 22.47
+ ID2= 2 (8282): 773.94 8.554 7.25 24.81
|-----|
| ID = 3 (8278): 2253.65 24.428 7.50 23.28

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6394) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
|-----|

```

```

-----
|----- DATA FOR SECTION (3941.0) -----
|-----|
| Distance | Elevation | Manning |
|-----|
| 0.00 | 283.00 | 0.0380 |
| 13.48 | 282.87 | 0.0380 |
| 53.92 | 280.08 | 0.0380 |
| 74.13 | 276.62 | 0.0380 |
| 97.72 | 265.45 | 0.0380 |
| 114.57 | 256.93 | 0.0380 |
| 131.42 | 253.04 | 0.0380 |
| 134.79 | 252.58 | 0.0380 |
| 138.53 | 251.74 | 0.0380 / 0.0300 Main Channel |
| 139.03 | 251.20 | 0.0300 Main Channel |
| 141.53 | 251.20 | 0.0300 Main Channel |
| 144.03 | 251.20 | 0.0300 Main Channel |
| 144.53 | 251.74 | 0.0300 / 0.0380 Main Channel |
| 148.27 | 252.69 | 0.0380 |
| 151.64 | 252.97 | 0.0380 |
| 185.34 | 255.08 | 0.0380 |
| 219.03 | 257.54 | 0.0380 |
| 262.84 | 259.43 | 0.0380 |
| 310.02 | 262.80 | 0.0380 |
| 333.60 | 283.00 | 0.0380 |

```

```

-----
|----- TRAVEL TIME TABLE -----
|-----|
| DEPTH | ELEV | VOLUME | FLOW RATE | VELOCITY | TRAV.TIME |
| (m) | (m) | (cu.m.) | (cms) | (m/s) | (min) |
|-----|
| 0.54 | 251.74 | .827E+04 | 4.5 | 1.51 | 30.71 |
| 2.19 | 253.39 | .758E+05 | 73.2 | 2.69 | 17.27 |
| 3.83 | 255.03 | .282E+06 | 331.8 | 3.27 | 14.17 |
| 5.48 | 256.68 | .633E+06 | 917.3 | 4.03 | 11.51 |
| 7.12 | 258.32 | .112E+07 | 1889.5 | 4.69 | 9.90 |
| 8.77 | 259.97 | .179E+07 | 3410.2 | 5.31 | 8.73 |
| 10.41 | 261.61 | .259E+07 | 5627.6 | 6.05 | 7.66 |
| 12.06 | 263.26 | .351E+07 | 8605.1 | 6.83 | 6.79 |
| 13.70 | 264.90 | .447E+07 | 12544.9 | 7.81 | 5.94 |
| 15.35 | 266.55 | .548E+07 | 17103.4 | 8.71 | 5.32 |
| 16.99 | 268.19 | .648E+07 | 22251.7 | 9.56 | 4.85 |
| 18.64 | 269.84 | .752E+07 | 27979.0 | 10.36 | 4.48 |
| 20.28 | 271.48 | .858E+07 | 34269.0 | 11.11 | 4.17 |
| 21.93 | 273.13 | .967E+07 | 41210.8 | 11.83 | 3.92 |
| 23.57 | 274.77 | .108E+08 | 48495.9 | 12.51 | 3.71 |
| 25.22 | 276.42 | .119E+08 | 56418.1 | 13.17 | 3.52 |
| 26.86 | 278.06 | .131E+08 | 64410.6 | 13.69 | 3.39 |
| 28.51 | 279.71 | .143E+08 | 73002.1 | 14.18 | 3.27 |
| 30.15 | 281.35 | .156E+08 | 81379.3 | 14.50 | 3.20 |

```

<---- hydrograph ----> <-pipe / channel->

```

-----
| AREA | QPEAK | TPEAK | R.V. | MAX DEPTH | MAX VEL |
| (ha) | (cms) | (hrs) | (mm) | (m) | (m/s) |
INFLOW : ID= 2 (8278) 2253.65 24.43 7.50 23.28 1.02 1.73
OUTFLOW: ID= 1 (6394) 2253.65 23.85 8.00 23.28 1.00 1.72
|-----|

```

```

-----
| ADD HYD (8276) |
| 1 + 2 = 3 |
|-----|
| ID1= 1 (0394): 325.45 3.577 7.25 23.32
+ ID2= 2 (6394): 2253.65 23.853 8.00 23.28
|-----|
| ID = 3 (8276): 2579.10 27.252 8.00 23.16

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6396) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
|-----|

```

```

-----
|----- DATA FOR SECTION (3961.0) -----
|-----|
| Distance | Elevation | Manning |
|-----|
| 0.00 | 263.00 | 0.0410 |
| 11.75 | 257.14 | 0.0410 |
| 23.50 | 253.97 | 0.0410 |
| 41.13 | 247.83 | 0.0410 |
| 76.38 | 232.09 | 0.0410 |
| 135.13 | 229.07 | 0.0410 |
| 149.82 | 228.97 | 0.0410 |
| 152.75 | 228.96 | 0.0410 / 0.0300 Main Channel |
| 154.19 | 228.73 | 0.0300 Main Channel |
| 154.69 | 228.20 | 0.0300 Main Channel |
| 155.69 | 228.20 | 0.0300 Main Channel |
| 156.69 | 228.20 | 0.0300 Main Channel |
| 157.19 | 228.73 | 0.0300 Main Channel |
| 158.63 | 228.95 | 0.0300 / 0.0410 Main Channel |
| 161.57 | 228.96 | 0.0410 |
| 164.51 | 229.71 | 0.0410 |
| 196.82 | 241.70 | 0.0410 |
| 223.26 | 249.21 | 0.0410 |
| 246.76 | 255.13 | 0.0410 |
| 290.82 | 263.51 | 0.0410 |

```

```

-----
|----- TRAVEL TIME TABLE -----
|-----|
| DEPTH | ELEV | VOLUME | FLOW RATE | VELOCITY | TRAV.TIME |
| (m) | (m) | (cu.m.) | (cms) | (m/s) | (min) |
|-----|
| 0.75 | 228.95 | .780E+04 | 2.2 | 0.88 | 58.06 |
| 2.54 | 230.74 | .277E+06 | 145.1 | 1.78 | 31.76 |
| 4.33 | 232.53 | .786E+06 | 609.1 | 2.63 | 21.50 |
| 6.13 | 234.33 | .140E+07 | 1481.5 | 3.59 | 15.77 |
| 7.92 | 236.12 | .207E+07 | 2677.3 | 4.39 | 12.90 |
| 9.71 | 237.91 | .280E+07 | 4184.8 | 5.09 | 11.14 |
| 11.50 | 239.70 | .357E+07 | 6001.4 | 5.71 | 9.93 |
| 13.29 | 241.49 | .441E+07 | 8123.1 | 6.27 | 9.03 |
| 15.09 | 243.29 | .530E+07 | 10549.4 | 6.77 | 8.37 |
| 16.88 | 245.08 | .625E+07 | 13300.6 | 7.24 | 7.83 |
| 18.67 | 246.87 | .726E+07 | 16396.2 | 7.68 | 7.38 |
| 20.46 | 248.66 | .834E+07 | 19808.8 | 8.07 | 7.02 |
| 22.26 | 250.46 | .949E+07 | 23532.5 | 8.43 | 6.72 |
| 24.05 | 252.25 | .107E+08 | 27641.2 | 8.77 | 6.46 |
| 25.84 | 254.04 | .120E+08 | 32155.2 | 9.10 | 6.22 |
| 27.63 | 255.83 | .134E+08 | 36901.0 | 9.37 | 6.05 |
| 29.42 | 257.62 | .149E+08 | 42147.7 | 9.64 | 5.88 |
| 31.22 | 259.42 | .164E+08 | 48149.1 | 9.97 | 5.68 |
| 33.01 | 261.21 | .181E+08 | 54644.8 | 10.29 | 5.51 |

```

```

-----
|----- hydrograph ----> <-pipe / channel->
| AREA | QPEAK | TPEAK | R.V. | MAX DEPTH | MAX VEL |
| (ha) | (cms) | (hrs) | (mm) | (m) | (m/s) |
INFLOW : ID= 2 (8276) 2579.10 27.25 8.00 23.16 1.06 1.06
OUTFLOW: ID= 1 (6396) 2579.10 24.64 9.00 23.16 1.03 1.05
|-----|

```

```

-----
| ADD HYD (8274) |
| 1 + 2 = 3 |
|-----|
| ID1= 1 (0396): 305.21 4.736 7.50 34.96
+ ID2= 2 (6396): 2579.10 24.639 9.00 23.16
|-----|
| ID = 3 (8274): 2884.31 28.747 8.75 24.40

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8270) |
| 1 + 2 = 3 |
|-----|
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |

```

```

-----
      (ha)  (cms)  (hrs)  (mm)
ID1= 1 (8272): 2776.21 20.788 10.25 25.87
+ ID2= 2 (8274): 2884.31 28.747  8.75 24.40
=====
ID = 3 (8270): 5660.52 48.391  9.25 25.12

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
RESERVOIR (5506)
IN= 2--> OUT= 1
DT= 15.0 min
-----
      OUTFLOW STORAGE  OUTFLOW STORAGE
      (cms)  (ha.m.)  (cms)  (ha.m.)
0.0000  0.0000  60.8810 135.6832
31.1490 24.6697  96.2770 900.4431
36.8120 37.0045  96.4770 900.4431
45.3070 86.3439  0.0000  0.0000
-----
      AREA  QPEAK  TPEAK  R.V.
      (ha)  (cms)  (hrs)  (mm)
INFLOW : ID= 2 (8270) 5660.520 48.391  9.25 25.12
OUTFLOW: ID= 1 (5506) 5660.520 35.821 12.25 25.12
-----
      PEAK FLOW REDUCTION [Qout/qin](%)= 74.02
      TIME SHIFT OF PEAK FLOW (min)=180.00
      MAXIMUM STORAGE USED (ha.m.)= 34.8735

```

```

-----
CALIB (0406)
NASHYD (0406) Area (ha)= 142.65 Curve Number (CN)= 66.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.59
-----

```

Unit Hyd Qpeak (cms)= 4.135

```

PEAK FLOW (cms)= 3.269 (1)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 31.802
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.359

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB (0404)
NASHYD (0404) Area (ha)= 246.46 Curve Number (CN)= 47.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.98
-----

```

Unit Hyd Qpeak (cms)= 4.280

```

PEAK FLOW (cms)= 2.141 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 18.650
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.211

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB (0402)
NASHYD (0402) Area (ha)= 244.00 Curve Number (CN)= 61.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.07
-----

```

Unit Hyd Qpeak (cms)= 3.879

```

PEAK FLOW (cms)= 3.028 (1)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 28.095
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.317

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB (0400)
NASHYD (0400) Area (ha)= 93.97 Curve Number (CN)= 52.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.44
-----

```

Unit Hyd Qpeak (cms)= 3.630

```

PEAK FLOW (cms)= 1.763 (1)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 21.182
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.239

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB (0398)
NASHYD (0398) Area (ha)= 328.19 Curve Number (CN)= 55.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.83
-----

```

Unit Hyd Qpeak (cms)= 6.759

```

PEAK FLOW (cms)= 4.178 (1)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 23.611
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.267

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
ADD HYD (8268)
1 + 2 = 3
-----
      AREA  QPEAK  TPEAK  R.V.
      (ha)  (cms)  (hrs)  (mm)
ID1= 1 (0398): 328.19 4.178  7.00 23.61
+ ID2= 2 (0400): 93.97 1.763  6.50 21.18
=====
ID = 3 (8268): 422.16 5.776  6.75 23.07

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ROUTE CHN (6402)
IN= 2--> OUT= 1
Routing time step (min)= 15.00
-----

```

<----- DATA FOR SECTION (4021.0) ----->

Distance	Elevation	Manning
0.00	138.50	0.0360
11.50	238.00	0.0360
23.00	237.93	0.0360
34.49	236.39	0.0360
63.24	233.98	0.0360
97.73	228.15	0.0360
123.60	227.08	0.0360
126.48	226.61	0.0360
127.60	226.47	0.0360 / 0.0330
127.85	225.25	0.0330
129.35	225.25	0.0330
130.85	225.25	0.0330
131.10	226.47	0.0330 / 0.0360
132.22	226.59	0.0360
143.72	227.42	0.0360
169.59	227.88	0.0360
192.59	231.19	0.0360
218.46	233.02	0.0360
241.45	235.50	0.0360
284.57	236.43	0.0360

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.41	225.66	.391E+04	1.7	1.36	38.31
0.81	226.06	.804E+04	5.0	1.93	27.02
1.22	226.47	1.248E+05	9.1	2.30	22.61
1.84	227.09	3.08E+05	23.3	2.36	22.03
2.46	227.71	9.04E+05	62.0	2.14	24.31
3.09	228.34	2.23E+06	186.3	2.63	19.75
3.71	228.96	3.79E+06	410.2	3.38	15.39
4.33	229.58	5.50E+06	711.5	4.04	12.89
4.95	230.20	7.37E+06	1091.3	4.62	11.25
5.58	230.83	9.39E+06	1550.6	5.15	10.10
6.20	231.45	1.16E+07	2070.4	5.59	9.31
6.82	232.07	1.40E+07	2659.2	5.93	8.77
7.44	232.69	1.67E+07	3345.8	6.27	8.30
8.07	233.32	1.95E+07	4159.0	6.64	7.83
8.69	233.94	2.26E+07	5100.8	7.03	7.39
9.31	234.56	2.59E+07	6081.3	7.32	7.11
9.93	235.18	2.95E+07	7183.1	7.60	6.84
10.56	235.81	3.34E+07	8445.2	7.62	6.83
11.18	236.43	3.79E+07	9115.0	7.51	6.92

```

-----
hydrograph
-----
      AREA  QPEAK  TPEAK  R.V.  MAX DEPTH  MAX VEL
      (ha)  (cms)  (hrs)  (mm)  (m)  (m/s)
INFLOW : ID= 2 (8268) 422.16  5.78  6.75 23.07  0.89  1.99
OUTFLOW: ID= 1 (6402) 422.16  5.53  7.25 23.07  0.87  1.97

```

```

-----
ADD HYD (8266)
1 + 2 = 3
-----
      AREA  QPEAK  TPEAK  R.V.
      (ha)  (cms)  (hrs)  (mm)

```

ID1= 1 (0402): 244.00 3.028 7.50 28.09
 + ID2= 2 (6402): 422.16 8.530 7.25 23.07
 =====
 ID = 3 (8266): 666.16 8.550 7.25 24.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | ADD HYD (8264) |
 | 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 | (ha) (cms) (hrs) (mm) |
 ID1= 1 (0404): 246.46 2.141 7.25 18.65
 + ID2= 2 (8266): 666.16 8.550 7.25 24.91
 =====
 ID = 3 (8264): 912.62 10.690 7.25 23.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | ADD HYD (8262) |
 | 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 | (ha) (cms) (hrs) (mm) |
 ID1= 1 (0406): 142.65 3.269 6.50 31.80
 + ID2= 2 (8264): 912.62 10.690 7.25 23.22
 =====
 ID = 3 (8262): 1055.27 13.662 7.00 24.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | ADD HYD (8260) |
 | 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 | (ha) (cms) (hrs) (mm) |
 ID1= 1 (5506): 5660.52 35.821 12.25 25.12
 + ID2= 2 (8262): 1055.27 13.662 7.00 24.38
 =====
 ID = 3 (8260): 6715.79 41.240 10.75 25.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | ADD HYD (8258) |
 | 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 | (ha) (cms) (hrs) (mm) |
 ID1= 1 (0408): 231.62 3.942 6.75 25.55
 + ID2= 2 (8260): 6715.79 41.240 10.75 25.01
 =====
 ID = 3 (8258): 6947.41 42.741 10.00 25.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | ADD HYD (8256) |
 | 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 | (ha) (cms) (hrs) (mm) |
 ID1= 1 (0410): 572.01 3.821 8.00 19.33
 + ID2= 2 (8258): 6947.41 42.741 10.00 25.03
 =====
 ID = 3 (8256): 7519.42 46.169 10.00 24.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | RESERVOIR (5507) |
 | IN= 2--> OUT= 1 |
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	90.6140	67.8416
39.6440	12.3348	*****	160.3529
48.1390	18.5023	*****	160.3629
67.9600	37.0045	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8256)	7519.421	46.169	10.00
OUTFLOW : ID= 1 (5507)	7519.421	43.949	12.25

PEAK FLOW REDUCTION [Qout/qin](%)	TIME SHIFT OF PEAK FLOW (min)	MAXIMUM STORAGE USED (ha.m.)
95.19	135.00	15.4685

 | CALIB |

 | NASHYD (0420) | Area (ha)= 175.82 Curve Number (CN)= 53.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 | U.H. Tp(hrs)= 0.81 |

Unit Hyd Qpeak (cms)= 3.692

PEAK FLOW (cms)= 2.139 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 22.269
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.252

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | CALIB |
 | NASHYD (0418) | Area (ha)= 182.79 Curve Number (CN)= 64.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 | U.H. Tp(hrs)= 1.05 |

Unit Hyd Qpeak (cms)= 2.966

PEAK FLOW (cms)= 2.512 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 30.508
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.345

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | CALIB |
 | NASHYD (0416) | Area (ha)= 439.30 Curve Number (CN)= 64.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 | U.H. Tp(hrs)= 1.29 |

Unit Hyd Qpeak (cms)= 5.832

PEAK FLOW (cms)= 5.193 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 30.583
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.346

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | CALIB |
 | NASHYD (0412) | Area (ha)= 238.70 Curve Number (CN)= 54.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 | U.H. Tp(hrs)= 0.80 |

Unit Hyd Qpeak (cms)= 5.088

PEAK FLOW (cms)= 3.028 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 22.921
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.259

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | CALIB |
 | STANHYD (4141) | Area (ha)= 43.70
 | ID= 1 DT=15.0 min | Total Imp(%)= 36.00 Dir. Conn.(%)= 17.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	15.73	27.97
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	539.75	40.00
Manning's n	0.013	0.250

Max. Eff. Inten. (mm/hr)	116.82	72.91
over (min)	15.00	30.00
Storage Coeff. (min)	8.13 (ii)	20.27 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.10	0.05

	TOTALS
PEAK FLOW (cms)	2.15 2.85 3.979 (iii)
TIME TO PEAK (hrs)	6.00 6.25 6.00
RUNOFF VOLUME (mm)	86.50 36.78 45.23
TOTAL RAINFALL (mm)	88.50 88.50 88.50
RUNOFF COEFFICIENT	0.98 0.42 0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

CN* = 64.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5414)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	3.4720	1.9177
	0.3820	0.3152	3.8720	1.9277
	1.8280	0.4690	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (4141)	43.700	3.979	6.00	45.23
OUTFLOW: ID= 1 (5414)	43.700	2.035	6.50	45.23

PEAK FLOW REDUCTION [Qout/Qin](%)= 51.13
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 0.6598

CALIB			
STANDHYD (4142)			
ID= 1 DT=15.0 min			
	Area	(ha)	Dir. Conn.(%)
		144.30	10.00
	Total Imp(%)	21.00	

	IMPERVIOUS	PERVIOUS (i)
	(mm)	(mm)
Surface Area	30.30	114.00
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	980.82	40.00
Manning's n	0.013	0.250

	Max. Eff. Inten. (mm/hr)	116.82	58.69
	cover (min)	15.00	30.00
	Storage Coeff. (min)	11.63 (ii)	24.87 (ii)
	Unit Hyd. Tpeak (min)	15.00	30.00
	Unit Hyd. peak (cms)	0.08	0.04
			TOTALS
PEAK FLOW	(cms)	3.76	8.48
TIME TO PEAK	(hrs)	6.00	6.25
RUNOFF VOLUME	(mm)	86.50	39.04
TOTAL RAINFALL	(mm)	88.50	88.50
RUNOFF COEFFICIENT		0.98	0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 64.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8254)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (4142):	144.30	9.979	6.25	39.04
+ ID2= 2 (5414):	43.70	2.035	6.50	45.23
ID = 3 (8254):	188.00	11.926	6.25	40.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8252)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1 = 1 (0412):	238.70	3.028	7.00	22.92
+ ID2= 2 (8254):	188.00	11.926	6.25	40.48
ID = 3 (8252):	426.70	14.425	6.25	30.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (4161)			
IN= 2--> OUT= 1			
Routing time step (min)= 15.00			
<----- DATA FOR SECTION (4161.0) ----->			
Distance	Elevation	Manning	
0.00	270.07	0.0340	
20.67	267.91	0.0340	
62.01	264.33	0.0340	

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.27	246.02	.295E+04	0.6	0.75	88.26
1.53	247.28	.168E+06	69.7	1.65	40.13
2.79	248.54	.450E+06	299.5	2.42	27.28
4.06	249.81	.962E+06	740.1	3.05	21.67
5.32	251.07	.158E+07	1440.0	3.61	18.33
6.58	252.33	.235E+07	2443.8	4.12	16.06
7.84	253.59	.327E+07	3811.2	4.63	14.30
9.10	254.85	.432E+07	5565.1	5.11	12.94
10.37	256.12	.550E+07	7721.3	5.57	11.88
11.63	257.38	.682E+07	10309.9	6.00	11.02
12.89	258.64	.827E+07	13369.6	6.42	10.31
14.15	259.90	.985E+07	16884.5	6.81	9.72
15.42	261.17	.116E+08	20818.4	7.14	9.26
16.68	262.43	.134E+08	25510.4	7.53	8.78
17.94	263.69	.154E+08	30958.0	7.97	8.30
19.20	264.95	.175E+08	36988.3	8.38	7.89
20.46	266.21	.197E+08	43617.1	8.78	7.53
21.73	267.48	.220E+08	50865.1	9.17	7.22
22.99	268.74	.244E+08	58854.0	9.56	6.92

TRAVEL TIME TABLE						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.27	246.02	.295E+04	0.6	0.75	88.26	
1.53	247.28	.168E+06	69.7	1.65	40.13	
2.79	248.54	.450E+06	299.5	2.42	27.28	
4.06	249.81	.962E+06	740.1	3.05	21.67	
5.32	251.07	.158E+07	1440.0	3.61	18.33	
6.58	252.33	.235E+07	2443.8	4.12	16.06	
7.84	253.59	.327E+07	3811.2	4.63	14.30	
9.10	254.85	.432E+07	5565.1	5.11	12.94	
10.37	256.12	.550E+07	7721.3	5.57	11.88	
11.63	257.38	.682E+07	10309.9	6.00	11.02	
12.89	258.64	.827E+07	13369.6	6.42	10.31	
14.15	259.90	.985E+07	16884.5	6.81	9.72	
15.42	261.17	.116E+08	20818.4	7.14	9.26	
16.68	262.43	.134E+08	25510.4	7.53	8.78	
17.94	263.69	.154E+08	30958.0	7.97	8.30	
19.20	264.95	.175E+08	36988.3	8.38	7.89	
20.46	266.21	.197E+08	43617.1	8.78	7.53	
21.73	267.48	.220E+08	50865.1	9.17	7.22	
22.99	268.74	.244E+08	58854.0	9.56	6.92	

<---- hydrograph ---->						
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8252)	426.70	14.42	6.25	30.66	0.52	0.84
OUTFLOW: ID= 1 (6416)	426.70	6.92	7.25	30.66	0.39	0.79

ADD HYD (8250)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0416):	439.30	5.193	8.00	30.58
+ ID2= 2 (6416):	426.70	6.920	7.25	30.66
ID = 3 (8250):	866.00	11.994	7.50	30.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8248)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1 = 1 (0418):	182.79	2.512	7.50	30.51
+ ID2= 2 (8250):	866.00	11.994	7.50	30.62
ID = 3 (8248):	1048.79	14.506	7.50	30.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8246)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1 = 1 (0420):	175.82	2.139	7.00	22.27
+ ID2= 2 (8248):	1048.79	14.506	7.50	30.60
ID = 3 (8246):	1224.61	16.598	7.25	29.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8244)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1 = 1 (5507):	7519.42	43.949	12.25	24.59
+ ID2= 2 (8246):	1224.61	16.598	7.25	29.40

ID = 3 (8244): 8744.03 53.161 10.00 25.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ADD HYD (8242) |
1 + 2 = 3
AREA
(ha)
ID1= 1 (0422): 780.20
+ ID2= 2 (8244): 8744.03

ID = 3 (8242): 9524.23

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| RESERVOIR (5508) |
| IN= 2--> OUT= 1 |
DT= 15.0 min
OUTFLOW
(cms)
0.0000
76.4550

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8242) 9524.231	58.066	10.00	25.09
OUTFLOW : ID= 1 (5508) 9524.231	56.167	11.75	25.09

PEAK FLOW REDUCTION [Qout/Qin](%)= 96.73
TIME SHIFT OF PEAK FLOW (min)=105.00
MAXIMUM STORAGE USED (ha.m.)= 22.6576

| CALIB |
| NASHYD (0336) |
ID= 1 DT=15.0 min
Area (ha)=2785.00
Ia (mm)= 5.00
U.H. Tp(hrs)= 15.39

Unit Hyd Qpeak (cms)= 3.090

PEAK FLOW (cms)= 4.639 (i)
TIME TO PEAK (hrs)= 22.250
RUNOFF VOLUME (mm)= 38.196
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.432

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| STANDHYD (3382) |
ID= 1 DT=15.0 min
Area (ha)= 525.90
Total Imp(%)= 37.00

Surface Area (ha)= 194.58
Dep. Storage (mm)= 2.00
Average Slope (%)= 0.50
Length (m)= 1874.43
Mannings n = 0.013

Max.Eff.Inten.(mm/hr)= 116.82
over (min)= 15.00
Storage Coeff. (min)= 17.14 (ii)
Unit Hyd. Tpeak (min)= 15.00
Unit Hyd. peak (cms)= 0.07

PEAK FLOW (cms)= 19.91
TIME TO PEAK (hrs)= 6.00
RUNOFF VOLUME (mm)= 86.50
TOTAL RAINFALL (mm)= 88.50
RUNOFF COEFFICIENT = 0.98

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 66.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| STANDHYD (3381) |
ID= 1 DT=15.0 min
Area (ha)= 73.00
Total Imp(%)= 49.00

Surface Area (ha)= 35.77
Dep. Storage (mm)= 2.00
Average Slope (%)= 0.50
Length (m)= 697.61
Mannings n = 0.013

Max.Eff.Inten.(mm/hr)= 116.82
over (min)= 15.00
Storage Coeff. (min)= 9.48 (ii)
Unit Hyd. Tpeak (min)= 15.00
Unit Hyd. peak (cms)= 0.09

PEAK FLOW (cms)= 4.67
TIME TO PEAK (hrs)= 6.00
RUNOFF VOLUME (mm)= 86.50
TOTAL RAINFALL (mm)= 88.50
RUNOFF COEFFICIENT = 0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 66.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| RESERVOIR (5338) |
| IN= 2--> OUT= 1 |
DT= 15.0 min
OUTFLOW
(cms)
0.0000
0.3230
0.9360
1.7590
2.4110

Area (ha)= 73.00
QPEAK (cms)= 7.996
TPEAK (hrs)= 6.00
R.V. (mm)= 52.41

INFLOW : ID= 2 (3381) 73.000
OUTFLOW : ID= 1 (5338) 73.000

PEAK FLOW REDUCTION [Qout/Qin](%)= 55.24
TIME SHIFT OF PEAK FLOW (min)= 30.00
MAXIMUM STORAGE USED (ha.m.)= 1.3680

| ADD HYD (8310) |
1 + 2 = 3
AREA
(ha)
ID1= 1 (3382): 525.90
+ ID2= 2 (5338): 73.00

ID = 3 (8310): 598.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| CALIB |
| NASHYD (3342) |
ID= 1 DT=15.0 min
Area (ha)= 587.10
Ia (mm)= 5.00
U.H. Tp(hrs)= 8.19

Unit Hyd Qpeak (cms)= 1.224

PEAK FLOW (cms)= 1.173 (i)
TIME TO PEAK (hrs)= 15.250
RUNOFF VOLUME (mm)= 24.620
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.278

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| STANDHYD (3341) |
ID= 1 DT=15.0 min
Area (ha)= 33.00
Total Imp(%)= 51.00

Surface Area (ha)= 16.83
Dep. Storage (mm)= 2.00
Average Slope (%)= 0.50
Length (m)= 469.04
Mannings n = 0.013

Max.Eff.Inten.(mm/hr)= 116.82
over (min)= 15.00
Storage Coeff. (min)= 7.47 (ii)
Unit Hyd. Tpeak (min)= 15.00

PEAK FLOW (cms)= 15.00
TIME TO PEAK (hrs)= 20.52 (ii)
RUNOFF VOLUME (mm)= 30.00
TOTAL RAINFALL (mm)= 30.00
RUNOFF COEFFICIENT = 0.34

Unit Hyd. peak (cms)= 0.10 0.05 *TOTALS*
 PEAK FLOW (cms)= 3.02 1.37 3.881 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 86.50 28.70 46.62
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.32 0.53

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 52.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5334)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.6540	0.4604
0.0390	0.1699	0.8330	0.5249
0.1860	0.2610	1.0130	0.5896
0.3750	0.3534	1.4130	0.5996
0.4340	0.3813	0.0000	0.0000

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (3341) 33.000 3.881 6.00 46.62
 OUTFLOW: ID= 1 (5334) 33.000 1.849 6.25 46.58

PEAK FLOW REDUCTION [Qout/Qin](%)= 47.65
 TIME SHIFT OF PEAK FLOW (min)= 15.00
 MAXIMUM STORAGE USED (ha.m.)= 0.6246

ADD HYD (8314)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (3342):	587.10	1.173	15.25	24.62
+ ID2= 2 (5334):	33.00	1.849	6.25	46.58
ID = 3 (8314):	620.10	2.136	6.25	25.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0306)
 NASHYD (0306) Area (ha)= 283.97 Curve Number (CN)= 52.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 6.44

Unit Hyd Qpeak (cms)= 0.753

PEAK FLOW (cms)= 0.637 (i)
 TIME TO PEAK (hrs)= 13.750
 RUNOFF VOLUME (mm)= 21.914
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.248

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0286)
 NASHYD (0286) Area (ha)= 260.51 Curve Number (CN)= 84.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.16

Unit Hyd Qpeak (cms)= 3.834

PEAK FLOW (cms)= 5.908 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 52.435
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.592

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0282)
 NASHYD (0282) Area (ha)= 449.38 Curve Number (CN)= 77.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.47

Unit Hyd Qpeak (cms)= 5.226

PEAK FLOW (cms)= 6.939 (i)
 TIME TO PEAK (hrs)= 8.000

RUNOFF VOLUME (mm)= 43.497
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.491

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0284)
 NASHYD (0284) Area (ha)= 78.93 Curve Number (CN)= 84.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.57

Unit Hyd Qpeak (cms)= 2.344

PEAK FLOW (cms)= 3.150 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 51.645
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.584

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8388)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0282):	449.38	6.939	8.00	43.50
+ ID2= 2 (0284):	78.93	3.150	6.50	51.65
ID = 3 (8388):	528.31	9.369	7.25	44.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (5286)
 IN= 2--> OUT= 1 Routing time step (min)= 15.00

DATA FOR SECTION (2861.0) ----->

Distance	Elevation	Manning
0.00	233.00	0.0450
20.58	228.51	0.0450
41.17	227.74	0.0450
61.46	227.41	0.0450
81.77	225.96	0.0450
102.09	223.94	0.0450
122.42	220.84	0.0450
142.77	220.66	0.0450
163.13	220.22	0.0450
183.50	220.01	0.0450 / 0.0350
203.88	219.70	0.0350
224.28	219.72	0.0350
244.69	220.06	0.0350
265.11	220.23	0.0350
285.54	221.64	0.0350
305.98	224.98	0.0450
326.43	432.25	0.0450
346.89	457.98	0.0450
367.36	483.71	0.0450
387.84	509.44	0.0450

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.31	220.01	.382E+04	0.8	0.57	82.89
1.06	220.76	.525E+05	12.1	0.66	72.51
1.81	221.51	.240E+06	88.6	1.05	45.21
2.56	222.26	.510E+06	261.7	1.46	32.47
3.31	223.01	.843E+06	533.8	1.81	26.34
4.06	223.76	.124E+07	933.6	2.10	22.64
4.81	224.51	.171E+07	1399.0	2.34	20.33
5.56	225.26	.225E+07	2051.0	2.60	18.27
6.31	226.01	.285E+07	2907.0	2.91	16.35
7.06	226.76	.352E+07	3902.5	3.16	15.04
7.82	227.52	.426E+07	5069.9	3.39	14.01
8.57	228.27	.507E+07	6433.0	3.62	13.13
9.32	229.02	.593E+07	8069.2	3.89	12.24
10.07	229.77	.681E+07	9926.7	4.16	11.44
10.82	230.52	.772E+07	11962.8	4.42	10.75
11.57	231.27	.864E+07	14167.1	4.68	10.17
12.32	232.02	.959E+07	16537.1	4.92	9.66
13.07	232.77	.105E+08	19070.7	5.16	9.22
13.82	233.52	.115E+08	21790.4	5.39	8.82

hydrograph ----> <-pipe / channel-->

INFLOW : ID= 2 (8388)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW: ID= 1 (6286)	528.31	9.37	7.25	44.71	0.88	0.63
	528.31	7.94	9.00	44.71	0.78	0.62

```

-----
| ADD HYD (8386) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0286): 260.51 5.908 7.50 52.44 |
| + ID2= 2 (6286): 528.31 7.939 9.00 44.71 |
|=====|
| ID = 3 (8386): 788.82 13.331 8.25 47.26 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0302) | Area (ha)= 473.90 Curve Number (CN)= 58.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 1.66 |
-----

```

Unit Hyd Qpeak (cms) = 4.874

```

PEAK FLOW (cms) = 3.864 (1)
TIME TO PEAK (hrs) = 8.500
RUNOFF VOLUME (mm) = 25.946
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.293

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0300) | Area (ha)= 258.93 Curve Number (CN)= 52.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 4.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 1.03 |
-----

```

Unit Hyd Qpeak (cms) = 4.292

```

PEAK FLOW (cms) = 2.593 (1)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 22.167
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.250

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0298) | Area (ha)= 330.51 Curve Number (CN)= 45.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 1.26 |
-----

```

Unit Hyd Qpeak (cms) = 4.461

```

PEAK FLOW (cms) = 2.236 (1)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 17.571
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.199

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8395) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0298): 330.51 2.236 8.00 17.57 |
| + ID2= 2 (0300): 258.93 2.593 7.50 22.17 |
|=====|
| ID = 3 (8395): 589.44 4.798 7.75 19.59 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0288) | Area (ha)= 340.83 Curve Number (CN)= 78.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 2.21 |
-----

```

Unit Hyd Qpeak (cms) = 2.629

```

PEAK FLOW (cms) = 3.883 (1)
TIME TO PEAK (hrs) = 9.250
RUNOFF VOLUME (mm) = 44.801
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.506

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0290) | Area (ha)= 269.18 Curve Number (CN)= 78.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 1.07 |
-----

```

Unit Hyd Qpeak (cms) = 4.279

```

PEAK FLOW (cms) = 5.447 (1)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 44.529
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.503

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8397) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0288): 340.83 3.883 9.25 44.80 |
| + ID2= 2 (0290): 269.18 5.447 7.25 44.53 |
|=====|
| ID = 3 (8397): 610.01 9.010 8.00 44.68 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0296) | Area (ha)= 293.65 Curve Number (CN)= 76.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 1.13 |
-----

```

Unit Hyd Qpeak (cms) = 4.437

```

PEAK FLOW (cms) = 5.388 (1)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 42.226
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.477

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0292) | Area (ha)= 738.49 Curve Number (CN)= 68.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 1.82 |
-----

```

Unit Hyd Qpeak (cms) = 8.289

```

PEAK FLOW (cms) = 8.591 (1)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 34.153
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.386

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0294) | Area (ha)= 274.15 Curve Number (CN)= 57.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 0.87 |
-----

```

Unit Hyd Qpeak (cms) = 5.367

```

PEAK FLOW (cms) = 3.559 (1)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 25.026
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.283

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8398) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0292): 738.49 8.591 8.00 34.15 |
| + ID2= 2 (0294): 274.15 3.559 7.00 25.03 |
|=====|
| ID = 3 (8398): 1012.64 11.892 8.00 31.68 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ROUTE CN (6296) |

| IN= 2--> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (2961.0) -----				
Distance	Elevation	Manning		
0.00	243.98	0.0400		
42.59	243.18	0.0400		
85.17	241.81	0.0400		
120.66	240.50	0.0400		
156.15	239.56	0.0400		
198.74	236.15	0.0400		
237.78	234.01	0.0400		
241.33	233.82	0.0400		
248.77	233.12	0.0400 / 0.0400	Main Channel	
249.87	232.32	0.0400	Main Channel	
250.37	231.80	0.0400	Main Channel	
250.87	232.23	0.0400	Main Channel	
251.87	233.10	0.0400 / 0.0400	Main Channel	
255.37	233.22	0.0400		
259.07	233.87	0.0400		
262.62	234.12	0.0400		
266.17	234.23	0.0400		
283.91	234.73	0.0400		
337.15	241.75	0.0400		
351.34	244.00	0.0400		

----- TRAVEL TIME TABLE -----					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.43	232.23	.510E+03	0.1	0.53	81.07
0.87	232.67	.213E+04	0.7	0.83	51.49
1.30	233.10	.501E+04	2.2	1.10	38.75
1.98	233.78	.242E+05	13.6	1.34	31.93
2.66	234.46	.738E+05	42.1	1.46	29.21
3.34	235.14	.177E+06	127.9	1.84	23.13
4.02	235.62	.314E+06	281.4	2.29	18.61
4.70	236.50	.481E+06	509.4	2.71	15.73
5.38	237.18	.672E+06	815.6	3.11	13.73
6.06	237.86	.886E+06	1200.7	3.46	12.30
6.74	238.54	.112E+07	1659.4	3.80	11.23
7.42	239.22	.139E+07	2226.9	4.11	10.38
8.10	239.90	.168E+07	2808.4	4.28	9.95
8.78	240.58	.202E+07	3470.4	4.40	9.68
9.46	241.26	.240E+07	4331.7	4.61	9.24
10.14	241.94	.283E+07	5335.2	4.83	8.84
10.82	242.62	.330E+07	6482.6	5.03	8.48
11.50	243.30	.381E+07	7764.1	5.21	8.18
12.18	243.98	.439E+07	9101.2	5.31	8.04

		----- hydrograph -----				<-pipe / channel-->			
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)			
INFLOW : ID= 2 (8398) :	1012.64	11.89	8.00	31.68	1.93	1.32			
OUTFLOW : ID= 1 (6296) :	1012.64	11.62	8.25	31.68	1.91	1.31			

ADD HYD (8396)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0296) :	293.65	5.388	7.50	42.23
+ ID2= 2 (6296) :	1012.64	11.616	8.25	31.68
ID = 3 (8396) :	1306.29	16.780	8.00	34.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8394)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8396) :	1306.29	16.780	8.00	34.05
+ ID2= 2 (8397) :	610.01	9.010	8.00	44.68
ID = 3 (8394) :	1916.30	25.790	8.00	37.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8392)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8394) :	1916.30	25.790	8.00	37.44
+ ID2= 2 (8395) :	589.44	4.798	7.75	19.59
ID = 3 (8392) :	2505.74	30.563	8.00	33.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ROUTE CHN (6302) | Routing time step (min)= 15.00

----- DATA FOR SECTION (3021.0) -----				
Distance	Elevation	Manning		
0.00	228.10	0.0400		
18.47	227.12	0.0400		
36.95	226.12	0.0400		
46.18	225.84	0.0400		
55.42	225.58	0.0400		
272.47	222.88	0.0400		
277.09	222.76	0.0400		
281.71	222.58	0.0400		
288.54	222.18	0.0400 / 0.0300	Main Channel	
288.64	221.00	0.0300	Main Channel	
290.94	221.00	0.0300	Main Channel	
291.04	221.00	0.0300	Main Channel	
291.54	222.75	0.0300 / 0.0400	Main Channel	
300.18	222.83	0.0400		
304.80	223.04	0.0400		
309.42	223.25	0.0400		
318.65	223.69	0.0400		
360.22	225.57	0.0400		
397.16	227.60	0.0400		
457.20	228.35	0.0400		

----- TRAVEL TIME TABLE -----					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.29	221.29	.228E+04	0.2	0.32	165.63
0.59	221.59	.463E+04	0.7	0.45	116.15
0.88	221.88	.713E+04	1.2	0.54	96.87
1.18	222.18	.971E+04	1.9	0.61	86.20
1.57	222.57	.175E+05	3.3	0.59	88.86
1.97	222.97	.427E+05	6.4	0.47	111.38
2.36	223.36	.115E+06	16.1	0.44	119.14
2.76	223.76	.237E+06	37.1	0.49	106.49
3.15	224.15	.409E+06	73.9	0.56	93.40
3.55	224.55	.631E+06	127.0	0.63	82.76
3.94	224.94	.903E+06	202.4	0.70	74.37
4.34	225.34	.123E+07	301.8	0.77	67.68
4.73	225.73	.160E+07	433.5	0.85	61.37
5.13	226.13	.200E+07	602.2	0.95	55.29
5.52	226.52	.242E+07	804.7	1.05	50.13
5.92	226.92	.286E+07	1034.2	1.14	46.11
6.31	227.31	.332E+07	1290.5	1.22	42.88
6.71	227.71	.380E+07	1560.9	1.29	40.55
7.10	228.10	.432E+07	1835.6	1.34	39.19

		----- hydrograph -----				<-pipe / channel-->			
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)			
INFLOW : ID= 2 (8392) :	2505.74	30.56	8.00	33.24	2.64	0.47			
OUTFLOW : ID= 1 (6302) :	2505.74	24.15	10.00	33.24	2.52	0.46			

ADD HYD (8390)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0302) :	473.90	3.864	8.50	25.95
+ ID2= 2 (6302) :	2505.74	24.155	10.00	33.24
ID = 3 (8390) :	2979.64	27.780	10.00	32.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8348)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8386) :	786.82	13.331	8.25	47.26
+ ID2= 2 (8390) :	2979.64	27.780	10.00	32.08
ID = 3 (8348) :	3766.46	40.043	9.50	35.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB				
NASHYD (0304) :	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res. (N)
ID= 1 DT=15.0 min :	292.37	5.00	63.0	1.50
	U.H. Tp (hrs)			
		2.78		

Unit Hyd Qpeak (cms) = 1.793
 PEAK FLOW (cms) = 1.844 (1)
 TIME TO PEAK (hrs) = 10.250
 RUNOFF VOLUME (mm) = 29.899

TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.338

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
| NASHYD (0280) | Area (ha)= 299.86 Curve Number (CN)= 82.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----| U.H. Tp(hrs)= 0.85

Unit Hyd Qpeak (cms)= 6.009

PEAK FLOW (cms)= 8.228 (1)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 49.420
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.558

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
| NASHYD (0278) | Area (ha)= 485.49 Curve Number (CN)= 82.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----| U.H. Tp(hrs)= 1.52

Unit Hyd Qpeak (cms)= 5.453

PEAK FLOW (cms)= 8.411 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 49.794
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.563

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
| NASHYD (0276) | Area (ha)= 90.89 Curve Number (CN)= 79.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----| U.H. Tp(hrs)= 0.67

Unit Hyd Qpeak (cms)= 2.302

PEAK FLOW (cms)= 2.744 (1)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 45.323
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.512

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
| NASHYD (0274) | Area (ha)= 392.49 Curve Number (CN)= 75.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----| U.H. Tp(hrs)= 1.08

Unit Hyd Qpeak (cms)= 6.182

PEAK FLOW (cms)= 7.225 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 41.085
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.464

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8360) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
| ID1= 1 (0274): 392.49 7.225 7.25 41.09
+ ID2= 2 (0276): 90.89 2.744 6.75 45.32
=====|
ID = 3 (8360): 483.38 9.821 7.00 41.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8358) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
| ID1= 1 (0278): 485.49 8.411 8.00 49.79
+ ID2= 2 (8360): 483.38 9.821 7.00 41.88
=====|

ID = 3 (8358): 968.87 17.878 7.50 45.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6280) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (2801.0) -----
Distance Elevation Manning
0.00 241.14 0.0500
13.32 240.80 0.0500
39.95 240.07 0.0500
96.54 236.21 0.0500
113.19 234.15 0.0500
123.18 232.35 0.0500
143.15 225.80 0.0500
149.81 225.62 0.0500
153.14 225.40 0.0500
157.30 224.76 0.0500 / 0.0300 Main Channel
159.80 224.26 0.0300 Main Channel
162.30 224.85 0.0300 Main Channel
162.55 224.97 0.0300 / 0.0500 Main Channel
167.80 225.05 0.0500
186.43 229.14 0.0500
213.06 234.75 0.0500
236.37 237.09 0.0500
266.33 237.31 0.0500
292.96 237.83 0.0500
329.58 241.50 0.0500

----- TRAVEL TIME TABLE -----
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.50 224.76 .263E+04 0.8 0.65 58.31
1.36 225.62 .278E+05 13.4 1.10 34.54
2.22 226.48 .855E+05 52.6 1.40 27.09
3.09 227.35 .157E+06 120.4 1.74 21.78
3.95 228.21 .242E+06 216.3 2.04 18.64
4.81 229.07 .340E+06 341.8 2.29 16.56
5.67 229.93 .450E+06 498.3 2.52 15.06
6.53 230.79 .574E+06 688.3 2.73 13.90
7.40 231.66 .711E+06 913.8 2.93 12.97
8.26 232.52 .861E+06 1173.4 3.11 12.23
9.12 233.38 .103E+07 1461.3 3.24 11.72
9.98 234.24 .121E+07 1793.3 3.37 11.26
10.85 235.11 .142E+07 2141.0 3.45 11.02
11.71 235.97 .165E+07 2530.4 3.50 10.86
12.57 236.83 .192E+07 2954.0 3.51 10.81
13.43 237.69 .226E+07 3165.3 3.18 11.93
14.29 238.55 .271E+07 3869.4 3.26 11.67
15.16 239.42 .320E+07 4740.3 3.38 11.24
16.02 240.28 .373E+07 5688.4 3.48 10.92

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8358) 968.87 17.88 7.50 45.85 1.46 1.13
OUTFLOW: ID= 1 (6280) 968.87 17.31 8.25 45.85 1.45 1.12

ADD HYD (8354) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
+ ID1= 1 (0280): 299.86 8.228 7.00 49.42
+ ID2= 2 (6280): 968.87 17.313 8.25 45.85
=====|
ID = 3 (8354): 1268.73 24.661 7.75 46.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB |
| NASHYD (0272) | Area (ha)= 157.38 Curve Number (CN)= 75.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----| U.H. Tp(hrs)= 1.09

Unit Hyd Qpeak (cms)= 2.456

PEAK FLOW (cms)= 2.876 (1)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 41.090
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.464

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |

NASHYD (0270) Area (ha)= 243.61 Curve Number (CN)= 81.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 0.87

Unit Hyd Qpeak (cms)= 3.429
 PEAK FLOW (cms)= 4.722 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 48.074
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.543

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0268) Area (ha)= 215.76 Curve Number (CN)= 75.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 0.69

Unit Hyd Qpeak (cms)= 3.807
 PEAK FLOW (cms)= 4.211 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 40.714
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.460

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0264) Area (ha)= 353.96 Curve Number (CN)= 69.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.30

Unit Hyd Qpeak (cms)= 3.313
 PEAK FLOW (cms)= 3.548 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 35.001
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.395

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0266) Area (ha)= 508.09 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.63

Unit Hyd Qpeak (cms)= 3.786
 PEAK FLOW (cms)= 3.687 (i)
 TIME TO PEAK (hrs)= 9.000
 RUNOFF VOLUME (mm)= 30.617
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.346

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8380)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0264): 353.96 3.548 8.00 35.00
 + ID2= 2 (0266): 508.09 3.687 9.00 30.62
 =====
 ID = 3 (8380): 862.05 7.203 8.50 32.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6270)
 IN= 2--> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2681.0) ----->

Distance	Elevation	Manning
0.00	274.03	0.0500
10.55	272.73	0.0500
21.10	271.28	0.0500
50.12	266.83	0.0500
87.04	260.36	0.0500
92.32	260.06	0.0500
94.85	259.83	0.0500
101.00	256.87	0.0500
103.00	256.58	0.0500 / 0.0350 Main Channel
104.00	256.32	0.0350 Main Channel

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
106.00	256.05	0.0350			
108.00	256.25	0.0350			
110.78	256.65	0.0350 / 0.0500			
113.42	256.81	0.0500			
116.06	257.00	0.0500			
155.62	260.34	0.0500			
187.27	263.35	0.0500			
211.01	267.22	0.0500			
224.20	269.60	0.0500			
261.13	272.68	0.0500			

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.53	256.58	.697E+04	2.1	0.97	55.64
1.38	257.43	.477E+05	25.9	1.76	30.66
2.22	258.27	.133E+06	84.3	2.22	24.26
3.07	259.12	.231E+06	185.4	2.60	20.72
3.92	259.97	.371E+06	335.8	2.92	18.42
4.77	260.82	.560E+06	547.6	3.15	17.06
5.61	261.66	.790E+06	845.6	3.46	15.57
6.46	262.51	.106E+07	1225.0	3.75	14.37
7.31	263.36	.136E+07	1693.3	4.02	13.37
8.16	264.21	.169E+07	2283.2	4.35	12.36
9.00	265.05	.206E+07	2959.1	4.66	11.54
9.85	265.90	.245E+07	3754.4	4.96	10.86
10.70	266.75	.286E+07	4643.0	5.24	10.28
11.55	267.60	.331E+07	5636.1	5.50	9.78
12.39	268.44	.378E+07	6746.2	5.76	9.34
13.24	269.29	.428E+07	7972.2	6.01	8.96
14.09	270.14	.482E+07	9218.9	6.18	8.71
14.94	270.99	.539E+07	10590.5	6.32	8.52
15.78	271.83	.601E+07	12024.7	6.46	8.33

<---- hydrograph ----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8380)	862.05	7.20	8.50	32.42	0.71
OUTFLOW: ID= 1 (6268)	862.05	7.01	9.75	32.42	0.70

ADD HYD (8382)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0268): 215.76 4.211 7.00 40.71
 + ID2= 2 (6268): 862.05 7.011 9.75 32.42
 =====
 ID = 3 (8382): 1077.81 10.204 8.50 34.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6270)
 IN= 2--> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2701.0) ----->

Distance	Elevation	Manning
0.00	245.98	0.0500
8.14	245.66	0.0500
16.28	245.16	0.0500
20.35	244.84	0.0500
38.66	242.98	0.0500
48.83	240.65	0.0500
63.07	235.91	0.0500
65.11	235.18	0.0500
68.43	234.34	0.0500 / 0.0300 Main Channel
68.68	233.95	0.0300 Main Channel
69.18	233.89	0.0300 Main Channel
69.68	233.95	0.0300 Main Channel
71.21	234.48	0.0300 / 0.0500 Main Channel
81.38	236.44	0.0500
95.63	236.66	0.0500
120.04	237.00	0.0500
148.53	241.77	0.0500
158.70	242.34	0.0500
187.18	244.03	0.0500
201.42	244.36	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.45	234.34	.217E+04	0.7	0.99	52.83
0.98	234.87	.966E+04	4.6	1.49	35.20
1.50	235.39	.250E+05	13.7	1.73	30.41
2.03	235.92	.475E+05	29.5	1.95	26.87
2.56	236.45	.772E+05	52.5	2.14	24.50
3.09	236.98	.142E+06	82.4	1.83	28.73
3.61	237.50	.246E+06	158.1	2.03	25.91
4.14	238.03	.357E+06	262.0	2.31	22.73
4.67	238.56	.477E+06	392.1	2.59	20.27
5.20	239.09	.604E+06	547.8	2.86	18.38

5.72	239.61	.739E+06	729.2	3.11	16.90
6.25	240.14	.882E+06	936.5	3.34	15.70
6.78	240.67	.103E+07	1169.7	3.57	14.72
7.31	241.20	.119E+07	1425.9	3.77	13.94
7.83	241.72	.136E+07	1710.4	3.96	13.26
8.36	242.25	.154E+07	1967.9	4.02	13.07
8.89	242.78	.174E+07	2264.9	4.09	12.83
9.42	243.31	.196E+07	2589.3	4.15	12.65
9.94	243.83	.221E+07	2953.5	4.22	12.46

<---- hydrograph ----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8382) 1077.81	10.20	8.50	34.08	1.30	1.63
OUTFLOW: ID= 1 (6270) 1077.81	10.06	9.25	34.08	1.29	1.62

ADD HYD (8384)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID= 1 (0270):	243.61	4.722	7.25	48.07
+ ID= 2 (6270):	1077.81	10.057	9.25	34.08
=====	=====	=====	=====	=====
ID = 3 (8384):	1321.42	14.172	8.75	36.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 82.0
NASHYD (0262)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.30	
ID= 1 DT=15.0 min	U.H. Tp(hrs)=	1.01		

Unit Hyd Qpeak (cms)= 4.094

PEAK FLOW	(cms)= 5.995 (1)
TIME TO PEAK	(hrs)= 7.500
RUNOFF VOLUME	(mm)= 49.519
TOTAL RAINFALL	(mm)= 88.500
RUNOFF COEFFICIENT	= 0.560

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 82.0
NASHYD (0260)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.30	
ID= 1 DT=15.0 min	U.H. Tp(hrs)=	1.33		

Unit Hyd Qpeak (cms)= 4.360

PEAK FLOW	(cms)= 6.777 (1)
TIME TO PEAK	(hrs)= 8.000
RUNOFF VOLUME	(mm)= 49.681
TOTAL RAINFALL	(mm)= 88.500
RUNOFF COEFFICIENT	= 0.561

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 79.0
NASHYD (0258)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.30	
ID= 1 DT=15.0 min	U.H. Tp(hrs)=	1.18		

Unit Hyd Qpeak (cms)= 1.881

PEAK FLOW	(cms)= 2.620 (1)
TIME TO PEAK	(hrs)= 8.000
RUNOFF VOLUME	(mm)= 45.751
TOTAL RAINFALL	(mm)= 88.500
RUNOFF COEFFICIENT	= 0.517

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 73.0
NASHYD (0252)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.30	
ID= 1 DT=15.0 min	U.H. Tp(hrs)=	1.04		

Unit Hyd Qpeak (cms)= 3.761

PEAK FLOW	(cms)= 4.280 (1)
TIME TO PEAK	(hrs)= 8.000
RUNOFF VOLUME	(mm)= 38.873
TOTAL RAINFALL	(mm)= 88.500
RUNOFF COEFFICIENT	= 0.439

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 67.0
NASHYD (0256)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.30	
ID= 1 DT=15.0 min	U.H. Tp(hrs)=	1.04		

Unit Hyd Qpeak (cms)= 1.707

PEAK FLOW	(cms)= 1.643 (1)
TIME TO PEAK	(hrs)= 8.000
RUNOFF VOLUME	(mm)= 33.068
TOTAL RAINFALL	(mm)= 88.500
RUNOFF COEFFICIENT	= 0.374

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 55.0
NASHYD (0254)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.30	
ID= 1 DT=15.0 min	U.H. Tp(hrs)=	1.22		

Unit Hyd Qpeak (cms)= 4.028

PEAK FLOW	(cms)= 2.850 (1)
TIME TO PEAK	(hrs)= 8.000
RUNOFF VOLUME	(mm)= 33.726
TOTAL RAINFALL	(mm)= 88.500
RUNOFF COEFFICIENT	= 0.268

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6256)	Routing time step (min)= 15.00
------------------	--------------------------------

DATA FOR SECTION (2561.0) ----->			
Distance	Elevation	Manning	
0.00	276.07	0.0400	
11.68	273.71	0.0400	
23.36	271.35	0.0400	
29.19	270.30	0.0400	
35.03	269.44	0.0400	
55.47	267.30	0.0400	
78.82	266.24	0.0400	
90.50	265.63	0.0400	
102.18	265.40	0.0400	
105.10	264.95	0.0400 / 0.0350	Main Channel
108.02	264.39	0.0350	Main Channel
110.94	264.72	0.0350	Main Channel
113.86	265.19	0.0350 / 0.0400	Main Channel
116.78	265.49	0.0400	
143.05	268.24	0.0400	
172.25	270.53	0.0400	
207.28	271.95	0.0400	
233.55	273.72	0.0400	
256.91	274.98	0.0400	
289.02	275.97	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.56	264.95	.723E+04	1.9	0.90	62.51
1.14	265.53	.307E+05	13.5	1.49	37.92
1.72	266.11	.951E+05	48.6	1.73	32.62
2.30	266.69	.191E+06	117.8	2.09	27.03
2.88	267.27	.314E+06	225.2	2.43	23.24
3.46	267.85	.464E+06	375.5	2.74	20.59
4.04	268.43	.641E+06	573.2	3.03	18.62
4.62	269.01	.846E+06	821.9	3.29	17.15
5.20	269.59	.108E+07	1135.0	3.56	15.87
5.78	270.17	.134E+07	1525.1	3.86	14.64
6.36	270.75	.162E+07	1971.1	4.12	13.71
6.94	271.33	.194E+07	2474.0	4.33	13.04
7.52	271.91	.228E+07	3065.0	4.55	12.42
8.10	272.49	.266E+07	3773.4	4.80	11.75
8.68	273.07	.306E+07	4566.3	5.05	11.17
9.26	273.65	.348E+07	5443.7	5.30	10.66
9.84	274.23	.393E+07	6389.6	5.51	10.25
10.42	274.81	.440E+07	7426.2	5.72	9.88
11.00	275.39	.490E+07	8490.1	5.86	9.63

<---- hydrograph ----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (0254) 403.00	2.85	8.00	23.73	0.61	0.93
OUTFLOW: ID= 1 (6256) 403.00	2.68	9.75	23.73	0.60	0.93


```

-----
| ADD HYD (8370) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0256): 145.79 1.643 8.00 33.07 |
| + ID2= 2 (6256): 403.00 2.683 9.75 23.73 |
|-----|
| ID = 3 (8370): 548.79 4.183 9.00 26.21 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0250) | Area (ha)= 192.88 Curve Number (CN)= 70.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30 |
|-----|
| U.H. Tp(hrs)= 1.22 |
|
| Unit Hyd Qpeak (cms)= 1.930 |
|
| PEAK FLOW (cms)= 2.099 (1) |
| TIME TO PEAK (hrs)= 8.000 |
| RUNOFF VOLUME (mm)= 35.932 |
| TOTAL RAINFALL (mm)= 88.500 |
| RUNOFF COEFFICIENT = 0.406 |

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0246) | Area (ha)= 759.61 Curve Number (CN)= 55.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30 |
|-----|
| U.H. Tp(hrs)= 1.81 |
|
| Unit Hyd Qpeak (cms)= 5.121 |
|
| PEAK FLOW (cms)= 3.958 (1) |
| TIME TO PEAK (hrs)= 10.000 |
| RUNOFF VOLUME (mm)= 23.809 |
| TOTAL RAINFALL (mm)= 88.500 |
| RUNOFF COEFFICIENT = 0.269 |

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0248) | Area (ha)= 146.04 Curve Number (CN)= 64.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30 |
|-----|
| U.H. Tp(hrs)= 0.78 |
|
| Unit Hyd Qpeak (cms)= 2.271 |
|
| PEAK FLOW (cms)= 1.874 (1) |
| TIME TO PEAK (hrs)= 7.250 |
| RUNOFF VOLUME (mm)= 30.328 |
| TOTAL RAINFALL (mm)= 88.500 |
| RUNOFF COEFFICIENT = 0.343 |

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8364) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0246): 759.61 3.958 10.00 23.81 |
| + ID2= 2 (0248): 146.04 1.874 7.25 30.33 |
|-----|
| ID = 3 (8364): 905.65 5.601 8.25 24.86 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6250) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00 |

```

<----- DATA FOR SECTION (2501.0) ----->

Distance	Elevation	Manning
0.00	269.96	0.0500
8.56	268.55	0.0500
17.13	266.91	0.0500
21.41	266.13	0.0500
40.68	263.15	0.0500
62.09	260.75	0.0500
85.64	258.02	0.0500
88.20	257.69	0.0500
93.20	257.05	0.0500 /0.0350 Main Channel
93.45	256.88	0.0350 Main Channel
94.20	256.56	0.0350 Main Channel

Distance	Elevation	Manning
94.95	256.83	0.0350 Main Channel
95.20	257.08	0.0350 /0.0500 Main Channel
100.62	257.45	0.0500
115.61	258.57	0.0500
139.16	260.43	0.0500
152.01	261.95	0.0500
171.27	264.63	0.0500
188.40	267.90	0.0500
211.95	274.18	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.49	257.05	.205E+04	0.4	0.68	89.65
1.17	257.73	.248E+05	5.7	0.85	72.16
1.85	258.41	.841E+05	25.2	1.10	55.60
2.53	259.09	.180E+06	66.5	1.36	45.14
3.21	259.77	.312E+06	135.7	1.60	38.36
3.89	260.45	.481E+06	238.3	1.82	33.61
4.57	261.13	.681E+06	383.5	2.07	29.60
5.25	261.81	.911E+06	568.9	2.29	26.70
5.93	262.49	.117E+07	800.5	2.51	24.36
6.61	263.17	.146E+07	1079.0	2.72	22.49
7.28	263.84	.177E+07	1413.4	2.94	20.85
7.96	264.52	.210E+07	1797.0	3.14	19.50
8.64	265.20	.246E+07	2241.6	3.35	18.28
9.32	265.88	.283E+07	2739.9	3.55	17.25
10.00	266.56	.323E+07	3295.7	3.75	16.34
10.68	267.24	.364E+07	3908.9	3.94	15.54
11.36	267.92	.408E+07	4578.5	4.12	14.84
12.04	268.60	.453E+07	5320.1	4.32	14.18
12.72	269.28	.499E+07	6109.1	4.50	13.61

<---- hydrograph ----> <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (8364)	905.65	5.60	8.25	24.86	1.15	0.84
OUTFLOW: ID= 1 (6250)	905.65	5.25	10.25	24.86	1.11	0.83

```

-----
| ADD HYD (8366) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0250): 192.88 2.099 8.00 35.93 |
| + ID2= 2 (6250): 905.65 5.247 10.25 24.86 |
|-----|
| ID = 3 (8366): 1098.53 7.167 9.75 26.80 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8368) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (8366): 1098.53 7.167 9.75 26.80 |
| + ID2= 2 (8370): 548.79 4.183 9.00 26.21 |
|-----|
| ID = 3 (8368): 1647.32 11.318 9.50 26.61 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8372) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0252): 319.99 4.280 8.00 38.87 |
| + ID2= 2 (8368): 1647.32 11.318 9.50 26.61 |
|-----|
| ID = 3 (8372): 1967.31 15.243 9.00 28.60 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6258) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00 |

```

<----- DATA FOR SECTION (2581.0) ----->

Distance	Elevation	Manning
0.00	252.88	0.0500
15.47	251.89	0.0500
46.41	248.45	0.0500
73.48	245.81	0.0500
88.95	243.75	0.0500
112.15	242.00	0.0500
135.35	240.23	0.0500
162.42	239.76	0.0500
170.97	239.52	0.0500 /0.0350 Main Channel

171.58	239.03	0.0350	Main Channel
174.02	239.03	0.0350	Main Channel
176.46	239.03	0.0350	Main Channel
177.07	239.52	0.0350 / 0.0500	Main Channel
185.63	239.67	0.0500	
208.83	239.87	0.0500	
239.77	240.14	0.0500	
274.57	244.93	0.0500	
336.45	249.51	0.0500	
363.52	249.77	0.0500	
382.85	251.78	0.0500	

TRAVEL TIME TABLE					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.49	239.52	.138E+05	2.5	0.92	91.32
1.14	240.17	.175E+06	25.6	0.73	114.22
1.78	240.81	.532E+06	123.5	1.17	71.85
2.43	241.46	.932E+06	285.3	1.54	54.48
3.07	242.10	1.38E+07	505.5	1.85	45.25
3.72	242.75	1.86E+07	786.8	2.12	39.42
4.36	243.39	2.39E+07	1127.3	2.37	35.33
5.01	244.04	2.96E+07	1535.8	2.61	32.12
5.65	244.68	3.58E+07	2014.3	2.84	29.48
6.30	245.33	4.20E+07	2535.9	3.03	27.60
6.94	245.97	4.88E+07	3108.4	3.20	26.16
7.59	246.62	5.61E+07	3740.6	3.35	24.98
8.23	247.26	6.38E+07	4446.7	3.50	23.93
8.88	247.91	7.21E+07	5229.7	3.64	22.98
9.52	248.55	8.09E+07	6094.3	3.78	22.12
10.17	249.20	9.01E+07	7050.4	3.93	21.31
10.81	249.84	1.00E+08	7773.0	3.90	21.46
11.46	250.49	1.11E+08	8998.8	4.07	20.56
12.10	251.13	1.22E+08	10318.2	4.24	19.75

<---- hydrograph ----> <-pipe / channel->						
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (8372)	1967.31	15.24	9.00	28.60	0.85	0.81
OUTFLOW : ID= 1 (6258)	1967.31	13.61	11.25	28.60	0.80	0.82

ADD HYD (8374)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0258)	181.99	2.620	8.00	45.75	
+ ID2= 2 (6258)	1967.31	13.612	11.25	28.60	
ID = 3 (8374)	2149.30	15.662	11.00	30.05	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8376)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0260)	476.24	6.777	8.00	49.68	
+ ID2= 2 (8374)	2149.30	15.662	11.00	30.05	
ID = 3 (8376)	2625.54	21.578	10.25	33.61	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8378)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0262)	341.31	5.995	7.50	49.52	
+ ID2= 2 (8376)	2625.54	21.578	10.25	33.61	
ID = 3 (8378)	2966.85	26.472	10.00	35.44	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8362)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (8378)	2966.85	26.472	10.00	35.44	
+ ID2= 2 (8384)	1321.42	14.172	8.75	36.66	
ID = 3 (8362)	4288.27	40.199	9.25	35.82	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6272) |
IN= 2--> OUT= 1 | Routing time step (min)= 15.00

DATA FOR SECTION (2721.0)			
Distance	Elevation	Manning	
0.00	231.01	0.0450	
23.01	223.65	0.0450	
34.51	222.46	0.0450	
51.77	222.11	0.0450	
69.02	221.87	0.0450	
161.06	221.92	0.0450	
166.81	221.91	0.0450	
172.56	221.89	0.0450	
180.57	221.40	0.0450 / 0.0300	Main Channel
181.57	220.60	0.0300	Main Channel
184.07	220.16	0.0300	Main Channel
195.57	221.85	0.0300 / 0.0450	Main Channel
201.32	221.82	0.0450	
207.07	221.77	0.0450	
212.83	221.72	0.0450	
253.09	222.52	0.0450	
408.40	222.65	0.0450	
460.17	223.20	0.0450	
517.69	224.84	0.0450	
569.46	232.57	0.0450	

TRAVEL TIME TABLE					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.41	220.57	.189E+04	0.5	0.49	60.18
0.83	220.99	.697E+04	3.2	0.82	35.79
1.24	221.40	1.45E+05	8.8	1.07	27.45
1.84	222.00	6.43E+05	29.3	0.81	36.61
2.44	222.60	2.82E+06	132.9	0.83	35.41
3.04	223.20	7.07E+06	413.7	1.04	28.50
3.64	223.80	1.18E+07	876.8	1.31	22.46
4.24	224.40	1.68E+07	1494.8	1.57	18.75
4.84	225.00	2.21E+07	2261.1	1.82	16.23
5.44	225.60	2.74E+07	3196.3	2.06	14.29
6.04	226.20	3.28E+07	4258.6	2.30	12.84
6.65	226.81	3.83E+07	5444.7	2.52	11.72
7.25	227.41	4.38E+07	6749.7	2.73	10.82
7.85	228.01	4.94E+07	8169.4	2.93	10.08
8.45	228.61	5.51E+07	9700.7	3.12	9.46
9.05	229.21	6.08E+07	11340.8	3.30	8.94
9.65	229.81	6.66E+07	13087.4	3.48	8.48
10.25	230.41	7.24E+07	14938.5	3.65	8.08
10.85	231.01	7.83E+07	16892.4	3.82	7.73

<---- hydrograph ----> <-pipe / channel->						
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (8362)	4288.27	40.20	9.25	35.82	1.90	0.81
OUTFLOW : ID= 1 (6272)	4288.27	39.68	10.00	35.82	1.90	0.81

ADD HYD (8356)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0272)	157.38	2.876	7.50	41.09	
+ ID2= 2 (6272)	4288.27	39.678	10.00	35.82	
ID = 3 (8356)	4445.65	41.748	10.00	36.00	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8352)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (8354)	1268.73	24.661	7.75	46.69	
+ ID2= 2 (8356)	4445.65	41.748	10.00	36.00	
ID = 3 (8352)	5714.38	62.779	9.00	38.38	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6304) |
IN= 2--> OUT= 1 | Routing time step (min)= 15.00

DATA FOR SECTION (3041.0)		
Distance	Elevation	Manning
0.00	232.08	0.0500
19.00	231.87	0.0500
38.00	231.33	0.0500
66.51	230.44	0.0500
104.51	228.25	0.0500

118.76	225.17	0.0500	
128.26	219.86	0.0500	
175.77	219.17	0.0500	
185.27	218.90	0.0500 / 0.0300	Main Channel
185.52	218.65	0.0300	Main Channel
190.02	218.37	0.0300	Main Channel
194.52	218.60	0.0300	Main Channel
194.77	218.85	0.0300 / 0.0500	Main Channel
204.27	219.60	0.0500	
299.28	220.91	0.0500	
327.78	222.36	0.0500	
375.28	225.71	0.0500	
403.79	229.37	0.0500	
432.29	230.43	0.0500	
470.29	232.00	0.0500	

----->>> TRAVEL TIME TABLE <<<-----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.48	218.85	.121E+05	1.2	0.36	173.94
1.17	219.54	.839E+05	9.8	0.44	142.60
1.86	220.23	.323E+06	41.0	0.47	131.43
2.56	220.93	.708E+06	109.5	0.58	107.67
3.25	221.62	.118E+07	224.3	0.71	87.36
3.94	222.31	.168E+07	376.5	0.84	74.48
4.63	223.00	.222E+07	567.9	0.96	65.26
5.32	223.69	.279E+07	794.7	1.07	58.58
6.02	224.39	.339E+07	1056.2	1.17	53.52
6.71	225.08	.402E+07	1352.4	1.26	49.52
7.40	225.77	.468E+07	1677.3	1.34	46.46
8.09	226.46	.536E+07	2050.0	1.43	43.58
8.79	227.16	.607E+07	2457.9	1.52	41.14
9.48	227.85	.680E+07	2901.0	1.60	39.05
10.17	228.54	.755E+07	3352.7	1.66	37.53
10.86	229.23	.834E+07	3812.5	1.71	36.48
11.55	229.92	.919E+07	4241.8	1.73	36.11
12.25	230.62	.101E+08	4697.0	1.74	35.93
12.94	231.31	.111E+08	5186.3	1.74	35.83

----->>> hydrograph <<<----- <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8352) 5714.38	62.78	9.00	38.38	2.08	0.50
OUTFLOW : ID= 1 (6304) 5714.38	52.13	11.25	38.38	1.98	0.49

----->>> ADD HYD (8350) <<<-----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0304):	292.37	1.844	10.25	29.90
+ ID2= 2 (6304):	5714.38	52.127	11.25	38.38
ID = 3 (8350):	6006.75	53.944	11.25	37.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----->>> ADD HYD (8346) <<<-----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8348):	3768.46	40.043	9.50	35.26
+ ID2= 2 (8350):	6006.75	53.944	11.25	37.96
ID = 3 (8346):	9775.21	91.832	10.50	36.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----->>> ADD HYD (8344) <<<-----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0306):	283.97	0.637	13.75	21.91
+ ID2= 2 (8346):	9775.21	91.832	10.50	36.92
ID = 3 (8344):	10059.18	92.399	10.50	36.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----->>> RESERVOIR (5509) <<<-----

DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	*****	370.0451	
41.0590	56.7403	*****	863.4386	
48.1390	86.3439	*****	*****	
56.6340	*****	*****	*****	

67.9600 ***** | 0.0000 0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8344) *****	92.399	10.50	36.50
OUTFLOW : ID= 1 (5509) *****	55.794	15.75	36.50

PEAK FLOW REDUCTION [Qout/Qin](%)= 60.38
TIME SHIFT OF PEAK FLOW (min)=315.00
MAXIMUM STORAGE USED (ha.m.)=119.7052

----->>> CALIB (0332) <<<-----

Area (ha)	Ia (mm)	U.H. Tp(hrs)	Curve Number (CN)	# of Linear Res.(N)
NASHYD (0332)	393.44	5.00	75.0	1.50
ID= 1 DT=15.0 min	5.00	2.32		

Unit Hyd Qpeak (cms)= 2.894

PEAK FLOW (cms)= 3.974 (1)
TIME TO PEAK (hrs)= 9.500
RUNOFF VOLUME (mm)= 41.340
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.467

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----->>> CALIB (0330) <<<-----

Area (ha)	Ia (mm)	U.H. Tp(hrs)	Curve Number (CN)	# of Linear Res.(N)
NASHYD (0330)	468.30	5.00	80.0	1.50
ID= 1 DT=15.0 min	5.00	1.09		

Unit Hyd Qpeak (cms)= 7.335

PEAK FLOW (cms)= 9.926 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 47.004
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.531

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----->>> ROUTE CHN (6332) <<<-----

In= 2-->	Out= 1	Routing time step (min)*= 15.00
----------	--------	---------------------------------

----->>> DATA FOR SECTION (3321.0) <<<-----

Distance	Elevation	Manning
0.00	234.00	0.0380
25.85	227.20	0.0380
96.94	226.44	0.0380
168.03	227.38	0.0380
219.73	225.62	0.0380
342.52	221.57	0.0380
368.37	221.42	0.0380
374.83	221.23	0.0380
379.79	220.98	0.0380 / 0.0300
380.29	220.47	0.0300
381.29	220.47	0.0300
382.79	220.47	0.0300
383.29	220.98	0.0300 / 0.0380
394.22	221.22	0.0380
400.68	221.33	0.0380
407.14	221.44	0.0380
491.16	225.70	0.0380
568.71	227.55	0.0380
607.49	230.14	0.0380
639.80	234.08	0.0380

----->>> TRAVEL TIME TABLE <<<-----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.51	220.98	.554E+04	0.9	0.56	107.27
1.20	221.67	.885E+05	12.1	0.49	122.32
1.88	222.35	.309E+06	64.7	0.76	79.70
2.57	223.04	.615E+06	165.4	0.97	61.99
3.25	223.72	.101E+07	322.1	1.16	52.06
3.94	224.41	.148E+07	542.9	1.33	45.51
4.62	225.09	.204E+07	835.4	1.48	40.77
5.31	225.78	.269E+07	1202.8	1.62	37.27
5.99	226.46	.344E+07	1618.0	1.70	35.46
6.68	227.15	.447E+07	1932.0	1.56	38.56
7.36	227.83	.580E+07	2763.9	1.72	34.96
8.05	228.52	.718E+07	3861.9	1.95	30.97
8.73	229.20	.859E+07	5115.4	2.16	27.98
9.42	229.89	.100E+08	6519.2	2.35	25.64
10.10	230.57	.115E+08	8092.3	2.55	23.69
10.79	231.26	.130E+08	9921.5	2.73	22.05
11.47	231.94	.145E+08	11690.0	2.92	20.68
12.16	232.63	.160E+08	13694.9	3.09	19.52
12.84	233.31	.176E+08	15833.9	3.26	18.52

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (0330) 468.30 9.93 7.25 47.00 1.06 0.50
OUTFLOW: ID= 1 (6332) 468.30 7.46 9.50 47.00 0.91 0.52

```

```

-----
| ADD HYD (8322) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID= 1 (0332): 393.44 3.974 9.50 41.34 |
| + ID= 2 (6332): 468.30 7.455 9.50 47.00 |
|=====|
| ID = 3 (8322): 861.74 11.429 9.50 44.42 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB (0328) | Area (ha)= 492.92 Curve Number (CN)= 77.0
| NASHYD (0328) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.91
|-----|
Unit Hyd Qpeak (cms)= 4.411
PEAK FLOW (cms)= 6.150 (i)
TIME TO PEAK (hrs)= 8.750
RUNOFF VOLUME (mm)= 43.578
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.492

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0326) | Area (ha)= 678.91 Curve Number (CN)= 80.0
| NASHYD (0326) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.95
|-----|
Unit Hyd Qpeak (cms)= 5.941

```

```

PEAK FLOW (cms)= 9.054 (i)
TIME TO PEAK (hrs)= 8.750
RUNOFF VOLUME (mm)= 47.251
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.534

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0324) | Area (ha)= 615.64 Curve Number (CN)= 79.0
| NASHYD (0324) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.90
|-----|
Unit Hyd Qpeak (cms)= 5.544

```

```

PEAK FLOW (cms)= 8.170 (i)
TIME TO PEAK (hrs)= 8.500
RUNOFF VOLUME (mm)= 45.986
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.520

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0320) | Area (ha)= 278.74 Curve Number (CN)= 81.0
| NASHYD (0320) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.36
|-----|
Unit Hyd Qpeak (cms)= 3.499

```

```

PEAK FLOW (cms)= 5.113 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 48.415
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.547

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (3182) | Area (ha)= 457.40 Curve Number (CN)= 78.0
| NASHYD (3182) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.49
|-----|

```

Unit Hyd Qpeak (cms)= 5.259

```

PEAK FLOW (cms)= 7.201 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 44.687
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.505

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (3181) | Area (ha)= 26.00
| STANDHYD (3181) | Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00
| ID= 1 DT=15.0 min |
|-----|

```

```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 21.06 4.94
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 416.33 40.00
Mannings n = 0.013 0.250

```

```

Max.Eff.Inten.(mm/hr)= 116.82 67.96
over (min) 15.00 30.00
Storage Coeff. (min)= 6.96 (ii) 19.44 (iii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.10 0.05

```

TOTALS

```

PEAK FLOW (cms)= 6.31 0.48 6.617 (iiii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 86.50 43.75 78.38
TOTAL RAINFALL (mm)= 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.49 0.89

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

```

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 77.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

```

-----
| RESERVOIR (5318) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
|-----|
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
0.0000 0.0000 | 3.3800 0.7654
0.0510 0.4423 | 4.1500 0.8231
0.8700 0.5378 | 4.8750 0.8778
1.9590 0.6105 | 5.2750 0.8878
2.5620 0.6796 | 0.0000 0.0000

```

```

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (3181) 26.000 6.617 6.00 78.38
OUTFLOW: ID= 1 (5318) 26.000 3.934 6.25 78.29

```

```

PEAK FLOW REDUCTION [Qout/Qin](%)= 59.45
TIME SHIFT OF PEAK FLOW (min)= 15.00
MAXIMUM STORAGE USED (ha.m.)= 0.8924

```

```

-----
| ADD HYD (8334) |
| 1 + 2 = 3 |
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID= 1 (3182): 457.40 7.201 8.00 44.69 |
| + ID= 2 (5318): 26.00 3.934 6.25 78.29 |
|=====|
| ID = 3 (8334): 483.40 8.715 6.25 46.49 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CN (6320) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
|-----|

```

```

<----- DATA FOR SECTION (3201.0) ----->
Distance Elevation Manning
0.00 249.00 0.0380
22.92 245.86 0.0380
45.83 244.87 0.0380
91.66 243.11 0.0380
126.03 239.53 0.0380
160.41 237.17 0.0380
166.14 237.06 0.0380
177.59 237.13 0.0380
183.32 237.20 0.0380
189.05 236.70 0.0380 /0.0350 Main Channel

```

193.05	235.89	0.0350	Main Channel
197.05	236.64	0.0350 / 0.0380	Main Channel
200.51	236.74	0.0380	
206.24	237.03	0.0380	
246.34	238.82	0.0380	
263.53	243.87	0.0380	
389.56	247.64	0.0380	
452.58	247.74	0.0380	
498.41	248.60	0.0380	
567.16	249.84	0.0380	

530.58	242.08	0.0350
586.43	242.93	0.0350
691.16	248.00	0.0350

TRAVEL TIME TABLE						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	(min)
0.37	235.62	.215E+04	0.7	0.77	53.88	
0.75	236.00	.634E+04	3.5	1.36	30.45	
1.46	236.71	.297E+05	19.3	1.62	25.66	
2.16	237.41	.103E+06	75.8	1.84	22.60	
2.87	238.12	.229E+06	202.8	2.21	18.79	
3.57	238.82	.407E+06	423.3	2.59	16.03	
4.28	239.53	.654E+06	727.7	2.77	14.97	
4.99	240.24	.103E+07	1242.0	3.01	13.76	
5.69	240.94	.151E+07	2002.3	3.31	12.54	
6.40	241.65	.230E+07	2987.9	3.55	11.70	
7.10	242.35	.286E+07	4302.8	3.75	11.07	
7.81	243.06	.374E+07	6165.3	4.10	10.11	
8.52	243.76	.468E+07	8642.4	4.60	9.03	
9.22	244.47	.565E+07	11496.3	5.06	8.00	
9.93	245.18	.666E+07	14714.1	5.50	7.54	
10.63	245.88	.770E+07	18290.3	5.91	7.02	
11.34	246.59	.877E+07	22221.9	6.30	6.58	
12.04	247.29	.988E+07	26464.4	6.67	6.22	
12.75	248.00	.110E+08	31061.3	7.01	5.91	

TRAVEL TIME TABLE						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	(min)
0.37	236.26	.270E+04	0.4	0.58	107.96	
0.75	236.64	.108E+05	2.6	0.92	68.01	
1.48	237.37	.497E+05	27.4	1.17	53.35	
2.20	238.09	.277E+06	122.1	1.65	37.83	
2.93	238.82	.540E+06	299.7	2.08	30.01	
3.66	239.55	.857E+06	587.8	2.57	24.29	
4.39	240.28	.120E+07	977.8	3.04	20.52	
5.11	241.00	.158E+07	1458.1	3.46	18.02	
5.84	241.73	.198E+07	2028.6	3.84	16.23	
6.57	242.46	.240E+07	2690.5	4.19	14.86	
7.29	243.18	.285E+07	3431.8	4.50	13.84	
8.02	243.91	.335E+07	4149.1	4.64	13.44	
8.75	244.64	.393E+07	4746.6	4.51	13.80	
9.47	245.36	.463E+07	5618.6	4.53	13.75	
10.20	246.09	.545E+07	6781.8	4.66	13.39	
10.93	246.82	.635E+07	8274.5	4.87	12.79	
11.66	247.55	.733E+07	9980.0	5.10	12.23	
12.38	248.27	.853E+07	11823.0	5.01	12.45	
13.11	249.00	.989E+07	13749.7	5.20	11.98	

hydrograph						
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	(m/s)
INFLW : ID= 2 (8334)	483.40	8.72	6.25	46.49	0.93	0.97
OUTFLOW: ID= 1 (6320)	483.40	7.07	9.00	46.49	0.88	0.95

hydrograph						
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	(m/s)
INFLW : ID= 2 (8334)	483.40	8.72	6.25	46.49	0.93	0.97
OUTFLOW: ID= 1 (6320)	483.40	7.07	9.00	46.49	0.88	0.95

CALIB			
NASHYD (0316)	Area (ha)	Curve Number (CN)	82.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50	
U.H. Tp(hrs)= 1.01			

Unit Hyd Qpeak (cms) = 3.923
 PEAK FLOW (cms) = 5.548 (l)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 49.565
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.560

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8332)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)	(mm)
ID1= 1 (0320):	278.74	5.113	8.00	48.41
+ ID2= 2 (6320):	483.40	7.067	9.00	46.49
ID = 3 (8332):	762.14	11.950	8.50	47.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8338)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)	(mm)
ID1= 1 (0316):	232.34	5.548	7.25	49.57
+ ID2= 2 (6316):	165.20	4.376	7.25	44.18
ID = 3 (8338):	397.54	9.924	7.25	47.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB			
NASHYD (0314)	Area (ha)	Curve Number (CN)	78.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50	
U.H. Tp(hrs)= 0.71			

Unit Hyd Qpeak (cms) = 3.978

PEAK FLOW (cms) = 4.656 (l)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 44.178
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.499

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB			
NASHYD (0312)	Area (ha)	Curve Number (CN)	80.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50	
U.H. Tp(hrs)= 1.36			

Unit Hyd Qpeak (cms) = 4.529

PEAK FLOW (cms) = 6.425 (l)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 47.122
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.532

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6316)	
IN= 2-->	OUT= 1
Routing time step (min)= 15.00	

DATA FOR SECTION (3161.0)			
Distance	Elevation	Manning	
0.00	248.54	0.0350	
27.93	248.34	0.0350	
41.89	246.61	0.0350	
62.83	243.09	0.0350	
132.65	239.00	0.0350	
188.50	236.74	0.0350	
195.48	236.54	0.0350	
202.46	236.32	0.0350	
205.48	236.14	0.0350 / 0.0310	Main Channel
205.98	235.61	0.0310	Main Channel
207.98	235.25	0.0310	Main Channel
209.98	235.53	0.0310	Main Channel
210.48	236.00	0.0310 / 0.0350	Main Channel
216.42	236.73	0.0350	
258.31	239.09	0.0350	
328.12	239.84	0.0350	
439.83	241.47	0.0350	

CALIB			
NASHYD (0308)	Area (ha)	Curve Number (CN)	62.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50	
U.H. Tp(hrs)= 1.62			

Unit Hyd Qpeak (cms) = 5.575

PEAK FLOW (cms) = 4.939 (l)
 TIME TO PEAK (hrs) = 8.250
 RUNOFF VOLUME (mm) = 29.006
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.328

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0310) Area (ha)= 138.28 Curve Number (CN)= 65.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.76

Unit Hyd Opeak (cms)= 3.102

PEAK FLOW (cms)= 2.521 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 31.169
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.352

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8342)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
ID1= 1 (0308): 529.30 4.939 8.25 29.01
+ ID2= 2 (0312): 138.28 2.521 7.00 31.17
ID = 3 (8342): 667.58 7.113 7.75 29.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6312)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

Table with columns: Distance, Elevation, Manning. Rows show channel data points from 0.00 to 538.37.

TRAVEL TIME TABLE
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)

hydrograph
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
INFLOW : ID= 2 (8342) 667.58 7.11 7.75 29.45 0.56 1.85
OUTFLOW: ID= 1 (6312) 667.58 7.11 7.75 29.45 0.55 1.85

ADD HYD (8340)

1 + 2 = 3 AREA QPEAK TPEAK R.V.
ID1= 1 (0312): 359.44 6.425 8.00 47.12
+ ID2= 2 (6312): 667.58 7.109 7.75 29.45
ID = 3 (8340): 1027.02 13.531 8.00 35.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8336)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
ID1= 1 (8338): 397.54 9.924 7.25 47.33
+ ID2= 2 (8340): 1027.02 13.531 8.00 35.64
ID = 3 (8336): 1424.56 23.290 7.50 38.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8330)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
ID1= 1 (8332): 762.14 11.950 8.50 47.20
+ ID2= 2 (8336): 1424.56 23.290 7.50 38.90
ID = 3 (8330): 2186.70 34.722 7.75 41.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6324)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

Table with columns: Distance, Elevation, Manning. Rows show channel data points from 0.00 to 830.89.

TRAVEL TIME TABLE
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)

hydrograph
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
INFLOW : ID= 2 (8330) 2186.70 34.72 7.75 41.79 1.63 0.79
OUTFLOW: ID= 1 (6324) 2186.70 29.73 9.50 41.79 1.56 0.78

ADD HYD (8328)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8324):	615.64	8.170	8.50	45.99
+ ID2= 2 (8324):	2186.70	29.727	9.50	41.79
=====				
ID = 3 (8328):	2802.34	37.743	9.25	42.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	Curve Number (CN)= 80.0
NASHYD (0322)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min	U.H. Tp(hrs)=	1.75	

Unit Hyd Qpeak (cms)= 5.020

PEAK FLOW (cms)= 7.498 (1)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 47.219
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.534

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8326)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0322):	513.13	7.498	8.25	47.22
+ ID2= 2 (8328):	2802.34	37.743	9.25	42.71
=====				
ID = 3 (8326):	3315.47	45.039	9.25	43.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8324)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0326):	678.91	9.054	8.75	47.25
+ ID2= 2 (8326):	3315.47	45.039	9.25	43.41
=====				
ID = 3 (8324):	3994.38	54.016	9.25	44.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6328)	Routing time step (min)= 15.00
------------------	--------------------------------

<----- DATA FOR SECTION (3281.0) ----->

Distance	Elevation	Manning	
0.00	228.00	0.0380	
18.58	224.97	0.0380	
78.98	223.52	0.0380	
125.44	223.28	0.0380	
171.90	221.71	0.0380	
213.72	219.65	0.0380	
218.36	219.40	0.0380	
223.01	219.19	0.0380	
225.95	219.14	0.0380 / 0.0320	Main Channel
226.45	218.14	0.0320	Main Channel
236.95	218.14	0.0320	Main Channel
245.95	218.14	0.0320	Main Channel
245.95	219.14	0.0320 / 0.0380	Main Channel
246.24	219.16	0.0380	
250.88	219.24	0.0380	
255.53	219.39	0.0380	
325.22	221.47	0.0380	
367.03	223.14	0.0380	
404.20	225.17	0.0380	
459.95	228.04	0.0380	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELRV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	218.64	.373E+05	3.2	0.33	192.37
1.00	219.14	.752E+05	10.1	0.51	124.71
1.52	219.66	.149E+06	22.2	0.57	112.06
2.04	220.18	.279E+06	43.4	0.59	107.06
2.56	220.70	.464E+06	77.0	0.63	100.52
3.08	221.22	.705E+06	125.7	0.68	93.96
3.61	221.75	.100E+07	192.7	0.73	86.62
4.13	222.27	.135E+07	278.1	0.79	80.92
4.65	222.79	.176E+07	385.9	0.84	75.86
5.17	223.31	.222E+07	514.9	0.89	71.82

5.69	223.83	.280E+07	662.7	0.90	70.40
6.21	224.35	.345E+07	867.7	0.96	66.35
6.73	224.87	.417E+07	1107.0	1.01	62.81
7.25	225.39	.494E+07	1400.0	1.08	58.80
7.78	225.92	.573E+07	1732.2	1.15	55.17
8.30	226.44	.655E+07	2099.9	1.22	52.02
8.82	226.96	.740E+07	2502.8	1.29	49.29
9.34	227.48	.828E+07	2940.9	1.36	46.90
9.86	228.00	.918E+07	3414.3	1.42	44.79

<---- hydrograph ----> <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8324) 3994.38	54.02	9.25	44.06	2.21	0.61
OUTFLOW: ID= 1 (6328) 3994.38	46.11	11.00	44.06	2.08	0.60

ADD HYD (8320)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0328):	492.92	6.150	8.75	43.58
+ ID2= 2 (8328):	3994.38	46.106	11.00	44.06
=====				
ID = 3 (8320):	4487.30	51.569	11.00	44.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8318)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8320):	4487.30	51.569	11.00	44.01
+ ID2= 2 (8322):	861.74	11.429	9.50	44.42
=====				
ID = 3 (8318):	5349.04	62.499	10.75	44.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8316)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8509):	10059.38	55.794	15.75	36.50
+ ID2= 2 (8318):	5349.04	62.499	10.75	44.08
=====				
ID = 3 (8316):	15408.22	109.731	11.75	39.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8312)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8314):	620.10	2.136	6.25	25.79
+ ID2= 2 (8316):	15408.22	109.731	11.75	39.13
=====				
ID = 3 (8312):	16028.32	110.987	11.75	38.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8308)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8310):	598.90	44.535	6.25	47.72
+ ID2= 2 (8312):	16028.32	110.987	11.75	38.61
=====				
ID = 3 (8308):	16627.22	113.364	11.75	38.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5510)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1				
DT= 15.0 min	0.0000	0.0000	*****	74.0090
	66.5450	18.5023	*****	*****
	98.5430	37.0045	*****	*****

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8308) *****	113.364	11.75	38.94

OUTFLOW: ID= 1 (5510) ***** 105.320 13.75 38.94

PEAK FLOW REDUCTION [Qout/Qin](%)= 92.90
TIME SHIFT OF PEAK FLOW (min)=120.00
MAXIMUM STORAGE USED (ha.m.)= 44.9262

ADD HYD (8240)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0336): 2785.00 4.639 22.25 38.20
+ ID2= 2 (5510): 16627.22 105.320 13.75 38.94
ID = 3 (8240): 19412.22 109.428 14.00 38.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8238)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (5508): 9524.23 56.167 11.75 25.09
+ ID2= 2 (8240): 19412.22 109.428 14.00 38.84
ID = 3 (8238): 28936.45 163.719 13.00 34.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8236)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0344): 1004.58 5.230 10.00 23.19
+ ID2= 2 (8238): 28936.45 163.719 13.00 34.32
ID = 3 (8236): 29941.03 168.365 12.75 33.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8234)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8232): 285.80 10.972 6.50 32.52
+ ID2= 2 (8236): 29941.03 168.365 12.75 33.94
ID = 3 (8234): 30226.83 168.774 12.50 33.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8230)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0344): 344.00 4.771 7.00 26.48
+ ID2= 2 (8234): 30226.83 168.774 12.50 33.93
ID = 3 (8230): 30570.83 170.215 12.50 33.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8228)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8226): 1952.06 15.927 8.75 22.21
+ ID2= 2 (8230): 30570.83 170.215 12.50 33.84
ID = 3 (8228): 32522.89 180.663 12.00 33.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8190)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0356): 1015.22 6.685 8.00 18.15
+ ID2= 2 (8228): 32522.89 180.663 12.00 33.15
ID = 3 (8190): 33538.11 185.041 12.00 32.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM
Filename: C:\Users\jsscott\AppData\Local\Temp\3e280798-92ee-4282-809c-79f5caed0add\9255d239
Comments: 50-Year 12-Hour SCS II Design Storm

***** SIMULATION NUMBER: 11 *****

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	2.02	3.25	3.23	6.25	14.54	9.25	2.83
0.50	2.02	3.50	3.23	6.50	14.54	9.50	2.83
0.75	2.02	3.75	3.23	6.75	6.46	9.75	2.83
1.00	2.02	4.00	3.23	7.00	6.46	10.00	2.83
1.25	2.02	4.25	4.85	7.25	4.85	10.25	1.62
1.50	2.02	4.50	4.85	7.50	4.85	10.50	1.62
1.75	2.02	4.75	6.46	7.75	4.85	10.75	1.62
2.00	2.02	5.00	6.46	8.00	4.85	11.00	1.62
2.25	2.42	5.25	9.70	8.25	2.83	11.25	1.62
2.50	2.42	5.50	9.70	8.50	2.83	11.50	1.62
2.75	2.42	5.75	38.78	8.75	2.83	11.75	1.62
3.00	2.42	6.00	106.66	9.00	2.83	12.00	1.62

CALIB
NASHYD (0356) Area (ha)=1015.22 Curve Number (CN)= 46.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.37

Unit Hyd Qpeak (cms)= 12.651
PEAK FLOW (cms)= 5.609 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 15.266
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.189

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0354) Area (ha)= 262.68 Curve Number (CN)= 37.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.38

Unit Hyd Qpeak (cms)= 3.252
PEAK FLOW (cms)= 1.056 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 11.232
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.139

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0352) Area (ha)= 381.43 Curve Number (CN)= 54.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.82

Unit Hyd Qpeak (cms)= 7.980
PEAK FLOW (cms)= 4.018 (1)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 19.394
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.240

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0346) Area (ha)= 350.93 Curve Number (CN)= 70.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 6.254
PEAK FLOW (cms)= 5.278 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 30.777
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.381

EXISTING

WEST HOLLAND RIVER REGIONAL STORM (RF1000 - RF927)

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V V I SSSS U U A L
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
WV I SSSS UUUU A A LLLL

OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y M M O O
O O T T H H Y M O O
OOO T T H H Y M M OOO Company Serial
    
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***** DETAILED OUTPUT *****

Input filename: C:\Program Files\Visual Gchymo 2.4r\VO2\voin.dat
 Output filename: C:\Users\jscott\AppData\Local\Temp\96b8d007-9cc9-4be4-b3c8-096f6a228b82\Scenario.out
 Summary filename: C:\Users\jscott\AppData\Local\Temp\96b8d007-9cc9-4be4-b3c8-096f6a228b82\Scenario.sum

DATE: 08/22/2012 TIME: 01:35:05

USER:

COMMENTS: _____

** SIMULATION NUMBER: 1 **

READ STORM Filename: C:\Users\jscott\AppData\Local\Temp\96b8d007-9cc9-4be4-b3c8-096f6a228b82\487f093f
 Ptotal=212.00 mm Comments: 12-Hour Hurricane Hazel

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	6.00	3.25	13.00	6.25	23.00	9.25	53.00
0.50	6.00	3.50	13.00	6.50	23.00	9.50	53.00
0.75	6.00	3.75	13.00	6.75	23.00	9.75	53.00
1.00	6.00	4.00	13.00	7.00	23.00	10.00	53.00
1.25	4.00	4.25	17.00	7.25	13.00	10.25	38.00
1.50	4.00	4.50	17.00	7.50	13.00	10.50	38.00
1.75	4.00	4.75	17.00	7.75	13.00	10.75	38.00
2.00	4.00	5.00	17.00	8.00	13.00	11.00	38.00
2.25	6.00	5.25	13.00	8.25	13.00	11.25	13.00
2.50	6.00	5.50	13.00	8.50	13.00	11.50	13.00
2.75	6.00	5.75	13.00	8.75	13.00	11.75	13.00
3.00	6.00	6.00	13.00	9.00	13.00	12.00	13.00

CALIB (0356) Area (ha)=1015.22 Curve Number (CN)= 66.4
 NASHVD (0356) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.37

Unit Hyd Qpeak (cms)= 12.651

PEAK FLOW (cms)= 47.576 (i)
 TIME TO PEAK (hrs)= 11.750
 RUNOFF VOLUME (mm)= 126.889
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.599

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0354) Area (ha)= 262.68 Curve Number (CN)= 57.0
 NASHVD (0354) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.38

Unit Hyd Qpeak (cms)= 3.252

PEAK FLOW (cms)= 10.497 (i)
 TIME TO PEAK (hrs)= 12.000
 RUNOFF VOLUME (mm)= 106.815

TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.504

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0352) Area (ha)= 381.43 Curve Number (CN)= 72.9
 NASHVD (0352) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.82

Unit Hyd Qpeak (cms)= 7.980

PEAK FLOW (cms)= 26.072 (i)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 140.195
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.661

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0346) Area (ha)= 350.93 Curve Number (CN)= 85.4
 NASHVD (0346) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 6.254

PEAK FLOW (cms)= 25.363 (i)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 169.245
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.798

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0350) Area (ha)= 366.84 Curve Number (CN)= 69.3
 NASHVD (0350) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 5.831

PEAK FLOW (cms)= 20.648 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 132.873
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.627

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0348) Area (ha)= 590.18 Curve Number (CN)= 68.0
 NASHVD (0348) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.91

Unit Hyd Qpeak (cms)= 5.267

PEAK FLOW (cms)= 22.856 (i)
 TIME TO PEAK (hrs)= 12.000
 RUNOFF VOLUME (mm)= 130.716
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.617

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6350) Routing time step (min)= 15.00

<----- DATA FOR SECTION (3501.0) ----->

Distance	Elevation	Manning
0.00	287.00	0.0500
23.66	283.72	0.0500
70.98	280.94	0.0500
112.38	280.23	0.0500
171.52	276.80	0.0500
260.24	274.46	0.0500
266.16	274.26	0.0500
272.07	274.12	0.0500
276.49	274.12	0.0500 / 0.0300 Main Channel
276.99	273.82	0.0300 Main Channel
277.99	273.82	0.0300 Main Channel
279.49	273.82	0.0300 Main Channel
280.49	274.27	0.0300 / 0.0500 Main Channel
283.90	274.27	0.0500
289.81	274.57	0.0500

325.30	275.75	0.0500
396.28	278.98	0.0500
449.51	280.97	0.0500
496.83	283.90	0.0500
585.54	287.92	0.0500

```

----- TRAVEL TIME TABLE -----
DEPTH   ELEV   VOLUME   FLOW RATE   VELOCITY   TRAV.TIME
(m)      (m)      (cu.m.)   (cms)        (m/s)      (min)
0.30    274.12   .273E+04   0.9          0.99       49.81
0.98    274.80   .583E+05   20.8         1.05       46.71
1.66    275.48   .203E+06   95.2         1.38       35.53
2.33    276.15   .438E+06   256.0        1.73       28.50
3.01    276.83   .755E+06   524.8        2.05       23.97
3.69    277.51   .114E+07   935.8        2.43       20.26
4.37    278.19   .157E+07   1472.0       2.76       17.82
5.05    278.87   .206E+07   2140.7       3.06       16.06
5.72    279.54   .261E+07   2938.6       3.33       14.79
6.40    280.22   .321E+07   3887.6       3.57       13.77
7.08    280.90   .390E+07   4795.4       3.63       13.56
7.76    281.58   .468E+07   5716.2       3.91       12.58
8.43    282.25   .550E+07   6783.9       4.19       11.74
9.11    282.93   .637E+07   7901.9       4.45       11.05
9.79    283.61   .728E+07   9154.8       4.70       10.46
10.47   284.29   .823E+07   10445.3      4.96       9.91
11.15   284.97   .922E+07   11879.4      5.21       9.43
11.82   285.64   .103E+08   13452.0     5.45       9.02
12.50   286.32   .113E+08   15174.3     5.69       8.65

```

```

----- hydrograph ----- <-pipe / channel->
AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
(ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
INFLW : ID= 2 (0348) 590.18  22.86  12.00  130.72  1.00  1.06
OUTFLOW: ID= 1 (6350) 590.18  21.81  12.75  130.72  0.99  1.06

```

```

ADD HYD (8220)
1 + 2 = 3
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0350): 366.84  20.648  11.25  132.87
+ ID2= 2 (6350): 590.18  21.810  12.75  130.72
=====
ID = 3 (8220):  957.02  40.682  12.00  131.54

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8222)
1 + 2 = 3
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0346): 350.93  25.363  11.00  169.25
+ ID2= 2 (8220): 957.02  40.682  12.00  131.54
=====
ID = 3 (8222): 1307.95  64.525  11.75  141.66

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6352)
IN= 2----> OUT= 1 | Routing time step (min)'= 15.00

```

```

----- DATA FOR SECTION (3521.0) -----
Distance  Elevation  Manning
0.00      257.95     0.0500
7.83      257.34     0.0500
15.66     256.19     0.0500
21.53     254.01     0.0500
41.11     244.06     0.0500
76.35     241.38     0.0500
111.58    239.74     0.0500
113.54    239.61     0.0500
113.75    239.41     0.0500 / 0.0300 Main Channel
113.85    238.81     0.0300 Main Channel
115.50    238.81     0.0300 Main Channel
116.15    238.81     0.0300 Main Channel
116.25    239.41     0.0300 Main Channel
117.46    239.52     0.0500
119.41    239.72     0.0500
121.37    240.04     0.0500
131.16    241.84     0.0500
156.61    247.03     0.0500
176.19    251.46     0.0500
193.80    258.79     0.0500

```

```

----- TRAVEL TIME TABLE -----
DEPTH   ELEV   VOLUME   FLOW RATE   VELOCITY   TRAV.TIME
(m)      (m)      (cu.m.)   (cms)        (m/s)      (min)
0.60    239.41   .224E+04   2.2          1.52       17.12
1.58    240.39   .216E+05   22.3         1.61       16.16

```

2.55	241.36	.804E+05	101.0	1.96	13.27
3.53	242.34	.173E+06	275.6	2.48	10.46
4.50	243.31	.293E+06	553.9	2.95	8.80
5.48	244.29	.438E+06	961.8	3.42	7.60
6.45	245.26	.599E+06	1534.4	3.99	6.51
7.43	246.24	.770E+06	2228.4	4.51	5.76
8.41	247.22	.951E+06	3043.6	4.98	5.21
9.38	248.19	.114E+07	3982.1	5.43	4.78
10.36	249.17	.134E+07	5040.1	5.85	4.44
11.33	250.14	.155E+07	6218.5	6.24	4.16
12.31	251.12	.177E+07	7518.6	6.61	3.93
13.29	252.10	.200E+07	8969.0	6.98	3.72
14.26	253.07	.224E+07	10554.1	7.35	3.53
15.24	254.05	.248E+07	12257.1	7.71	3.37
16.21	255.02	.272E+07	14045.1	8.03	3.23
17.19	256.00	.298E+07	15954.8	8.34	3.11
18.16	256.97	.325E+07	17980.4	8.53	3.04

```

----- hydrograph ----- <-pipe / channel->
AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
(ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
INFLW : ID= 2 (8224) 1307.95  64.52  11.75  141.66  2.10  1.78
OUTFLOW: ID= 1 (6352) 1307.95  64.34  12.00  141.66  2.10  1.78

```

```

ADD HYD (8224)
1 + 2 = 3
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0352): 381.43  26.072  11.00  140.20
+ ID2= 2 (6352): 1307.95  64.338  12.00  141.66
=====
ID = 3 (8224): 1689.38  88.700  11.50  141.33

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6354)
IN= 2----> OUT= 1 | Routing time step (min)'= 15.00

```

```

----- DATA FOR SECTION (3541.0) -----
Distance  Elevation  Manning
0.00      253.92     0.0500
7.95      251.83     0.0500
15.89     249.97     0.0500
19.87     249.05     0.0500
47.68     242.00     0.0500
67.55     237.10     0.0500
83.44     231.11     0.0500
85.43     230.75     0.0500
85.66     230.61     0.0500 / 0.0300 Main Channel
85.76     230.01     0.0300 Main Channel
87.41     230.01     0.0300 Main Channel
88.06     230.01     0.0300 Main Channel
88.16     230.61     0.0300 / 0.0500 Main Channel
89.40     230.72     0.0500
91.39     230.88     0.0500
93.37     231.03     0.0500
133.11    233.96     0.0500
150.99    235.39     0.0500
172.84    239.03     0.0500
196.68    250.00     0.0500

```

```

----- TRAVEL TIME TABLE -----
DEPTH   ELEV   VOLUME   FLOW RATE   VELOCITY   TRAV.TIME
(m)      (m)      (cu.m.)   (cms)        (m/s)      (min)
0.60    230.61   .451E+04   1.5          1.06       49.40
1.62    231.63   .394E+05   14.6         1.16       45.13
2.64    232.65   .128E+06   58.8         1.44       36.30
3.66    233.67   .269E+06   149.7        1.74       30.00
4.68    234.69   .463E+06   301.5        2.04       25.58
5.70    235.71   .705E+06   530.5        2.36       22.14
6.72    236.73   .980E+06   848.4        2.71       19.25
7.74    237.75   .128E+07   1237.9       3.02       17.29
8.76    238.77   .162E+07   1706.7       3.30       15.83
9.78    239.79   .199E+07   2291.9       3.61       14.45
10.81   240.82   .237E+07   2973.7       3.92       13.30
11.83   241.84   .278E+07   3742.2       4.21       12.38
12.85   242.86   .321E+07   4599.1       4.49       11.62
13.87   243.88   .365E+07   5544.6       4.75       10.98
14.89   244.90   .412E+07   6579.7       5.00       10.44
15.91   245.92   .461E+07   7705.9       5.23       9.97
16.93   246.94   .512E+07   8924.6       5.46       9.55
17.95   247.96   .564E+07   10237.5      5.68       9.19
18.97   248.98   .619E+07   11646.2      5.89       8.86

```

```

----- hydrograph ----- <-pipe / channel->
AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
(ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
INFLW : ID= 2 (8224) 1689.38  88.70  11.50  141.33  2.98  1.52
OUTFLOW: ID= 1 (6354) 1689.38  85.97  12.25  141.33  2.95  1.52

```

THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8226) |
| 1 + 2 = 3 |
-----
| ID1= 1 (0354): 262.68 10.497 12.00 106.81
+ ID2= 2 (6354): 168.38 85.967 12.25 141.33
=====
| ID = 3 (8226): 1952.06 96.397 12.00 136.68
-----
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0344) | Area (ha)= 344.00 Curve Number (CN)= 77.3
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.87
-----
  
```

Unit Hyd Qpeak (cms)= 6.790

```

PEAK FLOW (cms)= 24.075 (i)
TIME TO PEAK (hrs)= 11.000
RUNOFF VOLUME (mm)= 150.243
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.709
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANBYD (3402) | Area (ha)= 234.60
| ID= 1 DT=15.0 min | Total Imp(%)= 28.00 Dir. Conn.(%)= 13.00
-----
  
```

```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 65.69 168.91
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1250.60 40.00
Mannings n = 0.013 0.250
  
```

```

Max.Eff.Inten.(mm/hr)= 53.00 54.11
over (min)= 15.00 45.00
Storage Coeff. (min)= 18.46 (ii) 32.14 (ii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.06 0.03
  
```

TOTALS

```

PEAK FLOW (cms)= 4.36 20.95 24.397 (iii)
TIME TO PEAK (hrs)= 10.00 10.50 10.50
RUNOFF VOLUME (mm)= 210.00 141.46 150.37
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.67 0.71
  
```

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 68.3 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANBYD (3401) | Area (ha)= 51.20
| ID= 1 DT=15.0 min | Total Imp(%)= 46.00 Dir. Conn.(%)= 21.00
-----
  
```

```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 23.55 27.65
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 584.24 40.00
Mannings n = 0.013 0.250
  
```

```

Max.Eff.Inten.(mm/hr)= 53.00 68.14
over (min)= 15.00 30.00
Storage Coeff. (min)= 11.69 (ii) 24.16 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04
  
```

TOTALS

```

PEAK FLOW (cms)= 1.58 4.63 6.187 (iii)
TIME TO PEAK (hrs)= 10.00 10.25 10.00
RUNOFF VOLUME (mm)= 210.00 150.46 162.96
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.71 0.77
  
```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 68.3 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

```

-----
| ADD HYD (8232) |
| 1 + 2 = 3 |
-----
| ID1= 1 (3401): 51.20 6.187 10.00 162.96
+ ID2= 2 (3402): 234.60 24.397 10.50 150.37
=====
| ID = 3 (8232): 285.80 30.260 10.25 152.63
-----
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0342) | Area (ha)=1004.58 Curve Number (CN)= 73.3
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 2.57
-----
  
```

Unit Hyd Qpeak (cms)= 6.686

```

PEAK FLOW (cms)= 33.547 (i)
TIME TO PEAK (hrs)= 12.250
RUNOFF VOLUME (mm)= 142.700
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.673
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0422) | Area (ha)= 780.20 Curve Number (CN)= 73.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.95
-----
  
```

Unit Hyd Qpeak (cms)= 6.838

```

PEAK FLOW (cms)= 32.113 (i)
TIME TO PEAK (hrs)= 12.000
RUNOFF VOLUME (mm)= 141.843
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.669
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0410) | Area (ha)= 572.01 Curve Number (CN)= 68.7
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.46
-----
  
```

Unit Hyd Qpeak (cms)= 6.698

```

PEAK FLOW (cms)= 26.726 (i)
TIME TO PEAK (hrs)= 12.000
RUNOFF VOLUME (mm)= 132.000
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.623
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0408) | Area (ha)= 231.62 Curve Number (CN)= 76.4
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.64
-----
  
```

Unit Hyd Qpeak (cms)= 6.198

```

PEAK FLOW (cms)= 18.415 (i)
TIME TO PEAK (hrs)= 11.000
RUNOFF VOLUME (mm)= 147.127
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.694
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0376) | Area (ha)= 463.85 Curve Number (CN)= 87.6
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.07
-----
  
```

Unit Hyd Qpeak (cms)= 7.380

```

PEAK FLOW (cms)= 32.317 (i)
TIME TO PEAK (hrs)= 11.250
  
```

RUNOFF VOLUME (mm)= 174.746
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.824

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0374) Area (ha)= 545.70 Curve Number (CN)= 78.0
NASHYD (0374) Area (ha)= 545.70 Curve Number (CN)= 78.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.51

Unit Hyd Qpeak (cms)= 6.158

PEAK FLOW (cms)= 28.262 (i)
TIME TO PEAK (hrs)= 11.750
RUNOFF VOLUME (mm)= 152.931
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.721

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0372) Area (ha)= 110.42 Curve Number (CN)= 57.4
NASHYD (0372) Area (ha)= 110.42 Curve Number (CN)= 57.4
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 1.954

PEAK FLOW (cms)= 5.467 (i)
TIME TO PEAK (hrs)= 11.250
RUNOFF VOLUME (mm)= 107.173
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.506

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0370) Area (ha)= 191.85 Curve Number (CN)= 80.3
NASHYD (0370) Area (ha)= 191.85 Curve Number (CN)= 80.3
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.67

Unit Hyd Qpeak (cms)= 4.860

PEAK FLOW (cms)= 15.542 (i)
TIME TO PEAK (hrs)= 11.000
RUNOFF VOLUME (mm)= 156.193
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.737

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0368) Area (ha)= 159.48 Curve Number (CN)= 66.3
NASHYD (0368) Area (ha)= 159.48 Curve Number (CN)= 66.3
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.12

Unit Hyd Qpeak (cms)= 2.433

PEAK FLOW (cms)= 8.386 (i)
TIME TO PEAK (hrs)= 11.500
RUNOFF VOLUME (mm)= 126.385
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.596

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0366) Area (ha)= 462.62 Curve Number (CN)= 79.1
NASHYD (0366) Area (ha)= 462.62 Curve Number (CN)= 79.1
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.06

Unit Hyd Qpeak (cms)= 7.451

PEAK FLOW (cms)= 29.747 (i)
TIME TO PEAK (hrs)= 11.250
RUNOFF VOLUME (mm)= 154.856
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.730

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0364) Area (ha)= 155.27 Curve Number (CN)= 73.6
NASHYD (0364) Area (ha)= 155.27 Curve Number (CN)= 73.6

ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.70

Unit Hyd Qpeak (cms)= 3.782

PEAK FLOW (cms)= 11.501 (i)
TIME TO PEAK (hrs)= 11.000
RUNOFF VOLUME (mm)= 141.255
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.666

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8302) AREA QPEAK TPEAK R.V.
1 + 2 = 3 (ha) (cms) (hrs) (mm)
ID= 1 (0364): 155.27 11.501 11.00 141.25
+ ID= 2 (0366): 462.62 29.747 11.25 154.86
ID = 3 (8302): 617.89 41.038 11.00 151.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0362) Area (ha)= 118.78
STANDHYD (0362) Area (ha)= 118.78
ID= 1 DT=15.0 min Total Imp(%)= 22.00 Dir. Conn.(%)= 8.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 26.13 92.65
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 889.87 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten.(mm/hr)= 53.00 50.34
over (min) 15.00 30.00
Storage Coeff. (min)= 15.05 (ii) 29.13 (iii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.07 0.04

PEAK FLOW (cms)= 1.38 11.05 *TOTALS*
TIME TO PEAK (hrs)= 10.00 10.25 12.195 (iii)
RUNOFF VOLUME (mm)= 210.00 132.29 138.51
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.62 0.65

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.5 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0358) Area (ha)= 429.87 Curve Number (CN)= 55.3
NASHYD (0358) Area (ha)= 429.87 Curve Number (CN)= 55.3
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.03

Unit Hyd Qpeak (cms)= 7.091

PEAK FLOW (cms)= 19.682 (i)
TIME TO PEAK (hrs)= 11.250
RUNOFF VOLUME (mm)= 102.916
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.485

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0360) Area (ha)= 138.37 Curve Number (CN)= 66.3
NASHYD (0360) Area (ha)= 138.37 Curve Number (CN)= 66.3
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.60

Unit Hyd Qpeak (cms)= 3.957

PEAK FLOW (cms)= 9.965 (i)
TIME TO PEAK (hrs)= 11.000
RUNOFF VOLUME (mm)= 124.697
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.588

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
ADD HYD (8306)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0358): 429.87 19.682 11.25 102.92
+ ID2= 2 (0360): 138.37 9.965 11.00 124.70
=====
ID = 3 (8306): 568.24 29.364 11.00 108.22
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ROUTE CHN (6362)
IN= 2--> OUT= 1
Routing time step (min)= 15.00
-----

```

```

<----- DATA FOR SECTION (3621.0) ----->
Distance Elevation Manning
0.00 251.46 0.0550
27.86 254.23 0.0550
51.07 251.96 0.0550
74.29 250.77 0.0550
97.50 249.91 0.0550
125.36 249.40 0.0550
150.93 247.40 0.0550
155.93 247.33 0.0550 / 0.0350 Main Channel
157.93 246.85 0.0350 Main Channel
159.18 246.65 0.0350 Main Channel
160.18 246.63 0.0350 Main Channel
160.93 246.85 0.0350 Main Channel
161.93 247.18 0.0350 / 0.0550 Main Channel
163.18 248.03 0.0550
168.18 248.58 0.0550
183.18 250.18 0.0550
201.97 252.59 0.0550
213.57 256.02 0.0550
225.18 260.31 0.0550
229.82 261.00 0.0550

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.55 247.18 .556E+04 1.9 1.03 49.95
1.28 247.91 .336E+05 16.7 1.53 33.61
2.00 248.63 .913E+05 53.6 1.81 28.40
2.73 249.36 .185E+06 122.3 2.05 25.18
3.46 250.09 .344E+06 224.6 2.02 25.52
4.19 250.82 .574E+06 417.9 2.25 22.91
4.91 251.54 .855E+06 699.1 2.53 20.38
5.64 252.27 .118E+07 1075.3 2.82 18.26
6.37 253.00 .153E+07 1559.9 3.15 16.38
7.10 253.73 .191E+07 2137.6 3.46 14.89
7.82 254.45 .231E+07 2815.2 3.77 13.67
8.55 255.18 .272E+07 3608.5 4.10 12.58
9.28 255.91 .315E+07 4488.2 4.41 11.69
10.01 256.64 .358E+07 5458.0 4.71 10.95
10.73 257.36 .403E+07 6512.4 4.99 10.32
11.46 258.09 .449E+07 7649.8 5.27 9.78
12.19 258.82 .496E+07 8869.5 5.53 9.32
12.92 259.55 .544E+07 10171.2 5.78 8.91
13.64 260.27 .593E+07 11554.8 6.02 8.55

```

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8306) 568.24 29.36 11.00 108.22 1.53 1.62
OUTFLOW: ID= 1 (6362) 568.24 28.03 11.75 108.22 1.50 1.61

```

```

-----
ADD HYD (8304)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0362): 118.78 12.195 10.25 138.53
+ ID2= 2 (6362): 568.24 28.029 11.75 108.22
=====
ID = 3 (8304): 687.02 36.680 11.00 113.46
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ADD HYD (8300)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8302): 617.89 41.038 11.00 151.44
+ ID2= 2 (8304): 687.02 36.680 11.00 113.46
=====
ID = 3 (8300): 1304.91 77.718 11.00 131.44
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ROUTE CHN (6368)
IN= 2--> OUT= 1
Routing time step (min)= 15.00
-----

```

```

<----- DATA FOR SECTION (3681.0) ----->
Distance Elevation Manning
0.00 230.00 0.0370
18.48 223.26 0.0370
36.96 223.05 0.0370
64.67 222.94 0.0370
110.87 222.86 0.0370
133.96 222.74 0.0370
147.82 222.65 0.0370
170.92 222.31 0.0370
174.79 222.26 0.0370 / 0.0300 Main Channel
174.89 221.86 0.0300 Main Channel
175.54 221.86 0.0300 Main Channel
176.19 221.86 0.0300 Main Channel
176.29 222.26 0.0300 / 0.0370 Main Channel
180.16 222.25 0.0370
184.78 222.28 0.0370
189.40 222.31 0.0370
332.60 222.37 0.0370
450.00 230.00 0.0370

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.40 222.26 .771E+03 0.3 0.52 41.74
0.81 222.67 .784E+05 30.3 0.51 43.17
1.21 223.07 .210E+06 123.2 0.77 28.42
1.62 223.48 .384E+06 301.0 1.03 21.25
2.03 223.89 .563E+06 550.2 1.28 17.06
2.44 224.30 .747E+06 860.7 1.51 14.46
2.84 224.70 .934E+06 1228.1 1.73 12.68
3.25 225.11 1.13E+07 1649.3 1.92 11.37
3.66 225.52 1.32E+07 2122.3 2.11 10.37
4.07 225.93 1.52E+07 2645.5 2.29 9.58
4.47 226.33 1.72E+07 3217.8 2.45 8.92
4.88 226.74 1.93E+07 3838.3 2.61 8.38
5.29 227.15 2.14E+07 4506.3 2.76 7.92
5.70 227.56 2.36E+07 5221.3 2.91 7.52
6.10 227.96 2.58E+07 5983.0 3.05 7.17
6.51 228.37 2.80E+07 6790.9 3.19 6.87
6.92 228.78 3.03E+07 7645.0 3.32 6.60
7.33 229.19 3.26E+07 8545.1 3.45 6.35
7.73 229.59 3.49E+07 9491.0 3.57 6.13

```

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8300) 1304.91 77.72 11.00 131.44 1.02 0.61
OUTFLOW: ID= 1 (6368) 1304.91 73.77 11.50 131.44 1.00 0.60

```

```

-----
ADD HYD (8298)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0368): 159.48 8.386 11.50 126.39
+ ID2= 2 (6368): 1304.91 73.772 11.50 131.44
=====
ID = 3 (8298): 1464.39 82.157 11.50 130.89
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ADD HYD (8296)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0370): 191.85 15.542 11.00 156.19
+ ID2= 2 (8298): 1464.39 82.157 11.50 130.89
=====
ID = 3 (8296): 1656.24 96.562 11.50 133.82
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ROUTE CHN (6372)
IN= 2--> OUT= 1
Routing time step (min)= 15.00
-----

```

```

<----- DATA FOR SECTION (3721.0) ----->
Distance Elevation Manning
0.00 225.00 0.0390
30.80 219.38 0.0390
61.61 219.30 0.0390
77.01 219.27 0.0390
469.76 219.14 0.0390

```

477.46	219.13	0.0390	
485.16	219.10	0.0390	
492.86	219.09	0.0390	
495.56	219.09	0.0390 / 0.0310	Main Channel
495.66	218.51	0.0310	Main Channel
500.56	218.51	0.0310	Main Channel
505.46	218.51	0.0310	Main Channel
505.56	219.09	0.0310 / 0.0390	Main Channel
508.26	219.09	0.0390	
515.96	219.10	0.0390	
523.67	219.21	0.0390	
562.17	219.32	0.0390	
654.58	219.43	0.0390	
731.59	219.46	0.0390	
762.39	225.00	0.0390	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.29	218.80	431E+04	1.7	0.58	43.48
0.58	219.09	867E+04	5.1	0.89	28.32
0.93	219.44	200E+06	58.2	0.44	57.35
1.28	219.79	568E+06	278.6	0.74	33.99
1.62	220.13	939E+06	630.2	1.01	24.94
1.97	220.48	131E+07	1090.4	1.25	20.05
2.32	220.83	169E+07	1647.7	1.47	17.06
2.67	221.18	206E+07	2294.6	1.68	14.99
3.01	221.52	244E+07	3025.6	1.87	13.46
3.36	221.87	282E+07	3836.5	2.05	12.27
3.71	222.22	321E+07	4723.7	2.22	11.31
4.06	222.57	359E+07	5684.6	2.39	10.53
4.40	222.91	398E+07	6716.6	2.55	9.87
4.75	223.26	437E+07	7817.7	2.70	9.31
5.10	223.61	476E+07	8986.2	2.85	8.83
5.45	223.96	515E+07	10220.4	2.99	8.40
5.79	224.30	555E+07	11519.1	3.13	8.03
6.14	224.65	594E+07	12880.8	3.27	7.69
6.49	225.00	634E+07	14304.5	3.40	7.39

----- hydrograph ----- <-pipe / channel-->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8296)	1656.24	96.56	11.50	133.82	0.29	0.47
OUTFLOW : ID= 1 (6372)	1656.24	87.59	12.25	133.82	0.97	0.46

----- ADD HYD (8294) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0372)	110.42	5.467	11.25	107.17
+ ID2= 2 (6372)	1656.24	87.590	12.25	133.82
ID = 3 (8294)	1766.66	92.535	12.25	132.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ADD HYD (8292) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0374)	545.70	28.262	11.75	152.93
+ ID2= 2 (8294)	1766.66	92.535	12.25	132.16
ID = 3 (8292)	2312.36	120.216	12.25	137.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- RESERVOIR (5505) -----

IN= 2 --> OUT= 1	DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
		0.0000	0.0000	65.1290	345.3754
		25.4850	24.6597	84.9510	456.3890
		31.1490	98.6787	*****	838.7689
		39.6440	*****	*****	838.7789
		48.1390	*****	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8292)	2312.360	120.216	12.25	137.06
OUTFLOW : ID= 1 (5505)	2312.360	39.146	17.00	137.06

PEAK FLOW REDUCTION [Qout/Qin](%)= 32.56
 TIME SHIFT OF PEAK FLOW (min)=285.00
 MAXIMUM STORAGE USED (ha.m.)=168.3679

----- ADD HYD (8272) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0376)	463.85	32.317	11.25	174.75
+ ID2= 2 (5505)	2312.36	39.146	17.00	137.06
ID = 3 (8272)	2776.21	60.930	11.75	143.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- CALIB NASHVD (0396) -----

ID= 1 DT=15.0 min	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res.(N)
	305.21	5.00	84.1	1.50
			U.H. Tp(hrs)=	1.08

Unit Hyd Qpeak (cms)= 4.811

PEAK FLOW (cms)= 20.475 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 166.498
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.785

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB NASHVD (0394) -----

ID= 1 DT=15.0 min	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res.(N)
	325.45	5.00	71.5	1.50
			U.H. Tp(hrs)=	0.92

Unit Hyd Qpeak (cms)= 6.013

PEAK FLOW (cms)= 20.454 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 137.415
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.648

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB NASHVD (0390) -----

ID= 1 DT=15.0 min	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res.(N)
	420.00	5.00	73.6	1.50
			U.H. Tp(hrs)=	1.07

Unit Hyd Qpeak (cms)= 6.683

PEAK FLOW (cms)= 25.081 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 142.415
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.672

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB NASHVD (0388) -----

ID= 1 DT=15.0 min	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res.(N)
	220.77	5.00	76.3	1.50
			U.H. Tp(hrs)=	0.99

Unit Hyd Qpeak (cms)= 3.819

PEAK FLOW (cms)= 14.258 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 148.317
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.700

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB NASHVD (0386) -----

ID= 1 DT=15.0 min	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res.(N)
	241.27	5.00	78.0	1.50
			U.H. Tp(hrs)=	0.90

Unit Hyd Qpeak (cms)= 4.562

PEAK FLOW (cms)= 16.644 (i)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 151.953
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.717

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

79.39	279.30	0.0300	Main Channel
79.64	279.30	0.0300	Main Channel
79.79	279.64	0.0300 / 0.0380	Main Channel
89.98	279.78	0.0380	
119.09	281.86	0.0380	
145.55	282.87	0.0380	
198.48	284.85	0.0380	
211.71	286.31	0.0380	
230.23	287.59	0.0380	
261.99	294.00	0.0380	

```

-----
| ADD HYD (8286) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0386): 241.27 16.644 11.00 151.95 |
| + ID2= 2 (0388): 220.77 14.258 11.25 148.32 |
| ===== |
| ID = 3 (8286): 462.04 30.850 11.00 150.22 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0384) | Area (ha)= 199.07 Curve Number (CN)= 64.1 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
| U.H. Tp(hrs)= 0.96 |
-----

```

Unit Hyd Qpeak (cms) = 3.537

PEAK FLOW (cms) = 11.033 (i)
 TIME TO PEAK (hrs) = 11.250
 RUNOFF VOLUME (mm) = 121.359
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.572

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0380) | Area (ha)= 182.01 Curve Number (CN)= 59.9 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
| U.H. Tp(hrs)= 0.55 |
-----

```

Unit Hyd Qpeak (cms) = 5.609

PEAK FLOW (cms) = 12.262 (i)
 TIME TO PEAK (hrs) = 11.000
 RUNOFF VOLUME (mm) = 110.871
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.523

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0382) | Area (ha)= 216.59 Curve Number (CN)= 71.5 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
| U.H. Tp(hrs)= 0.64 |
-----

```

Unit Hyd Qpeak (cms) = 5.733

PEAK FLOW (cms) = 16.195 (i)
 TIME TO PEAK (hrs) = 11.000
 RUNOFF VOLUME (mm) = 136.296
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.643

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8290) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0380): 182.01 12.262 11.00 110.87 |
| + ID2= 2 (0382): 216.59 16.195 11.00 136.30 |
| ===== |
| ID = 3 (8290): 398.60 28.457 11.00 124.69 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6384) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00 |
-----

```

<----- DATA FOR SECTION (3841.0) ----->

Distance	Elevation	Manning
0.00	294.40	0.0380
10.59	291.93	0.0380
21.17	289.19	0.0380
26.46	287.99	0.0380
31.76	286.79	0.0380
71.45	279.97	0.0380
74.10	279.79	0.0380
76.74	279.71	0.0380
78.99	279.64	0.0380 / 0.0300
79.14	279.30	0.0300

```

-----
| TRAVEL TIME TABLE |
| DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV TIME |
| (m) (m) (cu.m.) (cms) (m/s) (min) |
-----
| 0.34 279.64 .715E+03 0.2 0.85 63.42 |
| 1.10 280.40 .507E+05 21.9 1.40 38.54 |
| 1.85 281.15 .141E+06 91.8 2.10 25.69 |
| 2.61 281.91 .269E+06 218.9 2.63 20.46 |
| 3.36 282.66 .445E+06 407.1 2.96 18.23 |
| 4.12 283.42 .681E+06 698.3 3.31 16.26 |
| 4.87 284.17 .977E+06 1111.8 3.68 14.65 |
| 5.63 284.93 .133E+07 1674.3 4.06 13.27 |
| 6.39 285.69 .173E+07 2457.8 4.59 11.73 |
| 7.14 286.44 .215E+07 3378.8 5.07 10.63 |
| 7.90 287.20 .261E+07 4418.4 5.47 9.85 |
| 8.65 287.95 .310E+07 5676.3 5.92 9.11 |
| 9.41 288.71 .361E+07 7150.5 6.40 8.42 |
| 10.17 289.47 .414E+07 8783.1 6.86 7.86 |
| 10.92 290.22 .469E+07 10575.1 7.30 7.38 |
| 11.68 290.98 .525E+07 12520.4 7.72 6.98 |
| 12.43 291.73 .582E+07 14619.0 8.12 6.64 |
| 13.19 292.49 .642E+07 16863.2 8.50 6.34 |
| 13.94 293.24 .703E+07 19259.8 8.86 6.08 |
-----

```

```

-----
| <---- hydrograph ----> <-pipe / channel-> |
| AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL |
| (ha) (cms) (hrs) (mm) (m) (m/s) |
| INFLOW : ID= 2 (8290) 398.60 28.46 11.00 124.69 1.17 1.44 |
| OUTFLOW : ID= 1 (6384) 398.60 26.39 11.50 124.69 1.14 1.43 |
-----

```

```

-----
| ADD HYD (8288) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0384): 199.07 11.033 11.25 121.36 |
| + ID2= 2 (6384): 398.60 26.395 11.50 124.69 |
| ===== |
| ID = 3 (8288): 597.67 37.357 11.25 123.58 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8284) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (8286): 462.04 30.850 11.00 150.22 |
| + ID2= 2 (8288): 597.67 37.357 11.25 123.58 |
| ===== |
| ID = 3 (8284): 1059.71 68.206 11.25 135.19 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8280) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0390): 420.00 25.081 11.25 142.42 |
| + ID2= 2 (8284): 1059.71 68.206 11.25 135.19 |
| ===== |
| ID = 3 (8280): 1479.71 93.286 11.25 137.24 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0392) | Area (ha)= 167.22 Curve Number (CN)= 79.2 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
| U.H. Tp(hrs)= 0.74 |
-----

```

Unit Hyd Qpeak (cms) = 3.837

PEAK FLOW (cms) = 12.849 (i)
 TIME TO PEAK (hrs) = 11.000
 RUNOFF VOLUME (mm) = 154.075
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.727

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALLIB (0378) Area (ha)= 606.72 Curve Number (CN)= 74.3
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.18

Unit Hyd Qpeak (cms)= 8.771
PEAK FLOW (cms)= 34.617 (1)
TIME TO PEAK (hrs)= 11.500
RUNOFF VOLUME (mm)= 144.163
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.680

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8282)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0378): 606.72 34.617 11.50 144.16
+ ID2= 2 (0392): 167.22 12.849 11.00 154.08
ID= 3 (8282): 773.94 47.183 11.25 146.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8278)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8280): 1479.71 93.286 11.25 137.24
+ ID2= 2 (8282): 773.94 47.183 11.25 146.30
ID= 3 (8278): 2253.65 140.470 11.25 140.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6394)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

Table with 4 columns: Distance, Elevation, Manning, and Routing time step (min)'= 15.00. Contains data for section 3941.0.

Table with 6 columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV.TIME. Contains data for section 3941.0.

hydrograph <---> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8278) 2253.65 140.47 11.25 140.35 2.61 2.82
OUTFLOW : ID= 1 (6394) 2253.65 138.80 11.50 140.35 2.60 2.81

ADD HYD (8276)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0394): 325.45 20.454 11.25 137.41
+ ID2= 2 (6394): 2253.65 138.801 11.50 140.35
ID= 3 (8276): 2579.10 158.950 11.50 139.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6396)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

Table with 4 columns: Distance, Elevation, Manning, and Routing time step (min)'= 15.00. Contains data for section 3961.0.

Table with 6 columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV.TIME. Contains data for section 3961.0.

hydrograph <---> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8276) 2579.10 158.95 11.50 139.98 2.60 1.80
OUTFLOW : ID= 1 (6396) 2579.10 155.83 12.00 139.98 2.58 1.80

ADD HYD (8274)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0396): 305.21 20.475 11.25 166.50
+ ID2= 2 (6396): 2579.10 155.832 12.00 139.98
ID= 3 (8274): 2884.31 175.498 11.75 142.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8270)	1	2	3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8272):	2776.21	60.930	11.75	143.36			
+ ID2= 2 (8274):	2884.31	175.498	11.75	142.79			
=====							
ID = 3 (8270):	5660.52	236.428	11.75	143.07			

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5506)	IN= 2--> OUT= 1	DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
			0.0000	0.0000	60.8810	135.6832
			31.1490	24.6697	96.2770	900.4431
			36.8120	37.0045	96.6770	900.4531
			45.3070	86.3439	0.0000	0.0000

INFLOW : ID= 2 (8270)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	5660.520	236.428	11.75	143.07
OUTFLOW: ID= 1 (5506)	5660.520	70.731	18.00	143.07

PEAK FLOW REDUCTION (Qout/Qin)(%)= 29.92
 TIME SHIFT OF PEAK FLOW (min)=375.00
 MAXIMUM STORAGE USED (ha.m.)=348.4974

CALIB	Area (ha)	QPEAK (cms)	Curve Number (CN)	R. V. (mm)
NASHYD (0406)	142.65	4.135	82.0	82.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	U.H. Tp(hrs)= 0.59	# of Linear Res.(N)= 1.50	

Unit Hyd Qpeak (cms) = 4.135

PEAK FLOW (cms) = 12.335 (1)
 TIME TO PEAK (hrs) = 11.000
 RUNOFF VOLUME (mm) = 159.436
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.752

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	QPEAK (cms)	Curve Number (CN)	R. V. (mm)
NASHYD (0404)	246.46	4.280	66.6	66.6
ID= 1 DT=15.0 min	Ia (mm)= 5.00	U.H. Tp(hrs)= 0.98	# of Linear Res.(N)= 1.50	

Unit Hyd Qpeak (cms) = 4.280

PEAK FLOW (cms) = 14.014 (1)
 TIME TO PEAK (hrs) = 11.250
 RUNOFF VOLUME (mm) = 126.803
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.598

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	QPEAK (cms)	Curve Number (CN)	R. V. (mm)
NASHYD (0402)	244.00	3.879	78.0	78.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	U.H. Tp(hrs)= 1.07	# of Linear Res.(N)= 1.50	

Unit Hyd Qpeak (cms) = 3.879

PEAK FLOW (cms) = 15.381 (1)
 TIME TO PEAK (hrs) = 11.250
 RUNOFF VOLUME (mm) = 152.367
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.719

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	QPEAK (cms)	Curve Number (CN)	R. V. (mm)
NASHYD (0400)	93.97	3.630	71.1	71.1
ID= 1 DT=15.0 min	Ia (mm)= 5.00	U.H. Tp(hrs)= 0.44	# of Linear Res.(N)= 1.50	

Unit Hyd Qpeak (cms) = 3.630

PEAK FLOW (cms) = 7.874 (1)
 TIME TO PEAK (hrs) = 11.000
 RUNOFF VOLUME (mm) = 133.418
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.629

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	QPEAK (cms)	Curve Number (CN)	R. V. (mm)
NASHYD (0398)	328.19	6.759	73.9	73.9
ID= 1 DT=15.0 min	Ia (mm)= 5.00	U.H. Tp(hrs)= 0.83	# of Linear Res.(N)= 1.50	

Unit Hyd Qpeak (cms) = 6.759

PEAK FLOW (cms) = 22.535 (1)
 TIME TO PEAK (hrs) = 11.000
 RUNOFF VOLUME (mm) = 142.469
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.672

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8268)	1	2	3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0398):	328.19	22.535	11.00	142.47			
+ ID2= 2 (0400):	93.97	7.874	11.00	133.42			
=====							
ID = 3 (8268):	422.16	30.408	11.00	140.45			

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6402)	IN= 2--> OUT= 1	Routing time step (min) = 15.00
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Distance	Elevation	Manning
0.00	238.50	0.0360
11.50	238.00	0.0360
23.00	237.93	0.0360
34.49	236.39	0.0360
63.24	233.98	0.0360
97.73	228.15	0.0360
123.60	227.08	0.0360
126.48	226.61	0.0360
127.60	226.47	0.0360 / 0.0330
127.85	225.25	0.0330
129.35	225.25	0.0330
130.85	225.25	0.0330
131.10	226.47	0.0330 / 0.0360
132.22	226.59	0.0360
143.72	227.42	0.0360
169.59	227.88	0.0360
192.59	231.19	0.0360
218.46	233.02	0.0360
241.45	235.50	0.0360
284.57	236.43	0.0360

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.41	225.66	.391E+04	1.7	1.36	38.31
0.81	226.06	.804E+04	5.0	1.93	27.02
1.22	226.47	.124E+05	9.1	2.30	22.61
1.84	227.09	.308E+05	23.3	2.36	22.03
2.46	227.71	.904E+05	62.0	2.14	24.31
3.09	228.34	.223E+06	188.3	2.63	19.75
3.71	228.96	.379E+06	410.2	3.38	15.39
4.33	229.58	.550E+06	711.5	4.04	12.89
4.95	230.20	.737E+06	1091.3	4.62	11.25
5.58	230.83	.939E+06	1550.6	5.15	10.10
6.20	231.45	.116E+07	2072.4	5.59	9.31
6.82	232.07	.140E+07	2659.2	5.93	8.77
7.44	232.69	.167E+07	3345.8	6.27	8.30
8.07	233.32	.195E+07	4159.0	6.64	7.83
8.69	233.94	.226E+07	5100.8	7.03	7.39
9.31	234.56	.259E+07	6081.3	7.32	7.11
9.93	235.18	.295E+07	7183.1	7.60	6.84
10.56	235.81	.334E+07	8145.2	7.62	6.83
11.18	236.43	.379E+07	9116.0	7.51	6.92

INFLOW : ID= 2 (8268)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
	422.16	30.41	11.00	140.45	1.96	2.32
OUTFLOW: ID= 1 (6402)	422.16	28.99	11.50	140.45	1.93	2.33

ADD HYD (8266)

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0402):	244.00	15.381	11.25	152.37
+ ID2= 2 (6402):	422.16	28.995	11.50	140.45
=====				
ID = 3 (8266):	666.16	44.353	11.25	144.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8264)	1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0404):		246.46	14.014	11.25	126.80
+ ID2= 2 (8264):		666.16	44.353	11.25	144.82
=====					
ID = 3 (8264):		912.62	58.366	11.25	139.95

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8262)	1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0406):		142.65	12.335	11.00	159.44
+ ID2= 2 (8264):		912.62	58.366	11.25	139.95
=====					
ID = 3 (8262):		1055.27	70.165	11.25	142.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8260)	1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (5506):		5660.52	70.731	18.00	143.07
+ ID2= 2 (8260):		1055.27	70.165	11.25	142.59
=====					
ID = 3 (8260):		6715.79	131.675	11.25	142.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8258)	1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0408):		231.62	18.415	11.00	147.13
+ ID2= 2 (8260):		6715.79	131.675	11.25	142.99
=====					
ID = 3 (8258):		6947.41	149.479	11.25	143.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8256)	1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0410):		572.01	26.726	12.00	132.00
+ ID2= 2 (8256):		6947.41	149.479	11.25	143.13
=====					
ID = 3 (8256):		7519.42	175.654	11.25	142.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5507)	DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
		0.0000	0.0000	90.6140	67.8416
		39.6440	12.3348	*****	160.3529
		48.1390	18.5023	*****	160.3629
		67.9600	37.0045	0.0000	0.0000

INFLOW : ID= 2 (8256)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	7519.421	175.654	11.25	142.28

OUTFLOW : ID= 1 (5507)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	7519.421	118.016	14.25	142.28

PEAK FLOW REDUCTION [Qout/Qin](%)= 67.19
 TIME SHIFT OF PEAK FLOW (min)=180.00
 MAXIMUM STORAGE USED (ha.m.)=120.5190

CALIB	NASHYD (0420)	Area (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	Curve Number (CN)=	# of Linear Res. (N)=
ID= 1 DT=15.0 min		175.82	5.00	0.81		71.8	1.50

Unit Hyd Qpeak (cms)= 3.692

PEAK FLOW (cms)= 11.871 (i)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 137.745
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.650

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD (0418)	Area (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	Curve Number (CN)=	# of Linear Res. (N)=
ID= 1 DT=15.0 min		182.79	5.00	1.05		80.5	1.50

Unit Hyd Qpeak (cms)= 2.966

PEAK FLOW (cms)= 11.988 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 158.060
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.746

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD (0416)	Area (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	Curve Number (CN)=	# of Linear Res. (N)=
ID= 1 DT=15.0 min		439.30	5.00	1.29		81.0	1.50

Unit Hyd Qpeak (cms)= 5.832

PEAK FLOW (cms)= 25.927 (i)
 TIME TO PEAK (hrs)= 11.500
 RUNOFF VOLUME (mm)= 159.607
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.753

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD (0412)	Area (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	Curve Number (CN)=	# of Linear Res. (N)=
ID= 1 DT=15.0 min		238.70	5.00	0.80		73.4	1.50

Unit Hyd Qpeak (cms)= 5.088

PEAK FLOW (cms)= 16.572 (i)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 141.253
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.666

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (4141)	Area (ha)	Total Imp(%)	Dir. Conn.(%)
ID= 1 DT=15.0 min		43.70	36.00	17.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	15.73 27.97
Dep. Storage (mm)=	2.00 5.00
Average Slope (%)=	0.50 0.50
Length (m)=	539.75 40.00
Mannings n =	0.013 0.250

Max. Eff. Inten.(mm/hr)= 53.00 65.12
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 11.15 (ii) 23.85 (iii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

PEAK FLOW (cms)= 1.09 4.53 *TOTALS*
 TIME TO PEAK (hrs)= 10.00 10.00 5.621 (iiii)
 RUNOFF VOLUME (mm)= 210.00 171.31 177.89
 TOTAL RAINFALL (mm)= 212.00 212.00 212.00
 RUNOFF COEFFICIENT = 0.99 0.81 0.84

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 81.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (4142) | Area (ha)= 144.30
ID= 1 DT=15.0 min | Total Imp(%)= 21.00 Dir. Conn.(%)= 10.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	30.30	114.00
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	980.82	40.00
Mannings n =	0.013	0.250
Max.Eff.Inten.(mm/hr)=	53.00	56.49
over (min)=	15.00	30.00
Storage Coeff. (min)=	15.95 (ii)	29.40 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.07	0.04
PEAK FLOW (cms)=	2.09	15.44
TIME TO PEAK (hrs)=	10.00	10.25
RUNOFF VOLUME (mm)=	210.00	166.70
TOTAL RAINFALL (mm)=	212.00	212.00
RUNOFF COEFFICIENT =	0.99	0.79

TOTALS
17.281 (iii)

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 81.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8254) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (4141): 43.70 5.621 10.00 177.89
+ ID2= 2 (4142): 144.30 17.281 10.00 171.03
ID = 3 (8254): 188.00 22.901 10.00 172.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8252) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0412): 238.70 16.572 11.00 141.25
+ ID2= 2 (8254): 188.00 22.901 10.00 172.63
ID = 3 (8252): 426.70 36.423 10.25 155.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6416) |
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (4161.0) ----->

Distance	Elevation	Manning	
0.00	270.07	0.0340	
20.67	267.91	0.0340	
62.01	264.33	0.0340	
113.69	259.75	0.0340	
165.37	253.30	0.0340	
227.38	246.29	0.0340	
232.55	246.03	0.0340	
237.72	246.16	0.0340	
241.39	246.02	0.0340 / 0.0300	Main Channel
241.64	245.75	0.0300	Main Channel
242.89	245.75	0.0300	Main Channel
244.14	245.75	0.0300	Main Channel
244.39	246.02	0.0300 / 0.0340	Main Channel
248.06	246.20	0.0340	
253.22	246.28	0.0340	
258.39	246.63	0.0340	
346.25	252.57	0.0340	
413.43	257.77	0.0340	
465.11	261.78	0.0340	
511.62	270.00	0.0340	

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.27	246.02	.295E+04	0.6	0.75	88.26
1.53	247.28	.168E+06	69.7	1.65	40.13
2.79	248.54	.490E+06	299.5	2.42	27.28
4.06	249.81	.962E+06	740.1	3.05	21.67
5.32	251.07	.158E+07	1440.0	3.61	18.33
6.58	252.33	.235E+07	2443.8	4.12	16.06
7.84	253.59	.327E+07	3811.2	4.63	14.30
9.10	254.85	.432E+07	5565.1	5.11	12.94
10.37	256.12	.550E+07	7721.3	5.57	11.88
11.63	257.38	.682E+07	10309.9	6.00	11.02
12.89	258.64	.827E+07	13360.6	6.42	10.31
14.15	259.90	.985E+07	16884.5	6.81	9.72
15.42	261.17	.116E+08	20818.4	7.14	9.26
16.68	262.43	.134E+08	25510.4	7.53	8.78
17.94	263.69	.154E+08	30958.0	7.97	8.30
19.20	264.95	.175E+08	36988.3	8.38	7.89
20.46	266.21	.197E+08	43617.1	8.78	7.53
21.73	267.48	.220E+08	50866.1	9.17	7.22
22.99	268.74	.244E+08	58854.0	9.56	6.92

<---- hydrograph ----> <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8252)	426.70	36.42	10.25	155.08	0.92
OUTFLOW: ID= 1 (6416)	426.70	31.82	11.25	155.08	0.84

ADD HYD (8250) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0416): 439.30 25.927 11.50 159.61
+ ID2= 2 (6416): 426.70 31.818 11.25 155.08
ID = 3 (8250): 866.00 57.651 11.25 157.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8248) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0418): 182.79 11.988 11.25 158.06
+ ID2= 2 (8250): 866.00 57.651 11.25 157.37
ID = 3 (8248): 1048.79 69.639 11.25 157.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8246) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0420): 175.82 11.871 11.00 137.75
+ ID2= 2 (8248): 1048.79 69.639 11.25 157.49
ID = 3 (8246): 1224.61 81.403 11.25 154.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8244) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (5507): 7519.42 118.016 14.25 142.28
+ ID2= 2 (8246): 1224.61 81.403 11.25 154.66
ID = 3 (8244): 8744.03 181.100 12.00 144.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8242) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0422): 780.20 32.113 12.00 141.84
+ ID2= 2 (8244): 8744.03 181.100 12.00 144.02
ID = 3 (8242): 9524.23 213.214 12.00 143.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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RESERVOIR (5508)
IN= 2--> OUT= 1
DT= 15.0 min
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	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	*****	197.3574
	76.4550	30.8371	*****	394.7148
	*****	61.6742	*****	394.7248
	*****	*****	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8242)	9524.231	213.214	12.00	143.84
OUTFLOW: ID= 1 (5508)	9524.231	162.331	15.50	143.84


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PEAK FLOW REDUCTION [Qout/Qin](%)= 76.14
TIME SHIFT OF PEAK FLOW (min)=210.00
MAXIMUM STORAGE USED (ha.m.)=142.1958
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CALIB
NASHVD (0336)
ID= 1 DT=15.0 min
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Area	(ha)=	2785.00	Curve Number (CN)=	86.4
Ia	(mm)=	5.00	# of Linear Res.(N)=	1.50
U.H. Tp	(hrs)=	15.39		


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Unit Hyd Qpeak (cms)= 3.090
PEAK FLOW (cms)= 20.993 (i)
TIME TO PEAK (hrs)= 24.000
RUNOFF VOLUME (mm)= 173.232
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.817
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(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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CALIB
STANDHYD (3382)
ID= 1 DT=15.0 min
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Area	(ha)=	525.90	Dir. Conn.(%)=	17.00
Total Imp	(%)=	37.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	194.58	331.32
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1872.43	40.00
Mannings n =	0.013	0.250

Max.Eff.Inten.(mm/hr)=	53.00	66.50
over (min)	30.00	45.00
Storage Coeff. (min)=	23.52 (ii)	36.11 (ii)
Unit Hyd. Tpeak (min)=	30.00	45.00
Unit Hyd. peak (cms)=	0.04	0.03

	PEAK FLOW (cms)=	11.98	49.95	61.079 (iii)
TIME TO PEAK (hrs)=	10.00	10.50	10.25	
RUNOFF VOLUME (mm)=	210.00	173.23	179.48	
TOTAL RAINFALL (mm)=	212.00	212.00	212.00	
RUNOFF COEFFICIENT =	0.99	0.82	0.85	

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 82.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB
STANDHYD (3381)
ID= 1 DT=15.0 min
-----

```

Area	(ha)=	73.00	Dir. Conn.(%)=	23.00
Total Imp	(%)=	49.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	35.77	37.23
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	697.61	40.00
Mannings n =	0.013	0.250

Max.Eff.Inten.(mm/hr)=	53.00	76.96
over (min)	15.00	30.00
Storage Coeff. (min)=	13.00 (ii)	24.88 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04

	PEAK FLOW (cms)=	2.45	7.09	9.539 (iii)
TIME TO PEAK (hrs)=	10.00	10.00	10.00	
RUNOFF VOLUME (mm)=	210.00	177.49	184.97	
TOTAL RAINFALL (mm)=	212.00	212.00	212.00	
RUNOFF COEFFICIENT =	0.99	0.84	0.87	

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 82.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
ADD HYD (8310)
1 + 2 = 3
-----

```

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (3381):	73.00	9.539	10.00	184.97
+ ID= 2 (3382):	525.90	61.079	10.25	179.48
ID = 3 (8310):	598.90	70.136	10.25	180.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
CALIB
NASHVD (3342)
ID= 1 DT=15.0 min
-----

```

Area	(ha)=	587.10	Curve Number (CN)=	75.0
Ia	(mm)=	5.00	# of Linear Res.(N)=	1.50
U.H. Tp	(hrs)=	8.19		


```

Unit Hyd Qpeak (cms)= 1.224
PEAK FLOW (cms)= 6.972 (i)
TIME TO PEAK (hrs)= 17.250
RUNOFF VOLUME (mm)= 146.846
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.693
-----

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB
STANDHYD (3341)
ID= 1 DT=15.0 min
-----

```

Area	(ha)=	33.00	Dir. Conn.(%)=	31.00
Total Imp	(%)=	51.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	16.83	16.17
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	469.04	40.00
Mannings n =	0.013	0.250

Max.Eff.Inten.(mm/hr)=	53.00	66.54
over (min)	15.00	30.00
Storage Coeff. (min)=	10.25 (ii)	22.84 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.04

	PEAK FLOW (cms)=	1.50	2.67	4.174 (iii)
TIME TO PEAK (hrs)=	10.00	10.00	10.00	
RUNOFF VOLUME (mm)=	210.00	153.82	171.23	
TOTAL RAINFALL (mm)=	212.00	212.00	212.00	
RUNOFF COEFFICIENT =	0.99	0.73	0.81	

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
ADD HYD (8314)
1 + 2 = 3
-----

```

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (3341):	33.00	4.174	10.00	171.23
+ ID= 2 (3342):	587.10	6.972	17.25	146.85
ID = 3 (8314):	620.10	7.956	11.00	148.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
CALIB
NASHVD (0306)
ID= 1 DT=15.0 min
-----

```

Area	(ha)=	283.97	Curve Number (CN)=	71.2
Ia	(mm)=	5.00	# of Linear Res.(N)=	1.50
U.H. Tp	(hrs)=	6.44		


```

Unit Hyd Qpeak (cms)= 0.753
PEAK FLOW (cms)= 4.007 (i)
-----

```

TIME TO PEAK (hrs)= 15.750
 RUNOFF VOLUME (mm)= 138.250
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.652

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHVD (0286) | Area (ha)= 260.51 | Curve Number (CN)= 93.4
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.16

Unit Hyd Qpeak (cms)= 3.834

PEAK FLOW (cms)= 18.264 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 188.924
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.891

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHVD (0282) | Area (ha)= 449.38 | Curve Number (CN)= 89.1
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.47

Unit Hyd Qpeak (cms)= 5.226

PEAK FLOW (cms)= 26.730 (i)
 TIME TO PEAK (hrs)= 11.750
 RUNOFF VOLUME (mm)= 178.945
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.844

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHVD (0284) | Area (ha)= 78.93 | Curve Number (CN)= 93.3
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.57

Unit Hyd Qpeak (cms)= 2.344

PEAK FLOW (cms)= 7.418 (i)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 185.839
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.877

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8388) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0282): 449.38 26.730 11.75 178.94
 + ID2= 2 (0284): 78.93 7.418 11.00 185.84
 =====
 ID = 3 (8388): 528.31 33.585 11.25 179.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6286) |
 IN= 2 -> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2861.0) ----->

Distance	Elevation	Manning
0.00	233.00	0.0450
20.58	228.51	0.0450
41.17	227.74	0.0450
51.46	227.41	0.0450
97.77	225.96	0.0450
149.23	223.94	0.0450
200.69	220.84	0.0450
226.42	220.66	0.0450
238.85	220.22	0.0450
241.35	220.01	0.0450 / 0.0350 Main Channel
241.85	219.70	0.0350 Main Channel
245.85	219.72	0.0350 Main Channel
246.35	220.06	0.0350 Main Channel
248.85	220.23	0.0350 Main Channel
303.60	221.64	0.0350 Main Channel
380.79	224.98	0.0450
432.25	229.54	0.0450

457.98 233.33 0.0450
 483.71 234.27 0.0450
 509.44 233.81 0.0450

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.31	220.01	382E+04	0.8	0.57	82.89
1.06	220.76	525E+05	12.1	0.66	72.51
1.81	221.51	240E+06	88.6	1.05	45.21
2.56	222.26	510E+06	261.7	1.46	32.47
3.31	223.01	843E+06	533.8	1.81	26.34
4.06	223.76	124E+07	933.6	2.10	22.64
4.81	224.51	171E+07	1399.0	2.34	20.33
5.56	225.26	225E+07	2051.0	2.60	18.27
6.31	226.01	285E+07	2907.0	2.91	16.35
7.06	226.76	352E+07	3903.5	3.16	15.04
7.82	227.52	426E+07	5069.9	3.39	14.01
8.57	228.27	507E+07	6433.0	3.62	13.13
9.32	229.02	593E+07	8069.2	3.89	12.24
10.07	229.77	681E+07	9926.7	4.16	11.44
10.82	230.52	772E+07	11962.8	4.42	10.75
11.57	231.27	864E+07	14167.1	4.68	10.17
12.32	232.02	959E+07	16537.1	4.92	9.66
13.07	232.77	105E+08	19070.7	5.16	9.22
13.82	233.52	115E+08	21790.4	5.39	8.82

<---- hydrograph ----> <-pipe / channel-->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW: ID= 2 (8388) 528.31 33.59 11.25 179.97 1.27 0.73
 OUTFLOW: ID= 1 (6286) 528.31 30.63 12.25 179.97 1.24 0.72

ADD HYD (8386) |
 1 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0286): 260.51 18.264 11.25 188.92
 + ID2= 2 (6286): 528.31 30.634 12.25 179.97
 =====
 ID = 3 (8386): 788.82 48.029 12.00 182.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHVD (0302) | Area (ha)= 473.90 | Curve Number (CN)= 75.7
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.66

Unit Hyd Qpeak (cms)= 4.874

PEAK FLOW (cms)= 22.555 (i)
 TIME TO PEAK (hrs)= 12.000
 RUNOFF VOLUME (mm)= 147.792
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.697

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHVD (0300) | Area (ha)= 258.93 | Curve Number (CN)= 70.8
 ID= 1 DT=15.0 min | Ia (mm)= 4.00 | # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.03

Unit Hyd Qpeak (cms)= 4.292

PEAK FLOW (cms)= 15.281 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 136.980
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.646

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHVD (0298) | Area (ha)= 330.51 | Curve Number (CN)= 64.6
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.26

Unit Hyd Qpeak (cms)= 4.461

PEAK FLOW (cms)= 15.779 (i)
 TIME TO PEAK (hrs)= 11.500
 RUNOFF VOLUME (mm)= 122.883
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.580

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8395) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0298): 330.51 15.779 11.50 122.88 |
| + ID2= 2 (0300): 258.93 15.281 11.25 136.98 |
| ===== |
| ID = 3 (8395): 589.44 30.954 11.50 129.08 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0288) | Area (ha)= 340.83 Curve Number (CN)= 89.9 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 2.21 |
-----

```

Unit Hyd Qpeak (cms)= 2.629

```

PEAK FLOW (cms)= 15.781 (1)
TIME TO PEAK (hrs)= 12.000
RUNOFF VOLUME (mm)= 161.352
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.855

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0290) | Area (ha)= 269.18 Curve Number (CN)= 90.9 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 2.07 |
-----

```

Unit Hyd Qpeak (cms)= 4.279

```

PEAK FLOW (cms)= 19.260 (1)
TIME TO PEAK (hrs)= 11.250
RUNOFF VOLUME (mm)= 182.662
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.862

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8397) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0288): 340.83 15.781 12.00 181.35 |
| + ID2= 2 (0290): 269.18 19.260 11.25 182.66 |
| ===== |
| ID = 3 (8397): 610.01 34.406 11.50 181.93 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0296) | Area (ha)= 293.65 Curve Number (CN)= 88.9 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 1.13 |
-----

```

Unit Hyd Qpeak (cms)= 4.437

```

PEAK FLOW (cms)= 20.151 (1)
TIME TO PEAK (hrs)= 11.250
RUNOFF VOLUME (mm)= 177.972
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.839

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0292) | Area (ha)= 738.49 Curve Number (CN)= 83.9 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 1.52 |
-----

```

Unit Hyd Qpeak (cms)= 8.289

```

PEAK FLOW (cms)= 40.850 (1)
TIME TO PEAK (hrs)= 11.750
RUNOFF VOLUME (mm)= 166.632
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.786

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0294) | Area (ha)= 274.15 Curve Number (CN)= 75.3 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 0.87 |
-----

```

Unit Hyd Qpeak (cms)= 5.367

```

PEAK FLOW (cms)= 18.657 (1)
TIME TO PEAK (hrs)= 11.000
RUNOFF VOLUME (mm)= 145.749
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.687

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8398) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0292): 738.49 40.850 11.75 166.63 |
| + ID2= 2 (0294): 274.15 18.657 11.00 145.75 |
| ===== |
| ID = 3 (8398): 1012.64 58.892 11.50 160.98 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CEN (6296) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00 |
-----

```

<----- DATA FOR SECTION (2961.0) ----->

Distance	Elevation	Manning
0.00	243.98	0.0400
42.59	243.18	0.0400
85.17	241.81	0.0400
120.66	240.50	0.0400
156.15	239.56	0.0400
198.74	236.15	0.0400
237.78	234.01	0.0400
241.33	233.82	0.0400
248.77	233.12	0.0400 /0.0400
249.87	232.32	0.0400
250.37	231.80	0.0400
250.87	232.23	0.0400
251.97	233.10	0.0400 /0.0400
255.37	233.22	0.0400
259.07	233.87	0.0400
262.62	234.12	0.0400
266.17	234.23	0.0400
283.91	234.73	0.0400
337.15	241.75	0.0400
351.34	244.00	0.0400

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.43	232.23	.510E+03	0.1	0.53	81.07
0.87	232.67	.213E+04	0.7	0.83	51.49
1.30	233.10	.501E+04	2.2	1.10	38.75
1.98	233.78	.242E+05	12.6	1.34	31.93
2.66	234.46	.738E+05	42.1	1.46	29.21
3.34	235.14	.177E+06	127.9	1.64	23.13
4.02	235.82	.314E+06	281.4	2.29	18.61
4.70	236.50	.481E+06	509.4	2.71	15.73
5.38	237.18	.672E+06	815.6	3.11	13.73
6.06	237.86	.895E+06	1200.7	3.46	12.30
6.74	238.54	.112E+07	1669.4	3.80	11.23
7.42	239.22	.139E+07	2226.9	4.11	10.38
8.10	239.90	.168E+07	2808.4	4.28	9.95
8.78	240.58	.202E+07	3470.4	4.40	9.68
9.46	241.26	.240E+07	4331.7	4.61	9.24
10.14	241.94	.283E+07	5335.2	4.83	8.84
10.82	242.62	.330E+07	6492.6	5.03	8.48
11.50	243.30	.381E+07	7764.1	5.21	8.18
12.18	243.98	.439E+07	9101.2	5.31	8.04

<---- hydrograph ---->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8398)	1012.64	58.89	11.50	160.98	2.79	1.52
OUTFLOW: ID= 1 (6296)	1012.64	57.83	12.00	160.98	2.78	1.52

```

-----
| ADD HYD (8396) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
|-----|

```

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-----
      (ha)   (cms)   (hrs)   (mm)
ID1= 1 (0296): 293.65 20.151 11.25 177.97
+ ID2= 2 (6296): 1012.64 57.830 12.00 160.98
=====
ID = 3 (8396): 1306.29 77.347 11.75 164.80
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8394) |
| 1 + 2 = 3 |
      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)   (hrs)   (mm)
ID1= 1 (8396): 1306.29 77.347 11.75 164.80
+ ID2= 2 (8397): 610.01 34.406 11.50 181.93
=====
ID = 3 (8394): 1916.30 111.656 11.75 170.25
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8392) |
| 1 + 2 = 3 |
      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)   (hrs)   (mm)
ID1= 1 (8394): 1916.30 111.656 11.75 170.25
+ ID2= 2 (8395): 589.44 30.954 11.50 129.08
=====
ID = 3 (8392): 2505.74 142.366 11.75 160.57
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6302) |
| IN= 2--> OUT= 1 |
      Routing time step (min) = 15.00
-----

```

```

-----
<----- DATA FOR SECTION (3021.0) ----->
Distance   Elevation   Manning
-----
0.00       228.10       0.0400
18.47     227.12       0.0400
36.95     226.12       0.0400
46.18     225.84       0.0400
55.42     225.58       0.0400
272.47    222.88       0.0400
277.09    222.76       0.0400
281.71    222.58       0.0400
288.54    222.18       0.0400 / 0.0300 Main Channel
288.64    221.00       0.0300 Main Channel
290.94    221.00       0.0300 Main Channel
291.04    221.00       0.0300 Main Channel
291.54    222.75       0.0300 / 0.0400 Main Channel
300.18    222.83       0.0400
304.80    223.04       0.0400
309.42    223.25       0.0400
318.65    223.69       0.0400
360.22    225.57       0.0400
397.16    227.60       0.0400
457.20    228.35       0.0400
-----

```

```

-----
<----- TRAVEL TIME TABLE ----->
DEPTH   ELEV   VOLUME   FLOW RATE   VELOCITY   TRAV.TIME
(m)     (m)     (cu.m.)   (cms)       (m/s)     (min)
-----
0.29    221.29  .228E+04  0.2         0.32      165.63
0.59    221.59  .465E+04  0.7         0.45      116.15
0.88    221.88  .713E+04  1.2         0.54      96.87
1.18    222.18  .971E+04  1.9         0.61      86.20
1.57    222.57  .175E+05  3.3         0.59      88.86
1.97    222.97  .427E+05  6.4         0.47      111.39
2.36    223.36  .115E+06  16.1        0.44      119.14
2.76    223.76  .237E+06  37.1        0.49      106.49
3.15    224.15  .409E+06  72.9        0.56      93.40
3.55    224.55  .631E+06  127.0       0.63      82.76
3.94    224.94  .903E+06  202.4       0.70      74.37
4.34    225.34  .123E+07  301.8       0.77      67.68
4.73    225.73  .160E+07  433.5       0.85      61.37
5.13    226.13  .200E+07  602.2       0.95      55.29
5.52    226.52  .242E+07  804.7       1.05      50.13
5.92    226.92  .286E+07  1034.2      1.14      46.11
6.31    227.31  .332E+07  1290.5      1.22      42.88
6.71    227.71  .380E+07  1560.9      1.29      40.55
7.10    228.10  .432E+07  1835.6      1.34      39.19
-----

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-----
      AREA   QPEAK   TPEAK   R.V.   <-pipe / channel->
      (ha)   (cms)   (hrs)   (mm)   (m)   (m/s)
INFLOW : ID= 2 (8392) 2505.74 142.37 11.75 160.57 3.63 0.65
OUTFLOW: ID= 1 (6302) 2505.74 125.22 12.50 160.57 3.53 0.63
-----

```

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-----
| ADD HYD (8390) |
| 1 + 2 = 3 |
      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)   (hrs)   (mm)
ID1= 1 (0302): 473.90 22.555 12.00 147.79
+ ID2= 2 (6302): 2505.74 125.217 12.50 160.57
=====
ID = 3 (8390): 2979.64 146.953 12.50 158.53
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8348) |
| 1 + 2 = 3 |
      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)   (hrs)   (mm)
ID1= 1 (8388): 788.82 48.039 12.00 182.93
+ ID2= 2 (8390): 2979.64 146.953 12.50 158.53
=====
ID = 3 (8348): 3768.46 194.008 12.25 163.64
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB
| NASHYD (0304) | Area (ha)= 292.37 Curve Number (CN)= 80.3
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 2.78
-----

```

Unit Hyd Qpeak (cms) = 1.793

```

PEAK FLOW (cms) = 10.078 (i)
TIME TO PEAK (hrs) = 12.500
RUNOFF VOLUME (mm) = 158.751
TOTAL RAINFALL (mm) = 212.000
RUNOFF COEFFICIENT = 0.749

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB
| NASHYD (0280) | Area (ha)= 299.86 Curve Number (CN)= 92.3
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.85
-----

```

Unit Hyd Qpeak (cms) = 6.009

```

PEAK FLOW (cms) = 24.229 (i)
TIME TO PEAK (hrs) = 11.000
RUNOFF VOLUME (mm) = 185.349
TOTAL RAINFALL (mm) = 212.000
RUNOFF COEFFICIENT = 0.874

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB
| NASHYD (0278) | Area (ha)= 485.49 Curve Number (CN)= 92.3
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.52
-----

```

Unit Hyd Qpeak (cms) = 5.453

```

PEAK FLOW (cms) = 29.133 (i)
TIME TO PEAK (hrs) = 11.750
RUNOFF VOLUME (mm) = 186.750
TOTAL RAINFALL (mm) = 212.000
RUNOFF COEFFICIENT = 0.881

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB
| NASHYD (0276) | Area (ha)= 90.89 Curve Number (CN)= 90.9
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.67
-----

```

Unit Hyd Qpeak (cms) = 2.302

```

PEAK FLOW (cms) = 8.011 (i)
TIME TO PEAK (hrs) = 11.000
RUNOFF VOLUME (mm) = 180.981
TOTAL RAINFALL (mm) = 212.000
RUNOFF COEFFICIENT = 0.854

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB
| NASHYD (0274) | Area (ha)= 392.49 Curve Number (CN)= 88.3
-----

```

|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.08

Unit Hyd Qpeak (cms) = 6.182

PEAK FLOW (cms) = 27.372 (i)
 TIME TO PEAK (hrs) = 11.250
 RUNOFF VOLUME (mm) = 176.439
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.832

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | ADD HYD (8360) |
 | 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0274): 392.49 27.372 11.25 176.44
 + ID2= 2 (0276): 90.89 8.011 11.00 180.98
 =====
 ID = 3 (8360): 483.38 35.253 11.00 177.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | ADD HYD (8358) |
 | 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0278): 485.49 29.133 11.75 186.75
 + ID2= 2 (8360): 483.38 35.253 11.00 177.29
 =====
 ID = 3 (8358): 968.87 64.013 11.25 182.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | ROUTE CHN (6280) |
 | ID= 2--> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2801.0) ----->

Distance	Elevation	Manning
0.00	241.14	0.0500
13.32	240.80	0.0500
39.95	240.07	0.0500
96.54	236.21	0.0500
113.19	234.15	0.0500
123.18	232.35	0.0500
143.15	225.80	0.0500
149.81	225.62	0.0500
153.14	225.40	0.0500
157.30	224.76	0.0500 /0.0300 Main Channel
159.80	224.26	0.0300 Main Channel
162.30	224.85	0.0300 Main Channel
162.55	224.97	0.0300 /0.0500 Main Channel
167.80	225.05	0.0500
186.43	229.14	0.0500
213.06	234.75	0.0500
236.37	237.09	0.0500
266.33	237.31	0.0500
292.96	237.83	0.0500
329.58	241.50	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.50	224.76	.263E+04	0.8	0.65	58.31
1.36	225.62	.278E+05	13.4	1.10	34.54
2.22	226.48	.855E+05	52.6	1.40	27.09
3.09	227.35	.157E+06	120.4	1.74	21.78
3.95	228.21	.242E+06	216.3	2.04	18.64
4.81	229.07	.340E+06	341.8	2.29	16.56
5.67	229.93	.450E+06	498.3	2.52	15.06
6.53	230.79	.574E+06	688.3	2.73	13.90
7.40	231.66	.711E+06	913.8	2.93	12.97
8.26	232.52	.861E+06	1173.4	3.11	12.23
9.12	233.38	.103E+07	1461.3	3.24	11.72
9.98	234.24	.121E+07	1793.3	3.37	11.26
10.85	235.11	.142E+07	2141.0	3.45	11.02
11.71	235.97	.165E+07	2530.4	3.50	10.86
12.57	236.83	.192E+07	2954.0	3.51	10.81
13.43	237.69	.226E+07	3160.3	3.18	11.93
14.29	238.55	.271E+07	3869.4	3.26	11.67
15.16	239.42	.320E+07	4740.3	3.38	11.24
16.02	240.28	.373E+07	5688.4	3.48	10.92

<----- hydrograph ----> <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8358)	968.87	64.01	11.25	182.03	2.37	1.45
OUTFLOW: ID= 1 (6280)	968.87	62.71	11.75	182.03	2.35	1.44

 | ADD HYD (8354) |
 | 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0280): 299.86 24.228 11.00 185.35
 + ID2= 2 (6280): 968.87 62.714 11.75 182.03
 =====
 ID = 3 (8354): 1268.73 85.722 11.50 182.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | CALIB (0272) | Area (ha)= 157.38 Curve Number (CN)= 88.3
 | NASHVD (0272) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 | ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.09

Unit Hyd Qpeak (cms) = 2.456

PEAK FLOW (cms) = 10.925 (i)
 TIME TO PEAK (hrs) = 11.250
 RUNOFF VOLUME (mm) = 176.461
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.832

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | CALIB (0270) | Area (ha)= 243.61 Curve Number (CN)= 91.6
 | NASHVD (0270) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 | ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.87

Unit Hyd Qpeak (cms) = 3.429

PEAK FLOW (cms) = 16.128 (i)
 TIME TO PEAK (hrs) = 11.000
 RUNOFF VOLUME (mm) = 183.560
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.866

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | CALIB (0268) | Area (ha)= 215.76 Curve Number (CN)= 88.3
 | NASHVD (0268) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 | ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.69

Unit Hyd Qpeak (cms) = 3.807

PEAK FLOW (cms) = 15.631 (i)
 TIME TO PEAK (hrs) = 11.000
 RUNOFF VOLUME (mm) = 174.848
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.825

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | CALIB (0264) | Area (ha)= 353.96 Curve Number (CN)= 84.1
 | NASHVD (0264) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 | ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.30

Unit Hyd Qpeak (cms) = 3.313

PEAK FLOW (cms) = 17.065 (i)
 TIME TO PEAK (hrs) = 11.750
 RUNOFF VOLUME (mm) = 166.685
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.786

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | CALIB (0266) | Area (ha)= 508.09 Curve Number (CN)= 81.1
 | NASHVD (0266) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 | ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.63

Unit Hyd Qpeak (cms) = 3.786

PEAK FLOW (cms) = 20.326 (i)
 TIME TO PEAK (hrs) = 12.000
 RUNOFF VOLUME (mm) = 160.016

TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.755

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8380)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1 = 1 (0264):	353.96	17.065	11.75	166.69
+ ID2 = 2 (0265):	508.09	20.326	12.00	160.02
=====				
ID = 3 (8380):	862.05	37.389	12.00	162.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6268) |
 IN= 2----> OUT= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (2681.0) -----

Distance	Elevation	Manning	
0.00	274.03	0.0500	
10.55	272.73	0.0500	
21.10	271.28	0.0500	
50.12	266.83	0.0500	
87.04	260.36	0.0500	
92.32	260.06	0.0500	
94.95	259.93	0.0500	
101.00	256.87	0.0500	
103.00	256.58	0.0500 / 0.0350	Main Channel
104.00	256.32	0.0350	Main Channel
106.00	256.05	0.0350	Main Channel
108.00	256.25	0.0350	Main Channel
110.78	256.65	0.0350 / 0.0500	Main Channel
113.42	256.81	0.0500	
116.06	257.00	0.0500	
155.62	260.24	0.0500	
187.27	263.35	0.0500	
211.01	267.22	0.0500	
224.20	269.60	0.0500	
261.13	272.68	0.0500	

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.53	256.58	697E+04	2.1	0.97	55.64
1.38	257.43	477E+05	25.9	1.76	30.66
2.22	258.27	123E+06	84.3	2.22	24.26
3.07	259.12	231E+06	185.4	2.60	20.72
3.92	259.97	371E+06	335.8	2.92	18.42
4.77	260.82	560E+06	547.6	3.15	17.06
5.61	261.66	790E+06	845.6	3.46	15.57
6.46	262.51	106E+07	1225.0	3.75	14.37
7.31	263.36	136E+07	1693.3	4.02	13.37
8.16	264.21	169E+07	2283.2	4.35	12.36
9.00	265.05	206E+07	2969.1	4.66	11.54
9.85	265.90	245E+07	3754.4	4.96	10.86
10.70	266.75	286E+07	4643.0	5.24	10.28
11.55	267.60	331E+07	5636.1	5.50	9.78
12.39	268.44	378E+07	6746.2	5.76	9.34
13.24	269.29	428E+07	7972.2	6.01	8.96
14.09	270.14	482E+07	9218.9	6.18	8.71
14.94	270.99	539E+07	10550.5	6.32	8.52
15.78	271.83	601E+07	12024.7	6.46	8.33

INFLOW : ID= 2 (8380)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW: ID= 1 (6268)	862.05	36.80	12.25	162.75	1.53	1.82

ADD HYD (8382)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1 = 1 (0268):	215.76	15.631	11.00	174.85
+ ID2 = 2 (6268):	862.05	36.804	12.25	162.75
=====				
ID = 3 (8382):	1077.81	50.727	12.00	165.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6270) |
 IN= 2----> OUT= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (2701.0) -----

Distance	Elevation	Manning	
0.00	245.89	0.0500	
8.14	245.66	0.0500	
16.28	245.16	0.0500	
20.35	244.84	0.0500	
38.66	242.98	0.0500	
48.83	240.65	0.0500	
63.07	235.91	0.0500	
65.11	235.18	0.0500	
68.43	234.34	0.0500 / 0.0300	Main Channel
68.68	233.95	0.0300	Main Channel
69.18	233.89	0.0300	Main Channel
69.68	233.95	0.0300	Main Channel
71.21	234.48	0.0300 / 0.0500	Main Channel
81.38	236.44	0.0500	
95.63	236.66	0.0500	
120.04	237.00	0.0500	
148.53	241.77	0.0500	
158.70	242.34	0.0500	
187.18	244.03	0.0500	
201.42	244.36	0.0500	

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.45	234.34	217E+04	0.7	0.99	52.83
0.98	234.87	966E+04	4.6	1.49	35.20
1.50	235.39	250E+05	13.7	1.73	30.41
2.03	235.92	475E+05	29.5	1.95	26.87
2.56	236.45	772E+05	52.5	2.14	24.50
3.09	236.98	142E+06	82.4	1.83	28.73
3.61	237.50	246E+06	158.1	2.03	25.91
4.14	238.03	357E+06	262.0	2.31	22.73
4.67	238.56	477E+06	392.1	2.59	20.27
5.20	239.09	604E+06	547.8	2.86	18.38
5.72	239.61	739E+06	729.2	3.11	16.90
6.25	240.14	882E+06	936.5	3.34	15.70
6.78	240.67	103E+07	1169.7	3.57	14.72
7.31	241.20	119E+07	1425.9	3.77	13.94
7.83	241.72	136E+07	1710.4	3.96	13.26
8.36	242.25	154E+07	1967.9	4.02	13.07
8.89	242.78	174E+07	2264.9	4.09	12.83
9.42	243.31	196E+07	2589.3	4.15	12.65
9.94	243.83	221E+07	2953.5	4.22	12.46

INFLOW : ID= 2 (8382)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW: ID= 1 (6270)	1077.81	50.73	12.00	165.18	2.52	2.13
		50.21	12.25	165.18	2.50	2.12

ADD HYD (8384)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1 = 1 (0270):	243.61	16.128	11.00	183.56
+ ID2 = 2 (6270):	1077.81	50.214	12.25	165.18
=====				
ID = 3 (8384):	1321.42	65.392	12.00	168.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHVD (0262)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	341.31	92.3
	5.00	# of Linear Res. (N)= 1.30
	U.H. Tp(hrs)= 1.01	
Unit Hyd Qpeak (cms)=	4.094	
PEAK FLOW (cms)=	20.776 (i)	
TIME TO PEAK (hrs)=	11.250	
RUNOFF VOLUME (mm)=	185.722	
TOTAL RAINFALL (mm)=	212.000	
RUNOFF COEFFICIENT =	0.876	

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHVD (0260)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	476.24	92.3
	5.00	# of Linear Res. (N)= 1.30
	U.H. Tp(hrs)= 1.33	
Unit Hyd Qpeak (cms)=	4.360	
PEAK FLOW (cms)=	24.646 (i)	
TIME TO PEAK (hrs)=	11.750	
RUNOFF VOLUME (mm)=	186.327	
TOTAL RAINFALL (mm)=	212.000	

RUNOFF COEFFICIENT = 0.879

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

207.28	271.95	0.0400
233.55	273.72	0.0400
256.91	274.98	0.0400
289.02	275.97	0.0400

CALIB (0258) | Area (ha)= 181.99 | Curve Number (CN)= 91.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.18

Unit Hyd Qpeak (cms)= 1.881

PEAK FLOW (cms)= 10.024 (i)
 TIME TO PEAK (hrs)= 11.500
 RUNOFF VOLUME (mm)= 182.929
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.863

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0252) | Area (ha)= 319.99 | Curve Number (CN)= 86.9
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.04

Unit Hyd Qpeak (cms)= 3.761

PEAK FLOW (cms)= 18.292 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 172.824
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.815

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0256) | Area (ha)= 145.79 | Curve Number (CN)= 82.9
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.04

Unit Hyd Qpeak (cms)= 1.707

PEAK FLOW (cms)= 7.959 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 163.436
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.771

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0254) | Area (ha)= 403.00 | Curve Number (CN)= 74.4
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.22

Unit Hyd Qpeak (cms)= 4.028

PEAK FLOW (cms)= 17.953 (i)
 TIME TO PEAK (hrs)= 11.750
 RUNOFF VOLUME (mm)= 144.288
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.681

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6256) | Routing time step (min)= 15.00
 IN= 2--> OUT= 1

----- DATA FOR SECTION (2561.0) -----

Distance	Elevation	Manning
0.00	276.07	0.0400
11.68	273.71	0.0400
23.36	271.35	0.0400
29.19	270.30	0.0400
35.03	269.44	0.0400
55.47	267.90	0.0400
78.82	266.24	0.0400
90.50	265.63	0.0400
102.18	265.40	0.0400
105.10	264.95	0.0400 / 0.0350 Main Channel
108.02	264.39	0.0350 Main Channel
110.94	264.72	0.0350 Main Channel
113.86	265.19	0.0350 / 0.0400 Main Channel
116.78	265.49	0.0400
143.05	268.24	0.0400
172.25	270.53	0.0400

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.56	264.95	723E+04	1.9	0.90	62.51
1.14	265.53	307E+05	13.5	1.49	37.92
1.72	266.11	951E+05	48.6	1.73	32.62
2.30	266.69	191E+06	117.8	2.09	27.03
2.88	267.27	314E+06	225.2	2.43	23.24
3.46	267.85	464E+06	375.5	2.74	20.59
4.04	268.43	641E+06	573.2	3.03	18.62
4.62	269.01	846E+06	821.9	3.29	17.15
5.20	269.59	108E+07	1135.0	3.56	15.87
5.78	270.17	134E+07	1525.1	3.86	14.64
6.36	270.75	162E+07	1971.1	4.12	13.71
6.94	271.33	194E+07	2474.0	4.33	13.04
7.52	271.91	228E+07	3065.0	4.55	12.42
8.10	272.49	266E+07	3773.4	4.80	11.75
8.68	273.07	306E+07	4566.3	5.05	11.17
9.26	273.65	348E+07	5443.7	5.30	10.66
9.84	274.23	393E+07	6389.6	5.51	10.25
10.42	274.81	440E+07	7426.2	5.72	9.88
11.00	275.39	490E+07	8490.1	5.86	9.63

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0254)	403.00	17.95	11.75	144.29	1.21
OUTFLOW : ID= 1 (6256)	403.00	17.53	12.25	144.29	1.20

ADD HYD (8370)

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (0256):	145.79	7.959	11.25	163.44
+ ID= 2 (6256):	403.00	17.537	12.25	144.29
ID= 3 (8370):	548.79	25.263	12.00	149.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0250) | Area (ha)= 192.88 | Curve Number (CN)= 85.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.22

Unit Hyd Qpeak (cms)= 1.930

PEAK FLOW (cms)= 9.790 (i)
 TIME TO PEAK (hrs)= 11.750
 RUNOFF VOLUME (mm)= 168.680
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.796

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0246) | Area (ha)= 759.61 | Curve Number (CN)= 74.2
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.81

Unit Hyd Qpeak (cms)= 5.121

PEAK FLOW (cms)= 25.684 (i)
 TIME TO PEAK (hrs)= 12.000
 RUNOFF VOLUME (mm)= 144.339
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.681

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0248) | Area (ha)= 146.04 | Curve Number (CN)= 81.1
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 0.78

Unit Hyd Qpeak (cms)= 2.271

PEAK FLOW (cms)= 9.220 (i)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 158.507
 TOTAL RAINFALL (mm)= 212.000

RUNOFF COEFFICIENT = 0.748

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8364)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0246):	759.61	25.684	12.00	144.34
+ ID2= 2 (0248):	146.04	9.220	11.00	158.51
ID = 3 (8364):	905.65	34.343	12.00	146.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6250) | IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

Distance	Elevation	Manning
0.00	269.96	0.0500
8.56	268.55	0.0500
17.13	266.91	0.0500
21.41	266.13	0.0500
40.68	263.15	0.0500
62.09	260.75	0.0500
85.64	258.02	0.0500
88.00	257.69	0.0500
93.20	257.05	0.0500 / 0.0350
93.45	256.88	0.0350
94.20	256.56	0.0350
94.95	256.83	0.0350
95.20	257.08	0.0350 / 0.0500
100.62	257.45	0.0500
115.61	258.57	0.0500
139.16	260.43	0.0500
152.01	261.95	0.0500
171.27	264.63	0.0500
188.40	267.90	0.0500
211.95	274.18	0.0500

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.49	257.05	.205E+04	0.4	0.68	89.65
1.17	257.73	.248E+05	5.7	0.85	72.16
1.85	258.41	.841E+05	25.2	1.10	55.60
2.53	259.09	.180E+06	66.5	1.36	45.14
3.21	259.77	.312E+06	135.7	1.60	38.36
3.89	260.45	.481E+06	236.3	1.82	33.61
4.57	261.13	.681E+06	383.5	2.07	29.60
5.25	261.81	.911E+06	568.9	2.29	26.70
5.93	262.49	.117E+07	800.5	2.51	24.36
6.61	263.17	.146E+07	1079.0	2.72	22.49
7.28	263.84	.177E+07	1413.4	2.94	20.85
7.96	264.52	.210E+07	1797.0	3.14	19.50
8.64	265.20	.246E+07	2241.6	3.35	18.28
9.32	265.88	.283E+07	2739.9	3.55	17.25
10.00	266.56	.323E+07	3295.7	3.75	16.34
10.68	267.24	.364E+07	3908.9	3.94	15.54
11.36	267.92	.408E+07	4578.5	4.12	14.84
12.04	268.60	.453E+07	5320.1	4.32	14.18
12.72	269.28	.499E+07	6109.1	4.50	13.61

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (8364)	905.65	34.34	12.00	146.62	2.00	1.15
OUTFLOW : ID= 1 (6250)	905.65	32.61	12.50	146.62	1.97	1.14

ADD HYD (8366)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0250):	192.88	9.790	11.75	168.68
+ ID2= 2 (6250):	905.65	32.614	12.50	146.62
ID = 3 (8366):	1098.53	41.960	12.25	150.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8368)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8366):	1098.53	41.960	12.25	150.50

+ ID2= 2 (8370): 548.79 25.263 12.00 149.37
ID = 3 (8368): 1647.32 67.068 12.25 150.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8372)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0252):	319.99	18.292	11.25	172.82
+ ID2= 2 (8368):	1647.32	67.068	12.25	150.12
ID = 3 (8372):	1967.31	84.865	12.00	153.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6258) | IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

Distance	Elevation	Manning
0.00	252.88	0.0500
15.47	251.89	0.0500
46.41	248.45	0.0500
73.48	245.81	0.0500
88.95	243.75	0.0500
112.15	242.00	0.0500
135.35	240.23	0.0500
162.42	239.76	0.0500
170.97	239.52	0.0500 / 0.0350
171.58	239.03	0.0350
174.02	239.03	0.0350
176.46	239.03	0.0350
177.07	239.52	0.0350 / 0.0500
185.63	239.67	0.0500
208.83	239.87	0.0500
239.77	240.14	0.0500
274.57	244.93	0.0500
336.45	249.51	0.0500
363.52	249.77	0.0500
382.85	251.78	0.0500

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.49	239.52	.135E+05	2.5	0.92	91.32
1.14	240.17	.175E+06	25.6	0.73	114.22
1.78	240.81	.532E+06	123.5	1.17	71.85
2.43	241.46	.932E+06	285.3	1.54	54.48
3.07	242.10	.138E+07	506.5	1.85	45.25
3.72	242.75	.186E+07	786.8	2.12	39.42
4.36	243.39	.239E+07	1127.3	2.37	35.33
5.01	244.04	.296E+07	1535.8	2.61	32.12
5.65	244.68	.356E+07	2014.3	2.84	29.48
6.30	245.33	.420E+07	2535.9	3.03	27.60
6.94	245.97	.488E+07	3108.4	3.20	26.16
7.59	246.62	.561E+07	3749.6	3.35	24.98
8.23	247.26	.638E+07	4446.7	3.50	23.93
8.88	247.91	.721E+07	5229.7	3.64	22.98
9.52	248.55	.809E+07	6094.3	3.78	22.12
10.17	249.20	.901E+07	7050.4	3.93	21.31
10.81	249.84	.100E+08	7773.0	3.90	21.46
11.46	250.49	.111E+08	8998.8	4.07	20.56
12.10	251.13	.122E+08	10318.2	4.24	19.75

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (8372)	1967.31	84.87	12.00	153.82	1.53	0.95
OUTFLOW : ID= 1 (6258)	1967.31	75.75	13.25	153.82	1.46	0.90

ADD HYD (8374)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0258):	181.99	10.024	11.50	182.93
+ ID2= 2 (6258):	1967.31	75.748	13.25	153.82
ID = 3 (8374):	2149.30	84.408	13.00	156.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8376)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				

	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0260):	476.24	24.646	11.75	186.33
+ ID2= 2 (8374):	2149.30	84.408	13.00	156.28
=====				
ID = 3 (8376):	2625.54	106.930	12.75	161.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8378)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0262):	341.31	20.776	11.25	185.72
+ ID2= 2 (8376):	2625.54	106.930	12.75	161.73
=====				
ID = 3 (8378):	2966.85	125.359	12.50	164.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8362)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8378):	2966.85	125.359	12.50	164.49
+ ID2= 2 (8384):	1321.42	65.392	12.00	168.56
=====				
ID = 3 (8362):	4288.27	189.854	12.25	165.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6272)		Routing time step (min) = 15.00
IN= 2-->	OUT= 1	

----- DATA FOR SECTION (2721.0) -----

Distance	Elevation	Manning	
0.00	231.01	0.0450	
23.01	223.65	0.0450	
34.51	222.46	0.0450	
51.77	222.11	0.0450	
69.02	221.87	0.0450	
161.06	221.92	0.0450	
166.81	221.91	0.0450	
172.56	221.89	0.0450	
180.57	221.40	0.0450 / 0.0300	Main Channel
181.57	220.60	0.0300	Main Channel
184.07	220.16	0.0300	Main Channel
195.57	221.85	0.0300 / 0.0450	Main Channel
201.32	221.82	0.0450	
207.07	221.77	0.0450	
212.83	221.72	0.0450	
253.09	222.52	0.0450	
408.40	222.65	0.0450	
460.17	223.20	0.0450	
517.69	224.84	0.0450	
569.46	232.57	0.0450	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.41	220.57	189E+04	0.5	0.49	60.18
0.83	220.99	697E+04	3.2	0.82	35.79
1.24	221.40	145E+05	8.8	1.07	27.45
1.84	222.00	643E+05	29.3	0.81	36.61
2.44	222.60	282E+06	132.9	0.83	35.41
3.04	223.20	707E+06	413.7	1.04	28.50
3.64	223.80	118E+07	876.8	1.31	22.46
4.24	224.40	168E+07	1494.8	1.57	18.75
4.84	225.00	221E+07	2264.1	1.82	16.23
5.44	225.60	274E+07	3196.3	2.06	14.29
6.04	226.20	328E+07	4258.6	2.30	12.84
6.65	226.81	383E+07	5444.7	2.52	11.72
7.25	227.41	438E+07	6749.7	2.73	10.82
7.85	228.01	494E+07	8169.4	2.93	10.08
8.45	228.61	551E+07	9700.7	3.12	9.46
9.05	229.21	608E+07	11340.8	3.30	8.94
9.65	229.81	666E+07	13087.4	3.48	8.48
10.25	230.41	724E+07	14938.5	3.65	8.08
10.85	231.01	783E+07	16892.4	3.82	7.73

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8362)	4288.27	189.85	12.25	165.75	2.56
OUTFLOW : ID= 1 (6272)	4288.27	185.57	12.75	165.75	2.55

ADD HYD (8356)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0272):	357.38	10.925	11.25	176.46
+ ID2= 2 (6272):	4288.27	185.571	12.75	165.75
=====				
ID = 3 (8356):	4445.65	194.398	12.75	166.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8352)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8354):	1268.73	85.722	11.50	182.82
+ ID2= 2 (8356):	4445.65	194.398	12.75	166.12
=====				
ID = 3 (8352):	5714.38	272.950	12.25	169.83

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6304)		Routing time step (min) = 15.00
IN= 2-->	OUT= 1	

----- DATA FOR SECTION (3041.0) -----

Distance	Elevation	Manning	
0.00	232.08	0.0500	
19.00	231.87	0.0500	
38.00	231.33	0.0500	
66.51	230.44	0.0500	
104.51	228.25	0.0500	
118.76	225.17	0.0500	
128.26	219.86	0.0500	
175.77	219.17	0.0500	
185.27	218.90	0.0500 / 0.0300	Main Channel
185.52	218.65	0.0300	Main Channel
190.02	218.37	0.0300	Main Channel
194.52	218.60	0.0300	Main Channel
194.77	218.85	0.0300 / 0.0500	Main Channel
204.27	219.60	0.0500	
299.28	220.91	0.0500	
327.78	222.36	0.0500	
375.28	225.71	0.0500	
403.79	229.37	0.0500	
432.29	230.43	0.0500	
470.29	232.00	0.0500	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.48	218.85	121E+05	1.2	0.36	173.94
1.17	219.54	839E+05	9.8	0.44	142.60
1.86	220.23	323E+06	41.0	0.47	131.43
2.56	220.93	708E+06	109.5	0.58	107.67
3.25	221.62	118E+07	224.3	0.71	87.36
3.94	222.31	168E+07	376.5	0.84	74.48
4.63	223.00	222E+07	567.9	0.96	65.26
5.32	223.69	279E+07	794.7	1.07	58.58
6.02	224.39	339E+07	1056.2	1.17	53.52
6.71	225.08	402E+07	1352.4	1.26	49.52
7.40	225.77	468E+07	1677.3	1.34	46.46
8.09	226.46	536E+07	2050.0	1.43	43.58
8.79	227.16	607E+07	2457.9	1.52	41.14
9.48	227.85	680E+07	2901.0	1.60	39.05
10.17	228.54	755E+07	3352.7	1.66	37.53
10.86	229.23	834E+07	3812.5	1.71	36.48
11.55	229.92	919E+07	4241.8	1.73	36.13
12.25	230.62	1018E+08	4697.0	1.74	35.93
12.94	231.31	1118E+08	5186.3	1.74	35.83

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8352)	5714.38	272.95	12.25	169.83	3.47
OUTFLOW : ID= 1 (6304)	5714.38	244.13	13.25	169.83	3.33

ADD HYD (8350)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0304):	292.37	10.078	12.50	158.75
+ ID2= 2 (6304):	5714.38	244.133	13.25	169.83
=====				
ID = 3 (8350):	6006.75	254.018	13.25	169.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| ADD HYD (8346) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
-----
ID1= 1 (8348): 3768.46 194.008 12.25 163.64
+ ID2= 2 (8350): 6006.75 254.018 13.25 169.29
-----
ID = 3 (8346): 9775.21 442.837 12.75 167.11
-----

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| ADD HYD (8344) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
-----
ID1= 1 (0306): 283.97 4.007 15.75 138.25
+ ID2= 2 (8346): 9775.21 442.837 12.75 167.11
-----
ID = 3 (8344): 10059.18 446.536 12.75 166.30
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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| RESERVOIR (5509) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
| OUTFLOW | STORAGE | OUTFLOW | STORAGE |
| (cms) | (ha.m.) | (cms) | (ha.m.) |
-----
| 0.0000 | 0.0000 | ***** | 370.0451 |
| 41.0590 | 56.7403 | ***** | 863.4386 |
| 48.1390 | 86.3439 | ***** | ***** |
| 56.6340 | ***** | ***** | ***** |
| 67.9600 | ***** | ***** | 0.0000 |
| 0.0000 | 0.0000 | ***** | 370.0451 |
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-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
-----
INFLOW : ID= 2 (8344) ***** 446.536 12.75 166.30
OUTFLOW : ID= 1 (5509) ***** 122.099 21.25 166.30
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PEAK FLOW REDUCTION [Qout/Qin](%)= 27.34
TIME SHIFT OF PEAK FLOW (min)=510.00
MAXIMUM STORAGE USED (ha.m.)=909.2796
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-----
| CALIB |
| NASHYD (0332) | Area (ha)= 393.44 Curve Number (CN)= 68.4
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 2.32
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Unit Hyd Qpeak (cms)= 2.894
PEAK FLOW (cms)= 17.291 (i)
TIME TO PEAK (hrs)= 12.000
RUNOFF VOLUME (mm)= 177.773
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.839
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(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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| CALIB |
| NASHYD (0330) | Area (ha)= 468.30 Curve Number (CN)= 91.4
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.09
-----

```

```

Unit Hyd Qpeak (cms)= 7.335
PEAK FLOW (cms)= 33.391 (i)
TIME TO PEAK (hrs)= 11.250
RUNOFF VOLUME (mm)= 183.908
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.867
-----

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ROUTE CHN (6332) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
-----

```

```

<----- DATA FOR SECTION (3321.0) ----->
Distance Elevation Manning
0.00 234.00 0.0380
25.85 227.20 0.0380
96.94 226.44 0.0380
168.03 227.38 0.0380
219.73 225.62 0.0380
342.52 221.57 0.0380
368.37 221.42 0.0380
374.83 221.23 0.0380
-----

```

```

379.79 220.98 0.0380 /0.0380 Main Channel
380.29 220.47 0.0300 Main Channel
381.29 220.47 0.0300 Main Channel
382.79 220.47 0.0300 Main Channel
383.29 220.98 0.0300 /0.0380 Main Channel
394.22 221.22 0.0380
400.68 221.33 0.0380
407.14 221.44 0.0380
491.16 225.70 0.0380
568.71 227.55 0.0380
607.49 230.14 0.0380
639.80 234.08 0.0380
-----

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.51 220.98 554E+04 0.9 0.56 107.27
1.20 221.67 885E+05 12.1 0.49 122.32
1.88 222.35 309E+06 64.7 0.76 79.70
2.57 223.04 615E+06 165.4 0.97 61.99
3.25 223.72 101E+07 322.1 1.16 52.06
3.94 224.41 148E+07 542.9 1.33 45.51
4.62 225.09 204E+07 835.4 1.48 40.77
5.31 225.78 269E+07 1202.8 1.62 37.27
5.99 226.46 344E+07 1618.0 1.70 35.46
6.68 227.15 447E+07 1932.0 1.56 38.56
7.36 227.83 580E+07 2763.9 1.72 34.96
8.05 228.52 718E+07 3861.9 1.95 30.97
8.73 229.20 859E+07 5115.4 2.16 27.98
9.42 229.89 100E+08 6519.2 2.35 25.64
10.10 230.57 115E+08 8092.3 2.55 23.69
10.79 231.26 130E+08 9821.5 2.73 22.05
11.47 231.94 145E+08 11690.0 2.92 20.68
12.16 232.63 160E+08 13694.9 3.09 19.52
12.84 233.31 176E+08 15833.9 3.26 18.52
-----

```

```

<----- hydrograph -----> <-pipe / channel->
| AREA | QPEAK | TPEAK | R.V. | MAX DEPTH | MAX VEL |
| (ha) | (cms) | (hrs) | (mm) | (m) | (m/s) |
-----
INFLOW : ID= 2 (0330) 468.30 33.39 11.25 183.91 1.47 0.57
OUTFLOW : ID= 1 (6332) 468.30 27.64 12.50 183.91 1.40 0.55
-----

```

```

-----
| ADD HYD (8322) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
-----
ID1= 1 (0332): 393.44 17.291 12.00 177.77
+ ID2= 2 (6332): 468.30 27.640 12.50 183.91
-----
ID = 3 (8322): 861.74 44.899 12.25 181.11
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0328) | Area (ha)= 492.92 Curve Number (CN)= 89.1
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.91
-----

```

```

Unit Hyd Qpeak (cms)= 4.411
PEAK FLOW (cms)= 25.084 (i)
TIME TO PEAK (hrs)= 12.000
RUNOFF VOLUME (mm)= 179.281
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.846
-----

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0326) | Area (ha)= 678.91 Curve Number (CN)= 91.3
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.95
-----

```

```

Unit Hyd Qpeak (cms)= 5.941
PEAK FLOW (cms)= 34.798 (i)
TIME TO PEAK (hrs)= 12.000
RUNOFF VOLUME (mm)= 184.631
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.871
-----

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0324) | Area (ha)= 615.64 Curve Number (CN)= 91.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
-----

```

U.H. Tp(hrs)= 1.90

Unit Hyd Qpeak (cms)= 5.544

PEAK FLOW (cms)= 32.052 (i)
TIME TO PEAK (hrs)= 12.000
RUNOFF VOLUME (mm)= 183.871
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.867

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (8320) Area (ha)= 278.74 Curve Number (CN)= 92.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.36

Unit Hyd Qpeak (cms)= 3.499

PEAK FLOW (cms)= 17.763 (i)
TIME TO PEAK (hrs)= 11.500
RUNOFF VOLUME (mm)= 185.834
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.877

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (3182) Area (ha)= 457.40 Curve Number (CN)= 90.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.49

Unit Hyd Qpeak (cms)= 5.259

PEAK FLOW (cms)= 27.258 (i)
TIME TO PEAK (hrs)= 11.750
RUNOFF VOLUME (mm)= 181.131
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.854

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANHYD (3181) Area (ha)= 26.00
ID= 1 DT=15.0 min Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 21.06 4.94
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 416.33 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten.(mm/hr)= 53.00 51.40
over (min)= 15.00 30.00
Storage Coeff. (min)= 9.54 (ii) 23.50 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.09 0.04

TOTALS

PEAK FLOW (cms)= 3.10 0.64 3.733 (iii)
TIME TO PEAK (hrs)= 10.00 10.00
RUNOFF VOLUME (mm)= 210.00 179.74 204.25
TOTAL RAINFALL (mm)= 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.85 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
TO THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8334)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (3181): 26.00 3.733 10.00 204.25
+ ID2= 2 (3182): 457.40 27.258 11.75 181.13
=====
ID = 3 (8334): 483.40 29.279 11.00 182.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6320)

IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

Table with 3 columns: Distance, Elevation, Manning. Rows show data for section 3201.0 with values ranging from 0.00 to 248.60.

TRAVEL TIME TABLE

Table with 6 columns: DEPTH (m), ELEV (m), VOLUME (cu.m.), FLOW RATE (cms), VELOCITY (m/s), TRAV. TIME (min). Rows show data points for the travel time table.

hydrograph table with columns: AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm), MAX DEPTH (m), MAX VEL (m/s). Rows show INFLOW and OUTFLOW data for IDs 2 and 1.

ADD HYD (8332)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8320): 278.74 17.763 11.50 185.83
+ ID2= 2 (6320): 483.40 27.435 12.00 182.37
=====
ID = 3 (8332): 762.14 44.940 12.00 183.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHYD (0314) Area (ha)= 165.20 Curve Number (CN)= 89.9
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.71

Unit Hyd Qpeak (cms)= 3.978

PEAK FLOW (cms)= 14.198 (i)
TIME TO PEAK (hrs)= 11.000
RUNOFF VOLUME (mm)= 178.932
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.844

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6316)
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

Table with 3 columns: Distance, Elevation, Manning. Rows show data for section 3161.0 with values ranging from 0.00 to 41.89.

62.83	243.09	0.0350
132.65	239.00	0.0350
188.50	236.74	0.0350
195.48	236.54	0.0350
202.46	236.32	0.0350
205.48	236.14	0.0350 / 0.0310
205.98	235.61	0.0310
207.98	235.25	0.0310
209.98	235.53	0.0310
210.48	236.00	0.0310 / 0.0350
216.42	236.73	0.0350
258.31	239.09	0.0350
328.12	239.84	0.0350
439.83	241.47	0.0350
530.58	242.08	0.0350
586.43	242.93	0.0350
691.16	248.00	0.0350

----- TRAVEL TIME TABLE -----

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.37	235.62	.215E+04	0.7	0.77	53.88
0.75	236.00	.634E+04	3.5	1.36	30.45
1.46	236.71	.297E+05	19.3	1.62	25.66
2.16	237.41	.103E+06	75.8	1.84	22.60
2.87	238.12	.229E+06	202.8	2.21	18.79
3.57	238.82	.407E+06	423.3	2.59	16.03
4.28	239.53	.654E+06	727.7	2.77	14.97
4.99	240.24	.103E+07	1242.0	3.01	13.76
5.69	240.94	.151E+07	2002.3	3.31	12.54
6.40	241.65	.210E+07	2987.9	3.55	11.70
7.10	242.35	.286E+07	4303.8	3.75	11.07
7.81	243.06	.374E+07	6165.3	4.10	10.11
8.51	243.76	.468E+07	8642.4	4.60	9.03
9.22	244.47	.565E+07	11496.3	5.06	8.20
9.93	245.18	.666E+07	14714.1	5.50	7.54
10.63	245.88	.770E+07	18290.3	5.91	7.02
11.34	246.59	.877E+07	22221.9	6.30	6.58
12.04	247.29	.988E+07	26464.4	6.67	6.22
12.75	248.00	.110E+08	31061.3	7.01	5.91

----- hydrograph ----- <-pipe / channel-->

	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (0314)	165.20	14.20	11.00	178.83	1.23	1.52
OUTFLOW : ID= 1 (6316)	165.20	13.54	11.25	178.83	1.20	1.51

----- CALIB -----

NASHVD (0316)	Area (ha)= 232.34	Curve Number (CN)= 92.3
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.01		
Unit Hyd Qpeak (cms)= 3.923		
PEAK FLOW (cms)= 17.290 (i)		
TIME TO PEAK (hrs)= 11.000		
RUNOFF VOLUME (mm)= 185.893		
TOTAL RAINFALL (mm)= 212.000		
RUNOFF COEFFICIENT = 0.877		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- ADD HYD (8338) -----

1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID= 1 (0316):	232.34	17.290	11.00	185.89
+ ID= 2 (6316):	165.20	13.544	11.25	178.83
ID = 3 (8338):	397.54	30.818	11.25	182.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- CALIB -----

NASHVD (0312)	Area (ha)= 359.44	Curve Number (CN)= 91.1
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.36		
Unit Hyd Qpeak (cms)= 4.529		
PEAK FLOW (cms)= 22.783 (i)		
TIME TO PEAK (hrs)= 11.500		
RUNOFF VOLUME (mm)= 183.641		
TOTAL RAINFALL (mm)= 212.000		
RUNOFF COEFFICIENT = 0.866		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB -----

NASHVD (0308)	Area (ha)= 529.30	Curve Number (CN)= 79.2
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.62		

Unit Hyd Qpeak (cms)= 5.575

PEAK FLOW (cms)= 26.714 (i)
TIME TO PEAK (hrs)= 12.000
RUNOFF VOLUME (mm)= 155.772
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.735

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB -----

NASHVD (0310)	Area (ha)= 138.28	Curve Number (CN)= 81.9
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.76		

Unit Hyd Qpeak (cms)= 3.102

PEAK FLOW (cms)= 10.810 (i)
TIME TO PEAK (hrs)= 12.000
RUNOFF VOLUME (mm)= 160.351
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.756

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- ADD HYD (8342) -----

1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID= 1 (0308):	529.30	26.714	12.00	155.77		
+ ID= 2 (0310):	138.28	10.810	11.00	160.35		
ID = 3 (8342):	667.58	36.681	11.50	156.72		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ROUTE CWN (6312) -----

Routing time step (min)= 15.00

----- DATA FOR SECTION (3121.0) -----

Distance	Elevation	Manning
0.00	265.94	0.0360
38.07	265.43	0.0360
59.82	263.98	0.0360
103.32	254.59	0.0360
157.70	252.16	0.0360
217.52	250.45	0.0360
233.84	247.69	0.0360
234.71	247.27	0.0360
239.71	246.38	0.0360
244.71	246.12	0.0360 / 0.0330
247.71	245.17	0.0330
249.71	245.19	0.0330
251.71	245.64	0.0330 / 0.0360
259.71	246.67	0.0360
282.78	247.12	0.0360
315.41	251.60	0.0360
424.17	256.13	0.0360
478.55	257.04	0.0360
516.62	259.37	0.0360
538.37	266.00	0.0360

----- TRAVEL TIME TABLE -----

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.47	245.64	.111E+04	3.1	1.78	6.07
1.54	246.71	.103E+05	53.9	3.39	3.19
2.61	247.78	.414E+05	281.3	4.40	2.45
3.68	248.85	.839E+05	749.1	5.79	1.87
4.74	249.91	.136E+06	1463.2	6.97	1.55
5.81	250.98	.201E+06	2354.8	7.61	1.42
6.88	252.05	.295E+06	3533.0	7.76	1.39
7.95	253.12	.427E+06	5303.9	8.05	1.34
9.02	254.19	.594E+06	7856.5	8.58	1.26
10.09	255.26	.792E+06	11449.2	9.37	1.15
11.15	256.32	.101E+07	15862.4	10.15	1.06
12.22	257.39	.127E+07	21074.5	10.75	1.01
13.29	258.46	.155E+07	28184.1	11.78	0.92
14.36	259.53	.185E+07	36386.2	12.77	0.85
15.43	260.60	.215E+07	46110.6	13.89	0.78
16.50	261.67	.246E+07	56822.5	14.95	0.72
17.56	262.73	.278E+07	68498.1	15.97	0.68

18.63 263.80 .310E+07 81118.8 16.94 0.64
19.70 264.87 .344E+07 93603.2 17.66 0.61

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8342) 667.58 36.68 11.50 156.72 1.18 2.60
OUTFLOW: ID= 1 (6312) 667.58 36.77 11.25 156.72 1.18 2.60

| ADD HYD (8340) |
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0312): 358.44 22.783 11.50 183.64
+ ID2= 2 (6312): 667.58 36.772 11.25 156.72
=====

ID = 3 (8340): 1027.02 59.491 11.25 166.14
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ADD HYD (8336) |
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8338): 397.54 30.818 11.25 182.96
+ ID2= 2 (8340): 1027.02 59.491 11.25 166.14
=====

ID = 3 (8336): 1424.56 90.309 11.25 170.84
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ADD HYD (8330) |
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8332): 762.14 44.940 12.00 183.64
+ ID2= 2 (8336): 1424.56 90.309 11.25 170.84
=====

ID = 3 (8330): 2186.70 134.425 11.50 175.30
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ROUTE CHN (6324) |
IN= 2 -> OUT= 1
Routing time step (min) = 15.00

<----- DATA FOR SECTION (3241.0) ----->
Distance Elevation Manning
0.00 234.38 0.0360
33.57 232.48 0.0360
67.14 230.14 0.0360
83.93 228.80 0.0360
134.29 227.62 0.0360
209.82 225.10 0.0360
218.21 224.86 0.0360
226.61 224.47 0.0360
234.00 223.86 0.0360 / 0.0300 Main Channel
234.10 223.66 0.0300 Main Channel
235.00 223.66 0.0300 Main Channel
235.90 223.66 0.0300 Main Channel
236.00 223.86 0.0300 / 0.0360 Main Channel
243.39 224.92 0.0360
251.78 224.89 0.0360
335.71 225.64 0.0360
562.32 226.53 0.0360
637.85 228.36 0.0360
705.00 229.80 0.0360
830.89 234.00 0.0360

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.20 223.86 .145E+04 0.2 0.43 147.07
0.73 224.39 .159E+05 2.8 0.68 93.11
1.27 224.93 .561E+05 11.0 0.75 85.19
1.80 225.46 .217E+06 45.6 0.80 79.20
2.33 225.99 .566E+06 134.4 0.91 70.20
2.87 226.53 .122E+07 338.1 1.06 59.97
3.40 227.06 .208E+07 744.3 1.38 46.16
3.94 227.60 .298E+07 1295.9 1.66 38.38
4.47 228.13 .399E+07 1979.9 1.90 33.59
5.00 228.66 .509E+07 2807.5 2.11 30.21
5.54 229.20 .627E+07 3815.1 2.32 27.41
6.07 229.73 .753E+07 4984.1 2.53 25.17
6.60 230.26 .884E+07 6346.3 2.74 23.20
7.14 230.80 .102E+08 7861.2 2.95 21.61
7.67 231.33 .116E+08 9526.4 3.14 20.29

8.21 231.87 .131E+08 11342.2 3.32 19.18
8.74 232.40 .146E+08 13309.6 3.49 18.22
9.27 232.93 .161E+08 15414.0 3.66 17.41
9.81 233.47 .177E+08 17671.0 3.81 16.70

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8330) 2186.70 134.42 11.50 175.30 2.33 0.91
OUTFLOW: ID= 1 (6324) 2186.70 121.79 12.50 175.30 2.26 0.89

| ADD HYD (8328) |
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0324): 615.64 32.052 12.00 183.87
+ ID2= 2 (6324): 2186.70 121.791 12.50 175.30
=====

ID = 3 (8328): 2802.34 153.169 12.25 177.18
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| CALIB (0322) |
IN= 1 DT=15.0 min
Area (ha) = 513.13 Curve Number (CN) = 91.4
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U.N. Tp (hrs) = 1.75

Unit Hyd Qpeak (cms) = 5.020

PEAK FLOW (cms) = 28.218 (1)
TIME TO PEAK (hrs) = 12.000
RUNOFF VOLUME (mm) = 184.747
TOTAL RAINFALL (mm) = 212.000
RUNOFF COEFFICIENT = 0.871

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| ADD HYD (8326) |
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0322): 513.13 28.218 12.00 184.75
+ ID2= 2 (8328): 2802.34 153.169 12.25 177.18
=====

ID = 3 (8326): 3315.47 180.979 12.25 178.35
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ADD HYD (8324) |
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0326): 678.91 34.798 12.00 184.63
+ ID2= 2 (8326): 3315.47 180.979 12.25 178.35
=====

ID = 3 (8324): 3994.38 215.469 12.25 179.42
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ROUTE CHN (6328) |
IN= 2 -> OUT= 1
Routing time step (min) = 15.00

<----- DATA FOR SECTION (3281.0) ----->
Distance Elevation Manning
0.00 228.00 0.0380
18.58 224.97 0.0380
78.98 223.52 0.0380
125.44 223.28 0.0380
171.90 221.71 0.0380
213.72 219.65 0.0380
218.36 219.40 0.0380
223.01 219.19 0.0380
225.95 219.14 0.0380 / 0.0320 Main Channel
226.45 218.14 0.0320 Main Channel
236.95 218.14 0.0320 Main Channel
245.85 218.14 0.0320 Main Channel
245.95 219.14 0.0320 / 0.0380 Main Channel
246.24 219.16 0.0380
250.88 219.24 0.0380
255.53 219.39 0.0380
325.22 221.47 0.0380
367.03 223.14 0.0380
404.20 225.17 0.0380
459.95 228.04 0.0380

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.50	216.64	373E+05	3.2	0.33	192.37
1.00	219.14	752E+05	10.1	0.51	124.71
1.52	219.66	149E+06	22.2	0.57	112.06
2.04	220.18	279E+06	43.4	0.59	107.06
2.56	220.70	464E+06	77.0	0.63	100.52
3.08	221.22	705E+06	125.7	0.68	93.56
3.61	221.75	100E+07	192.7	0.73	86.62
4.13	222.27	135E+07	278.1	0.79	80.92
4.65	222.79	176E+07	385.9	0.84	75.86
5.17	223.31	222E+07	514.9	0.89	71.82
5.69	223.83	280E+07	662.7	0.90	70.40
6.21	224.35	345E+07	867.7	0.96	66.35
6.73	224.87	417E+07	1107.0	1.01	62.81
7.25	225.39	494E+07	1400.0	1.08	58.80
7.78	225.92	573E+07	1732.2	1.15	55.17
8.30	226.44	655E+07	2099.9	1.22	52.02
8.82	226.96	740E+07	2502.8	1.29	49.09
9.34	227.48	828E+07	2940.9	1.36	46.90
9.86	228.00	918E+07	3414.3	1.42	44.79

<----- hydrograph -----> <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8324) 3994.38	215.47	12.25	179.42	3.74	0.75
OUTFLOW : ID= 1 (6328) 3994.38	189.16	13.25	179.42	3.57	0.73

ADD HYD (8320)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0328):	492.92	25.084	12.00	179.28
+ ID2= 2 (6328):	3994.38	189.161	13.25	179.42
ID = 3 (8320):	4487.30	212.288	13.00	179.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8318)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8320):	4487.30	212.288	13.00	179.40
+ ID2= 2 (8322):	861.74	44.899	12.25	181.11
ID = 3 (8318):	5349.04	259.943	13.00	179.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8316)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (5509):	10059.18	122.099	21.25	166.30
+ ID2= 2 (8318):	5349.04	259.943	13.00	179.68
ID = 3 (8316):	15408.22	361.763	13.00	170.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8312)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8314):	620.10	7.956	11.00	148.14
+ ID2= 2 (8316):	15408.22	361.763	13.00	170.94
ID = 3 (8312):	16028.32	369.090	13.00	170.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8308)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8310):	598.90	70.136	10.25	180.15
+ ID2= 2 (8312):	16028.32	369.090	13.00	170.06
ID = 3 (8308):	16627.22	385.156	12.25	170.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5510)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IR= 2-> OUT= 1	0.0000	0.0000	*****	74.0090
DT= 15.0 min	66.5450	18.5023	*****	*****
	98.5430	37.0045	*****	*****

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8308)	*****	385.156	12.25
OUTFLOW : ID= 1 (5510)	*****	136.187	28.75

PEAK FLOW REDUCTION [Qout/Qin](%)= 35.36
 TIME SHIFT OF PEAK FLOW (min)=990.00
 MAXIMUM STORAGE USED (ha.m.)=858.5957

ADD HYD (8240)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0336):	2785.00	20.993	24.00	173.23
+ ID2= 2 (5510):	16627.22	361.87	28.75	170.42
ID = 3 (8240):	19412.22	157.058	25.25	170.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8238)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (5508):	9524.23	162.331	15.50	143.84
+ ID2= 2 (8240):	19412.22	157.058	25.25	170.86
ID = 3 (8238):	28936.45	314.841	16.00	161.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8236)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0342):	1004.58	33.547	12.25	142.70
+ ID2= 2 (8238):	28936.45	314.841	16.00	161.97
ID = 3 (8236):	29941.03	341.954	14.75	161.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8234)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8232):	285.80	30.260	10.25	152.63
+ ID2= 2 (8236):	29941.03	341.954	14.75	161.32
ID = 3 (8234):	30226.83	342.050	14.75	161.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8230)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0344):	344.00	24.075	11.00	150.24
+ ID2= 2 (8234):	30226.83	342.050	14.75	161.34
ID = 3 (8230):	30570.83	357.917	12.25	161.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8228)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8226):	1952.06	96.397	12.00	136.68
+ ID2= 2 (8230):	30570.83	357.917	12.25	161.11
ID = 3 (8228):	32522.89	454.139	12.25	159.65

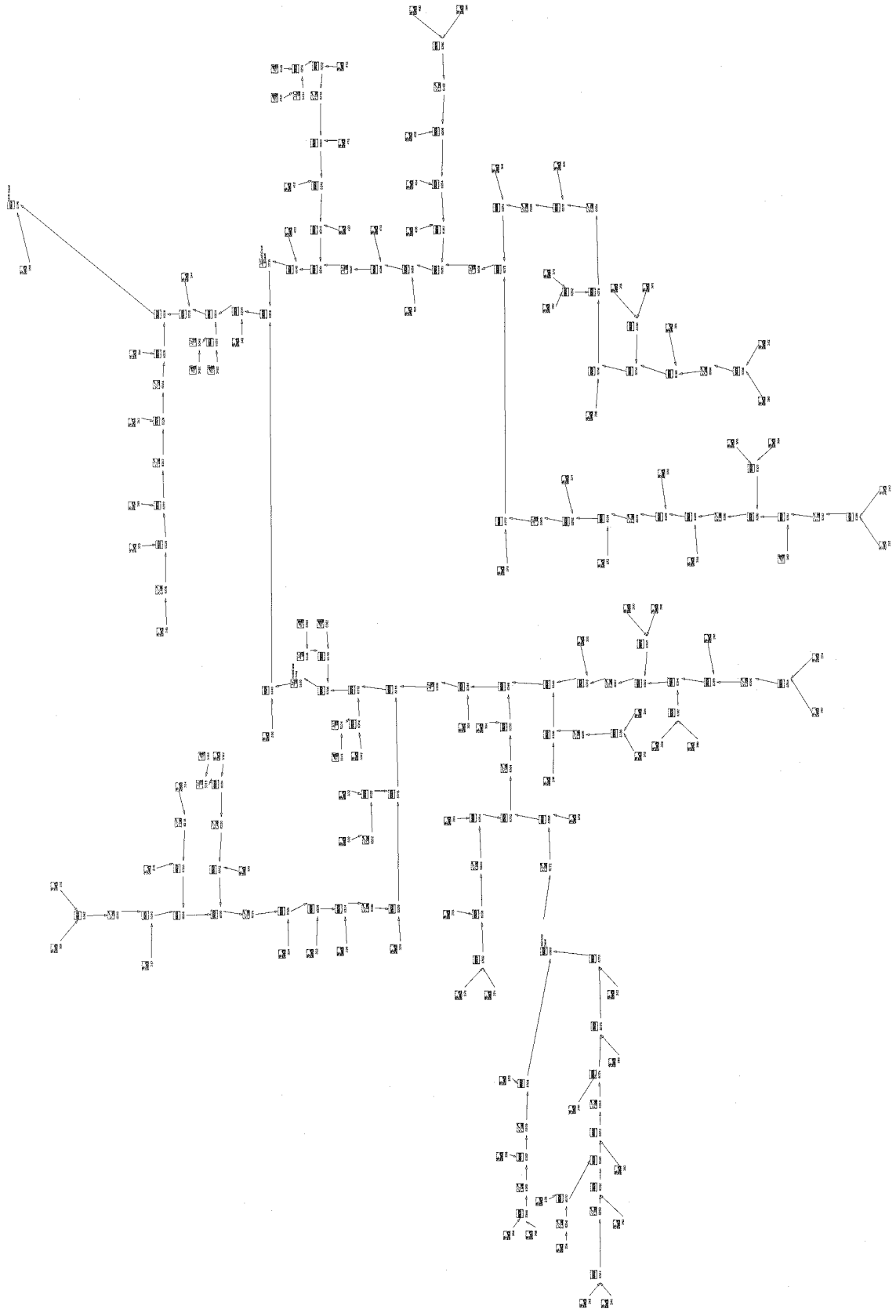
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ADD HYD (8190) |
1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
ID1= 1 (0356): 1015.22 47.576 11.75 126.89
+ ID2= 2 (8228): 32522.89 454.139 12.25 159.65
=====

ID = 3 (8190): 33538.11 501.128 12.00 158.66
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH
=====

WEST HOLLAND RIVER SUBWATERSHED



EXISTING
WEST HOLLAND RIVER 2-100 YEAR STORMS

V V I SSSS U U A L
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
WV I SSSS UUUU A A LLLL

OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y M M O O
O O T T H H Y M O O Company
OOO T T H H Y M M OOO Serial

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***** DETAILED OUTPUT *****

Input filename: C:\Program Files\Visual Othymo 2.4r\VO2\voin.dat
Output filename: C:\Users\jscott\AppData\Local\Temp\3e280798-92ee-4282-809c-79f5caed0add\Scenario.out
Summary filename: C:\Users\jscott\AppData\Local\Temp\3e280798-92ee-4282-809c-79f5caed0add\Scenario.sum

DATE: 08/22/2012 TIME: 01:32:16

USER:

COMMENTS: _____

** SIMULATION NUMBER: 1 **

Filename: C:\Users\jscott\AppData\Local\Temp\3e280798-92ee-4282-809c-79f5caed0add\Scad6041
Comments: 2-Year 12-Hour SCS II Design Storm

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.05	3.25	1.68	6.25	7.56	9.25	1.47
0.50	1.05	3.50	1.68	6.50	7.56	9.50	1.47
0.75	1.05	3.75	1.68	6.75	3.36	9.75	1.47
1.00	1.05	4.00	1.68	7.00	3.36	10.00	1.47
1.25	1.05	4.25	2.52	7.25	2.52	10.25	0.84
1.50	1.05	4.50	2.52	7.50	2.52	10.50	0.84
1.75	1.05	4.75	3.36	7.75	2.52	10.75	0.84
2.00	1.05	5.00	3.36	8.00	2.52	11.00	0.84
2.25	1.26	5.25	5.04	8.25	1.47	11.25	0.84
2.50	1.26	5.50	5.04	8.50	1.47	11.50	0.84
2.75	1.26	5.75	20.16	8.75	1.47	11.75	0.84
3.00	1.26	6.00	55.44	9.00	1.47	12.00	0.84

CALIB (0356) Area (ha)=1015.22 Curve Number (CN)= 46.0
NASHYD (0356) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.37

Unit Hyd Qpeak (cms)= 12.651

PEAK FLOW (cms)= 1.461 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 4.058
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.097

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0354) Area (ha)= 262.68 Curve Number (CN)= 37.0
NASHYD (0354) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.38

Unit Hyd Qpeak (cms)= 3.252

PEAK FLOW (cms)= 0.268 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 2.897

TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.069

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0352) Area (ha)= 381.43 Curve Number (CN)= 54.0
NASHYD (0352) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.82

Unit Hyd Qpeak (cms)= 7.980

PEAK FLOW (cms)= 1.066 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 5.329
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.127

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0346) Area (ha)= 350.93 Curve Number (CN)= 70.0
NASHYD (0346) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 6.254

PEAK FLOW (cms)= 1.534 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 9.284
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.221

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0350) Area (ha)= 366.84 Curve Number (CN)= 48.0
NASHYD (0350) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 5.831

PEAK FLOW (cms)= 0.676 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 4.345
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.103

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0348) Area (ha)= 590.18 Curve Number (CN)= 48.0
NASHYD (0348) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.91

Unit Hyd Qpeak (cms)= 5.267

PEAK FLOW (cms)= 0.711 (i)
TIME TO PEAK (hrs)= 9.250
RUNOFF VOLUME (mm)= 4.368
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.104

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6350) Routing time step (min)= 15.00

----- DATA FOR SECTION (3501.0) -----

Distance	Elevation	Manning
0.00	287.00	0.0500
23.66	283.72	0.0500
70.98	280.94	0.0500
112.38	280.23	0.0500
171.52	276.80	0.0500
260.24	274.46	0.0500
266.16	274.26	0.0500
272.07	274.12	0.0500
276.49	274.12	0.0500 / 0.0300
276.99	273.82	0.0300
277.99	273.82	0.0300
279.49	273.82	0.0300
280.49	274.27	0.0300 / 0.0500
283.90	274.27	0.0500
289.81	274.57	0.0500

325.30	275.75	0.0500
396.28	278.98	0.0500
449.51	280.97	0.0500
496.83	283.90	0.0500
585.54	287.92	0.0500

2.55	241.36	.804E+05	101.0	1.96	13.27
3.53	242.34	.173E+06	275.6	2.48	10.46
4.50	243.31	.293E+06	553.9	2.95	8.80
5.48	244.29	.438E+06	961.8	3.42	7.60
6.45	245.26	.599E+06	1534.4	3.99	6.51
7.43	246.24	.770E+06	2228.4	4.51	5.76
8.41	247.22	.951E+06	3043.6	4.98	5.21
9.38	248.19	1.14E+07	3982.1	5.43	4.78
10.36	249.17	1.34E+07	5040.1	5.85	4.44
11.33	250.14	1.55E+07	6218.5	6.24	4.16
12.31	251.12	1.77E+07	7518.6	6.61	3.93
13.29	252.10	2.00E+07	8969.0	6.98	3.72
14.26	253.07	2.24E+07	10554.1	7.35	3.53
15.24	254.05	2.48E+07	12257.1	7.71	3.37
16.21	255.02	2.72E+07	14045.1	8.03	3.23
17.19	256.00	2.98E+07	15954.8	8.34	3.11
18.16	256.97	3.25E+07	17980.4	8.53	3.04

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.30	274.12	.273E+04	0.9	0.99	49.81
0.98	274.80	.583E+05	20.8	1.05	46.71
1.66	275.48	1.203E+06	95.2	1.38	35.53
2.33	276.15	2.438E+06	256.0	1.73	28.50
3.01	276.83	4.755E+06	524.8	2.05	23.97
3.69	277.51	7.114E+07	935.8	2.43	20.26
4.37	278.19	1.578E+07	1472.0	2.76	17.82
5.05	278.87	2.008E+07	2140.7	3.06	16.06
5.72	279.54	2.61E+07	2938.6	3.33	14.79
6.40	280.22	3.321E+07	3887.6	3.57	13.77
7.08	280.90	3.90E+07	4795.4	3.63	13.56
7.76	281.58	4.68E+07	6196.2	3.91	12.58
8.43	282.25	5.50E+07	7803.9	4.19	11.74
9.11	282.93	6.37E+07	9601.9	4.45	11.05
9.79	283.61	7.28E+07	11594.8	4.70	10.46
10.47	284.29	8.23E+07	13845.3	4.96	9.91
11.15	284.97	9.22E+07	16293.4	5.21	9.43
11.82	285.64	1.03E+08	18952.0	5.45	9.02
12.50	286.32	1.13E+08	21824.3	5.69	8.65

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLW : ID= 2 (8222)	1307.95	2.71	8.00	5.68	0.63
OUTFLOW: ID= 1 (6352)	1307.95	2.69	8.25	5.68	0.62

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLW : ID= 2 (0348)	590.18	0.71	9.25	4.37	0.23
OUTFLOW: ID= 1 (6350)	590.18	0.70	10.25	4.37	0.23

ADD HYD (8224) |

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0352):	381.43	1.066	7.00	5.33
+ ID2= 2 (6352):	1307.95	2.690	8.25	5.68
ID = 3 (8224):	1689.38	3.668	8.00	5.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8220) |

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0350):	366.84	0.676	7.75	4.35
+ ID2= 2 (6350):	590.18	0.695	10.25	4.37
ID = 3 (8220):	957.02	1.262	8.75	4.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8222) |

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0346):	350.93	1.534	7.25	9.28
+ ID2= 2 (8220):	957.02	1.262	8.75	4.36
ID = 3 (8222):	1307.95	2.712	8.00	5.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6352) |

IN= 2--> OUT= 1 | Routing time step (min)= 15.00

ROUTE CHN (6354) |

IN= 2--> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (3541.0) -----

Distance	Elevation	Manning
0.00	253.92	0.0500
7.95	251.83	0.0500
15.89	249.97	0.0500
19.87	249.05	0.0500
47.68	242.00	0.0500
67.55	237.10	0.0500
83.44	231.11	0.0500
85.43	230.75	0.0500
85.66	230.61	0.0500 / 0.0300
85.76	230.01	0.0300
87.41	230.01	0.0300
88.06	230.01	0.0300
88.16	230.61	0.0300 / 0.0500
89.40	230.72	0.0500
91.39	230.88	0.0500
93.37	231.03	0.0500
133.11	233.96	0.0500
150.99	235.39	0.0500
172.84	239.03	0.0500
196.68	250.00	0.0500

----- DATA FOR SECTION (3521.0) -----

Distance	Elevation	Manning
0.00	257.95	0.0500
7.83	257.34	0.0500
15.66	256.19	0.0500
21.53	254.01	0.0500
41.11	244.06	0.0500
76.35	241.38	0.0500
111.58	239.74	0.0500
113.54	239.61	0.0500
113.75	239.41	0.0500 / 0.0300
113.85	238.81	0.0300
115.50	238.81	0.0300
116.15	238.81	0.0300
116.25	239.41	0.0300
117.46	239.52	0.0500
119.41	239.72	0.0500
121.37	240.04	0.0500
131.16	241.84	0.0500
156.61	247.03	0.0500
176.19	251.46	0.0500
193.80	258.79	0.0500

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.60	239.41	.224E+04	2.2	1.52	17.12
1.58	240.39	.216E+05	22.3	1.61	16.16

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.60	230.61	.451E+04	1.5	1.06	49.40
1.62	231.63	.394E+05	14.6	1.16	45.13
2.64	232.65	1.28E+06	58.8	1.44	36.30
3.66	233.67	2.69E+06	149.7	1.74	30.00
4.68	234.69	4.46E+06	301.5	2.04	25.58
5.70	235.71	7.05E+06	530.5	2.36	22.14
6.72	236.73	9.80E+06	846.4	2.71	19.25
7.74	237.75	1.28E+07	1237.9	3.02	17.29
8.76	238.77	1.62E+07	1706.7	3.30	15.83
9.78	239.79	1.99E+07	2291.9	3.61	14.45
10.81	240.82	2.37E+07	2973.7	3.92	13.30
11.83	241.84	2.78E+07	3742.2	4.21	12.38
12.85	242.86	3.21E+07	4599.1	4.49	11.62
13.87	243.88	3.65E+07	5544.6	4.75	10.98
14.89	244.90	4.12E+07	6579.7	5.00	10.44
15.91	245.92	4.61E+07	7705.9	5.23	9.97
16.93	246.94	5.12E+07	8924.6	5.46	9.55
17.95	247.96	5.64E+07	10237.5	5.68	9.19
18.97	248.98	6.19E+07	11646.2	5.89	8.86

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLW : ID= 2 (8224)	1689.38	3.67	8.00	5.60	0.77
OUTFLOW: ID= 1 (6354)	1689.38	3.41	9.00	5.60	0.75

```

-----
| ADD HYD (8226) |
| 1 + 2 = 3 |
-----
| ID= 1 (0354): 262.68 0.268 8.25 2.90 |
| + ID= 2 (6354): 1689.38 3.413 9.00 5.60 |
| ===== |
| ID = 3 (8226): 1952.06 3.673 9.00 5.24 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0344) | Area (ha)= 344.00 Curve Number (CN)= 59.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
| U.H. Tp(hrs)= 0.87 |
-----

```

```

Unit Hyd Qpeak (cms)= 6.790

PEAK FLOW (cms)= 1.096 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 6.331
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.151

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANSHYD (3402) | Area (ha)= 234.60 |
| ID= 1 DT=15.0 min | Total Imp(%)= 28.00 Dir. Conn.(%)= 13.00 |
-----

```

```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 65.69 168.91
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1250.60 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 55.44 4.42
over (min) 15.00 60.00
Storage Coeff. (min)= 18.13 (ii) 55.39 (ii)
Unit Hyd. Tpeak (min)= 15.00 60.00
Unit Hyd. peak (cms)= 0.06 0.02

*TOTALS*
PEAK FLOW (cms)= 3.14 1.13 3.461 (iii)
TIME TO PEAK (hrs)= 6.00 6.75 6.00
RUNOFF VOLUME (mm)= 40.00 5.40 9.90
TOTAL RAINFALL (mm)= 42.00 42.00 42.00
RUNOFF COEFFICIENT = 0.95 0.13 0.24

```

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 48.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANSHYD (3401) | Area (ha)= 51.20 |
| ID= 1 DT=15.0 min | Total Imp(%)= 46.00 Dir. Conn.(%)= 21.00 |
-----

```

```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 23.55 27.65
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 584.24 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 55.44 8.48
over (min) 15.00 45.00
Storage Coeff. (min)= 11.48 (ii) 40.19 (ii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.08 0.03

*TOTALS*
PEAK FLOW (cms)= 1.34 0.35 1.474 (iii)
TIME TO PEAK (hrs)= 6.00 6.50 6.00
RUNOFF VOLUME (mm)= 40.00 6.57 13.59
TOTAL RAINFALL (mm)= 42.00 42.00 42.00
RUNOFF COEFFICIENT = 0.95 0.16 0.32

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 48.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| RESERVOIR (5340) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
| OUTFLOW STORAGE | OUTFLOW STORAGE |
| (cms) (ha.m.) | (cms) (ha.m.) |
| 0.0000 0.0000 | 5.3360 1.7877 |
| 0.1730 0.6348 | 7.2780 2.2185 |
| 1.8360 0.8250 | 9.1880 2.6486 |
| 2.1540 1.1051 | 9.5880 2.6586 |
| 3.3950 1.3506 | 0.0000 0.0000 |
-----

```

```

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (3401) 51.200 1.474 6.00 13.59
OUTFLOW : ID= 1 (5340) 51.200 0.126 8.75 13.57

```

```

PEAK FLOW REDUCTION [Qout/Qin](%)= 8.58
TIME SHIFT OF PEAK FLOW (min)=165.00
MAXIMUM STORAGE USED (ha.m.)= 0.4644

```

```

-----
| ADD HYD (8232) |
| 1 + 2 = 3 |
-----
| ID= 1 (3402): 234.60 3.461 6.00 9.90 |
| + ID= 2 (5340): 51.20 0.126 8.75 13.57 |
| ===== |
| ID = 3 (8232): 285.80 3.513 6.00 10.55 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0342) | Area (ha)=1004.58 Curve Number (CN)= 54.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
| U.H. Tp(hrs)= 2.57 |
-----

```

```

Unit Hyd Qpeak (cms)= 6.686

PEAK FLOW (cms)= 1.208 (i)
TIME TO PEAK (hrs)= 10.250
RUNOFF VOLUME (mm)= 5.390
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.128

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0422) | Area (ha)= 780.20 Curve Number (CN)= 54.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
| U.H. Tp(hrs)= 1.95 |
-----

```

```

Unit Hyd Qpeak (cms)= 6.838

PEAK FLOW (cms)= 1.144 (i)
TIME TO PEAK (hrs)= 9.500
RUNOFF VOLUME (mm)= 5.383
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.128

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0410) | Area (ha)= 572.01 Curve Number (CN)= 48.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
| U.H. Tp(hrs)= 1.46 |
-----

```

```

Unit Hyd Qpeak (cms)= 6.698

PEAK FLOW (cms)= 0.844 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 4.360
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.104

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0408) | Area (ha)= 231.62 Curve Number (CN)= 58.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
| U.H. Tp(hrs)= 0.64 |
-----

```

Unit Hyd Qpeak (cms)= 6.198
PEAK FLOW (cms)= 0.897 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 6.074
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.145

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0376) Area (ha)= 463.85 Curve Number (CN)= 74.0
NASHYD (0376) ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 7.380

PEAK FLOW (cms)= 2.165 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 10.744
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.256

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0374) Area (ha)= 545.70 Curve Number (CN)= 61.0
NASHYD (0374) ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.51

Unit Hyd Qpeak (cms)= 6.158

PEAK FLOW (cms)= 1.234 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 6.828
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.163

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0372) Area (ha)= 110.42 Curve Number (CN)= 37.0
NASHYD (0372) ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 1.954

PEAK FLOW (cms)= 0.145 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 2.885
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.069

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0370) Area (ha)= 191.85 Curve Number (CN)= 63.0
NASHYD (0370) ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.67

Unit Hyd Qpeak (cms)= 4.860

PEAK FLOW (cms)= 0.850 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 7.219
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.172

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0368) Area (ha)= 159.48 Curve Number (CN)= 46.0
NASHYD (0368) ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.12

Unit Hyd Qpeak (cms)= 2.433

PEAK FLOW (cms)= 0.266 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 4.049
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.096

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0366) Area (ha)= 462.62 Curve Number (CN)= 62.0
NASHYD (0366) ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.06

Unit Hyd Qpeak (cms)= 7.451

PEAK FLOW (cms)= 1.407 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 7.039
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.168

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0364) Area (ha)= 155.27 Curve Number (CN)= 55.0
NASHYD (0364) ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.70

Unit Hyd Qpeak (cms)= 3.782

PEAK FLOW (cms)= 0.504 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 5.495
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.131

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8302)
1 2 3
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0364): 155.27 0.504 7.00 5.50
+ ID2= 2 (0366): 462.62 1.407 7.75 7.04

ID = 3 (8302): 617.89 1.886 7.25 6.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0362) Area (ha)= 118.78
STANDHYD (0362) ID= 1 DT=15.0 min Total Imp(%)= 22.00 Dir. Conn.(%)= 8.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 26.13 92.65
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 889.87 40.00
Manning's n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 55.44 3.78
over (min)= 15.00 60.00
Storage Coeff. (min)= 14.78 (ii) 54.45 (ii)
Unit Hyd. Tpeak (min)= 15.00 60.00
Unit Hyd. peak (cms)= 0.07 0.02

TOTALS
PEAK FLOW (cms)= 1.07 0.54 1.224 (iii)
TIME TO PEAK (hrs)= 6.00 6.75 6.00
RUNOFF VOLUME (mm)= 40.00 4.74 7.56
TOTAL RAINFALL (mm)= 42.00 42.00 42.00
RUNOFF COEFFICIENT = 0.95 0.11 0.18

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 45.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0358) Area (ha)= 429.87 Curve Number (CN)= 35.0
NASHYD (0358) ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.03

Unit Hyd Qpeak (cms)= 7.091

PEAK FLOW (cms)= 0.496 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 2.665
TOTAL RAINFALL (mm)= 42.000

RUNOFF COEFFICIENT = 0.063

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0360) | Area (ha)= 138.37 Curve Number (CN)= 46.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.60

```

Unit Hyd Qpeak (cms)= 3.957

```

PEAK FLOW (cms)= 0.367 (1)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 3.995
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.095

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8306) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (0358): 429.87 0.496 7.75 2.66
| + ID2= 2 (0360): 138.37 0.367 6.75 4.00
|-----|
| ID = 3 (8306): 568.24 0.839 7.00 2.99

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6362) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00

```

<----- DATA FOR SECTION (3621.0) ----->

Distance	Elevation	Manning
0.00	261.46	0.0550
27.86	254.23	0.0550
51.07	251.96	0.0550
74.29	250.77	0.0550
97.50	249.91	0.0550
125.96	249.40	0.0550
150.93	247.40	0.0550
155.93	247.33	0.0550 / 0.0350
157.93	246.85	0.0350
159.18	246.65	0.0350
160.18	246.63	0.0350
160.93	246.85	0.0350
161.93	247.18	0.0350 / 0.0550
163.18	248.03	0.0550
168.18	248.58	0.0550
183.18	250.18	0.0550
201.97	252.59	0.0550
213.57	256.02	0.0550
225.18	260.31	0.0550
229.82	261.00	0.0550

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.55	247.18	.558E+04	1.9	1.03	49.95
1.28	247.91	.336E+05	16.7	1.53	33.61
2.00	248.63	.913E+05	53.6	1.81	28.40
2.73	249.36	.185E+06	122.3	2.05	25.18
3.46	250.09	.344E+06	224.6	2.02	25.52
4.19	250.82	.574E+06	417.9	2.25	22.91
4.91	251.54	.855E+06	699.1	2.53	20.38
5.64	252.27	.118E+07	1075.3	2.82	18.26
6.37	253.00	.153E+07	1559.9	3.15	16.38
7.10	253.73	.191E+07	2137.6	3.46	14.89
7.82	254.45	.231E+07	2815.2	3.77	13.67
8.55	255.18	.272E+07	3608.5	4.10	12.58
9.28	255.91	.315E+07	4488.2	4.41	11.69
10.01	256.64	.358E+07	5458.0	4.71	10.95
10.73	257.36	.403E+07	6512.4	4.99	10.32
11.46	258.09	.449E+07	7649.8	5.27	9.78
12.19	258.82	.496E+07	8869.5	5.53	9.32
12.92	259.55	.544E+07	10171.2	5.78	8.91
13.64	260.27	.593E+07	11554.8	6.02	8.55

<----- hydrograph -----> <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8306)	568.24	0.84	7.00	2.99	0.25
OUTFLOW: ID= 1 (6362)	568.24	0.75	8.25	2.99	0.22

```

-----
| ADD HYD (8304) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (0362): 118.78 1.224 6.00 7.56
| + ID2= 2 (6362): 568.24 0.746 8.25 2.99
|-----|
| ID = 3 (8304): 687.02 1.321 6.00 3.78

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8300) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (8302): 617.89 1.886 7.25 6.45
| + ID2= 2 (8304): 687.02 1.321 6.00 3.78
|-----|
| ID = 3 (8300): 1304.91 3.036 7.25 5.14

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6368) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00

```

<----- DATA FOR SECTION (3681.0) ----->

Distance	Elevation	Manning
0.00	230.00	0.0370
18.48	223.26	0.0370
36.96	223.05	0.0370
64.67	222.94	0.0370
110.87	222.86	0.0370
133.96	222.74	0.0370
147.82	222.65	0.0370
170.92	222.31	0.0370
174.79	222.26	0.0370 / 0.0300
174.89	221.86	0.0300
175.54	221.86	0.0300
176.19	221.86	0.0300
176.29	222.26	0.0300 / 0.0370
180.16	222.25	0.0370
184.78	222.28	0.0370
189.40	222.31	0.0370
332.60	222.37	0.0370
450.00	230.00	0.0370

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.40	222.26	.771E+03	0.3	0.52	41.74
0.81	222.67	.784E+05	30.3	0.51	43.17
1.21	223.07	.210E+06	123.2	0.77	28.42
1.62	223.48	.384E+06	301.0	1.03	21.25
2.03	223.89	.563E+06	550.2	1.28	17.06
2.44	224.30	.747E+06	860.7	1.51	14.46
2.84	224.70	.934E+06	1228.1	1.73	12.68
3.25	225.11	.113E+07	1649.3	1.92	11.37
3.66	225.52	.132E+07	2122.3	2.11	10.37
4.07	225.93	.152E+07	2645.5	2.29	9.58
4.47	226.33	.172E+07	3217.8	2.45	8.92
4.88	226.74	.193E+07	3838.3	2.61	8.38
5.29	227.15	.214E+07	4506.3	2.76	7.92
5.70	227.56	.236E+07	5221.3	2.91	7.52
6.10	227.96	.258E+07	5983.0	3.05	7.17
6.51	228.37	.280E+07	6790.9	3.19	6.87
6.92	228.78	.303E+07	7645.0	3.32	6.60
7.33	229.19	.326E+07	8545.1	3.45	6.35
7.73	229.59	.349E+07	9491.0	3.57	6.13

<----- hydrograph -----> <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8300)	1304.91	3.04	7.25	5.14	0.44
OUTFLOW: ID= 1 (6368)	1304.91	2.84	8.25	5.14	0.43

```

-----
| ADD HYD (8298) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (0368): 159.48 0.266 8.00 4.05
| + ID2= 2 (6368): 1304.91 2.837 8.25 5.14
|-----|
| ID = 3 (8298): 1464.39 3.099 8.25 5.02

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8296)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0370):	191.85	0.850	6.75	7.22
+ ID2= 2 (8296):	1464.39	3.099	8.25	5.02
=====				
ID = 3 (8296):	1656.24	3.807	8.00	5.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN= (5372) Routing time step (min)'= 15.00
IN= 2--> OUT= 1

<----- DATA FOR SECTION (3721.0) ----->

Distance	Elevation	Manning	
0.00	225.00	0.0390	
30.80	219.38	0.0390	
61.62	219.30	0.0390	
77.01	219.27	0.0390	
469.76	219.14	0.0390	
477.46	219.13	0.0390	
485.16	219.10	0.0390	
492.86	219.09	0.0390	
495.56	219.09	0.0390 / 0.0310	Main Channel
495.66	218.51	0.0310	Main Channel
500.56	218.51	0.0310	Main Channel
505.46	218.51	0.0310	Main Channel
505.56	219.09	0.0310 / 0.0390	Main Channel
508.26	219.09	0.0390	
515.96	219.10	0.0390	
523.67	219.21	0.0390	
562.17	219.32	0.0390	
654.58	219.43	0.0390	
731.59	219.46	0.0390	
762.39	225.00	0.0390	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.29	218.80	.431E+04	1.7	0.58	43.48
0.58	219.09	.867E+04	5.1	0.89	28.32
0.93	219.44	.200E+06	58.2	0.44	57.35
1.28	219.79	.568E+06	278.6	0.74	33.99
1.62	220.13	.939E+06	630.2	1.01	24.84
1.97	220.48	.131E+07	1090.4	1.25	20.05
2.32	220.83	.169E+07	1647.7	1.47	17.06
2.67	221.18	.206E+07	2294.6	1.68	14.99
3.01	221.52	.244E+07	3025.6	1.87	13.46
3.36	221.87	.282E+07	3836.5	2.05	12.07
3.71	222.22	.321E+07	4723.7	2.22	11.31
4.06	222.57	.359E+07	5684.6	2.39	10.53
4.40	222.91	.398E+07	6716.6	2.55	9.87
4.75	223.26	.437E+07	7817.7	2.70	9.31
5.10	223.61	.476E+07	8986.2	2.85	8.83
5.45	223.96	.515E+07	10220.4	2.99	8.40
5.79	224.30	.555E+07	11519.1	3.13	8.03
6.14	224.65	.594E+07	12880.8	3.27	7.69
6.49	225.00	.634E+07	14304.5	3.40	7.39

<---- hydrograph ----> <-pipe / channel-->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8296)	1656.24	3.81	8.00	5.27	0.47	0.74
OUTFLOW: ID= 1 (6372)	1656.24	3.70	8.50	5.27	0.46	0.73

ADD HYD (8294)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0372):	110.42	0.145	7.50	2.88
+ ID2= 2 (6372):	1656.24	3.698	8.50	5.27
=====				
ID = 3 (8294):	1766.66	3.833	8.50	5.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8292)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0374):	545.70	1.214	8.25	6.83
+ ID2= 2 (8294):	1766.66	3.833	8.50	5.13
=====				
ID = 3 (8292):	2312.36	5.066	8.50	5.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5505)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1	0.0000	0.0000	65.1290	345.3754
DT= 15.0 min	25.4850	24.6697	84.9510	456.3890
	31.1490	98.6787	*****	838.7689
	39.6440	*****	*****	838.7789
	48.1390	*****	0.0000	0.0000

INFLOW : ID= 2 (8292)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	2312.360	5.066	8.50	5.53
OUTFLOW: ID= 1 (5505)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	2312.360	3.659	11.25	5.53

PEAK FLOW REDUCTION [Qout/Qin](%)= 72.23
TIME SHIFT OF PEAK FLOW (min)=165.00
MAXIMUM STORAGE USED (ha.m.)= 3.5421

ADD HYD (8272)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0376):	463.85	2.165	7.50	10.74
+ ID2= 2 (5505):	2312.36	3.659	11.25	5.53
=====				
ID = 3 (8272):	2776.21	5.098	10.25	6.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0396)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	305.21	69.0
	U.H. Tp(hrs)=	1.08

Unit Hyd Qpeak (cms)= 4.811

PEAK FLOW (cms)= 1.174 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 8.977
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.214

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0394)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	325.45	53.0
	U.H. Tp(hrs)=	0.92

Unit Hyd Qpeak (cms)= 6.013

PEAK FLOW (cms)= 0.799 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 5.160
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.123

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0390)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	420.00	55.0
	U.H. Tp(hrs)=	1.07

Unit Hyd Qpeak (cms)= 6.683

PEAK FLOW (cms)= 0.992 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 5.541
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.132

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0388)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	220.77	58.0
	U.H. Tp(hrs)=	0.99

Unit Hyd Qpeak (cms)= 3.819

PEAK FLOW (cms)= 0.615 (i)
TIME TO PEAK (hrs)= 7.500

RUNOFF VOLUME (mm)= 6.132
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.146

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0386) | Area (ha)= 241.27 | Curve Number (CN)= 61.0
NASHYD (0386) | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.90

Unit Hyd Opeak (cms)= 4.562
PEAK FLOW (cms)= 0.799 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 6.784
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.162

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8286) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0386): 241.27 0.799 7.25 6.78
+ ID2= 2 (0388): 220.77 0.615 7.50 6.13
ID = 3 (8286): 462.04 1.411 7.25 6.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0384) | Area (ha)= 199.07 | Curve Number (CN)= 44.0
NASHYD (0384) | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.96

Unit Hyd Opeak (cms)= 3.537
PEAK FLOW (cms)= 0.343 (1)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 3.759
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.089

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0380) | Area (ha)= 182.01 | Curve Number (CN)= 40.0
NASHYD (0380) | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.55

Unit Hyd Opeak (cms)= 5.609
PEAK FLOW (cms)= 0.407 (1)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 3.195
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.076

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0382) | Area (ha)= 216.59 | Curve Number (CN)= 53.0
NASHYD (0382) | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.64

Unit Hyd Opeak (cms)= 5.733
PEAK FLOW (cms)= 0.697 (1)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 5.118
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.122

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8290) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0380): 182.01 0.407 6.75 3.20
+ ID2= 2 (0382): 216.59 0.697 6.75 5.12

ID = 3 (8290): 398.60 1.104 6.75 4.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6384) |
ID= 2 -> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (3841.0) -----

Distance	Elevation	Manning
0.00	294.40	0.0380
10.59	291.93	0.0380
21.17	289.19	0.0380
26.46	287.99	0.0380
31.76	286.79	0.0380
71.45	279.97	0.0380
74.10	279.79	0.0380
76.74	279.71	0.0380
78.99	279.64	0.0380 / 0.0300
79.14	279.30	0.0300
79.39	279.30	0.0300
79.64	279.30	0.0300
79.79	279.64	0.0300 / 0.0380
89.98	279.78	0.0380
119.09	281.86	0.0380
145.55	282.87	0.0380
198.48	284.85	0.0380
211.71	286.31	0.0380
230.23	287.59	0.0380
261.99	294.00	0.0380

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELUV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.34	279.64	.715E+03	0.2	0.85	63.42
1.10	280.40	.507E+05	21.9	1.40	38.54
1.85	281.15	.141E+06	91.6	2.10	25.69
2.61	281.91	.269E+06	218.9	2.63	20.46
3.36	282.66	.445E+06	407.1	2.96	18.23
4.12	283.42	.681E+06	698.3	3.31	16.26
4.87	284.17	.977E+06	1111.8	3.68	14.65
5.63	284.93	.133E+07	1674.3	4.06	13.27
6.39	285.69	.173E+07	2457.8	4.59	11.73
7.14	286.44	.215E+07	3378.8	5.07	10.63
7.90	287.20	.261E+07	4418.4	5.47	9.85
8.65	287.95	.310E+07	5676.3	5.92	9.11
9.41	288.71	.361E+07	7150.5	6.40	8.42
10.17	289.47	.414E+07	8783.1	6.86	7.86
10.92	290.22	.469E+07	10575.1	7.30	7.38
11.68	290.98	.525E+07	12520.4	7.72	6.98
12.43	291.73	.582E+07	14619.0	8.12	6.64
13.19	292.49	.642E+07	16863.2	8.50	6.34
13.94	293.24	.703E+07	19259.8	8.86	6.08

----- hydrograph ----- <-pipe / channel-->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8290) 398.60 1.10 6.75 4.24 0.37 0.86
OUTFLOW: ID= 1 (6384) 398.60 0.87 8.00 4.24 0.36 0.86

ADD HYD (8288) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0384): 199.07 0.343 7.50 3.76
+ ID2= 2 (6384): 398.60 0.866 8.00 4.24
ID = 3 (8288): 597.67 1.204 8.00 4.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8284) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8286): 462.04 1.411 7.25 6.47
+ ID2= 2 (8288): 597.67 1.204 8.00 4.08
ID = 3 (8284): 1059.71 2.590 7.75 5.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8280) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)

ID1= 1 (0390): 420.00 0.392 7.75 5.54
 + ID2= 2 (8284): 1059.71 2.590 7.75 5.12
 =====
 ID = 3 (8280): 1479.71 3.582 7.75 5.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0392) | Area (ha)= 167.22 Curve Number (CN)= 62.0
 NASHYD (0392) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.74

Unit Hyd Qpeak (cms)= 3.837
 PEAK FLOW (cms)= 0.666 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 6.993
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.166

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0378) | Area (ha)= 606.72 Curve Number (CN)= 55.0
 NASHYD (0378) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.18

Unit Hyd Qpeak (cms)= 8.771
 PEAK FLOW (cms)= 1.341 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 5.547
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.132

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8282) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0378): 606.72 1.341 8.00 5.55
 + ID2= 2 (0392): 167.22 0.666 7.00 6.99
 =====
 ID = 3 (8282): 773.94 1.958 7.50 5.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8278) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8280): 1479.71 3.582 7.75 5.24
 + ID2= 2 (8284): 773.94 1.958 7.50 5.86
 =====
 ID = 3 (8278): 2253.65 5.533 7.75 5.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6394) |
 IN= 2 -> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3941.0) ----->
 Distance Elevation Manning
 0.00 283.00 0.0380
 13.48 282.87 0.0380
 53.92 280.08 0.0380
 74.13 276.62 0.0380
 97.72 265.45 0.0380
 114.57 256.93 0.0380
 131.42 253.04 0.0380
 134.79 252.58 0.0380
 138.53 251.74 0.0380 / 0.0300 Main Channel
 139.03 251.20 0.0300 Main Channel
 141.53 251.20 0.0300 Main Channel
 144.03 251.20 0.0300 Main Channel
 144.53 251.74 0.0300 / 0.0380 Main Channel
 148.27 252.69 0.0380
 151.64 252.97 0.0380
 185.34 255.08 0.0380
 219.03 257.54 0.0380
 262.84 259.43 0.0380
 310.02 262.80 0.0380
 333.60 283.00 0.0380

<----- TRAVEL TIME TABLE ----->
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
 (m) (m) (cu.m.) (cms) (m/s) (min)
 0.54 251.74 .827E+04 4.5 1.51 30.71
 2.19 251.39 .758E+05 73.2 2.69 17.27
 3.83 255.03 .282E+06 331.8 3.27 14.17
 5.48 256.68 .633E+06 917.3 4.03 11.51
 7.12 258.32 .112E+07 1889.5 4.69 9.90
 8.77 259.97 .179E+07 3410.2 5.31 8.73
 10.41 261.61 .259E+07 5627.6 6.05 7.66
 12.06 263.26 .351E+07 8605.1 6.83 6.79
 13.70 264.90 .447E+07 12544.9 7.81 5.94
 15.35 266.55 .548E+07 17101.4 8.71 5.32
 16.99 268.19 .648E+07 22251.7 9.56 4.85
 18.64 269.84 .752E+07 27979.0 10.36 4.48
 20.28 271.48 .858E+07 34269.0 11.11 4.17
 21.93 273.13 .967E+07 41110.8 11.83 3.92
 23.57 274.77 .108E+08 48495.9 12.51 3.71
 25.22 276.42 .119E+08 56418.1 13.17 3.52
 26.86 278.06 .131E+08 64410.6 13.69 3.39
 28.51 279.71 .143E+08 73002.1 14.18 3.27
 30.15 281.35 .156E+08 81379.3 14.50 3.20

<---- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8278) 2253.65 5.53 7.75 5.45 0.57 1.52
 OUTFLOW : ID= 1 (6394) 2253.65 5.37 8.25 5.45 0.56 1.52

ADD HYD (8276) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0394): 325.45 0.799 7.25 5.16
 + ID2= 2 (6394): 2253.65 5.366 8.25 5.45
 =====
 ID = 3 (8276): 2579.10 6.118 8.25 5.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6396) |
 IN= 2 -> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3961.0) ----->
 Distance Elevation Manning
 0.00 263.00 0.0410
 11.75 257.14 0.0410
 23.50 253.97 0.0410
 41.13 247.83 0.0410
 76.38 232.09 0.0410
 135.13 229.07 0.0410
 149.82 228.97 0.0410
 152.75 228.96 0.0410 / 0.0300 Main Channel
 154.19 228.73 0.0300 Main Channel
 154.69 228.20 0.0300 Main Channel
 155.69 228.20 0.0300 Main Channel
 156.69 228.20 0.0300 Main Channel
 157.19 228.73 0.0300 Main Channel
 158.63 228.95 0.0300 / 0.0410 Main Channel
 161.57 228.96 0.0410
 164.51 229.71 0.0410
 196.82 241.70 0.0410
 223.26 249.21 0.0410
 246.76 255.13 0.0410
 290.82 263.51 0.0410

<----- TRAVEL TIME TABLE ----->
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
 (m) (m) (cu.m.) (cms) (m/s) (min)
 0.75 228.95 .780E+04 2.2 0.98 58.06
 2.54 230.74 .277E+06 145.1 1.78 31.76
 4.33 232.53 .786E+06 609.1 2.63 21.50
 6.13 234.33 .140E+07 1481.5 3.59 15.77
 7.92 236.12 .207E+07 2677.3 4.39 12.90
 9.71 237.91 .280E+07 4184.8 5.09 11.14
 11.50 239.70 .357E+07 6001.4 5.71 9.93
 13.29 241.49 .441E+07 8129.1 6.27 9.03
 15.09 243.29 .530E+07 10549.4 6.77 8.37
 16.88 245.08 .625E+07 13300.6 7.24 7.83
 18.67 246.87 .726E+07 16396.2 7.68 7.38
 20.46 248.66 .834E+07 19809.8 8.07 7.02
 22.26 250.46 .949E+07 23532.5 8.43 6.72
 24.05 252.25 .107E+08 27641.2 8.77 6.46
 25.84 254.04 .120E+08 32155.2 9.10 6.22
 27.63 255.83 .134E+08 36901.0 9.37 6.05
 29.42 257.62 .149E+08 42147.7 9.64 5.88
 31.22 259.42 .164E+08 48145.1 9.97 5.68
 33.01 261.21 .181E+08 54644.8 10.29 5.51

<---- hydrograph ----> <-pipe / channel->

AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8276) 2579.10 6.12 8.25 5.42 0.80 0.99
 OUTFLOW: ID= 1 (6396) 2579.10 5.54 9.25 5.42 0.79 0.99

ADD HYD (8274)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0396): 305.21 1.174 7.75 8.98
 + ID2= 2 (6396): 2579.10 5.542 9.25 5.42
 =====
 ID = 3 (8274): 2884.31 6.553 9.25 5.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8270)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8272): 2776.21 5.098 10.25 6.40
 + ID2= 2 (8274): 2884.31 6.553 9.25 5.79
 =====
 ID = 3 (8270): 5660.52 11.498 9.50 6.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5506)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	60.8810	135.6832
31.1490	24.6697	96.2770	900.4431
36.8120	37.0045	96.6770	900.4531
45.3070	86.3439	0.0000	0.0000

AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (8270) 5660.520 11.498 9.50 6.09
 OUTFLOW: ID= 1 (5506) 5660.520 9.572 12.00 6.09

PEAK FLOW REDUCTION [Qout/Qin](%)= 83.25
 TIME SHIFT OF PEAK FLOW (min)=150.00
 MAXIMUM STORAGE USED (ha.m.)= 7.5866

CALIB (0406) Area (ha)= 142.65 Curve Number (CN)= 66.0
 NASHYD (0406) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.59

Unit Hyd Opeak (cms)= 4.135

PEAK FLOW (cms)= 0.781 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 7.974
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.190

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0404) Area (ha)= 246.46 Curve Number (CN)= 47.0
 NASHYD (0404) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.98

Unit Hyd Opeak (cms)= 4.280

PEAK FLOW (cms)= 0.467 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 4.188
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.100

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0402) Area (ha)= 244.00 Curve Number (CN)= 61.0
 NASHYD (0402) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.07

Unit Hyd Opeak (cms)= 3.879

PEAK FLOW (cms)= 0.710 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 6.803
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.162

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0400) Area (ha)= 93.97 Curve Number (CN)= 52.0
 NASHYD (0400) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.44

Unit Hyd Opeak (cms)= 3.630

PEAK FLOW (cms)= 0.390 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 4.872
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.116

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0398) Area (ha)= 328.19 Curve Number (CN)= 55.0
 NASHYD (0398) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.83

Unit Hyd Opeak (cms)= 6.759

PEAK FLOW (cms)= 0.938 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 5.517
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.131

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8268)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0398): 328.19 0.938 7.00 5.52
 + ID2= 2 (0400): 93.97 0.390 6.50 4.87
 =====
 ID = 3 (8268): 422.16 1.286 7.00 5.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (5402) Routing time step (min)'= 15.00
 IN= 2--> OUT= 1

<<----- DATA FOR SECTION (4021.0) ----->>

Distance	Elevation	Manning
0.00	238.50	0.0360
11.50	238.00	0.0360
23.00	237.93	0.0360
34.49	236.39	0.0360
63.24	233.98	0.0360
97.73	228.15	0.0360
123.60	227.08	0.0360
126.48	226.61	0.0360
127.60	226.47	0.0360 /0.0330 Main Channel
127.85	225.25	0.0330 Main Channel
129.35	225.25	0.0330 Main Channel
130.85	225.25	0.0330 Main Channel
131.10	226.47	0.0330 /0.0360 Main Channel
132.22	226.59	0.0360
143.72	227.42	0.0360
169.59	227.88	0.0360
192.59	231.19	0.0360
218.46	233.02	0.0360
241.45	235.50	0.0360
284.57	236.43	0.0360

----- TRAVEL TIME TABLE ----->>

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.41	225.66	.391E+04	1.7	1.36	38.31
0.81	226.06	.804E+04	5.0	1.93	27.02
1.22	226.47	.124E+05	9.1	2.30	22.61
1.84	227.09	.308E+05	23.3	2.36	22.03
2.46	227.71	.904E+05	62.0	2.14	24.31
3.09	228.34	.223E+06	188.3	2.63	19.75
3.71	228.96	.379E+06	410.2	3.38	15.39
4.33	229.58	.550E+06	711.5	4.04	12.89
4.95	230.20	.737E+06	1091.3	4.62	11.25

5.58	230.83	.939E+06	1550.6	5.15	10.10
6.20	231.45	.116E+07	2072.4	5.59	9.31
6.82	232.07	.140E+07	2659.2	5.93	8.77
7.44	232.69	.167E+07	3345.8	6.27	8.30
8.07	233.32	.195E+07	4159.0	6.64	7.83
8.69	233.94	.226E+07	5100.8	7.03	7.39
9.31	234.56	.259E+07	6081.3	7.32	7.11
9.93	235.18	.295E+07	7183.1	7.60	6.84
10.56	235.81	.334E+07	8345.2	7.62	6.83
11.18	236.43	.379E+07	9116.0	7.51	6.92

	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8268)	422.16	1.29	7.00	5.37	0.31	1.36
OUTFLOW : ID= 1 (6402)	422.16	1.15	7.75	5.37	0.28	1.36

ADD HYD (8266)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0402):	244.00	0.710	7.75	6.80
+ ID2= 2 (6402):	422.16	1.155	7.75	5.37
ID = 3 (8266):	666.16	1.865	7.75	5.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8264)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0404):	246.46	0.467	7.50	4.19
+ ID2= 2 (8266):	666.16	1.865	7.75	5.90
ID = 3 (8264):	912.62	2.329	7.75	5.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8262)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0406):	142.65	0.781	7.75	7.97
+ ID2= 2 (8264):	912.62	2.329	7.75	5.44
ID = 3 (8262):	1055.27	3.000	7.50	5.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8260)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (5506):	5660.52	9.572	12.00	6.09
+ ID2= 2 (8262):	1055.27	3.000	7.50	5.78
ID = 3 (8260):	6715.79	10.967	11.25	6.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8258)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0408):	231.62	0.897	6.75	6.07
+ ID2= 2 (8260):	6715.79	10.967	11.25	6.04
ID = 3 (8258):	6947.41	11.251	11.25	6.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8256)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0410):	572.01	0.844	8.25	4.36
+ ID2= 2 (8258):	6947.41	11.251	11.25	6.04
ID = 3 (8256):	7519.42	11.941	11.00	5.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5507)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IMS 2--> OUT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min	0.0000	0.0000	90.6140	67.8416
	39.6440	12.3348	*****	160.3529
	48.1390	18.5023	*****	160.3629
	67.9600	37.0045	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8256)	7519.421	11.941	11.00	5.91
OUTFLOW : ID= 1 (5507)	7519.421	11.702	12.00	5.91

PEAK FLOW REDUCTION [Qout/Qin](%)= 98.00
 TIME SHIFT OF PEAK FLOW (min)= 60.00
 MAXIMUM STORAGE USED (ha.m.)= 3.6414

NASHYD (0420)	Area	(ha)	Curve Number	(CN)= 53.0
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50	
	U.H. Tp(hrs)=	0.81		

Unit Hyd Qpeak (cms)= 3.692

PEAK FLOW (cms)= 0.476 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 5.148
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.123

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 64.0
NASHYD (0418)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50	
ID= 1 DT=15.0 min	U.H. Tp(hrs)=	1.05		

Unit Hyd Qpeak (cms)= 2.966

PEAK FLOW (cms)= 0.600 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 7.539
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.179

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 64.0
NASHYD (0416)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50	
ID= 1 DT=15.0 min	U.H. Tp(hrs)=	1.29		

Unit Hyd Qpeak (cms)= 5.832

PEAK FLOW (cms)= 1.251 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 7.557
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.180

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 54.0
NASHYD (0412)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50	
ID= 1 DT=15.0 min	U.H. Tp(hrs)=	0.80		

Unit Hyd Qpeak (cms)= 5.088

PEAK FLOW (cms)= 0.677 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 5.327
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.127

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Dir. Conn.(%)= 17.00
STANDHYD (4141)	Total Imp(%)= 36.00		
ID= 1 DT=15.0 min			

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	15.73	27.97	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	539.75	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	55.44	11.49	
over (min)	15.00	45.00	
Storage Coeff. (min)=	10.95 (ii)	36.37 (ii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.08	0.03	
			TOTALS
PEAK FLOW (cms)=	0.94	0.50	1.138 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	40.00	9.81	14.94
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.23	0.36

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING:FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 64.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5414)				
IN= 2 -> OUT= 1				
DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	3.4720	1.9177
	0.3820	0.3152	3.8720	1.9277
	1.8280	0.4690	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (4141)	43.700	1.138	6.00	14.94
OUTFLOW: ID= 1 (5414)	43.700	0.329	7.25	14.94

PEAK FLOW REDUCTION [Qout/qin](%)= 28.91
 TIME SHIFT OF PEAK FLOW (min)= 75.00
 MAXIMUM STORAGE USED (ha.m.)= 0.2718

CALIB STANDBYD (4142)	Area (ha)= 144.30	Dir. Conn.(%)= 10.00
ID= 1 DT=15.0 min	Total Imp(%)= 21.00	

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	30.30	114.00	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	980.82	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	55.44	8.83	
over (min)	15.00	45.00	
Storage Coeff. (min)=	15.67 (ii)	43.91 (ii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.07	0.03	
			TOTALS
PEAK FLOW (cms)=	1.59	1.42	2.138 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	40.00	8.68	11.81
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.21	0.28

***** WARNING:FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 64.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8254)				
1 + 2 = 3				
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (4142):	144.30	2.138	6.00	11.81
+ ID2= 2 (5414):	43.70	0.329	7.25	14.94

ID = 3 (8254):	188.00	2.278	6.00	12.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8252)				
1 + 2 = 3				
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0412):	238.70	0.677	7.00	5.33
+ ID2= 2 (8254):	188.00	2.278	6.00	12.54

ID = 3 (8252):	426.70	2.814	6.50	8.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6416)		
IN= 2 -> OUT= 1		
	Routing time step (min)= 15.00	

----- DATA FOR SECTION (4161.0) -----			
Distance	Elevation	Manning	
0.00	270.07	0.0340	
20.67	267.91	0.0340	
62.01	264.33	0.0340	
113.69	259.75	0.0340	
165.37	253.30	0.0340	
227.38	246.29	0.0340	
232.55	246.03	0.0340	
237.72	246.16	0.0340	
241.39	246.02	0.0340 / 0.0300	Main Channel
241.64	245.75	0.0300	Main Channel
242.89	245.75	0.0300	Main Channel
244.14	245.75	0.0300	Main Channel
244.39	246.02	0.0300 / 0.0340	Main Channel
248.06	246.20	0.0340	
253.22	246.20	0.0340	
259.39	246.63	0.0340	
346.25	252.57	0.0340	
413.43	257.77	0.0340	
465.11	261.78	0.0340	
511.62	270.00	0.0340	

----- TRAVEL TIME TABLE -----					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.27	246.02	.295E+04	0.6	0.75	88.26
1.53	247.28	.168E+06	69.7	1.65	40.13
2.79	248.54	.490E+06	299.5	2.42	27.28
4.06	249.81	.962E+06	740.1	3.05	21.67
5.32	251.07	.158E+07	1440.0	3.61	18.33
6.58	252.33	.235E+07	2443.8	4.12	16.06
7.84	253.59	.327E+07	3811.2	4.63	14.30
9.10	254.85	.432E+07	5565.1	5.11	12.94
10.37	256.12	.550E+07	7721.3	5.57	11.88
11.63	257.38	.682E+07	10309.9	6.00	11.02
12.89	258.64	.827E+07	13360.6	6.42	10.31
14.15	259.90	.985E+07	16884.5	6.81	9.72
15.42	261.17	.116E+08	20818.4	7.14	9.26
16.68	262.43	.134E+08	25510.4	7.53	8.78
17.94	263.69	.154E+08	30958.0	7.97	8.30
19.20	264.95	.175E+08	36988.3	8.38	7.89
20.46	266.21	.197E+08	43617.1	8.78	7.53
21.73	267.48	.220E+08	50868.1	9.17	7.22
22.99	268.74	.244E+08	58854.0	9.56	6.92

	----- hydrograph -----				<- pipe / channel ->	
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8252)	426.70	2.81	6.50	8.50	0.31	0.76
OUTFLOW: ID= 1 (6416)	426.70	1.66	7.75	8.50	0.29	0.76

ADD HYD (8250)				
1 + 2 = 3				
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0416):	439.30	1.251	8.00	7.56
+ ID2= 2 (6416):	426.70	1.663	7.75	8.50

ID = 3 (8250):	866.00	2.906	8.00	8.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8248)				
1 + 2 = 3				
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0418):	182.79	0.600	7.50	7.54
+ ID2= 2 (8250):	866.00	2.906	8.00	8.02

ID = 3 (8248):	1048.79	3.503	8.00	7.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8246)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8242):	176.82	0.476	7.00	5.15
+ ID2= 2 (8248):	1048.79	3.503	8.00	7.94
=====				
ID = 3 (8246):	1224.61	3.958	7.75	7.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8244)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (5507):	7519.42	11.702	12.00	5.91
+ ID2= 2 (8246):	1224.61	3.958	7.75	7.54
=====				
ID = 3 (8244):	8744.03	13.941	11.25	6.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8242)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0422):	780.20	1.144	9.50	5.38
+ ID2= 2 (8244):	8744.03	13.941	11.25	6.14
=====				
ID = 3 (8242):	9524.23	15.007	11.00	6.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5508)				
IN= 2--> OUT= 1	OUTFLOW	STORAGE	OUTFLOW	STORAGE
DT= 15.0 min	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	*****	197.3574
	76.4550	30.8371	*****	394.7148
	*****	61.5742	*****	394.7248
	*****	*****	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8242)	9524.231	15.007	11.00	6.08
OUTFLOW: ID= 1 (5508)	9524.231	14.571	12.25	6.08

PEAK FLOW REDUCTION [Qout/Qin](%)= 97.09
 TIME SHIFT OF PEAK FLOW (min)= 75.00
 MAXIMUM STORAGE USED (ha.m.)= 5.8793

CALIB				
NASHYD (0336)	Area	(ha)	(cms)	Curve Number (CN)= 72.0
ID= 1 DT=15.0 min	Ia	(mm)	= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)	= 15.39		

Unit Hyd Qpeak (cms)= 3.090

PEAK FLOW (cms)= 1.223 (i)
 TIME TO PEAK (hrs)= 22.500
 RINFLOW VOLUME (mm)= 10.068
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.240

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB				
STANDHYD (3382)	Area	(ha)	= 525.90	Dir. Conn.(%)= 17.00
ID= 1 DT=15.0 min	Total Imp(%)	= 37.00		

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 194.58 331.32
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1872.43 40.00
 Mannings n = 0.013 0.250
 Max. Ref. Inten.(mm/hr)= 37.80 12.71
 over (min) 30.00 60.00
 Storage Coeff. (min)= 26.92 (ii) 51.33 (ii)
 Unit Hyd. Tpeak (min)= 30.00 60.00

Unit Hyd. peak (cms)= 0.04 0.02 *TOTALS*
 PEAK FLOW (cms)= 6.48 5.14 9.230 (iii)
 TIME TO PEAK (hrs)= 6.25 6.75 6.25
 RINFLOW VOLUME (mm)= 40.00 10.61 15.61
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.25 0.37

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB			
STANDHYD (3381)	Area	(ha)	= 73.00
ID= 1 DT=15.0 min	Total Imp(%)	= 49.00	Dir. Conn.(%)= 23.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 35.77 37.23
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 697.61 40.00
 Mannings n = 0.013 0.250

Max. Ref. Inten.(mm/hr)= 55.44 16.56
 over (min) 15.00 45.00
 Storage Coeff. (min)= 12.77 (ii) 34.73 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03

TOTALS
 PEAK FLOW (cms)= 2.01 0.99 2.410 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RINFLOW VOLUME (mm)= 40.00 11.94 18.39
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.28 0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5338)				
IN= 2--> OUT= 1	OUTFLOW	STORAGE	OUTFLOW	STORAGE
DT= 15.0 min	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	3.3350	1.2400
	0.3230	0.3074	4.3560	1.3520
	0.9360	0.5374	5.3290	1.4590
	1.7590	0.8302	5.7290	1.4690
	2.4110	1.0550	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (3381)	73.000	2.410	6.00	18.39
OUTFLOW: ID= 1 (5338)	73.000	0.897	7.00	18.39

PEAK FLOW REDUCTION [Qout/Qin](%)= 37.23
 TIME SHIFT OF PEAK FLOW (min)= 60.00
 MAXIMUM STORAGE USED (ha.m.)= 0.5258

ADD HYD (8310)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (3382):	525.90	9.230	6.25	15.61
+ ID2= 2 (5338):	73.00	0.897	7.00	18.39
=====				
ID = 3 (8310):	598.90	9.798	6.25	15.95

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB				
NASHYD (3342)	Area	(ha)	= 587.10	Curve Number (CN)= 56.0
ID= 1 DT=15.0 min	Ia	(mm)	= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)	= 8.19		

Unit Hyd Qpeak (cms)= 1.224

PEAK FLOW (cms)= 0.275 (i)
 TIME TO PEAK (hrs)= 15.500

RUNOFF VOLUME (mm)= 5.784
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.138

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (3341) | Area (ha)= 33.00 |
| ID= 1 DT=15.0 min | Total Imp(%)= 51.00 | Dir. Conn.(%)= 31.00 |
-----
| IMPERVIOUS | PERVIOUS (i) |
| Surface Area (ha)= 16.83 | 16.17 |
| Dep. Storage (mm)= 2.00 | 5.00 |
| Average Slope (%)= 0.50 | 0.50 |
| Length (m)= 469.04 | 40.00 |
| Mannings n = 0.013 | 0.250 |
-----
| Max.Eff.Inten.(mm/hr)= 55.44 | 9.00 |
| over (min)= 15.00 | 45.00 |
| Storage Coeff. (min)= 10.07 (ii) | 38.09 (ii) |
| Unit Hyd. Tpeak (min)= 15.00 | 45.00 |
| Unit Hyd. peak (cms)= 0.09 | 0.03 |
-----
| *TOTALS* |
| PEAK FLOW (cms)= 1.33 | 0.22 | 1.415 (iii) |
| TIME TO PEAK (hrs)= 6.00 | 6.50 | 6.00 |
| RUNOFF VOLUME (mm)= 40.00 | 7.21 | 17.38 |
| TOTAL RAINFALL (mm)= 42.00 | 42.00 | 42.00 |
| RUNOFF COEFFICIENT = 0.95 | 0.17 | 0.41 |
  
```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 52.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| RESERVOIR (5334) |
| IIS 2--> OUT= 1 |
| DT= 15.0 min |
-----
| OUTFLOW | STORAGE | OUTFLOW | STORAGE |
| (cms) | (ha.m.) | (cms) | (ha.m.) |
|-----|-----|-----|-----|
| 0.0000 | 0.0000 | 0.6540 | 0.4604 |
| 0.0390 | 0.1699 | 0.8330 | 0.5249 |
| 0.1860 | 0.2610 | 1.0130 | 0.5896 |
| 0.3750 | 0.3534 | 1.4130 | 0.5996 |
| 0.4340 | 0.3813 | 0.0000 | 0.0000 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) | |
|---|---|---|---|---|
| INFLOW : ID= 2 (3341) | 33.000 | 1.415 | 6.00 | 17.38 |
| OUTFLOW: ID= 1 (5334) | 33.000 | 0.280 | 7.00 | 17.34 |
-----
| PEAK FLOW REDUCTION [Qout/Qin](%)= 19.76 |
| TIME SHIFT OF PEAK FLOW (min)= 60.00 |
| MAXIMUM STORAGE USED (ha.m.)= 0.3077 |
  
```

```

-----
| ADD HYD (8314) |
| 1 + 2 + 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) | |
|---|---|---|---|---|
| ID1= 1 (3342): | 587.10 | 0.275 | 15.50 | 5.78 |
| + ID2= 2 (5334): | 33.00 | 0.280 | 7.00 | 17.34 |
|-----|-----|-----|-----|
| ID = 3 (8314): | 620.10 | 0.389 | 7.25 | 6.40 |
-----
| NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY. |
  
```

```

-----
| CALIB (0306) | Area (ha)= 283.97 | Curve Number (CN)= 52.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50 |
| U.H. Tp(hrs)= 6.44 |
-----
| Unit Hyd Qpeak (cms)= 0.753 |
-----
| PEAK FLOW (cms)= 0.146 (i) |
| TIME TO PEAK (hrs)= 14.000 |
| RUNOFF VOLUME (mm)= 5.040 |
| TOTAL RAINFALL (mm)= 42.000 |
| RUNOFF COEFFICIENT = 0.120 |
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0286) | Area (ha)= 260.51 | Curve Number (CN)= 84.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50 |
  
```

U.H. Tp(hrs)= 1.16

Unit Hyd Qpeak (cms)= 3.834

```

| PEAK FLOW (cms)= 1.732 (i) |
| TIME TO PEAK (hrs)= 7.750 |
| RUNOFF VOLUME (mm)= 15.903 |
| TOTAL RAINFALL (mm)= 42.000 |
| RUNOFF COEFFICIENT = 0.379 |
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0282) | Area (ha)= 449.38 | Curve Number (CN)= 77.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50 |
| U.H. Tp(hrs)= 1.47 |
  
```

Unit Hyd Qpeak (cms)= 5.226

```

| PEAK FLOW (cms)= 1.868 (i) |
| TIME TO PEAK (hrs)= 8.250 |
| RUNOFF VOLUME (mm)= 12.059 |
| TOTAL RAINFALL (mm)= 42.000 |
| RUNOFF COEFFICIENT = 0.287 |
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0284) | Area (ha)= 78.93 | Curve Number (CN)= 84.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50 |
| U.H. Tp(hrs)= 0.57 |
  
```

Unit Hyd Qpeak (cms)= 2.344

```

| PEAK FLOW (cms)= 0.907 (i) |
| TIME TO PEAK (hrs)= 6.500 |
| RUNOFF VOLUME (mm)= 15.663 |
| TOTAL RAINFALL (mm)= 42.000 |
| RUNOFF COEFFICIENT = 0.373 |
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8388) |
| 1 + 2 + 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) | |
|---|---|---|---|---|
| ID1= 1 (0282): | 449.38 | 1.868 | 8.25 | 12.06 |
| + ID2= 2 (0284): | 78.93 | 0.907 | 6.50 | 15.66 |
|-----|-----|-----|-----|
| ID = 3 (8388): | 528.31 | 2.560 | 7.50 | 12.60 |
-----
| NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY. |
  
```

```

-----
| ROUTE CHN (6286) |
| IN= 2--> OUT= 1 |
-----
| Routing time step (min)'= 15.00 |
  
```

```

-----
| <----- DATA FOR SECTION (2861.0) -----> |
| Distance | Elevation | Manning |
|-----|-----|-----|
| 0.00 | 233.00 | 0.0450 |
| 20.58 | 228.51 | 0.0450 |
| 41.17 | 227.74 | 0.0450 |
| 51.46 | 227.41 | 0.0450 |
| 97.77 | 225.96 | 0.0450 |
| 149.23 | 223.94 | 0.0450 |
| 200.69 | 220.84 | 0.0450 |
| 226.42 | 220.66 | 0.0450 |
| 238.85 | 220.22 | 0.0450 |
| 241.35 | 220.01 | 0.0450 / 0.0350 |
| 241.85 | 219.70 | 0.0350 |
| 245.85 | 219.72 | 0.0350 |
| 246.35 | 220.06 | 0.0350 |
| 248.85 | 220.23 | 0.0350 |
| 303.60 | 221.64 | 0.0350 |
| 380.79 | 224.98 | 0.0450 |
| 432.25 | 229.54 | 0.0450 |
| 457.98 | 233.33 | 0.0450 |
| 483.71 | 234.27 | 0.0450 |
| 509.44 | 233.81 | 0.0450 |
  
```

```

-----
| <----- TRAVEL TIME TABLE -----> |
| DEPTH | ELEV | VOLUME | FLOW RATE | VELOCITY | TRAV.TIME |
| (m) | (m) | (cu.m.) | (cms) | (m/s) | (min) |
|-----|-----|-----|-----|-----|-----|
| 0.31 | 220.01 | .382E+04 | 0.8 | 0.57 | 82.89 |
| 1.06 | 220.76 | .533E+05 | 15.1 | 0.66 | 72.51 |
| 1.81 | 221.51 | .240E+06 | 88.6 | 1.05 | 45.21 |
| 2.56 | 222.26 | .510E+06 | 261.7 | 1.46 | 32.47 |
| 3.31 | 223.01 | .843E+06 | 533.8 | 1.81 | 26.34 |
  
```


4.06	223.76	.124E+07	913.6	2.10	22.64
4.81	224.51	.171E+07	1399.0	2.34	20.33
5.56	225.26	.225E+07	2051.0	2.60	18.27
6.31	226.01	.285E+07	2907.0	2.91	16.35
7.06	226.76	.352E+07	3902.5	3.16	15.04
7.82	227.52	.426E+07	5059.9	3.39	14.01
8.57	228.27	.507E+07	6433.0	3.62	13.13
9.32	229.02	.593E+07	8069.2	3.89	12.24
10.07	229.77	.681E+07	9926.7	4.16	11.44
10.82	230.52	.772E+07	11962.8	4.42	10.75
11.57	231.27	.864E+07	14167.1	4.68	10.17
12.32	232.02	.959E+07	16537.1	4.92	9.66
13.07	232.77	.105E+08	19070.7	5.16	9.22
13.82	233.52	.115E+08	21790.4	5.39	8.82

```

<---- hydrograph ----> <-pipe / channel->
AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
(ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
INFLOW : ID= 2 (8388) 528.31 2.56 7.50 12.60 0.43 0.59
OUTFLOW: ID= 1 (6286) 528.31 2.18 9.25 12.60 0.40 0.58

```

```

-----
| ADD HYD (8386) |
| 1 + 2 = 3 |
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0286): 260.51 1.732 7.75 15.90
+ ID2= 2 (6286): 528.31 2.176 9.25 12.60
=====
ID = 3 (8386): 788.82 3.749 8.50 13.69

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0302) | Area (ha)= 473.90 Curve Number (CN)= 58.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.66

```

Unit Hyd Qpeak (cms)= 4.874

```

PEAK FLOW (cms)= 0.899 (1)
TIME TO PEAK (hrs)= 8.750
RUNOFF VOLUME (mm)= 6.167
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.147

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0300) | Area (ha)= 258.93 Curve Number (CN)= 52.0
|ID= 1 DT=15.0 min | Ia (mm)= 4.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.03

```

Unit Hyd Qpeak (cms)= 4.292

```

PEAK FLOW (cms)= 0.599 (1)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 5.248
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.125

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0298) | Area (ha)= 330.51 Curve Number (CN)= 45.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.26

```

Unit Hyd Qpeak (cms)= 4.461

```

PEAK FLOW (cms)= 0.488 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 3.912
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.093

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8395) |
| 1 + 2 = 3 |
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0298): 330.51 0.488 8.00 3.91
+ ID2= 2 (0300): 258.93 0.599 7.50 5.25
=====

```

ID = 3 (8395): 589.44 1.081 8.00 4.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0288) | Area (ha)= 340.83 Curve Number (CN)= 78.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 2.21

```

Unit Hyd Qpeak (cms)= 2.629

PEAK FLOW (cms)= 1.075 (1)

TIME TO PEAK (hrs)= 9.750

RUNOFF VOLUME (mm)= 12.562

TOTAL RAINFALL (mm)= 42.000

RUNOFF COEFFICIENT = 0.299

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0290) | Area (ha)= 269.18 Curve Number (CN)= 78.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.07

```

Unit Hyd Qpeak (cms)= 4.279

PEAK FLOW (cms)= 1.470 (1)

TIME TO PEAK (hrs)= 7.500

RUNOFF VOLUME (mm)= 12.485

TOTAL RAINFALL (mm)= 42.000

RUNOFF COEFFICIENT = 0.297

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8397) |
| 1 + 2 = 3 |
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0288): 340.83 1.075 9.75 12.56
+ ID2= 2 (0290): 269.18 1.470 7.50 12.49
=====
ID = 3 (8397): 610.01 2.456 8.00 12.53

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0296) | Area (ha)= 293.65 Curve Number (CN)= 76.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.13

```

Unit Hyd Qpeak (cms)= 4.437

PEAK FLOW (cms)= 1.427 (1)

TIME TO PEAK (hrs)= 7.750

RUNOFF VOLUME (mm)= 11.580

TOTAL RAINFALL (mm)= 42.000

RUNOFF COEFFICIENT = 0.276

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0292) | Area (ha)= 738.49 Curve Number (CN)= 68.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.52

```

Unit Hyd Qpeak (cms)= 8.289

PEAK FLOW (cms)= 2.131 (1)

TIME TO PEAK (hrs)= 8.250

RUNOFF VOLUME (mm)= 8.698

TOTAL RAINFALL (mm)= 42.000

RUNOFF COEFFICIENT = 0.207

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0294) | Area (ha)= 274.15 Curve Number (CN)= 57.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.87

```

Unit Hyd Qpeak (cms)= 5.367

PEAK FLOW (cms)= 0.809 (1)

TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 5.913
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.141

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8398)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0292):	738.49	2.131	8.25	8.70
+ ID2= 2 (0294):	274.15	0.809	7.25	5.91
=====				
ID = 3 (8398):	1012.64	2.892	8.00	7.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6296)
 IN= 2--> OUT= 1
 Routing time step (min)= 15.00

----- DATA FOR SECTION (2961.0) -----

Distance	Elevation	Manning	
0.00	243.98	0.0400	
42.59	243.18	0.0400	
85.17	241.81	0.0400	
120.66	240.50	0.0400	
156.15	239.56	0.0400	
198.74	236.15	0.0400	
237.78	234.01	0.0400	
241.33	233.82	0.0400	
248.77	233.12	0.0400 / 0.0400	Main Channel
249.87	232.32	0.0400	Main Channel
250.37	231.80	0.0400	Main Channel
250.87	232.23	0.0400	Main Channel
251.97	233.10	0.0400 / 0.0400	Main Channel
255.37	233.22	0.0400	
259.07	233.87	0.0400	
262.62	234.12	0.0400	
266.17	234.23	0.0400	
283.91	234.73	0.0400	
337.15	241.75	0.0400	
351.34	244.00	0.0400	

----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.43	232.23	.510E+03	0.1	0.53	81.07
0.87	232.67	.213E+04	0.7	0.83	51.49
1.30	233.10	.501E+04	2.2	1.10	38.75
1.98	233.78	.242E+05	12.6	1.34	31.93
2.66	234.46	.738E+05	42.1	1.46	29.21
3.34	235.14	.177E+06	127.9	1.84	23.13
4.02	235.82	.314E+06	281.4	2.29	18.61
4.70	236.50	.481E+06	509.4	2.71	15.73
5.38	237.18	.672E+06	815.6	3.11	13.73
6.06	237.86	.886E+06	1200.7	3.46	12.30
6.74	238.54	.112E+07	1669.4	3.80	11.23
7.42	239.22	.139E+07	2226.9	4.11	10.38
8.10	239.90	.168E+07	2808.4	4.28	9.95
8.78	240.58	.202E+07	3470.4	4.40	9.68
9.46	241.26	.240E+07	4331.7	4.61	9.24
10.14	241.94	.283E+07	5335.2	4.83	8.84
10.82	242.62	.330E+07	6482.6	5.03	8.48
11.50	243.30	.381E+07	7764.1	5.21	8.18
12.18	243.98	.439E+07	9101.2	5.31	8.04

----- hydrograph -----> <-pipe / channel->

	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8398)	1012.64	2.89	8.00	7.94	1.35	1.11
OUTFLOW: ID= 1 (6296)	1012.64	2.79	8.75	7.94	1.34	1.11

ADD HYD (8396)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0296):	293.65	1.427	7.75	11.58
+ ID2= 2 (6296):	1012.64	2.793	8.75	7.94
=====				
ID = 3 (8396):	1306.29	4.146	8.50	8.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8394)

1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8396):	1306.29	4.146	8.50	8.76
+ ID2= 2 (8397):	610.01	2.456	8.00	12.53
=====				
ID = 3 (8394):	1916.30	6.591	8.25	9.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8392)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8394):	1916.30	6.591	8.25	9.96
+ ID2= 2 (8393):	589.44	1.081	8.00	4.50
=====				
ID = 3 (8392):	2505.74	7.655	8.25	8.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6302)
 IN= 2--> OUT= 1
 Routing time step (min)= 15.00

----- DATA FOR SECTION (3021.0) ----->

Distance	Elevation	Manning	
0.00	228.10	0.0400	
18.47	227.12	0.0400	
36.95	226.12	0.0400	
46.18	225.84	0.0400	
55.42	225.58	0.0400	
272.47	222.88	0.0400	
277.09	222.76	0.0400	
281.71	222.58	0.0400	
288.54	222.18	0.0400 / 0.0300	Main Channel
288.64	221.00	0.0300	Main Channel
290.94	221.00	0.0300	Main Channel
291.04	221.00	0.0300	Main Channel
291.54	222.75	0.0300 / 0.0400	Main Channel
300.18	222.83	0.0400	
304.80	223.04	0.0400	
309.42	223.25	0.0400	
318.65	223.69	0.0400	
360.22	225.57	0.0400	
397.16	227.60	0.0400	
457.20	228.35	0.0400	

----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.29	221.29	.228E+04	0.2	0.32	165.63
0.59	221.59	.465E+04	0.7	0.45	116.15
0.88	221.88	.713E+04	1.2	0.54	96.87
1.18	222.18	.971E+04	1.9	0.61	86.20
1.57	222.57	.175E+05	3.3	0.59	88.86
1.97	222.97	.427E+05	6.4	0.47	111.39
2.36	223.36	.115E+06	16.1	0.44	119.14
2.76	223.76	.237E+06	37.1	0.49	106.49
3.15	224.15	.409E+06	72.9	0.56	93.40
3.55	224.55	.631E+06	127.0	0.63	82.76
3.94	224.94	.903E+06	202.4	0.70	74.37
4.34	225.34	.123E+07	301.8	0.77	67.68
4.73	225.73	.160E+07	433.5	0.85	61.37
5.13	226.13	.200E+07	602.2	0.95	55.29
5.52	226.52	.242E+07	804.7	1.05	50.13
5.92	226.92	.288E+07	1034.2	1.14	46.11
6.31	227.31	.332E+07	1290.5	1.22	42.88
6.71	227.71	.380E+07	1560.9	1.29	40.55
7.10	228.10	.432E+07	1835.6	1.34	39.19

----- hydrograph -----> <-pipe / channel->

	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8392)	2505.74	7.66	8.25	8.68	2.02	0.47
OUTFLOW: ID= 1 (6302)	2505.74	6.18	10.50	8.68	1.94	0.48

ADD HYD (8390)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0302):	473.90	0.899	8.75	6.17
+ ID2= 2 (6302):	2505.74	6.182	10.50	8.68
=====				
ID = 3 (8390):	2979.64	7.014	10.50	8.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8348) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID# 1 (8345): 788.62 3.749 8.50 13.69 |
| + ID# 2 (8390): 2979.64 7.014 10.50 8.28 |
|=====|
| ID = 3 (8348): 3768.46 10.463 9.75 9.41 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0304) | Area (ha)= 292.37 Curve Number (CN)= 63.0 |
| ID# 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|=====|
| U.H. Tp(hrs)= 2.78 |
-----

```

Unit Hyd Qpeak (cms)= 1.793

```

PEAK FLOW (cms)= 0.450 (1)
TIME TO PEAK (hrs)= 10.500
RUNOFF VOLUME (mm)= 7.337
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.175

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0280) | Area (ha)= 299.86 Curve Number (CN)= 82.0 |
| ID# 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|=====|
| U.H. Tp(hrs)= 0.85 |
-----

```

Unit Hyd Qpeak (cms)= 6.009

```

PEAK FLOW (cms)= 2.320 (1)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 14.568
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.347

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0278) | Area (ha)= 485.49 Curve Number (CN)= 82.0 |
| ID# 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|=====|
| U.H. Tp(hrs)= 1.82 |
-----

```

Unit Hyd Qpeak (cms)= 5.453

```

PEAK FLOW (cms)= 2.409 (1)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 14.678
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.349

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0276) | Area (ha)= 90.89 Curve Number (CN)= 79.0 |
| ID# 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|=====|
| U.H. Tp(hrs)= 0.67 |
-----

```

Unit Hyd Qpeak (cms)= 2.302

```

PEAK FLOW (cms)= 0.742 (1)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 12.858
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.306

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0274) | Area (ha)= 392.49 Curve Number (CN)= 75.0 |
| ID# 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|=====|
| U.H. Tp(hrs)= 1.08 |
-----

```

Unit Hyd Qpeak (cms)= 6.182

```

PEAK FLOW (cms)= 1.889 (1)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 11.150
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.265

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8360) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID# 1 (0274): 392.49 1.889 7.50 11.15 |
| + ID# 2 (0276): 90.89 0.742 6.75 12.86 |
|=====|
| ID = 3 (8360): 483.38 2.585 7.25 11.47 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8358) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID# 1 (0278): 485.49 2.409 8.25 14.68 |
| + ID# 2 (8360): 483.38 2.585 7.25 11.47 |
|=====|
| ID = 3 (8358): 968.87 4.901 7.75 13.08 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6280) |
| IN# 2--> OUT# 1 | Routing time step (min)= 15.00
-----

```

<----- DATA FOR SECTION (2801.0) ----->						
Distance	Elevation	Manning				
0.00	241.14	0.0500				
13.32	240.80	0.0500				
39.95	240.07	0.0500				
96.54	236.21	0.0500				
113.19	234.15	0.0500				
123.18	232.35	0.0500				
143.15	225.80	0.0500				
149.81	225.62	0.0500				
153.14	225.40	0.0500				
157.30	224.76	0.0500 / 0.0300	Main Channel			
159.80	224.26	0.0300	Main Channel			
162.30	224.85	0.0300	Main Channel			
162.85	224.97	0.0300 / 0.0500	Main Channel			
167.80	225.05	0.0500				
186.43	229.14	0.0500				
213.06	234.75	0.0500				
236.37	237.09	0.0500				
266.33	237.31	0.0500				
292.96	237.83	0.0500				
329.58	241.50	0.0500				

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.50	224.76	.263E+04	0.8	0.65	58.31
1.36	225.62	.278E+05	13.4	1.10	34.54
2.22	226.48	.855E+05	52.6	1.40	27.09
3.09	227.35	.157E+06	120.4	1.74	21.78
3.95	228.21	.242E+06	216.3	2.04	18.64
4.81	229.07	.340E+06	341.8	2.29	16.56
5.67	229.93	.450E+06	498.3	2.52	15.06
6.53	230.79	.574E+06	688.3	2.73	13.90
7.40	231.66	.711E+06	913.8	2.93	12.97
8.26	232.52	.861E+06	1173.4	3.11	12.23
9.12	233.38	.103E+07	1461.3	3.24	11.72
9.98	234.24	.121E+07	1793.3	3.37	11.26
10.85	235.11	.142E+07	2141.0	3.45	11.02
11.71	235.97	.165E+07	2530.4	3.50	10.86
12.57	236.83	.192E+07	2954.0	3.51	10.81
13.43	237.69	.226E+07	3160.3	3.18	11.93
14.29	238.55	.271E+07	3869.4	3.26	11.67
15.16	239.42	.320E+07	4740.3	3.38	11.24
16.02	240.28	.373E+07	5686.4	3.48	10.92

```

-----
|<---- hydrograph ----> |<-pipe / channel->|
| AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL |
| (ha) (cms) (hrs) (mm) (m) (m/s) |
| INFLOW : ID# 2 (8358) 968.87 4.90 7.75 13.08 0.78 0.75 |
| OUTFLOW: ID# 1 (6280) 968.87 4.56 8.75 13.08 0.76 0.74 |
-----

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-----
| ADD HYD (8354) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID# 1 (0280): 299.86 2.320 7.00 14.57 |
| + ID# 2 (6280): 968.87 4.564 8.75 13.08 |
|=====|
| ID = 3 (8354): 1268.73 6.519 8.25 13.43 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHYD (0272) Area (ha)= 157.38 Curve Number (CN)= 75.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.09

Unit Hyd Qpeak (cms)= 2.456

PEAK FLOW (cms)= 0.753 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 11.152
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.266

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0270) Area (ha)= 243.61 Curve Number (CN)= 81.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
U.H. Tp(hrs)= 0.87

Unit Hyd Qpeak (cms)= 3.429

PEAK FLOW (cms)= 1.320 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 13.984
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.333

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0268) Area (ha)= 215.76 Curve Number (CN)= 75.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
U.H. Tp(hrs)= 0.69

Unit Hyd Qpeak (cms)= 3.807

PEAK FLOW (cms)= 1.091 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 11.050
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.263

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0264) Area (ha)= 353.96 Curve Number (CN)= 69.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
U.H. Tp(hrs)= 1.30

Unit Hyd Qpeak (cms)= 3.313

PEAK FLOW (cms)= 0.888 (i)
TIME TO PEAK (hrs)= 8.500
RUNOFF VOLUME (mm)= 8.987
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.214

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0266) Area (ha)= 508.09 Curve Number (CN)= 64.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
U.H. Tp(hrs)= 1.63

Unit Hyd Qpeak (cms)= 3.786

PEAK FLOW (cms)= 0.901 (i)
TIME TO PEAK (hrs)= 9.750
RUNOFF VOLUME (mm)= 7.566
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.180

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8380)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0264): 353.96 0.888 8.50 8.99

+ ID2= 2 (0266): 508.09 0.901 9.75 7.57
ID = 3 (8380): 862.05 1.777 9.00 8.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (0266) |
IN= 2---> OUT= 1 | Routing time step (min)'= 15.00

Table with 4 columns: Distance, Elevation, Manning, and Main Channel. Rows show data points for section 2681.0.

TRAVEL TIME TABLE
Columns: DEPTH (m), ELEV (m), VOLUME (cu.m.), FLOW RATE (cms), VELOCITY (m/s), TRAV.TIME (min). Rows show data points for section 2681.0.

Summary table with columns: AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm), MAX DEPTH (m), MAX VEL (m/s). Rows show INFLOW and OUTFLOW data.

ADD HYD (8382)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0268): 215.76 1.091 7.00 11.05
+ ID2= 2 (0266): 508.09 0.901 9.75 7.57
ID = 3 (8382): 1077.81 2.539 9.00 8.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (0270) |
IN= 2---> OUT= 1 | Routing time step (min)'= 15.00

Table with 4 columns: Distance, Elevation, Manning, and Main Channel. Rows show data points for section 2701.0.

69.68	233.95	0.0300	Main Channel
71.21	234.48	0.0300 / 0.0500	Main Channel
81.38	236.44	0.0500	
95.63	236.66	0.0500	
120.04	237.00	0.0500	
148.53	241.77	0.0500	
158.70	242.34	0.0500	
187.18	244.03	0.0500	
201.42	244.36	0.0500	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.45	234.34	.217E+04	0.7	0.99	52.83
0.98	234.87	.966E+04	4.6	1.49	35.20
1.50	235.39	.250E+05	13.7	1.73	30.41
2.03	235.92	.473E+05	29.5	1.95	26.87
2.56	236.45	.772E+05	52.5	2.14	24.50
3.09	236.98	.142E+06	82.4	1.83	28.73
3.61	237.50	.246E+06	156.1	2.03	25.91
4.14	238.03	.357E+06	262.0	2.31	22.73
4.67	238.56	.477E+06	392.1	2.59	20.27
5.20	239.09	.604E+06	547.8	2.86	18.38
5.72	239.61	.739E+06	729.2	3.11	16.90
6.25	240.14	.882E+06	936.5	3.34	15.70
6.78	240.67	.103E+07	1169.7	3.57	14.72
7.31	241.20	.119E+07	1425.9	3.77	13.94
7.83	241.72	.136E+07	1710.4	3.96	13.26
8.36	242.25	.154E+07	1967.9	4.02	13.07
8.89	242.78	.174E+07	2264.9	4.09	12.83
9.42	243.31	.196E+07	2589.3	4.15	12.65
9.94	243.83	.221E+07	2953.5	4.22	12.46

----- hydrograph ----- <-pipe / channel-->

INFLOW : ID= 2 (8382)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
1077.81	2.54	9.00	8.73	0.70	1.18	
OUTFLOW : ID= 1 (6270)	1077.81	2.49	9.75	8.73	0.69	1.18

ADD HYD (8384)

ID#	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID# 1 (0270):	243.61	1.320	7.50	13.98
+ ID# 2 (6270):	1077.81	2.492	9.75	8.73
ID = 3 (8384):	1321.42	3.620	9.25	9.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB

NASHVD (0262)	Area (ha)= 341.31	Curve Number (CN)= 82.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.30
	U.H. Tp(hrs)= 1.01	

Unit Hyd Qpeak (cms)= 4.094

PEAK FLOW (cms)= 1.720 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 14.597
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.348

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

NASHVD (0260)	Area (ha)= 476.24	Curve Number (CN)= 82.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.30
	U.H. Tp(hrs)= 1.33	

Unit Hyd Qpeak (cms)= 4.360

PEAK FLOW (cms)= 1.943 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 14.645
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.349

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

NASHVD (0258)	Area (ha)= 181.99	Curve Number (CN)= 79.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.30
	U.H. Tp(hrs)= 1.18	

Unit Hyd Qpeak (cms)= 1.881

PEAK FLOW (cms)= 0.722 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 12.980
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.309

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

NASHVD (0252)	Area (ha)= 319.99	Curve Number (CN)= 73.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.30
	U.H. Tp(hrs)= 1.04	

Unit Hyd Qpeak (cms)= 3.761

PEAK FLOW (cms)= 1.110 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 10.343
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.246

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

NASHVD (0256)	Area (ha)= 145.79	Curve Number (CN)= 67.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.30
	U.H. Tp(hrs)= 1.04	

Unit Hyd Qpeak (cms)= 1.707

PEAK FLOW (cms)= 0.405 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 8.355
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.199

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

NASHVD (0254)	Area (ha)= 403.00	Curve Number (CN)= 55.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.30
	U.H. Tp(hrs)= 1.22	

Unit Hyd Qpeak (cms)= 4.028

PEAK FLOW (cms)= 0.650 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 5.543
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.132

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6256)

IN= 2--> OUT= 1 Routing time step (min)= 15.00

----- DATA FOR SECTION (2561.0) -----

Distance	Elevation	Manning
0.00	276.07	0.0400
11.68	273.71	0.0400
23.36	271.35	0.0400
29.19	270.30	0.0400
35.03	269.44	0.0400
55.47	267.90	0.0400
78.82	266.24	0.0400
90.50	265.63	0.0400
102.18	265.40	0.0400
105.10	264.95	0.0400 / 0.0350
108.02	264.39	0.0350
110.94	264.72	0.0350
113.86	265.19	0.0350 / 0.0400
116.78	265.49	0.0400
143.05	268.24	0.0400
172.25	270.53	0.0400
207.38	271.95	0.0400
233.55	273.72	0.0400
256.91	274.98	0.0400
289.02	275.97	0.0400

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.56	264.95	.723E+04	1.9	0.90	62.51
1.14	265.53	.307E+05	13.5	1.49	37.92
1.72	266.11	.951E+05	48.6	1.73	32.62
2.30	266.69	.191E+06	117.8	2.09	27.03

2.88	267.27	.314E+06	225.2	2.43	23.24
3.46	267.85	.464E+06	375.5	2.74	20.59
4.04	268.43	.641E+06	573.2	3.03	18.62
4.62	269.01	.846E+06	821.9	3.29	17.15
5.20	269.59	.108E+07	1135.0	3.56	15.87
5.78	270.17	.134E+07	1525.1	3.86	14.64
6.36	270.75	.162E+07	1971.1	4.12	13.71
6.94	271.33	.194E+07	2474.0	4.33	13.04
7.52	271.91	.228E+07	3065.0	4.55	12.42
8.10	272.49	.266E+07	3773.4	4.80	11.75
8.68	273.07	.306E+07	4566.3	5.05	11.17
9.26	273.65	.348E+07	5443.7	5.30	10.66
9.84	274.23	.393E+07	6389.6	5.51	10.25
10.42	274.81	.440E+07	7426.2	5.72	9.88
11.00	275.39	.490E+07	8490.1	5.86	9.63

<---- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0254) 403.00 0.65 8.25 5.54 0.13 0.90
 OUTFLOW : ID= 1 (6256) 403.00 0.62 10.00 5.54 0.18 0.90

ADD HYD (8370)
 1 + 2 = 3
 ID1= 1 (0256): 145.79 0.405 8.00 8.36
 + ID2= 2 (6256): 403.00 0.617 10.00 5.54
 ID = 3 (8370): 548.79 0.986 9.50 6.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0250) Area (ha)= 192.88 Curve Number (CN)= 70.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.22

Unit Hyd Qpeak (cms)= 1.930

PEAK FLOW (cms)= 0.529 (1)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 9.304
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.222

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0246) Area (ha)= 759.61 Curve Number (CN)= 55.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.81

Unit Hyd Qpeak (cms)= 5.121

PEAK FLOW (cms)= 0.922 (1)
 TIME TO PEAK (hrs)= 10.000
 RUNOFF VOLUME (mm)= 5.563
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.132

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0248) Area (ha)= 146.04 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 0.78

Unit Hyd Qpeak (cms)= 2.271

PEAK FLOW (cms)= 0.446 (1)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 7.494
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.178

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8364)
 1 + 2 = 3
 ID1= 1 (0246): 759.61 0.922 10.00 5.56
 + ID2= 2 (0248): 146.04 0.446 7.50 7.49

ROUTE CHN (6250)
 ID = 3 (8364): 905.65 1.303 8.50 5.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6250)
 ID= 2 -> OUTF= 1 Routing time step (min)= 15.00

DATA FOR SECTION (2501.0)

Distance	Elevation	Manning
0.00	269.96	0.0500
8.56	268.55	0.0500
17.13	266.91	0.0500
21.41	266.13	0.0500
40.68	263.15	0.0500
62.09	260.75	0.0500
85.64	258.02	0.0500
88.20	257.69	0.0500
93.20	257.05	0.0500 / 0.0350
93.45	256.88	0.0350
94.20	256.56	0.0350
94.95	256.83	0.0350
95.20	257.08	0.0350 / 0.0500
100.62	257.45	0.0500
115.61	258.57	0.0500
139.16	260.43	0.0500
152.01	261.95	0.0500
171.27	264.63	0.0500
188.40	267.90	0.0500
211.95	274.18	0.0500

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.49	257.05	.205E+04	0.4	0.68	89.65
1.17	257.73	.248E+05	5.7	0.85	72.16
1.85	258.41	.841E+05	25.2	1.10	55.60
2.53	259.09	.180E+06	66.5	1.36	45.14
3.21	259.77	.312E+06	135.7	1.60	38.36
3.89	260.45	.481E+06	238.3	1.82	33.61
4.57	261.13	.681E+06	383.5	2.07	29.60
5.25	261.81	.911E+06	568.9	2.29	26.70
5.93	262.49	.117E+07	800.5	2.51	24.36
6.61	263.17	.146E+07	1079.0	2.72	22.49
7.28	263.84	.177E+07	1413.4	2.94	20.85
7.96	264.52	.210E+07	1797.0	3.14	19.50
8.64	265.20	.246E+07	2241.6	3.35	18.28
9.32	265.88	.283E+07	2739.9	3.55	17.25
10.00	266.56	.323E+07	3295.7	3.75	16.34
10.68	267.24	.364E+07	3908.9	3.94	15.54
11.36	267.92	.408E+07	4578.5	4.12	14.84
12.04	268.60	.453E+07	5320.1	4.32	14.18
12.72	269.28	.499E+07	6103.1	4.50	13.61

<---- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8364) 905.65 1.30 8.50 5.87 0.61 0.71
 OUTFLOW : ID= 1 (6250) 905.65 1.21 10.75 5.87 0.59 0.70

ADD HYD (8366)
 1 + 2 = 3
 ID1= 1 (0250): 192.88 0.529 8.25 9.30
 + ID2= 2 (6250): 905.65 1.210 10.75 5.87
 ID = 3 (8366): 1098.53 1.691 10.25 6.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8368)
 1 + 2 = 3
 ID1= 1 (8366): 1098.53 1.691 10.25 6.48
 + ID2= 2 (8370): 548.79 0.986 9.50 6.29
 ID = 3 (8368): 1647.32 2.669 10.00 6.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8372)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)

```

ID1= 1 (0252): 319.99 1.110 8.00 10.34
+ ID2= 2 (8368): 1647.32 2.669 10.00 6.41
=====
ID = 3 (8372): 1967.31 3.671 9.50 7.05

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6258) |
IN= 2--> OUT= 1 |

```

----- DATA FOR SECTION (2581.0) ----->

Distance	Elevation	Manning	
0.00	252.88	0.0500	
15.47	251.89	0.0500	
46.41	248.45	0.0500	
73.48	245.81	0.0500	
88.95	243.75	0.0500	
112.15	242.00	0.0500	
135.35	240.23	0.0500	
162.42	239.76	0.0500	
170.97	239.52	0.0500 / 0.0350	Main Channel
171.58	239.03	0.0350	Main Channel
174.02	239.03	0.0350	Main Channel
176.46	239.03	0.0350	Main Channel
177.07	239.52	0.0350 / 0.0500	Main Channel
185.63	239.67	0.0500	
208.83	239.87	0.0500	
239.77	240.14	0.0500	
274.57	244.93	0.0500	
336.45	249.51	0.0500	
363.52	249.77	0.0500	
382.85	251.78	0.0500	

----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.49	239.52	.135E+05	2.5	0.92	91.32
1.14	240.17	.175E+06	25.6	0.73	114.22
1.78	240.81	.532E+06	123.5	1.17	71.85
2.43	241.46	.932E+06	285.3	1.54	54.48
3.07	242.10	.138E+07	505.5	1.85	45.25
3.72	242.75	.186E+07	786.8	2.12	39.42
4.36	243.39	.239E+07	1127.3	2.37	35.33
5.01	244.04	.296E+07	1535.8	2.61	32.12
5.65	244.68	.358E+07	2014.3	2.84	29.48
6.30	245.33	.420E+07	2535.9	3.03	27.60
6.94	245.97	.488E+07	3108.4	3.20	26.16
7.59	246.62	.561E+07	3740.6	3.35	24.98
8.23	247.26	.638E+07	4446.7	3.50	23.93
8.88	247.91	.721E+07	5229.7	3.64	22.98
9.52	248.55	.809E+07	6094.3	3.78	22.12
10.17	249.20	.901E+07	7050.4	3.93	21.31
10.81	249.84	.100E+08	7773.0	3.90	21.46
11.46	250.49	.111E+08	8998.8	4.07	20.56
12.10	251.13	.122E+08	10318.2	4.24	19.75

----- hydrograph -----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLW : ID= 2 (8372) 1967.31	3.67	9.50	7.05	0.52	0.90
OUTFLOW: ID= 1 (6258) 1967.31	3.39	11.25	7.05	0.52	0.91

```

| ADD HYD (8374) |
| 1 + 2 = 3 |
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0258): 181.99 0.722 8.00 12.98
+ ID2= 2 (6258): 1967.31 3.386 11.25 7.05
=====
ID = 3 (8374): 2149.30 3.981 11.00 7.56

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ADD HYD (8376) |
| 1 + 2 = 3 |
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0260): 476.24 1.943 8.25 14.65
+ ID2= 2 (8374): 2149.30 3.981 11.00 7.56
=====
ID = 3 (8376): 2625.54 5.750 10.25 8.84

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ADD HYD (8378) |

```

```

| 1 + 2 = 3 |
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0262): 341.31 1.720 8.00 14.60
+ ID2= 2 (8376): 2625.54 5.750 10.25 8.84
=====
ID = 3 (8378): 2966.85 7.221 10.00 9.50

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ADD HYD (8362) |
| 1 + 2 = 3 |
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8378): 2966.85 7.221 10.00 9.50
+ ID2= 2 (8384): 1321.42 3.620 9.25 9.70
=====
ID = 3 (8362): 4288.27 10.785 9.75 9.56

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6272) |
IN= 2--> OUT= 1 |

```

----- DATA FOR SECTION (2721.0) ----->

Distance	Elevation	Manning	
0.00	231.01	0.0450	
23.01	223.65	0.0450	
34.81	222.46	0.0450	
51.77	222.11	0.0450	
69.02	221.87	0.0450	
161.06	221.92	0.0450	
166.81	221.91	0.0450	
172.56	221.89	0.0450	
180.57	221.40	0.0450 / 0.0300	Main Channel
181.57	220.60	0.0300	Main Channel
184.07	220.16	0.0300	Main Channel
195.57	221.85	0.0300 / 0.0450	Main Channel
201.32	221.82	0.0450	
207.07	221.77	0.0450	
212.83	221.72	0.0450	
253.09	222.52	0.0450	
408.40	222.65	0.0450	
460.17	223.20	0.0450	
517.69	224.84	0.0450	
569.46	232.57	0.0450	

----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.41	220.57	.189E+04	0.5	0.49	60.18
0.83	220.99	.697E+04	3.2	0.82	35.79
1.24	221.40	.145E+05	8.8	1.07	27.45
1.84	222.00	.643E+05	29.3	0.81	36.61
2.44	222.60	.202E+06	132.9	0.83	35.41
3.04	223.20	.707E+06	413.7	1.04	28.50
3.64	223.80	.118E+07	876.8	1.31	22.46
4.24	224.40	.168E+07	1494.8	1.57	18.75
4.84	225.00	.221E+07	2264.1	1.82	16.23
5.44	225.60	.274E+07	3196.3	2.06	14.29
6.04	226.20	.328E+07	4258.6	2.30	12.84
6.65	226.81	.383E+07	5444.7	2.52	11.72
7.25	227.41	.438E+07	6749.7	2.73	10.82
7.85	228.01	.494E+07	8169.4	2.93	10.08
8.45	228.61	.551E+07	9700.7	3.12	9.46
9.05	229.21	.608E+07	11340.8	3.30	8.94
9.65	229.81	.666E+07	13087.4	3.48	8.48
10.25	230.41	.724E+07	14938.5	3.65	8.08
10.85	231.01	.783E+07	16892.4	3.82	7.73

----- hydrograph -----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLW : ID= 2 (8362) 4288.27	10.79	9.75	9.56	1.30	1.04
OUTFLOW: ID= 1 (6272) 4288.27	10.70	10.25	9.56	1.30	1.04

```

| ADD HYD (8356) |
| 1 + 2 = 3 |
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0272): 157.38 0.753 7.75 11.15
+ ID2= 2 (6272): 4288.27 10.700 10.25 9.56
=====
ID = 3 (8356): 4445.65 11.269 10.00 9.62

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8352) |
| 1 + 2 = 3 |
-----
| ID1= 1 (8354): 1268.73 6.519 8.25 13.43
+ ID2= 2 (8356): 4445.65 11.269 10.00 9.62
=====
ID = 3 (8352): 5714.38 17.204 9.25 10.47

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6304) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)= 15.00

```

```

-----
| <----- DATA FOR SECTION (3041.0) ----->
| Distance Elevation Manning
0.00 232.08 0.0500
19.00 231.87 0.0500
38.00 231.33 0.0500
66.51 230.44 0.0500
104.51 228.25 0.0500
118.76 225.17 0.0500
128.26 219.86 0.0500
175.77 219.17 0.0500
185.27 218.90 0.0500 / 0.0300 Main Channel
185.52 218.65 0.0300 Main Channel
190.02 218.37 0.0300 Main Channel
194.52 218.60 0.0300 Main Channel
194.77 218.85 0.0300 / 0.0500 Main Channel
204.27 219.60 0.0500
299.28 220.91 0.0500
327.78 222.36 0.0500
375.28 225.71 0.0500
403.79 229.37 0.0500
432.29 230.43 0.0500
470.29 232.00 0.0500

```

```

-----
| <----- TRAVEL TIME TABLE ----->
| DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
| (m) (m) (cu.m.) (cms) (m/s) (min)
0.48 218.85 .121E+05 1.2 0.36 173.94
1.17 219.54 .839E+05 9.8 0.44 142.60
1.86 220.23 .323E+06 41.0 0.47 131.43
2.56 220.93 .708E+06 109.5 0.58 107.67
3.25 221.62 .118E+07 224.3 0.71 87.36
3.94 222.31 .168E+07 376.5 0.84 74.48
4.63 223.00 .222E+07 567.9 0.96 65.26
5.32 223.69 .279E+07 794.7 1.07 58.58
6.02 224.39 .339E+07 1056.2 1.17 53.52
6.71 225.08 .402E+07 1352.4 1.26 49.52
7.40 225.77 .468E+07 1677.3 1.34 46.46
8.09 226.46 .535E+07 2050.0 1.43 43.58
8.79 227.16 .607E+07 2457.9 1.52 41.14
9.48 227.85 .680E+07 2901.0 1.60 39.05
10.17 228.54 .755E+07 3352.7 1.66 37.53
10.86 229.23 .834E+07 3812.5 1.71 36.48
11.55 229.92 .919E+07 4241.8 1.73 36.13
12.25 230.62 .101E+08 4697.0 1.74 35.93
12.94 231.31 .111E+08 5186.3 1.74 35.83

```

```

-----
| <----- hydrograph -----> <-pipe / channel->
| AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
| (ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8352) 5714.38 17.20 9.25 10.47 1.34 0.45
OUTFLOW : ID= 1 (6304) 5714.38 14.25 12.00 10.47 1.27 0.44

```

```

-----
| ADD HYD (8350) |
| 1 + 2 = 3 |
-----
| ID1= 1 (0304): 292.37 0.450 10.50 7.34
+ ID2= 2 (6304): 5714.38 14.247 12.00 10.47
=====
ID = 3 (8350): 6006.75 14.687 12.00 10.31

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8346) |
| 1 + 2 = 3 |
-----
| ID1= 1 (8348): 3768.46 10.463 9.75 9.41
+ ID2= 2 (8350): 6006.75 14.687 12.00 10.31
=====
ID = 3 (8346): 9775.21 24.489 11.00 9.97

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8344) |
| 1 + 2 = 3 |
-----
| ID1= 1 (0306): 283.97 0.146 14.00 5.04
+ ID2= 2 (8346): 9775.21 24.489 11.00 9.97
=====
ID = 3 (8344): 10059.18 24.623 11.00 9.83

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| RESERVOIR (5509) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
| OUTFLOW STORAGE OUTFLOW STORAGE
| (cms) (ha.m.) (cms) (ha.m.)
0.0000 0.0000 | ***** 370.0451
41.0590 56.7403 | ***** 863.4386
48.1390 86.3439 | *****
56.6340 ***** | *****
67.9600 ***** | 0.0000 0.0000
-----
| AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (8344) ***** 24.623 11.00 9.83
OUTFLOW : ID= 1 (5509) ***** 19.107 14.50 9.83

```

PEAK FLOW REDUCTION [Qout/Qin](%)= 77.60
TIME SHIFT OF PEAK FLOW (min)=210.00
MAXIMUM STORAGE USED (ha.m.)= 26.4092

```

-----
| CALIB |
| NASHYD (0332) | Area (ha)= 393.44 Curve Number (CN)= 75.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 2.32

```

Unit Hyd Qpeak (cms)= 2.894
PEAK FLOW (cms)= 1.069 (i)
TIME TO PEAK (hrs)= 10.000
RUNOFF VOLUME (mm)= 11.219
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.267

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0330) | Area (ha)= 468.30 Curve Number (CN)= 80.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.09

```

Unit Hyd Qpeak (cms)= 7.335
PEAK FLOW (cms)= 2.745 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 13.500
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.321

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ROUTE CHN (6332) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)= 15.00

```

```

-----
| <----- DATA FOR SECTION (3321.0) ----->
| Distance Elevation Manning
0.00 234.00 0.0380
25.85 227.20 0.0380
96.94 226.44 0.0380
168.03 227.38 0.0380
219.73 225.62 0.0380
342.52 221.57 0.0380
368.37 221.42 0.0380
374.83 221.23 0.0380
379.79 220.98 0.0380 / 0.0300 Main Channel
380.29 220.47 0.0300 Main Channel
381.29 220.47 0.0300 Main Channel
382.79 220.47 0.0300 Main Channel
383.29 220.98 0.0300 / 0.0380 Main Channel
394.22 221.22 0.0380
400.68 221.33 0.0380
407.14 221.44 0.0380
491.16 225.70 0.0380
568.71 227.55 0.0380
607.49 230.14 0.0380
639.80 234.08 0.0380

```


----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.51	220.98	.554E+04	0.9	0.56	107.27
1.20	221.67	.885E+05	12.1	0.49	122.32
1.88	222.35	.309E+06	64.7	0.76	79.70
2.57	223.04	.615E+06	165.4	0.97	61.99
3.25	223.72	.101E+07	322.1	1.16	52.06
3.94	224.41	.148E+07	542.9	1.33	45.51
4.62	225.09	.204E+07	835.4	1.48	40.77
5.31	225.78	.269E+07	1202.8	1.62	37.27
5.99	226.46	.344E+07	1616.0	1.70	35.46
6.68	227.15	.447E+07	1932.0	1.56	38.56
7.36	227.83	.580E+07	2763.9	1.72	34.96
8.05	228.52	.718E+07	3861.9	1.95	30.97
8.73	229.20	.859E+07	5115.4	2.16	27.98
9.42	229.89	.100E+08	6519.2	2.35	25.64
10.10	230.57	.115E+08	8092.3	2.55	23.69
10.79	231.26	.130E+08	9821.5	2.73	22.05
11.47	231.94	.145E+08	11690.0	2.92	20.68
12.16	232.63	.160E+08	13694.9	3.09	19.52
12.84	233.31	.176E+08	15833.9	3.26	18.52

<---- hydrograph ----> <-pipe / channel-->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (0330)	468.30	2.74	7.50	13.50	0.63
OUTFLOW: ID= 1 (6332)	468.30	2.16	9.75	13.50	0.59

ADD HYD (8322)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID= 1 (0332):	393.44	1.069	10.00	11.22
+ ID= 2 (6332):	468.30	2.156	9.75	13.50
ID = 3 (8322):	861.74	3.223	9.75	12.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)
NASHYD (0328)	492.92	77.0		
ID= 1 DT=15.0 min	5.00	1.50		
U.H. Tp(hrs)=	1.91			
Unit Hyd Qpeak	(cms)=	4.411		
PEAK FLOW	(cms)=	1.670 (i)		
TIME TO PEAK	(hrs)=	9.000		
RUNOFF VOLUME	(mm)=	12.082		
TOTAL RAINFALL	(mm)=	42.000		
RUNOFF COEFFICIENT	=	0.288		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)
NASHYD (0326)	678.91	80.0		
ID= 1 DT=15.0 min	5.00	1.50		
U.H. Tp(hrs)=	1.95			
Unit Hyd Qpeak	(cms)=	5.941		
PEAK FLOW	(cms)=	2.547 (i)		
TIME TO PEAK	(hrs)=	9.000		
RUNOFF VOLUME	(mm)=	13.570		
TOTAL RAINFALL	(mm)=	42.000		
RUNOFF COEFFICIENT	=	0.323		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)
NASHYD (0324)	615.64	79.0		
ID= 1 DT=15.0 min	5.00	1.50		
U.H. Tp(hrs)=	1.90			
Unit Hyd Qpeak	(cms)=	5.544		
PEAK FLOW	(cms)=	2.268 (i)		
TIME TO PEAK	(hrs)=	9.000		
RUNOFF VOLUME	(mm)=	13.047		
TOTAL RAINFALL	(mm)=	42.000		
RUNOFF COEFFICIENT	=	0.311		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)
NASHYD (0320)	278.74	81.0		
ID= 1 DT=15.0 min	5.00	1.50		
U.H. Tp(hrs)=	1.36			
Unit Hyd Qpeak	(cms)=	3.499		
PEAK FLOW	(cms)=	1.448 (i)		
TIME TO PEAK	(hrs)=	8.000		
RUNOFF VOLUME	(mm)=	14.083		
TOTAL RAINFALL	(mm)=	42.000		
RUNOFF COEFFICIENT	=	0.335		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)
NASHYD (3182)	457.40	78.0		
ID= 1 DT=15.0 min	5.00	1.50		
U.H. Tp(hrs)=	1.49			
Unit Hyd Qpeak	(cms)=	5.259		
PEAK FLOW	(cms)=	1.960 (i)		
TIME TO PEAK	(hrs)=	8.250		
RUNOFF VOLUME	(mm)=	12.530		
TOTAL RAINFALL	(mm)=	42.000		
RUNOFF COEFFICIENT	=	0.298		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Dir. Conn.(%)
STANDHYD (3181)	26.00	81.00	81.00
ID= 1 DT=15.0 min			

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	21.06
Dep. Storage (mm)=	2.00
Average Slope (%)=	0.50
Length (m)=	416.33
Mannings n =	0.013

Max. Eff. Inten. (mm/hr)=	55.44	11.13
over (min)	15.00	45.00
Storage Coeff. (min)=	9.37 (ii)	35.11 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.09	0.03
PEAK FLOW (cms)=	2.79	0.09
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	40.00	12.13
TOTAL RAINFALL (mm)=	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.29

TOTALS

2.825 (iii)

6.00

34.70

42.00

0.83

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 77.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5318)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min	0.0000	0.0000	3.3800	0.7654
	0.0510	0.4423	4.1500	0.8231
	0.8700	0.5378	4.8750	0.8778
	1.9590	0.6105	5.2750	0.8878
	2.5620	0.6796	0.0000	0.0000

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (3181)	26.000	2.825	6.00
OUTFLOW: ID= 1 (5318)	26.000	0.751	6.50
PEAK FLOW REDUCTION [Qout/Qin](%)=	26.57		
TIME SHIFT OF PEAK FLOW (min)=	30.00		
MAXIMUM STORAGE USED (ha.m.)=	0.5295		

ADD HYD (8334)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID= 1 (3182):	457.40	1.960	8.25	12.53

+ ID2= 2 (5318): 26.00 0.751 6.50 34.62
 ID = 3 (8334): 483.40 2.212 6.75 13.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | ROUTE CHN (5320) |
 | IN= 2----> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3201.0) ----->

Distance	Elevation	Manning
0.00	249.00	0.0380
22.92	245.86	0.0380
45.83	244.87	0.0380
91.66	243.13	0.0380
126.03	239.53	0.0380
160.41	237.17	0.0380
166.14	237.06	0.0380
177.59	237.13	0.0380
183.32	237.20	0.0380
189.05	236.70	0.0380 /0.0350
193.05	235.89	0.0350
197.05	236.64	0.0350 /0.0380
200.51	236.74	0.0380
206.24	237.03	0.0380
246.34	238.82	0.0380
263.53	243.87	0.0380
389.56	247.64	0.0380
452.58	247.74	0.0380
498.41	248.60	0.0380
567.16	249.84	0.0380

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.37	236.26	.270E+04	0.4	0.58	107.96
0.75	236.64	.108E+05	2.6	0.92	68.01
1.48	237.37	.877E+05	27.4	1.17	53.35
2.20	238.09	.277E+06	122.1	1.65	37.83
2.93	238.82	.540E+06	299.7	2.08	30.01
3.66	239.55	.857E+06	587.8	2.57	24.29
4.39	240.28	.120E+07	977.8	3.04	20.52
5.11	241.00	.158E+07	1458.1	3.46	18.02
5.84	241.73	.198E+07	2028.6	3.84	16.23
6.57	242.46	.240E+07	2690.5	4.19	14.86
7.29	243.18	.285E+07	3431.8	4.50	13.84
8.02	243.91	.335E+07	4149.1	4.64	13.44
8.75	244.64	.393E+07	4745.6	4.51	13.80
9.47	245.36	.463E+07	5618.6	4.83	13.75
10.20	246.09	.545E+07	6781.8	4.66	13.39
10.93	246.82	.635E+07	8274.5	4.87	12.79
11.66	247.55	.733E+07	9980.0	5.10	12.23
12.38	248.27	.853E+07	11422.0	5.01	12.45
13.11	249.00	.989E+07	13749.7	5.20	11.98

<---- hydrograph ----> <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8334)	483.40	2.21	6.75	13.72	0.68
OUTFLOW: ID= 1 (6320)	483.40	1.92	9.50	13.72	0.63

 | ADD HYD (8332) |
 | 1 + 2 = 3 |
 | ID1= 1 (0320): 278.74 1.448 8.00 14.08
 + ID2= 2 (6320): 483.40 1.921 9.50 13.72
 ID = 3 (8332): 762.14 3.286 8.75 13.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | CALIB (0314) | Area (ha)= 165.20 Curve Number (CN)= 78.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.71

Unit Hyd Qpeak (cms)= 3.978
 PEAK FLOW (cms)= 1.241 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 12.387
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.295

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | ROUTE CHN (6316) |
 | IN= 2----> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3161.0) ----->

Distance	Elevation	Manning
0.00	248.54	0.0350
27.93	248.34	0.0350
41.89	246.61	0.0350
62.83	243.09	0.0350
132.65	239.00	0.0350
188.50	236.74	0.0350
195.48	236.54	0.0350
202.46	236.32	0.0350
205.48	236.14	0.0350 /0.0310
205.98	235.61	0.0310
207.98	235.25	0.0310
209.98	235.53	0.0310
210.48	236.00	0.0310 /0.0350
216.42	236.73	0.0350
259.31	239.09	0.0350
328.12	239.84	0.0350
439.83	241.47	0.0350
530.58	242.08	0.0350
586.43	242.93	0.0350
691.16	248.00	0.0350

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.37	235.62	.215E+04	0.7	0.77	53.88
0.75	236.00	.634E+04	3.5	1.36	30.45
1.46	236.71	.297E+05	19.3	1.62	25.66
2.16	237.41	.103E+06	75.8	1.84	22.60
2.87	238.12	.229E+06	202.8	2.21	18.79
3.57	238.82	.407E+06	423.3	2.59	16.03
4.28	239.53	.654E+06	727.7	2.77	14.97
4.99	240.24	.103E+07	1242.0	3.01	13.76
5.69	240.94	.151E+07	2002.3	3.31	12.54
6.40	241.65	.210E+07	2987.9	3.55	11.70
7.10	242.35	.286E+07	4302.8	3.75	11.07
7.81	243.06	.374E+07	6165.3	4.10	10.11
8.51	243.76	.468E+07	8642.4	4.60	9.03
9.22	244.47	.565E+07	11496.3	5.06	8.20
9.93	245.18	.666E+07	14714.1	5.50	7.54
10.63	245.88	.770E+07	18290.3	5.91	7.02
11.34	246.59	.877E+07	22221.9	6.30	6.58
12.04	247.29	.988E+07	26464.4	6.67	6.22
12.75	248.00	.110E+08	31061.3	7.01	5.91

<---- hydrograph ----> <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0314)	165.20	1.24	7.00	12.39	0.45
OUTFLOW: ID= 1 (6316)	165.20	1.06	8.00	12.39	0.43

 | CALIB (0316) | Area (ha)= 232.34 Curve Number (CN)= 82.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.01

Unit Hyd Qpeak (cms)= 3.923
 PEAK FLOW (cms)= 1.569 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 14.611
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.348

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | ADD HYD (8338) |
 | 1 + 2 = 3 |
 | ID1= 1 (0316): 232.34 1.569 7.25 14.61
 + ID2= 2 (6316): 165.20 1.059 8.00 12.39
 ID = 3 (8338): 397.54 2.608 7.75 13.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | CALIB (0312) | Area (ha)= 359.44 Curve Number (CN)= 80.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.36

Unit Hyd Qpeak (cms)= 4.529

PEAK FLOW (cms)= 1.796 (1)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 13.533
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.322

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0308) | Area (ha)= 529.30 Curve Number (CN)= 62.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.62
  
```

Unit Hyd Qpeak (cms)= 5.575

PEAK FLOW (cms)= 1.176 (1)
 TIME TO PEAK (hrs)= 8.500
 RUNOFF VOLUME (mm)= 7.070
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.168

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0310) | Area (ha)= 138.28 Curve Number (CN)= 65.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.76
  
```

Unit Hyd Qpeak (cms)= 3.102

PEAK FLOW (cms)= 0.603 (1)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 7.758
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.185

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8342) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (0308): 529.30 1.176 8.50 7.07
| + ID2= 2 (0310): 138.28 0.603 7.00 7.76
|-----|
| ID = 3 (8342): 667.58 1.702 8.00 7.21
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6312) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
  
```

<----- DATA FOR SECTION (3121.0) ----->

Distance	Elevation	Manning
0.00	265.94	0.0360
38.07	265.43	0.0360
59.82	263.98	0.0360
103.32	254.59	0.0360
157.70	252.16	0.0360
217.52	250.45	0.0360
233.84	247.69	0.0360
234.71	247.27	0.0360
239.71	246.38	0.0360
244.71	246.12	0.0360 /0.0330 Main Channel
247.71	245.17	0.0330 Main Channel
249.71	245.19	0.0330 Main Channel
251.71	245.64	0.0330 /0.0360 Main Channel
259.71	246.47	0.0360
282.78	247.12	0.0360
315.41	251.60	0.0360
424.17	256.13	0.0360
478.55	257.04	0.0360
516.62	259.37	0.0360
538.37	266.00	0.0360

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.47	245.64	.111E+04	3.1	1.78	6.07
1.54	246.71	.103E+05	53.9	3.39	3.19
2.61	247.78	.414E+05	281.3	4.40	2.45
3.68	248.85	.839E+05	749.1	5.79	1.87
4.74	249.91	.136E+06	1463.2	6.97	1.55
5.81	250.98	.201E+06	2354.8	7.61	1.42
6.88	252.05	.295E+06	3533.0	7.76	1.39
7.95	253.12	.427E+06	5303.9	8.05	1.34

9.02	254.19	.594E+06	7856.5	8.58	1.26
10.09	255.26	.792E+06	11449.2	9.37	1.15
11.15	256.32	.101E+07	15862.4	10.15	1.06
12.22	257.39	.127E+07	21074.5	10.75	1.01
13.29	258.46	.155E+07	28184.1	11.78	0.92
14.36	259.53	.185E+07	36386.2	12.77	0.85
15.43	260.60	.215E+07	46110.6	13.89	0.78
16.50	261.67	.246E+07	58222.5	14.95	0.72
17.56	262.73	.278E+07	66498.1	15.97	0.68
18.63	263.80	.310E+07	81118.8	16.94	0.64
19.70	264.87	.344E+07	93603.2	17.66	0.61

```

-----
|----- hydrograph -----| <-pipe / channel->
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) (m) (m/s) |
|-----|
| INFLOW: ID= 2 (8342) 667.58 1.70 8.00 7.21 0.26 1.78 |
|-----|
| OUTFLOW: ID= 1 (6312) 667.58 1.70 8.00 7.21 0.26 1.78 |
  
```

```

-----
| ADD HYD (8340) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (0312): 359.44 1.796 8.00 13.53
| + ID2= 2 (6312): 667.58 1.699 8.00 7.21
|-----|
| ID = 3 (8340): 1027.02 3.494 8.00 9.42
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8336) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (8338): 397.54 2.608 7.75 13.69
| + ID2= 2 (8340): 1027.02 3.494 8.00 9.42
|-----|
| ID = 3 (8336): 1424.56 6.086 8.00 10.61
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8330) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (8332): 762.14 3.286 8.75 13.85
| + ID2= 2 (8336): 1424.56 6.086 8.00 10.61
|-----|
| ID = 3 (8330): 2186.70 9.233 8.25 11.74
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6324) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
  
```

<----- DATA FOR SECTION (3241.0) ----->

Distance	Elevation	Manning
0.00	234.38	0.0360
33.57	232.48	0.0360
67.14	230.14	0.0360
83.93	228.80	0.0360
134.29	227.62	0.0360
209.82	225.10	0.0360
218.21	224.86	0.0360
226.61	224.47	0.0360
234.00	223.86	0.0360 /0.0300 Main Channel
234.10	223.66	0.0300 Main Channel
235.00	223.66	0.0300 Main Channel
235.90	223.66	0.0300 Main Channel
236.00	223.86	0.0300 /0.0360 Main Channel
243.39	224.92	0.0360
251.78	224.89	0.0360
335.71	225.64	0.0360
562.32	226.53	0.0360
637.85	228.36	0.0360
705.00	229.80	0.0360
830.89	234.00	0.0360

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.20	223.86	.145E+04	0.2	0.43	147.27
0.73	224.39	.159E+05	2.8	0.68	93.11
1.27	224.93	.561E+05	11.0	0.75	85.19
1.80	225.46	.121E+06	45.6	0.80	79.20
2.33	225.99	.566E+06	134.4	0.91	70.20
2.87	226.53	.122E+07	336.1	1.06	59.97

3.40	227.06	.208E+07	744.3	1.38	46.16
3.94	227.60	.298E+07	1295.9	1.66	38.38
4.47	228.13	.399E+07	1979.9	1.90	33.59
5.00	228.66	.509E+07	2807.5	2.11	30.21
5.54	229.20	.627E+07	3815.1	2.32	27.41
6.07	229.73	.753E+07	4984.1	2.53	25.17
6.60	230.26	.884E+07	6346.3	2.74	23.20
7.14	230.80	.102E+08	7861.2	2.95	21.61
7.67	231.33	.116E+08	9526.4	3.14	20.29
8.21	231.87	.131E+08	11342.2	3.32	19.18
8.74	232.40	.146E+08	13309.6	3.49	18.22
9.27	232.93	.161E+08	15414.0	3.66	17.41
9.81	233.47	.177E+08	17671.0	3.81	16.70

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8330) 2186.70 9.23 8.25 11.74 1.15 0.73
OUTFLOW : ID= 1 (6324) 2186.70 7.97 10.00 11.74 1.07 0.72

```

```

ADD HYD (8328)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8324): 615.64 2.268 9.00 13.05
+ ID2= 2 (6324): 2186.70 7.966 10.00 11.74
=====
ID = 3 (8328): 2802.34 10.191 9.75 12.03

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB
NASHYD (0322) | Area (ha)= 513.13 Curve Number (CN)= 80.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.75

```

Unit Hyd Qpeak (cms) = 5.020

```

PEAK FLOW (cms) = 2.099 (1)
TIME TO PEAK (hrs) = 8.500
RUNOFF VOLUME (mm) = 13.561
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.323

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8326)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8322): 513.13 2.099 8.50 13.56
+ ID2= 2 (8328): 2802.34 10.191 9.75 12.03
=====
ID = 3 (8326): 3315.47 12.230 9.75 12.27

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8324)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8326): 678.91 2.547 9.00 13.57
+ ID2= 2 (8326): 3315.47 12.230 9.75 12.27
=====
ID = 3 (8324): 3994.38 14.756 9.75 12.49

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CEN (8328)
IN= 2 ----> OUT= 1
Routing time step (min) = 15.00

```

<----- DATA FOR SECTION (3281.0) ----->

Distance	Elevation	Manning
0.00	228.00	0.0380
18.58	224.97	0.0380
78.98	223.52	0.0380
125.44	223.28	0.0380
171.90	221.71	0.0380
213.72	219.65	0.0380
218.36	219.40	0.0380
223.01	219.19	0.0380
225.95	219.14	0.0380 / 0.0320
226.45	218.14	0.0320
236.95	218.14	0.0320

245.85	218.14	0.0320	Main Channel
245.95	219.14	0.0320 / 0.0380	Main Channel
246.24	219.16	0.0380	
250.88	219.24	0.0380	
255.53	219.39	0.0380	
325.22	221.47	0.0380	
367.03	223.14	0.0380	
404.20	225.17	0.0380	
459.95	228.04	0.0380	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.50	218.64	.373E+05	3.2	0.33	192.37
1.00	219.14	.752E+05	10.1	0.51	124.71
1.52	219.66	.149E+06	22.2	0.57	112.06
2.04	220.18	.279E+06	43.4	0.59	107.06
2.56	220.70	.464E+06	77.0	0.63	100.52
3.08	221.22	.705E+06	125.7	0.68	93.56
3.61	221.75	.100E+07	192.7	0.73	86.62
4.13	222.27	.135E+07	278.1	0.79	80.92
4.65	222.79	.176E+07	385.9	0.84	75.86
5.17	223.31	.222E+07	514.9	0.89	71.82
5.69	223.83	.280E+07	662.7	0.90	70.40
6.21	224.35	.345E+07	867.7	0.96	66.35
6.73	224.87	.417E+07	1107.0	1.01	62.81
7.25	225.39	.494E+07	1400.0	1.08	58.80
7.78	225.92	.573E+07	1732.2	1.15	55.17
8.30	226.44	.655E+07	2099.9	1.22	52.02
8.82	226.96	.740E+07	2502.8	1.29	49.29
9.34	227.48	.828E+07	2940.9	1.36	46.90
9.86	228.00	.918E+07	3414.3	1.42	44.79

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8324) 3994.38 14.76 9.75 12.49 1.20 0.53
OUTFLOW : ID= 1 (6328) 3994.38 12.61 11.50 12.49 1.11 0.52

```

```

ADD HYD (8320)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8328): 492.92 1.670 9.00 12.08
+ ID2= 2 (6328): 3994.38 12.607 11.50 12.49
=====
ID = 3 (8320): 4487.30 14.097 11.25 12.44

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8318)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8320): 4487.30 14.097 11.25 12.44
+ ID2= 2 (8322): 861.74 3.223 9.75 12.46
=====
ID = 3 (8318): 5349.04 17.133 11.00 12.45

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8316)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8309): 10059.18 19.107 14.50 9.83
+ ID2= 2 (8318): 5349.04 17.133 11.00 12.45
=====
ID = 3 (8316): 15408.22 33.914 13.00 10.74

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8312)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8314): 620.10 0.389 7.25 6.40
+ ID2= 2 (8316): 15408.22 33.914 13.00 10.74
=====
ID = 3 (8312): 16028.32 34.228 13.00 10.57

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8308)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8310): 598.90 9.798 6.25 15.95
+ ID2= 2 (8312): 16028.32 34.228 13.00 10.57
=====
ID = 3 (8308): 16627.22 34.888 12.50 10.76

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

RESERVOIR (5510)
IN= 2--> OUT= 1
DT= 15.0 min
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
0.0000 0.0000 | ***** 74.0090
66.5450 18.5023 | *****
98.5430 37.0045 | *****

```

```

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (8308) ***** 34.888 12.50 10.76
OUTFLOW : ID= 1 (5510) ***** 34.412 13.25 10.76

```

```

PEAK FLOW REDUCTION [Qout/Qin](%)= 98.64
TIME SHIFT OF PEAK FLOW (min)= 45.00
MAXIMUM STORAGE USED (ha.m.)= 9.5730

```

```

ADD HYD (8240)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0336): 2785.00 1.223 22.50 10.07
+ ID2= 2 (5510): 16627.22 34.412 13.25 10.76
=====
ID = 3 (8240): 19412.22 35.468 13.50 10.66

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8238)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (5508): 9524.23 14.571 12.25 6.08
+ ID2= 2 (8240): 19412.22 35.468 13.50 10.66
=====
ID = 3 (8238): 28936.45 49.761 13.00 9.15

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8236)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0342): 1004.58 1.208 10.25 5.39
+ ID2= 2 (8238): 28936.45 49.761 13.00 9.15
=====
ID = 3 (8236): 29941.03 50.838 13.00 9.03

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8234)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8232): 285.80 3.513 6.00 10.55
+ ID2= 2 (8236): 29941.03 50.838 13.00 9.03
=====
ID = 3 (8234): 30226.83 51.024 13.00 9.04

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8230)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0344): 344.00 1.036 7.25 6.33
+ ID2= 2 (8234): 30226.83 51.024 13.00 9.04
=====
ID = 3 (8230): 30570.83 51.336 12.75 9.01

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8228)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8226): 1952.06 3.673 9.00 5.24
+ ID2= 2 (8230): 30570.83 51.336 12.75 9.01
=====
ID = 3 (8228): 32522.89 53.703 12.75 8.79

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8190)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0356): 1015.22 1.461 8.25 4.06
+ ID2= 2 (8228): 32522.89 53.703 12.75 8.79
=====
ID = 3 (8190): 33538.11 54.595 12.50 8.64

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

*****
** SIMULATION NUMBER: 2 **
*****

```

```

READ STORM
Filename: C:\Users\jscott\AppData
Local\Temp\
5a280798-92ee-4282-809c-79f5caed0add\5a76734a
Total= 54.40 mm
Comments: 5-Year 12-Hour SCS II Design Storm

```

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.36	3.25	2.18	6.25	9.79	9.25	1.90
0.50	1.36	3.50	2.18	6.50	9.79	9.50	1.90
0.75	1.36	3.75	2.18	6.75	4.35	9.75	1.90
1.00	1.36	4.00	2.18	7.00	4.35	10.00	1.90
1.25	1.36	4.25	3.26	7.25	3.26	10.25	1.09
1.50	1.36	4.50	3.26	7.50	3.26	10.50	1.09
1.75	1.36	4.75	4.35	7.75	3.26	10.75	1.09
2.00	1.36	5.00	4.35	8.00	3.26	11.00	1.09
2.25	1.63	5.25	6.53	8.25	1.90	11.25	1.09
2.50	1.63	5.50	6.53	8.50	1.90	11.50	1.09
2.75	1.63	5.75	26.11	8.75	1.90	11.75	1.09
3.00	1.63	6.00	71.83	9.00	1.90	12.00	1.09

```

CALIB
NASHYD (0356) Area (ha)=1015.22 Curve Number (CN)= 46.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.37

```

```

Unit Hyd Qpeak (cms)= 12.651
PEAK FLOW (cms)= 2.534 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 6.976
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.128

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0354) Area (ha)= 262.68 Curve Number (CN)= 37.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.38

```

```

Unit Hyd Qpeak (cms)= 3.252
PEAK FLOW (cms)= 0.468 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 5.032
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.093

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0352) Area (ha)= 381.43 Curve Number (CN)= 54.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.82

```

Unit Hyd Qpeak (cms)= 7.980

PEAK FLOW (cms)= 1.840 (1)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 9.056
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.166

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHYD (0346) | Area (ha)= 350.93 Curve Number (CN)= 70.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 6.254

PEAK FLOW (cms)= 2.564 (1)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 15.253
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.280

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHYD (0350) | Area (ha)= 366.84 Curve Number (CN)= 48.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 5.831

PEAK FLOW (cms)= 1.170 (1)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 7.450
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.137

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHYD (0348) | Area (ha)= 590.18 Curve Number (CN)= 48.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.91

Unit Hyd Qpeak (cms)= 5.267

PEAK FLOW (cms)= 1.224 (1)
 TIME TO PEAK (hrs)= 9.250
 RUNOFF VOLUME (mm)= 7.490
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.138

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6350) |
 IN= 2--> OUT= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (3501.0) ----->
 Distance Elevation Manning
 0.00 287.00 0.0500
 23.66 283.72 0.0500
 70.98 280.94 0.0500
 112.38 280.23 0.0500
 171.52 276.80 0.0500
 260.24 274.46 0.0500
 266.16 274.26 0.0500
 272.07 274.12 0.0500
 276.49 274.12 0.0500 /0.0300 Main Channel
 276.99 273.82 0.0300 Main Channel
 277.99 273.82 0.0300 Main Channel
 279.49 273.82 0.0300 Main Channel
 280.49 274.27 0.0300 /0.0500 Main Channel
 283.00 274.27 0.0500
 289.81 274.57 0.0500
 325.30 275.75 0.0500
 396.38 276.98 0.0500
 449.51 280.97 0.0500
 496.83 283.90 0.0500
 585.54 287.92 0.0500

<----- TRAVEL TIME TABLE ----->
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
 (m) (m) (cu.m.) (cms) (m/s) (min)
 0.30 274.12 273E+04 0.9 0.89 49.81
 0.98 274.80 583E+05 20.8 1.05 46.71
 1.66 275.48 203E+06 95.2 1.38 35.53
 2.33 276.15 438E+06 256.0 1.73 28.50

3.01	276.83	.755E+06	524.8	2.05	23.97
3.69	277.51	.114E+07	935.8	2.43	20.26
4.37	278.19	.157E+07	1472.0	2.76	17.82
5.05	278.87	.206E+07	2140.7	3.06	16.06
5.72	279.54	.263E+07	2938.6	3.33	14.79
6.40	280.22	.321E+07	3887.6	3.57	13.77
7.08	280.90	.390E+07	4795.4	3.63	13.56
7.76	281.58	.468E+07	6196.2	3.91	12.58
8.43	282.25	.550E+07	7803.9	4.19	11.74
9.11	282.93	.637E+07	9601.9	4.45	11.05
9.79	283.61	.728E+07	11594.8	4.70	10.46
10.47	284.29	.823E+07	13845.3	4.96	9.91
11.15	284.97	.922E+07	16293.4	5.21	9.43
11.82	285.64	.103E+08	18952.0	5.45	9.02
12.50	286.32	.113E+08	21824.3	5.69	8.65

<---- hydrograph ----> <-pipe / channel-->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0348) 590.18 1.22 9.25 7.49 0.31 0.99
 OUTFLOW : ID= 1 (6350) 590.18 1.20 10.25 7.49 0.31 0.99

ADD HYD (8220) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0350): 366.84 1.170 7.75 7.45
 + ID2= 2 (6350): 590.18 1.197 10.25 7.49
 ID = 3 (8220): 957.02 2.179 8.75 7.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8222) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0346): 350.93 2.564 7.25 15.25
 + ID2= 2 (8220): 957.02 2.179 8.75 7.47
 ID = 3 (8222): 1307.95 4.585 8.00 9.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6352) |
 IN= 2--> OUT= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (3521.0) ----->
 Distance Elevation Manning
 0.00 257.95 0.0500
 7.83 257.34 0.0500
 15.66 256.19 0.0500
 21.53 254.01 0.0500
 41.11 244.06 0.0500
 76.38 241.38 0.0500
 111.58 239.74 0.0500
 113.54 239.61 0.0500
 113.75 239.41 0.0500 /0.0300 Main Channel
 113.85 238.81 0.0300 Main Channel
 115.50 238.81 0.0300 Main Channel
 116.15 238.81 0.0300 Main Channel
 116.25 239.41 0.0300 Main Channel
 117.46 239.52 0.0500
 119.41 239.72 0.0500
 121.37 240.04 0.0500
 131.16 241.84 0.0500
 156.61 247.03 0.0500
 176.19 251.46 0.0500
 193.80 258.79 0.0500

<----- TRAVEL TIME TABLE ----->
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
 (m) (m) (cu.m.) (cms) (m/s) (min)
 0.60 239.41 224E+04 2.2 1.52 17.12
 1.58 240.39 216E+05 22.3 1.61 16.16
 2.55 241.36 804E+05 101.0 1.96 13.27
 3.53 242.34 173E+06 275.6 2.48 10.46
 4.50 243.31 293E+06 553.9 2.95 8.80
 5.48 244.29 438E+06 961.8 3.42 7.60
 6.45 245.26 599E+06 1534.4 3.99 6.51
 7.43 246.24 770E+06 2228.4 4.51 5.76
 8.41 247.22 951E+06 3043.6 4.98 5.21
 9.38 248.19 114E+07 3982.1 5.43 4.78
 10.36 249.17 134E+07 5040.1 5.85 4.44
 11.33 250.14 153E+07 6218.5 6.24 4.16
 12.31 251.12 177E+07 7518.6 6.61 3.93
 13.29 252.10 200E+07 8969.0 6.98 3.72
 14.26 253.07 224E+07 10554.1 7.35 3.53

13.24 254.05 .248E+07 12257.1 7.71 3.37
 16.21 255.02 .272E+07 14045.1 8.03 3.23
 17.19 256.00 .298E+07 15954.8 8.34 3.11
 18.16 256.97 .325E+07 17780.4 8.53 3.04

<----- hydrograph -----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8222) 1307.95 4.55 8.00 9.56 0.72 1.53
 OUTFLOW: ID= 1 (6352) 1307.95 4.55 8.25 9.56 0.71 1.53

ADD HYD (8224)
 1 2 3
 ID1= 1 (0352): 381.43 1.840 7.00 9.06
 + ID2= 2 (6352): 1307.95 4.551 8.25 9.56
 ID = 3 (8224): 1689.38 6.228 8.00 9.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6354)
 IN= 2 -> OUT= 1 Routing time step (min) = 15.00

DATA FOR SECTION (3541.0) ----->
 Distance Elevation Manning
 0.00 253.92 0.0500
 7.95 251.83 0.0500
 15.89 249.97 0.0500
 19.87 249.05 0.0500
 47.68 242.00 0.0500
 67.55 237.10 0.0500
 83.44 231.11 0.0500
 85.43 230.75 0.0500
 85.66 230.61 0.0500 / 0.0300 Main Channel
 85.76 230.01 0.0300 Main Channel
 87.41 230.01 0.0300 Main Channel
 88.06 230.01 0.0300 Main Channel
 88.16 230.61 0.0300 / 0.0500 Main Channel
 89.40 230.72 0.0500
 91.39 230.88 0.0500
 93.37 231.03 0.0500
 133.11 233.96 0.0500
 150.99 235.39 0.0500
 172.84 239.03 0.0500
 196.68 250.00 0.0500

TRAVEL TIME TABLE ----->
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
 (m) (m) (cu.m.) (cms) (m/s) (min)
 0.60 230.61 451B+04 1.5 1.06 49.40
 1.62 231.63 394E+05 14.6 1.16 45.13
 2.64 232.65 128E+06 58.8 1.44 36.30
 3.66 233.67 269E+06 149.7 1.74 30.00
 4.68 234.69 463E+06 301.5 2.04 25.58
 5.70 235.71 708E+06 530.5 2.36 22.14
 6.72 236.73 980E+06 848.4 2.71 19.25
 7.74 237.75 128E+07 1237.9 3.02 17.29
 8.76 238.77 162E+07 1706.7 3.30 15.83
 9.78 239.79 199E+07 2291.9 3.61 14.48
 10.81 240.82 237E+07 2973.7 3.92 13.30
 11.83 241.84 278E+07 3742.2 4.21 12.38
 12.85 242.86 41 321E+07 4599.1 4.49 11.62
 13.87 243.88 365E+07 5544.6 4.75 10.98
 14.89 244.90 412E+07 6579.7 5.00 10.44
 15.91 245.92 461E+07 7705.9 5.23 9.97
 16.93 246.94 512E+07 8924.6 5.46 9.55
 17.95 247.96 564E+07 10237.5 5.68 9.19
 18.97 248.98 619E+07 11646.2 5.89 8.86

<----- hydrograph -----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8224) 1689.38 6.23 8.00 9.45 0.97 1.09
 OUTFLOW: ID= 1 (6354) 1689.38 5.80 9.00 9.45 0.93 1.09

ADD HYD (8226)
 1 2 3
 ID1= 1 (0354): 262.68 0.468 8.00 5.03
 + ID2= 2 (6354): 1689.38 5.795 9.00 9.45
 ID = 3 (8226): 1952.06 6.251 8.75 8.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHED (0344) Area (ha)= 344.00 Curve Number (CN)= 59.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.87

Unit Hyd Qpeak (cms)= 6.790

PEAK FLOW (cms)= 1.873 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 10.666
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.196

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (3402) Area (ha)= 234.60
 ID= 1 DT=15.0 min Total Imp(%)= 28.00 Dir. Conn.(%)= 13.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 65.69 168.91
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1250.60 40.00
 Mannings n = 0.013 0.250

Max. Ref. Inten.(mm/hr)= 71.81 9.73
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 16.35 (ii) 43.52 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.07 0.03

PEAK FLOW (cms)= 4.27 2.34 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.50 5.191 (iii)
 RUNOFF VOLUME (mm)= 52.40 9.09 14.72
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.17 0.27

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 48.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (3401) Area (ha)= 51.20
 ID= 1 DT=15.0 min Total Imp(%)= 46.00 Dir. Conn.(%)= 21.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 23.55 27.65
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 584.24 40.00
 Mannings n = 0.013 0.250

Max. Ref. Inten.(mm/hr)= 71.81 14.25
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 10.35 (ii) 33.67 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.09 0.03

PEAK FLOW (cms)= 1.79 0.64 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.50 2.052 (iii)
 RUNOFF VOLUME (mm)= 52.40 10.87 19.59
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.20 0.36

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 48.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5340)
 IN= 2 -> OUT= 1
 DT= 15.0 min
 OUTFLOW STORAGE OUTFLOW STORAGE
 (cms) (ha.m.) (cms) (ha.m.)
 0.0000 0.0000 5.3860 1.7877
 0.1730 0.6348 7.2780 2.2185
 1.8360 0.8250 9.1880 2.6486
 2.1540 1.1051 9.5880 2.6586

3.3950 1.3506 | 0.0000 0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (3401)	51.200	2.052	6.00	19.59
OUTFLOW: ID= 1 (5340)	51.200	0.284	8.00	19.57

PEAK FLOW REDUCTION [Qout/Qin](%)= 13.83
 TIME SHIFT OF PEAK FLOW (min)=120.00
 MAXIMUM STORAGE USED (ha.m.)= 0.6479

ADD HYD (8232)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (3402):	234.60	5.191	6.00	14.72
+ ID2= 2 (5340):	51.20	0.284	8.00	19.57

ID = 3 (8232):	285.80	5.262	6.00	15.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0342)	1004.58	54.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 2.57	

Unit Hyd Qpeak (cms)= 6.686

PEAK FLOW (cms)= 2.057 (i)
 TIME TO PEAK (hrs)= 10.250
 RUNOFF VOLUME (mm)= 9.159
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.168

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0422)	780.20	54.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 1.95	

Unit Hyd Qpeak (cms)= 6.838

PEAK FLOW (cms)= 1.954 (i)
 TIME TO PEAK (hrs)= 9.250
 RUNOFF VOLUME (mm)= 9.147
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.168

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0410)	572.01	48.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 1.46	

Unit Hyd Qpeak (cms)= 6.698

PEAK FLOW (cms)= 1.460 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 7.475
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.137

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0408)	231.62	58.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 0.64	

Unit Hyd Qpeak (cms)= 6.198

PEAK FLOW (cms)= 1.542 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 10.251
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.188

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

NASHYD (0376)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 1.07	

Unit Hyd Qpeak (cms)= 7.380

PEAK FLOW (cms)= 3.568 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 17.440
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.321

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0374)	545.70	61.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 1.51	

Unit Hyd Qpeak (cms)= 6.158

PEAK FLOW (cms)= 2.092 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 11.459
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.211

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0372)	110.42	37.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 0.96	

Unit Hyd Qpeak (cms)= 1.954

PEAK FLOW (cms)= 0.255 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 5.010
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.092

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0370)	191.85	63.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 0.67	

Unit Hyd Qpeak (cms)= 4.860

PEAK FLOW (cms)= 1.448 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 12.065
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.222

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0368)	159.48	46.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 1.12	

Unit Hyd Qpeak (cms)= 2.433

PEAK FLOW (cms)= 0.461 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 6.960
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.128

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0366)	462.62	62.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 1.06	

Unit Hyd Qpeak (cms)= 7.451

PEAK FLOW (cms)= 2.387 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 11.788
 TOTAL RAINFALL (mm)= 54.400

RUNOFF COEFFICIENT = 0.217

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0364) | Area (ha)= 155.27 Curve Number (CN)= 55.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.70

```

Unit Hyd Qpeak (cms)= 3.782

```

PEAK FLOW (cms)= 0.868 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 9.324
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.171

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8302) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (0364): | 155.27 0.868 7.00 9.32
| + ID2= 2 (0366): | 462.62 2.387 7.50 11.79
|-----|
| ID = 3 (8302): | 617.89 3.218 7.25 11.17

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| STANHYD (0362) | Area (ha)= 118.78
| ID= 1 DT=15.0 min | Total Imp(%)= 22.00 Dir. Conn.(%)= 8.00
|-----|
| IMPERVIOUS PERVIOUS (i)
| Surface Area (ha)= 26.13 92.65
| Dep. Storage (mm)= 2.00 5.00
| Average Slope (%)= 0.50 0.50
| Length (m)= 889.87 40.00
| Mannings n = 0.013 0.250
|
| Max.Eff.Inten.(mm/hr)= 71.81 8.34
| over (min) 15.00 45.00
| Storage Coeff. (min)= 13.33 (ii) 42.23 (ii)
| Unit Hyd. Tpeak (min)= 15.00 45.00
| Unit Hyd. peak (cms)= 0.08 0.03
|
|-----|
| PEAK FLOW (cms)= 1.45 1.12 *TOTALS* 1.889 (iii)
| TIME TO PEAK (hrs)= 6.00 6.50 6.00
| RUNOFF VOLUME (mm)= 52.40 8.03 11.58
| TOTAL RAINFALL (mm)= 54.40 54.40 54.40
| RUNOFF COEFFICIENT = 0.96 0.15 0.21

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 45.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0358) | Area (ha)= 429.87 Curve Number (CN)= 35.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.03

```

Unit Hyd Qpeak (cms)= 7.091

```

PEAK FLOW (cms)= 0.870 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 4.638
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.085

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0360) | Area (ha)= 138.37 Curve Number (CN)= 46.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.60

```

Unit Hyd Qpeak (cms)= 3.957

```

PEAK FLOW (cms)= 0.641 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 6.867
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.126

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8306) |
| 1 + 2 + 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (0358): | 429.87 0.870 7.50 4.64
| + ID2= 2 (0360): | 138.37 0.641 6.75 6.87
|-----|
| ID = 3 (8306): | 568.24 1.473 7.00 5.18

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CN (8362) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00
|-----|

```

----- DATA FOR SECTION (3621.0) -----

Distance	Elevation	Manning
0.00	261.46	0.0550
27.86	254.23	0.0550
51.07	251.96	0.0550
74.29	250.77	0.0550
97.50	249.91	0.0550
125.36	249.40	0.0550
150.93	247.40	0.0550
155.93	247.33	0.0550 / 0.0350
157.93	246.85	0.0350
159.18	246.65	0.0350
160.18	246.63	0.0350
160.93	246.85	0.0350
161.93	247.18	0.0350 / 0.0550
163.18	248.03	0.0550
168.18	248.58	0.0550
183.18	250.18	0.0550
201.97	252.59	0.0550
213.57	256.02	0.0550
225.18	260.31	0.0550
229.82	261.00	0.0550

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.55	247.18	.556E+04	1.9	1.03	49.95
1.28	247.91	.336E+05	16.7	1.53	33.61
2.00	248.63	.913E+05	53.6	1.81	28.40
2.73	249.36	.185E+06	122.3	2.05	25.18
3.46	250.09	.344E+06	224.6	2.02	25.52
4.19	250.82	.574E+06	417.9	2.25	22.91
4.91	251.54	.855E+06	699.1	2.53	20.38
5.64	252.27	.118E+07	1075.3	2.82	18.26
6.37	253.00	.153E+07	1559.9	3.15	16.38
7.10	253.73	.191E+07	2137.6	3.46	14.89
7.82	254.45	.231E+07	2815.2	3.77	13.67
8.55	255.18	.272E+07	3608.5	4.10	12.58
9.28	255.91	.315E+07	4488.2	4.41	11.69
10.01	256.64	.358E+07	5458.0	4.71	10.95
10.73	257.36	.403E+07	6512.4	4.99	10.32
11.46	258.09	.449E+07	7649.8	5.27	9.78
12.19	258.82	.495E+07	8869.5	5.53	9.32
12.92	259.55	.544E+07	10171.2	5.78	8.91
13.64	260.27	.593E+07	11554.8	6.02	8.55

```

----- hydrograph ----- <-pipe / channel->
|-----|
| AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
| (ha) (cms) (hrs) (mm) (m) (m/s)
| INFLOW: ID= 2 (8306) 568.24 1.47 7.00 5.18 0.44 1.03
|-----|
| OUTFLOW: ID= 1 (8362) 568.24 1.30 6.25 5.18 0.38 1.03

```

```

-----
| ADD HYD (8304) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (0362): | 118.78 1.889 6.00 11.58
| + ID2= 2 (8362): | 568.24 1.303 8.25 5.18
|-----|
| ID = 3 (8304): | 687.02 2.147 6.50 6.29

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8300)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8302):	617.89	3.218	7.25	11.17
+ ID2= 2 (8304):	687.02	2.147	6.50	6.29
=====				
ID = 3 (8300):	1304.91	5.155	7.00	8.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6368) |
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3681.0) ----->				
Distance	Elevation	Manning		
0.00	230.00	0.0370		
18.48	223.26	0.0370		
36.96	223.05	0.0370		
64.67	222.94	0.0370		
110.87	222.86	0.0370		
133.96	222.74	0.0370		
147.82	222.65	0.0370		
170.92	222.31	0.0370		
174.79	222.26	0.0370 / 0.0300	Main Channel	
174.89	221.86	0.0300	Main Channel	
175.54	221.86	0.0300	Main Channel	
176.19	221.86	0.0300	Main Channel	
176.29	222.26	0.0300 / 0.0370	Main Channel	
180.16	222.25	0.0370		
184.78	222.28	0.0370		
189.40	222.31	0.0370		
332.60	222.37	0.0370		
450.00	230.00	0.0370		

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.40	222.26	771E+03	0.3	0.52	41.74
0.81	222.67	784E+05	30.3	0.51	43.17
1.21	223.07	210E+06	123.2	0.77	28.42
1.62	223.48	384E+06	301.0	1.03	21.25
2.03	223.89	563E+06	550.2	1.28	17.06
2.44	224.30	747E+06	860.7	1.51	14.46
2.84	224.70	934E+06	1228.1	1.73	12.68
3.25	225.11	1135E+07	1649.3	1.92	11.37
3.66	225.52	1328E+07	2122.3	2.11	10.37
4.07	225.93	1528E+07	2645.5	2.29	9.58
4.47	226.33	1728E+07	3217.8	2.46	8.92
4.88	226.74	1938E+07	3838.3	2.61	8.38
5.29	227.15	2148E+07	4506.3	2.76	7.92
5.70	227.56	2368E+07	5221.3	2.91	7.52
6.10	227.96	2588E+07	5993.0	3.05	7.17
6.51	228.37	2808E+07	6790.9	3.19	6.87
6.92	228.78	3038E+07	7645.0	3.32	6.60
7.33	229.19	3268E+07	8545.1	3.45	6.35
7.73	229.59	3498E+07	9491.0	3.57	6.13

<---- hydrograph ---->						<-pipe / channel->					
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8300)	1304.91	5.16	7.00	8.60	0.47	0.52					
OUTFLOW: ID= 1 (6368)	1304.91	4.77	8.25	8.60	0.46	0.52					

ADD HYD (8298)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0368):	159.48	0.461	7.75	6.96
+ ID2= 2 (6368):	1304.91	4.771	8.25	8.60
=====				
ID = 3 (8298):	1464.39	5.224	8.25	8.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8296)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0370):	191.85	1.448	6.75	12.06
+ ID2= 2 (8298):	1464.39	5.224	8.25	8.42
=====				
ID = 3 (8296):	1656.24	6.423	8.00	8.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6372) |

IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3721.0) ----->			
Distance	Elevation	Manning	
0.00	225.00	0.0390	
30.80	219.38	0.0390	
61.61	219.30	0.0390	
77.01	219.27	0.0390	
469.76	219.14	0.0390	
477.46	219.13	0.0390	
485.16	219.10	0.0390	
492.86	219.09	0.0390	
495.56	219.09	0.0390 / 0.0310	Main Channel
495.66	218.51	0.0310	Main Channel
500.56	218.51	0.0310	Main Channel
505.46	218.51	0.0310	Main Channel
505.56	219.09	0.0310 / 0.0390	Main Channel
508.26	219.09	0.0390	
515.96	219.10	0.0390	
523.67	219.21	0.0390	
562.17	219.32	0.0390	
654.58	219.43	0.0390	
731.59	219.46	0.0390	
762.39	225.00	0.0390	

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.29	218.80	431E+04	1.7	0.58	43.48
0.58	219.09	867E+04	5.1	0.89	28.32
0.93	219.44	200E+06	58.2	0.44	57.35
1.28	219.79	568E+06	278.6	0.74	33.99
1.62	220.13	939E+06	630.2	1.01	24.84
1.97	220.48	131E+07	1090.4	1.25	20.05
2.32	220.83	169E+07	1647.7	1.47	17.06
2.67	221.18	206E+07	2294.6	1.68	14.99
3.01	221.52	244E+07	3025.6	1.87	13.46
3.36	221.87	282E+07	3836.5	2.05	12.27
3.71	222.22	321E+07	4723.7	2.22	11.31
4.06	222.57	359E+07	5684.6	2.39	10.53
4.40	222.91	398E+07	6716.6	2.55	9.87
4.75	223.26	437E+07	7817.7	2.70	9.31
5.10	223.61	476E+07	8986.2	2.85	8.83
5.45	223.96	515E+07	10220.4	2.99	8.40
5.79	224.30	555E+07	11519.1	3.13	8.03
6.14	224.65	594E+07	12880.8	3.27	7.69
6.49	225.00	634E+07	14304.5	3.40	7.39

<---- hydrograph ---->						<-pipe / channel->					
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8296)	1656.24	6.42	8.00	8.84	0.59	0.87					
OUTFLOW: ID= 1 (6372)	1656.24	6.26	8.50	8.84	0.59	0.87					

ADD HYD (8294)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0372):	116.42	0.255	7.50	5.01
+ ID2= 2 (6372):	1656.24	6.262	8.50	8.84
=====				
ID = 3 (8294):	1766.66	6.496	8.50	8.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8292)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0374):	545.70	2.092	8.25	11.46
+ ID2= 2 (8294):	1766.66	6.496	8.50	8.60
=====				
ID = 3 (8292):	2312.36	8.581	8.50	9.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5505)			
IN= 2--> OUT= 1	DT= 15.0 min	OUTFLOW	STORAGE
(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	65.1290	345.3754
25.4850	24.6697	84.9510	456.3890
31.1490	98.6787	*****	838.7689
39.6440	*****	*****	838.7789
48.1390	*****	0.0000	0.0000

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)

INFLOW : ID= 2 (8292) 2312.360 8.581 8.50 9.28
OUTFLOW: ID= 1 (5505) 2312.360 6.211 11.00 9.28

PEAK FLOW REDUCTION [Qout/Qin](%)= 72.38
TIME SHIFT OF PEAK FLOW (min)=150.00
MAXIMUM STORAGE USED (ha.m.)= 6.0141

| ADD HYD (8272) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
ID1= 1 (0376): 463.85 3.568 7.50 17.44
+ ID2= 2 (5505): 2312.36 6.211 11.00 9.28
=====

ID = 3 (8272): 2776.21 8.592 10.25 10.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| CALIB |
| NASHYD (0395) | Area (ha)= 305.21 Curve Number (CN)= 69.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.08

Unit Hyd Qpeak (cms)= 4.811

PEAK FLOW (cms)= 1.961 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 14.789
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.272

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHYD (0394) | Area (ha)= 325.45 Curve Number (CN)= 53.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.92

Unit Hyd Qpeak (cms)= 6.013

PEAK FLOW (cms)= 1.379 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 8.784
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.161

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHYD (0390) | Area (ha)= 420.00 Curve Number (CN)= 55.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 6.683

PEAK FLOW (cms)= 1.700 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 9.400
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.173

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHYD (0388) | Area (ha)= 220.77 Curve Number (CN)= 58.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.99

Unit Hyd Qpeak (cms)= 3.819

PEAK FLOW (cms)= 1.051 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 10.350
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.190

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHYD (0386) | Area (ha)= 241.27 Curve Number (CN)= 61.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.90

Unit Hyd Qpeak (cms)= 4.562

PEAK FLOW (cms)= 1.360 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 11.386
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.209

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| ADD HYD (8286) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
ID1= 1 (0386): 241.27 1.360 7.25 11.39
+ ID2= 2 (0388): 220.77 1.051 7.50 10.35
=====

ID = 3 (8286): 462.04 2.411 7.25 10.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| CALIB |
| NASHYD (0384) | Area (ha)= 199.07 Curve Number (CN)= 44.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 3.537

PEAK FLOW (cms)= 0.598 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 6.477
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.119

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHYD (0380) | Area (ha)= 182.01 Curve Number (CN)= 40.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.55

Unit Hyd Qpeak (cms)= 5.609

PEAK FLOW (cms)= 0.715 (i)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 5.531
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.102

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHYD (0382) | Area (ha)= 216.59 Curve Number (CN)= 53.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.64

Unit Hyd Qpeak (cms)= 5.733

PEAK FLOW (cms)= 1.207 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 8.712
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.160

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| ADD HYD (8290) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
ID1= 1 (0380): 182.01 0.715 6.50 5.53
+ ID2= 2 (0382): 216.59 1.207 6.75 8.71
=====

ID = 3 (8290): 398.60 1.920 6.75 7.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ROUTE CHN (6384) |
| IN= 2 -> OUT= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (3841.0) ----->
Distance Elevation Manning
0.00 294.40 0.0360

10.59	291.93	0.0380	
21.17	289.19	0.0380	
26.46	287.99	0.0380	
31.76	286.79	0.0380	
71.45	279.97	0.0380	
74.10	279.79	0.0380	
76.74	279.71	0.0380	
78.99	279.64	0.0380 / 0.0300	Main Channel
79.14	279.30	0.0300	Main Channel
79.39	279.30	0.0300	Main Channel
79.64	279.30	0.0300	Main Channel
79.79	279.64	0.0300 / 0.0380	Main Channel
89.98	279.78	0.0380	
119.09	281.86	0.0380	
145.55	282.87	0.0380	
198.48	284.85	0.0380	
211.71	286.31	0.0380	
230.23	287.59	0.0380	
261.99	294.00	0.0380	

TRAVEL TIME TABLE						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.34	279.64	.715E+03	0.2	0.65	63.42	
1.10	280.40	.507E+05	21.9	1.40	38.54	
1.85	281.15	.141E+06	91.8	2.10	25.69	
2.61	281.91	.269E+06	218.9	2.63	20.46	
3.36	282.66	.445E+06	407.1	2.96	18.23	
4.12	283.42	.681E+06	698.3	3.31	16.26	
4.87	284.17	.977E+06	1111.8	3.68	14.65	
5.63	284.93	.133E+07	1674.3	4.06	13.27	
6.39	285.69	.173E+07	2457.8	4.59	11.73	
7.14	286.44	.215E+07	3378.8	5.07	10.63	
7.90	287.20	.261E+07	4418.4	5.47	9.85	
8.65	287.95	.310E+07	5676.3	5.92	9.11	
9.41	288.71	.361E+07	7150.5	6.40	8.42	
10.17	289.47	.414E+07	8783.1	6.86	7.86	
10.92	290.22	.469E+07	10575.1	7.30	7.38	
11.68	290.98	.525E+07	12520.4	7.72	6.98	
12.43	291.73	.582E+07	14619.0	8.12	6.64	
13.19	292.49	.642E+07	16863.2	8.50	6.34	
13.94	293.24	.703E+07	19259.8	8.86	6.08	

<---- hydrograph ----> <-pipe / channel->						
INFLOW : ID=	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
OUTFLOW : ID=	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
2 (8290)	398.60	1.92	6.75	7.26	0.40	0.88
1 (6384)	398.60	1.50	8.00	7.26	0.39	0.87

ADD HYD (8288)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0384):	199.07	0.598	7.25	6.48
+ ID2= 2 (6384):	398.60	1.505	8.00	7.26
ID = 3 (8288):	597.67	2.089	8.00	7.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8284)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8286):	462.04	2.411	7.25	10.89
+ ID2= 2 (8288):	597.67	2.089	8.00	7.00
ID = 3 (8284):	1059.71	4.450	7.75	8.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8280)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0390):	420.00	1.700	7.50	9.40
+ ID2= 2 (8284):	1059.71	4.450	7.75	8.70
ID = 3 (8280):	1479.71	6.150	7.75	8.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB				
NASHYD (0392)	Area	(ha)= 167.22	Curve Number	(CN)= 62.0
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)=	1.50
	U.H. Tp(hrs)=	0.74		

Unit Hyd Qpeak (cms)= 3.837

PEAK FLOW (cms)= 1.133 (1)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 11.712
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.215

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB				
NASHYD (0378)	Area	(ha)= 606.72	Curve Number	(CN)= 55.0
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)=	1.50
	U.H. Tp(hrs)=	1.18		

Unit Hyd Qpeak (cms)= 8.771

PEAK FLOW (cms)= 2.295 (1)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 9.412
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.173

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8282)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0378):	606.72	2.295	8.00	9.41
+ ID2= 2 (0392):	167.22	1.133	7.00	11.71
ID = 3 (8282):	773.94	3.350	7.50	9.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8278)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8280):	1479.71	6.150	7.75	8.90
+ ID2= 2 (8282):	773.94	3.350	7.50	9.91
ID = 3 (8278):	2253.65	9.491	7.50	9.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6394)	
IR= 2--> ODF= 1	Routing time step (min)= 15.00

<----- DATA FOR SECTION (3941.0) ----->

Distance	Elevation	Manning
0.00	283.00	0.0380
13.48	282.87	0.0380
53.92	280.08	0.0380
74.13	276.62	0.0380
97.72	265.45	0.0380
114.57	256.93	0.0380
131.42	253.04	0.0380
134.79	252.58	0.0380
138.53	251.74	0.0380 / 0.0300
139.03	251.20	0.0300
141.53	251.20	0.0300
144.03	251.20	0.0300
144.53	251.74	0.0300 / 0.0380
148.27	252.69	0.0380
151.64	252.97	0.0380
165.34	255.08	0.0380
219.03	257.54	0.0380
262.84	259.43	0.0380
310.02	262.80	0.0380
333.60	283.00	0.0380

TRAVEL TIME TABLE						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.54	251.74	.827E+04	4.5	1.51	30.71	
2.19	253.39	.758E+05	73.2	2.69	17.27	
3.83	255.03	.282E+06	331.8	3.27	14.17	
5.48	256.68	.633E+06	917.3	4.03	11.51	
7.12	258.32	.112E+07	1889.5	4.69	9.90	
8.77	259.97	.179E+07	3410.2	5.21	8.73	
10.41	261.61	.259E+07	5927.6	6.05	7.66	
12.06	263.26	.351E+07	8605.1	6.83	6.79	
13.70	264.90	.447E+07	12544.9	7.81	5.94	
15.35	266.55	.546E+07	17101.4	8.71	5.32	

16.39	268.19	.648E+07	22251.7	9.56	4.85
18.64	269.84	.752E+07	27979.0	10.36	4.48
20.28	271.48	.858E+07	34269.0	11.11	4.17
21.93	273.13	.967E+07	41110.8	11.83	3.92
23.57	274.77	.108E+08	48495.9	12.51	3.71
25.22	276.42	.119E+08	56418.1	13.17	3.52
26.86	278.06	.131E+08	64410.6	13.69	3.39
28.51	279.71	.143E+08	73002.1	14.18	3.27
30.15	281.35	.156E+08	81379.3	14.50	3.20

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<---- hydrograph ----> <-pipe / channel->
      AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
      (ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
INFLOW : ID= 2 (8278) 2253.65  9.49  7.50  9.24  0.66  1.56
OUTFLOW: ID= 1 (6394) 2253.65  9.22  8.25  9.24  0.65  1.56

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ADD HYD (8276) |
1 + 2 = 3 |
      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)   (hrs)   (mm)
+ ID1= 1 (0394): 325.45  1.379  7.25  8.78
+ ID2= 2 (6394): 2253.65  9.215  8.25  9.24
ID = 3 (8276): 2579.10  10.506  8.00  9.19

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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ROUTE CHN (6396) |
IN= 2--> OUT= 1 |
Routing time step (min)= 15.00

```

<----- DATA FOR SECTION (3961.0) ----->

Distance	Elevation	Manning
0.00	263.00	0.0410
11.75	257.14	0.0410
23.50	253.97	0.0410
41.13	247.83	0.0410
76.38	232.09	0.0410
135.13	229.07	0.0410
149.82	228.97	0.0410
152.75	228.96	0.0410 / 0.0300
154.19	228.73	0.0300
154.69	228.20	0.0300
155.69	228.20	0.0300
156.69	228.20	0.0300
157.19	228.73	0.0300
158.63	228.95	0.0300 / 0.0410
161.57	228.96	0.0410
164.51	229.71	0.0410
196.82	241.70	0.0410
223.26	249.21	0.0410
246.76	255.13	0.0410
290.82	263.51	0.0410

```

<----- TRAVEL TIME TABLE ----->
DEPTH  ELEV  VOLUME  FLOW RATE  VELOCITY  TRAV.TIME
(m)    (m)    (cu.m.)  (cms)      (m/s)     (min)
0.75  228.95  .780E+04  2.2  0.98  58.06
2.54  230.74  .277E+06  145.1  1.78  31.76
4.33  232.53  .786E+06  609.1  2.63  21.50
6.13  234.33  .140E+07  1483.5  3.59  15.77
7.92  236.12  .207E+07  2677.8  4.39  12.90
9.71  237.91  .280E+07  4184.8  5.09  11.14
11.50 239.70  .357E+07  6001.4  5.71  9.93
13.29 241.49  .441E+07  8123.1  6.27  9.03
15.09 243.29  .530E+07  10549.4  6.77  8.37
16.88 245.08  .625E+07  13300.6  7.24  7.83
18.67 246.87  .726E+07  16396.2  7.68  7.38
20.46 248.66  .834E+07  19809.8  8.07  7.02
22.26 250.46  .949E+07  23532.5  8.43  6.72
24.05 252.25  .107E+08  27641.2  8.77  6.46
25.84 254.04  .120E+08  32155.2  9.10  6.22
27.63 255.83  .134E+08  36901.0  9.37  6.05
29.42 257.62  .149E+08  42147.7  9.64  5.88
31.22 259.42  .164E+08  48149.1  9.97  5.68
33.01 261.21  .181E+08  54644.8  10.29  5.51

```

```

<---- hydrograph ----> <-pipe / channel->
      AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
      (ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
INFLOW : ID= 2 (8276) 2579.10  10.51  8.00  9.19  0.85  1.00
OUTFLOW: ID= 1 (6396) 2579.10  9.52  9.25  9.19  0.84  1.00

```

```

ADD HYD (8274) |
1 + 2 = 3 |
      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)   (hrs)   (mm)
ID1= 1 (0396): 305.21  1.961  7.50  14.79

```

```

+ ID2= 2 (6396): 2579.10  9.515  9.25  9.19
ID = 3 (8274): 2884.31  11.197  9.00  9.78

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8270) |
1 + 2 = 3 |
      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)   (hrs)   (mm)
ID1= 1 (8272): 2776.21  8.592  10.25  10.64
+ ID2= 2 (8274): 2884.31  11.197  9.00  9.78
ID = 3 (8270): 5660.52  19.501  9.50  10.20

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

RESERVOIR (5506) |
IN= 2--> OUT= 1 |
DT= 15.0 min |
      OUTFLOW  STORAGE  OUTFLOW  STORAGE
      (cms)    (ha.m.)  (cms)    (ha.m.)
0.0000  0.0000  60.8810  135.6832
31.1490  24.6697  96.2770  900.4431
36.8120  37.0045  96.6770  900.4531
45.3070  86.3439  0.0000  0.0000

```

```

      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)   (hrs)   (mm)
INFLOW : ID= 2 (8270) 5660.520  19.501  9.50  10.20
OUTFLOW: ID= 1 (5506) 5660.520  16.142  11.75  10.20

```

PEAK FLOW REDUCTION [Qout/Qin](%)= 82.78
TIME SHIFT OF PEAK FLOW (min)=135.00
MAXIMUM STORAGE USED (ha.m.)= 12.7854

```

CALIB (0406) |
NASHVD (0406) |
ID= 1 DT=15.0 min |
      Area (ha)= 142.65  Curve Number (CN)= 66.0
      Ia (mm)= 5.00  # of Linear Res.(N)= 1.50
      U.H. Tp(hrs)= 0.59

```

Unit Hyd Qpeak (cms)= 4.135

PEAK FLOW (cms)= 1.319 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 13.237
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.243

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB (0404) |
NASHVD (0404) |
ID= 1 DT=15.0 min |
      Area (ha)= 246.46  Curve Number (CN)= 47.0
      Ia (mm)= 5.00  # of Linear Res.(N)= 1.50
      U.H. Tp(hrs)= 0.98

```

Unit Hyd Qpeak (cms)= 4.280

PEAK FLOW (cms)= 0.810 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 7.191
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.132

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB (0402) |
NASHVD (0402) |
ID= 1 DT=15.0 min |
      Area (ha)= 244.00  Curve Number (CN)= 61.0
      Ia (mm)= 5.00  # of Linear Res.(N)= 1.50
      U.H. Tp(hrs)= 1.07

```

Unit Hyd Qpeak (cms)= 3.879

PEAK FLOW (cms)= 1.206 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 11.417
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.210

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB (0400) |
NASHVD (0400) |
ID= 1 DT=15.0 min |
      Area (ha)= 93.97  Curve Number (CN)= 52.0
      Ia (mm)= 5.00  # of Linear Res.(N)= 1.50

```

U.H. Tp(hrs)= 0.44
 Unit Hyd Qpeak (cms) = 3.630
 PEAK FLOW (cms) = 0.675 (1)
 TIME TO PEAK (hrs) = 6.500
 RUNOFF VOLUME (mm) = 8.305
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.153

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHYD (0398) | Area (ha) = 328.19 Curve Number (CN) = 55.0
 ID= 1 DT=15.0 min | Ia (mm) = 5.00 # of Linear Res.(N) = 1.50
 U.H. Tp(hrs) = 0.83

Unit Hyd Qpeak (cms) = 6.759
 PEAK FLOW (cms) = 1.617 (1)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 9.360
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.172

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8268) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0398): 328.19 1.617 7.00 9.36
 + ID2= 2 (0400): 93.97 0.675 6.50 8.30
 ID = 3 (8268): 422.16 2.224 6.75 9.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6402) |
 IN= 2--> OUT= 1 | Routing time step (min) = 15.00

DATA FOR SECTION (4021.0) ----->

Distance	Elevation	Manning	
0.00	238.50	0.0360	
11.50	238.00	0.0360	
23.00	237.93	0.0360	
34.49	236.39	0.0360	
63.24	233.98	0.0360	
97.73	228.15	0.0360	
123.60	227.08	0.0360	
126.48	226.61	0.0360	
127.60	226.47	0.0360 / 0.0330	Main Channel
127.85	225.25	0.0330	Main Channel
129.35	225.25	0.0330	Main Channel
130.85	225.25	0.0330	Main Channel
131.10	226.47	0.0330 / 0.0360	Main Channel
132.22	226.59	0.0360	
143.72	227.42	0.0360	
169.59	227.88	0.0360	
192.59	231.19	0.0360	
218.46	233.02	0.0360	
241.45	235.50	0.0360	
284.57	236.43	0.0360	

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.41	225.66	.391E+04	1.7	1.36	38.31
0.81	226.06	.804E+04	5.0	1.93	27.02
1.22	226.47	.124E+05	9.1	2.30	22.61
1.84	227.09	.309E+05	23.3	2.36	22.03
2.46	227.71	.904E+05	62.0	2.14	24.31
3.09	228.34	.223E+06	188.3	2.63	19.75
3.71	228.96	.379E+06	410.2	3.38	15.39
4.33	229.58	.550E+06	711.5	4.04	12.89
4.95	230.20	.737E+06	1091.3	4.62	11.25
5.58	230.83	.939E+06	1550.6	5.15	10.10
6.20	231.45	.116E+07	2072.4	5.59	9.31
6.82	232.07	.140E+07	2655.2	5.93	8.77
7.44	232.69	.167E+07	3345.8	6.27	8.30
8.07	233.32	.195E+07	4159.0	6.64	7.83
8.69	233.94	.226E+07	5100.8	7.03	7.39
9.31	234.56	.259E+07	6081.3	7.32	7.11
9.93	235.18	.295E+07	7183.1	7.60	6.84
10.56	235.81	.334E+07	8145.2	7.62	6.83
11.18	236.43	.379E+07	9116.0	7.51	6.92

hydrograph -----> <- pipe / channel ->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL

INFLOW : ID= 2 (8268) 422.16 (ha) (cms) (hrs) (mm) (m) (m/s)
 2.22 6.75 9.12 0.47 1.43
 OUTFLOW: ID= 1 (6402) 422.16 2.00 7.75 9.12 0.44 1.39

ADD HYD (8266) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0402): 244.00 1.206 7.50 11.42
 + ID2= 2 (6402): 422.16 1.996 7.75 9.12
 ID = 3 (8266): 666.16 3.200 7.75 9.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8264) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0404): 246.46 0.810 7.50 7.19
 + ID2= 2 (8266): 666.16 3.200 7.75 9.96
 ID = 3 (8264): 912.62 4.005 7.75 9.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8262) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0406): 142.65 1.319 6.75 13.24
 + ID2= 2 (8264): 912.62 4.005 7.75 9.12
 ID = 3 (8262): 1055.27 5.146 7.25 9.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8260) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (5506): 5660.52 16.142 11.75 10.20
 + ID2= 2 (8262): 1055.27 5.146 7.25 9.76
 ID = 3 (8260): 6715.79 18.516 11.25 10.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8258) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0408): 231.62 1.542 6.75 10.25
 + ID2= 2 (8260): 6715.79 18.516 11.25 10.13
 ID = 3 (8258): 6947.41 18.994 11.00 10.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8256) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0410): 572.01 1.460 8.25 7.48
 + ID2= 2 (8258): 6947.41 18.994 11.00 10.14
 ID = 3 (8256): 7519.42 20.184 11.00 9.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5507) |
 IN= 2--> OUT= 1 | DT= 15.0 min |

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	90.6140	67.8416
39.6440	12.3348	*****	160.3529
48.1300	18.5023	*****	160.3529
67.9600	37.0045	0.0000	0.0000

AREA QPEAK TPEAK R.V.

INFLW : ID= 2 (8256) (ha) (cms) (hrs) (mm)
 7519.421 20.184 11.00 9.93
 OUTFLOW: ID= 1 (5507) 7519.421 19.753 12.00 9.93

PEAK FLOW REDUCTION [Qout/Qin](%)= 97.87
 TIME SHIFT OF PEAK FLOW (min)= 60.00
 MAXIMUM STORAGE USED (ha.m.)= 6.1500

CALIB |
 NASHYD (0420) | Area (ha)= 175.82 Curve Number (CN)= 53.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.81

Unit Hyd Qpeak (cms)= 3.692
 PEAK FLOW (cms)= 0.822 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 8.762
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.161

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHYD (0418) | Area (ha)= 182.79 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.05

Unit Hyd Qpeak (cms)= 2.966
 PEAK FLOW (cms)= 1.014 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 12.572
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.231

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHYD (0416) | Area (ha)= 439.20 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.29

Unit Hyd Qpeak (cms)= 5.832
 PEAK FLOW (cms)= 2.108 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 12.603
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.232

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHYD (0412) | Area (ha)= 238.70 Curve Number (CN)= 54.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.80

Unit Hyd Qpeak (cms)= 5.088
 PEAK FLOW (cms)= 1.169 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 9.052
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.166

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 STANSHYD (4141) | Area (ha)= 43.70
 ID= 1 DT=15.0 min | Total Imp(%)= 36.00 Dir. Conn.(%)= 17.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	15.73	27.97
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	539.75	40.00
Mannings n	=	0.013	0.250
Max. Eff. Inten. (mm/hr)=		71.81	18.99
over (min)		15.00	45.00
Storage Coeff. (min)=		9.87 (ii)	30.67 (iii)
Unit Hyd. Tpeak (min)=		15.00	45.00
Unit Hyd. peak (cms)=		0.09	0.03

TOTALS

PEAK FLOW (cms)= 1.26 0.90 1.628 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 52.40 15.90 22.10
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.29 0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 64.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5414) |
 ID= 2 --> OUT= 1 |
 DT= 15.0 min |

		OUTFLOW	STORAGE	OUTFLOW	STORAGE
		(cms)	(ha.m.)	(cms)	(ha.m.)
		0.0000	0.0000	3.4720	1.9177
		0.3820	0.3152	3.4720	1.9277
		1.8280	0.4690	0.0000	0.0000

		AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
INFLW : ID= 2 (4141)		43.700	1.628	6.00	22.10
OUTFLOW: ID= 1 (5414)		43.700	0.795	6.75	22.10

PEAK FLOW REDUCTION [Qout/Qin](%)= 48.83
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 0.3604

CALIB |
 STANSHYD (4142) | Area (ha)= 144.30
 ID= 1 DT=15.0 min | Total Imp(%)= 21.00 Dir. Conn.(%)= 10.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	30.30	114.00
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	980.82	40.00
Mannings n	=	0.013	0.250
Max. Eff. Inten. (mm/hr)=		71.81	14.82
over (min)		15.00	45.00
Storage Coeff. (min)=		14.13 (ii)	37.09 (iii)
Unit Hyd. Tpeak (min)=		15.00	45.00
Unit Hyd. peak (cms)=		0.07	0.03

		PEAK FLOW	TPEAK	R.V.
		(cms)	(hrs)	(mm)
		2.15	2.62	3.220 (iii)
		6.00	6.50	6.50
		52.40	14.26	18.07
		54.40	54.40	54.40
		0.96	0.26	0.33

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 64.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8254) |
 1 + 2 = 3 |

		AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (4142):		144.30	3.220	6.50	18.07
+ ID2= 2 (5414):		43.70	0.795	6.75	22.10
=====					
ID = 3 (8254):		188.00	3.727	6.50	19.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8252) |
 1 + 2 = 3 |

		AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0412):		238.70	1.169	7.00	9.05
+ ID2= 2 (8254):		188.00	3.727	6.50	19.01
=====					
ID = 3 (8252):		426.70	4.816	6.50	13.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6416) |
| IN= 2--> OUT= 1 |
-----
| Routing time step (min)= 15.00
-----
<----- DATA FOR SECTION (4161.0) ----->
Distance      Elevation      Manning
0.00          270.07         0.0340
20.67        267.91         0.0340
62.01        264.33         0.0340
113.69       259.75         0.0340
165.37       253.30         0.0340
227.38       246.29         0.0340
232.55       246.03         0.0340
237.72       246.16         0.0340
241.39       246.02         0.0340 / 0.0300 Main Channel
241.64       245.75         0.0300 Main Channel
242.89       245.75         0.0300 Main Channel
244.14       245.75         0.0300 Main Channel
244.39       246.02         0.0300 / 0.0340 Main Channel
248.06       246.20         0.0340
253.22       246.28         0.0340
258.39       246.63         0.0340
346.25       252.57         0.0340
413.43       257.77         0.0340
465.11       261.78         0.0340
511.62       270.00         0.0340

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH      ELVY      VOLUME      FLOW RATE      VELOCITY      TRAV.TIME
(m)        (m)        (cu.m.)     (cms)          (m/s)         (min)
0.27       246.02     .295E+04    0.6            0.75          88.26
1.53       247.28     .168E+06    69.7          1.65          40.13
2.79       246.54     .490E+06    299.5         2.42          27.28
4.06       249.81     .962E+06    740.1         3.05          21.67
5.32       251.07     .158E+07    1440.0        3.61          18.33
6.58       252.33     .235E+07    2443.8        4.12          16.06
7.84       253.59     .327E+07    3811.2        4.63          14.30
9.10       254.85     .432E+07    5565.1        5.11          12.94
10.37      256.12     .550E+07    7721.3        5.57          11.88
11.63      257.38     .682E+07    10309.9       6.00          11.02
12.89      258.64     .827E+07    13360.6       6.42          10.31
14.15      259.90     .985E+07    16884.5       6.81          9.72
15.42      261.17     .116E+08    20818.4       7.14          9.26
16.68      262.43     .134E+08    25510.4       7.53          8.78
17.94      263.69     .154E+08    30958.0       7.97          8.30
19.20      264.95     .175E+08    36988.3       8.38          7.89
20.46      266.21     .197E+08    43617.1       8.78          7.53
21.73      267.48     .220E+08    50865.1       9.17          7.22
22.99      268.74     .244E+08    58854.0       9.56          6.92

```

```

<---- hydrograph ----> <-pipe / channel->
AREA      QPEAK      TPEAK      R.V.      MAX DEPTH  MAX VEL
(ha)      (cms)      (hrs)      (mm)      (m)         (m/s)
INFLOW : ID= 2 (8252) 426.70    4.82    6.50    13.44    0.35    0.78
OUTFLOW: ID= 1 (6416) 426.70    2.78    7.50    13.44    0.31    0.76

```

```

-----
| ADD HYD (8250) |
| 1 + 2 = 3 |
-----
| AREA      QPEAK      TPEAK      R.V.
| (ha)      (cms)      (hrs)      (mm)
ID1= 1 (0416): 439.30    2.108    8.00    12.60
+ ID2= 2 (6416): 426.70    2.775    7.50    13.44
=====
ID = 3 (8250): 866.00    4.861    7.75    13.01

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8248) |
| 1 + 2 = 3 |
-----
| AREA      QPEAK      TPEAK      R.V.
| (ha)      (cms)      (hrs)      (mm)
ID1= 1 (0418): 182.79    1.014    7.50    12.57
+ ID2= 2 (8250): 866.00    4.861    7.75    13.01
=====
ID = 3 (8248): 1048.79    5.871    7.75    12.94

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8246) |
| 1 + 2 = 3 |
-----
| AREA      QPEAK      TPEAK      R.V.
| (ha)      (cms)      (hrs)      (mm)
ID1= 1 (0420): 175.82    0.822    7.00    8.76
+ ID2= 2 (8248): 1048.79    5.871    7.75    12.94
=====
ID = 3 (8246): 1224.61    6.658    7.50    12.34

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8244) |
| 1 + 2 = 3 |
-----
| AREA      QPEAK      TPEAK      R.V.
| (ha)      (cms)      (hrs)      (mm)
ID1= 1 (5507): 7519.42    19.753    12.00    9.93
+ ID2= 2 (8246): 1224.61    6.658    7.50    12.34
=====
ID = 3 (8244): 8744.03    23.417    11.25    10.27

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8242) |
| 1 + 2 = 3 |
-----
| AREA      QPEAK      TPEAK      R.V.
| (ha)      (cms)      (hrs)      (mm)
ID1= 1 (0422): 780.20    1.954    9.25    9.15
+ ID2= 2 (8244): 8744.03    23.417    11.25    10.27
=====
ID = 3 (8242): 9524.23    25.245    11.00    10.18

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| RESERVOIR (5508) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
| OUTFLOW      STORAGE      OUTFLOW      STORAGE
| (cms)        (ha.m.)      (cms)        (ha.m.)
| 0.0000      0.0000      | *****    197.3574
| 76.4550    30.8371    | *****    394.7148
| *****    61.6742    | *****    394.7248
| *****    *****    | 0.0000     0.0000
-----
| AREA      QPEAK      TPEAK      R.V.
| (ha)      (cms)      (hrs)      (mm)
INFLOW : ID= 2 (8242) 9524.231    25.245    11.00    10.18
OUTFLOW: ID= 1 (5508) 9524.231    24.490    12.25    10.18
-----
| PEAK FLOW REDUCTION [Qout/Qin](%)= 97.01
| TIME SPLIT OF PEAK FLOW (min)= 75.00
| MAXIMUM STORAGE USED (ha.m.)= 9.8788
-----

```

```

-----
| CALIB |
| NASHYD (0336) |
| ID= 1 DT=15.0 min |
| U.H. Tp(hrs)= 15.39 |
-----
| Area (ha)= 2785.00
| Curve Number (CN)= 72.0
| # of Linear Res.(N)= 1.50
-----
| Unit Hyd Qpeak (cms)= 3.090

```

```

| PEAK FLOW (cms)= 1.997 (i)
| TIME TO PEAK (hrs)= 22.250
| RUNOFF VOLUME (mm)= 16.446
| TOTAL RAINFALL (mm)= 54.400
| RUNOFF COEFFICIENT = 0.302

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANDHYD (3382) |
| ID= 1 DT=15.0 min |
| Area (ha)= 525.90
| Total Imp(%)= 37.00
| Dir. Conn.(%)= 17.00
-----
| IMPERVIOUS      PERVIOUS (i)
| Surface Area (ha)= 194.58    331.32
| Dep. Storage (mm)= 2.00      5.00
| Average Slope (%)= 0.50      0.50
| Length (m)= 1872.43         40.00
| Mannings n = 0.013          0.250
-----
| Max.Eff.Inten.(mm/hr)= 48.96    20.86
| over (min)= 30.00              45.00
| Storage Coeff. (min)= 24.27 (ii) 44.30 (ii)
| Unit Hyd. Tpeak (min)= 30.00     45.00
| Unit Hyd. peak (cms)= 0.04       0.03
-----
| PEAK FLOW (cms)= 8.78    9.65    15.834 (iii)
| TIME TO PEAK (hrs)= 6.25    6.50    6.25
| RUNOFF VOLUME (mm)= 52.40    17.08    23.08
| TOTAL RAINFALL (mm)= 54.40    54.40    54.40
| RUNOFF COEFFICIENT = 0.96    0.31    0.42

```

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA. (1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

CN* = 66.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (3381) Area (ha)= 73.00
ID= 1 DT=15.0 min Total Imp(%)= 49.00 Dir. Conn.(%)= 23.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	35.77	37.23	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	697.61	40.00	
Mannings n	= 0.013	0.250	
Max.Eff.Inten.(mm/hr)=	71.81	43.61	
over (min)	15.00	30.00	
Storage Coeff. (min)=	11.52 (ii)	26.43 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.08	0.04	
TOTALS			
PEAK FLOW (cms)=	2.70	1.99	3.937 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	52.40	18.95	26.64
TOTAL RAINFALL (mm)=	54.40	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.35	0.49

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 66.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.3350	1.2400
0.3230	0.3074	4.3560	1.3520
0.9360	0.5374	5.3290	1.4590
1.7590	0.8302	5.7290	1.4690
2.4110	1.0550	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
INFLOW : ID= 2 (3381)	73.000	3.937	6.00	26.64
OUTFLOW: ID= 1 (5338)	73.000	1.561	6.75	26.64

PEAK FLOW REDUCTION [Qout/Qin](%)= 39.64
TIME SHIFT OF PEAK FLOW (min)= 45.00
MAXIMUM STORAGE USED (ha.m.)= 0.7662

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
ADD HYD (8310) 1 + 2 = 3				
ID1= 1 (3382):	525.90	15.834	6.25	23.09
+ ID2= 2 (5338):	73.00	1.561	6.75	26.64
ID = 3 (8310):	598.90	17.031	6.25	23.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
CALIB NASHYD (3342) ID= 1 DT=15.0 min	587.10	0.467	15.50	9.80
U.H. Tp(hrs)=	8.19			

Unit Hyd Qpeak (cms)= 1.224
PEAK FLOW (cms)= 0.467 (i)
TIME TO PEAK (hrs)= 15.50
RUNOFF VOLUME (mm)= 9.797
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.180

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

AREA (ha)	Dir. Conn.(%)	
CALIB STANDHYD (3341) ID= 1 DT=15.0 min	33.00	31.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	16.83	16.17
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	469.04	40.00
Mannings n	= 0.013	0.250

Max.Eff.Inten.(mm/hr)=	71.81	15.09	
over (min)	15.00	45.00	
Storage Coeff. (min)=	9.08 (ii)	31.87 (ii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.09	0.03	
TOTALS			
PEAK FLOW (cms)=	1.77	0.41	1.938 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	52.40	11.90	24.45
TOTAL RAINFALL (mm)=	54.40	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.22	0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 52.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.6540	0.4604
0.0390	0.1699	0.8330	0.5249
0.1860	0.2610	1.0130	0.5896
0.3750	0.3534	1.4130	0.5996
0.4340	0.3813	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
INFLOW : ID= 2 (3341)	33.000	1.938	6.00	24.45
OUTFLOW: ID= 1 (5334)	33.000	0.481	6.75	24.42

PEAK FLOW REDUCTION [Qout/Qin](%)= 24.84
TIME SHIFT OF PEAK FLOW (min)= 45.00
MAXIMUM STORAGE USED (ha.m.)= 0.3985

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
ADD HYD (8314) 1 + 2 = 3				
ID1= 1 (3342):	587.10	0.467	15.50	9.80
+ ID2= 2 (5334):	33.00	0.481	6.75	24.42
ID = 3 (8314):	620.10	0.655	7.00	10.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
CALIB NASHYD (0306) ID= 1 DT=15.0 min	283.97	5.00	5.00	1.50
U.H. Tp(hrs)=	6.44			

Unit Hyd Qpeak (cms)= 0.753
PEAK FLOW (cms)= 0.250 (i)
TIME TO PEAK (hrs)= 13.750
RUNOFF VOLUME (mm)= 8.592
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.158

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
CALIB NASHYD (0286) ID= 1 DT=15.0 min	260.51	5.00	5.00	1.50
U.H. Tp(hrs)=	1.16			

Unit Hyd Qpeak (cms)= 3.834
PEAK FLOW (cms)= 2.734 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 24.753
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.455

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0282) | Area (ha)= 449.38 Curve Number (CN)= 77.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 3.47
-----
Unit Hyd Qpeak (cms)= 5.226

PEAK FLOW (cms)= 3.035 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 19.368
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.356

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

```

-----
| CALIB |
| NASHYD (0284) | Area (ha)= 78.93 Curve Number (CN)= 84.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.57
-----
Unit Hyd Qpeak (cms)= 2.344

PEAK FLOW (cms)= 1.445 (1)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 24.380
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.448

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

```

-----
| ADD HYD (8388) |
| 1 + 2 = 3 |
-----
ID1= 1 (0282): 449.38 3.035 8.00 19.37
+ ID2= 2 (0284): 78.93 1.445 6.50 24.38
-----
ID = 3 (8388): 528.31 4.143 7.25 20.12
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

```

-----
| ROUTE CHN (6286) |
| IN= 2---> OUT= 1 | Routing time step (min)= 15.00
-----
<----- DATA FOR SECTION (2861.0) ----->
Distance Elevation Manning
0.00 233.00 0.0450
20.58 228.51 0.0450
41.17 227.74 0.0450
51.46 227.41 0.0450
97.77 225.96 0.0450
149.23 223.94 0.0450
200.69 220.84 0.0450
226.42 220.66 0.0450
238.85 220.22 0.0450
241.35 220.01 0.0450 / 0.0350 Main Channel
241.85 219.70 0.0350 Main Channel
245.85 219.72 0.0350 Main Channel
246.35 220.06 0.0350 Main Channel
248.85 220.23 0.0350 Main Channel
303.60 221.64 0.0350 Main Channel
380.79 224.98 0.0450
432.25 229.54 0.0450
457.98 233.33 0.0450
483.71 234.27 0.0450
509.44 233.81 0.0450

```

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-----
<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.31 220.01 .382E+04 0.8 0.57 82.89
1.06 220.76 .525E+05 12.1 0.66 72.51
1.81 221.51 .240E+06 88.6 1.05 45.21
2.56 222.26 .510E+06 261.7 1.46 32.47
3.31 223.01 .843E+06 533.8 1.81 26.34
4.06 223.76 .124E+07 913.6 2.10 22.64
4.81 224.51 .171E+07 1399.0 2.34 20.33
5.56 225.26 .225E+07 2051.0 2.60 18.27
6.31 226.01 .285E+07 2907.0 2.91 16.35
7.06 226.76 .352E+07 3902.5 3.16 15.04
7.82 227.52 .426E+07 5069.9 3.39 14.01
8.57 228.27 .507E+07 6433.0 3.62 13.13
9.32 229.02 .593E+07 8069.2 3.89 12.24
10.07 229.77 .681E+07 9925.7 4.16 11.44
10.82 230.52 .772E+07 11962.8 4.42 10.75
11.57 231.27 .864E+07 14167.1 4.68 10.17
12.32 232.02 .959E+07 16537.1 4.92 9.66

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```

13.07 232.77 .105E+08 19070.7 5.16 9.22
13.82 233.52 .115E+08 21790.4 5.39 8.82
-----
<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8388) 528.31 4.14 7.25 20.12 0.53 0.60
OUTFLOW: ID= 1 (6286) 528.31 3.51 9.25 20.12 0.49 0.59

```

```

-----
| ADD HYD (8386) |
| 1 + 2 = 3 |
-----
ID1= 1 (0286): 260.51 2.734 7.50 24.75
+ ID2= 2 (6286): 528.31 3.506 9.25 20.12
-----
ID = 3 (8386): 788.82 5.992 8.50 21.65
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

```

-----
| CALIB |
| NASHYD (0302) | Area (ha)= 473.90 Curve Number (CN)= 58.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.66
-----
Unit Hyd Qpeak (cms)= 4.874

PEAK FLOW (cms)= 1.530 (1)
TIME TO PEAK (hrs)= 8.500
RUNOFF VOLUME (mm)= 10.409
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.191

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

```

-----
| CALIB |
| NASHYD (0300) | Area (ha)= 258.93 Curve Number (CN)= 52.0
| ID= 1 DT=15.0 min | Ia (mm)= 4.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.03
-----
Unit Hyd Qpeak (cms)= 4.292

PEAK FLOW (cms)= 1.017 (1)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 8.830
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.162

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

```

-----
| CALIB |
| NASHYD (0298) | Area (ha)= 330.51 Curve Number (CN)= 45.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.26
-----
Unit Hyd Qpeak (cms)= 4.461

PEAK FLOW (cms)= 0.846 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 6.733
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.124

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

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-----
| ADD HYD (8395) |
| 1 + 2 = 3 |
-----
ID1= 1 (0298): 330.51 0.846 8.00 6.73
+ ID2= 2 (0300): 258.93 1.017 7.50 8.83
-----
ID = 3 (8395): 589.44 1.852 7.75 7.65
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

```

-----
| CALIB |
| NASHYD (0288) | Area (ha)= 340.83 Curve Number (CN)= 78.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 2.21
-----
Unit Hyd Qpeak (cms)= 2.629

```

PEAK FLOW (cms)= 1.727 (i)
 TIME TO PEAK (hrs)= 9.500
 RUNOFF VOLUME (mm)= 20.098
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.369

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHYD (0290) | Area (ha)= 269.18 Curve Number (CN)= 78.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 4.279

PEAK FLOW (cms)= 2.388 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 19.976
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.367

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8397) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0288): 340.83 1.727 9.50 20.10
 + ID2= 2 (0290): 269.18 2.388 7.50 19.98
 ID = 3 (8397): 610.01 3.977 8.00 20.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB |
 NASHYD (0296) | Area (ha)= 293.65 Curve Number (CN)= 76.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.13

Unit Hyd Qpeak (cms)= 4.437

PEAK FLOW (cms)= 2.330 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 18.668
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.343

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHYD (0292) | Area (ha)= 738.49 Curve Number (CN)= 68.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.52

Unit Hyd Qpeak (cms)= 8.289

PEAK FLOW (cms)= 3.557 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 14.367
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.264

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHYD (0294) | Area (ha)= 274.15 Curve Number (CN)= 57.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.87

Unit Hyd Qpeak (cms)= 5.367

PEAK FLOW (cms)= 1.387 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 9.999
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.184

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8398) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)

(ha) (cms) (hrs) (mm)
 ID1= 1 (0292): 738.49 3.557 8.25 14.37
 + ID2= 2 (0294): 274.15 1.387 7.25 10.00
 ID = 3 (8398): 1012.64 4.858 8.00 13.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6296) |
 IN= 2--> OUT= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (2961.0) ----->

Distance	Elevation	Manning
0.00	243.98	0.0400
42.89	243.18	0.0400
85.17	241.81	0.0400
120.66	240.50	0.0400
156.15	239.56	0.0400
198.74	236.15	0.0400
237.78	234.01	0.0400
241.33	233.82	0.0400
248.77	233.12	0.0400 / 0.0400
249.87	232.32	0.0400
250.37	231.80	0.0400
250.87	232.23	0.0400
251.97	233.10	0.0400 / 0.0400
255.37	233.22	0.0400
259.07	233.87	0.0400
262.62	234.12	0.0400
266.17	234.23	0.0400
283.91	234.73	0.0400
337.15	241.75	0.0400
351.34	244.00	0.0400

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.43	232.23	.510E+03	0.1	0.53	81.07
0.87	232.67	.213E+04	0.7	0.83	51.49
1.30	233.10	.501E+04	2.2	1.10	38.75
1.98	233.78	.242E+05	12.6	1.34	31.93
2.66	234.46	.738E+05	42.1	1.46	29.21
3.34	235.14	.177E+06	127.9	1.84	23.13
4.02	235.82	.314E+06	281.4	2.29	18.61
4.70	236.50	.481E+06	509.4	2.71	15.73
5.38	237.18	.672E+06	815.6	3.11	13.73
6.06	237.86	.886E+06	1200.7	3.46	12.30
6.74	238.54	.112E+07	1669.4	3.80	11.23
7.42	239.22	.139E+07	2226.9	4.11	10.38
8.10	239.90	.168E+07	2808.4	4.28	9.95
8.78	240.58	.202E+07	3470.4	4.40	9.68
9.46	241.26	.240E+07	4331.7	4.61	9.24
10.14	241.94	.283E+07	5335.2	4.83	8.84
10.82	242.62	.330E+07	6482.6	5.03	8.48
11.50	243.30	.381E+07	7764.1	5.21	8.18
12.18	243.98	.439E+07	9101.2	5.31	8.04

<---- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8398) 1012.64 4.86 8.00 13.18 1.48 1.15
 OUTFLOW: ID= 1 (6296) 1012.64 4.69 8.75 13.18 1.46 1.15

ADD HYD (8396) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0296): 293.65 2.330 7.50 18.67
 + ID2= 2 (6296): 1012.64 4.688 8.75 13.18
 ID = 3 (8396): 1306.29 6.905 8.25 14.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8394) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8396): 1306.29 6.905 8.25 14.42
 + ID2= 2 (8397): 610.01 3.977 8.00 20.04
 ID = 3 (8394): 1916.30 10.857 8.25 16.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8392)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8394):	1916.30	10.857	8.25	16.21
+ ID2= 2 (8395):	589.44	1.852	7.75	7.65
ID = 3 (8392):	2505.74	12.674	8.25	14.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CWN (8302)
IN# 2--> OUT# 1

Routing time step (min)'= 15.00

Distance	Elevation	Manning	
0.00	228.10	0.0400	
18.47	227.12	0.0400	
36.95	226.12	0.0400	
46.18	225.84	0.0400	
55.42	225.58	0.0400	
272.47	222.88	0.0400	
277.09	222.76	0.0400	
281.71	222.58	0.0400	
288.54	222.18	0.0400 /0.0300	Main Channel
288.64	221.00	0.0300	Main Channel
290.94	221.00	0.0300	Main Channel
291.04	221.00	0.0300	Main Channel
291.54	222.75	0.0300 /0.0400	Main Channel
300.18	222.83	0.0400	
304.80	223.04	0.0400	
309.42	223.25	0.0400	
318.65	223.69	0.0400	
360.22	225.57	0.0400	
397.16	227.60	0.0400	
457.20	228.35	0.0400	

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.29	221.29	.228E+04	0.2	0.32	165.63
0.59	221.59	.455E+04	0.7	0.45	116.15
0.88	221.88	.713E+04	1.2	0.54	96.87
1.18	222.18	.971E+04	1.9	0.61	86.20
1.57	222.57	.175E+05	3.3	0.59	88.86
1.97	222.97	.427E+05	6.4	0.47	111.39
2.36	223.36	.115E+06	16.1	0.44	119.14
2.76	223.76	.237E+06	37.1	0.49	106.49
3.15	224.15	.409E+06	73.9	0.56	93.40
3.55	224.55	.631E+06	127.0	0.63	82.76
3.94	224.94	.903E+06	202.4	0.70	74.37
4.34	225.34	.123E+07	301.8	0.77	67.68
4.73	225.73	.160E+07	433.5	0.85	61.37
5.13	226.13	.200E+07	602.2	0.95	55.29
5.52	226.52	.242E+07	804.7	1.05	50.13
5.92	226.92	.286E+07	1034.2	1.14	46.11
6.31	227.31	.332E+07	1290.5	1.22	42.88
6.71	227.71	.380E+07	1560.9	1.29	40.55
7.10	228.10	.432E+07	1835.6	1.34	39.19

INFLOW : ID= 2 (8392)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
ID= 1 (8302)	2505.74	12.67	8.25	14.20	2.23	0.45
OUTFLOW: ID= 1 (8302)	2505.74	10.20	10.50	14.20	2.12	0.46

ADD HYD (8390)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0302):	473.90	1.530	8.50	10.41
+ ID2= 2 (8302):	2505.74	10.198	10.50	14.20
ID = 3 (8390):	2979.64	11.602	10.25	13.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8348)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8386):	788.82	5.992	8.50	21.65
+ ID2= 2 (8390):	2979.64	11.602	10.25	13.59
ID = 3 (8348):	3768.46	17.080	9.75	15.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 63.0
NASHYD (0304)	292.37	5.00	# of Linear Res.(N)= 1.50	
ID= 1 DT=15.0 min	5.00	2.78		

Unit Hyd Qpeak (cms)= 1.793

PEAK FLOW (cms)= 0.753 (i)
TIME TO PEAK (hrs)= 10.500
RUNOFF VOLUME (mm)= 12.262
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.225

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 82.0
NASHYD (0280)	299.86	5.00	# of Linear Res.(N)= 1.50	
ID= 1 DT=15.0 min	5.00	0.85		

Unit Hyd Qpeak (cms)= 6.009

PEAK FLOW (cms)= 3.720 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 22.907
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.421

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 82.0
NASHYD (0278)	485.49	5.00	# of Linear Res.(N)= 1.50	
ID= 1 DT=15.0 min	5.00	1.52		

Unit Hyd Qpeak (cms)= 5.453

PEAK FLOW (cms)= 3.831 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 23.080
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.424

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 79.0
NASHYD (0276)	90.89	5.00	# of Linear Res.(N)= 1.50	
ID= 1 DT=15.0 min	5.00	0.67		

Unit Hyd Qpeak (cms)= 2.302

PEAK FLOW (cms)= 1.207 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 20.490
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.377

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 75.0
NASHYD (0274)	392.49	5.00	# of Linear Res.(N)= 1.50	
ID= 1 DT=15.0 min	5.00	1.08		

Unit Hyd Qpeak (cms)= 6.182

PEAK FLOW (cms)= 3.104 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 18.038
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.332

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8360)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0274):	392.49	3.104	7.50	18.04
+ ID2= 2 (0276):	90.89	1.207	6.75	20.49
ID = 3 (8360):	483.38	4.235	7.25	18.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8358)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0278):	485.49	3.831	8.00	23.08
+ ID2= 2 (8360):	483.38	4.235	7.25	18.50
ID = 3 (8358):	968.87	7.908	7.75	20.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6280) |
 IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2801.0) ----->

Distance	Elevation	Manning
0.00	241.14	0.0500
13.32	240.80	0.0500
39.95	240.07	0.0500
96.54	236.21	0.0500
113.19	234.15	0.0500
123.18	232.35	0.0500
143.15	225.80	0.0500
149.81	225.62	0.0500
153.14	225.40	0.0500
157.30	224.76	0.0500 / 0.0300
159.80	224.26	0.0300
162.30	224.85	0.0300
162.55	224.97	0.0300 / 0.0500
167.80	225.05	0.0500
186.43	229.14	0.0500
213.06	234.75	0.0500
236.37	237.09	0.0500
266.33	237.31	0.0500
292.96	237.83	0.0500
329.58	241.50	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.50	224.76	.263E+04	0.8	0.65	58.31
1.36	225.62	.278E+05	13.4	1.10	34.54
2.22	226.48	.855E+05	52.6	1.40	27.09
3.09	227.35	.157E+06	120.4	1.74	21.78
3.95	228.21	.242E+06	216.3	2.04	18.64
4.81	229.07	.340E+06	341.8	2.29	16.56
5.67	229.93	.450E+06	498.3	2.52	15.06
6.53	230.79	.574E+06	688.3	2.73	13.90
7.40	231.66	.711E+06	913.8	2.93	12.97
8.26	232.52	.861E+06	1173.4	3.11	12.23
9.12	233.38	.103E+07	1461.3	3.24	11.72
9.98	234.24	.121E+07	1793.3	3.37	11.26
10.85	235.11	.142E+07	2141.0	3.45	11.02
11.71	235.97	.165E+07	2530.4	3.50	10.86
12.57	236.83	.192E+07	2954.0	3.51	10.81
13.43	237.69	.226E+07	3160.3	3.18	11.93
14.29	238.55	.271E+07	3869.4	3.26	11.67
15.16	239.42	.320E+07	4740.3	3.38	11.24
16.02	240.28	.373E+07	5688.4	3.48	10.92

<----- hydrograph -----> <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8358)	968.87	7.91	7.75	20.79	0.99	0.85
OUTFLOW: ID= 1 (6280)	968.87	7.48	8.50	20.79	0.95	0.83

ADD HYD (8354)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0260):	299.86	3.720	7.00	22.91
+ ID2= 2 (6280):	968.87	7.477	8.50	20.79
ID = 3 (8354):	1268.73	10.661	8.00	21.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0272)	Area (ha)	(ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	Curve Number (CN)	# of Linear Res.(N)
ID= 1 DT=15.0 min	157.38	5.00	1.507	9.50	12.62	75.0	1.50
U.H. Tp(hrs)=	1.09						

Unit Hyd Qpeak (cms)= 2.456

PEAK FLOW (cms)=	1.236 (i)
TIME TO PEAK (hrs)=	7.500
RUNOFF VOLUME (mm)=	18.040
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.332

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0270)	Area (ha)	(ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	Curve Number (CN)	# of Linear Res.(N)
ID= 1 DT=15.0 min	243.61	5.00	3.429	7.500	22.91	81.0	1.30
U.H. Tp(hrs)=	0.87						

Unit Hyd Qpeak (cms)= 3.429

PEAK FLOW (cms)=	2.117 (i)
TIME TO PEAK (hrs)=	7.500
RUNOFF VOLUME (mm)=	22.991
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.406

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0268)	Area (ha)	(ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	Curve Number (CN)	# of Linear Res.(N)
ID= 1 DT=15.0 min	215.76	5.00	3.807	7.000	17.875	75.0	1.30
U.H. Tp(hrs)=	0.69						

Unit Hyd Qpeak (cms)= 3.807

PEAK FLOW (cms)=	1.800 (i)
TIME TO PEAK (hrs)=	7.000
RUNOFF VOLUME (mm)=	17.875
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.329

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0264)	Area (ha)	(ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	Curve Number (CN)	# of Linear Res.(N)
ID= 1 DT=15.0 min	353.96	5.00	3.313	8.250	14.806	69.0	1.30
U.H. Tp(hrs)=	1.30						

Unit Hyd Qpeak (cms)= 3.313

PEAK FLOW (cms)=	1.477 (i)
TIME TO PEAK (hrs)=	8.250
RUNOFF VOLUME (mm)=	14.806
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.272

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0266)	Area (ha)	(ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	Curve Number (CN)	# of Linear Res.(N)
ID= 1 DT=15.0 min	508.09	5.00	3.786	9.500	12.617	64.0	1.30
U.H. Tp(hrs)=	1.63						

Unit Hyd Qpeak (cms)= 3.786

PEAK FLOW (cms)=	1.507 (i)
TIME TO PEAK (hrs)=	9.500
RUNOFF VOLUME (mm)=	12.617
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.232

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8380)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0264):	353.96	1.477	8.25	14.81
+ ID2= 2 (0266):	508.09	1.507	9.50	12.62
ID = 3 (8380):	862.05	2.967	8.75	13.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6268) |
 IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2681.0) ----->

Distance	Elevation	Manning	
0.00	274.03	0.0500	
10.55	272.73	0.0500	
21.10	271.28	0.0500	
30.12	266.83	0.0500	
37.04	260.36	0.0500	
42.32	260.06	0.0500	
49.95	259.93	0.0500	
101.00	256.87	0.0500	
103.00	256.58	0.0500 / 0.0350	Main Channel
104.00	256.32	0.0350	Main Channel
106.00	256.05	0.0350	Main Channel
108.00	256.25	0.0350	Main Channel
110.78	256.65	0.0350 / 0.0500	Main Channel
113.42	256.81	0.0500	
116.06	257.00	0.0500	
155.42	260.24	0.0500	
187.27	263.35	0.0500	
211.01	267.22	0.0500	
224.20	269.60	0.0500	
261.13	272.68	0.0500	

(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.45	234.34	.227E+04	0.7	0.89	52.83
0.98	234.87	.966E+04	4.6	1.49	35.20
1.50	235.39	.250E+05	13.7	1.73	30.41
2.03	235.92	.475E+05	25.5	1.95	26.87
2.56	236.45	.772E+05	52.5	2.14	24.50
3.09	236.98	.142E+06	82.4	1.83	28.73
3.61	237.50	.246E+06	158.1	2.03	25.91
4.14	238.03	.357E+06	267.0	2.31	22.73
4.67	238.56	.477E+06	392.1	2.59	20.27
5.20	239.09	.604E+06	547.8	2.86	18.38
5.72	239.61	.739E+06	729.2	3.11	16.90
6.25	240.14	.882E+06	936.5	3.34	15.70
6.78	240.67	.103E+07	1169.7	3.57	14.72
7.31	241.20	.119E+07	1425.9	3.77	13.94
7.83	241.72	.136E+07	1710.4	3.96	13.26
8.36	242.25	.154E+07	1967.9	4.02	13.07
8.89	242.78	.174E+07	2264.9	4.09	12.83
9.42	243.31	.196E+07	2589.3	4.15	12.65
9.94	243.83	.221E+07	2953.5	4.22	12.46

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.53	256.58	.697E+04	2.1	0.97	55.64
1.38	257.43	.477E+05	25.9	1.76	30.66
2.22	258.27	.123E+06	84.3	2.22	24.26
3.07	259.12	.231E+06	185.4	2.60	20.72
3.92	259.97	.371E+06	335.8	2.92	18.42
4.77	260.82	.560E+06	547.6	3.15	17.06
5.61	261.66	.790E+06	845.6	3.46	15.37
6.46	262.51	.106E+07	1225.0	3.75	14.37
7.31	263.36	.136E+07	1693.3	4.02	13.37
8.16	264.21	.169E+07	2283.2	4.35	12.36
9.00	265.05	.206E+07	2969.1	4.66	11.54
9.85	265.90	.245E+07	3754.4	4.96	10.86
10.70	266.75	.286E+07	4643.0	5.24	10.28
11.55	267.60	.331E+07	5636.1	5.50	9.78
12.39	268.44	.378E+07	6746.2	5.76	9.34
13.24	269.29	.428E+07	7972.2	6.01	8.96
14.09	270.14	.482E+07	9318.9	6.18	8.71
14.94	270.99	.539E+07	10850.5	6.32	8.52
15.78	271.83	.601E+07	12024.7	6.46	8.33

INFLOW	ID= 2 (8380)	862.05	2.97	8.75	13.52	0.56	0.98
OUTFLOW	ID= 1 (6268)	862.05	2.89	10.25	13.52	0.56	0.98

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8382)	1077.81	4.22	8.75	14.39	0.93
OUTFLOW : ID= 1 (6270)	1077.81	4.19	9.25	14.39	0.92

ADD HYD (8384)	1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0270)		243.61	2.117	7.50	22.09
+ ID2= 2 (6270)		1077.81	4.186	9.25	14.39
ID = 3 (8384)		1321.42	6.059	8.75	15.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	NASHYD (0262)	Area	(ha)	Curve Number	(CN)= 82.0
ID= 1 DT=15.0 min	Ia	(mm)	5.00	# of Linear Res.(N)=	1.30
	U.H. Tp(hrs)=	1.01			

Unit Hyd Qpeak (cms)= 4.094

PEAK FLOW (cms)=	2.732 (i)
TIME TO PEAK (hrs)=	8.000
RUNOFF VOLUME (mm)=	22.953
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.422

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD (0260)	Area	(ha)	Curve Number	(CN)= 82.0
ID= 1 DT=15.0 min	Ia	(mm)	5.00	# of Linear Res.(N)=	1.30
	U.H. Tp(hrs)=	1.33			

Unit Hyd Qpeak (cms)= 4.360

PEAK FLOW (cms)=	3.089 (i)
TIME TO PEAK (hrs)=	8.250
RUNOFF VOLUME (mm)=	23.028
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.423

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD (0258)	Area	(ha)	Curve Number	(CN)= 79.0
ID= 1 DT=15.0 min	Ia	(mm)	5.00	# of Linear Res.(N)=	1.30
	U.H. Tp(hrs)=	1.18			

Unit Hyd Qpeak (cms)= 1.881

PEAK FLOW (cms)=	1.165 (i)
TIME TO PEAK (hrs)=	8.000
RUNOFF VOLUME (mm)=	20.683
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.380

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Distance	Elevation	Manning	
0.00	245.98	0.0500	
8.14	245.66	0.0500	
16.28	245.16	0.0500	
20.35	244.84	0.0500	
38.66	242.98	0.0500	
48.83	240.65	0.0500	
63.07	235.91	0.0500	
65.11	235.18	0.0500	
68.43	234.34	0.0500 / 0.0300	Main Channel
68.68	233.95	0.0300	Main Channel
69.18	233.89	0.0300	Main Channel
69.68	233.95	0.0300	Main Channel
71.21	234.48	0.0300 / 0.0500	Main Channel
81.38	236.44	0.0500	
95.63	236.66	0.0500	
120.04	237.00	0.0500	
148.53	241.77	0.0500	
158.70	242.34	0.0500	
187.18	244.03	0.0500	
201.42	246.36	0.0500	

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
-------	------	--------	-----------	----------	-----------

CALIB

NASHYD (0252) Area (ha)= 319.99 Curve Number (CN)= 73.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.04

Unit Hyd Qpeak (cms)= 3.761

PEAK FLOW (cms)= 1.827 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 16.843
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.310

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0256) Area (ha)= 145.79 Curve Number (CN)= 67.0
 NASHYD (0256) Area (ha)= 145.79 Curve Number (CN)= 67.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.04

Unit Hyd Qpeak (cms)= 1.707

PEAK FLOW (cms)= 0.678 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 13.836
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.254

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0254) Area (ha)= 403.00 Curve Number (CN)= 55.0
 NASHYD (0254) Area (ha)= 403.00 Curve Number (CN)= 55.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.22

Unit Hyd Qpeak (cms)= 4.028

PEAK FLOW (cms)= 1.113 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 9.405
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.173

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6256) Routing time step (min)'= 15.00
 IN= 2--> OUT= 1

<----- DATA FOR SECTION (2561.0) ----->

Distance	Elevation	Manning	
0.00	276.07	0.0400	
11.68	273.71	0.0400	
23.36	271.35	0.0400	
29.19	270.30	0.0400	
35.03	269.44	0.0400	
55.47	267.90	0.0400	
78.82	266.24	0.0400	
90.50	265.63	0.0400	
102.18	265.40	0.0400	
105.10	264.95	0.0400 / 0.0350	Main Channel
108.02	264.39	0.0350	Main Channel
110.94	264.72	0.0350	Main Channel
113.86	265.19	0.0350 / 0.0400	Main Channel
116.78	265.49	0.0400	
143.05	268.24	0.0400	
172.25	270.53	0.0400	
207.28	271.95	0.0400	
233.55	273.72	0.0400	
256.91	274.98	0.0400	
289.02	275.97	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.56	264.95	.723E+04	1.9	0.90	62.51
1.14	265.53	.307E+05	13.5	1.49	37.92
1.72	266.11	.951E+05	48.6	1.73	32.62
2.30	266.69	.191E+06	117.8	2.09	27.03
2.88	267.27	.314E+06	225.2	2.43	23.24
3.46	267.85	.464E+06	375.5	2.74	20.59
4.04	268.43	.641E+06	573.2	3.03	18.62
4.62	269.01	.846E+06	821.9	3.29	17.15
5.20	269.59	.108E+07	1135.0	3.56	15.87
5.78	270.17	.134E+07	1525.1	3.86	14.64
6.36	270.75	.162E+07	1971.1	4.12	13.71
6.94	271.33	.194E+07	2474.0	4.33	13.04
7.52	271.91	.229E+07	3065.0	4.55	12.42
8.10	272.49	.266E+07	3773.4	4.80	11.75
8.68	273.07	.306E+07	4566.3	5.05	11.17
9.26	273.65	.348E+07	5443.7	5.30	10.66

9.84 274.23 .392E+07 6389.6 5.51 10.25
 10.42 274.81 .440E+07 7426.2 5.72 9.88
 11.00 275.39 .490E+07 8490.1 5.86 9.63

<----- hydrograph -----> <-pipe / channel->

AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0254) 403.00 1.11 8.25 9.41 0.32 0.90
 OUTFLOW: ID= 1 (6256) 403.00 1.05 10.00 9.41 0.31 0.90

ADD HYD (8370)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0256): 145.79 0.678 8.00 13.84
 + ID2= 2 (6256): 403.00 1.052 10.00 9.41
 ID = 3 (8370): 548.79 1.667 9.25 10.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0250) Area (ha)= 192.88 Curve Number (CN)= 70.0
 NASHYD (0250) Area (ha)= 192.88 Curve Number (CN)= 70.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.22

Unit Hyd Qpeak (cms)= 1.930

PEAK FLOW (cms)= 0.878 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 15.287
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.281

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0246) Area (ha)= 759.61 Curve Number (CN)= 55.0
 NASHYD (0246) Area (ha)= 759.61 Curve Number (CN)= 55.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.81

Unit Hyd Qpeak (cms)= 5.121

PEAK FLOW (cms)= 1.566 (i)
 TIME TO PEAK (hrs)= 10.000
 RUNOFF VOLUME (mm)= 9.438
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.173

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0248) Area (ha)= 146.04 Curve Number (CN)= 64.0
 NASHYD (0248) Area (ha)= 146.04 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 0.78

Unit Hyd Qpeak (cms)= 2.271

PEAK FLOW (cms)= 0.754 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 12.498
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.230

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8364)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0246): 759.61 1.566 10.00 9.44
 + ID2= 2 (0248): 146.04 0.754 7.25 12.50
 ID = 3 (8364): 905.65 2.217 8.25 9.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6250) Routing time step (min)'= 15.00
 IN= 2--> OUT= 1

<----- DATA FOR SECTION (2501.0) ----->

Distance Elevation Manning

0.00	269.96	0.0500
8.56	268.25	0.0500
17.13	266.91	0.0500
21.41	266.13	0.0500
40.68	263.15	0.0500
62.09	260.75	0.0500
85.64	258.02	0.0500
88.20	257.69	0.0500
93.20	257.05	0.0500 / 0.0350
93.45	256.88	0.0350
94.20	256.56	0.0350
94.95	256.83	0.0350
95.20	257.08	0.0350 / 0.0500
100.62	257.45	0.0500
115.61	258.57	0.0500
139.16	260.43	0.0500
152.01	261.95	0.0500
171.27	264.63	0.0500
188.40	267.90	0.0500
211.95	274.18	0.0500

----- DATA FOR SECTION (2581.0) -----

Distance	Elevation	Manning
0.00	252.88	0.0500
15.47	251.89	0.0500
46.41	248.45	0.0500
73.48	245.81	0.0500
88.95	243.75	0.0500
112.15	242.00	0.0500
135.35	240.23	0.0500
162.42	239.76	0.0500
170.97	239.52	0.0500 / 0.0350
171.58	239.03	0.0350
174.02	239.03	0.0350
176.46	239.03	0.0350
177.07	239.52	0.0350 / 0.0500
185.63	239.67	0.0500
208.83	239.87	0.0500
239.77	240.14	0.0500
274.57	244.93	0.0500
336.45	249.51	0.0500
363.52	249.77	0.0500
382.85	251.78	0.0500

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.49	257.05	.205E+04	0.4	0.68	89.65
1.17	257.73	.248E+05	5.7	0.85	72.16
1.85	258.41	.841E+05	25.2	1.10	55.60
2.53	259.09	.190E+06	66.5	1.36	45.14
3.21	259.77	.312E+06	135.7	1.60	38.36
3.89	260.45	.481E+06	238.3	1.82	33.61
4.57	261.13	.681E+06	383.5	2.07	29.60
5.25	261.81	.911E+06	568.9	2.29	26.70
5.93	262.49	.117E+07	800.5	2.51	24.36
6.61	263.17	.146E+07	1079.0	2.72	22.49
7.28	263.84	.177E+07	1413.4	2.94	20.85
7.96	264.52	.210E+07	1797.0	3.14	19.50
8.64	265.20	.246E+07	2241.6	3.35	18.28
9.32	265.88	.283E+07	2739.9	3.55	17.25
10.00	266.56	.323E+07	3295.7	3.75	16.34
10.68	267.24	.364E+07	3908.9	3.94	15.54
11.36	267.92	.408E+07	4578.5	4.12	14.84
12.04	268.60	.453E+07	5320.1	4.32	14.18
12.72	269.28	.499E+07	6109.1	4.50	13.61

----- hydrograph ----- <-pipe / channel-->

INFLOW : ID=	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
2 (8364)	905.65	2.22	8.25	9.93	0.72	0.73	
OUTFLOW : ID=	1 (6250)	905.65	2.06	10.50	9.93	0.70	0.73

ADD HYD (8366)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0250):	192.88	0.878	8.00	15.29
+ ID2= 2 (6250):	905.65	2.056	10.50	9.93
ID = 3 (8366):	1098.53	2.855	10.00	10.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8368)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8366):	1098.53	2.855	10.00	10.87
+ ID2= 2 (8370):	548.79	1.667	9.25	10.58
ID = 3 (8368):	1647.32	4.503	10.00	10.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8372)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0252):	319.99	1.827	8.00	16.84
+ ID2= 2 (8368):	1647.32	4.503	10.00	10.78
ID = 3 (8372):	1967.31	6.156	9.25	11.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6258) |
IN= 2--> OUT= 1 | Routing time step (min)= 15.00

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.49	239.52	.135E+05	2.5	0.92	91.32
1.14	240.17	.175E+06	25.6	0.73	114.22
1.78	240.81	.532E+06	123.5	1.17	71.85
2.43	241.46	.932E+06	285.3	1.54	54.48
3.07	242.10	.138E+07	506.5	1.85	45.25
3.72	242.75	.186E+07	786.8	2.12	39.42
4.36	243.39	.239E+07	1127.3	2.37	35.33
5.01	244.04	.296E+07	1535.8	2.61	32.12
5.65	244.68	.356E+07	2014.3	2.84	29.48
6.30	245.33	.420E+07	2535.9	3.03	27.60
6.94	245.97	.488E+07	3108.4	3.20	26.16
7.59	246.62	.561E+07	3740.6	3.35	24.98
8.23	247.26	.638E+07	4446.7	3.50	23.93
8.88	247.91	.721E+07	5229.7	3.64	22.98
9.52	248.55	.809E+07	6094.3	3.78	22.12
10.17	249.20	.901E+07	7050.4	3.93	21.31
10.81	249.84	.100E+08	7773.0	3.90	21.46
11.46	250.49	.111E+08	8998.8	4.07	20.56
12.10	251.13	.122E+08	10318.2	4.24	19.75

----- hydrograph ----- <-pipe / channel-->

INFLOW : ID=	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
2 (8372)	1967.31	6.16	9.25	11.76	0.59	0.88	
OUTFLOW : ID=	1 (6258)	1967.31	5.64	11.25	11.76	0.58	0.89

ADD HYD (8374)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0258):	181.99	1.165	8.00	20.68
+ ID2= 2 (6258):	1967.31	5.637	11.25	11.76
ID = 3 (8374):	2149.30	6.579	11.00	12.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8376)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0260):	476.24	3.089	8.25	23.03
+ ID2= 2 (8374):	2149.30	6.579	11.00	12.52
ID = 3 (8376):	2625.54	9.354	10.25	14.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8378)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0262):	341.31	2.732	8.00	22.95
+ ID2= 2 (8376):	2625.54	9.354	10.25	14.42
ID = 3 (8378):	2966.85	11.655	10.00	15.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.


```

ADD HYD (8362)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8378): 2966.85 11.655 10.00 15.41
+ ID2= 2 (8384): 1321.42 6.059 8.75 15.81
=====
ID = 3 (8362): 4288.27 17.537 9.50 15.53

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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ROUTE CHN (8272)
IN= 2--> OUT= 1
Routing time step (min)'= 15.00

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----- DATA FOR SECTION (2721.0) -----
Distance Elevation Manning
0.00 231.01 0.0450
23.01 223.65 0.0450
34.51 222.46 0.0450
51.77 222.13 0.0450
69.02 221.87 0.0450
161.06 221.92 0.0450
166.81 221.91 0.0450
172.56 221.89 0.0450
180.57 221.40 0.0450 /0.0300 Main Channel
181.57 220.60 0.0300 Main Channel
184.07 220.16 0.0300 Main Channel
195.57 221.85 0.0300 /0.0450 Main Channel
201.32 221.82 0.0450
207.07 221.77 0.0450
212.83 221.72 0.0450
253.09 222.52 0.0450
408.40 222.65 0.0450
460.17 223.20 0.0450
517.69 224.84 0.0450
569.46 232.57 0.0450

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----- TRAVEL TIME TABLE -----
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.41 220.57 .189E+04 0.5 0.49 60.18
0.83 220.99 .697E+04 3.2 0.82 35.79
1.24 221.40 .145E+05 8.8 1.07 27.45
1.84 222.00 .643E+05 29.3 0.81 36.61
2.44 222.60 .282E+06 132.9 0.83 35.41
3.04 223.20 .707E+06 413.7 1.04 28.50
3.64 223.80 .118E+07 876.8 1.31 22.46
4.24 224.40 .168E+07 1494.8 1.57 18.75
4.84 225.00 .221E+07 2264.1 1.82 16.23
5.44 225.60 .274E+07 3195.3 2.06 14.29
6.04 226.20 .328E+07 4258.6 2.30 12.84
6.65 226.81 .383E+07 5444.7 2.52 11.72
7.25 227.41 .438E+07 6749.7 2.73 10.82
7.85 228.01 .494E+07 8169.4 2.93 10.08
8.45 228.61 .551E+07 9700.7 3.12 9.46
9.05 229.21 .608E+07 11340.8 3.30 8.94
9.65 229.81 .666E+07 13087.4 3.48 8.48
10.25 230.41 .724E+07 14938.5 3.65 8.08
10.85 231.01 .783E+07 16892.4 3.82 7.73

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----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8362) 4288.27 17.54 9.50 15.53 1.50 0.94
OUTFLOW: ID= 1 (8272) 4288.27 17.32 10.25 15.53 1.49 0.94

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ADD HYD (8356)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0272): 157.38 1.236 7.50 18.04
+ ID2= 2 (8272): 4288.27 17.324 10.25 15.53
=====
ID = 3 (8356): 4445.65 18.249 10.00 15.62

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8352)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8354): 1266.73 10.681 8.00 21.29
+ ID2= 2 (8356): 4445.65 18.249 10.00 15.62
=====
ID = 3 (8352): 5714.38 27.725 9.00 16.88

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6304)
IN= 2--> OUT= 1
Routing time step (min)'= 15.00

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----- DATA FOR SECTION (3041.0) -----
Distance Elevation Manning
0.00 232.08 0.0500
19.00 231.87 0.0500
38.00 231.33 0.0500
66.51 230.44 0.0500
104.51 228.25 0.0500
118.76 225.17 0.0500
128.26 219.86 0.0500
175.77 219.17 0.0500
185.27 218.90 0.0500 /0.0300 Main Channel
185.52 218.65 0.0300 Main Channel
190.02 218.37 0.0300 Main Channel
194.52 218.60 0.0300 Main Channel
194.77 218.85 0.0300 /0.0500 Main Channel
204.27 219.60 0.0500
299.28 220.91 0.0500
327.78 222.36 0.0500
375.28 225.71 0.0500
403.79 229.37 0.0500
432.29 230.43 0.0500
470.29 232.00 0.0500

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----- TRAVEL TIME TABLE -----
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.48 218.85 .121E+05 1.2 0.36 173.94
1.17 219.54 .839E+05 9.8 0.44 142.60
1.86 220.23 .323E+06 41.0 0.47 131.43
2.56 220.93 .708E+06 109.5 0.58 107.67
3.25 221.62 .118E+07 224.3 0.74 87.36
3.94 222.31 .168E+07 376.5 0.84 74.48
4.63 223.00 .222E+07 567.9 0.96 65.26
5.32 223.69 .279E+07 794.7 1.07 58.58
6.02 224.39 .339E+07 1056.2 1.17 53.52
6.71 225.08 .402E+07 1352.4 1.26 49.52
7.40 225.77 .468E+07 1677.3 1.34 46.46
8.09 226.46 .536E+07 2050.0 1.43 43.58
8.79 227.16 .607E+07 2457.9 1.52 41.14
9.48 227.85 .680E+07 2901.0 1.60 39.05
10.17 228.54 .755E+07 3352.7 1.66 37.53
10.86 229.23 .834E+07 3812.5 1.71 36.48
11.55 229.92 .919E+07 4241.8 1.73 36.13
12.25 230.62 .101E+08 4697.0 1.74 35.93
12.94 231.31 .111E+08 5186.3 1.74 35.83

```

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8352) 5714.38 27.72 9.00 16.88 1.57 0.46
OUTFLOW: ID= 1 (6304) 5714.38 22.85 11.75 16.88 1.46 0.45

```

```

ADD HYD (8350)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0304): 292.37 0.753 10.50 12.26
+ ID2= 2 (6304): 5714.38 22.850 11.75 16.88
=====
ID = 3 (8350): 6006.75 23.587 11.75 16.65

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8346)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8348): 3768.46 17.080 9.75 15.28
+ ID2= 2 (8350): 6006.75 23.587 11.75 16.65
=====
ID = 3 (8346): 9775.21 39.706 10.75 16.12

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8344)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0306): 283.97 0.250 13.75 8.59
+ ID2= 2 (8346): 9775.21 39.706 10.75 16.12
=====
ID = 3 (8344): 10059.18 39.930 10.75 15.91

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5509)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	*****	370.0451
41.0590	56.7403	*****	863.4386
46.1390	86.3439	*****	*****
56.6340	*****	*****	*****
67.9600	*****	0.0000	0.0000

AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (8344) ***** 39.930 10.75 15.91
 OUTFLOW: ID= 1 (5509) ***** 30.768 14.50 15.91

PEAK FLOW REDUCTION [Qout/Qin](%)= 77.05
 TIME SHIFT OF PEAK FLOW (min)=225.00
 MAXIMUM STORAGE USED (ha.m.)= 42.5230

CALIB (0332)
 NASHYD (0332) Area (ha)= 393.44 Curve Number (CN)= 75.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 2.32

Unit Hyd Opeak (cms)= 2.894
 PEAK FLOW (cms)= 1.734 (i)
 TIME TO PEAK (hrs)= 10.000
 RUNOFF VOLUME (mm)= 18.150
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.334

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0330)
 NASHYD (0330) Area (ha)= 468.30 Curve Number (CN)= 80.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.09

Unit Hyd Opeak (cms)= 7.335
 PEAK FLOW (cms)= 4.424 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 21.421
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.394

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6332)
 IN= 2--> OUT= 1 Routing time step (min)= 15.00

<----- DATA FOR SECTION (3321.0) ----->

Distance	Elevation	Manning
0.00	234.00	0.0380
25.85	227.20	0.0380
96.94	226.44	0.0380
168.03	227.38	0.0380
219.73	225.62	0.0380
342.52	221.57	0.0380
368.37	221.42	0.0380
374.83	221.23	0.0380
379.79	220.98	0.0380 /0.0300 Main Channel
380.29	220.47	0.0300 Main Channel
381.29	220.47	0.0300 Main Channel
382.79	220.47	0.0300 Main Channel
383.29	220.98	0.0300 /0.0380 Main Channel
394.22	221.22	0.0380
400.68	221.33	0.0380
407.14	221.44	0.0380
491.16	225.70	0.0380
568.71	227.55	0.0380
607.49	230.14	0.0380
639.80	234.08	0.0380

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.51	220.98	.554E+04	0.9	0.56	107.27
1.20	221.67	.885E+05	12.1	0.49	122.32
1.88	222.35	.309E+06	64.7	0.76	79.70
2.57	223.04	.615E+06	165.4	0.97	61.99
3.25	223.72	.101E+07	323.1	1.16	52.06
3.94	224.41	.148E+07	542.9	1.33	45.51
4.62	225.09	.204E+07	835.4	1.48	40.77
5.31	225.78	.269E+07	1202.8	1.62	37.27

5.99	226.46	.344E+07	1618.0	1.70	35.46
6.68	227.15	.447E+07	1932.0	1.56	38.56
7.36	227.83	.580E+07	2763.9	1.72	34.96
8.05	228.52	.718E+07	3861.9	1.95	30.97
8.73	229.20	.859E+07	5115.4	2.16	27.98
9.42	229.89	1.00E+08	6519.2	2.35	25.64
10.10	230.57	.115E+08	8092.3	2.55	23.69
10.79	231.26	.130E+08	9821.5	2.73	22.05
11.47	231.94	.145E+08	11690.0	2.92	20.68
12.16	232.63	.160E+08	13694.9	3.09	19.52
12.84	233.31	.176E+08	15833.9	3.26	18.52

<---- hydrograph ----> <-pipe / channel->
 AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0330) 468.30 4.42 7.50 21.42 0.73 0.54
 OUTFLOW: ID= 1 (6332) 468.30 3.43 9.75 21.42 0.67 0.54

ADD HYD (8322)
 1 + 2 = 3
 ID= 1 (0332): 393.44 1.734 10.00 18.15
 + ID= 2 (6332): 468.30 3.427 9.75 21.42
 ID= 3 (8322): 861.74 5.160 9.75 19.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0328)
 NASHYD (0328) Area (ha)= 492.92 Curve Number (CN)= 77.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.91

Unit Hyd Opeak (cms)= 4.411
 PEAK FLOW (cms)= 2.703 (i)
 TIME TO PEAK (hrs)= 8.750
 RUNOFF VOLUME (mm)= 19.405
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.357

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0326)
 NASHYD (0326) Area (ha)= 678.91 Curve Number (CN)= 80.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.95

Unit Hyd Opeak (cms)= 5.941
 PEAK FLOW (cms)= 4.073 (i)
 TIME TO PEAK (hrs)= 9.000
 RUNOFF VOLUME (mm)= 21.534
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.396

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0324)
 NASHYD (0324) Area (ha)= 615.64 Curve Number (CN)= 79.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.90

Unit Hyd Opeak (cms)= 5.544
 PEAK FLOW (cms)= 3.645 (i)
 TIME TO PEAK (hrs)= 8.750
 RUNOFF VOLUME (mm)= 20.790
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.382

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0320)
 NASHYD (0320) Area (ha)= 278.74 Curve Number (CN)= 81.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.36

Unit Hyd Opeak (cms)= 3.499
 PEAK FLOW (cms)= 2.314 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 22.248

TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.409

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (3182) Area (ha)= 457.40 Curve Number (CN)= 78.0
 NASHED (3182) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.49

Unit Hyd Qpeak (cms)= 5.259

PEAK FLOW (cms)= 3.172 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 20.047
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.369

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (3181) Area (ha)= 26.00
 STANDHYD (3181) Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00
 ID= 1 DT=15.0 min

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 21.06 4.94
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 416.33 40.00
 Mannings n = 0.013 0.250

Max. Ref. Inten.(mm/hr)= 71.81 18.34
 over (min) 15.00 30.00
 Storage Coeff. (min)= 8.45 (ii) 29.53 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.04

TOTALS

PEAK FLOW (cms)= 3.71 0.17 3.819 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 52.40 19.48 46.14
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.36 0.85

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 77.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5318)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.3800	0.7654
0.0510	0.4423	4.1500	0.8231
0.8700	0.5378	4.8750	0.8778
1.9590	0.6105	5.2750	0.8878
2.5620	0.6796	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (3181)	26.000	3.819	6.00	46.14
OUTFLOW: ID= 1 (5318)	26.000	1.977	6.25	46.06

PEAK FLOW REDUCTION [Qout/Qin](%)= 51.76
 TIME SHIFT OF PEAK FLOW (min)= 15.00
 MAXIMUM STORAGE USED (ha.m.)= 0.6441

ADD HYD (8334)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (3182):	457.40	3.172	8.00	20.05
+ ID= 2 (5318):	26.00	1.977	6.25	46.06

ID = 3 (8334):	483.40	3.965	6.25	21.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6320)
 IN= 2--> OUT= 1

Routing time step (min)'= 15.00

----- DATA FOR SECTION (3201.0) -----

Distance	Elevation	Manning
0.00	249.00	0.0380
22.92	245.86	0.0380
45.83	244.87	0.0380
91.66	243.11	0.0380
126.03	239.53	0.0380
160.41	237.17	0.0380
166.14	237.06	0.0380
177.59	237.13	0.0380
183.32	237.20	0.0380
189.05	236.70	0.0380 / 0.0350
193.05	235.89	0.0350
197.05	236.64	0.0350 / 0.0380
200.51	236.74	0.0380
206.24	237.03	0.0380
246.34	238.82	0.0380
263.83	243.87	0.0380
389.56	247.64	0.0380
452.58	247.74	0.0380
498.41	248.60	0.0380
567.16	249.84	0.0380

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.37	236.26	.270E+04	0.4	0.58	107.96
0.75	236.64	.108E+05	2.6	0.92	68.01
1.48	237.37	.877E+05	27.4	1.17	53.35
2.20	238.09	.277E+06	122.1	1.65	37.83
2.93	238.82	.540E+06	299.7	2.08	30.01
3.66	239.55	.857E+06	587.8	2.57	24.29
4.39	240.28	.120E+07	977.8	3.04	20.52
5.11	241.00	.158E+07	1458.1	3.46	18.02
5.84	241.73	.198E+07	2028.6	3.84	16.23
6.57	242.46	.240E+07	2690.5	4.19	14.86
7.29	243.18	.285E+07	3431.8	4.50	13.84
8.02	243.91	.335E+07	4149.1	4.64	13.44
8.75	244.64	.393E+07	4746.6	4.51	13.80
9.47	245.36	.463E+07	5618.6	4.53	13.75
10.20	246.09	.545E+07	6781.8	4.66	13.39
10.93	246.82	.635E+07	8274.5	4.87	12.79
11.66	247.55	.733E+07	9988.0	5.10	12.23
12.38	248.27	.853E+07	11422.0	5.01	12.45
13.11	249.00	.989E+07	13749.7	5.20	11.98

----- hydrograph ----- <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8334)	483.40	3.97	6.25	21.45	0.79	0.93
OUTFLOW: ID= 1 (6320)	483.40	3.17	9.00	21.45	0.77	0.92

ADD HYD (8332)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (0320):	278.74	2.314	8.00	22.25
+ ID= 2 (6320):	483.40	3.174	9.00	21.45

ID = 3 (8332):	762.14	5.391	8.50	21.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0314) Area (ha)= 165.20 Curve Number (CN)= 78.0
 NASHED (0314) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.71

Unit Hyd Qpeak (cms)= 3.978

PEAK FLOW (cms)= 2.028 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 19.819
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.364

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6316)
 IN= 2--> OUT= 1

Routing time step (min)'= 15.00

----- DATA FOR SECTION (3161.0) -----

Distance	Elevation	Manning
0.00	248.54	0.0350
27.93	248.34	0.0350
41.89	246.61	0.0350
62.83	243.09	0.0350
132.65	239.00	0.0350
188.50	236.74	0.0350

195.48	236.54	0.0350	
202.46	236.32	0.0350	
205.48	236.14	0.0350 /0.0310	Main Channel
205.98	235.61	0.0310	Main Channel
207.98	235.25	0.0310	Main Channel
209.98	235.53	0.0310	Main Channel
210.48	236.00	0.0310 /0.0350	Main Channel
216.42	236.73	0.0350	
258.31	239.09	0.0350	
328.12	239.84	0.0350	
439.83	241.47	0.0350	
530.58	242.08	0.0350	
586.43	242.93	0.0350	
691.16	248.00	0.0350	

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.37	235.62	.215E+04	0.7	0.77	53.88
0.75	236.00	.634E+04	3.5	1.36	30.45
1.46	236.71	.1297E+05	19.3	1.62	25.66
2.16	237.41	.103E+06	75.8	1.84	22.60
2.87	238.12	.229E+06	202.8	2.21	18.79
3.57	238.82	.407E+06	423.3	2.59	16.03
4.28	239.53	.654E+06	727.7	2.77	14.97
4.99	240.24	.103E+07	1242.0	3.01	13.76
5.69	240.94	.151E+07	2002.3	3.31	12.54
6.40	241.65	.210E+07	2987.9	3.55	11.70
7.10	242.35	.286E+07	4302.8	3.75	11.07
7.81	243.06	.374E+07	6165.3	4.10	10.11
8.51	243.76	.468E+07	8642.4	4.60	9.03
9.22	244.47	.563E+07	11496.3	5.06	8.20
9.93	245.18	.666E+07	14714.1	5.50	7.54
10.63	245.88	.770E+07	18290.3	5.91	7.02
11.34	246.59	.877E+07	22221.9	6.30	6.58
12.04	247.29	.988E+07	26464.4	6.67	6.22
12.75	248.00	.110E+08	31061.3	7.01	5.91

<----- hydrograph -----> <-pipe / channel->

	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (0314)	165.20	2.03	6.75	19.82	0.56	0.98
OUTFLOW: ID= 1 (6316)	165.20	1.77	7.75	19.82	0.52	0.93

CALIB	NASHYD (0316)	Area	(ha)	Curve Number	(CN)= 82.0
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50		
	U.H. Tp(hrs)=	1.01			
Unit Hyd Qpeak	(cms)	3.923			
PEAK FLOW	(cms)	2.513 (l)			
TIME TO PEAK	(hrs)	7.250			
RUNOFF VOLUME	(mm)	22.974			
TOTAL RAINFALL	(mm)	54.400			
RUNOFF COEFFICIENT		0.422			

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8338)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID= 1 (0316):	232.34	2.513	7.25	22.97
+ ID= 2 (6316):	165.20	1.774	7.75	19.82
ID = 3 (8338):	397.54	4.262	7.50	21.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	NASHYD (0312)	Area	(ha)	Curve Number	(CN)= 80.0
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50		
	U.H. Tp(hrs)=	1.36			
Unit Hyd Qpeak	(cms)	4.529			
PEAK FLOW	(cms)	2.884 (l)			
TIME TO PEAK	(hrs)	8.000			
RUNOFF VOLUME	(mm)	21.475			
TOTAL RAINFALL	(mm)	54.400			
RUNOFF COEFFICIENT		0.395			

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

NASHYD (0308)	Area	(ha)	Curve Number	(CN)= 62.0
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50	
	U.H. Tp(hrs)=	1.62		
Unit Hyd Qpeak	(cms)	5.575		
PEAK FLOW	(cms)	1.987 (l)		
TIME TO PEAK	(hrs)	8.500		
RUNOFF VOLUME	(mm)	11.840		
TOTAL RAINFALL	(mm)	54.400		
RUNOFF COEFFICIENT		0.218		

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD (0310)	Area	(ha)	Curve Number	(CN)= 65.0
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50		
	U.H. Tp(hrs)=	0.76			
Unit Hyd Qpeak	(cms)	3.102			
PEAK FLOW	(cms)	1.019 (l)			
TIME TO PEAK	(hrs)	7.000			
RUNOFF VOLUME	(mm)	12.908			
TOTAL RAINFALL	(mm)	54.400			
RUNOFF COEFFICIENT		0.237			

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8342)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID= 1 (0308):	529.30	1.987	8.50	11.84
+ ID= 2 (0310):	138.28	1.019	7.00	12.91
ID = 3 (8342):	667.58	2.871	8.00	12.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6312)

IN= 2--> OUT= 1

Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3121.0) ----->

Distance	Elevation	Manning
0.00	265.94	0.0360
38.07	265.43	0.0360
59.82	263.98	0.0360
103.32	254.59	0.0360
157.70	252.16	0.0360
217.52	250.45	0.0360
233.84	247.69	0.0360
234.71	247.27	0.0360
239.71	246.38	0.0360
244.71	246.12	0.0360 /0.0330
259.71	246.67	0.0360
249.71	245.19	0.0330
251.71	245.64	0.0330 /0.0360
282.78	247.12	0.0360
315.41	251.60	0.0360
424.17	256.13	0.0360
478.55	257.04	0.0360
516.62	259.37	0.0360
538.37	266.00	0.0360

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.47	245.64	.111E+04	3.1	1.78	6.07
1.54	246.71	.103E+05	53.9	3.39	3.19
2.61	247.78	.414E+05	281.3	4.40	2.45
3.68	248.85	.839E+05	749.1	5.79	1.87
4.74	249.91	.135E+06	1463.2	6.97	1.55
5.81	250.98	.201E+06	2354.8	7.61	1.42
6.88	252.05	.295E+06	3533.0	7.76	1.39
7.95	253.12	.427E+06	5303.9	8.05	1.34
9.02	254.19	.594E+06	7856.5	8.58	1.26
10.09	255.26	.792E+06	11449.2	9.37	1.15
11.15	256.32	.101E+07	15862.4	10.15	1.06
12.22	257.39	.127E+07	21074.5	10.75	1.01
13.29	258.46	.155E+07	28184.1	11.78	0.92
14.36	259.53	.185E+07	36386.2	12.77	0.85
15.43	260.60	.215E+07	46110.6	13.89	0.78
16.50	261.67	.246E+07	56822.5	14.95	0.72
17.56	262.73	.278E+07	68498.1	15.97	0.68
18.63	263.80	.310E+07	81118.8	16.94	0.64
19.70	264.87	.344E+07	93603.2	17.66	0.61

```

<---- hydrograph ----> <-pipe / channel->
AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
(ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
INFLOW : ID= 2 (8342) 667.58  2.87  8.00  12.06  0.44  1.78
OUTFLOW : ID= 1 (6312) 667.58  2.87  8.00  12.06  0.44  1.78

```

```

| ADD HYD (8340) |
| 1 + 2 = 3 |
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0312): 359.44  2.884  8.00  21.47
+ ID2= 2 (6312): 667.58  2.869  8.00  12.06
=====
ID = 3 (8340): 1027.02  5.753  8.00  15.36

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ADD HYD (8336) |
| 1 + 2 = 3 |
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (8338): 397.54  4.262  7.50  21.66
+ ID2= 2 (8340): 1027.02  5.753  8.00  15.36
=====
ID = 3 (8336): 1424.56  9.969  7.75  17.12

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ADD HYD (8330) |
| 1 + 2 = 3 |
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (8332): 762.14  5.391  8.50  21.74
+ ID2= 2 (8336): 1424.56  9.969  7.75  17.12
=====
ID = 3 (8330): 2186.70  15.231  8.00  18.73

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ROUTE CHN (6324) |
| IN# 2--> OUT# 1 | Routing time step (min)= 15.00

```

----- DATA FOR SECTION (3241.0) -----

Distance	Elevation	Manning	
0.00	234.38	0.0360	
33.57	232.48	0.0360	
67.14	230.14	0.0360	
83.93	228.80	0.0360	
134.29	227.62	0.0360	
209.82	225.10	0.0360	
218.21	224.86	0.0360	
226.61	224.47	0.0360	
234.00	223.86	0.0360 / 0.0300	Main Channel
234.10	223.66	0.0300	Main Channel
235.00	223.66	0.0300	Main Channel
235.90	223.66	0.0300	Main Channel
236.00	223.86	0.0300 / 0.0360	Main Channel
243.39	224.92	0.0360	
251.78	224.89	0.0360	
335.71	225.64	0.0360	
562.32	226.53	0.0360	
637.85	228.36	0.0360	
705.00	229.80	0.0360	
830.89	234.00	0.0360	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.20	223.86	.145E+04	0.2	0.43	147.27
0.73	224.39	.159E+05	2.8	0.68	93.11
1.27	224.93	.561E+05	11.0	0.75	85.19
1.80	225.46	.217E+06	45.6	0.80	79.20
2.33	225.99	.566E+06	134.4	0.91	70.20
2.87	226.53	.122E+07	338.1	1.06	59.97
3.40	227.06	.206E+07	744.3	1.38	46.16
3.94	227.60	.298E+07	1295.9	1.66	38.38
4.47	228.13	.399E+07	1979.9	1.90	33.59
5.00	228.66	.509E+07	2807.5	2.11	30.21
5.54	229.20	.627E+07	3815.1	2.32	27.41
6.07	229.73	.753E+07	4984.1	2.53	25.17
6.60	230.26	.884E+07	6346.3	2.74	23.20
7.14	230.80	.102E+08	7861.2	2.95	21.61
7.67	231.33	.118E+08	9526.4	3.14	20.29
8.21	231.87	.131E+08	11342.2	3.32	19.18
8.74	232.40	.146E+08	13309.6	3.49	18.22
9.27	232.93	.161E+08	15414.0	3.66	17.41

```

9.81  233.47  .177E+08  17671.0  3.81  16.70
<---- hydrograph ----> <-pipe / channel->
AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
(ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
INFLOW : ID= 2 (8330) 2186.70  15.23  8.00  18.73  1.33  0.75
OUTFLOW : ID= 1 (6324) 2186.70  13.09  9.75  18.73  1.30  0.75

```

```

| ADD HYD (8328) |
| 1 + 2 = 3 |
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0324): 615.64  3.645  8.75  20.79
+ ID2= 2 (6324): 2186.70  13.089  9.75  18.73
=====
ID = 3 (8328): 2802.34  16.666  9.75  19.18

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| CALIB NASHYD (0322) |
| ID= 1 DT=15.0 min |
-----
Area (ha)= 513.13 Curve Number (CN)= 80.0
Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.75

```

Unit Hyd Qpeak (cms)= 5.020

```

PEAK FLOW (cms)= 3.366 (i)
TIME TO PEAK (hrs)= 8.500
RUNOFF VOLUME (mm)= 21.519
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.396

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

| ADD HYD (8326) |
| 1 + 2 = 3 |
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0322): 513.13  3.366  8.50  21.52
+ ID2= 2 (8328): 2802.34  16.666  9.75  19.18
=====
ID = 3 (8326): 3315.47  19.923  9.50  19.54

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ADD HYD (8324) |
| 1 + 2 = 3 |
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0326): 678.91  4.073  9.00  21.53
+ ID2= 2 (8326): 3315.47  19.923  9.50  19.54
=====
ID = 3 (8324): 3994.38  23.966  9.50  19.88

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ROUTE CHN (6328) |
| IN# 2--> OUT# 1 | Routing time step (min)= 15.00

```

----- DATA FOR SECTION (3281.0) -----

Distance	Elevation	Manning	
0.00	228.00	0.0380	
18.58	224.97	0.0380	
78.98	223.52	0.0380	
125.44	223.28	0.0380	
171.90	221.71	0.0380	
213.72	219.65	0.0380	
218.36	219.40	0.0380	
223.01	219.19	0.0380	
225.95	219.14	0.0380 / 0.0320	Main Channel
226.45	218.14	0.0320	Main Channel
236.95	218.14	0.0320	Main Channel
245.95	218.14	0.0320	Main Channel
245.95	219.14	0.0320 / 0.0380	Main Channel
246.24	219.16	0.0380	
250.88	219.24	0.0380	
255.53	219.39	0.0380	
325.22	221.47	0.0380	
367.03	223.14	0.0380	
404.20	225.17	0.0380	
459.95	228.04	0.0380	

----- TRAVEL TIME TABLE -----

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
-------	------	--------	-----------	----------	-----------

(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.50	218.64	.373E+05	3.2	0.33	192.37
1.00	219.14	.752E+05	10.1	0.51	124.71
1.52	219.66	.149E+06	22.2	0.57	113.06
2.04	220.18	.279E+06	43.4	0.59	107.06
2.56	220.70	.464E+06	77.0	0.63	100.52
3.08	221.22	.705E+06	125.7	0.68	93.56
3.61	221.75	.100E+07	192.7	0.73	86.62
4.13	222.27	.135E+07	279.1	0.79	80.92
4.65	222.79	.176E+07	385.9	0.84	75.86
5.17	223.31	.222E+07	514.9	0.89	71.82
5.69	223.83	.280E+07	662.7	0.90	70.40
6.21	224.35	.345E+07	867.7	0.96	66.35
6.73	224.87	.417E+07	1107.0	1.01	62.81
7.25	225.39	.494E+07	1400.0	1.08	58.80
7.78	225.92	.573E+07	1732.2	1.15	55.17
8.30	226.44	.655E+07	2099.9	1.22	52.02
8.82	226.96	.740E+07	2502.8	1.29	49.29
9.34	227.48	.828E+07	2940.9	1.36	46.90
9.86	228.00	.918E+07	3414.3	1.42	44.79

```

<---- hydrograph ----> <-pipe / channel->
      AREA   QPEAK  TPEAK  R.V.  MAX DEPTH  MAX VEL
      (ha)   (cms)  (hrs)  (mm)  (m)        (m/s)
INFLOW : ID= 2 (8324) 3994.38  23.97  9.50 19.88  1.57  0.57
OUTFLOW: ID= 1 (6328) 3994.38  20.57 11.25 19.88  1.45  0.56

```

```

ADD HYD (8320)
  1 + 2 = 3
  ID1= 1 (0328): 492.92  2.703  8.75 19.40
+ ID2= 2 (6328): 3994.38 20.574 11.25 19.88
=====
ID = 3 (8320): 4487.30 22.991 11.00 19.83

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8318)
  1 + 2 = 3
  ID1= 1 (8320): 4487.30 22.991 11.00 19.83
+ ID2= 2 (8322): 861.74  5.160  9.75 19.93
=====
ID = 3 (8318): 5349.04 27.883 11.00 19.84

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8316)
  1 + 2 = 3
  ID1= 1 (5509): 1009.18 30.768 14.50 15.51
+ ID2= 2 (8316): 5349.04 27.883 11.00 19.84
=====
ID = 3 (8316): 15408.22 54.401 12.50 17.28

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8312)
  1 + 2 = 3
  ID1= 1 (8314): 620.10  0.655  7.00 10.58
+ ID2= 2 (8316): 15408.22 54.401 12.50 17.28
=====
ID = 3 (8312): 16028.32 54.930 12.50 17.02

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8308)
  1 + 2 = 3
  ID1= 1 (8310): 598.90 17.031  6.25 23.52
+ ID2= 2 (8312): 16028.32 54.930 12.50 17.02
=====
ID = 3 (8308): 16627.22 55.984 12.25 17.25

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

RESERVOIR (5510)
IN= 2--> OUF= 1
DT= 15.0 min
      OUTFLOW  STORAGE  OUTFLOW  STORAGE
      (cms)   (ha.m.)  (cms)   (ha.m.)
66.5450  18.5023  *****  74.0090
98.5430  37.0045  *****  *****

```

```

      AREA  QPEAK  TPEAK  R.V.
      (ha)  (cms)  (hrs)  (mm)
INFLOW : ID= 2 (8308) ***** 55.984 12.25 17.25
OUTFLOW: ID= 1 (5510) ***** 55.092 13.25 17.25

```

```

PEAK FLOW REDUCTION [Qout/Qin](%) = 98.41
TIME SHIFT OF PEAK FLOW (min) = 60.00
MAXIMUM STORAGE USED (ha.m.) = 15.3262

```

```

ADD HYD (8240)
  1 + 2 = 3
  ID1= 1 (0336): 2785.00 1.997 22.25 16.45
+ ID2= 2 (5510): 16627.22 55.092 13.25 17.25
=====
ID = 3 (8240): 19412.22 56.803 13.25 17.14

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8238)
  1 + 2 = 3
  ID1= 1 (5508): 9524.23 24.490 12.25 10.18
+ ID2= 2 (8240): 19412.22 56.803 13.25 17.14
=====
ID = 3 (8238): 28936.45 80.945 12.75 14.85

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8236)
  1 + 2 = 3
  ID1= 1 (0342): 1004.58 2.057 10.25 9.16
+ ID2= 2 (8238): 28936.45 80.945 12.75 14.85
=====
ID = 3 (8236): 29941.03 82.805 12.75 14.66

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8234)
  1 + 2 = 3
  ID1= 1 (8232): 285.80  5.262  6.00 15.59
+ ID2= 2 (8236): 29941.03 82.805 12.75 14.66
=====
ID = 3 (8234): 30226.83 83.076 12.75 14.67

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8230)
  1 + 2 = 3
  ID1= 1 (0344): 344.00  1.873  7.00 10.67
+ ID2= 2 (8234): 30226.83 83.076 12.75 14.67
=====
ID = 3 (8230): 30570.83 83.630 12.75 14.62

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8228)
  1 + 2 = 3
  ID1= 1 (8226): 1952.06  6.251  8.75  8.85
+ ID2= 2 (8230): 30570.83 83.630 12.75 14.62
=====
ID = 3 (8228): 32522.89 87.695 12.50 14.27

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8238)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (5508): 9524.23 42.683 12.25 17.78
+ ID2= 2 (8240): 19412.22 86.298 13.50 28.47
=====
ID = 3 (8238): 28936.45 130.148 12.75 24.95

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8236)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0342): 1004.58 3.660 10.00 16.26
+ ID2= 2 (8238): 28936.45 130.148 12.75 24.95
=====
ID = 3 (8236): 29941.03 133.430 12.75 24.66

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8234)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8232): 285.80 8.040 6.00 24.37
+ ID2= 2 (8236): 29941.03 133.430 12.75 24.66
=====
ID = 3 (8234): 30226.83 133.766 12.75 24.65

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8230)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0344): 344.00 3.342 7.00 18.72
+ ID2= 2 (8234): 30226.83 133.766 12.75 24.65
=====
ID = 3 (8230): 30570.83 134.707 12.75 24.59

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8228)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8226): 1952.06 11.133 8.75 15.62
+ ID2= 2 (8230): 30570.83 134.707 12.75 24.59
=====
ID = 3 (8228): 32522.89 141.797 12.50 24.05

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8190)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0356): 1015.22 4.610 8.00 12.58
+ ID2= 2 (8228): 32522.89 141.797 12.50 24.05
=====
ID = 3 (8190): 33538.11 144.663 12.25 23.70

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

** SIMULATION NUMBER: 13 **

```

READ STORM
Filename: C:\Users\jascott\AppData
Local\Temp\
3e280798-92ee-4282-809c-79f5caed0add\fs74c8d3
Ptotal= 62.70 mm
Comments: 10-Year 12-Hour SCS II Design Storm

```

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.57	3.25	2.51	6.25	11.29	9.25	2.19
0.50	1.57	3.50	2.51	6.50	11.29	9.50	2.19
0.75	1.57	3.75	2.51	6.75	5.02	9.75	2.19
1.00	1.57	4.00	2.51	7.00	5.02	10.00	2.19
1.25	1.57	4.25	3.76	7.25	3.76	10.25	1.25
1.50	1.57	4.50	3.76	7.50	3.76	10.50	1.25

1.75	1.57	4.75	5.02	7.75	3.76	10.75	1.25
2.00	1.57	5.00	5.02	8.00	3.76	11.00	1.25
2.25	1.88	5.25	7.52	8.25	2.19	11.25	1.25
2.50	1.88	5.50	7.52	8.50	2.19	11.50	1.25
2.75	1.98	5.75	30.10	8.75	2.19	11.75	1.25
3.00	1.88	6.00	82.76	9.00	2.19	12.00	1.25

```

CALIB
NASHYD (0356) Area (ha)=1015.22 Curve Number (CN)= 46.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.37

```

Unit Hyd Qpeak (cms)= 12.651

```

PEAK FLOW (cms)= 3.392 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 9.295
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.148

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0354) Area (ha)= 262.68 Curve Number (CN)= 37.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.38

```

Unit Hyd Qpeak (cms)= 3.252

```

PEAK FLOW (cms)= 0.631 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 6.749
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.108

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0352) Area (ha)= 381.43 Curve Number (CN)= 54.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.82

```

Unit Hyd Qpeak (cms)= 7.980

```

PEAK FLOW (cms)= 2.453 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 11.980
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.191

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0346) Area (ha)= 350.93 Curve Number (CN)= 70.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.96

```

Unit Hyd Qpeak (cms)= 6.254

```

PEAK FLOW (cms)= 3.349 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 19.771
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.315

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0350) Area (ha)= 366.84 Curve Number (CN)= 48.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.07

```

Unit Hyd Qpeak (cms)= 5.831

```

PEAK FLOW (cms)= 1.564 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 9.910
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.158

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0348) Area (ha)= 590.18 Curve Number (CN)= 48.0
 NASHVD (0348) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.91

Unit Hyd Qpeak (cms)= 5.267
 PEAK FLOW (cms)= 1.632 (1)
 TIME TO PEAK (hrs)= 9.000
 RUNOFF VOLUME (mm)= 9.963
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.159

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6350) |
 IN= 2--> OUT= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (3501.0) ----->

Distance	Elevation	Manning
0.00	287.00	0.0500
23.66	283.72	0.0500
70.98	280.94	0.0500
112.38	280.23	0.0500
171.52	276.80	0.0500
260.24	274.46	0.0500
266.16	274.26	0.0500
272.07	274.12	0.0500
276.49	274.12	0.0500 / 0.0300 Main Channel
276.99	273.82	0.0300 Main Channel
277.99	273.82	0.0300 Main Channel
279.49	273.82	0.0300 Main Channel
280.49	274.27	0.0300 / 0.0500 Main Channel
283.90	274.27	0.0500
289.81	274.57	0.0500
325.30	275.75	0.0500
396.38	278.98	0.0500
449.51	280.97	0.0500
496.83	283.90	0.0500
585.54	287.92	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.30	274.12	273E+04	0.9	0.89	49.81
0.98	274.80	583E+05	20.8	1.05	46.71
1.66	275.48	203E+06	95.2	1.38	35.53
2.33	276.15	438E+06	256.0	1.73	28.50
3.01	276.83	755E+06	524.8	2.05	23.97
3.69	277.51	114E+07	935.8	2.43	20.26
4.37	278.19	157E+07	1472.0	2.76	17.82
5.05	278.87	206E+07	2140.7	3.06	16.06
5.72	279.54	261E+07	2938.6	3.33	14.79
6.40	280.22	321E+07	3887.6	3.57	13.77
7.08	280.90	390E+07	4795.4	3.63	13.56
7.76	281.58	468E+07	6196.2	3.91	12.58
8.43	282.25	550E+07	7803.9	4.19	11.74
9.11	282.93	637E+07	9601.9	4.45	11.05
9.79	283.61	728E+07	11594.8	4.70	10.46
10.47	284.29	823E+07	13845.3	4.96	9.91
11.15	284.97	922E+07	16293.4	5.21	9.43
11.82	285.64	103E+08	18952.0	5.45	9.02
12.50	286.32	113E+08	21824.3	5.69	8.65

<---- hydrograph ----> <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (0348)	590.18	1.63	9.00	9.96	0.32	0.99
OUTFLOW : ID= 1 (6350)	590.18	1.59	10.25	9.96	0.32	0.99

ADD HYD (8220) |
 1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (0350): 366.84 1.564 7.50 9.91
 + ID2= 2 (6350): 590.18 1.595 10.25 9.96
 ID = 3 (8220): 957.02 2.909 8.75 9.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8222) |
 1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (0346): 350.93 3.349 7.25 19.77
 + ID2= 2 (8220): 957.02 2.909 8.75 9.94

ID = 3 (8222): 1307.95 6.043 8.00 12.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6352) |
 IN= 2--> OUT= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (3521.0) ----->

Distance	Elevation	Manning
0.00	257.95	0.0500
7.83	257.34	0.0500
15.66	256.19	0.0500
21.53	254.01	0.0500
41.11	244.06	0.0500
76.35	241.38	0.0500
111.58	239.74	0.0500
113.54	239.61	0.0500
113.75	239.41	0.0500 / 0.0300 Main Channel
113.85	238.81	0.0300 Main Channel
115.50	238.81	0.0300 Main Channel
116.15	238.81	0.0300 Main Channel
116.25	239.41	0.0300 Main Channel
117.46	239.52	0.0500
119.41	239.72	0.0500
121.37	240.04	0.0500
131.16	241.84	0.0500
156.61	247.03	0.0500
176.19	251.46	0.0500
193.80	258.79	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.60	239.41	224E+04	2.2	1.52	17.12
1.58	240.39	216E+05	22.3	1.61	16.16
2.55	241.36	804E+05	101.0	1.96	13.27
3.53	242.34	173E+06	275.6	2.48	10.46
4.50	243.31	293E+06	553.9	2.95	8.80
5.48	244.29	438E+06	961.8	3.42	7.60
6.45	245.26	599E+06	1534.4	3.99	6.51
7.43	246.24	770E+06	2228.4	4.51	5.76
8.41	247.22	951E+06	3043.6	4.98	5.21
9.38	248.19	114E+07	3982.1	5.43	4.78
10.36	249.17	134E+07	5040.1	5.85	4.44
11.33	250.14	155E+07	6218.5	6.24	4.16
12.31	251.12	177E+07	7518.6	6.61	3.93
13.29	252.10	200E+07	8969.0	6.98	3.72
14.26	253.07	224E+07	10554.1	7.35	3.53
15.24	254.05	248E+07	12257.1	7.71	3.37
16.21	255.02	272E+07	14045.1	8.03	3.23
17.19	256.00	298E+07	15954.8	8.34	3.11
18.16	256.97	325E+07	17780.4	8.53	3.04

<---- hydrograph ----> <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (8222)	1307.95	6.04	8.00	12.58	0.79	1.53
OUTFLOW : ID= 1 (6352)	1307.95	6.00	8.25	12.58	0.78	1.53

ADD HYD (8224) |
 1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (0352): 381.43 2.453 7.00 11.98
 + ID2= 2 (6352): 1307.95 5.998 8.25 12.58
 ID = 3 (8224): 1689.38 8.227 8.00 12.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6354) |
 IN= 2--> OUT= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (3541.0) ----->

Distance	Elevation	Manning
0.00	253.92	0.0500
7.95	251.83	0.0500
15.89	249.97	0.0500
19.87	249.05	0.0500
47.68	242.00	0.0500
67.55	237.10	0.0500
83.44	231.11	0.0500
85.43	230.75	0.0500
85.66	230.61	0.0500 / 0.0300 Main Channel
85.76	230.01	0.0300 Main Channel
87.41	230.01	0.0300 Main Channel
88.06	230.01	0.0300 Main Channel
88.16	230.61	0.0300 / 0.0500 Main Channel

89.40	230.72	0.0500
91.39	230.88	0.0500
93.37	231.03	0.0500
133.11	233.96	0.0500
150.39	235.39	0.0500
172.84	239.03	0.0500
196.68	250.00	0.0500

-----> TRAVEL TIME TABLE <-----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.60	230.61	.451E+04	1.5	1.06	49.40
1.62	231.63	.394E+05	14.6	1.16	45.13
2.64	232.65	.128E+06	58.8	1.44	36.30
3.66	233.67	.269E+06	149.7	1.74	30.00
4.68	234.69	.463E+06	301.5	2.04	25.58
5.70	235.71	.708E+06	530.5	2.36	22.14
6.72	236.73	.980E+06	848.4	2.71	19.25
7.74	237.75	.128E+07	1237.9	3.02	17.29
8.76	238.77	.162E+07	1705.7	3.30	15.83
9.78	239.79	.199E+07	2291.9	3.61	14.45
10.81	240.82	.237E+07	2973.7	3.92	13.30
11.83	241.84	.278E+07	3742.2	4.21	12.38
12.85	242.86	.321E+07	4599.1	4.49	11.62
13.87	243.88	.365E+07	5544.6	4.75	10.98
14.89	244.90	.412E+07	6579.7	5.00	10.44
15.91	245.92	.461E+07	7705.9	5.23	9.97
16.93	246.94	.512E+07	8924.6	5.46	9.55
17.95	247.96	.564E+07	10237.5	5.68	9.19
18.97	248.98	.619E+07	11646.2	5.89	8.86

<---- hydrograph ----> <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8224) 1689.38	8.23	8.00	12.44	1.12	1.11
OUTFLOW: ID= 1 (6354) 1689.38	7.67	8.75	12.44	1.08	1.10

ADD HYD (8226)

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0354):	262.68	0.631	8.00	6.75
+ ID2= 2 (6354):	1689.38	7.666	8.75	12.44
ID= 3 (8226):	1952.06	8.283	8.75	11.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (3402)

Area (ha)	(ha)= 344.00	Curve Number (CN)= 59.0
Ia (mm)	= 5.00	# of Linear Res.(N)= 1.50
U.H. Tp(hrs)	= 0.87	

Unit Hyd Qpeak (cms)= 6.790

PEAK FLOW (cms)	= 2.485 (i)
TIME TO PEAK (hrs)	= 7.000
RUNOFF VOLUME (mm)	= 14.035
TOTAL RAINFALL (mm)	= 62.700
RUNOFF COEFFICIENT	= 0.224

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (3402)

Area (ha)	(ha)= 234.60	Dir. Conn.(%)= 13.00
Total Imp(%)	= 28.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	65.69	168.91
Dep. Storage (mm)	5.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1250.60	40.00
Mannings n	0.013	0.250

Max.Eff.Inten.(mm/hr)=	82.76	12.93
over (min)	15.00	45.00
Storage Coeff. (min)=	15.44 (ii)	39.69 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.07	0.03
PEAK FLOW (cms)=	5.04	3.27
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	60.70	11.98
TOTAL RAINFALL (mm)=	62.70	62.70
RUNOFF COEFFICIENT	0.97	0.19

TOTALS

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (3401)	Area (ha)= 51.20	Dir. Conn.(%)= 21.00
ID= 1 DT=15.0 min	Total Imp(%)= 46.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	23.55	27.65
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	584.24	40.00
Mannings n	0.013	0.250

Max.Eff.Inten.(mm/hr)=	82.76	18.77
over (min)	15.00	45.00
Storage Coeff. (min)=	9.78 (ii)	30.67 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.09	0.03

PEAK FLOW (cms)=	2.10	0.89
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	60.70	14.21
TOTAL RAINFALL (mm)=	62.70	62.70
RUNOFF COEFFICIENT	0.97	0.23

TOTALS

(iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5340)

IN=	OUT=	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
DT= 15.0 min	1	0.0000	0.0000	5.3360	1.7877
		0.1730	0.6348	7.2780	2.2185
		1.8360	0.8250	9.1880	2.6486
		2.1540	1.1051	9.5880	2.6586
		3.3950	1.3506	0.0000	0.0000

INFLOW : ID=	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 2 (3401)	51.200	2.465	6.00	23.97
OUTFLOW: ID= 1 (5340)	51.200	0.624	7.00	23.95

PEAK FLOW REDUCTION [Qout/Qin](%)=	25.34
TIME SHIFT OF PEAK FLOW (min)=	60.00
MAXIMUM STORAGE USED (ha.m.)=	0.6899

ADD HYD (8232)

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (3402):	234.60	6.356	6.00	18.31
+ ID2= 2 (5340):	51.20	0.624	7.00	23.95
ID= 3 (8232):	285.80	6.442	6.00	19.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0342)

Area (ha)	(ha)=1004.58	Curve Number (CN)= 54.0
Ia (mm)	= 5.00	# of Linear Res.(N)= 1.50
U.H. Tp(hrs)	= 2.57	

Unit Hyd Qpeak (cms)= 6.686

PEAK FLOW (cms)=	2.724 (i)
TIME TO PEAK (hrs)=	10.250
RUNOFF VOLUME (mm)=	12.117
TOTAL RAINFALL (mm)=	62.700
RUNOFF COEFFICIENT	= 0.193

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0422)

Area (ha)	(ha)= 780.20	Curve Number (CN)= 54.0
-----------	--------------	-------------------------

|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.95

Unit Hyd Qpeak (cms)= 6.838

PEAK FLOW (cms)= 2.591 (i)
TIME TO PEAK (hrs)= 9.250
RUNOFF VOLUME (mm)= 12.102
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.193

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHVD (0410) | Area (ha)= 572.01 Curve Number (CN)= 48.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.46

Unit Hyd Qpeak (cms)= 6.698

PEAK FLOW (cms)= 1.949 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 9.944
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.159

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHVD (0408) | Area (ha)= 231.62 Curve Number (CN)= 58.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.64

Unit Hyd Qpeak (cms)= 6.198

PEAK FLOW (cms)= 2.048 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 13.505
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.215

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHVD (0376) | Area (ha)= 463.85 Curve Number (CN)= 74.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 7.380

PEAK FLOW (cms)= 4.625 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 22.449
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.358

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHVD (0374) | Area (ha)= 545.70 Curve Number (CN)= 61.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.51

Unit Hyd Qpeak (cms)= 6.158

PEAK FLOW (cms)= 2.759 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 15.043
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.240

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHVD (0372) | Area (ha)= 110.42 Curve Number (CN)= 37.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 1.954

PEAK FLOW (cms)= 0.343 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 6.719
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.107

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHVD (0370) | Area (ha)= 191.85 Curve Number (CN)= 63.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.67

Unit Hyd Qpeak (cms)= 4.860

PEAK FLOW (cms)= 1.913 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 15.799
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.252

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHVD (0369) | Area (ha)= 159.48 Curve Number (CN)= 46.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.12

Unit Hyd Qpeak (cms)= 2.433

PEAK FLOW (cms)= 0.617 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 9.274
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.148

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHVD (0366) | Area (ha)= 462.62 Curve Number (CN)= 62.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.06

Unit Hyd Qpeak (cms)= 7.451

PEAK FLOW (cms)= 3.151 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 15.457
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.247

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHVD (0364) | Area (ha)= 155.27 Curve Number (CN)= 55.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.70

Unit Hyd Qpeak (cms)= 3.782

PEAK FLOW (cms)= 1.155 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 12.322
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.197

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| ADD HYD (8302) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
ID1= 1 (0364): 155.27 1.155 6.75 12.32
+ ID2= 2 (0366): 462.62 3.151 7.50 15.46
=====

ID = 3 (8302): 617.89 4.258 7.25 14.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| CALIB |
| STANSHVD (0362) | Area (ha)= 118.78
|ID= 1 DT=15.0 min | Total Imp(%)= 22.00 Dir. Conn.(%)= 8.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 26.13 92.65
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 889.87 40.00

Mannings n = 0.013 0.250
 Max.Eff.Inten.(mm/hr)= 82.76 11.12
 over (min) 15.00 45.00
 Storage Coeff. (min)= 12.59 (ii) 38.34 (iii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03

TOTALS
 PEAK FLOW (cms)= 1.71 1.58 2.335 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 60.70 10.62 14.63
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.17 0.23

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0358) Area (ha)= 429.87 Curve Number (CN)= 35.0
 NASHYD (0358) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.03

Unit Hyd Qpeak (cms)= 7.091

PEAK FLOW (cms)= 1.174 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 6.228
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.099

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0360) Area (ha)= 138.37 Curve Number (CN)= 46.0
 NASHYD (0360) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.60

Unit Hyd Qpeak (cms)= 3.957

PEAK FLOW (cms)= 0.860 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 9.151
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.146

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8306) AREA QPEAK TPEAK R.V.
 1 + 2 = 3 (ha) (cms) (hrs) (mm)
 ID= 1 (0358): 429.87 1.174 7.50 6.23
 + ID2= 2 (0360): 138.37 0.860 6.75 9.15
 ID = 3 (8306): 568.24 1.984 7.00 6.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6362) Routing time step (min)= 15.00
 IN= 2--> OUT= 1

----- DATA FOR SECTION (3621.0) -----
 Distance Elevation Manning
 0.00 261.46 0.0550
 27.86 254.23 0.0550
 51.07 251.96 0.0550
 74.29 250.77 0.0550
 97.50 249.91 0.0550
 125.36 249.40 0.0550
 150.93 247.40 0.0550
 155.93 247.33 0.0550 /0.0350 Main Channel
 157.93 246.85 0.0350 Main Channel
 159.18 246.65 0.0350 Main Channel
 160.18 246.63 0.0350 Main Channel
 160.93 246.85 0.0350 Main Channel
 161.93 247.18 0.0350 /0.0550 Main Channel
 163.18 248.03 0.0550
 168.18 248.58 0.0550
 183.18 250.18 0.0550
 201.97 252.59 0.0550

213.57 256.02 0.0550
 225.18 260.31 0.0550
 229.82 261.00 0.0550

----- TRAVEL TIME TABLE -----
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
 (m) (m) (cu.m.) (cms) (m/s) (min)
 0.55 247.18 .556E+04 1.9 1.03 49.95
 1.28 247.91 .338E+05 16.7 1.53 23.61
 2.00 248.63 .913E+05 53.6 1.81 28.40
 2.73 249.36 .185E+06 122.3 2.05 25.18
 3.46 250.09 .344E+06 224.6 2.02 25.52
 4.19 250.82 .574E+06 417.9 2.25 22.91
 4.91 251.54 .855E+06 699.1 2.53 20.38
 5.64 252.27 .118E+07 1075.3 2.82 18.26
 6.37 253.00 .153E+07 1559.9 3.15 16.38
 7.10 253.73 .191E+07 2137.6 3.46 14.89
 7.82 254.45 .231E+07 2815.2 3.77 13.67
 8.55 255.18 .272E+07 3608.5 4.10 12.58
 9.28 255.91 .315E+07 4488.2 4.41 11.69
 10.01 256.64 .388E+07 5458.0 4.71 10.95
 10.73 257.36 .403E+07 6512.4 4.99 10.32
 11.46 258.09 .449E+07 7649.8 5.27 9.78
 12.19 258.82 .495E+07 8869.5 5.53 9.32
 12.92 259.55 .544E+07 10171.2 5.78 8.91
 13.64 260.27 .593E+07 11554.8 6.02 8.55

----- hydrograph ----- <-pipe / channel-->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8306) 568.24 1.98 7.00 6.94 0.56 1.03
 OUTFLOW: ID= 1 (6362) 568.24 1.75 6.25 6.94 0.52 1.03

ADD HYD (8304) AREA QPEAK TPEAK R.V.
 1 + 2 = 3 (ha) (cms) (hrs) (mm)
 ID1= 1 (0362): 118.78 2.335 6.00 14.63
 + ID2= 2 (6362): 568.24 1.750 8.25 6.94
 ID = 3 (8304): 687.02 2.882 6.50 8.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8300) AREA QPEAK TPEAK R.V.
 1 + 2 = 3 (ha) (cms) (hrs) (mm)
 ID1= 1 (8302): 617.89 4.258 7.25 14.67
 + ID2= 2 (8304): 687.02 2.882 6.50 8.27
 ID = 3 (8300): 1304.91 6.844 7.00 11.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6368) Routing time step (min)= 15.00
 IN= 2--> OUT= 1

----- DATA FOR SECTION (3681.0) -----
 Distance Elevation Manning
 0.00 230.00 0.0370
 18.48 223.26 0.0370
 36.96 223.05 0.0370
 64.67 222.94 0.0370
 110.87 222.86 0.0370
 133.96 222.74 0.0370
 147.82 222.65 0.0370
 170.92 222.31 0.0370
 174.79 222.26 0.0370 /0.0300 Main Channel
 174.89 221.86 0.0300 Main Channel
 175.54 221.86 0.0300 Main Channel
 176.19 221.86 0.0300 Main Channel
 176.29 222.26 0.0300 /0.0370 Main Channel
 180.16 222.25 0.0370
 184.78 222.28 0.0370
 189.40 222.31 0.0370
 332.60 222.37 0.0370
 450.00 230.00 0.0370

----- TRAVEL TIME TABLE -----
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
 (m) (m) (cu.m.) (cms) (m/s) (min)
 0.40 222.26 .771E+03 0.3 0.52 41.74
 0.81 222.67 .784E+05 30.3 0.51 43.17
 1.21 223.07 .210E+06 123.2 0.77 28.42
 1.62 223.48 .384E+06 301.0 1.03 21.25
 2.03 223.89 .563E+06 550.2 1.28 17.06
 2.44 224.30 .747E+06 860.7 1.51 14.46

2.84	224.70	.934E+06	1228.1	1.73	12.68
3.25	225.11	.113E+07	1649.3	1.92	11.37
3.66	225.52	.132E+07	2122.3	2.11	10.37
4.07	225.93	.152E+07	2645.5	2.29	9.58
4.47	226.33	.172E+07	3217.8	2.45	8.92
4.88	226.74	.193E+07	3838.3	2.61	8.38
5.29	227.15	.214E+07	4506.3	2.76	7.92
5.70	227.56	.236E+07	5221.3	2.91	7.52
6.10	227.96	.258E+07	5993.0	3.05	7.17
6.51	228.37	.280E+07	6790.9	3.19	6.87
6.92	228.78	.303E+07	7645.0	3.32	6.60
7.33	229.19	.326E+07	8545.1	3.45	6.35
7.73	229.59	.349E+07	9491.0	3.57	6.13

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8300) 1304.91 6.84 7.00 11.30 0.49 0.52
OUTFLOW : ID= 1 (6368) 1304.91 6.29 8.25 11.30 0.48 0.52

```

```

ADD HYD (8298)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0368): 159.48 0.617 7.75 9.27
+ ID2= 2 (6368): 1304.91 6.294 8.25 11.30
=====
ID = 3 (8298): 1464.39 6.900 8.00 11.08

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8296)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0370): 191.85 1.913 6.75 15.80
+ ID2= 2 (8298): 1464.39 6.900 8.00 11.08
=====
ID = 3 (8296): 1656.24 8.479 7.75 11.63

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6372)
IN= 2----> OUT= 1 1
Routing time step (min)= 15.00

```

----- DATA FOR SECTION (3721.0) -----

Distance	Elevation	Manning
0.00	225.00	0.0390
30.80	219.38	0.0390
61.61	219.30	0.0390
77.01	219.27	0.0390
469.76	219.14	0.0390
477.46	219.13	0.0390
485.16	219.10	0.0390
492.86	219.09	0.0390
495.56	219.09	0.0390 / 0.0310 Main Channel
495.66	218.51	0.0310 Main Channel
500.56	218.51	0.0310 Main Channel
505.46	218.51	0.0310 Main Channel
505.56	219.09	0.0310 / 0.0390 Main Channel
508.26	219.09	0.0390
515.96	219.10	0.0390
523.67	219.21	0.0390
562.17	219.32	0.0390
654.58	219.43	0.0390
731.59	219.46	0.0390
762.39	225.00	0.0390

----- TRAVEL TIME TABLE -----

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.29	218.80	.431E+04	1.7	0.58	43.48
0.58	219.09	.867E+04	5.1	0.89	28.32
0.93	219.44	.200E+06	58.2	0.44	57.35
1.28	219.79	.568E+06	278.6	0.74	33.99
1.62	220.13	.939E+06	630.2	1.01	24.84
1.97	220.48	.131E+07	1090.4	1.25	20.05
2.32	220.83	.169E+07	1647.7	1.47	17.06
2.67	221.18	.208E+07	2294.6	1.68	14.99
3.01	221.52	.244E+07	3025.6	1.87	13.46
3.36	221.87	.282E+07	3836.5	2.05	12.27
3.71	222.22	.321E+07	4723.7	2.22	11.31
4.06	222.57	.359E+07	5684.6	2.39	10.53
4.40	222.91	.398E+07	6716.6	2.55	9.87
4.75	223.26	.437E+07	7817.7	2.70	9.31
5.10	223.61	.476E+07	8986.2	2.85	8.83
5.45	223.96	.515E+07	10220.4	2.99	8.40
5.79	224.30	.555E+07	11519.1	3.13	8.03

6.14	224.65	.594E+07	12880.8	3.27	7.69
6.49	225.00	.634E+07	14304.5	3.40	7.39

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8296) 1656.24 8.48 7.75 11.63 0.60 0.83
OUTFLOW : ID= 1 (6372) 1656.24 8.21 8.50 11.63 0.60 0.84

```

```

ADD HYD (8294)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0372): 110.42 0.343 7.25 6.72
+ ID2= 2 (6372): 1656.24 8.215 8.50 11.63
=====
ID = 3 (8294): 1766.66 8.529 8.50 11.32

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8292)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0374): 545.70 2.759 8.25 15.04
+ ID2= 2 (8294): 1766.66 8.529 8.50 11.32
=====
ID = 3 (8292): 2312.36 11.277 8.50 12.20

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

RESERVOIR (5505)
IN= 2----> OUT= 1
DT= 15.0 min
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
0.0000 0.0000 | 65.1290 345.3754
25.4850 24.6697 | 84.9510 456.3890
31.1490 98.6787 | ***** 838.7689
39.6440 ***** | ***** 838.7789
48.1390 ***** | 0.0000 0.0000

```

```

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (8292) 2312.360 11.277 8.50 12.20
OUTFLOW : ID= 1 (5505) 2312.360 8.244 11.25 12.20

```

PEAK FLOW REDUCTION [Qout/Qin]= 73.11
TIME SHIFT OF PEAK FLOW (min)=165.00
MAXIMUM STORAGE USED (ha.m.)= 7.9877

```

ADD HYD (8272)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0376): 463.85 4.625 7.50 22.45
+ ID2= 2 (5505): 2312.36 8.244 11.25 12.20
=====
ID = 3 (8272): 2776.21 11.221 10.25 13.91

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB (0396) Area (ha)= 305.21 Curve Number (CN)= 69.0
NASHYD (0396) Area (ha)= 305.21 Curve Number (CN)= 69.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.08

```

Unit Hyd Qpeak (cms)= 4.811
PEAK FLOW (cms)= 2.564 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 19.201
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.306

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB (0394) Area (ha)= 325.45 Curve Number (CN)= 53.0
NASHYD (0394) Area (ha)= 325.45 Curve Number (CN)= 53.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.92

```

Unit Hyd Qpeak (cms)= 6.013

```

PEAK FLOW      (cms)= 1.838 (1)
TIME TO PEAK   (hrs)= 7.250
RUNOFF VOLUME  (mm)= 11.632
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.186

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB
NASHYD (0390) | Area (ha)= 420.00 Curve Number (CN)= 55.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.07
-----

```

Unit Hyd Qpeak (cms)= 6.683

```

PEAK FLOW      (cms)= 2.260 (1)
TIME TO PEAK   (hrs)= 7.500
RUNOFF VOLUME  (mm)= 12.424
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.198

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB
NASHYD (0388) | Area (ha)= 220.77 Curve Number (CN)= 58.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.99
-----

```

Unit Hyd Qpeak (cms)= 3.819

```

PEAK FLOW      (cms)= 1.394 (1)
TIME TO PEAK   (hrs)= 7.250
RUNOFF VOLUME  (mm)= 13.635
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.217

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB
NASHYD (0386) | Area (ha)= 241.27 Curve Number (CN)= 61.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.90
-----

```

Unit Hyd Qpeak (cms)= 4.562

```

PEAK FLOW      (cms)= 1.798 (1)
TIME TO PEAK   (hrs)= 7.250
RUNOFF VOLUME  (mm)= 14.947
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.238

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
ADD HYD (8286) | AREA QPEAK TPEAK R.V.
1 + 2 = 3 | (ha) (cms) (hrs) (mm)
ID1= 1 (0386): 241.27 1.798 7.25 14.95
+ ID2= 2 (0388): 220.77 1.394 7.25 13.64
=====
ID = 3 (8286): 462.04 3.192 7.25 14.32

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
CALIB
NASHYD (0384) | Area (ha)= 199.07 Curve Number (CN)= 44.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.96
-----

```

Unit Hyd Qpeak (cms)= 3.537

```

PEAK FLOW      (cms)= 0.803 (1)
TIME TO PEAK   (hrs)= 7.250
RUNOFF VOLUME  (mm)= 8.644
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.138

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB
NASHYD (0380) | Area (ha)= 182.01 Curve Number (CN)= 40.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
-----

```

----- U.H. Tp(hrs)= 0.55

Unit Hyd Qpeak (cms)= 5.609

```

PEAK FLOW      (cms)= 0.964 (1)
TIME TO PEAK   (hrs)= 6.500
RUNOFF VOLUME  (mm)= 7.404
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.118

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB
NASHYD (0382) | Area (ha)= 216.59 Curve Number (CN)= 53.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.64
-----

```

Unit Hyd Qpeak (cms)= 5.733

```

PEAK FLOW      (cms)= 1.611 (1)
TIME TO PEAK   (hrs)= 6.750
RUNOFF VOLUME  (mm)= 11.537
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.184

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
ADD HYD (8290) | AREA QPEAK TPEAK R.V.
1 + 2 = 3 | (ha) (cms) (hrs) (mm)
ID1= 1 (0380): 182.01 0.964 6.50 7.40
+ ID2= 2 (0382): 216.59 1.611 6.75 11.54
=====
ID = 3 (8290): 398.60 2.571 6.75 9.65

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ROUTE CHN (6384) |
IN= 2 ---> OUT= 1 Routing time step (min) = 15.00
-----

```

```

----- DATA FOR SECTION (3841.0) -----
Distance Elevation Manning
0.00 294.40 0.0380
10.59 291.93 0.0380
21.17 289.19 0.0380
26.46 287.99 0.0380
31.76 286.79 0.0380
37.05 285.59 0.0380
42.34 284.39 0.0380
47.63 283.19 0.0380
52.92 281.99 0.0380
58.21 280.79 0.0380
63.50 279.59 0.0380
68.79 278.39 0.0380
74.08 277.19 0.0380
79.37 275.99 0.0380
84.66 274.79 0.0380
89.95 273.59 0.0380
95.24 272.39 0.0380
100.53 271.19 0.0380
105.82 269.99 0.0380
111.11 268.79 0.0380
116.40 267.59 0.0380
121.69 266.39 0.0380
126.98 265.19 0.0380
132.27 263.99 0.0380
137.56 262.79 0.0380
142.85 261.59 0.0380
148.14 260.39 0.0380
153.43 259.19 0.0380
158.72 257.99 0.0380
164.01 256.79 0.0380
169.30 255.59 0.0380
174.59 254.39 0.0380
179.88 253.19 0.0380
185.17 251.99 0.0380
190.46 250.79 0.0380
195.75 249.59 0.0380
201.04 248.39 0.0380
206.33 247.19 0.0380
211.62 245.99 0.0380
216.91 244.79 0.0380
222.20 243.59 0.0380

```

```

----- TRAVEL TIME TABLE -----
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.34 279.64 .715E+03 0.2 0.85 63.42
1.10 280.40 .507E+05 21.9 1.40 38.54
1.85 281.15 .141E+06 91.8 2.10 25.69
2.61 281.91 .289E+06 218.9 2.63 20.46
3.36 282.66 .445E+06 407.1 2.96 18.23
4.12 283.42 .681E+06 698.3 3.31 16.26
4.87 284.17 .977E+06 1114.8 3.68 14.65
5.63 284.93 .133E+07 1674.3 4.06 13.07
6.39 285.69 .173E+07 2457.8 4.59 11.73
7.14 286.44 .215E+07 3378.8 5.07 10.63
7.90 287.20 .261E+07 4418.4 5.47 9.85
8.65 287.95 .310E+07 5676.3 5.92 9.11
9.41 288.71 .361E+07 7150.5 6.40 8.42
10.17 289.47 .414E+07 8783.1 6.86 7.86
10.92 290.22 .469E+07 10575.1 7.30 7.38
11.68 290.98 .525E+07 12520.4 7.72 6.98
12.43 291.73 .582E+07 14619.0 8.12 6.64
13.19 292.49 .642E+07 16883.2 8.50 6.34
13.94 293.24 .703E+07 19259.8 8.86 6.08

```

```

----- hydrograph ----- <-pipe / channel>
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL

```

INFLOW : ID= 2 (8290) 398.60 2.57 6.75 9.65 0.42 0.89
 OUTFLOW : ID= 1 (6384) 398.60 2.01 8.00 9.65 0.40 0.88

ADD HYD (8288)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0384): 199.07 0.803 7.25 8.64
 + ID2= 2 (6384): 398.60 2.015 8.00 9.65
 ID = 3 (8288): 597.67 2.797 8.00 9.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8284)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8286): 462.04 3.192 7.25 14.32
 + ID2= 2 (8288): 597.67 2.797 8.00 9.31
 ID = 3 (8284): 1059.71 5.920 7.75 11.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8280)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0390): 420.00 2.260 7.50 12.42
 + ID2= 2 (8284): 1059.71 5.920 7.75 11.50
 ID = 3 (8280): 1479.71 8.177 7.50 11.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0392) Area (ha)= 167.22 Curve Number (CN)= 62.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.74
 Unit Hyd Qpeak (cms)= 3.837
 PEAK FLOW (cms)= 1.497 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 15.356
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.245

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0378) Area (ha)= 606.72 Curve Number (CN)= 55.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.18

Unit Hyd Qpeak (cms)= 8.771
 PEAK FLOW (cms)= 3.046 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 12.439
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.198

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8282)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0378): 606.72 3.046 8.00 12.44
 + ID2= 2 (0392): 167.22 1.497 7.00 15.36
 ID = 3 (8282): 773.94 4.443 7.25 13.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8278)

1 + 2 = 3 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8280): 1479.71 8.177 7.50 11.76
 + ID2= 2 (8282): 773.94 4.443 7.25 13.07
 ID = 3 (8278): 2253.65 12.619 7.50 12.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6394)
 ID= 2 -> OUT= 1 Routing time step (min)'= 15.00

DATA FOR SECTION (3941.0)

Distance	Elevation	Manning
0.00	283.00	0.0380
13.48	282.87	0.0380
53.92	280.08	0.0380
74.13	276.62	0.0380
97.72	265.45	0.0380
114.57	256.93	0.0380
131.42	253.04	0.0380
134.79	252.58	0.0380
138.53	251.74	0.0380 /0.0300 Main Channel
139.03	251.20	0.0300 Main Channel
141.53	251.20	0.0300 Main Channel
144.03	251.20	0.0300 Main Channel
144.53	251.74	0.0300 /0.0380 Main Channel
148.27	252.69	0.0380
151.64	252.97	0.0380
185.34	255.08	0.0380
219.03	257.54	0.0380
262.84	259.43	0.0380
310.02	262.80	0.0380
333.60	283.00	0.0380

TRAVEL TIME TABLE

DEPTH (m)	SECT (cu.m.)	VOLUME (cms)	FLOW RATE (m/s)	VELOCITY (min)	TRAV. TIME (min)
0.54	251.74	.827E+04	4.5	1.51	30.71
2.19	253.39	.758E+05	73.2	2.69	17.27
3.83	255.03	.282E+06	331.8	3.27	14.17
5.48	256.68	.633E+06	917.3	4.03	11.51
7.12	258.32	.112E+07	1889.5	4.69	9.90
8.77	259.97	.179E+07	3410.2	5.21	8.73
10.41	261.61	.259E+07	5927.6	6.05	7.66
12.06	263.26	.351E+07	8605.1	6.83	6.79
13.70	264.90	.447E+07	12544.9	7.81	5.94
15.35	266.55	.546E+07	17103.4	8.71	5.32
16.99	268.19	.648E+07	22253.7	9.56	4.85
18.64	269.84	.752E+07	27979.0	10.36	4.48
20.28	271.48	.858E+07	34269.0	11.11	4.17
21.93	273.13	.967E+07	41110.6	11.83	3.92
23.57	274.77	.108E+08	48495.9	12.51	3.71
25.22	276.42	.119E+08	56418.1	13.17	3.52
26.86	278.06	.131E+08	64410.6	13.69	3.39
28.51	279.71	.143E+08	73003.1	14.18	3.27
30.15	281.35	.156E+08	81379.3	14.50	3.20

hydrograph

INFLOW : ID= 2 (8278)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
2253.65	12.62	7.50	12.21	0.73	1.59	
OUTFLOW : ID= 1 (6394)	2253.65	12.25	8.25	12.21	0.73	1.59

ADD HYD (8276)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0394): 325.45 1.838 7.25 11.63
 + ID2= 2 (6394): 2253.65 12.251 8.25 12.21
 ID = 3 (8276): 2579.10 13.996 8.00 12.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6396)
 ID= 2 -> OUT= 1 Routing time step (min)'= 15.00

DATA FOR SECTION (3961.0)

Distance	Elevation	Manning
0.00	263.00	0.0410
11.75	257.14	0.0410
23.50	253.97	0.0410
41.13	247.83	0.0410
146.38	232.09	0.0410
135.13	229.07	0.0410
149.82	228.97	0.0410
152.75	228.96	0.0410 /0.0300 Main Channel

154.19	228.73	0.0300	Main Channel
154.69	228.20	0.0300	Main Channel
155.69	228.20	0.0300	Main Channel
156.69	228.20	0.0300	Main Channel
157.19	228.73	0.0300	Main Channel
159.63	228.95	0.0300 / 0.0410	Main Channel
161.57	228.96	0.0410	
164.51	229.71	0.0410	
196.82	241.70	0.0410	
223.26	249.21	0.0410	
246.76	255.13	0.0410	
290.82	263.51	0.0410	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (cu.m.)	VOLUME (cms)	FLOW RATE (m/s)	VELOCITY (min)	TRAV.TIME (m/s)
0.75	228.95	.780E+04	2.2	0.98	58.06
2.54	230.74	.277E+06	145.1	1.78	31.76
4.33	232.53	.786E+06	609.1	2.63	21.50
6.13	234.33	.140E+07	1481.5	3.59	15.77
7.92	236.12	.207E+07	2677.3	4.39	12.90
9.71	237.91	.280E+07	4184.8	5.09	11.14
11.50	239.70	.357E+07	6001.4	5.71	9.93
13.29	241.49	.441E+07	8123.1	6.27	9.03
15.09	243.29	.530E+07	10549.4	6.77	8.37
16.88	245.08	.625E+07	13300.6	7.24	7.83
18.67	246.87	.726E+07	16396.2	7.68	7.38
20.46	248.66	.834E+07	19809.8	8.07	7.02
22.26	250.46	.949E+07	23532.5	8.43	6.72
24.05	252.25	.107E+08	27641.2	8.77	6.46
25.84	254.04	.120E+08	32155.2	9.10	6.22
27.63	255.83	.134E+08	36901.0	9.37	6.05
29.42	257.62	.149E+08	42147.7	9.64	5.88
31.22	259.42	.164E+08	48149.1	9.97	5.68
33.01	261.21	.181E+08	54644.8	10.29	5.51

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8276)	2579.10	14.00	8.00	12.14	0.90
OUTFLOW : ID= 1 (6396)	2579.10	12.65	9.25	12.14	0.88

ADD HYD (8274)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0396):	305.21	2.564	7.50	19.20
+ ID2= 2 (6396):	2579.10	12.651	9.25	12.14
ID = 3 (8274):	2884.31	14.864	9.00	12.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8270)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8272):	2776.21	11.221	10.25	13.91
+ ID2= 2 (8274):	2884.31	14.864	9.00	12.88
ID = 3 (8270):	5660.52	25.641	9.25	13.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5506)	DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	60.8820	135.6832		
31.1490	24.6597	96.2770	900.4531		
36.8120	37.0045	96.6770	900.4531		
45.3070	86.3439	0.0000	0.0000		

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8270)	5660.520	25.641	9.25
OUTFLOW : ID= 1 (5506)	5660.520	21.225	11.75

PEAK FLOW REDUCTION [Qout/Qin](%) = 82.78
 TIME SHFT OF PEAK FLOW (min)=150.00
 MAXIMUM STORAGE USED (ha.m.)= 16.8111

CALIB	NASHVD (0406)	Area (ha)	(ha)	Curve Number (CN)	(CN)
ID= 1 DT=15.0 min		142.65	5.00	66.0	1.50
U.H. Tp(hrs)=		0.59			

Unit Hyd Qpeak (cms)= 4.135

PEAK FLOW (cms)= 1.735 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 17.263
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.275

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHVD (0404)	Area (ha)	(ha)	Curve Number (CN)	(CN)
ID= 1 DT=15.0 min		246.46	5.00	47.0	1.50
U.H. Tp(hrs)=		0.98			

Unit Hyd Qpeak (cms)= 4.280

PEAK FLOW (cms)= 1.084 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 9.573
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.153

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHVD (0402)	Area (ha)	(ha)	Curve Number (CN)	(CN)
ID= 1 DT=15.0 min		244.00	5.00	61.0	1.50
U.H. Tp(hrs)=		1.07			

Unit Hyd Qpeak (cms)= 3.879

PEAK FLOW (cms)= 1.593 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 14.988
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.239

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHVD (0400)	Area (ha)	(ha)	Curve Number (CN)	(CN)
ID= 1 DT=15.0 min		93.97	5.00	52.0	1.50
U.H. Tp(hrs)=		0.44			

Unit Hyd Qpeak (cms)= 3.630

PEAK FLOW (cms)= 0.902 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 11.008
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.176

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHVD (0398)	Area (ha)	(ha)	Curve Number (CN)	(CN)
ID= 1 DT=15.0 min		328.19	5.00	55.0	1.50
U.H. Tp(hrs)=		0.83			

Unit Hyd Qpeak (cms)= 6.759

PEAK FLOW (cms)= 2.154 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 12.370
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.197

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8268)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0398):	328.19	2.154	7.00	12.37
+ ID2= 2 (0400):	93.97	0.902	6.50	11.01
ID = 3 (8268):	422.16	2.966	6.75	12.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6402)

| IN= 2--> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (4021.0) -----					
Distance	Elevation	Manning			
0.00	238.50	0.0360			
11.50	238.00	0.0360			
23.00	237.93	0.0360			
34.49	236.39	0.0360			
63.24	233.98	0.0360			
97.73	228.15	0.0360			
123.60	227.08	0.0360			
126.48	226.61	0.0360			
127.60	226.47	0.0360 / 0.0330	Main Channel		
127.85	225.25	0.0330	Main Channel		
129.35	225.25	0.0330	Main Channel		
130.85	225.25	0.0330	Main Channel		
131.10	226.47	0.0330 / 0.0360	Main Channel		
132.22	226.59	0.0360			
143.72	227.42	0.0360			
169.59	227.88	0.0360			
192.59	231.19	0.0360			
218.46	233.02	0.0360			
241.45	235.50	0.0360			
284.57	236.43	0.0360			

----- TRAVEL TIME TABLE -----					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.41	225.66	.391E+04	1.7	1.36	38.31
0.81	226.06	.804E+04	5.0	1.93	27.02
1.22	226.47	.124E+05	9.1	2.30	22.61
1.84	227.09	.308E+05	23.3	2.36	22.03
2.46	227.71	.904E+05	62.0	2.14	24.31
3.09	228.34	.223E+06	188.3	2.63	19.75
3.71	228.96	.379E+06	410.2	3.38	15.39
4.33	229.58	.550E+06	711.5	4.04	12.89
4.95	230.20	.737E+06	1091.3	4.62	11.25
5.58	230.83	.939E+06	1550.6	5.15	10.10
6.20	231.45	.115E+07	2072.4	5.59	9.31
6.82	232.07	.140E+07	2659.2	5.93	8.77
7.44	232.69	.167E+07	3345.8	6.27	8.30
8.07	233.32	.195E+07	4159.0	6.64	7.83
8.69	233.94	.226E+07	5100.8	7.03	7.39
9.31	234.56	.259E+07	6081.3	7.32	7.11
9.93	235.18	.295E+07	7183.1	7.60	6.84
10.56	235.81	.334E+07	8145.2	7.62	6.83
11.18	236.43	.379E+07	9116.0	7.51	6.92

		<--- hydrograph --->				<- pipe / channel ->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (8268)	422.16	2.97	6.75	12.07	0.56	1.53	
OUTFLOW : ID= 1 (6402)	422.16	2.70	7.50	12.07	0.53	1.49	

ADD HYD (8266)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0402):	244.00	1.593	7.50	14.99
+ ID2= 2 (6402):	422.16	2.701	7.50	12.07
=====				
ID = 3 (8266):	666.16	4.294	7.50	13.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8264)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0404):	246.46	1.084	7.50	9.57
+ ID2= 2 (8264):	666.16	4.294	7.50	13.14
=====				
ID = 3 (8264):	912.62	5.378	7.50	12.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8262)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0406):	142.65	1.735	6.75	17.26
+ ID2= 2 (8264):	912.62	5.378	7.50	12.17
=====				
ID = 3 (8262):	1055.27	6.905	7.25	12.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8260)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (5506):	5660.52	21.225	11.75	13.39
+ ID2= 2 (8262):	1055.27	6.905	7.25	12.86
=====				
ID = 3 (8260):	6715.79	24.324	11.25	13.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8258)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0408):	231.62	2.048	6.75	13.51
+ ID2= 2 (8260):	6715.79	24.324	11.25	13.31
=====				
ID = 3 (8258):	6947.41	24.951	11.00	13.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8256)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0410):	572.01	1.949	8.25	9.94
+ ID2= 2 (8258):	6947.41	24.951	11.00	13.31
=====				
ID = 3 (8256):	7519.42	26.526	11.00	13.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5507)					
IN= 2--> OUT= 1					
	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)	R.V. (mm)
DT= 15.0 min	0.0000	0.0000	90.6140	67.8416	
	39.6440	12.3348	*****	160.3529	
	48.1390	18.5023	*****	160.3529	
	67.9600	37.0045	0.0000	0.0000	

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8256)	7519.421	26.526	11.00	13.06
OUTFLOW : ID= 1 (5507)	7519.421	25.968	11.75	13.06

PEAK FLOW REDUCTION [Qout/Qin](%)= 97.89
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 8.0848

CALIB (0420)			
NASHYD	Area (ha)	Curve Number (CN)	# of Linear Res.(N)
ID= 1 DT=15.0 min	175.82	53.0	1.50
	Ia (mm)= 5.00		
	U.H. Tp(hrs)= 0.81		

Unit Hyd Opeak (cms)= 3.692

PEAK FLOW (cms)= 1.097 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 11.603
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.185

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0418)			
NASHYD	Area (ha)	Curve Number (CN)	# of Linear Res.(N)
ID= 1 DT=15.0 min	182.79	53.0	1.50
	Ia (mm)= 5.00		
	U.H. Tp(hrs)= 1.05		

Unit Hyd Opeak (cms)= 2.966

PEAK FLOW (cms)= 1.335 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 16.442
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.262

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB			
-------	--	--	--

NASHYD (0416) Area (ha)= 439.30 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.29

Unit Hyd Qpeak (cms)= 5.832
 PEAK FLOW (cms)= 2.770 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 16.482
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.263

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0412) Area (ha)= 238.70 Curve Number (CN)= 54.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.80

Unit Hyd Qpeak (cms)= 5.088
 PEAK FLOW (cms)= 1.557 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 11.975
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.191

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (4141) Area (ha)= 43.70
 ID= 1 DT=15.0 min Total Imp(%)= 36.00 Dir. Conn.(%)= 17.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)	15.73	27.97	
Dep. Storage (mm)	2.00	5.00	
Average Slope (%)	0.50	0.50	
Length (m)	539.75	40.00	
Mannings n	0.013	0.250	
Max.Eff.Inten.(mm/hr)	82.76	24.71	
over (min)	15.00	30.00	
Storage Coeff. (min)	9.33 (ii)	28.04 (ii)	
Unit Hyd. Tpeak (min)	15.00	30.00	
Unit Hyd. peak (cms)	0.09	0.04	
		TOTALS	
PEAK FLOW (cms)	1.47	1.34	2.298 (iii)
TIME TO PEAK (hrs)	6.00	6.25	6.00
RUNOFF VOLUME (mm)	60.70	20.49	27.32
TOTAL RAINFALL (mm)	62.70	62.70	62.70
RUNOFF COEFFICIENT	0.97	0.33	0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 64.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5414)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.4720	1.9177
0.3820	0.3152	3.8720	1.9277
1.8280	0.4690	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (4141)	43.700	2.298	6.00	27.32
OUTFLOW: ID= 1 (5414)	43.700	1.248	6.50	27.32

PEAK FLOW REDUCTION [Qout/Qin](%)= 54.31
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 0.4093

CALIB
 STANDHYD (4142) Area (ha)= 144.30
 ID= 1 DT=15.0 min Total Imp(%)= 21.00 Dir. Conn.(%)= 10.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	30.30	114.00
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50

Length (m)= 980.82 40.00
 Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 82.76 19.43
 over (min) 15.00 45.00
 Storage Coeff. (min)= 13.35 (ii) 33.95 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03

TOTALS

	(cms)	(hrs)	(mm)
PEAK FLOW	2.53	3.58	4.256 (iii)
TIME TO PEAK	6.00	6.50	6.50
RUNOFF VOLUME	60.70	18.51	22.73
TOTAL RAINFALL	62.70	62.70	62.70
RUNOFF COEFFICIENT	0.97	0.30	0.36

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 64.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8254)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (4142):	144.30	4.256	6.50	22.73
+ ID2= 2 (5414):	43.70	1.248	6.50	27.32

ID = 3 (8254):	188.00	5.504	6.50	23.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8252)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0412):	238.70	1.557	7.00	11.98
+ ID2= 2 (8254):	188.00	5.504	6.50	23.79

ID = 3 (8252):	426.70	6.962	6.50	17.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6416) | Routing time step (min)'= 15.00

----- DATA FOR SECTION (4161.0) ----->

Distance	Elevation	Manning
0.00	270.07	0.0340
20.67	267.91	0.0340
62.01	264.33	0.0340
113.69	259.75	0.0340
165.37	253.30	0.0340
227.38	246.29	0.0340
232.55	246.03	0.0340
237.72	246.16	0.0340
241.39	246.02	0.0340 /0.0300
241.64	245.75	0.0300
242.89	245.75	0.0300
244.14	245.75	0.0300
244.39	246.02	0.0300 /0.0340
248.06	246.20	0.0340
253.22	246.28	0.0340
258.39	246.63	0.0340
346.25	252.57	0.0340
413.43	257.77	0.0340
465.11	261.78	0.0340
511.62	270.00	0.0340

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.27	246.02	.295E+04	0.6	0.75	88.26
1.53	247.28	.168E+06	69.7	1.65	40.13
2.79	248.54	.490E+06	299.5	2.42	27.28
4.06	249.81	.962E+06	740.1	3.05	21.67
5.32	251.07	.158E+07	1440.0	3.61	18.33
6.58	252.33	.235E+07	2443.8	4.12	16.06
7.84	253.59	.327E+07	3811.2	4.63	14.30
9.10	254.85	.432E+07	5565.1	5.11	12.94
10.37	256.12	.550E+07	7721.3	5.57	11.88
11.63	257.38	.682E+07	10309.9	6.00	11.02
12.89	258.64	.827E+07	13360.6	6.42	10.31
14.15	259.90	.985E+07	16884.5	6.81	9.72
15.42	261.17	.116E+08	20810.4	7.14	9.26

16.68	262.43	.134E+08	25510.4	7.53	8.78
17.94	263.69	.154E+08	30958.0	7.97	8.30
19.20	264.95	.175E+08	36988.3	8.38	7.89
20.46	266.21	.197E+08	43617.1	8.78	7.53
21.73	267.48	.220E+08	50865.1	9.17	7.22
22.99	268.74	.244E+08	58854.0	9.56	6.92

		<--- hydrograph --->				<-pipe / channel->	
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (8252)	426.70	6.96	6.50	17.18	0.39	0.79	
OUTFLOW: ID= 1 (6416)	426.70	3.67	7.50	17.18	0.33	0.77	

ADD HYD (8250)		AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0416):	439.30	2.770	8.00	16.48	
+ ID2= 2 (6416):	426.70	3.675	7.50	17.18	
ID = 3 (8250):	866.00	6.400	7.50	16.83	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8248)		AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0418):	382.79	1.335	7.50	16.44	
+ ID2= 2 (8250):	866.00	6.400	7.50	16.83	
ID = 3 (8248):	1048.79	7.735	7.50	16.76	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8246)		AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0420):	175.82	1.097	7.00	11.60	
+ ID2= 2 (8248):	1048.79	7.735	7.50	16.76	
ID = 3 (8246):	1224.61	8.801	7.50	16.02	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8244)		AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (5507):	7519.42	25.968	11.75	13.06	
+ ID2= 2 (8246):	1224.61	8.801	7.50	16.02	
ID = 3 (8244):	8744.03	30.680	11.00	13.47	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8242)		AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0422):	780.20	2.591	9.25	12.30	
+ ID2= 2 (8244):	8744.03	30.680	11.00	13.47	
ID = 3 (8242):	9524.23	33.097	11.00	13.36	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5508)		OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1	DT= 15.0 min	(cms)	(ha.m.)	(cms)	(ha.m.)
		0.0000	0.0000	*****	197.3574
		76.4550	30.8371	*****	394.7148
		*****	61.6742	*****	394.7248
		*****	*****	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8242)	9524.231	33.097	11.00	13.36
OUTFLOW: ID= 1 (5508)	9524.231	32.116	12.25	13.36

PEAK FLOW REDUCTION [Qout/Qin]= 97.04
 TIME SHIFT OF PEAK FLOW (min)= 75.00
 MAXIMUM STORAGE USED (ha.m.)= 12.9573

CALIB	Area	(ha)	Curve Number	(CN)= 72.0
NASHVD (0336)	2785.00			
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res. (N)= 1.50	
	U.H. Tp(hrs)=	15.39		

Unit Hyd Qpeak (cms)= 3.090

PEAK FLOW (cms)= 2.580 (i)
 TIME TO PEAK (hrs)= 22.250
 RUNOFF VOLUME (mm)= 21.246
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.339

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Total Imp(%)=	Dir. Conn.(%)= 17.00
STANDHYD (3382)	525.90		37.00	
ID= 1 DT=15.0 min				

Surface Area (ha)= 194.58 331.32
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1872.43 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 56.43 44.05
 over (min) 30.00 45.00
 Storage Coeff. (min)= 22.93 (ii) 37.78 (ii)
 Unit Hyd. Tpeak (min)= 30.00 45.00
 Unit Hyd. peak (cms)= 0.04 0.03

TOTALS
 20.457 (iii)

PEAK FLOW (cms)= 10.37 13.64
 TIME TO PEAK (hrs)= 6.25 6.50 6.25
 RUNOFF VOLUME (mm)= 60.70 21.93 28.52
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.35 0.45

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Total Imp(%)=	Dir. Conn.(%)= 23.00
STANDHYD (3381)	73.00		49.00	
ID= 1 DT=15.0 min				

Surface Area (ha)= 35.77 37.23
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 697.61 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 82.76 55.73
 over (min) 15.00 30.00
 Storage Coeff. (min)= 10.88 (ii) 24.40 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

TOTALS
 4.845 (iii)

PEAK FLOW (cms)= 3.17 2.65 4.845 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 60.70 24.15 32.56
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.39 0.52

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5338)		OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1	DT= 15.0 min	(cms)	(ha.m.)	(cms)	(ha.m.)
		0.0000	0.0000	3.3350	1.2400

0.3230	0.3074	4.3560	1.3520
0.9360	0.5374	5.3290	1.4590
1.7590	0.8302	5.7290	1.4690
2.4110	1.0550	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (3381)	73.000	4.845	6.00	32.56
OUTFLOW: ID= 1 (5338)	73.000	1.996	6.75	32.55

PEAK FLOW REDUCTION [Qout/Qin]= 41.19
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 0.9240

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ADD HYD (8310)				
1 + 2 = 3				
ID1= 1 (3382):	529.90	20.457	6.25	28.52
+ ID2= 2 (5338):	73.00	1.996	6.75	32.55
ID = 3 (8310):	598.90	22.328	6.50	29.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

	Area (ha)	Curve Number (CN)
CALIB (3342)	587.10	56.0
NASHYD (0306)	5.00	1.50
ID= 1 DT=15.0 min		
U.H. Tp(hrs)=	8.19	

Unit Hyd Qpeak (cms)= 1.224

PEAK FLOW (cms)= 0.616 (i)
 TIME TO PEAK (hrs)= 15.250
 RUNOFF VOLUME (mm)= 12.935
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.206

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

	Area (ha)	Dir. Conn. (%)
CALIB (3341)	33.00	31.00
STANDHYD (3341)	51.00	
ID= 1 DT=15.0 min		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	16.83	16.17
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	469.04	40.00
Mannings n =	0.013	0.250

Max. Ref. Inten. (mm/hr)=	82.76	19.84
over (min)=	15.00	30.00
Storage Coeff. (min)=	8.58 (ii)	29.01 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.04

TOTALS

PEAK FLOW (cms)=	2.07	0.61	2.446 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	60.70	15.50	29.51
TOTAL RAINFALL (mm)=	62.70	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.25	0.47

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 52.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
RESERVOIR (5334)				
IN= 2--> OUT= 1				
DT= 15.0 min				
0.0000	0.0000	0.4540	0.4604	
0.0390	0.1699	0.8330	0.5249	
0.1860	0.2610	1.0130	0.5896	
0.3750	0.3534	1.4130	0.5996	
0.4340	0.3813	0.0000	0.0000	

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (3341)	33.000	2.446	6.00	29.51
OUTFLOW: ID= 1 (5334)	33.000	0.677	6.75	29.48

PEAK FLOW REDUCTION [Qout/Qin]= 27.69

TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 0.4746

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ADD HYD (8314)				
1 + 2 = 3				
ID1= 1 (3342):	587.10	0.616	15.25	12.94
+ ID2= 2 (5334):	33.00	0.677	6.75	29.48
ID = 3 (8314):	620.10	0.893	6.75	13.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

	Area (ha)	Curve Number (CN)
CALIB (0306)	283.97	52.0
NASHYD (0306)	5.00	1.50
ID= 1 DT=15.0 min		
U.H. Tp(hrs)=	6.44	

Unit Hyd Qpeak (cms)= 0.753

PEAK FLOW (cms)= 0.331 (i)
 TIME TO PEAK (hrs)= 13.750
 RUNOFF VOLUME (mm)= 11.388
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.182

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

	Area (ha)	Curve Number (CN)
CALIB (0286)	260.51	84.0
NASHYD (0286)	5.00	1.50
ID= 1 DT=15.0 min		
U.H. Tp(hrs)=	1.16	

Unit Hyd Qpeak (cms)= 3.834

PEAK FLOW (cms)= 3.462 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 31.127
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.496

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

	Area (ha)	Curve Number (CN)
CALIB (0282)	449.38	77.0
NASHYD (0282)	5.00	1.50
ID= 1 DT=15.0 min		
U.H. Tp(hrs)=	1.47	

Unit Hyd Qpeak (cms)= 5.226

PEAK FLOW (cms)= 3.907 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 24.782
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.395

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

	Area (ha)	Curve Number (CN)
CALIB (0284)	78.93	84.0
NASHYD (0284)	5.00	1.50
ID= 1 DT=15.0 min		
U.H. Tp(hrs)=	0.57	

Unit Hyd Qpeak (cms)= 2.344

PEAK FLOW (cms)= 1.836 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 30.659
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.489

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ADD HYD (8388)				
1 + 2 = 3				
ID1= 1 (0282):	449.38	3.907	8.00	24.78
+ ID2= 2 (0284):	78.93	1.836	6.50	30.66
ID = 3 (8388):	528.31	5.316	7.25	25.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6286) |
| IN= 2--> OUP= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (2861.0) -----

Distance	Elevation	Manning	
0.00	233.00	0.0450	
20.58	228.51	0.0450	
41.17	227.74	0.0450	
61.46	227.41	0.0450	
97.77	225.96	0.0450	
149.23	223.94	0.0450	
200.69	220.84	0.0450	
226.42	220.66	0.0450	
238.85	220.22	0.0450	
241.35	220.01	0.0450 / 0.0350	Main Channel
241.85	219.70	0.0350	Main Channel
245.85	219.72	0.0350	Main Channel
246.35	220.06	0.0350	Main Channel
248.85	220.23	0.0350	Main Channel
303.60	221.64	0.0350	Main Channel
380.79	224.98	0.0450	
432.25	229.54	0.0450	
457.98	233.33	0.0450	
483.71	234.27	0.0450	
509.44	233.81	0.0450	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.31	220.01	.382E+04	0.8	0.57	82.89
1.06	220.76	.525E+05	12.1	0.66	72.51
1.81	221.51	.240E+06	88.6	1.05	45.21
2.56	222.26	.510E+06	261.7	1.46	32.47
3.31	223.01	.843E+06	533.8	1.81	26.34
4.06	223.76	.124E+07	913.6	2.10	22.64
4.81	224.51	.171E+07	1399.0	2.34	20.33
5.56	225.26	.225E+07	2051.0	2.60	18.27
6.31	226.01	.285E+07	2907.0	2.91	16.35
7.06	226.76	.352E+07	3902.5	3.16	15.04
7.82	227.52	.426E+07	5069.9	3.39	14.01
8.57	228.27	.507E+07	6433.0	3.62	13.13
9.32	229.02	.593E+07	8069.2	3.89	12.24
10.07	229.77	.681E+07	9924.7	4.16	11.44
10.82	230.52	.772E+07	11962.8	4.42	10.75
11.57	231.27	.864E+07	14167.1	4.68	10.17
12.32	232.02	.959E+07	16537.1	4.92	9.66
13.07	232.77	.105E+08	19070.7	5.16	9.22
13.82	233.52	.115E+08	21790.4	5.39	8.82

----- hydrograph ----- <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8388)	528.31	5.32	7.25	25.66	0.61	0.60
OUTFLOW : ID= 1 (6286)	528.31	4.49	9.00	25.66	0.56	0.60

ADD HYD (8386) |
| 1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 1 (0286): 260.51 3.462 7.50 31.13
+ ID2= 2 (6286): 528.31 4.493 9.00 25.66
=====

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0302) | Area (ha)= 473.90 Curve Number (CN)= 58.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.66

Unit Hyd Qpeak (cms)= 4.874

PEAK FLOW (cms)= 2.024 (i)
TIME TO PEAK (hrs)= 9.500
RUNOFF VOLUME (mm)= 13.712
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.219

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0300) | Area (ha)= 258.93 Curve Number (CN)= 52.0
ID= 1 DT=15.0 min | Ia (mm)= 4.00 # of Linear Res.(N)= 1.50

U.H. Tp(hrs)= 1.03

Unit Hyd Qpeak (cms)= 4.292

PEAK FLOW (cms)= 1.347 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 11.639
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.186

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0298) | Area (ha)= 330.51 Curve Number (CN)= 45.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.26

Unit Hyd Qpeak (cms)= 4.461

PEAK FLOW (cms)= 1.133 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 8.978
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.143

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8395) | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
| 1 + 2 = 3 |
ID1= 1 (0298): 330.51 1.133 8.00 8.98
+ ID2= 2 (0300): 258.93 1.347 7.50 11.64
=====

ID = 3 (8395): 589.44 2.465 7.75 10.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0288) | Area (ha)= 340.83 Curve Number (CN)= 78.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 2.21

Unit Hyd Qpeak (cms)= 2.629

PEAK FLOW (cms)= 2.210 (i)
TIME TO PEAK (hrs)= 9.500
RUNOFF VOLUME (mm)= 25.660
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.409

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0290) | Area (ha)= 269.18 Curve Number (CN)= 78.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 4.279

PEAK FLOW (cms)= 3.070 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 25.504
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.407

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8397) | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
| 1 + 2 = 3 |
ID1= 1 (0288): 340.83 2.210 9.50 25.66
+ ID2= 2 (0290): 269.18 3.070 7.50 25.50
=====

ID = 3 (8397): 610.01 5.105 8.00 25.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0296) | Area (ha)= 293.65 Curve Number (CN)= 76.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.13

Unit Hyd Qpeak (cms)= 4.437
 PEAK FLOW (cms)= 3.009 (1)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 23.935
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.382

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB | Area (ha)= 738.49 | Curve Number (CN)= 68.0
 NASHYD (0292) | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.52

Unit Hyd Qpeak (cms)= 8.289
 PEAK FLOW (cms)= 4.648 (1)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 18.682
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.298

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB | Area (ha)= 274.15 | Curve Number (CN)= 57.0
 NASHYD (0294) | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.87

Unit Hyd Qpeak (cms)= 5.367
 PEAK FLOW (cms)= 1.843 (1)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 13.187
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.210

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8398) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0292): 738.49 4.648 8.25 18.68
 + ID2= 2 (0294): 274.15 1.843 7.00 13.19
 ID = 3 (8398): 1012.64 6.373 8.00 17.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6296) |
 IN= 2--> OUT= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (2961.0) ----->

Distance	Elevation	Manning
0.00	243.98	0.0400
42.59	243.18	0.0400
85.17	241.81	0.0400
120.66	240.50	0.0400
156.15	239.56	0.0400
198.74	236.15	0.0400
237.78	234.01	0.0400
241.33	233.82	0.0400
248.77	233.12	0.0400 / 0.0400 Main Channel
249.87	232.32	0.0400 Main Channel
250.37	231.80	0.0400 Main Channel
250.87	232.23	0.0400 Main Channel
251.97	233.10	0.0400 / 0.0400 Main Channel
255.37	233.22	0.0400
259.87	233.87	0.0400
262.62	234.12	0.0400
266.17	234.23	0.0400
283.91	234.73	0.0400
337.15	241.75	0.0400
351.34	244.00	0.0400

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.43	232.23	.510E+03	0.1	0.53	81.07
0.87	232.67	.213E+04	0.7	0.83	51.49
1.30	233.10	.501E+04	2.2	1.10	38.75
1.98	233.78	.242E+05	12.6	1.34	31.93
2.66	234.46	.738E+05	42.1	1.46	29.21
3.34	235.14	.177E+06	127.9	1.64	23.13

4.02	235.82	.314E+06	281.4	2.29	18.61
4.70	236.50	.481E+06	509.4	2.71	15.73
5.38	237.18	.672E+06	815.6	3.11	13.73
6.06	237.86	.886E+06	1200.7	3.46	12.30
6.74	238.54	.112E+07	1659.4	3.60	11.23
7.42	239.22	.139E+07	2226.9	4.11	10.38
8.10	239.90	.168E+07	2808.4	4.28	9.95
8.78	240.58	.202E+07	3470.4	4.40	9.68
9.46	241.26	.240E+07	4331.7	4.61	9.24
10.14	241.94	.283E+07	5355.2	4.83	8.84
10.82	242.62	.330E+07	6482.6	5.03	8.48
11.50	243.30	.381E+07	7764.1	5.21	8.18
12.18	243.98	.439E+07	9101.2	5.31	8.04

<---- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8398) 1012.64 6.37 8.00 17.19 1.57 1.18
 OUTFLOW : ID= 1 (6296) 1012.64 6.16 8.50 17.19 1.56 1.18

 ADD HYD (8396) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0296): 293.65 3.009 7.50 23.94
 + ID2= 2 (6296): 1012.64 6.164 8.50 17.19
 ID = 3 (8396): 1306.29 9.032 8.25 18.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8394) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (8396): 1306.29 9.032 8.25 18.71
 + ID2= 2 (8397): 610.01 5.105 8.00 25.59
 ID = 3 (8394): 1916.30 14.100 8.25 20.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8392) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (8394): 1916.30 14.100 8.25 20.90
 + ID2= 2 (8395): 589.44 2.465 7.75 10.15
 ID = 3 (8392): 2505.74 16.538 8.00 18.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6302) |
 IN= 2--> OUT= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (3021.0) ----->

Distance	Elevation	Manning
0.00	228.10	0.0400
18.47	227.12	0.0400
36.95	226.12	0.0400
46.18	225.84	0.0400
55.42	225.58	0.0400
272.47	222.88	0.0400
277.09	222.76	0.0400
281.71	222.58	0.0400
288.54	222.18	0.0400 / 0.0300 Main Channel
288.64	221.00	0.0300 Main Channel
290.94	221.00	0.0300 Main Channel
291.04	221.00	0.0300 Main Channel
291.54	222.75	0.0300 / 0.0400 Main Channel
300.18	222.83	0.0400
304.80	223.04	0.0400
309.42	223.25	0.0400
318.65	223.69	0.0400
360.22	225.57	0.0400
397.16	227.60	0.0400
457.20	228.35	0.0400

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.29	221.29	.228E+04	0.2	0.32	165.63
0.59	221.59	.465E+04	0.7	0.45	116.15
0.88	221.88	.713E+04	1.2	0.54	96.87
1.18	222.18	.971E+04	1.9	0.61	86.20

1.57	222.57	.175E+05	3.3	0.59	88.86
1.97	222.97	.427E+05	6.4	0.47	111.39
2.36	223.36	.115E+06	16.1	0.44	119.14
2.76	223.76	.237E+06	37.1	0.49	106.49
3.15	224.15	.409E+06	72.9	0.56	93.40
3.55	224.55	.631E+06	127.0	0.63	82.76
3.94	224.94	.903E+06	202.4	0.70	74.37
4.34	225.34	1.23E+07	301.8	0.77	67.68
4.73	225.73	1.60E+07	433.5	0.85	61.37
5.13	226.13	2.00E+07	602.2	0.95	55.29
5.52	226.52	2.42E+07	804.7	1.05	50.13
5.92	226.92	2.86E+07	1034.2	1.14	46.11
6.31	227.31	3.32E+07	1290.5	1.22	42.88
6.71	227.71	3.80E+07	1560.9	1.29	40.55
7.10	228.10	4.32E+07	1835.6	1.34	39.19

<--- hydrograph ---> <-pipe / channel-->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8392) 2505.74	16.54	8.00	18.37	2.37	0.44
OUTFLOW: ID= 1 (8302) 2505.74	13.15	10.50	18.37	2.24	0.45

ADD HYD (8390)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0302):	473.90	2.024	8.50	13.71
+ ID2= 2 (8302):	2505.74	13.148	10.50	18.37
=====				
ID = 3 (8390):	2979.64	15.009	10.25	17.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8348)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8386):	788.82	7.643	8.50	27.47
+ ID2= 2 (8390):	2979.64	15.009	10.25	17.63
=====				
ID = 3 (8348):	3768.46	21.986	9.75	19.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	QPEAK	TPEAK	R.V.
NASHYD (0304)	(ha)	(cms)	(hrs)	(mm)	
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50		
	U.H. Tp(hrs)=	2.78			
Unit Hyd Opeak	(cms)=	1.793			
PEAK FLOW	(cms)=	0.987 (1)			
TIME TO PEAK	(hrs)=	10.500			
RUNOFF VOLUME	(mm)=	16.057			
TOTAL RAINFALL	(mm)=	62.700			
RUNOFF COEFFICIENT	=	0.256			

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	QPEAK	TPEAK	R.V.
NASHYD (0280)	(ha)	(cms)	(hrs)	(mm)	
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50		
	U.H. Tp(hrs)=	0.85			
Unit Hyd Opeak	(cms)=	6.009			
PEAK FLOW	(cms)=	4.745 (1)			
TIME TO PEAK	(hrs)=	7.000			
RUNOFF VOLUME	(mm)=	28.965			
TOTAL RAINFALL	(mm)=	62.700			
RUNOFF COEFFICIENT	=	0.462			

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	QPEAK	TPEAK	R.V.
NASHYD (0278)	(ha)	(cms)	(hrs)	(mm)	
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50		
	U.H. Tp(hrs)=	1.52			
Unit Hyd Opeak	(cms)=	5.453			
PEAK FLOW	(cms)=	4.873 (1)			
TIME TO PEAK	(hrs)=	8.000			
RUNOFF VOLUME	(mm)=	29.184			

TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.465

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	QPEAK	TPEAK	R.V.
NASHYD (0276)	(ha)	(cms)	(hrs)	(mm)	
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50		
	U.H. Tp(hrs)=	0.67			
Unit Hyd Opeak	(cms)=	2.302			
PEAK FLOW	(cms)=	1.552 (1)			
TIME TO PEAK	(hrs)=	6.750			
RUNOFF VOLUME	(mm)=	26.101			
TOTAL RAINFALL	(mm)=	62.700			
RUNOFF COEFFICIENT	=	0.416			

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	QPEAK	TPEAK	R.V.
NASHYD (0274)	(ha)	(cms)	(hrs)	(mm)	
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50		
	U.H. Tp(hrs)=	1.08			
Unit Hyd Opeak	(cms)=	6.182			
PEAK FLOW	(cms)=	4.016 (1)			
TIME TO PEAK	(hrs)=	7.500			
RUNOFF VOLUME	(mm)=	23.174			
TOTAL RAINFALL	(mm)=	62.700			
RUNOFF COEFFICIENT	=	0.370			

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8360)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0274):	392.49	4.016	7.50	23.17
+ ID2= 2 (0276):	90.89	1.552	6.75	26.10
=====				
ID = 3 (8360):	483.38	5.470	7.25	23.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8358)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0278):	485.49	4.873	8.00	29.18
+ ID2= 2 (8360):	483.38	5.470	7.25	23.72
=====				
ID = 3 (8358):	968.87	10.143	7.50	26.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6280) Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2801.0) ----->

Distance	Elevation	Manning
0.00	241.14	0.0500
13.32	240.80	0.0500
39.95	240.07	0.0500
96.54	236.21	0.0500
113.19	234.15	0.0500
123.18	232.35	0.0500
143.15	225.80	0.0500
149.81	225.62	0.0500
153.14	225.40	0.0500
157.30	224.76	0.0500 /0.0300 Main Channel
159.80	224.26	0.0300 Main Channel
162.30	224.85	0.0300 Main Channel
162.55	224.97	0.0300 /0.0500 Main Channel
167.80	225.05	0.0500
186.43	229.14	0.0500
213.06	234.75	0.0500
236.37	237.09	0.0500
266.33	237.31	0.0500
292.96	237.83	0.0500
329.58	241.50	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
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(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.50	224.76	.263E+04	0.8	0.65	58.31
1.36	225.62	.278E+05	13.4	1.10	34.54
2.22	226.48	.895E+05	52.6	1.40	27.09
3.09	227.35	.157E+06	120.4	1.74	21.76
3.95	228.21	.242E+06	216.3	2.04	18.64
4.81	229.07	.340E+06	341.8	2.29	16.56
5.67	229.93	.450E+06	498.3	2.52	15.06
6.53	230.79	.574E+06	686.3	2.73	13.90
7.40	231.66	.711E+06	913.8	2.93	12.97
8.26	232.52	.861E+06	1173.4	3.11	12.23
9.12	233.38	1.03E+07	1461.3	3.24	11.72
9.98	234.24	1.21E+07	1793.3	3.37	11.26
10.85	235.11	1.42E+07	2141.0	3.45	11.02
11.71	235.97	1.65E+07	2530.4	3.50	10.86
12.57	236.83	1.92E+07	2954.0	3.51	10.81
13.43	237.69	2.22E+07	3160.3	3.18	11.93
14.29	238.55	2.71E+07	3869.4	3.26	11.67
15.16	239.42	3.20E+07	4740.3	3.38	11.24
16.02	240.28	3.73E+07	5688.4	3.48	10.92

<--- hydrograph ---> <-pipe / channel-->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8358) 968.87 10.14 7.50 26.46 1.14 0.93
 OUTFLOW: ID= 1 (6280) 968.87 9.72 8.25 26.46 1.11 0.92

ADD HYD (8354)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID= 1 (0264):	299.86	4.745	7.00	28.96
+ ID= 2 (6280):	968.87	9.721	8.25	26.46
ID = 3 (8354):	1268.73	13.898	8.00	27.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 75.0
NASHYD (0272)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50	
ID= 1 DT=15.0 min	U.H. Tp(hrs)= 1.09			

Unit Hyd Qpeak (cms)= 2.456
 PEAK FLOW (cms)= 1.599 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 23.176
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.370

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 81.0
NASHYD (0270)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.30	
ID= 1 DT=15.0 min	U.H. Tp(hrs)= 0.87			

Unit Hyd Qpeak (cms)= 3.429
 PEAK FLOW (cms)= 2.706 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 28.005
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.447

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 75.0
NASHYD (0268)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.30	
ID= 1 DT=15.0 min	U.H. Tp(hrs)= 0.69			

Unit Hyd Qpeak (cms)= 3.807
 PEAK FLOW (cms)= 2.333 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 22.965
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.366

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

NASHYD (0264)	Area	(ha)= 353.96	Curve Number	(CN)= 69.0
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.30	
	U.H. Tp(hrs)= 1.30			

Unit Hyd Qpeak (cms)= 3.313
 PEAK FLOW (cms)= 1.927 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 19.223
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.307

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)= 508.09	Curve Number	(CN)= 64.0
NASHYD (0266)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.30	
ID= 1 DT=15.0 min	U.H. Tp(hrs)= 1.63			

Unit Hyd Qpeak (cms)= 3.786
 PEAK FLOW (cms)= 1.975 (i)
 TIME TO PEAK (hrs)= 9.250
 RUNOFF VOLUME (mm)= 16.500
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.263

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8380)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID= 1 (0264):	353.96	1.927	8.25	19.22
+ ID= 2 (0266):	508.09	1.975	9.25	16.50
ID = 3 (8380):	862.05	3.882	8.50	17.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6268)	Routing time step (min)= 15.00
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<----- DATA FOR SECTION (2681.0) ----->			
Distance	Elevation	Manning	
0.00	274.03	0.0500	
10.55	272.73	0.0500	
21.10	271.28	0.0500	
50.12	266.83	0.0500	
87.04	260.36	0.0500	
92.32	260.06	0.0500	
94.95	259.93	0.0500	
101.00	256.87	0.0500	
103.00	256.58	0.0500 / 0.0350	Main Channel
104.00	256.32	0.0350	Main Channel
106.00	256.05	0.0350	Main Channel
108.00	256.25	0.0350	Main Channel
110.78	256.65	0.0350 / 0.0500	Main Channel
113.42	256.81	0.0500	
116.06	257.00	0.0500	
155.62	260.24	0.0500	
187.27	263.35	0.0500	
211.01	267.22	0.0500	
224.20	269.60	0.0500	
261.13	272.68	0.0500	

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.53	256.58	.697E+04	2.1	0.97	55.64
1.38	257.43	.477E+05	25.9	1.76	30.66
2.22	258.27	.123E+06	84.3	2.22	24.26
3.07	259.12	.231E+06	185.4	2.60	20.72
3.92	259.97	.371E+06	335.8	2.92	18.42
4.77	260.82	.560E+06	547.6	3.15	17.06
5.61	261.66	.790E+06	845.6	3.46	15.57
6.46	262.51	1.06E+07	1225.0	3.75	14.37
7.31	263.36	1.36E+07	1693.3	4.02	13.37
8.16	264.21	1.69E+07	2283.2	4.35	12.36
9.00	265.05	2.06E+07	2969.1	4.66	11.54
9.85	265.90	2.43E+07	3754.4	4.96	10.86
10.70	266.75	2.86E+07	4643.0	5.24	10.28
11.55	267.60	3.31E+07	5636.1	5.50	9.78
12.39	268.44	3.78E+07	6746.2	5.76	9.34
13.24	269.29	4.28E+07	7972.2	6.01	8.96
14.09	270.14	4.82E+07	9218.9	6.18	8.71
14.94	270.99	5.39E+07	10550.5	6.32	8.52
15.78	271.83	6.01E+07	12024.7	6.46	8.33

```

<----- hydrograph ----> <-pipe / channel->
AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
(ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
INFLOW : ID= 2 (8380) 862.05   3.88   8.50 17.62   0.59   1.00
OUTFLOW: ID= 1 (6268) 862.05   3.78  10.00 17.62   0.59   1.00

```

```

ADD HYD (8382)
1 + 2 = 3
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0268): 215.76  2.333  7.00  22.96
+ ID2= 2 (6268): 862.05  3.775  10.00  17.62
=====
ID = 3 (8382): 1077.81  5.511  8.75  18.69

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6270)
IR= 2--> OUT= 1
Routing time step (min)= 15.00

```

<----- DATA FOR SECTION (2701.0) ----->

Distance	Elevation	Manning	
0.00	245.98	0.0500	
8.14	245.66	0.0500	
16.28	245.16	0.0500	
20.35	244.84	0.0500	
38.66	242.98	0.0500	
48.83	240.65	0.0500	
63.07	235.91	0.0500	
65.11	235.18	0.0500	
68.43	234.34	0.0500 / 0.0300	Main Channel
68.68	233.95	0.0300	Main Channel
69.18	233.89	0.0300	Main Channel
69.68	233.95	0.0300	Main Channel
71.21	234.48	0.0300 / 0.0500	Main Channel
81.38	236.44	0.0500	
95.63	236.66	0.0500	
120.04	237.00	0.0500	
148.53	241.77	0.0500	
158.70	242.34	0.0500	
187.18	244.03	0.0500	
201.42	244.36	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELRV (m)	VOLUME (cu.m)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.45	234.34	.217E+04	0.7	0.99	52.83
0.98	234.87	.966E+04	4.6	1.49	35.20
1.50	235.39	.250E+05	13.7	1.73	30.41
2.03	235.92	.475E+05	29.5	1.95	26.87
2.56	236.45	.772E+05	52.5	2.14	24.50
3.09	236.98	.142E+06	82.4	1.83	28.73
3.61	237.50	.246E+06	150.1	2.03	25.91
4.14	238.03	.357E+06	262.0	2.31	22.73
4.67	238.56	.477E+06	392.1	2.59	20.27
5.20	239.09	.604E+06	547.8	2.86	18.38
5.72	239.61	.739E+06	729.2	3.11	16.90
6.25	240.14	.882E+06	936.5	3.34	15.70
6.78	240.67	.103E+07	1169.7	3.57	14.72
7.31	241.20	.119E+07	1425.9	3.77	13.94
7.83	241.72	.136E+07	1710.4	3.96	13.26
8.36	242.25	.154E+07	1967.9	4.02	13.07
8.89	242.78	.174E+07	2264.9	4.09	12.83
9.42	243.31	.196E+07	2589.3	4.15	12.65
9.94	243.83	.221E+07	2953.5	4.22	12.46

```

<----- hydrograph ----> <-pipe / channel->
AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
(ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
INFLOW : ID= 2 (8382) 1077.81  5.51  8.75 18.69   1.03  1.51
OUTFLOW: ID= 1 (6270) 1077.81  5.44  9.50 18.69   1.03  1.51

```

```

ADD HYD (8384)
1 + 2 = 3
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0270): 243.61  2.706  7.25  28.01
+ ID2= 2 (6270): 1077.81  5.436  9.50  18.69
=====
ID = 3 (8384): 1321.42  7.795  8.75  20.41

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB (0262) | Area (ha)= 341.31 Curve Number (CN)= 82.0
NASHVD (0262) |

```

```

ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
U.H. Tp(hrs)= 1.01

```

Unit Hyd Qpeak (cms)= 4.094

```

PEAK FLOW (cms)= 3.472 (1)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 29.023
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.463

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB (0260) | Area (ha)= 476.24 Curve Number (CN)= 82.0
NASHVD (0260) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.33

```

Unit Hyd Qpeak (cms)= 4.360

```

PEAK FLOW (cms)= 3.926 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 29.117
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.464

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB (0258) | Area (ha)= 181.99 Curve Number (CN)= 79.0
NASHVD (0258) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.18

```

Unit Hyd Qpeak (cms)= 1.881

```

PEAK FLOW (cms)= 1.492 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 26.347
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.420

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB (0252) | Area (ha)= 319.99 Curve Number (CN)= 73.0
NASHVD (0252) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.04

```

Unit Hyd Qpeak (cms)= 3.761

```

PEAK FLOW (cms)= 2.368 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 21.720
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.346

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB (0256) | Area (ha)= 145.79 Curve Number (CN)= 67.0
NASHVD (0256) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.04

```

Unit Hyd Qpeak (cms)= 1.707

```

PEAK FLOW (cms)= 0.887 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 18.019
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.287

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB (0254) | Area (ha)= 403.00 Curve Number (CN)= 55.0
NASHVD (0254) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.22

```

Unit Hyd Qpeak (cms)= 4.028

```

PEAK FLOW (cms)= 1.477 (1)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 12.430
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.198

```


(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6256) | Routing time step (min)'= 15.00

DATA FOR SECTION (2561.0) table with columns: Distance, Elevation, Manning

TRAVEL TIME TABLE table with columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV.TIME

hydrograph table with columns: AREA, QPEAK, TPEAK, R.V., MAX DEPTH, MAX VEL

ADD HYD (8370) table with columns: AREA, QPEAK, TPEAK, R.V.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0250) | Area (ha)= 192.88 Curve Number (CN)= 70.0

Unit Hyd Qpeak (cms) = 1.930 PEAK FLOW (cms) = 1.144 (i)

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0246) | Area (ha) = 759.61 Curve Number (CN) = 55.0

ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30

Unit Hyd Qpeak (cms) = 5.121 PEAK FLOW (cms) = 2.071 (i)

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0248) | Area (ha) = 146.04 Curve Number (CN) = 64.0

Unit Hyd Qpeak (cms) = 2.271 PEAK FLOW (cms) = 0.994 (i)

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8364) table with columns: AREA, QPEAK, TPEAK, R.V.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6250) | Routing time step (min)'= 15.00

DATA FOR SECTION (2501.0) table with columns: Distance, Elevation, Manning

TRAVEL TIME TABLE table with columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV.TIME

hydrograph table with columns: AREA, QPEAK, TPEAK, R.V., MAX DEPTH, MAX VEL

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8364)	905.65	2.93	8.25	13.10	0.81	0.75
OUTFLOW: ID= 1 (6250)	905.65	2.72	10.50	13.10	0.79	0.75

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8372)	1967.31	8.08	9.25	15.39	0.65	0.86
OUTFLOW: ID= 1 (6258)	1967.31	7.36	11.25	15.39	0.63	0.87

```

ADD HYD (8366)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0250): 192.88 1.14 8.00 19.82
+ ID2= 2 (6250): 905.65 2.724 10.50 13.10
=====
ID = 3 (8366): 1098.53 3.765 10.00 14.28

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8374)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0258): 181.99 1.492 8.00 26.35
+ ID2= 2 (6258): 1967.31 7.363 11.25 15.39
=====
ID = 3 (8374): 2149.30 8.558 11.00 16.31

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8368)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8366): 1098.53 3.765 10.00 14.28
+ ID2= 2 (8370): 548.79 2.197 9.25 13.91
=====
ID = 3 (8368): 1647.32 5.937 9.75 14.16

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8376)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0260): 476.24 3.926 8.00 29.12
+ ID2= 2 (8374): 2149.30 8.558 11.00 16.31
=====
ID = 3 (8376): 2625.54 12.057 10.25 18.64

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8372)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0252): 319.99 2.368 8.00 21.72
+ ID2= 2 (8368): 1647.32 5.937 9.75 14.16
=====
ID = 3 (8372): 1967.31 8.081 9.25 15.39

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8378)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0262): 341.31 3.472 7.75 29.02
+ ID2= 2 (8376): 2625.54 12.057 10.25 18.64
=====
ID = 3 (8378): 2966.85 14.956 10.00 19.83

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6258)
IN= 2--> OUT= 1
Routing time step (min)= 15.00

```

```

<----- DATA FOR SECTION (2581.0) ----->
Distance Elevation Manning
0.00 252.88 0.0500
15.47 251.89 0.0500
46.41 248.45 0.0500
73.48 245.81 0.0500
88.95 243.75 0.0500
112.15 242.00 0.0500
135.35 240.23 0.0500
162.42 239.76 0.0500
170.97 239.52 0.0500 /0.0350 Main Channel
171.58 239.03 0.0350 Main Channel
174.02 239.03 0.0350 Main Channel
176.46 239.03 0.0350 Main Channel
177.07 239.52 0.0350 /0.0500 Main Channel
185.63 239.67 0.0500
208.83 239.87 0.0500
239.77 240.14 0.0500
274.57 244.93 0.0500
336.45 249.51 0.0500
363.52 249.77 0.0500
382.85 251.78 0.0500

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.49 239.52 135E+05 2.5 0.92 91.32
1.14 240.17 175E+06 25.6 0.73 114.22
1.78 240.81 532E+06 123.5 1.17 71.85
2.43 241.46 932E+06 285.3 1.54 54.48
3.07 242.10 138E+07 506.5 1.85 45.25
3.72 242.75 186E+07 786.8 2.12 39.42
4.36 243.39 239E+07 1127.3 2.37 35.33
5.01 244.04 296E+07 1535.8 2.61 32.12
5.65 244.68 356E+07 2014.3 2.84 29.48
6.30 245.33 420E+07 2535.9 3.03 27.60
6.94 245.97 488E+07 3108.4 3.20 26.16
7.59 246.62 561E+07 3740.6 3.35 24.98
8.23 247.26 638E+07 4446.7 3.50 23.93
8.88 247.91 721E+07 5229.7 3.64 22.98
9.52 248.55 809E+07 6094.3 3.78 22.12
10.17 249.20 901E+07 7050.4 3.93 21.31
10.81 249.84 1008E+08 7773.0 3.90 21.46
11.46 250.49 1111E+08 8998.8 4.07 20.56
12.10 251.13 1228E+08 10316.2 4.24 19.75

```

```

ADD HYD (8362)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8378): 2966.85 14.956 10.00 19.83
+ ID2= 2 (8384): 1321.42 7.795 8.75 20.41
=====
ID = 3 (8362): 4288.27 22.550 9.50 20.01

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6272)
IN= 2--> OUT= 1
Routing time step (min)= 15.00

```

```

<----- DATA FOR SECTION (2721.0) ----->
Distance Elevation Manning
0.00 231.01 0.0450
23.01 223.65 0.0450
34.51 222.46 0.0450
51.77 222.11 0.0450
69.02 221.87 0.0450
161.06 221.92 0.0450
166.81 221.91 0.0450
172.56 221.89 0.0450
180.57 221.40 0.0450 /0.0300 Main Channel
181.57 220.60 0.0300 Main Channel
184.07 220.16 0.0300 Main Channel
195.57 221.85 0.0300 /0.0450 Main Channel
201.32 221.82 0.0450
207.07 221.77 0.0450
212.83 221.72 0.0450
253.09 222.52 0.0450
408.40 222.65 0.0450
460.17 223.20 0.0450
517.69 224.84 0.0450
569.46 232.57 0.0450

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.41 220.57 189E+04 0.5 0.49 60.18
0.83 220.99 697E+04 3.2 0.82 35.79
1.24 221.40 145E+05 8.8 1.07 27.45
1.84 222.00 643E+05 29.3 0.61 36.61

```

2.44	222.60	.282E+06	132.9	0.83	35.41
3.04	223.20	.707E+06	413.7	1.04	28.50
3.64	223.80	.118E+07	876.8	1.31	22.46
4.24	224.40	.168E+07	1494.8	1.57	18.75
4.84	225.00	.223E+07	2264.1	1.82	16.23
5.44	225.60	.274E+07	3196.3	2.06	14.29
6.04	226.20	.328E+07	4258.6	2.30	12.84
6.65	226.81	.383E+07	5444.7	2.52	11.72
7.25	227.41	.438E+07	6749.7	2.73	10.82
7.85	228.01	.494E+07	8169.4	2.93	10.08
8.45	228.61	.551E+07	9700.7	3.12	9.46
9.05	229.21	.608E+07	11340.8	3.30	8.94
9.65	229.81	.666E+07	13087.4	3.48	8.48
10.25	230.41	.724E+07	14938.5	3.65	8.08
10.85	231.01	.783E+07	16892.4	3.82	7.73

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8362) 4288.27 22.55 9.50 20.01 1.64 0.88
OUTFLOW: ID= 1 (6272) 4288.27 22.22 10.25 20.01 1.63 0.88

```

```

ADD HYD (8356)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0272): 157.38 1.599 7.50 23.18
+ ID2= 2 (6272): 4288.27 22.221 10.25 20.01
=====
ID = 3 (8356): 4445.65 23.386 10.00 20.12

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8352)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8354): 1268.73 13.898 8.00 27.05
+ ID2= 2 (8356): 4445.65 23.386 10.00 20.12
=====
ID = 3 (8352): 5714.38 35.368 9.00 21.66

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6304)
IN= 2--> OUT= 1
Routing time step (min)= 15.00

```

```

<----- DATA FOR SECTION (3041.0) ----->
Distance Elevation Manning
0.00 232.08 0.0500
19.00 231.87 0.0500
38.00 231.33 0.0500
66.51 230.44 0.0500
104.51 228.25 0.0500
118.76 225.17 0.0500
128.26 219.86 0.0500
175.77 219.17 0.0500
185.27 218.90 0.0500 /0.0300 Main Channel
185.52 218.65 0.0300 Main Channel
190.02 218.37 0.0300 Main Channel
194.52 218.60 0.0300 Main Channel
194.77 218.85 0.0300 /0.0500 Main Channel
204.27 219.60 0.0500
299.28 220.91 0.0500
327.78 222.36 0.0500
375.28 225.71 0.0500
403.79 229.37 0.0500
432.29 230.43 0.0500
470.29 232.00 0.0500

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.48 218.85 121E+05 1.2 0.36 173.94
1.17 219.54 839E+05 9.8 0.44 142.60
1.86 220.23 323E+06 41.0 0.47 131.43
2.56 220.93 708E+06 109.5 0.58 107.67
3.25 221.62 118E+07 224.3 0.71 87.36
3.94 222.31 168E+07 376.5 0.84 74.48
4.63 223.00 222E+07 567.9 0.96 65.26
5.32 223.69 279E+07 794.7 1.07 58.58
6.02 224.39 339E+07 1056.2 1.17 53.52
6.71 225.08 402E+07 1352.4 1.26 49.52
7.40 225.77 468E+07 1677.2 1.34 46.46
8.09 226.46 536E+07 2050.0 1.43 43.58
8.79 227.16 607E+07 2457.9 1.52 41.14
9.48 227.85 680E+07 2901.0 1.60 39.05
10.17 228.54 755E+07 3352.7 1.66 37.53

```

10.86	229.23	.834E+07	3812.5	1.71	36.48
11.55	229.92	.919E+07	4241.8	1.73	36.13
12.25	230.62	.101E+08	4697.0	1.74	35.93
12.94	231.31	.111E+08	5186.3	1.74	35.83

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8352) 5714.38 35.37 9.00 21.66 1.74 0.47
OUTFLOW: ID= 1 (6304) 5714.38 29.22 11.50 21.66 1.60 0.46

```

```

ADD HYD (8350)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0304): 292.37 0.987 10.50 16.06
+ ID2= 2 (6304): 5714.38 29.219 11.50 21.66
=====
ID = 3 (8350): 6006.75 30.190 11.50 21.39

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8346)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8348): 3768.46 21.986 9.75 19.69
+ ID2= 2 (8350): 6006.75 30.190 11.50 21.39
=====
ID = 3 (8346): 9775.21 50.996 10.75 20.73

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8344)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0306): 283.97 0.331 13.75 11.39
+ ID2= 2 (8346): 9775.21 50.996 10.75 20.73
=====
ID = 3 (8344): 10059.18 51.294 10.75 20.47

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

RESERVOIR (5509)
IN= 2--> OUT= 1
DT= 15.0 min
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
0.0000 0.0000 | ***** 370.0451
41.0590 56.7403 | ***** 863.4386
48.1390 86.3439 | *****
56.6340 ***** | *****
67.9600 ***** | 0.0000 0.0000

```

```

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (8344) ***** 51.294 10.75 20.47
OUTFLOW: ID= 1 (5509) ***** 39.479 14.50 20.47

```

PEAK FLOW REDUCTION [Qout/Qin](%)= 76.97
TIME SHIFT OF PEAK FLOW (min)=225.00
MAXIMUM STORAGE USED (ha.m.)= 54.5767

```

CALIB NASHYD (0332)
Area (ha)= 393.44 Curve Number (CN)= 75.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 2.32

```

Unit Hyd Qpeak (cms) = 2.894
PEAK FLOW (cms) = 2.232 (i)
TIME TO PEAK (hrs) = 9.750
RUNOFF VOLUME (mm) = 23.317
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.372

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB NASHYD (0330)
Area (ha)= 468.30 Curve Number (CN)= 80.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.09

```

Unit Hyd Qpeak (cms)= 7.335
 PEAK FLOW (cms)= 5.661 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 27.223
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.434

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6332) | Routing time step (min)= 15.00
 IN= 2--> OUT= 1 |

Distance	Elevation	Manning
0.00	234.00	0.0380
25.85	227.20	0.0380
56.94	226.44	0.0380
168.03	227.38	0.0380
219.73	225.62	0.0380
342.52	221.57	0.0380
368.37	221.42	0.0380
374.83	221.23	0.0380
379.79	220.98	0.0380 / 0.0300
380.29	220.47	0.0300
381.29	220.47	0.0300
382.79	220.47	0.0300
383.29	220.98	0.0300 / 0.0380
384.22	221.22	0.0380
400.68	221.33	0.0380
407.14	221.44	0.0380
491.16	225.70	0.0380
569.71	227.55	0.0380
607.49	230.14	0.0380
639.80	234.08	0.0380

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.51	220.98	.554E+04	0.9	0.56	107.27
1.20	221.67	.885E+05	12.1	0.49	122.32
1.88	222.35	.309E+06	64.7	0.76	79.70
2.57	223.04	.615E+06	165.4	0.97	61.99
3.25	223.72	.101E+07	323.1	1.16	52.06
3.94	224.41	.148E+07	542.9	1.33	45.51
4.62	225.09	.204E+07	835.4	1.48	40.77
5.31	225.78	.269E+07	1202.8	1.62	37.27
5.99	226.46	.344E+07	1618.0	1.70	35.46
6.68	227.15	.447E+07	1932.0	1.56	38.56
7.36	227.83	.580E+07	2763.9	1.72	34.96
8.05	228.52	.718E+07	3861.9	1.95	30.97
8.73	229.20	.859E+07	5115.4	2.16	27.98
9.42	229.89	.100E+08	6519.2	2.35	25.64
10.10	230.57	.115E+08	8092.3	2.55	23.69
10.79	231.26	.130E+08	9821.5	2.73	22.05
11.47	231.94	.145E+08	11690.0	2.92	20.68
12.16	232.63	.160E+08	13694.9	3.09	19.52
12.84	233.31	.176E+08	15833.9	3.26	18.52

<---- hydrograph ----> <-pipe / channel-->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW: ID= 2 (0330)	468.30	5.66	7.50	27.22	0.80
OUTFLOW: ID= 1 (6332)	468.30	4.35	9.75	27.22	0.72

ADD HYD (8322)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID= 1 (0332):	393.44	2.232	9.75	23.32
+ ID= 2 (6332):	468.30	4.351	9.75	27.22
ID = 3 (8322):	861.74	6.582	9.75	25.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	(ha)	Curve Number	(CN)=
NASHYD (0328)	492.92	492.92	77.0	77.0	77.0
ID= 1 DT=15.0 min	5.00	5.00	# of Linear Res.(N)=	1.50	1.50
U.H. Tp(hrs)=	1.91				

Unit Hyd Qpeak (cms)= 4.411

PEAK FLOW (cms)= 3.474 (i)
 TIME TO PEAK (hrs)= 8.750
 RUNOFF VOLUME (mm)= 24.828
 TOTAL RAINFALL (mm)= 62.700

RUNOFF COEFFICIENT = 0.396

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	(ha)	Curve Number	(CN)=
NASHYD (0326)	678.91	678.91	80.0	80.0	80.0
ID= 1 DT=15.0 min	5.00	5.00	# of Linear Res.(N)=	1.50	1.50
U.H. Tp(hrs)=	1.95				

Unit Hyd Qpeak (cms)= 5.941
 PEAK FLOW (cms)= 5.199 (i)
 TIME TO PEAK (hrs)= 8.750
 RUNOFF VOLUME (mm)= 27.366
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.436

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	(ha)	Curve Number	(CN)=
NASHYD (0324)	615.64	615.64	79.0	79.0	79.0
ID= 1 DT=15.0 min	5.00	5.00	# of Linear Res.(N)=	1.50	1.50
U.H. Tp(hrs)=	1.90				

Unit Hyd Qpeak (cms)= 5.544
 PEAK FLOW (cms)= 4.663 (i)
 TIME TO PEAK (hrs)= 8.750
 RUNOFF VOLUME (mm)= 26.483
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.422

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	(ha)	Curve Number	(CN)=
NASHYD (0320)	278.74	278.74	81.0	81.0	81.0
ID= 1 DT=15.0 min	5.00	5.00	# of Linear Res.(N)=	1.50	1.50
U.H. Tp(hrs)=	1.36				

Unit Hyd Qpeak (cms)= 3.499
 PEAK FLOW (cms)= 2.949 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 28.204
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.450

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	(ha)	Curve Number	(CN)=
NASHYD (3182)	457.40	457.40	78.0	78.0	78.0
ID= 1 DT=15.0 min	5.00	5.00	# of Linear Res.(N)=	1.50	1.50
U.H. Tp(hrs)=	1.49				

Unit Hyd Qpeak (cms)= 5.259
 PEAK FLOW (cms)= 4.075 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 25.594
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.408

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	(ha)	Dir. Conn.(%)
STANDHYD (3181)	26.00	26.00	81.00	81.00
ID= 1 DT=15.0 min				

Surface Area	(ha)	IMPERVIOUS	PERVIOUS (i)
Dep. Storage	21.06	2.00	5.00
Average Slope	(%)	0.50	0.50
Length	(m)	416.33	40.00
Mannings n		0.013	0.250

Max. Eff. Inten.(mm/hr)=	82.76	23.72	
Storage Coeff. (min)=	15.00	30.00	
Unit Hyd. Tpeak (min)=	7.98 (ii)	27.00 (ii)	
Unit Hyd. peak (cms)=	0.10	0.04	
PEAK FLOW (cms)=	4.34	0.23	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	4.484 (iii)
RUNOFF VOLUME (mm)=	60.70	24.93	53.90
TOTAL RAINFALL (mm)=	62.70	62.70	62.70

RUNOFF COEFFICIENT = 0.97 0.40 0.86

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
CN* = 77.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: RESERVOIR (5318), IN= 2--> OUT= 1, DT= 15.0 min, OUTFLOW (cms), STORAGE (ha.m.), OUTFLOW (cms), STORAGE (ha.m.)

INFLOW : ID= 2 (3181) 26.000 4.484 6.00 53.90
OUTFLOW: ID= 1 (5318) 26.000 2.420 6.25 53.82

PEAK FLOW REDUCTION [Qout/Qin](%)= 53.97
TIME SHIFT OF PEAK FLOW (min)= 15.00
MAXIMUM STORAGE USED (ha.m.)= 0.7068

Table with columns: ADD HYD (8334), 1 + 2 = 3, AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm)

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6320) Routing time step (min)'= 15.00
IN= 2--> OUT= 1

Table with columns: DISTANCE, ELEVATION, MANNING

Table with columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV.TIME

Table with columns: AREA, QPEAK, TPEAK, R.V., MAX DEPTH, MAX VEL

Table with columns: ADD HYD (8332), 1 + 2 = 3, AREA, QPEAK, TPEAK, R.V.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: CALIB, NASHID (6314), Area (ha), Curve Number (CN), # of Linear Res. (N)

Unit Hyd Qpeak (cms)= 3.978

PEAK FLOW (cms)= 2.616 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 25.303
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.404

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6316) Routing time step (min)'= 15.00
IN= 2--> OUT= 1

Table with columns: DISTANCE, ELEVATION, MANNING

Table with columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV.TIME

Table with columns: AREA, QPEAK, TPEAK, R.V., MAX DEPTH, MAX VEL

```

-----
| CALIB |
| NASHVD (0316) | Area (ha)= 232.34 Curve Number (CN)= 82.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.01

```

```

Unit Hyd Qpeak (cms) = 3.923

PEAK FLOW (cms) = 3.203 (1)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 29.050
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.463

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8338) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (0316): | 232.34 3.203 7.25 29.05
| + ID2= 2 (6316): | 165.20 2.347 7.50 25.30
|-----|
| ID = 3 (8338): | 397.54 5.531 7.50 27.49

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHVD (0312) | Area (ha)= 359.44 Curve Number (CN)= 80.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.36

```

```

Unit Hyd Qpeak (cms) = 4.529

PEAK FLOW (cms) = 3.684 (1)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 27.291
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.435

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHVD (0308) | Area (ha)= 529.30 Curve Number (CN)= 62.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.62

```

```

Unit Hyd Qpeak (cms) = 5.575

PEAK FLOW (cms) = 2.615 (1)
TIME TO PEAK (hrs) = 8.500
RUNOFF VOLUME (mm) = 15.525
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.248

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHVD (0310) | Area (ha)= 138.28 Curve Number (CN)= 65.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.76

```

```

Unit Hyd Qpeak (cms) = 3.102

PEAK FLOW (cms) = 1.342 (1)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 16.858
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.269

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8342) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (0308): | 529.30 2.615 8.50 15.53
| + ID2= 2 (0310): | 138.28 1.342 7.00 16.86
|-----|
| ID = 3 (8342): | 667.58 3.777 8.00 15.80

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6312) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00
|-----|

```

----- DATA FOR SECTION (3121.0) -----				
Distance	Elevation	Manning		
0.00	265.94	0.0360		
38.07	265.43	0.0360		
59.82	263.98	0.0360		
103.32	254.59	0.0360		
157.70	252.16	0.0360		
217.52	250.45	0.0360		
233.84	247.69	0.0360		
234.71	247.27	0.0360		
239.71	246.38	0.0360		
244.71	246.12	0.0360 /0.0330	Main Channel	
247.71	245.17	0.0330	Main Channel	
249.71	245.19	0.0330	Main Channel	
251.71	245.64	0.0330 /0.0360	Main Channel	
259.71	246.67	0.0360		
282.78	247.12	0.0360		
315.41	251.60	0.0360		
424.17	256.13	0.0360		
478.55	257.04	0.0360		
516.62	259.37	0.0360		
538.37	266.00	0.0360		

```

----- TRAVEL TIME TABLE -----

```

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.47	245.64	.111E+04	3.1	1.78	6.07
1.54	246.71	.103E+05	53.9	3.39	3.19
2.61	247.78	.414E+05	281.3	4.40	2.45
3.68	248.85	.839E+05	749.1	5.79	1.87
4.74	249.91	.135E+06	1463.2	6.97	1.55
5.81	250.98	.201E+06	2354.8	7.61	1.42
6.88	252.05	.295E+06	3533.0	7.76	1.39
7.95	253.12	.427E+06	5303.9	8.05	1.34
9.02	254.19	.594E+06	7856.5	8.58	1.26
10.09	255.26	.792E+06	11449.2	9.37	1.15
11.15	256.32	.101E+07	15862.4	10.15	1.06
12.22	257.39	.127E+07	21074.5	10.75	1.01
13.29	258.46	.155E+07	28184.1	11.78	0.92
14.36	259.53	.185E+07	36386.2	12.77	0.85
15.43	260.60	.215E+07	46110.6	13.89	0.78
16.50	261.67	.246E+07	56822.5	14.95	0.72
17.56	262.73	.278E+07	68498.1	15.97	0.68
18.63	263.80	.310E+07	81118.8	16.94	0.64
19.70	264.87	.344E+07	93603.2	17.66	0.61

```

----- hydrograph ----- <-pipe / channel->
|-----|
| AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL |
| (ha) (cms) (hrs) (mm) (m) (m/s) |
INFLOW : ID= 2 (8342) 667.58 3.78 8.00 15.80 0.49 1.79
OUTFLOW : ID= 1 (6312) 667.58 3.78 8.00 15.80 0.49 1.79
|-----|

```

```

-----
| ADD HYD (8340) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (0312): | 359.44 3.684 8.00 27.29
| + ID2= 2 (6312): | 667.58 3.776 8.00 15.80
|-----|
| ID = 3 (8340): | 1027.02 7.460 8.00 19.82

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8336) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (8338): | 397.54 5.531 7.50 27.49
| + ID2= 2 (8340): | 1027.02 7.460 8.00 19.82
|-----|
| ID = 3 (8336): | 1424.56 12.900 7.75 21.96

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8330) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (8332): | 762.14 6.855 8.50 27.51
| + ID2= 2 (8336): | 1424.56 12.900 7.75 21.96
|-----|
| ID = 3 (8330): | 2186.70 19.539 8.00 23.90

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ID1= 1 (8322): 513.13 4.298 8.50 27.35
 + ID2= 2 (8328): 2802.34 21.348 9.50 24.46
 ID = 3 (8326): 3315.47 25.514 9.50 24.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6324)			
IN= 2--> OUTF= 1			
Routing time step (min)= 15.00			
----- DATA FOR SECTION (3241.0) -----			
Distance	Elevation	Manning	
0.00	234.38	0.0360	
33.57	232.48	0.0360	
67.14	230.14	0.0360	
83.93	228.80	0.0360	
134.29	227.62	0.0360	
209.82	225.10	0.0360	
218.21	224.86	0.0360	
226.61	224.47	0.0360	
234.00	223.86	0.0360 / 0.0300	Main Channel
234.10	223.66	0.0300	Main Channel
235.00	223.66	0.0300	Main Channel
235.90	223.66	0.0300	Main Channel
236.00	223.86	0.0300 / 0.0360	Main Channel
243.39	224.92	0.0360	
251.78	224.89	0.0360	
335.71	225.64	0.0360	
562.32	226.53	0.0360	
637.85	228.36	0.0360	
705.00	229.80	0.0360	
830.89	234.00	0.0360	

----- TRAVEL TIME TABLE -----						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.20	223.86	.145E+04	0.2	0.43	147.27	
0.73	224.39	.159E+05	2.8	0.68	93.11	
1.27	224.93	.561E+05	11.0	0.75	85.19	
1.80	225.46	.217E+06	45.6	0.80	79.20	
2.33	225.99	.566E+06	134.4	0.91	70.20	
2.87	226.53	.122E+07	338.1	1.06	59.97	
3.40	227.06	.205E+07	744.3	1.38	46.16	
3.94	227.60	.298E+07	1295.9	1.66	38.38	
4.47	228.13	.399E+07	1979.9	1.90	33.59	
5.00	228.66	.509E+07	2807.5	2.11	30.21	
5.54	229.20	.627E+07	3815.1	2.32	27.41	
6.07	229.73	.753E+07	4984.1	2.53	25.17	
6.60	230.26	.884E+07	6346.3	2.74	23.20	
7.14	230.80	.102E+08	7861.2	2.95	21.61	
7.67	231.33	.118E+08	9526.4	3.14	20.29	
8.21	231.87	.131E+08	11342.2	3.32	19.18	
8.74	232.40	.146E+08	13309.6	3.49	18.22	
9.27	232.93	.161E+08	15414.0	3.66	17.41	
9.81	233.47	.177E+08	17671.0	3.81	16.70	

----- hydrograph ---->		<-- pipe / channel -->	
AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8330) 2186.70	19.54	8.00	23.90
OUTFLOW: ID= 1 (6324) 2186.70	16.78	9.75	23.90
		1.35	0.76

ADD HYD (8328)				
1 + 2 = 3				
AREA	QPEAK	TPEAK	R.V.	
(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0324): 615.64	4.663	8.75	26.48	
+ ID2= 2 (6324): 2186.70	16.778	9.75	23.90	
ID = 3 (8328): 2802.34	21.348	9.50	24.46	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0322)				
IN= 1 OUF= 15.0 min				
Area	(ha)	Curve Number (CN)=	80.0	
U.H. Tp(hrs)=	1.75	# of Linear Res. (N)=	1.50	

Unit Hyd Qpeak (cms)= 5.020

PEAK FLOW (cms)= 4.298 (i)

TIME TO PEAK (hrs)= 8.500

RUNOFF VOLUME (mm)= 27.347

TOTAL RAINFALL (mm)= 62.700

RUNOFF COEFFICIENT = 0.436

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8326)				
1 + 2 = 3				
AREA	QPEAK	TPEAK	R.V.	
(ha)	(cms)	(hrs)	(mm)	

ADD HYD (8324)				
1 + 2 = 3				
AREA	QPEAK	TPEAK	R.V.	
(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0326): 678.91	5.199	8.75	27.37	
+ ID2= 2 (8326): 3315.47	25.514	9.50	24.91	
ID = 3 (8324): 3994.38	30.661	9.50	25.33	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6328)			
IN= 2--> OUTF= 1			
Routing time step (min)= 15.00			
----- DATA FOR SECTION (3281.0) -----			
Distance	Elevation	Manning	
0.00	228.00	0.0380	
18.58	224.97	0.0380	
78.98	223.52	0.0380	
125.44	223.28	0.0380	
171.90	221.71	0.0380	
213.72	219.65	0.0380	
218.36	219.40	0.0380	
223.01	219.19	0.0380	
225.95	219.14	0.0380 / 0.0320	Main Channel
226.45	218.14	0.0320	Main Channel
236.95	218.14	0.0320	Main Channel
245.85	218.14	0.0320	Main Channel
245.95	219.14	0.0320 / 0.0380	Main Channel
246.24	219.16	0.0380	
250.88	219.24	0.0380	
255.53	219.39	0.0380	
325.22	221.47	0.0380	
367.03	223.14	0.0380	
404.20	225.17	0.0380	
459.95	228.04	0.0380	

----- TRAVEL TIME TABLE -----						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.50	218.64	.373E+05	3.2	0.33	192.37	
1.00	219.14	.752E+05	10.1	0.51	124.71	
1.52	219.66	.149E+06	22.2	0.57	112.06	
2.04	220.18	.279E+06	43.4	0.59	107.06	
2.56	220.70	.464E+06	77.0	0.63	100.52	
3.08	221.22	.705E+06	125.7	0.68	93.56	
3.61	221.75	.100E+07	192.7	0.73	86.62	
4.13	222.27	.135E+07	278.1	0.79	80.92	
4.65	222.79	.176E+07	385.9	0.84	75.86	
5.17	223.31	.222E+07	514.9	0.89	71.82	
5.69	223.83	.280E+07	662.7	0.90	70.40	
6.21	224.35	.345E+07	867.7	0.96	66.35	
6.73	224.87	.417E+07	1107.0	1.01	62.81	
7.25	225.39	.494E+07	1400.0	1.08	58.80	
7.78	225.92	.573E+07	1732.2	1.15	55.17	
8.30	226.44	.655E+07	2099.9	1.22	52.02	
8.82	226.96	.740E+07	2502.8	1.29	49.29	
9.34	227.48	.828E+07	2940.9	1.36	46.90	
9.86	228.00	.918E+07	3414.3	1.42	44.79	

----- hydrograph ---->		<-- pipe / channel -->	
AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8324) 3994.38	30.66	9.50	25.33
OUTFLOW: ID= 1 (6328) 3994.38	26.24	11.25	25.33
		1.62	0.57

ADD HYD (8320)				
1 + 2 = 3				
AREA	QPEAK	TPEAK	R.V.	
(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0328): 492.92	3.474	8.75	24.83	
+ ID2= 2 (6328): 3994.38	26.237	11.25	25.33	
ID = 3 (8320): 4487.30	29.349	11.00	25.27	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ADD HYD (8316) |

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8320):	4487.30	29.349	11.00	25.27
+ ID2= 2 (8322):	861.74	6.582	9.75	25.44
ID = 3 (8318):	5349.04	35.607	10.75	25.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (5509):	10059.18	39.479	14.50	20.47
+ ID2= 2 (8318):	5349.04	35.607	10.75	25.30
ID = 3 (8316):	15408.22	70.146	12.75	22.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8314):	620.10	0.893	6.75	13.82
+ ID2= 2 (8316):	15408.22	70.146	12.75	22.15
ID = 3 (8312):	16028.32	70.826	12.75	21.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8310):	598.90	22.328	6.50	29.01
+ ID2= 2 (8312):	16028.32	70.826	12.75	21.82
ID = 3 (8308):	16627.22	71.998	12.25	22.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5510)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1				
DT= 15.0 min				
	0.0000	0.0000	*****	74.0090
	66.5450	18.5023	*****	*****
	98.5430	37.0045	*****	*****

INFLOW : ID= 2 (8308)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
*****		71.998	12.25	22.08
OUTFLOW: ID= 1 (5510)	*****	69.834	13.50	22.08

PEAK FLOW REDUCTION [Qout/qin](%)= 96.99
 TIME SHIFT OF PEAK FLOW (min)= 75.00
 MAXIMUM STORAGE USED (ha.m.)= 20.4048

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0336):	2785.00	2.580	22.25	21.25
+ ID2= 2 (5510):	16627.22	69.834	13.50	22.08
ID = 3 (8240):	19412.22	72.075	13.50	21.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (5508):	9524.23	32.116	12.25	13.36
+ ID2= 2 (8240):	19412.22	72.075	13.50	21.97
ID = 3 (8238):	28936.45	103.371	13.00	19.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0342):	1004.58	2.724	10.25	12.12
+ ID2= 2 (8238):	28936.45	103.371	13.00	19.13
ID = 3 (8236):	29941.03	105.769	13.00	18.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8232):	285.80	6.442	6.00	19.32
+ ID2= 2 (8236):	29941.03	105.769	13.00	18.90
ID = 3 (8234):	30226.83	106.051	12.75	18.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0344):	344.00	2.485	7.00	14.04
+ ID2= 2 (8234):	30226.83	106.051	12.75	18.90
ID = 3 (8230):	30570.83	106.769	12.75	18.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8226):	1952.06	8.283	8.75	11.68
+ ID2= 2 (8230):	30570.83	106.769	12.75	18.85
ID = 3 (8228):	32522.89	112.061	12.50	18.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0356):	1015.22	3.392	8.00	9.30
+ ID2= 2 (8228):	32522.89	112.061	12.50	18.42
ID = 3 (8190):	33538.11	114.186	12.25	18.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8324)	3994.38	46.68	9.25	38.24	2.09	0.60
OUTFLOW: ID= 1 (6328)	3994.38	39.91	11.00	38.24	1.96	0.59

ADD HYD (8320)						
1 + 2 = 3						
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)		
ID1= 1 (8328):	492.92	5.314	8.75	37.73		
+ ID2= 2 (6328):	3994.38	39.913	11.00	38.24		
ID = 3 (8320):	4487.30	44.685	10.75	38.18		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8318)						
1 + 2 = 3						
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)		
ID1= 1 (8320):	4487.30	44.685	10.75	38.18		
+ ID2= 2 (8322):	861.74	9.928	9.50	38.51		
ID = 3 (8318):	5349.04	54.210	10.75	38.23		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8316)						
1 + 2 = 3						
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)		
ID1= 1 (8309):	10059.30	50.223	6.50	41.92		
+ ID2= 2 (8318):	5349.04	54.210	10.75	38.23		
ID = 3 (8316):	15408.22	97.899	11.75	33.81		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8312)						
1 + 2 = 3						
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)		
ID1= 1 (8314):	620.10	1.884	6.50	21.94		
+ ID2= 2 (8316):	15408.22	97.899	11.75	33.81		
ID = 3 (8312):	16028.32	98.974	11.75	33.35		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8308)						
1 + 2 = 3						
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)		
ID1= 1 (8310):	598.90	33.223	6.50	41.92		
+ ID2= 2 (8312):	16028.32	98.974	11.75	33.35		
ID = 3 (8308):	16627.22	101.128	11.50	33.66		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5510)					
IN= 2--> OUT= 1					
DT= 15.0 min					
	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)	
	0.0000	0.0000	*****	74.0090	
	66.5450	18.5023	*****	*****	
	98.5430	37.0045	*****	*****	

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8308)	*****	101.128	11.50	33.66
OUTFLOW: ID= 1 (5510)	*****	96.575	13.25	33.66

PEAK FLOW REDUCTION [Qout/Qin] (%) = 95.50
 TIME SHIFT OF PEAK FLOW (min) = 105.00
 MAXIMUM STORAGE USED (ha.m.) = 35.8687

ADD HYD (8240)				
1 + 2 = 3				
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0336):	2785.00	3.992	22.25	32.86
+ ID2= 2 (5510):	16627.22	96.575	13.25	33.66
ID = 3 (8240):	19412.22	100.014	13.25	33.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8238)				
1 + 2 = 3				
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (5508):	9524.23	50.303	12.00	21.33
+ ID2= 2 (8240):	19412.22	100.014	13.25	33.55
ID = 3 (8238):	28936.45	149.326	12.75	29.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8236)				
1 + 2 = 3				
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0342):	1004.58	4.420	10.00	19.62
+ ID2= 2 (8238):	28936.45	149.326	12.75	29.53
ID = 3 (8236):	29941.03	153.277	12.75	29.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8234)				
1 + 2 = 3				
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8232):	285.80	9.337	6.50	28.35
+ ID2= 2 (8236):	29941.03	153.277	12.75	29.20
ID = 3 (8234):	30226.83	153.639	12.75	29.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8230)				
1 + 2 = 3				
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0344):	344.00	4.034	7.00	22.48
+ ID2= 2 (8234):	30226.83	153.639	12.75	29.19
ID = 3 (8230):	30570.83	154.850	12.50	29.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8228)				
1 + 2 = 3				
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8226):	1952.06	13.442	8.75	18.81
+ ID2= 2 (8230):	30570.83	154.850	12.50	29.11
ID = 3 (8228):	32522.89	163.469	12.25	28.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8190)				
1 + 2 = 3				
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0356):	1015.22	5.609	8.00	15.27
+ ID2= 2 (8228):	32522.89	163.469	12.25	28.49
ID = 3 (8190):	33538.11	166.987	12.25	28.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ** SIMULATION NUMBER: 12 **

READ STORM	Filename: C:\Users\jascott\AppData\Local\Temp\
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3e280798-92ee-4282-809c-79ef5caed0add\73alf747
 Ptotal= 73.10 mm
 Comments: 25-Year 12-Hour SCS II Design Storm

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.83	3.25	2.92	6.25	13.16	9.25	2.56
0.50	1.83	3.50	2.92	6.50	13.16	9.50	2.56
0.75	1.83	3.75	2.92	6.75	5.85	9.75	2.56
1.00	1.83	4.00	2.92	7.00	5.85	10.00	2.56
1.25	1.83	4.25	4.39	7.25	4.39	10.25	1.46
1.50	1.83	4.50	4.39	7.50	4.39	10.50	1.46
1.75	1.83	4.75	5.85	7.75	4.39	10.75	1.46
2.00	1.83	5.00	5.85	8.00	4.39	11.00	1.46
2.25	2.19	5.25	8.77	8.25	2.56	11.25	1.46
2.50	2.19	5.50	8.77	8.50	2.56	11.50	1.46
2.75	2.19	5.75	35.09	8.75	2.56	11.75	1.46
3.00	2.19	6.00	36.49	9.00	2.56	12.00	1.46

CALIB (0356) Area (ha)=1015.22 Curve Number (CN)= 46.0
 NASHYD (0356) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.37

Unit Hyd Qpeak (cms)= 12.651
 PEAK FLOW (cms)= 4.610 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 12.581
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.172

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0354) Area (ha)= 262.68 Curve Number (CN)= 37.0
 NASHYD (0354) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.38

Unit Hyd Qpeak (cms)= 3.252
 PEAK FLOW (cms)= 0.864 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 9.206
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.126

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0352) Area (ha)= 381.43 Curve Number (CN)= 54.0
 NASHYD (0352) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.82

Unit Hyd Qpeak (cms)= 7.980
 PEAK FLOW (cms)= 3.316 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 16.078
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.220

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0346) Area (ha)= 350.93 Curve Number (CN)= 70.0
 NASHYD (0346) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 6.254
 PEAK FLOW (cms)= 4.425 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 25.923
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.355

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0350) Area (ha)= 366.84 Curve Number (CN)= 48.0
 NASHYD (0350) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 5.831

PEAK FLOW (cms)= 2.124 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 13.386
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.183

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0348) Area (ha)= 590.18 Curve Number (CN)= 48.0
 NASHYD (0348) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.91

Unit Hyd Qpeak (cms)= 5.267

PEAK FLOW (cms)= 2.211 (i)
 TIME TO PEAK (hrs)= 9.000
 RUNOFF VOLUME (mm)= 13.458
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.184

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6350) Routing time step (min)= 15.00
 IN= 2--> OUT= 1

----- DATA FOR SECTION (3501.0) -----

Distance	Elevation	Manning
0.00	287.00	0.0500
23.66	283.72	0.0500
70.98	280.94	0.0500
112.38	280.23	0.0500
171.52	276.80	0.0500
260.24	274.46	0.0500
266.16	274.26	0.0500
272.07	274.12	0.0500
276.49	274.12	0.0500 / 0.0300
276.99	273.82	0.0300
277.99	273.82	0.0300
279.49	273.82	0.0300
280.49	274.27	0.0300 / 0.0500
283.90	274.27	0.0500
289.81	274.57	0.0500
325.30	275.75	0.0500
396.28	278.98	0.0500
449.51	280.97	0.0500
496.83	283.90	0.0500
585.54	287.92	0.0500

----- TRAVEL TIME TABLE -----

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.30	274.12	273E+04	0.9	0.99	49.81
0.98	274.80	583E+05	20.8	1.05	46.71
1.66	275.48	203E+06	95.2	1.38	35.53
2.33	276.15	438E+06	256.0	1.73	28.50
3.01	276.83	753E+06	524.8	2.05	23.97
3.69	277.51	114E+07	935.8	2.43	20.26
4.37	278.19	157E+07	1472.0	2.76	17.82
5.05	278.87	205E+07	2140.7	3.06	16.06
5.72	279.54	261E+07	2938.6	3.33	14.79
6.40	280.22	321E+07	3887.6	3.57	13.77
7.08	280.90	390E+07	4795.4	3.63	13.56
7.76	281.58	468E+07	6196.2	3.91	12.58
8.43	282.25	550E+07	7803.9	4.19	11.74
9.11	282.93	637E+07	9601.9	4.45	11.05
9.79	283.61	728E+07	11594.8	4.70	10.46
10.47	284.29	823E+07	13845.3	4.96	9.91
11.15	284.97	922E+07	16293.4	5.21	9.43
11.82	285.64	103E+08	18952.0	5.45	9.02
12.50	286.32	113E+08	21824.3	5.69	8.65

----- hydrograph ----- <-pipe / channel-->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0348) 590.18 2.21 9.00 13.46 0.34 0.99
 OUTFLOW : ID= 1 (6350) 590.18 2.16 10.25 13.46 0.34 0.99

ADD HYD (8220)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID= 1 (0350): 366.84 2.124 7.50 13.39
 + ID= 2 (6350): 590.18 2.158 10.25 13.46
 ID= 3 (8220): 957.02 3.943 8.75 13.43

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8222)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0344):	350.93	4.425	7.25	25.92
+ ID2= 2 (8220):	957.02	3.943	8.75	13.43
ID = 3 (8222):	1307.95	8.070	8.00	16.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6352)
 IN= 2--> OUT= 1 Routing time step (min)= 15.00

Distance	Elevation	Manning	
0.00	257.95	0.0500	
7.83	257.34	0.0500	
15.66	256.19	0.0500	
21.53	254.01	0.0500	
41.11	244.06	0.0500	
76.35	241.38	0.0500	
111.58	239.74	0.0500	
113.54	239.61	0.0500	
113.75	239.41	0.0500 / 0.0300	Main Channel
113.85	238.81	0.0300	Main Channel
115.50	238.81	0.0300	Main Channel
116.15	238.81	0.0300	Main Channel
116.25	239.41	0.0300	Main Channel
117.46	239.52	0.0500	
119.41	239.72	0.0500	
121.37	240.04	0.0500	
131.16	241.84	0.0500	
156.61	247.03	0.0500	
176.19	251.46	0.0500	
193.80	258.79	0.0500	

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.60	239.41	224E+04	2.2	1.52	17.12
1.58	240.39	21.6E+05	22.3	1.61	16.16
2.55	241.36	8.04E+05	101.0	1.96	13.27
3.53	242.34	1.73E+06	275.6	2.48	10.46
4.50	243.31	4.29E+06	553.9	2.95	8.80
5.48	244.29	4.38E+06	961.8	3.42	7.60
6.45	245.26	5.99E+06	1534.4	3.99	6.51
7.43	246.24	7.70E+06	2228.4	4.51	5.76
8.41	247.22	9.51E+06	3043.6	4.98	5.21
9.38	248.19	1.14E+07	3982.1	5.43	4.78
10.36	249.17	1.34E+07	5040.1	5.85	4.44
11.33	250.14	1.55E+07	6218.5	6.24	4.16
12.31	251.12	1.77E+07	7518.6	6.61	3.93
13.29	252.10	2.00E+07	8969.0	6.98	3.72
14.26	253.07	2.24E+07	10554.1	7.35	3.53
15.24	254.05	2.48E+07	12257.1	7.71	3.37
16.21	255.02	2.72E+07	14045.1	8.03	3.23
17.19	256.00	2.98E+07	15954.8	8.34	3.11
18.16	256.97	3.25E+07	17780.4	8.53	3.04

hydrograph

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8222)	1307.95	8.07	8.00	16.78	0.89
OUTFLOW: ID= 1 (6352)	1307.95	8.01	8.25	16.78	0.88

ADD HYD (8224)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0354):	361.43	3.316	7.00	16.08
+ ID2= 2 (6352):	1307.95	8.013	8.25	16.78
ID = 3 (8224):	1689.38	11.017	7.75	16.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6354)
 IN= 2--> OUT= 1 Routing time step (min)= 15.00

Distance	Elevation	Manning
0.00	253.92	0.0500
7.95	251.83	0.0500

15.89	249.97	0.0500	
19.87	249.05	0.0500	
47.68	242.00	0.0500	
67.55	237.10	0.0500	
83.44	231.11	0.0500	
85.43	230.75	0.0500	
85.66	230.61	0.0500 / 0.0300	Main Channel
85.76	230.01	0.0300	Main Channel
87.41	230.01	0.0300	Main Channel
88.06	230.01	0.0300	Main Channel
88.16	230.61	0.0300 / 0.0500	Main Channel
89.40	230.72	0.0500	
91.39	230.88	0.0500	
93.37	231.03	0.0500	
133.11	233.96	0.0500	
150.99	235.39	0.0500	
172.84	239.03	0.0500	
196.68	250.00	0.0500	

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.60	230.61	451E+04	1.5	1.06	49.40
1.62	231.63	394E+05	14.6	1.16	45.13
2.64	232.65	1.28E+06	58.8	1.44	36.30
3.66	233.67	2.69E+06	149.7	1.74	30.00
4.68	234.69	4.63E+06	301.5	2.04	25.58
5.70	235.71	7.05E+06	530.5	2.36	22.14
6.72	236.73	9.80E+06	848.4	2.71	19.25
7.74	237.75	1.28E+07	1237.9	3.02	17.29
8.76	238.77	1.62E+07	1706.7	3.30	15.83
9.78	239.79	1.99E+07	2291.9	3.61	14.45
10.81	240.82	2.37E+07	2973.7	3.92	13.30
11.83	241.84	2.78E+07	3742.2	4.21	12.38
12.85	242.86	3.21E+07	4599.1	4.49	11.62
13.87	243.88	3.65E+07	5544.6	4.75	10.98
14.89	244.90	4.12E+07	6579.7	5.00	10.44
15.91	245.92	4.61E+07	7705.9	5.23	9.97
16.93	246.94	5.12E+07	8924.6	5.46	9.55
17.95	247.96	5.64E+07	10237.5	5.68	9.19
18.97	248.98	6.19E+07	11646.2	5.89	8.86

hydrograph

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8224)	1689.38	11.02	7.75	16.62	1.34
OUTFLOW: ID= 1 (6354)	1689.38	10.29	8.75	16.62	1.28

ADD HYD (8226)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0354):	262.68	0.864	8.00	9.21
+ ID2= 2 (6354):	1689.38	10.290	8.75	16.62
ID = 3 (8226):	1952.06	11.133	8.75	15.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0344) Area (ha)= 344.00 Curve Number (CN)= 59.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.87

Unit Hyd Qpeak (cms)= 6.790
 PEAK FLOW (cms)= 3.342 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 18.719
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.256

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (3402) Area (ha)= 234.60
 ID= 1 DT=15.0 min Total Imp(%)= 28.00 Dir. Conn.(%)= 13.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 65.69 168.91
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1250.60 40.00
 Mannings n = 0.013 0.250
 Max.Eff.Inten.(mm/hr)= 96.49 17.46
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 14.53 (ii) 36.03 (ii)

Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.07 0.03

TOTALS
 PEAK FLOW (cms)= 6.04 4.65 7.929 (iii)
 TIME TO PEAK (hrs)= 6.00 6.00 6.00
 RUNOFF VOLUME (mm)= 73.10 16.33 23.19
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.22 0.32

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | CALIB |
 | STANDHYD (3401) | Area (ha)= 51.20
 | ID= 1 DT=15.0 min | Total Imp(%)= 46.00 Dir. Conn.(%)= 21.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	23.55	27.65
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	584.24	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	96.49	25.11
over (min)	15.00	30.00
Storage Coeff. (min)	9.20 (ii)	27.79 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.04

TOTALS
 PEAK FLOW (cms)= 2.49 1.35 3.325 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 71.10 18.84 29.81
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.26 0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | RESERVOIR (5340) |
 | IN= 2--> OUT= 1 |
 | DT= 15.0 min |

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	5.3360	1.7877
0.1730	0.6348	7.2780	2.2185
1.8360	0.8250	9.1880	2.6486
2.1540	1.1051	9.5880	2.6586
3.3950	1.3506	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (3401)	51.200	3.325	6.00	29.81
OUTFLOW: ID= 1 (5340)	51.200	1.201	6.75	29.79

PEAK FLOW REDUCTION [Qout/Qin](%)= 36.13
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 0.7607

 | ADD HYD (8232) |
 | 1 + 2 = 3 |

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (3402):	234.60	7.929	6.00	23.19
+ ID2= 2 (5340):	51.20	1.201	6.75	29.79
=====				
ID = 3 (8232):	285.80	8.040	6.00	24.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | CALIB |
 | NASHYD (0342) | Area (ha)=1004.58 Curve Number (CN)= 54.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 2.57

Unit Hyd Qpeak (cms)= 6.686

PEAK FLOW (cms)= 3.660 (i)
 TIME TO PEAK (hrs)= 10.000
 RUNOFF VOLUME (mm)= 16.262
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.222

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | CALIB |
 | NASHYD (0422) | Area (ha)= 780.20 Curve Number (CN)= 54.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.95

Unit Hyd Qpeak (cms)= 6.838

PEAK FLOW (cms)= 3.488 (i)
 TIME TO PEAK (hrs)= 9.000
 RUNOFF VOLUME (mm)= 16.241
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.222

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | CALIB |
 | NASHYD (0410) | Area (ha)= 572.01 Curve Number (CN)= 48.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.46

Unit Hyd Qpeak (cms)= 6.698

PEAK FLOW (cms)= 2.643 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 13.432
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.184

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | CALIB |
 | NASHYD (0408) | Area (ha)= 231.62 Curve Number (CN)= 58.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.64

Unit Hyd Qpeak (cms)= 6.198

PEAK FLOW (cms)= 2.757 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 18.036
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.247

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | CALIB |
 | NASHYD (0376) | Area (ha)= 463.85 Curve Number (CN)= 74.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 7.380

PEAK FLOW (cms)= 6.058 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 29.204
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.400

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | CALIB |
 | NASHYD (0374) | Area (ha)= 545.70 Curve Number (CN)= 61.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.51

Unit Hyd Qpeak (cms)= 6.158

PEAK FLOW (cms)= 3.687 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 20.009
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.274

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0372) Area (ha)= 110.42 Curve Number (CN)= 37.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 1.954
 PEAK FLOW (cms)= 0.471 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 9.165
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.125

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0370) Area (ha)= 191.85 Curve Number (CN)= 63.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.67

Unit Hyd Qpeak (cms)= 4.860
 PEAK FLOW (cms)= 2.560 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 20.954
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.287

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0368) Area (ha)= 159.48 Curve Number (CN)= 46.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.12

Unit Hyd Qpeak (cms)= 2.433
 PEAK FLOW (cms)= 0.838 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 12.552
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.172

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0366) Area (ha)= 462.62 Curve Number (CN)= 62.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.06

Unit Hyd Qpeak (cms)= 7.451
 PEAK FLOW (cms)= 4.212 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 20.530
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.281

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0364) Area (ha)= 155.27 Curve Number (CN)= 55.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.70

Unit Hyd Qpeak (cms)= 3.782
 PEAK FLOW (cms)= 1.561 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 16.518
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.226

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8302)
 1 + 2 = 3
 ID1= 1 (0364): 155.27 1.561 6.75 16.52
 + ID2= 2 (0366): 462.62 4.212 7.50 20.53
 ID = 3 (8302): 617.89 5.710 7.25 19.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANBYD (0362) Area (ha)= 118.78
 ID= 1 DT=15.0 min Total Imp(%)= 22.00 Dir. Conn.(%)= 8.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	26.13	92.65
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	889.87	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	96.49	15.08
Storage Coeff. (min)	15.00	45.00
Unit Hyd. Tpeak (min)	11.84 (ii)	34.64 (ii)
Unit Hyd. peak (cms)	15.00	45.00
	0.08	0.03

		TOTALS
PEAK FLOW (cms)	2.03	2.25
TIME TO PEAK (hrs)	6.00	6.50
RUNOFF VOLUME (mm)	71.10	14.28
TOTAL RAINFALL (mm)	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.20

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES;
 CN* = 45.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0358) Area (ha)= 429.87 Curve Number (CN)= 35.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.03

Unit Hyd Qpeak (cms)= 7.091
 PEAK FLOW (cms)= 1.611 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 8.508
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.116

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0360) Area (ha)= 138.37 Curve Number (CN)= 46.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.60

Unit Hyd Qpeak (cms)= 3.957
 PEAK FLOW (cms)= 1.171 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 12.385
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.169

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8306)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0358):	429.87	1.611	7.50	8.51
+ ID2= 2 (0360):	138.37	1.171	6.75	12.38
ID = 3 (8306):	568.24	2.716	7.00	9.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6362) | Routing time step (min)*= 15.00
 | IN= 2--> OUT= 1 |

Distance	Elevation	Manning
0.00	261.46	0.0550
27.86	254.23	0.0550
51.07	251.96	0.0550
74.29	250.77	0.0550
97.50	249.91	0.0550

125.36	249.40	0.0550	
150.93	247.40	0.0550	
155.93	247.33	0.0550 / 0.0350	Main Channel
157.93	246.85	0.0350	Main Channel
159.18	246.65	0.0350	Main Channel
160.18	246.63	0.0350	Main Channel
160.93	246.85	0.0350	Main Channel
161.93	247.18	0.0350 / 0.0550	Main Channel
163.18	248.03	0.0550	
168.18	248.58	0.0550	
183.18	250.18	0.0550	
201.97	252.59	0.0550	
213.57	256.02	0.0550	
225.18	260.31	0.0550	
229.82	261.00	0.0550	

332.60	222.37	0.0370
450.00	230.00	0.0370

```

----- TRAVEL TIME TABLE -----
DEPTH    ELEV    VOLUME    FLOW RATE    VELOCITY    TRAV.TIME
(m)      (m)      (cu.m.)   (cms)         (m/s)       (min)
-----
0.55    247.18    556E+04    1.9           1.03        49.95
1.28    247.91    1336E+05   16.7          1.53        33.61
2.00    248.63    913E+05    53.6          1.81        28.40
2.73    249.36    185E+06    122.3         2.05        25.18
3.46    250.09    348E+06    224.6         2.02        25.52
4.19    250.82    574E+06    417.9         2.25        22.91
4.91    251.54    855E+06    699.1         2.53        20.38
5.64    252.27    118E+07    1075.3        2.82        18.26
6.37    253.00    153E+07    1559.9        3.15        16.38
7.10    253.73    191E+07    2137.6        3.46        14.89
7.82    254.45    231E+07    2815.2        3.77        13.67
8.55    255.18    272E+07    3608.5        4.10        12.58
9.28    255.91    313E+07    4488.2        4.41        11.69
10.01   256.64    358E+07    5458.0        4.71        10.95
10.73   257.36    403E+07    6512.4        4.99        10.32
11.46   258.09    449E+07    7649.8        5.27        9.78
12.19   258.82    496E+07    8869.5        5.53        9.32
12.92   259.55    544E+07    10171.2       5.78        8.91
13.64   260.27    593E+07    11554.8       6.02        8.55

```

```

----- hydrograph ----- <-pipe / channel->
AREA      QPEAK    TPEAK    R.V.    MAX DEPTH  MAX VEL
(ha)      (cms)    (hrs)    (mm)    (m)         (m/s)
-----
INFLOW : ID= 2 (8306) 568.24  2.72  7.00  9.45  0.59  1.05
OUTFLOW: ID= 1 (6362) 568.24  2.40  8.25  9.45  0.58  1.04

```

```

----- ADD HYD (8304) -----
| 1 + 2 = 3 |
AREA      QPEAK    TPEAK    R.V.
(ha)      (cms)    (hrs)    (mm)
-----
ID1= 1 (0362): 118.78  2.946  6.00  18.83
+ ID2= 2 (6362): 568.24  2.400  8.25  9.45
-----
ID = 3 (8304): 687.02  3.953  6.50  11.07

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

----- ADD HYD (8300) -----
| 1 + 2 = 3 |
AREA      QPEAK    TPEAK    R.V.
(ha)      (cms)    (hrs)    (mm)
-----
ID1= 1 (8302): 617.89  5.710  7.25  19.52
+ ID2= 2 (8304): 687.02  3.953  6.50  11.07
-----
ID = 3 (8300): 1304.91  9.215  7.00  15.07

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

----- ROUTE CHN (6368) -----
| IN= 2--> OUT= 1 |
Routing time step (min)= 15.00

```

```

----- DATA FOR SECTION (3681.0) -----
Distance  Elevation  Manning
-----
0.00      230.00     0.0370
18.48     223.26     0.0370
36.96     223.05     0.0370
64.67     222.94     0.0370
110.87    222.86     0.0370
133.96    222.74     0.0370
147.82    222.65     0.0370
170.92    222.31     0.0370
174.79    222.26     0.0370 / 0.0300 Main Channel
174.89    221.86     0.0300 Main Channel
175.54    221.86     0.0300 Main Channel
176.19    221.86     0.0300 Main Channel
176.29    222.26     0.0300 / 0.0370 Main Channel
180.16    222.25     0.0370
184.78    222.28     0.0370
189.40    222.31     0.0370

```

```

----- TRAVEL TIME TABLE -----
DEPTH    ELEV    VOLUME    FLOW RATE    VELOCITY    TRAV.TIME
(m)      (m)      (cu.m.)   (cms)         (m/s)       (min)
-----
0.40    222.26    771E+03    0.3           0.52        41.74
0.81    222.67    784E+05    30.3          0.51        43.17
1.21    223.07    210E+06    122.2         0.77        28.42
1.62    223.48    384E+06    301.0         1.03        21.25
2.03    223.89    563E+06    550.2         1.28        17.06
2.44    224.30    747E+06    860.7         1.51        14.46
2.84    224.70    934E+06    1228.1        1.73        12.68
3.25    225.11    113E+07    1649.3        1.92        11.37
3.66    225.52    132E+07    2122.3        2.11        10.37
4.07    225.93    152E+07    2645.5        2.29        9.58
4.47    226.33    172E+07    3217.8        2.48        8.92
4.88    226.74    193E+07    3838.3        2.61        8.38
5.29    227.15    214E+07    4506.3        2.76        7.92
5.70    227.56    236E+07    5221.3        2.91        7.52
6.10    227.96    258E+07    5983.0        3.05        7.17
6.51    228.37    280E+07    6790.9        3.19        6.87
6.92    228.78    303E+07    7645.0        3.32        6.60
7.33    229.19    326E+07    8545.1        3.45        6.35
7.73    229.59    349E+07    9491.0        3.57        6.13

```

```

----- hydrograph ----- <-pipe / channel->
AREA      QPEAK    TPEAK    R.V.    MAX DEPTH  MAX VEL
(ha)      (cms)    (hrs)    (mm)    (m)         (m/s)
-----
INFLOW : ID= 2 (8300) 1304.91  9.22  7.00  15.07  0.52  0.52
OUTFLOW: ID= 1 (6368) 1304.91  8.43  8.25  15.07  0.51  0.52

```

```

----- ADD HYD (8298) -----
| 1 + 2 = 3 |
AREA      QPEAK    TPEAK    R.V.
(ha)      (cms)    (hrs)    (mm)
-----
ID1= 1 (0368): 159.48  0.838  7.75  12.55
+ ID2= 2 (6368): 1304.91  8.430  8.25  15.07
-----
ID = 3 (8298): 1464.39  9.262  8.00  14.80

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

----- ADD HYD (8296) -----
| 1 + 2 = 3 |
AREA      QPEAK    TPEAK    R.V.
(ha)      (cms)    (hrs)    (mm)
-----
ID1= 1 (0370): 191.85  2.560  6.75  20.95
+ ID2= 2 (8298): 1464.39  9.262  8.00  14.80
-----
ID = 3 (8296): 1656.24  11.367  7.75  15.51

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

----- ROUTE CHN (6372) -----
| IN= 2--> OUT= 1 |
Routing time step (min)= 15.00

```

```

----- DATA FOR SECTION (3721.0) -----
Distance  Elevation  Manning
-----
0.00      225.00     0.0390
30.80     219.38     0.0390
61.61     219.30     0.0390
77.01     219.27     0.0390
469.76     219.14     0.0390
477.46     219.13     0.0390
485.16     219.10     0.0390
492.86     219.09     0.0390
495.56     219.09     0.0390 / 0.0310 Main Channel
495.66     218.51     0.0310 Main Channel
500.56     218.51     0.0310 Main Channel
505.46     218.51     0.0310 Main Channel
505.56     219.09     0.0310 / 0.0390 Main Channel
508.26     219.09     0.0390
515.96     219.10     0.0390
523.67     219.21     0.0390
562.17     219.32     0.0390
654.58     219.43     0.0390
731.59     219.46     0.0390
762.39     225.00     0.0390

```

```

----- TRAVEL TIME TABLE -----
DEPTH    ELEV    VOLUME    FLOW RATE    VELOCITY    TRAV.TIME
(m)      (m)      (cu.m.)   (cms)         (m/s)       (min)
-----
0.29    218.80    431E+04    1.7           0.58        43.48
0.58    219.09    867E+04    5.1           0.89        28.32
0.93    219.44    200E+06    58.2          0.44        57.35
1.28    219.79    568E+06    278.6         0.74        33.99
1.62    220.13    939E+06    630.2         1.01        24.84

```

1.97	220.48	.131E+07	1090.4	1.25	20.05
2.32	220.83	.169E+07	1647.7	1.47	17.06
2.67	221.18	.206E+07	2294.6	1.68	14.99
3.01	221.52	.244E+07	3025.6	1.87	13.46
3.36	221.87	.282E+07	3836.5	2.05	12.27
3.71	222.22	.321E+07	4723.7	2.22	11.31
4.06	222.57	.359E+07	5684.6	2.39	10.53
4.40	222.91	.398E+07	6716.6	2.55	9.87
4.75	223.26	.437E+07	7817.7	2.70	9.31
5.10	223.61	.476E+07	8986.2	2.85	8.83
5.45	223.96	.515E+07	10220.4	2.99	8.40
5.79	224.30	.553E+07	11519.1	3.13	8.03
6.14	224.65	.594E+07	12880.8	3.27	7.69
6.49	225.00	.634E+07	14304.5	3.40	7.39

----- hydrograph -----

<-pipe / channel->

AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL

(ha) (cms) (hrs) (mm) (m) (m/s)

INFLOW : ID= 2 (8296) 1656.24 11.37 7.75 15.51 0.62 0.79

OUTFLOW: ID= 1 (6372) 1656.24 10.94 8.50 15.51 0.62 0.80

| ADD HYD (8294) |

| 1 + 2 = 3 |

AREA QPEAK TPEAK R.V.

(ha) (cms) (hrs) (mm)

ID1= 1 (0372): 110.42 0.471 7.25 9.16

+ ID2= 2 (6372): 1656.24 10.944 8.50 15.51

=====

ID = 3 (8294): 1766.66 11.372 8.50 15.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ADD HYD (8292) |

| 1 + 2 = 3 |

AREA QPEAK TPEAK R.V.

(ha) (cms) (hrs) (mm)

ID1= 1 (0374): 545.70 3.687 8.25 20.01

+ ID2= 2 (8294): 1766.66 11.372 8.50 15.12

=====

ID = 3 (8292): 2312.36 15.040 8.50 16.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| RESERVOIR (5505) |

| IN= 2--> OUT= 1 |

| DT= 15.0 min |

OUTFLOW STORAGE | OUTFLOW STORAGE

(cms) (ha.m.) | (cms) (ha.m.)

0.0000 0.0000 | 65.1290 345.3754

25.4850 24.6697 | 84.9510 456.3890

31.1490 98.6787 | ***** 838.7689

39.6440 ***** | ***** 838.7789

48.1390 ***** | 0.0000 0.0000

AREA QPEAK TPEAK R.V.

(ha) (cms) (hrs) (mm)

INFLOW : ID= 2 (8292) 2312.360 15.040 8.50 16.27

OUTFLOW: ID= 1 (5505) 2312.360 11.011 11.25 16.27

PEAK FLOW REDUCTION [Qout/Qin](%)= 73.21

TIME SHIFT OF PEAK FLOW (min)=165.00

MAXIMUM STORAGE USED (ha.m.)= 10.6689

| ADD HYD (8272) |

| 1 + 2 = 3 |

AREA QPEAK TPEAK R.V.

(ha) (cms) (hrs) (mm)

ID1= 1 (0376): 463.85 6.058 7.50 29.20

+ ID2= 2 (5505): 2312.36 11.011 11.25 16.27

=====

ID = 3 (8272): 2776.21 14.844 10.25 18.43

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| CALIB |

| NASHYD (0396) | Area (ha)= 305.21 Curve Number (CN)= 69.0

| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50

| U.H. Tp(hrs)= 1.08

Unit Hyd Opeak (cms)= 4.811

PEAK FLOW (cms)= 3.390 (1)

TIME TO PEAK (hrs)= 7.500

RUNOFF VOLUME (mm)= 25.220

TOTAL RAINFALL (mm)= 73.100

RUNOFF COEFFICIENT = 0.345

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |

| NASHYD (0394) | Area (ha)= 325.45 Curve Number (CN)= 53.0

| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50

U.H. Tp(hrs)= 0.92

Unit Hyd Opeak (cms)= 6.013

PEAK FLOW (cms)= 2.485 (1)

TIME TO PEAK (hrs)= 7.250

RUNOFF VOLUME (mm)= 15.628

TOTAL RAINFALL (mm)= 73.100

RUNOFF COEFFICIENT = 0.214

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |

| NASHYD (0390) | Area (ha)= 420.00 Curve Number (CN)= 55.0

| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50

U.H. Tp(hrs)= 1.07

Unit Hyd Opeak (cms)= 6.683

PEAK FLOW (cms)= 3.048 (1)

TIME TO PEAK (hrs)= 7.500

RUNOFF VOLUME (mm)= 16.653

TOTAL RAINFALL (mm)= 73.100

RUNOFF COEFFICIENT = 0.228

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |

| NASHYD (0388) | Area (ha)= 220.77 Curve Number (CN)= 58.0

| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50

U.H. Tp(hrs)= 0.99

Unit Hyd Opeak (cms)= 3.819

PEAK FLOW (cms)= 1.875 (1)

TIME TO PEAK (hrs)= 7.250

RUNOFF VOLUME (mm)= 18.209

TOTAL RAINFALL (mm)= 73.100

RUNOFF COEFFICIENT = 0.249

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |

| NASHYD (0386) | Area (ha)= 241.27 Curve Number (CN)= 61.0

| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50

U.H. Tp(hrs)= 0.90

Unit Hyd Opeak (cms)= 4.562

PEAK FLOW (cms)= 2.408 (1)

TIME TO PEAK (hrs)= 7.250

RUNOFF VOLUME (mm)= 19.881

TOTAL RAINFALL (mm)= 73.100

RUNOFF COEFFICIENT = 0.272

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| ADD HYD (8286) |

| 1 + 2 = 3 |

AREA QPEAK TPEAK R.V.

(ha) (cms) (hrs) (mm)

ID1= 1 (0386): 241.27 2.408 7.25 19.88

+ ID2= 2 (0386): 220.77 1.875 7.25 18.21

=====

ID = 3 (8286): 462.04 4.284 7.25 19.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| CALIB |

| NASHYD (0384) | Area (ha)= 199.07 Curve Number (CN)= 44.0

| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50

U.H. Tp(hrs)= 0.96

Unit Hyd Opeak (cms)= 3.537

PEAK FLOW (cms)= 1.095 (1)


```

-----
      (ha)   (cms)   (hrs)   (mm)
ID1= 1 (0378): 606.72 4.102 7.75 16.67
+ ID2= 2 (0392): 167.22 2.002 7.00 20.40
-----
ID = 3 (8282): 773.94 5.981 7.25 17.48

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8278) |
| 1 + 2 = 3 |
      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)   (hrs)   (mm)
-----
ID1= 1 (8280): 1479.71 11.051 7.50 15.77
+ ID2= 2 (8282): 773.94 5.981 7.25 17.48
-----
ID = 3 (8278): 2253.65 17.022 7.50 16.36

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6394) |
| IN= 2---> OUT= 1 | Routing time step (min)'= 15.00
-----

```

```

-----
<----- DATA FOR SECTION (3941.0) ----->
Distance      Elevation      Manning
-----
0.00          283.00         0.0380
13.48        282.87         0.0380
53.92        280.08         0.0380
74.13        276.42         0.0380
97.72        265.45         0.0380
114.57       256.93         0.0380
131.42       253.04         0.0380
134.79       252.58         0.0380
138.53       251.74         0.0380 /0.0300 Main Channel
139.03       251.20         0.0300 Main Channel
141.53       251.20         0.0300 Main Channel
144.03       251.20         0.0300 Main Channel
144.53       251.74         0.0300 /0.0380 Main Channel
148.27       252.69         0.0380
151.64       252.97         0.0380
185.34       255.08         0.0380
219.03       257.54         0.0380
262.84       259.43         0.0380
310.02       262.80         0.0380
333.60       283.00         0.0380

```

```

-----
<----- TRAVEL TIME TABLE ----->
DEPTH      ELEV      VOLUME      FLOW RATE      VELOCITY      TRAV.TIME
(m)        (m)        (cu.m.)     (cms)          (m/s)        (min)
-----
0.54       251.74     .827E+04     4.5            1.51         30.71
2.19       253.39     .758E+05     73.2          2.69         17.27
3.83       255.03     .282E+06     331.8         3.27         14.17
5.48       256.68     .633E+06     917.3         4.03         11.51
7.12       258.32     .112E+07     1889.5        4.69         9.90
8.77       259.97     .179E+07     3410.2        5.31         8.73
10.41      261.61     .259E+07     5627.6        6.05         7.66
12.06      263.26     .351E+07     8605.1        6.83         6.79
13.70      264.90     .447E+07     12544.9       7.81         5.94
15.35      266.55     .548E+07     17103.4       8.71         5.32
16.99      268.19     .648E+07     22251.7       9.56         4.85
18.64      269.84     .752E+07     27979.0       10.36        4.48
20.28      271.48     .858E+07     34269.0       11.11        4.17
21.93      273.13     .967E+07     41110.8       11.83        3.92
23.57      274.77     .108E+08     48495.9       12.51        3.71
25.22      276.42     .119E+08     56418.1       13.17        3.52
26.86      278.06     .131E+08     64410.6       13.69        3.39
28.51      279.71     .143E+08     73002.1       14.18        3.27
30.15      281.35     .156E+08     81379.3       14.50        3.20

```

```

-----
<----- hydrograph -----> <-pipe / channel->
AREA      QPEAK      TPEAK      R.V.      MAX DEPTH  MAX VEL
(ha)      (cms)      (hrs)      (mm)      (m)        (m/s)
-----
INFLOW: ID= 2 (8278) 2253.65 17.02 7.50 16.36 0.84 1.64
OUTFLOW: ID= 1 (6394) 2253.65 16.55 8.00 16.36 0.83 1.64

```

```

-----
| ADD HYD (8276) |
| 1 + 2 = 3 |
      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)   (hrs)   (mm)
-----
ID1= 1 (0394): 325.45 2.485 7.25 15.63
+ ID2= 2 (6394): 2253.65 16.553 8.00 16.36
-----
ID = 3 (8276): 2579.10 18.926 8.00 16.26

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6396) |

```

```

| IN= 2---> OUT= 1 | Routing time step (min)'= 15.00

```

```

-----
<----- DATA FOR SECTION (3961.0) ----->
Distance      Elevation      Manning
-----
0.00          263.00         0.0410
11.75        257.14         0.0410
23.50        253.97         0.0410
41.13        247.83         0.0410
76.38        232.09         0.0410
135.13       229.07         0.0410
149.82       228.97         0.0410
152.75       228.96         0.0410 /0.0300 Main Channel
154.19       228.73         0.0300 Main Channel
154.69       228.20         0.0300 Main Channel
155.69       228.20         0.0300 Main Channel
156.69       228.20         0.0300 Main Channel
157.19       228.73         0.0300 Main Channel
158.63       228.95         0.0300 /0.0410 Main Channel
161.57       228.96         0.0410
164.51       229.71         0.0410
196.82       241.70         0.0410
223.26       249.21         0.0410
246.76       255.13         0.0410
290.82       263.51         0.0410

```

```

-----
<----- TRAVEL TIME TABLE ----->
DEPTH      ELEV      VOLUME      FLOW RATE      VELOCITY      TRAV.TIME
(m)        (m)        (cu.m.)     (cms)          (m/s)        (min)
-----
0.75       228.95     .780E+04     2.2           0.98         58.06
2.54       230.74     .277E+06     145.1         1.78         31.76
4.33       232.53     .786E+06     609.1         2.63         21.50
6.13       234.33     .140E+07     1481.5        3.59         15.77
7.92       236.12     .207E+07     2677.3        4.39         12.90
9.71       237.91     .280E+07     4184.8        5.09         11.14
11.50      239.70     .357E+07     6001.4        5.71         9.93
13.29      241.49     .441E+07     8129.1        6.27         9.03
15.09      243.29     .530E+07     10549.4       6.77         8.37
16.88      245.08     .625E+07     13300.6       7.24         7.83
18.67      246.87     .725E+07     16396.2       7.68         7.38
20.46      248.66     .834E+07     19809.8       8.07         7.02
22.26      250.46     .949E+07     23532.5       8.43         6.72
24.05      252.25     .107E+08     27641.2       8.77         6.46
25.84      254.04     .120E+08     32155.2       9.10         6.22
27.63      255.83     .134E+08     36901.0       9.37         6.05
29.42      257.62     .149E+08     42147.7       9.64         5.88
31.22      259.42     .164E+08     48149.1       9.97         5.68
33.01      261.21     .181E+08     54644.8       10.29        5.51

```

```

-----
<----- hydrograph -----> <-pipe / channel->
AREA      QPEAK      TPEAK      R.V.      MAX DEPTH  MAX VEL
(ha)      (cms)      (hrs)      (mm)      (m)        (m/s)
-----
INFLOW: ID= 2 (8276) 2579.10 18.93 8.00 16.26 0.96 1.03
OUTFLOW: ID= 1 (6396) 2579.10 17.09 9.00 16.26 0.93 1.02

```

```

-----
| ADD HYD (8274) |
| 1 + 2 = 3 |
      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)   (hrs)   (mm)
-----
ID1= 1 (0396): 305.21 3.390 7.50 25.22
+ ID2= 2 (6396): 2579.10 17.092 9.00 16.26
-----
ID = 3 (8274): 2884.31 20.021 9.00 17.21

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8270) |
| 1 + 2 = 3 |
      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)   (hrs)   (mm)
-----
ID1= 1 (8272): 2776.21 14.844 10.25 18.43
+ ID2= 2 (8274): 2884.31 20.021 9.00 17.21
-----
ID = 3 (8270): 5660.52 34.206 9.25 17.81

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| RESERVOIR (5506) |
| IN= 2---> OUT= 1 |
| DT= 15.0 min |
      OUTFLOW  STORAGE | OUTFLOW  STORAGE
      (cms)   (ha.m.) | (cms)   (ha.m.)
-----
0.0000     0.0000 | 0.0000  60.8810
31.1490    24.6697 | 56.2770  900.4431
36.8120    37.0045 | 96.6770  900.4531
45.3070    86.3439 | 0.0000  0.0000

```

```

-----
AREA      QPEAK      TPEAK      R.V.
(ha)      (cms)      (hrs)      (mm)
-----
INFLOW: ID= 2 (8270) 5660.52 34.206 9.25 17.81

```

OUTFLOW: ID= 1 (5506) 5660.520 28.236 11.75 17.81

PEAK FLOW REDUCTION [Qout/Qin](%)= 82.54
TIME SHIFT OF PEAK FLOW (min)=150.00
MAXIMUM STORAGE USED (ha.m.)= 22.3717

CALIB
NASHYD (0406) Area (ha)= 142.65 Curve Number (CN)= 66.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.59

Unit Hyd Qpeak (cms)= 4.135

PEAK FLOW (cms)= 2.315 (I)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 22.790
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.312

(I) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0404) Area (ha)= 246.46 Curve Number (CN)= 47.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.98

Unit Hyd Qpeak (cms)= 4.280

PEAK FLOW (cms)= 1.475 (I)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 12.944
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.177

(I) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0402) Area (ha)= 244.00 Curve Number (CN)= 61.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 3.879

PEAK FLOW (cms)= 2.133 (I)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 19.936
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.273

(I) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0400) Area (ha)= 93.97 Curve Number (CN)= 52.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.44

Unit Hyd Qpeak (cms)= 3.630

PEAK FLOW (cms)= 1.222 (I)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 14.807
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.203

(I) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0398) Area (ha)= 328.19 Curve Number (CN)= 55.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.83

Unit Hyd Qpeak (cms)= 6.759

PEAK FLOW (cms)= 2.909 (I)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 16.581
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.227

(I) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8268)

1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0398): 328.19 2.909 7.00 16.58
+ ID2= 2 (0400): 93.97 1.222 6.50 14.81
ID = 3 (8268): 422.16 4.012 6.75 16.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6402) |
ID= 2 -> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (4021.0) ----->

Distance	Elevation	Manning
0.00	218.50	0.0360
11.50	238.00	0.0360
23.00	237.93	0.0360
34.49	236.39	0.0360
63.24	233.98	0.0360
97.73	228.15	0.0360
123.60	227.08	0.0360
126.48	226.61	0.0360
127.60	226.47	0.0360 / 0.0330
127.85	225.25	0.0330
129.35	225.25	0.0330
130.85	225.25	0.0330
131.10	226.47	0.0330 / 0.0360
132.22	226.59	0.0360
143.72	227.42	0.0360
169.59	227.88	0.0360
192.59	231.19	0.0360
218.46	233.02	0.0360
241.45	235.50	0.0360
284.57	236.43	0.0360

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.41	225.66	.391E+04	1.7	1.36	38.31
0.81	226.06	.804E+04	5.0	1.93	27.02
1.22	226.47	1.24E+05	9.1	2.30	22.61
1.84	227.09	3.08E+05	23.3	2.36	22.03
2.46	227.71	9.04E+05	62.0	2.14	24.31
3.09	228.34	2.23E+06	188.2	2.63	19.75
3.71	228.96	3.79E+06	410.2	3.38	15.39
4.33	229.58	5.50E+06	711.5	4.04	12.89
4.95	230.20	7.37E+06	1091.3	4.62	11.25
5.58	230.83	9.39E+06	1550.6	5.15	10.10
6.20	231.45	1.16E+07	2072.4	5.59	9.31
6.82	232.07	1.40E+07	2659.2	5.93	8.77
7.44	232.69	1.67E+07	3345.8	6.27	8.30
8.07	233.32	1.95E+07	4159.0	6.64	7.83
8.69	233.94	2.26E+07	5100.8	7.03	7.39
9.31	234.56	2.59E+07	6081.3	7.32	7.11
9.93	235.18	2.95E+07	7183.1	7.60	6.84
10.56	235.81	3.34E+07	8345.2	7.62	6.83
11.18	236.43	3.79E+07	9116.0	7.51	6.92

<----- hydrograph -----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8268) 422.16 4.01 6.75 16.19 0.70 1.72
OUTFLOW: ID= 1 (6402) 422.16 3.72 7.50 16.19 0.66 1.66

ADD HYD (8266) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0402): 244.00 2.133 7.50 19.94
+ ID2= 2 (6402): 422.16 3.724 7.50 16.19
ID = 3 (8266): 666.16 5.857 7.50 17.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8264) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0404): 246.46 1.475 7.25 12.94
+ ID2= 2 (8266): 666.16 5.857 7.50 17.56
ID = 3 (8264): 912.62 7.330 7.50 16.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8262) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
ID1= 1 (0406): 142.65 2.315 6.50 22.79
+ ID2= 2 (8264): 812.62 7.330 7.50 16.31
-----
ID = 3 (8262): 1055.27 9.418 7.25 17.19

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8260) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
ID1= 1 (5506): 5660.52 28.236 11.75 17.81
+ ID2= 2 (8262): 1055.27 9.418 7.25 17.19
-----
ID = 3 (8260): 6715.79 32.331 11.25 17.71

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8258) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
ID1= 1 (0408): 231.62 2.757 6.75 18.04
+ ID2= 2 (8260): 6715.79 32.331 11.25 17.71
-----
ID = 3 (8258): 6947.41 33.171 11.00 17.72

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8256) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
ID1= 1 (0410): 572.01 2.643 8.25 13.43
+ ID2= 2 (8258): 6947.41 33.171 11.00 17.72
-----
ID = 3 (8256): 7519.42 35.308 10.75 17.40

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| RESERVOIR (5507) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
| OUTFLOW STORAGE | OUTFLOW STORAGE |
| (cms) (ha.m.) | (cms) (ha.m.) |
0.0000 0.0000 | 90.6140 67.8416
39.6440 12.3348 | ***** 160.3529
48.1390 18.5023 | ***** 160.3629
67.9600 37.0045 | 0.0000 0.0000
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
INFLOW : ID= 2 (8256) 7519.421 35.308 10.75 17.40
OUTFLOW : ID= 1 (5507) 7519.421 34.570 11.75 17.40
-----
| PEAK FLOW REDUCTION [Qout/Qin](%)= 97.91 |
| TIME SHIFT OF PEAK FLOW (min)= 60.00 |
| MAXIMUM STORAGE USED (ha.m.)= 10.7597 |

```

```

-----
| CALIB (0420) |
| NASHYD (0420) | Area (ha)= 175.82 Curve Number (CN)= 53.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.81
-----
Unit Hyd Qpeak (cms)= 3.692
PEAK FLOW (cms)= 1.485 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 15.590
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.213

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0418) |
| NASHYD (0418) | Area (ha)= 182.79 Curve Number (CN)= 64.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.05
-----
Unit Hyd Qpeak (cms)= 2.966

```

```

PEAK FLOW (cms)= 1.780 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 21.774
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.298

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0416) |
| NASHYD (0416) | Area (ha)= 439.30 Curve Number (CN)= 64.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.29
-----
Unit Hyd Qpeak (cms)= 5.832

```

```

PEAK FLOW (cms)= 3.686 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 21.827
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.299

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0412) |
| NASHYD (0412) | Area (ha)= 238.70 Curve Number (CN)= 54.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.80
-----
Unit Hyd Qpeak (cms)= 5.088

```

```

PEAK FLOW (cms)= 2.105 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 16.071
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.220

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (4141) |
| STANDHYD (4141) | Area (ha)= 43.70
| ID= 1 DT=15.0 min | Total Imp(%)= 36.00 Dir. Conn.(%)= 17.00
-----

```

```

-----
| IMPERVIOUS PERVIOUS (i) |
| Surface Area (ha)= 15.73 27.97 |
| Dep. Storage (mm)= 2.00 5.00 |
| Average Slope (%)= 0.50 0.50 |
| Length (m)= 539.75 40.00 |
| Mannings n = 0.013 0.250 |
-----
Max. Eff. Inten. (mm/hr)= 96.49 52.82
over (min) 15.00 30.00
Storage Coeff. (min)= 8.77 (ii) 22.58 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.09 0.04
-----
PEAK FLOW (cms)= 1.74 1.96 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.25 2.978 (iii)
RUNOFF VOLUME (mm)= 71.10 26.72 34.27
TOTAL RAINFALL (mm)= 73.10 73.10 73.10
RUNOFF COEFFICIENT = 0.97 0.37 0.47

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| RESERVOIR (5414) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
| OUTFLOW STORAGE | OUTFLOW STORAGE |
| (cms) (ha.m.) | (cms) (ha.m.) |
0.0000 0.0000 | 3.4720 1.9177
0.3820 0.3152 | 3.8720 1.9277
1.8280 0.4690 | 0.0000 0.0000
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
INFLOW : ID= 2 (4141) 43.700 2.978 6.00 34.27
OUTFLOW : ID= 1 (5414) 43.700 1.837 6.50 34.26

```

PEAK FLOW REDUCTION [Qout/Qin](%)= 61.71
TIME SHIFT OF PEAK FLOW (min)= 30.00

MAXIMUM STORAGE USED (ha.m.)= 0.4849

CALLS STANHYD (4142) Area (ha)= 144.30 Dir. Conn.(%)= 10.00

Surface Area (ha)= 30.30 IMPERVIOUS 114.00
Dep. Storage (mm)= 2.00 PERVIOUS (i) 5.00
Average Slope (%)= 0.50

Max. Eff. Inten. (mm/hr)= 96.49 25.80
cover (min)= 15.00 45.00
Storage Coeff. (min)= 12.55 (ii) 30.95 (ii)
Unit Hyd. Tpeak (min)= 15.00 45.00

PEAK FLOW (cms)= 3.02 4.96 5.721 (iii)
TIME TO PEAK (hrs)= 6.00 6.50 6.50
RUNOFF VOLUME (mm)= 73.10 24.32 29.00
TOTAL RAINFALL (mm)= 73.10 73.10 73.10

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8254)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (4142): 144.30 5.721 6.50 29.00
+ ID2= 2 (5414): 43.70 1.837 6.50 34.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8252)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0412): 238.70 2.105 7.00 16.07
+ ID2= 2 (8251): 189.00 7.558 6.50 30.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6416)
IN= 2--> OUT= 1 Routing time step (min)= 15.00

Distance Elevation Manning
0.00 270.07 0.0340
20.67 267.91 0.0340
62.01 264.33 0.0340
113.69 259.75 0.0340
165.37 253.30 0.0340
227.38 246.29 0.0340
232.55 246.03 0.0340
237.72 246.16 0.0340
241.39 246.02 0.0340 /0.0300 Main Channel
241.64 245.75 0.0300 Main Channel
242.89 245.75 0.0300 Main Channel
244.14 245.75 0.0300 Main Channel
244.39 246.02 0.0300 /0.0340 Main Channel
248.06 246.20 0.0340
253.22 246.28 0.0340
258.39 246.63 0.0340
346.25 252.37 0.0340
413.43 257.77 0.0340
465.11 261.78 0.0340
511.62 270.00 0.0340

TRAVEL TIME TABLE
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.27 246.02 .295E+04 0.6 0.75 88.26

1.53 247.28 .168E+06 69.7 1.65 40.13
2.79 248.54 .490E+06 299.5 2.42 27.28
4.06 249.81 .962E+06 740.1 3.05 21.67
5.32 251.07 .158E+07 1440.0 3.61 18.33
6.58 252.33 .235E+07 2443.8 4.12 16.06
7.84 253.59 .327E+07 3811.2 4.63 14.30
9.10 254.85 .432E+07 5565.1 5.11 12.94
10.37 256.12 .550E+07 7721.3 5.57 11.88
11.63 257.38 .682E+07 10309.9 6.00 11.02
12.89 258.64 .827E+07 13360.6 6.42 10.31
14.15 259.90 .985E+07 16884.5 6.81 9.72
15.42 261.17 .116E+08 20818.4 7.14 9.26
16.68 262.43 .134E+08 25510.4 7.53 8.78
17.94 263.69 .154E+08 30958.0 7.97 8.30
19.20 264.95 .175E+08 36988.3 8.38 7.89
20.46 266.21 .197E+08 43617.1 8.78 7.53
21.73 267.48 .220E+08 50868.1 9.17 7.22
22.99 268.74 .244E+08 58854.0 9.56 6.92

hydrograph
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW: ID= 2 (8252) 426.70 9.54 6.50 22.31 0.43 0.81
OUTFLOW: ID= 1 (6416) 426.70 4.90 7.50 22.31 0.35 0.78

ADD HYD (8250)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0416): 439.30 3.686 8.00 21.83
+ ID2= 2 (6416): 426.70 4.904 7.50 22.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8248)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0418): 182.79 1.780 7.50 21.77
+ ID2= 2 (8250): 866.00 8.541 7.50 22.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8246)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0420): 175.82 1.485 7.00 15.59
+ ID2= 2 (8248): 1048.79 10.321 7.50 22.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8244)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (5507): 7519.42 34.570 11.75 17.40
+ ID2= 2 (8245): 1224.61 11.759 7.50 22.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8242)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0422): 780.20 3.488 9.00 16.24
+ ID2= 2 (8244): 8744.03 40.750 11.00 17.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5508)
IN= 2--> OUT= 1

DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	*****	197.3574
	76.4550	30.8371	*****	394.7148
	*****	61.5742	*****	394.7248
	*****	*****	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8242)	9524.231	43.985	11.00	17.78
OUTFLOW: ID= 1 (5508)	9524.231	42.683	12.25	17.78

PEAK FLOW REDUCTION [Qout/Qin]= 97.04
 TIME SHIFT OF PEAK FLOW (min)= 75.00
 MAXIMUM STORAGE USED (ha.m.)= 17.2250

CALIB (0336)	Area (ha)	Curve Number (CN)	R.V. (mm)
NASHYD	2785.00	72.0	6.00
ID= 1 DT=15.0 min	IA (mm)= 5.00	# of Linear Res.(N)= 1.50	
	U.H. Tp(hrs)= 15.39		

Unit Hyd Qpeak (cms)= 3.090
 PEAK FLOW (cms)= 3.370 (i)
 TIME TO PEAK (hrs)= 22.250
 RUNOFF VOLUME (mm)= 27.751
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.380

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3382)	Area (ha)	Total Imp(%)	Dir. Conn.(%)
ID= 1 DT=15.0 min	525.90	37.00	17.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	194.58	331.32
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1872.43	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten.(mm/hr)=	96.49	57.38
over (min)	15.00	45.00
Storage Coeff. (min)=	18.51 (ii)	31.86 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.06	0.03
	TOTALS	
PEAK FLOW (cms)=	15.86	19.45
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	71.10	28.48
TOTAL RAINFALL (mm)=	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.39

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3381)	Area (ha)	Total Imp(%)	Dir. Conn.(%)
ID= 1 DT=15.0 min	73.00	49.00	23.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	35.77	37.23
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	697.61	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten.(mm/hr)=	96.49	72.01
over (min)	15.00	30.00
Storage Coeff. (min)=	10.23 (ii)	22.43 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.04
	TOTALS	
PEAK FLOW (cms)=	3.77	3.58
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	71.10	31.13
TOTAL RAINFALL (mm)=	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

CN* = 66.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5338)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1	0.0000	0.0000	3.3350	1.2400
DT= 15.0 min	0.3230	0.3074	4.3560	1.3520
	0.9360	0.5374	5.3290	1.4590
	1.7590	0.8302	5.7290	1.4690
	2.4110	1.0550	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (3381)	73.000	6.061	6.00	40.32
OUTFLOW: ID= 1 (5338)	73.000	2.647	6.75	40.32

PEAK FLOW REDUCTION [Qout/Qin]= 43.67
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 1.1258

ADD HYD (8310)	1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (3382):		525.90	25.210	6.50	35.73
+ ID2= 2 (5338):		73.00	2.647	6.75	40.32

ID = 3 (8310):		598.90	27.843	6.50	36.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (3342)	Area (ha)	Curve Number (CN)	R.V. (mm)
ID= 1 DT=15.0 min	587.10	56.0	6.00
	IA (mm)= 5.00	# of Linear Res.(N)= 1.50	
	U.H. Tp(hrs)= 8.19		

Unit Hyd Qpeak (cms)= 1.224
 PEAK FLOW (cms)= 0.825 (i)
 TIME TO PEAK (hrs)= 15.250
 RUNOFF VOLUME (mm)= 17.318
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.237

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3341)	Area (ha)	Total Imp(%)	Dir. Conn.(%)
ID= 1 DT=15.0 min	33.00	51.00	31.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	16.83	16.17
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	469.04	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten.(mm/hr)=	96.49	43.28
over (min)	15.00	30.00
Storage Coeff. (min)=	8.06 (ii)	23.02 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.10	0.04
	TOTALS	
PEAK FLOW (cms)=	2.45	0.92
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	71.10	20.49
TOTAL RAINFALL (mm)=	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.28

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 52.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5334)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1				
DT= 15.0 min				

0.0000	0.0000	0.6540	0.4604
0.0390	0.1699	0.8330	0.5249
0.1860	0.2610	1.0130	0.5896
0.3750	0.3534	1.4130	0.5996
0.4340	0.3813	0.0000	0.0000

INFLW : ID= 2 (3341) 33.000 3.022 6.00 36.18
 OUTFLOW: ID= 1 (5334) 33.000 0.928 6.50 36.15

PEAK FLOW REDUCTION [Qout/Qin](%)= 30.69
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 0.5651

ADD HYD (8314)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (3342): 587.10 0.825 15.25 17.32
 + ID2= 2 (5334): 33.00 0.928 6.50 36.15
 ID = 3 (8314): 620.10 1.209 6.75 18.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0306)
 NASHYD (0306) Area (ha)= 283.97 Curve Number (CN)= 52.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 6.44

Unit Hyd Qpeak (cms)= 0.753

PEAK FLOW (cms)= 0.445 (1)
 TIME TO PEAK (hrs)= 13.750
 RUNOFF VOLUME (mm)= 15.318
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.210

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0286)
 NASHYD (0286) Area (ha)= 260.51 Curve Number (CN)= 84.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.16

Unit Hyd Qpeak (cms)= 3.834

PEAK FLOW (cms)= 4.421 (1)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 39.488
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.540

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0282)
 NASHYD (0282) Area (ha)= 449.38 Curve Number (CN)= 77.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.47

Unit Hyd Qpeak (cms)= 5.226

PEAK FLOW (cms)= 5.078 (1)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 32.027
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.438

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0284)
 NASHYD (0284) Area (ha)= 78.93 Curve Number (CN)= 84.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.57

Unit Hyd Qpeak (cms)= 2.344

PEAK FLOW (cms)= 2.351 (1)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 38.894
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.532

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8388)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0282): 449.38 5.078 8.00 32.03
 + ID2= 2 (0284): 78.93 2.351 6.50 38.89
 ID = 3 (8388): 528.31 6.886 7.25 33.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6286)
 IN= 2--> OUT= 1 Routing time step (min)= 15.00

DATA FOR SECTION (2861.0)			
Distance	Elevation	Manning	
0.00	233.00	0.0450	
20.58	228.51	0.0450	
41.17	227.74	0.0450	
51.46	227.41	0.0450	
97.77	225.96	0.0450	
149.23	223.94	0.0450	
200.69	220.84	0.0450	
226.42	220.66	0.0450	
238.85	220.22	0.0450	
241.35	220.01	0.0450 / 0.0350	Main Channel
241.85	219.70	0.0350	Main Channel
245.85	219.72	0.0350	Main Channel
246.35	220.06	0.0350	Main Channel
248.85	220.23	0.0350	Main Channel
303.60	221.64	0.0350	Main Channel
380.79	224.98	0.0450	
432.25	229.54	0.0450	
457.98	233.33	0.0450	
483.71	234.27	0.0450	
509.44	233.81	0.0450	

TRAVEL TIME TABLE					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.31	220.01	.382E+04	0.8	0.57	82.89
1.06	220.76	.528E+05	15.1	0.66	72.51
1.81	221.51	.240E+06	88.6	1.05	45.21
2.56	222.26	.510E+06	261.7	1.46	32.47
3.31	223.01	.843E+06	533.8	1.81	26.34
4.06	223.76	.124E+07	913.6	2.10	22.64
4.81	224.51	.171E+07	1399.0	2.34	20.33
5.56	225.26	.225E+07	2051.0	2.60	18.27
6.31	226.01	.285E+07	2907.0	2.91	16.35
7.06	226.76	.352E+07	3902.5	3.16	15.04
7.82	227.52	.426E+07	5069.9	3.39	14.01
8.57	228.27	.507E+07	6433.0	3.62	13.13
9.32	229.02	.593E+07	8069.2	3.89	12.24
10.07	229.77	.681E+07	9926.7	4.16	11.44
10.82	230.52	.772E+07	11962.8	4.42	10.75
11.57	231.27	.864E+07	14167.1	4.68	10.17
12.32	232.02	.959E+07	16537.1	4.92	9.66
13.07	232.77	.105E+08	19070.7	5.16	9.22
13.82	233.52	.115E+08	21790.4	5.39	8.82

INFLW : ID= 2 (8388) 528.31 6.89 7.25 33.05 0.72 0.62
 OUTFLOW: ID= 1 (6286) 528.31 5.82 9.00 33.05 0.64 0.61

ADD HYD (8386)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0286): 260.51 4.421 7.50 39.49
 + ID2= 2 (6286): 528.31 5.824 9.00 33.05
 ID = 3 (8386): 788.82 9.844 8.50 35.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0302)
 NASHYD (0302) Area (ha)= 473.90 Curve Number (CN)= 58.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.66

Unit Hyd Qpeak (cms)= 4.874

PEAK FLOW (cms)= 2.715 (1)

TIME TO PEAK (hrs)= 8.500
RUNOFF VOLUME (mm)= 18.313
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.251

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0300) Area (ha)= 258.93 Curve Number (CN)= 52.0
ID= 1 DT=15.0 min Ia (mm)= 4.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.03

Unit Hyd Qpeak (cms)= 4.292

PEAK FLOW (cms)= 1.811 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 15.576
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.213

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0298) Area (ha)= 330.51 Curve Number (CN)= 45.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.26

Unit Hyd Qpeak (cms)= 4.461

PEAK FLOW (cms)= 1.541 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 12.163
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.166

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8395)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0298): 330.51 1.541 8.00 12.16
+ ID2= 2 (0300): 258.93 1.811 7.50 15.58
=====

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHYD (0288) Area (ha)= 340.83 Curve Number (CN)= 78.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 2.21

Unit Hyd Qpeak (cms)= 2.629

PEAK FLOW (cms)= 2.858 (i)
TIME TO PEAK (hrs)= 9.250
RUNOFF VOLUME (mm)= 33.083
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.453

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0290) Area (ha)= 269.18 Curve Number (CN)= 78.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 4.279

PEAK FLOW (cms)= 3.986 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 32.883
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.450

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8397)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0288): 340.83 2.858 9.25 33.08

+ ID2= 2 (0290): 269.18 3.986 7.25 32.88
=====

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHYD (0296) Area (ha)= 293.65 Curve Number (CN)= 76.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.13

Unit Hyd Qpeak (cms)= 4.437

PEAK FLOW (cms)= 3.926 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 31.003
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.424

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0292) Area (ha)= 738.49 Curve Number (CN)= 68.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.52

Unit Hyd Qpeak (cms)= 8.289

PEAK FLOW (cms)= 6.145 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 24.582
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.336

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0294) Area (ha)= 274.15 Curve Number (CN)= 57.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.87

Unit Hyd Qpeak (cms)= 5.367

PEAK FLOW (cms)= 2.485 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 17.633
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.241

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8398)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0292): 738.49 6.145 8.25 24.58
+ ID2= 2 (0294): 274.15 2.485 7.00 17.63
=====

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (8296)
IN= 2--> OUT= 1 Routing time step (min)= 15.00

----- DATA FOR SECTION (2961.0) ----->
Distance Elevation Manning
0.00 243.98 0.0400
42.59 243.18 0.0400
85.17 241.81 0.0400
120.66 240.50 0.0400
156.15 239.56 0.0400
198.74 236.15 0.0400
237.98 234.01 0.0400
241.33 233.82 0.0400
248.77 233.12 0.0400 /0.0400 Main Channel
249.87 232.32 0.0400 Main Channel
250.37 231.80 0.0400 Main Channel
250.87 232.23 0.0400 Main Channel
251.97 233.10 0.0400 /0.0400 Main Channel
255.37 233.22 0.0400
259.07 233.87 0.0400
262.62 234.12 0.0400
266.17 234.23 0.0400
283.91 234.73 0.0400

337.15 241.75 0.0400
351.34 244.00 0.0400

318.65 223.69 0.0400
360.22 225.57 0.0400
397.16 227.60 0.0400
457.20 228.35 0.0400

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.43	232.23	.510E+03	0.1	0.53	81.07
0.87	232.67	.213E+04	0.7	0.83	51.49
1.30	233.10	.501E+04	2.2	1.10	38.75
1.98	233.78	.242E+05	12.6	1.34	31.93
2.66	234.46	.738E+05	42.1	1.46	29.21
3.34	235.14	.177E+06	127.9	1.84	23.13
4.02	235.82	.314E+06	281.4	2.29	18.61
4.70	236.50	.481E+06	509.4	2.71	15.73
5.38	237.18	.672E+06	815.6	3.11	13.73
6.06	237.86	.886E+06	1200.7	3.46	12.30
6.74	238.54	.112E+07	1669.4	3.80	11.23
7.42	239.22	.139E+07	2226.9	4.11	10.38
8.10	239.90	.168E+07	2808.4	4.28	9.95
8.78	240.58	.202E+07	3470.4	4.40	9.66
9.46	241.26	.240E+07	4331.7	4.61	9.24
10.14	241.94	.283E+07	5335.2	4.83	8.84
10.82	242.62	.330E+07	6482.6	5.03	8.48
11.50	243.30	.381E+07	7764.1	5.21	8.18
12.18	243.98	.439E+07	9101.2	5.31	8.04

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8398) 1012.64	8.46	8.00	22.70	1.71	1.23
OUTFLOW : ID= 1 (6296) 1012.64	8.22	8.50	22.70	1.69	1.22

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.29	221.29	.228E+04	0.2	0.32	165.63
0.59	221.59	.465E+04	0.7	0.45	116.15
0.88	221.88	.713E+04	1.2	0.54	96.87
1.18	222.18	.971E+04	1.9	0.61	86.20
1.57	222.57	.157E+05	3.3	0.59	88.86
1.97	222.97	.427E+05	6.4	0.47	111.39
2.36	223.36	.115E+06	16.1	0.44	119.14
2.76	223.76	.237E+06	37.1	0.49	106.49
3.15	224.15	.409E+06	72.9	0.56	93.40
3.55	224.55	.631E+06	127.0	0.63	82.76
3.94	224.94	.903E+06	202.4	0.70	74.37
4.34	225.34	.123E+07	301.8	0.77	67.68
4.73	225.73	.160E+07	433.5	0.85	61.37
5.13	226.13	.200E+07	602.2	0.95	55.29
5.52	226.52	.242E+07	804.7	1.05	50.13
5.92	226.92	.286E+07	1034.2	1.14	46.11
6.31	227.31	.332E+07	1290.5	1.22	42.88
6.71	227.71	.380E+07	1560.9	1.29	40.55
7.10	228.10	.432E+07	1835.6	1.34	39.19

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8392) 2505.74	21.86	8.00	24.05	2.47	0.45
OUTFLOW : ID= 1 (6302) 2505.74	17.19	10.25	24.05	2.38	0.44

----- ADD HYD (8396) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0296):	293.65	3.926	7.50	31.00
+ ID2= 2 (6296):	1012.64	8.218	8.50	22.70
ID = 3 (8396):	1306.29	11.968	8.25	24.57

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ADD HYD (8394) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8396):	1306.29	11.968	8.25	24.57
+ ID2= 2 (8397):	610.01	6.617	8.00	32.99
ID = 3 (8394):	1916.30	18.538	8.00	27.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ADD HYD (8392) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8394):	1916.30	18.538	8.00	27.25
+ ID2= 2 (8395):	589.44	3.331	7.75	13.66
ID = 3 (8392):	2505.74	21.856	8.00	24.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ROUTE CHN (6302) -----

IN= 2--> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (3021.0) -----

Distance	Elevation	Manning
0.00	228.10	0.0400
18.47	227.12	0.0400
36.95	226.12	0.0400
46.18	225.84	0.0400
55.42	225.58	0.0400
272.47	222.88	0.0400
277.09	222.76	0.0400
281.71	222.58	0.0400
288.54	222.18	0.0400 / 0.0300 Main Channel
288.64	221.00	0.0300 Main Channel
290.94	221.00	0.0300 Main Channel
291.04	221.00	0.0300 Main Channel
291.54	222.75	0.0300 / 0.0400 Main Channel
300.18	222.83	0.0400
304.80	223.04	0.0400
309.42	223.25	0.0400

----- ADD HYD (8390) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0302):	473.90	2.715	8.50	18.31
+ ID2= 2 (6302):	2505.74	17.159	10.25	24.05
ID = 3 (8390):	2979.64	19.699	10.25	23.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ADD HYD (8348) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8386):	788.82	9.844	8.50	35.18
+ ID2= 2 (8390):	2979.64	19.699	10.25	23.14
ID = 3 (8348):	3768.46	28.649	9.75	25.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- CALIB -----

NASHYD (0304)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	5.00	63.0
U.H. Tp(hrs)	2.78	# of Linear Res.(N)= 1.50

Unit Hyd Opeak (cms) = 1.793

PEAK FLOW (cms) = 1.311 (i)
TIME TO PEAK (hrs) = 10.250
RUNOFF VOLUME (mm) = 21.297
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.291

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB -----

NASHYD (0280)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	5.00	82.0
U.H. Tp(hrs)	0.65	# of Linear Res.(N)= 1.50

Unit Hyd Opeak (cms) = 6.009

PEAK FLOW (cms) = 6.103 (i)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 36.959
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.506

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.


```

-----
| CALIB |
| NASHYD (0278) | Area (ha)= 485.49 Curve Number (CN)= 82.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.52

```

```

Unit Hyd Qpeak (cms)= 5.453

PEAK FLOW (cms)= 6.254 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 37.239
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.509

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0276) | Area (ha)= 90.89 Curve Number (CN)= 79.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.67

```

```

Unit Hyd Qpeak (cms)= 2.302

PEAK FLOW (cms)= 2.014 (1)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 33.570
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.459

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0274) | Area (ha)= 392.49 Curve Number (CN)= 75.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.08

```

```

Unit Hyd Qpeak (cms)= 6.182

PEAK FLOW (cms)= 5.249 (1)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 30.083
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.412

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8360) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| (ha) (cms) (hrs) (mm)
| ID1= 1 (0274): 392.49 5.249 7.50 30.08
| + ID2= 2 (0276): 90.89 2.014 6.75 33.57
|-----|
| ID = 3 (8360): 483.38 7.141 7.00 30.74

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8358) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| (ha) (cms) (hrs) (mm)
| ID1= 1 (0278): 485.49 6.254 8.00 37.24
| + ID2= 2 (8360): 483.38 7.141 7.00 30.74
|-----|
| ID = 3 (8358): 968.87 13.139 7.50 34.00

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CIB (6280) |
| IN= 2 ---> OUT= 1 | Routing time step (min)'= 15.00
|-----|

```

```

<----- DATA FOR SECTION (2801.0) ----->
Distance Elevation Manning
0.00 241.14 0.0500
13.32 240.80 0.0500
39.95 240.07 0.0500
96.54 236.23 0.0500
113.19 234.15 0.0500
123.18 232.35 0.0500
143.15 225.80 0.0500
149.81 225.62 0.0500
153.14 225.40 0.0500
157.30 224.76 0.0500 /0.0300 Main Channel
159.80 224.26 0.0300 Main Channel

```

```

162.30 224.85 0.0300 Main Channel
162.55 224.97 0.0300 /0.0500 Main Channel
167.80 225.05 0.0500
186.43 229.14 0.0500
213.06 234.75 0.0500
236.37 237.09 0.0500
266.33 237.31 0.0500
292.96 237.83 0.0500
329.58 241.50 0.0500

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu. m) (cms) (m/s) (min)
0.50 224.76 .263E+04 0.8 0.65 58.31
1.36 225.62 .278E+05 13.4 1.10 34.54
2.22 226.48 .855E+05 52.6 1.40 27.09
3.09 227.35 .157E+06 120.4 1.74 21.78
3.95 228.21 .242E+06 216.3 2.04 18.64
4.81 229.07 .340E+06 341.8 2.29 16.56
5.67 229.93 .450E+06 498.3 2.52 15.06
6.53 230.79 .574E+06 688.3 2.73 13.90
7.40 231.66 .711E+06 913.8 2.93 12.97
8.26 232.52 .861E+06 1173.4 3.11 12.23
9.12 233.38 .103E+07 1463.3 3.24 11.72
9.98 234.24 .121E+07 1793.3 3.37 11.26
10.85 235.11 .142E+07 2141.0 3.45 11.02
11.71 235.97 .165E+07 2530.4 3.50 10.86
12.57 236.83 .192E+07 2954.0 3.51 10.81
13.43 237.69 .226E+07 3160.3 3.18 11.93
14.29 238.55 .271E+07 3869.4 3.26 11.67
15.16 239.42 .320E+07 4740.3 3.38 11.24
16.02 240.28 .373E+07 5688.4 3.48 10.92

```

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8358) 968.87 13.14 7.50 34.00 1.34 1.08
OUTFLOW : ID= 1 (6280) 968.87 12.85 8.00 34.00 1.32 1.06

```

```

-----
| ADD HYD (8354) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----|
| (ha) (cms) (hrs) (mm)
| ID1= 1 (0280): 299.86 6.103 7.00 36.96
| + ID2= 2 (6280): 968.87 12.849 8.00 34.00
|-----|
| ID = 3 (8354): 1268.73 18.341 7.75 34.70

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0272) | Area (ha)= 157.38 Curve Number (CN)= 75.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.09

```

```

Unit Hyd Qpeak (cms)= 2.456

PEAK FLOW (cms)= 2.090 (1)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 30.086
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.412

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0270) | Area (ha)= 243.61 Curve Number (CN)= 81.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
|-----|
| U.H. Tp(hrs)= 0.87

```

```

Unit Hyd Qpeak (cms)= 3.429

PEAK FLOW (cms)= 3.490 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 35.833
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.490

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0268) | Area (ha)= 215.76 Curve Number (CN)= 75.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
|-----|
| U.H. Tp(hrs)= 0.69

```

Unit Hyd Qpeak (cms)= 3.807

PEAK FLOW (cms)= 3.054 (1)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 29.811
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.408

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----
| CALIB |
| NASHYD (0264) | Area (ha)= 353.96 Curve Number (CN)= 69.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
|-----|
| U.H. Tp(hrs)= 1.30
```

Unit Hyd Qpeak (cms)= 3.313

PEAK FLOW (cms)= 2.544 (1)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 25.249
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.345

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----
| CALIB |
| NASHYD (0266) | Area (ha)= 508.09 Curve Number (CN)= 64.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
|-----|
| U.H. Tp(hrs)= 1.63
```

Unit Hyd Qpeak (cms)= 3.786

PEAK FLOW (cms)= 2.622 (1)
 TIME TO PEAK (hrs)= 9.250
 RUNOFF VOLUME (mm)= 21.852
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.299

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----
| ADD HYD (8380) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----| (ha) (cms) (hrs) (mm)
| ID= 1 (0264): 353.96 2.544 8.25 25.25
| + ID2= 2 (0266): 508.09 2.622 9.25 21.85
|-----|
| ID = 3 (8380): 862.05 5.142 8.50 23.25
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
-----
| ROUTE CHN (6268) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
```

<----- DATA FOR SECTION (2681.0) ----->

Distance	Elevation	Manning
0.00	274.03	0.0500
10.55	272.73	0.0500
21.10	271.28	0.0500
50.12	266.83	0.0500
87.04	260.36	0.0500
92.32	260.06	0.0500
94.95	259.93	0.0500
101.00	256.87	0.0500
103.00	256.58	0.0500 / 0.0350 Main Channel
104.00	256.32	0.0350 Main Channel
106.00	256.05	0.0350 Main Channel
108.00	256.25	0.0350 Main Channel
110.78	256.65	0.0350 / 0.0500 Main Channel
113.42	256.81	0.0500
116.06	257.00	0.0500
155.62	260.24	0.0500
187.27	263.35	0.0500
211.01	267.22	0.0500
224.20	269.60	0.0500
261.13	272.68	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.53	256.58	.697E+04	2.1	0.97	55.64
1.38	257.43	.477E+05	25.9	1.76	30.66
2.22	258.27	.123E+06	84.3	2.22	24.26
3.07	259.12	.231E+06	185.4	2.60	20.72
3.92	259.97	.371E+06	335.8	2.92	18.42
4.77	260.82	.560E+06	547.6	3.15	17.06
5.61	261.66	.790E+06	845.6	3.46	15.57
6.46	262.51	.106E+07	1225.0	3.75	14.37

7.31	263.36	.138E+07	1693.3	4.02	13.37
8.16	264.21	.169E+07	2283.2	4.35	12.36
9.00	265.05	.206E+07	2969.1	4.66	11.54
9.85	265.90	.245E+07	3754.4	4.96	10.86
10.70	266.75	.286E+07	4643.0	5.24	10.28
11.55	267.60	.331E+07	5636.1	5.50	9.78
12.39	268.44	.378E+07	6746.2	5.76	9.34
13.24	269.29	.428E+07	7972.2	6.01	8.96
14.09	270.14	.482E+07	9318.9	6.18	8.71
14.94	270.99	.539E+07	10750.5	6.32	8.52
15.78	271.83	.601E+07	12204.7	6.46	8.33

```
-----
| ADD HYD (8382) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----| (ha) (cms) (hrs) (mm)
| ID= 1 (0268): 215.76 3.054 7.00 29.81
| + ID2= 2 (6268): 862.05 5.000 10.00 23.25
|-----|
| ID = 3 (8382): 1077.81 7.285 8.75 24.56
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
-----
| ROUTE CHN (6270) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
```

<----- DATA FOR SECTION (2701.0) ----->

Distance	Elevation	Manning
0.00	245.98	0.0500
8.14	245.66	0.0500
16.28	245.16	0.0500
20.35	244.84	0.0500
38.66	242.98	0.0500
48.83	240.65	0.0500
63.07	235.91	0.0500
65.11	235.18	0.0500
68.43	234.34	0.0500 / 0.0300 Main Channel
68.68	233.95	0.0300 Main Channel
69.18	233.89	0.0300 Main Channel
69.68	233.95	0.0300 Main Channel
71.21	234.48	0.0300 / 0.0500 Main Channel
81.38	236.44	0.0500
95.63	236.66	0.0500
120.04	237.00	0.0500
148.53	241.77	0.0500
158.70	242.34	0.0500
187.18	244.03	0.0500
201.42	244.36	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.45	234.34	.217E+04	0.7	0.99	52.83
0.98	234.87	.966E+04	4.6	1.49	35.20
1.50	235.39	.250E+05	13.7	1.73	30.41
2.03	235.92	.475E+05	29.5	1.95	26.87
2.56	236.45	.772E+05	52.5	2.14	24.50
3.09	236.98	.142E+06	82.4	1.83	28.73
3.61	237.50	.246E+06	158.1	2.03	25.91
4.14	238.03	.357E+06	262.0	2.31	22.73
4.67	238.56	.477E+06	392.1	2.59	20.27
6.78	240.67	.103E+07	1169.7	2.86	18.38
7.31	241.20	.119E+07	1425.9	3.77	13.94
7.83	241.72	.136E+07	1710.4	3.96	13.26
8.36	242.25	.154E+07	1967.9	4.02	13.07
8.89	242.78	.174E+07	2264.9	4.09	12.83
9.42	243.31	.196E+07	2589.3	4.15	12.65
9.94	243.83	.221E+07	2953.5	4.22	12.46

```
-----
| ADD HYD (8384) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----| (ha) (cms) (hrs) (mm)
| INFLOW : ID= 2 (8382) 1077.81 7.29 8.75 24.56 1.13 1.56
| OUTFLOW: ID= 1 (6270) 1077.81 7.18 9.25 24.56 1.13 1.55
```

```

-----
      (ha)  (cms)  (hrs)  (mm)
ID1= 1 (0270): 243.61 3.490 7.25 35.83
+ ID2= 2 (6270): 1077.81 7.180 9.25 24.56
=====
ID = 3 (8384): 1321.42 10.209 8.75 26.64

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
CALIB
NASHYD (0262) | Area (ha)= 341.31 | Curve Number (CN)= 82.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
U.H. Tp(hrs)= 1.01

```

Unit Hyd Qpeak (cms)= 4.094

```

PEAK FLOW (cms)= 4.455 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 37.033
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.507

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB
NASHYD (0260) | Area (ha)= 476.24 | Curve Number (CN)= 82.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
U.H. Tp(hrs)= 1.33

```

Unit Hyd Qpeak (cms)= 4.360

```

PEAK FLOW (cms)= 5.038 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 37.154
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.508

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB
NASHYD (0258) | Area (ha)= 181.99 | Curve Number (CN)= 79.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
U.H. Tp(hrs)= 1.18

```

Unit Hyd Qpeak (cms)= 1.881

```

PEAK FLOW (cms)= 1.930 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 33.887
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.464

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB
NASHYD (0252) | Area (ha)= 319.99 | Curve Number (CN)= 73.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
U.H. Tp(hrs)= 1.04

```

Unit Hyd Qpeak (cms)= 3.761

```

PEAK FLOW (cms)= 3.102 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 28.314
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.387

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB
NASHYD (0256) | Area (ha)= 145.79 | Curve Number (CN)= 67.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
U.H. Tp(hrs)= 1.04

```

Unit Hyd Qpeak (cms)= 1.707

```

PEAK FLOW (cms)= 1.174 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 23.749
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.325

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB
NASHYD (0254) | Area (ha)= 403.00 | Curve Number (CN)= 55.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
U.H. Tp(hrs)= 1.22

```

Unit Hyd Qpeak (cms)= 4.028

```

PEAK FLOW (cms)= 1.990 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 16.662
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.228

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
ROUTE CHN (6256) | Routing time step (min)= 15.00
IN= 2--> OUT= 1 |

```

<----- DATA FOR SECTION (2561.0) ----->

Distance	Elevation	Manning	
0.00	276.07	0.0400	
11.68	273.71	0.0400	
23.36	271.35	0.0400	
29.19	270.30	0.0400	
35.03	269.44	0.0400	
55.47	267.90	0.0400	
78.82	266.24	0.0400	
90.50	265.63	0.0400	
102.18	265.40	0.0400	
105.10	264.95	0.0400 / 0.0350	Main Channel
108.02	264.39	0.0350	Main Channel
110.94	264.72	0.0350	Main Channel
113.86	265.19	0.0350 / 0.0400	Main Channel
116.78	265.49	0.0400	
143.05	268.24	0.0400	
172.25	270.53	0.0400	
207.28	271.95	0.0400	
233.55	273.72	0.0400	
256.91	274.98	0.0400	
289.02	275.97	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.56	264.95	.723E+04	1.9	0.90	62.51
1.14	265.53	.307E+05	13.5	1.49	37.92
1.72	266.11	.951E+05	48.6	1.73	32.62
2.30	266.69	.191E+06	117.8	2.09	27.03
2.88	267.27	.334E+06	225.2	2.43	23.24
3.46	267.85	.464E+06	375.5	2.74	20.59
4.04	268.43	.641E+06	573.2	3.03	18.62
4.62	269.01	.846E+06	821.9	3.29	17.15
5.20	269.59	.108E+07	1135.0	3.56	15.87
5.78	270.17	.134E+07	1525.1	3.86	14.64
6.36	270.75	.162E+07	1971.1	4.12	13.71
6.94	271.33	.194E+07	2474.0	4.33	13.04
7.52	271.91	.228E+07	3065.0	4.55	12.42
8.10	272.49	.266E+07	3773.4	4.80	11.75
8.68	273.07	.308E+07	4566.2	5.05	11.17
9.26	273.65	.348E+07	5443.7	5.30	10.66
9.84	274.23	.393E+07	6389.6	5.51	10.25
10.42	274.81	.440E+07	7426.2	5.72	9.88
11.00	275.39	.490E+07	8490.1	5.86	9.63

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(cms) (cms) (hrs) (mm) (m) (m/s)
INFLOW: ID= 2 (0254) 403.00 1.99 8.00 16.66 0.56 0.90
OUTFLOW: ID= 1 (6256) 403.00 1.87 9.75 16.66 0.54 0.90

```

```

-----
ADD HYD (8370) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0256): 145.79 1.174 8.00 23.75
+ ID2= 2 (6256): 403.00 1.869 9.75 16.66
=====
ID = 3 (8370): 548.79 2.934 9.25 18.54

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
CALIB
NASHYD (0250) | Area (ha)= 192.88 | Curve Number (CN)= 70.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
U.H. Tp(hrs)= 1.22

```

Unit Hyd Qpeak (cms)= 1.930

PEAK FLOW (cms)= 1.508 (1)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 25.980
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.355

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0246) | Area (ha)= 759.61 | Curve Number (CN)= 55.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
 | U.N. Tp(hrs)= 1.81

Unit Hyd Opeak (cms)= 5.121

PEAK FLOW (cms)= 2.778 (1)
 TIME TO PEAK (hrs)= 10.000
 RUNOFF VOLUME (mm)= 16.720
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.229

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0248) | Area (ha)= 146.04 | Curve Number (CN)= 64.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
 | U.N. Tp(hrs)= 0.78

Unit Hyd Opeak (cms)= 2.271

PEAK FLOW (cms)= 1.326 (1)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 21.646
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.296

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8364) |
 | 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0246): 759.61 2.778 10.00 16.72
 + ID2= 2 (0248): 146.04 1.326 7.25 21.65

 ID = 3 (8364): 905.65 3.933 8.25 17.51

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6250) | Routing time step (min)'= 15.00
 | IN= 2--> OUT= 1 |

<----- DATA FOR SECTION (2501.0) ----->

Distance	Elevation	Manning	
0.00	269.96	0.0500	
8.56	268.55	0.0500	
17.13	266.91	0.0500	
21.41	266.13	0.0500	
40.68	263.15	0.0500	
62.09	260.75	0.0500	
85.64	258.02	0.0500	
88.20	257.69	0.0500	
93.20	257.05	0.0500 /0.0350	Main Channel
93.45	256.88	0.0350	Main Channel
94.20	256.56	0.0350	Main Channel
94.95	256.83	0.0350	Main Channel
95.20	257.08	0.0350 /0.0500	Main Channel
100.62	257.45	0.0500	
115.61	258.57	0.0500	
139.16	260.43	0.0500	
152.01	261.95	0.0500	
171.27	264.63	0.0500	
188.40	267.90	0.0500	
211.95	274.18	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.49	257.05	.203E+04	0.4	0.68	89.65
1.17	257.73	.248E+05	5.7	0.85	72.16
1.85	258.41	.841E+05	25.2	1.10	55.60
2.53	259.09	.180E+06	66.5	1.36	45.14
3.21	259.77	.312E+06	135.7	1.60	38.36
3.89	260.45	.481E+06	238.3	1.82	33.61
4.57	261.13	.681E+06	383.5	2.07	29.60
5.25	261.81	.911E+06	568.9	2.29	26.70
5.93	262.49	.117E+07	800.5	2.51	24.36

6.61	263.17	.148E+07	1079.0	2.72	22.49
7.28	263.84	.177E+07	1413.4	2.94	20.85
7.96	264.52	.210E+07	1797.0	3.14	19.50
8.64	265.20	.246E+07	2241.6	3.35	18.28
9.32	265.88	.283E+07	2739.9	3.55	17.25
10.00	266.56	.323E+07	3295.7	3.75	16.34
10.68	267.24	.364E+07	3908.9	3.94	15.54
11.36	267.92	.408E+07	4578.5	4.12	14.84
12.04	268.60	.453E+07	5320.1	4.32	14.18
12.72	269.28	.499E+07	6109.1	4.50	13.61

<----- hydrograph -----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8364) 905.65 3.93 8.25 17.51 0.94 0.78
 OUTFLOW : ID= 1 (6250) 905.65 3.66 10.25 17.51 0.90 0.77

ADD HYD (8366) |
 | 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0250): 192.88 1.508 8.00 25.98
 + ID2= 2 (6250): 905.65 3.662 10.25 17.51

 ID = 3 (8366): 1098.53 5.036 10.00 19.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8368) |
 | 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8366): 1098.53 5.036 10.00 19.00
 + ID2= 2 (8370): 548.79 2.934 9.25 18.54

 ID = 3 (8368): 1647.32 7.940 9.75 18.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8372) |
 | 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0250): 319.99 3.102 8.00 28.31
 + ID2= 2 (8368): 1647.32 7.940 9.75 18.85

 ID = 3 (8372): 1967.31 10.760 9.00 20.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6258) | Routing time step (min)'= 15.00
 | IN= 2--> OUT= 1 |

<----- DATA FOR SECTION (2581.0) ----->

Distance	Elevation	Manning	
0.00	252.88	0.0500	
15.47	251.89	0.0500	
46.41	248.45	0.0500	
73.48	245.81	0.0500	
88.95	243.75	0.0500	
112.15	242.00	0.0500	
135.35	240.23	0.0500	
162.42	239.76	0.0500	
170.97	239.52	0.0500 /0.0350	Main Channel
171.58	239.03	0.0350	Main Channel
174.02	239.03	0.0350	Main Channel
176.46	239.03	0.0350	Main Channel
177.07	239.52	0.0350 /0.0500	Main Channel
185.63	239.67	0.0500	
208.83	239.87	0.0500	
239.77	240.14	0.0500	
274.57	244.93	0.0500	
336.45	249.51	0.0500	
363.52	249.77	0.0500	
382.85	251.78	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.49	239.52	.135E+05	2.5	0.92	91.32
1.14	240.17	.175E+06	25.6	0.73	114.22
1.78	240.81	.532E+06	123.5	1.17	71.85
2.43	241.46	.932E+06	285.3	1.54	54.48
3.07	242.10	.138E+07	506.5	1.85	45.25
3.72	242.75	.186E+07	786.8	2.12	39.42
4.36	243.39	.239E+07	1127.3	2.37	35.33

5.01	244.04	.298E+07	1535.8	2.61	32.12
5.65	244.68	.358E+07	2014.3	2.84	29.48
6.30	245.33	.420E+07	2535.9	3.03	27.60
6.94	245.97	.488E+07	3108.4	3.20	26.16
7.59	246.62	.563E+07	3745.6	3.35	24.98
8.23	247.26	.638E+07	4446.7	3.50	23.93
8.88	247.91	.721E+07	5229.7	3.64	22.98
9.52	248.55	.809E+07	6094.3	3.78	22.12
10.17	249.20	.903E+07	7050.4	3.93	21.31
10.81	249.84	.100E+08	7773.0	3.90	21.46
11.46	250.49	.111E+08	8998.8	4.07	20.56
12.10	251.13	.122E+08	10316.2	4.24	19.75

408.40	222.65	0.0450
460.17	223.20	0.0450
517.69	224.84	0.0450
569.46	232.57	0.0450

```

----- TRAVEL TIME TABLE -----
DEPTH  ELEV  VOLUME  FLOW RATE  VELOCITY  TRAV.TIME
(m)      (m)      (cu.m.)  (cms)      (m/s)      (min)
0.41  220.57  .189E+04  0.5         0.49       60.18
0.83  220.99  .697E+04  3.2         0.82       35.79
1.24  221.40  .145E+05  8.8         1.07       27.45
1.84  222.00  .643E+05  29.3        0.81       36.61
2.44  222.60  .282E+06  132.9       0.83       35.41
3.04  223.20  .707E+06  413.7       1.04       28.50
3.64  223.80  .118E+07  876.8       1.31       22.46
4.24  224.40  .168E+07  1494.8      1.57       18.75
4.84  225.00  .221E+07  2264.1      1.82       16.23
5.44  225.60  .274E+07  3196.3      2.06       14.29
6.04  226.20  .328E+07  4258.6      2.30       12.84
6.65  226.81  .383E+07  5444.7      2.52       11.72
7.25  227.41  .438E+07  6749.7      2.73       10.82
7.85  228.01  .494E+07  8169.4      2.93       10.08
8.45  228.61  .551E+07  9700.7      3.12       9.46
9.05  229.21  .608E+07  11345.8     3.30       8.94
9.65  229.81  .666E+07  13087.4     3.48       8.48
10.25  230.41  .724E+07  14938.5     3.65       8.08
10.85  231.01  .783E+07  16892.4     3.82       7.73

```

```

<---- hydrograph ----> <-pipe / channel->
AREA  QPEAK  TPEAK  R.V.  MAX DEPTH  MAX VEL
(ha)  (cms)  (hrs)  (mm)  (m)  (m/s)
INFLOW : ID= 2 (8372) 1967.31  10.76  9.00  20.39  0.72  0.84
OUTFLOW : ID= 1 (6258) 1967.31  9.73  11.25  20.39  0.69  0.85

```

```

<---- hydrograph ----> <-pipe / channel->
AREA  QPEAK  TPEAK  R.V.  MAX DEPTH  MAX VEL
(ha)  (cms)  (hrs)  (mm)  (m)  (m/s)
INFLOW : ID= 2 (8362) 4288.27  29.33  9.50  26.07  1.84  0.81
OUTFLOW : ID= 1 (6272) 4288.27  28.76  10.25  26.07  1.83  0.81

```

```

-----
| ADD HYD (8374) |
| 1 + 2 = 3 |
AREA  QPEAK  TPEAK  R.V.
(ha)  (cms)  (hrs)  (mm)
ID1= 1 (0258): 181.99  1.930  8.00  33.89
+ ID2= 2 (6258): 1967.31  9.734  11.25  20.39
-----
ID = 3 (8374): 2149.30  11.262  11.00  21.53

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8356) |
| 1 + 2 = 3 |
AREA  QPEAK  TPEAK  R.V.
(ha)  (cms)  (hrs)  (mm)
ID1= 1 (0272): 157.38  2.090  7.50  30.09
+ ID2= 2 (6272): 4288.27  28.759  10.25  26.07
-----
ID = 3 (8356): 4445.65  30.243  10.00  26.22

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8378) |
| 1 + 2 = 3 |
AREA  QPEAK  TPEAK  R.V.
(ha)  (cms)  (hrs)  (mm)
ID1= 1 (0262): 341.31  4.455  7.75  37.03
+ ID2= 2 (8376): 2625.54  15.711  10.25  24.37
-----
ID = 3 (8378): 2966.85  19.393  10.00  25.82

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8352) |
| 1 + 2 = 3 |
AREA  QPEAK  TPEAK  R.V.
(ha)  (cms)  (hrs)  (mm)
ID1= 1 (8354): 1268.73  18.341  7.75  34.70
+ ID2= 2 (8356): 4445.65  30.243  10.00  26.22
-----
ID = 3 (8352): 5714.38  45.383  8.75  28.10

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8362) |
| 1 + 2 = 3 |
AREA  QPEAK  TPEAK  R.V.
(ha)  (cms)  (hrs)  (mm)
ID1= 1 (8378): 2966.85  19.393  10.00  25.82
+ ID2= 2 (8384): 1321.42  10.209  8.75  26.64
-----
ID = 3 (8362): 4288.27  29.326  9.50  26.07

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6304) |
| IN= 2--> OUT= 1 |
Routing time step (min)= 15.00

```

```

----- DATA FOR SECTION (3041.0) -----
Distance  Elevation  Manning
0.00      232.08      0.0500
19.00     231.87      0.0500
38.00     231.33      0.0500
66.51     230.44      0.0500
104.51     228.25      0.0500
118.76     225.17      0.0500
128.26     219.86      0.0500
175.77     219.17      0.0500
185.27     218.90      0.0500 / 0.0300 Main Channel
185.52     218.65      0.0300 Main Channel
190.02     218.37      0.0300 Main Channel
194.52     218.60      0.0300 Main Channel
194.77     218.85      0.0300 / 0.0500 Main Channel
204.27     219.60      0.0500
299.28     220.91      0.0500
327.78     222.36      0.0500
375.28     225.71      0.0500
403.79     229.37      0.0500
432.29     230.43      0.0500
470.29     232.00      0.0500

```

```

-----
| ROUTE CHN (6272) |
| IN= 2--> OUT= 1 |
Routing time step (min)= 15.00

```

```

----- DATA FOR SECTION (2721.0) -----
Distance  Elevation  Manning
0.00      231.01      0.0450
23.01     223.65      0.0450
34.51     222.46      0.0450
51.77     222.11      0.0450
69.02     221.87      0.0450
161.06     221.92      0.0450
166.81     221.91      0.0450
172.56     221.89      0.0450
180.57     221.40      0.0450 / 0.0300 Main Channel
181.57     220.60      0.0300 Main Channel
184.07     220.16      0.0300 Main Channel
195.57     221.85      0.0300 / 0.0450 Main Channel
201.32     221.82      0.0450
207.07     221.77      0.0450
212.83     221.72      0.0450
253.09     222.52      0.0450

```

```

----- TRAVEL TIME TABLE -----
DEPTH  ELEV  VOLUME  FLOW RATE  VELOCITY  TRAV.TIME
(m)      (m)      (cu.m.)  (cms)      (m/s)      (min)
0.48  218.85  .121E+05  1.2         0.36       173.94
1.17  219.54  .839E+05  9.8         0.44       142.60
1.86  220.23  .323E+06  41.0        0.47       131.43

```

2.56	220.93	.708E+06	109.5	0.58	107.67
3.25	221.62	.118E+07	224.3	0.71	87.36
3.94	222.31	.168E+07	376.5	0.84	74.48
4.63	223.00	.222E+07	567.9	0.96	65.26
5.32	223.69	.279E+07	794.7	1.07	58.58
6.02	224.39	.339E+07	1056.2	1.17	53.52
6.71	225.08	.402E+07	1352.4	1.26	49.52
7.40	225.77	.468E+07	1677.3	1.34	46.46
8.09	226.46	.536E+07	2050.0	1.43	43.58
8.79	227.16	.607E+07	2457.9	1.52	41.14
9.48	227.85	.680E+07	2901.0	1.60	39.05
10.17	228.54	.755E+07	3352.7	1.66	37.53
10.86	229.23	.834E+07	3812.5	1.71	36.48
11.55	229.92	.919E+07	4241.8	1.73	36.13
12.25	230.62	.101E+08	4697.0	1.74	35.93
12.94	231.31	.111E+08	5186.3	1.74	35.83

<---- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8352) 5714.38 45.38 8.75 28.10 1.31 0.48
 OUTFLOW: ID= 1 (6304) 5714.38 37.83 11.50 28.10 1.80 0.47

ADD HYD (8350)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0304): 292.37 1.311 10.25 21.30
 + ID2= 2 (6304): 5714.38 37.830 11.50 28.10
 ID = 3 (8350): 6006.75 39.122 11.25 27.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8346)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8348): 3768.46 28.649 9.75 25.66
 + ID2= 2 (8350): 6006.75 39.122 11.25 27.77
 ID = 3 (8346): 9775.21 66.196 10.75 26.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8344)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0306): 283.97 0.445 13.75 15.32
 + ID2= 2 (8346): 9775.21 66.196 10.75 26.96
 ID = 3 (8344): 10059.18 66.598 10.75 26.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5509)
 IN= 2 --> OUT= 1
 DT= 15.0 min
 OUTFLOW STORAGE OUTFLOW STORAGE
 (cms) (ha.m.) (cms) (ha.m.)
 0.0000 0.0000 ***** 370.0451
 41.0590 56.7403 ***** 863.4386
 48.1390 86.3439 ***** *****
 56.6340 ***** *****
 67.9600 ***** 0.0000 0.0000

AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8344) ***** 66.598 10.75 26.63
 OUTFLOW: ID= 1 (5509) ***** 45.744 15.25 26.63

PEAK FLOW REDUCTION [Qout/Qin](%)= 68.69
 TIME SHIFT OF PEAK FLOW (min)=270.00
 MAXIMUM STORAGE USED (ha.m.)= 76.3485

CALIB (0332) Area (ha)= 393.44 Curve Number (CN)= 75.0
 NASHYD (0332) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 2.32

Unit Hyd Qpeak (cms)= 2.894

PEAK FLOW (cms)= 2.902 (1)
 TIME TO PEAK (hrs)= 9.750

RUNOFF VOLUME (mm)= 30.269
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.414

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0330) Area (ha)= 468.30 Curve Number (CN)= 80.0
 NASHYD (0330) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.09

Unit Hyd Qpeak (cms)= 7.335

PEAK FLOW (cms)= 7.311 (1)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 34.924
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.478

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6332) Routing time step (min)= 15.00
 IN= 2 --> OUT= 1

----- DATA FOR SECTION (3321.0) -----

Distance	Elevation	Manning
0.00	234.00	0.0380
25.85	227.20	0.0380
96.94	226.44	0.0380
168.03	227.38	0.0380
219.73	225.62	0.0380
342.52	221.57	0.0380
368.37	221.42	0.0380
374.83	221.23	0.0380
379.79	220.98	0.0380 / 0.0300
380.29	220.47	0.0300
381.29	220.47	0.0300
382.79	220.47	0.0300
383.29	220.98	0.0300 / 0.0380
394.22	221.22	0.0380
400.68	221.33	0.0380
407.14	221.44	0.0380
491.16	225.70	0.0380
568.71	227.55	0.0380
607.49	230.14	0.0380
639.80	234.08	0.0380

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.51	220.98	.554E+04	0.9	0.56	107.27
1.20	221.67	.885E+05	12.1	0.49	122.32
1.88	222.35	.309E+06	64.7	0.76	79.70
2.57	223.04	.615E+06	165.4	0.97	61.99
3.25	223.72	.101E+07	322.1	1.16	52.06
3.94	224.41	.148E+07	542.9	1.33	45.51
4.62	225.09	.204E+07	835.4	1.48	40.77
5.31	225.78	.269E+07	1202.8	1.62	37.27
5.99	226.46	.344E+07	1618.0	1.70	35.46
6.68	227.15	.447E+07	1932.0	1.56	38.56
7.36	227.83	.580E+07	2763.9	1.72	34.96
8.05	228.52	.718E+07	3861.9	1.95	30.97
8.73	229.20	.859E+07	5115.4	2.16	27.98
9.42	229.89	.100E+08	6519.2	2.35	25.64
10.10	230.57	.115E+08	8092.3	2.55	23.69
10.79	231.26	.130E+08	9821.5	2.73	22.05
11.47	231.94	.145E+08	11690.0	2.92	20.68
12.16	232.63	.160E+08	13694.9	3.09	19.52
12.84	233.31	.176E+08	15833.9	3.26	18.52

<---- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0330) 468.30 7.31 7.50 34.92 0.90 0.52
 OUTFLOW: ID= 1 (6332) 468.30 5.57 9.75 34.92 0.80 0.53

ADD HYD (8322) AREA QPEAK TPEAK R.V.
 1 + 2 = 3 (ha) (cms) (hrs) (mm)
 ID1= 1 (0332): 393.44 2.902 9.75 30.27
 + ID2= 2 (6332): 468.30 5.567 9.75 34.92
 ID = 3 (8322): 861.74 8.469 9.75 32.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0328) | Area (ha)= 492.92 Curve Number (CN)= 77.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.91
|
Unit Hyd Qpeak (cms)= 4.411
|
PEAK FLOW (cms)= 4.508 (i)
TIME TO PEAK (hrs)= 8.750
RUNOFF VOLUME (mm)= 32.087
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.439

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0326) | Area (ha)= 678.91 Curve Number (CN)= 80.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.95
|
Unit Hyd Qpeak (cms)= 5.941
|
PEAK FLOW (cms)= 6.697 (i)
TIME TO PEAK (hrs)= 8.750
RUNOFF VOLUME (mm)= 35.107
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.480

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0324) | Area (ha)= 615.64 Curve Number (CN)= 79.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.90
|
Unit Hyd Qpeak (cms)= 5.544
|
PEAK FLOW (cms)= 6.021 (i)
TIME TO PEAK (hrs)= 8.750
RUNOFF VOLUME (mm)= 34.061
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.466

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0320) | Area (ha)= 278.74 Curve Number (CN)= 81.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.36
|
Unit Hyd Qpeak (cms)= 3.499
|
PEAK FLOW (cms)= 3.792 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 36.087
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.494

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (3182) | Area (ha)= 457.40 Curve Number (CN)= 78.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.49
|
Unit Hyd Qpeak (cms)= 5.259
|
PEAK FLOW (cms)= 5.284 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 32.999
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.451

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANBYD (3181) | Area (ha)= 26.00
| ID= 1 DT=15.0 min | Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00
|-----|
|
Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 2.00 4.94
Average Slope (%)= 0.50 0.50
Length (m)= 416.33 40.00

```

```

Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 96.49 50.01
over (min)= 15.00 30.00
Storage Coeff. (min)= 7.51 (ii) 21.62 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.10 0.05
*TOTALS*
PEAK FLOW (cms)= 5.13 0.33 5.342 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 71.10 32.21 63.71
TOTAL RAINFALL (mm)= 73.10 73.10 73.10
RUNOFF COEFFICIENT = 0.97 0.44 0.87

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 77.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| RESERVOIR (5318) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
|-----|
| OUTFLOW STORAGE | OUTFLOW STORAGE |
| (cms) (ha.m.) | (cms) (ha.m.) |
| 0.0000 0.0000 | 3.3800 0.7654 |
| 0.0510 0.4423 | 4.1500 0.8231 |
| 0.8700 0.5378 | 4.8750 0.8778 |
| 1.9590 0.6105 | 5.2750 0.8878 |
| 2.5620 0.6796 | 0.0000 0.0000 |

```

```

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID= 2 (3181) 26.000 5.342 6.00 63.71
OUTFLOW: ID= 1 (5318) 26.000 2.943 6.25 63.63

```

```

PEAK FLOW REDUCTION [Qout/ Qin](%)= 55.08
TIME SHIFT OF PEAK FLOW (min)= 15.00
MAXIMUM STORAGE USED (ha.m.)= 0.7756

```

```

-----
| ADD HYD (8334) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (3182): 457.40 5.284 8.00 33.00 |
| + ID2= 2 (5318): 26.00 2.943 6.25 63.63 |
|=====|
| ID = 3 (8334): 483.40 6.378 6.25 34.65 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6320) |
| IN= 2--> OUT= 1 |
|-----|
Routing time step (min)= 15.00

```

```

-----
|-----> DATA FOR SECTION (3201.0) <-----|
| Distance Elevation Manning |
| 0.00 249.00 0.0380 |
| 22.92 245.86 0.0380 |
| 45.83 244.87 0.0380 |
| 91.66 243.11 0.0380 |
| 126.03 239.53 0.0380 |
| 160.41 237.17 0.0380 |
| 166.14 237.06 0.0380 |
| 177.59 237.13 0.0380 |
| 183.32 237.20 0.0380 |
| 189.05 236.70 0.0380 /0.0350 Main Channel |
| 193.05 235.89 0.0350 Main Channel |
| 197.05 236.64 0.0350 /0.0380 Main Channel |
| 200.51 236.74 0.0380 |
| 206.24 237.03 0.0380 |
| 246.34 238.82 0.0380 |
| 263.53 243.87 0.0380 |
| 389.56 247.64 0.0380 |
| 452.58 247.74 0.0380 |
| 498.41 248.60 0.0380 |
| 567.16 249.84 0.0380 |

```

```

-----
|-----> TRAVEL TIME TABLE <-----|
| DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME |
| (m) (m) (cu.m.) (cms) (m/s) (min) |
| 0.37 236.26 .270E+04 0.4 0.58 107.96 |
| 0.75 236.64 .108E+05 2.6 0.92 68.01 |
| 1.48 237.37 .877E+05 27.4 1.17 53.35 |
| 2.20 238.09 .277E+06 122.1 1.65 37.83 |
| 2.93 238.82 .540E+06 295.7 2.08 30.01 |
| 3.66 239.55 .857E+06 587.8 2.57 24.29 |
| 4.39 240.28 .120E+07 977.8 3.04 20.52 |
| 5.11 241.00 .158E+07 1456.1 3.46 18.02 |

```

5.84	241.73	.198E+07	2028.6	3.84	16.23
6.57	242.46	.240E+07	6990.5	4.19	14.86
7.29	243.18	.285E+07	3431.8	4.50	13.84
8.02	243.91	.335E+07	4149.1	4.64	13.44
8.75	244.64	.393E+07	4745.6	4.51	13.80
9.47	245.36	.463E+07	5618.6	4.53	13.75
10.20	246.09	.545E+07	6781.8	4.66	13.39
10.93	246.82	.635E+07	8274.5	4.87	12.79
11.66	247.55	.733E+07	9989.0	5.10	12.23
12.38	248.27	.853E+07	11422.0	5.01	12.45
13.11	249.00	.989E+07	13749.7	5.20	11.98

```

<--- hydrograph ---> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW: ID= 2 (8334) 483.40 6.38 6.25 34.65 0.86 0.95
OUTFLOW: ID= 1 (8320) 483.40 5.21 9.00 34.65 0.82 0.94

```

```

ADD HYD (8332)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0320): 278.74 3.792 8.00 36.09
+ ID2= 2 (6320): 483.40 5.207 9.00 34.65
=====
ID = 3 (8332): 762.14 8.824 8.50 35.17

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB (0314) Area (ha)= 165.20 Curve Number (CN)= 78.0
NASHYD (0314) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.71

```

```

Unit Hyd Qpeak (cms)= 3.978
PEAK FLOW (cms)= 3.404 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 32.624
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.446

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ROUTE CHN (6316) |
IN= 2--> OUT= 1 | Routing time step (min)"= 15.00

```

```

<----- DATA FOR SECTION (3161.0) ----->
Distance Elevation Manning
0.00 248.54 0.0350
205.48 236.14 0.0350 /0.0310 Main Channel
205.98 235.61 0.0310 Main Channel
207.98 235.25 0.0310 Main Channel
209.98 235.53 0.0310 Main Channel
210.48 236.00 0.0310 /0.0350 Main Channel
216.42 236.73 0.0350
258.31 239.09 0.0350
328.12 239.84 0.0350
439.83 241.47 0.0350
530.58 242.08 0.0350
586.43 242.93 0.0350
691.16 248.00 0.0350

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.37 235.62 .215E+04 0.7 0.77 53.88
0.75 236.00 .634E+04 3.5 1.36 30.45
1.46 236.71 .297E+05 19.3 1.62 25.66
2.16 237.41 .103E+06 75.8 1.84 22.60
2.87 238.12 .229E+06 202.8 2.21 18.79
3.57 238.82 .407E+06 423.3 2.59 16.03
4.28 239.53 .654E+06 727.7 2.77 14.97
4.99 240.24 .103E+07 1242.0 3.01 13.76
5.69 240.94 .151E+07 2002.3 3.31 12.54
6.40 241.65 .210E+07 2987.9 3.55 11.70
7.10 242.35 .286E+07 4302.8 3.75 11.07
7.81 243.06 .374E+07 6165.3 4.10 10.11
8.51 243.76 .468E+07 8642.4 4.60 9.03
9.22 244.47 .565E+07 11496.3 5.06 8.20
9.93 245.18 .666E+07 14714.1 5.50 7.54

```

10.63	245.88	.770E+07	18290.3	5.91	7.02
11.34	246.59	.877E+07	22221.9	6.30	6.58
12.04	247.29	.988E+07	26464.4	6.67	6.22
12.75	248.00	.110E+08	31061.3	7.01	5.91

```

<--- hydrograph ---> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW: ID= 2 (0314) 165.20 3.40 6.75 32.62 0.74 1.34
OUTFLOW: ID= 1 (6316) 165.20 3.22 7.25 32.62 0.71 1.27

```

```

CALIB (0316) Area (ha)= 232.34 Curve Number (CN)= 82.0
NASHYD (0316) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.01

```

```

Unit Hyd Qpeak (cms)= 3.923
PEAK FLOW (cms)= 4.118 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 37.068
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.507

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8338)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0316): 232.34 4.118 7.25 37.07
+ ID2= 2 (6316): 165.20 3.215 7.25 32.62
=====
ID = 3 (8338): 397.54 7.333 7.25 35.22

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB (0312) Area (ha)= 359.44 Curve Number (CN)= 80.0
NASHYD (0312) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.36

```

```

Unit Hyd Qpeak (cms)= 4.529
PEAK FLOW (cms)= 4.750 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 35.011
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.479

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB (0308) Area (ha)= 529.30 Curve Number (CN)= 62.0
NASHYD (0308) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.62

```

```

Unit Hyd Qpeak (cms)= 5.575
PEAK FLOW (cms)= 3.491 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 20.621
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.282

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB (0310) Area (ha)= 138.28 Curve Number (CN)= 65.0
NASHYD (0310) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.76

```

```

Unit Hyd Qpeak (cms)= 3.102
PEAK FLOW (cms)= 1.788 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 22.291
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.305

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8342)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0308):	529.30	3.451	8.25	20.62
+ ID2= 2 (0310):	138.28	1.788	7.00	22.29
ID = 3 (8342):	667.58	5.034	7.75	20.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6312) |
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (3121.0)				
Distance	Elevation	Manning		
0.00	265.94	0.0360		
30.07	265.43	0.0360		
59.82	263.98	0.0360		
103.32	254.59	0.0360		
157.70	252.16	0.0360		
217.52	250.45	0.0360		
233.84	247.69	0.0360		
234.71	247.27	0.0360		
239.71	246.38	0.0360		
244.71	246.12	0.0360 / 0.0330	Main Channel	
247.71	245.17	0.0330	Main Channel	
249.71	245.19	0.0330	Main Channel	
251.71	245.64	0.0330 / 0.0360	Main Channel	
259.71	246.47	0.0360		
282.78	247.12	0.0360		
315.41	251.60	0.0360		
424.17	256.13	0.0360		
478.55	257.04	0.0360		
516.62	259.37	0.0360		
538.37	266.00	0.0360		

TRAVEL TIME TABLE						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.47	245.64	.111E+04	3.1	1.78	6.07	
1.54	246.71	.103E+05	53.9	3.39	3.19	
2.61	247.78	.414E+05	281.3	4.40	2.45	
3.68	248.85	.839E+05	749.1	5.79	1.87	
4.74	249.91	.136E+06	1463.2	6.97	1.55	
5.81	250.98	.201E+06	2354.8	7.61	1.42	
6.88	252.05	.295E+06	3533.0	7.76	1.39	
7.95	253.12	.407E+06	5303.9	8.05	1.34	
9.02	254.19	.549E+06	7856.5	8.58	1.26	
10.09	255.26	.792E+06	11449.2	9.37	1.15	
11.15	256.32	.101E+07	15862.4	10.15	1.06	
12.22	257.39	.127E+07	21074.5	10.75	1.01	
13.29	258.46	.155E+07	28184.1	11.78	0.92	
14.36	259.53	.185E+07	36386.2	12.77	0.85	
15.43	260.60	.215E+07	46110.6	13.89	0.78	
16.50	261.67	.246E+07	56822.5	14.95	0.72	
17.56	262.73	.278E+07	68498.1	15.97	0.68	
18.63	263.80	.310E+07	81118.8	16.94	0.64	
19.70	264.87	.344E+07	93603.2	17.66	0.61	

<---- hydrograph ----> <-pipe / channel-->						
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8342)	667.58	5.03	7.75	20.97	0.51	1.81
OUTFLOW : ID= 1 (6312)	667.58	5.03	8.00	20.97	0.51	1.81

ADD HYD (8340)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0312):	359.44	4.750	8.00	35.01
+ ID2= 2 (6313):	667.58	5.033	8.00	20.97
ID = 3 (8340):	1027.02	9.783	8.00	25.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8336)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8338):	397.54	7.333	7.25	35.22
+ ID2= 2 (8340):	1027.02	9.783	8.00	25.88
ID = 3 (8336):	1424.56	16.950	7.50	28.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8330)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8332):	762.14	8.824	8.50	35.17
+ ID2= 2 (8336):	1424.56	16.950	7.50	28.49
ID = 3 (8330):	2186.70	25.333	7.75	30.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6324) |
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (3241.0)				
Distance	Elevation	Manning		
0.00	234.38	0.0360		
33.57	232.48	0.0360		
67.14	230.14	0.0360		
83.93	228.80	0.0360		
134.29	227.62	0.0360		
209.82	225.10	0.0360		
218.21	224.86	0.0360		
226.61	224.47	0.0360		
234.00	223.86	0.0360 / 0.0300	Main Channel	
234.10	223.66	0.0300	Main Channel	
235.00	223.66	0.0300	Main Channel	
235.90	223.66	0.0300	Main Channel	
236.00	223.86	0.0300 / 0.0360	Main Channel	
243.39	224.92	0.0360		
251.78	224.89	0.0360		
335.71	225.69	0.0360		
562.32	226.53	0.0360		
637.85	228.36	0.0360		
705.00	229.80	0.0360		
830.89	234.00	0.0360		

TRAVEL TIME TABLE						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.20	223.86	.145E+04	0.2	0.43	147.27	
0.73	224.39	.159E+05	2.8	0.68	93.11	
1.27	224.93	.561E+05	11.0	0.75	85.19	
1.80	225.46	.217E+06	45.6	0.80	79.20	
2.33	225.99	.566E+06	134.4	0.91	70.20	
2.87	226.53	.122E+07	338.1	1.06	59.97	
3.40	227.06	.206E+07	744.3	1.38	46.16	
3.94	227.60	.298E+07	1295.9	1.66	38.38	
4.47	228.13	.399E+07	1979.9	1.90	33.59	
5.00	228.66	.509E+07	2807.5	2.11	30.21	
5.54	229.20	.627E+07	3815.1	2.32	27.41	
6.07	229.73	.753E+07	4984.1	2.53	25.17	
6.60	230.26	.884E+07	6346.3	2.74	23.20	
7.14	230.80	.102E+08	7863.2	2.95	21.61	
7.67	231.33	.116E+08	9526.4	3.14	20.29	
8.21	231.87	.131E+08	11342.2	3.32	19.18	
8.74	232.40	.146E+08	13309.6	3.49	18.22	
9.27	232.93	.161E+08	15414.0	3.66	17.41	
9.81	233.47	.177E+08	17671.0	3.81	16.70	

<---- hydrograph ----> <-pipe / channel-->						
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8330)	2186.70	25.33	7.75	30.82	1.49	0.77
OUTFLOW : ID= 1 (6324)	2186.70	21.73	9.50	30.82	1.43	0.76

ADD HYD (8328)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0324):	615.64	6.021	8.75	34.06
+ ID2= 2 (6324):	2186.70	21.726	9.50	30.82
ID = 3 (8328):	2802.34	27.648	9.50	31.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB |
| NASHYD (0322) | Area (ha)= 513.13 Curve Number (CN)= 80.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.75

Unit Hyd Qpeak (cms)= 5.020
PEAK FLOW (cms)= 5.538 (1)
TIME TO PEAK (hrs)= 8.500

RUNOFF VOLUME (mm)= 35.083
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.480

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----
| ADD HYD (8326) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0326): 513.13 5.538 8.50 35.08 |
| + ID2= 2 (8328): 2802.34 27.648 9.50 31.53 |
|-----|
| ID = 3 (8326): 3315.47 32.995 9.50 32.08 |
|-----|
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
-----
| ADD HYD (8324) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0326): 678.91 6.697 8.75 35.11 |
| + ID2= 2 (8326): 3315.47 32.995 9.50 32.08 |
|-----|
| ID = 3 (8324): 3994.38 39.647 9.25 32.59 |
|-----|
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
-----
| ROUTE CHN (8328) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00
|-----|
```

```
-----
|----- DATA FOR SECTION (3281.0) -----|
| Distance Elevation Manning |
| 0.00 228.00 0.0380 |
| 18.58 224.97 0.0380 |
| 78.98 223.52 0.0380 |
| 125.44 223.28 0.0380 |
| 171.90 221.71 0.0380 |
| 213.72 219.65 0.0380 |
| 218.36 219.40 0.0380 |
| 223.01 219.19 0.0380 |
| 225.95 219.14 0.0380 /0.0320 Main Channel |
| 226.45 218.14 0.0320 Main Channel |
| 236.95 218.14 0.0320 Main Channel |
| 245.85 218.14 0.0320 Main Channel |
| 245.95 219.14 0.0320 /0.0380 Main Channel |
| 246.24 219.16 0.0380 |
| 250.88 219.24 0.0380 |
| 255.53 219.39 0.0380 |
| 325.22 221.47 0.0380 |
| 367.03 223.14 0.0380 |
| 404.20 225.17 0.0380 |
| 459.95 228.04 0.0380 |
|-----|
```

```
-----
|----- TRAVEL TIME TABLE -----|
| DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME |
| (m) (m) (cu.m.) (cms) (m/s) (min) |
| 0.50 218.64 .373E+05 3.2 0.33 192.37 |
| 1.00 219.14 .752E+05 10.1 0.51 124.71 |
| 1.52 219.66 .149E+06 22.2 0.57 112.06 |
| 2.04 220.18 .279E+06 43.4 0.59 107.06 |
| 2.56 220.70 .464E+06 77.0 0.63 100.52 |
| 3.08 221.22 .705E+06 125.7 0.68 93.56 |
| 3.61 221.75 .100E+07 192.7 0.73 86.62 |
| 4.13 222.27 .135E+07 278.1 0.79 80.92 |
| 4.65 222.79 .176E+07 385.9 0.84 75.86 |
| 5.17 223.31 .222E+07 514.9 0.89 71.82 |
| 5.69 223.83 .280E+07 662.7 0.90 70.40 |
| 6.21 224.35 .345E+07 867.7 0.96 66.35 |
| 6.73 224.87 .417E+07 1107.0 1.01 62.81 |
| 7.25 225.39 .494E+07 1400.0 1.08 58.80 |
| 7.78 225.92 .573E+07 1732.2 1.15 55.17 |
| 8.30 226.44 .655E+07 2099.9 1.22 52.02 |
| 8.82 226.96 .740E+07 2502.8 1.29 49.29 |
| 9.34 227.48 .828E+07 2940.9 1.36 46.90 |
| 9.86 228.00 .918E+07 3414.3 1.42 44.79 |
|-----|
```

```
-----
|----- hydrograph ----> <-pipe / channel->
| AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL |
| (ha) (cms) (hrs) (mm) (m) (m/s) |
| INFLOW : ID= 2 (8324) 3994.38 39.65 9.25 32.59 1.95 0.59 |
| OUTFLOW : ID= 1 (8328) 3994.38 33.85 11.00 32.59 1.81 0.58 |
|-----|
```

```
-----
| ADD HYD (8320) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0326): 513.13 5.538 8.50 35.08 |
| + ID2= 2 (8328): 2802.34 27.648 9.50 31.53 |
|-----|
| ID = 3 (8320): 3315.47 32.995 9.50 32.08 |
|-----|
```

```
-----
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0328): 492.92 4.508 8.75 32.09 |
| + ID2= 2 (8328): 3994.38 33.850 11.00 32.59 |
|-----|
| ID = 3 (8320): 4487.30 37.891 11.00 32.54 |
|-----|
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
-----
| ADD HYD (8318) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (8320): 4487.30 37.891 11.00 32.54 |
| + ID2= 2 (8322): 861.74 8.469 9.75 32.80 |
|-----|
| ID = 3 (8318): 5349.04 46.022 10.75 32.58 |
|-----|
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
-----
| ADD HYD (8316) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (8309): 10059.18 45.744 15.25 26.63 |
| + ID2= 2 (8318): 5349.04 46.022 10.75 32.58 |
|-----|
| ID = 3 (8316): 15408.22 86.971 11.75 28.69 |
|-----|
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
-----
| ADD HYD (8312) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (8314): 620.10 1.209 6.75 18.32 |
| + ID2= 2 (8316): 15408.22 86.971 11.75 28.69 |
|-----|
| ID = 3 (8312): 16028.32 87.875 11.75 28.29 |
|-----|
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
-----
| ADD HYD (8308) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (8310): 598.90 27.843 6.50 36.29 |
| + ID2= 2 (8312): 16028.32 87.875 11.75 28.29 |
|-----|
| ID = 3 (8308): 16627.22 89.765 11.50 28.58 |
|-----|
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
-----
| RESERVOIR (5510) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
|-----|
| OUTFLOW STORAGE | OUTFLOW STORAGE |
| (cms) (ha.m.) | (cms) (ha.m.) |
| 0.0000 0.0000 | ***** 74.0090 |
| 66.5450 18.5023 | ***** |
| 98.5430 37.0045 | ***** |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| INFLOW : ID= 2 (8308) ***** 89.765 11.50 28.58 |
| OUTFLOW : ID= 1 (5510) ***** 85.396 13.25 28.58 |
|-----|
```

PEAK FLOW REDUCTION [Qout/Qin] (%)= 95.13
 TIME SHIFT OF PEAK FLOW (min)=105.00
 MAXIMUM STORAGE USED (ha.m.)= 29.4137

```
-----
| ADD HYD (8240) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0336): 2785.00 3.370 22.25 27.75 |
| + ID2= 2 (5510): 16627.22 85.396 13.25 28.58 |
|-----|
| ID = 3 (8240): 19412.22 88.298 13.50 28.47 |
|-----|
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8238)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (5508): 9524.23 42.683 12.25 17.78
+ ID2= 2 (8240): 19412.22 86.298 13.50 28.47
=====
ID = 3 (8238): 28936.45 130.148 12.75 24.95

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8236)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0342): 1004.58 3.660 10.00 16.26
+ ID2= 2 (8238): 28936.45 130.148 12.75 24.95
=====
ID = 3 (8236): 29941.03 133.430 12.75 24.66

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8234)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8232): 285.80 8.040 6.00 24.37
+ ID2= 2 (8236): 29941.03 133.430 12.75 24.66
=====
ID = 3 (8234): 30226.83 133.766 12.75 24.65

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8230)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0344): 344.00 3.342 7.00 18.72
+ ID2= 2 (8234): 30226.83 133.766 12.75 24.65
=====
ID = 3 (8230): 30570.83 134.707 12.75 24.59

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8228)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8226): 1952.06 11.133 8.75 15.62
+ ID2= 2 (8230): 30570.83 134.707 12.75 24.59
=====
ID = 3 (8228): 32522.89 141.797 12.50 24.05

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8190)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0356): 1015.22 4.610 8.00 12.58
+ ID2= 2 (8228): 32522.89 141.797 12.50 24.05
=====
ID = 3 (8190): 33538.11 144.663 12.25 23.70

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

** SIMULATION NUMBER: 13 **

```

READ STORM
Filename: C:\Users\jascott\AppData
Local\Temp\
3e280798-92ee-4282-809c-79f5caed0add\fs74c8d3
Ptotal= 62.70 mm
Comments: 10-Year 12-Hour SCS II Design Storm

```

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.57	3.25	2.51	6.25	11.29	9.25	2.19
0.50	1.57	3.50	2.51	6.50	11.29	9.50	2.19
0.75	1.57	3.75	2.51	6.75	5.02	9.75	2.19
1.00	1.57	4.00	2.51	7.00	5.02	10.00	2.19
1.25	1.57	4.25	3.76	7.25	3.76	10.25	1.25
1.50	1.57	4.50	3.76	7.50	3.76	10.50	1.25

1.75	1.57	4.75	5.02	7.75	3.76	10.75	1.25
2.00	1.57	5.00	5.02	8.00	3.76	11.00	1.25
2.25	1.88	5.25	7.52	8.25	2.19	11.25	1.25
2.50	1.88	5.50	7.52	8.50	2.19	11.50	1.25
2.75	1.98	5.75	30.10	8.75	2.19	11.75	1.25
3.00	1.88	6.00	82.76	9.00	2.19	12.00	1.25

```

CALIB
NASHYD (0356) Area (ha)=1015.22 Curve Number (CN)= 46.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.37

```

Unit Hyd Qpeak (cms)= 12.651

PEAK FLOW (cms)= 3.392 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 9.295
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.148

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0354) Area (ha)= 262.68 Curve Number (CN)= 37.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.38

```

Unit Hyd Qpeak (cms)= 3.252

PEAK FLOW (cms)= 0.631 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 6.749
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.108

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0352) Area (ha)= 381.43 Curve Number (CN)= 54.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.82

```

Unit Hyd Qpeak (cms)= 7.980

PEAK FLOW (cms)= 2.453 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 11.980
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.191

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0346) Area (ha)= 350.93 Curve Number (CN)= 70.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.96

```

Unit Hyd Qpeak (cms)= 6.254

PEAK FLOW (cms)= 3.349 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 19.771
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.315

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0350) Area (ha)= 366.84 Curve Number (CN)= 48.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.07

```

Unit Hyd Qpeak (cms)= 5.831

PEAK FLOW (cms)= 1.564 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 9.910
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.158

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

OUTFLOW: ID= 1 (5510) ***** 105.320 13.75 38.94

PEAK FLOW REDUCTION [Qout/Qin](%)= 92.90
TIME SHIFT OF PEAK FLOW (min)=120.00
MAXIMUM STORAGE USED (ha.m.)= 44.9262

ADD HYD (8240)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0336): 2785.00 4.639 22.25 38.20
+ ID2= 2 (5510): 16627.22 105.320 13.75 38.94
ID = 3 (8240): 19412.22 109.428 14.00 38.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8238)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (5508): 9524.23 56.167 11.75 25.09
+ ID2= 2 (8240): 19412.22 109.428 14.00 38.84
ID = 3 (8238): 28936.45 163.719 13.00 34.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8236)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0344): 1004.58 5.230 10.00 23.19
+ ID2= 2 (8238): 28936.45 163.719 13.00 34.32
ID = 3 (8236): 29941.03 168.365 12.75 33.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8234)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8232): 285.80 10.972 6.50 32.52
+ ID2= 2 (8236): 29941.03 168.365 12.75 33.94
ID = 3 (8234): 30226.83 168.774 12.50 33.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8230)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0344): 344.00 4.771 7.00 26.48
+ ID2= 2 (8234): 30226.83 168.774 12.50 33.93
ID = 3 (8230): 30570.83 170.215 12.50 33.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8228)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8226): 1952.06 15.927 8.75 22.21
+ ID2= 2 (8230): 30570.83 170.215 12.50 33.84
ID = 3 (8228): 32522.89 180.663 12.00 33.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8190)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0356): 1015.22 6.685 8.00 18.15
+ ID2= 2 (8228): 32522.89 180.663 12.00 33.15
ID = 3 (8190): 33538.11 185.041 12.00 32.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM
Filename: C:\Users\jsscott\AppData\Local\Temp\3e280798-92ee-4282-809c-79f5caed0add\9255d239
3e280798-92ee-4282-809c-79f5caed0add\9255d239
Comments: 50-Year 12-Hour SCS II Design Storm

***** SIMULATION NUMBER: 11 *****

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	2.02	3.25	3.23	6.25	14.54	9.25	2.83
0.50	2.02	3.50	3.23	6.50	14.54	9.50	2.83
0.75	2.02	3.75	3.23	6.75	6.46	9.75	2.83
1.00	2.02	4.00	3.23	7.00	6.46	10.00	2.83
1.25	2.02	4.25	4.85	7.25	4.85	10.25	1.62
1.50	2.02	4.50	4.85	7.50	4.85	10.50	1.62
1.75	2.02	4.75	6.46	7.75	4.85	10.75	1.62
2.00	2.02	5.00	6.46	8.00	4.85	11.00	1.62
2.25	2.42	5.25	9.70	8.25	2.83	11.25	1.62
2.50	2.42	5.50	9.70	8.50	2.83	11.50	1.62
2.75	2.42	5.75	38.78	8.75	2.83	11.75	1.62
3.00	2.42	6.00	106.66	9.00	2.83	12.00	1.62

CALIB (0356) Area (ha)=1015.22 Curve Number (CN)= 46.0
NASHYD (0356) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.37

Unit Hyd Qpeak (cms)= 12.651
PEAK FLOW (cms)= 5.609 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 15.266
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.189

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0354) Area (ha)= 262.68 Curve Number (CN)= 37.0
NASHYD (0354) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.38

Unit Hyd Qpeak (cms)= 3.252
PEAK FLOW (cms)= 1.056 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 11.232
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.139

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0352) Area (ha)= 381.43 Curve Number (CN)= 54.0
NASHYD (0352) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.82

Unit Hyd Qpeak (cms)= 7.980
PEAK FLOW (cms)= 4.018 (1)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 19.394
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.240

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0346) Area (ha)= 350.93 Curve Number (CN)= 70.0
NASHYD (0346) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 6.254
PEAK FLOW (cms)= 5.278 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 30.777
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.381

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0350) Area (ha)= 366.84 Curve Number (CN)= 48.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.07

Unit Hyd Opeak (cms)= 5.831
PEAK FLOW (cms)= 2.583 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 16.221
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.201

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0348) Area (ha)= 590.18 Curve Number (CN)= 48.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.91

Unit Hyd Opeak (cms)= 5.267
PEAK FLOW (cms)= 2.683 (i)
TIME TO PEAK (hrs)= 9.000
RUNOFF VOLUME (mm)= 16.307
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.202

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6350)
IN= 2--> OUT= 1
Routing time step (min)'= 15.00

Table with columns: Distance, Elevation, Manning. Data points from 0.00 to 585.54.

TRAVEL TIME TABLE
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)

hydrograph <-pipe / channel->
AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)

ADD HYD (8220)
1 + 2 = 3
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8222)
1 + 2 = 3
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6352)
IN= 2--> OUT= 1
Routing time step (min)'= 15.00

DATA FOR SECTION (3521.0)
Distance Elevation Manning
0.00 287.95 0.0500

TRAVEL TIME TABLE
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)

hydrograph <-pipe / channel->
AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)

ADD HYD (8224)
1 + 2 = 3
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CN (6354) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
-----

```

```

-----
<----- DATA FOR SECTION (3541.0) ----->
Distance Elevation Manning
0.00 253.92 0.0500
7.95 251.83 0.0500
15.89 249.97 0.0500
19.87 249.05 0.0500
47.68 242.00 0.0500
67.55 237.10 0.0500
83.44 231.11 0.0500
85.43 230.75 0.0500
85.66 230.61 0.0500 /0.0300 Main Channel
85.76 230.01 0.0300 Main Channel
87.41 230.01 0.0300 Main Channel
88.06 230.01 0.0300 Main Channel
88.16 230.61 0.0300 /0.0500 Main Channel
89.40 230.72 0.0500
91.39 230.88 0.0500
93.37 231.03 0.0500
133.11 233.96 0.0500
150.99 235.39 0.0500
172.84 239.03 0.0500
196.68 250.00 0.0500
-----

```

```

-----
<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.60 230.61 .451E+04 1.5 1.06 49.40
1.62 231.63 .394E+05 14.6 1.16 45.13
2.64 232.65 .128E+06 58.8 1.44 36.30
3.66 233.67 .269E+06 149.7 1.74 30.00
4.68 234.69 .463E+06 301.5 2.04 25.58
5.70 235.71 .705E+06 530.5 2.36 22.14
6.72 236.73 .980E+06 848.4 2.71 19.25
7.74 237.75 .128E+07 1237.9 3.02 17.29
8.76 238.77 .162E+07 1706.7 3.30 15.83
9.78 239.79 .199E+07 2291.9 3.61 14.45
10.81 240.82 .237E+07 2973.7 3.92 13.30
11.83 241.84 .278E+07 3742.2 4.21 12.38
12.85 242.86 .321E+07 4599.1 4.49 11.62
13.87 243.88 .365E+07 5544.6 4.75 10.98
14.89 244.90 .412E+07 6579.7 5.00 10.44
15.91 245.92 .461E+07 7705.9 5.23 9.97
16.93 246.94 .512E+07 8924.6 5.46 9.55
17.95 247.96 .564E+07 10237.5 5.68 9.19
18.97 248.98 .619E+07 11646.2 5.89 8.86
-----

```

```

-----
<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8224) 1689.38 13.28 7.75 19.99 1.52 1.15
OUTFLOW : ID= 1 (6354) 1689.38 12.41 8.75 19.99 1.45 1.14
-----

```

```

-----
| ADD HYD (8226) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
+ ID1= 1 (0354): 262.68 1.056 8.00 11.23
+ ID2= 2 (6354): 1689.38 12.412 8.75 19.99
=====
ID = 3 (8226): 1952.06 13.442 8.75 18.81
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB (0344) |
| NASHVD (0344) | Area (ha)= 344.00 Curve Number (CN)= 59.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.87 |
-----

```

```

Unit Hyd Qpeak (cms)= 6.790
PEAK FLOW (cms)= 4.034 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 22.484
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.278
-----

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANDHYD (3402) | Area (ha)= 234.60
| ID= 1 DT=15.0 min | Total Imp(%)= 28.00 Dir. Conn.(%)= 13.00
-----

```

```

-----
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 65.69 168.91
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1250.60 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 106.66 21.15
over (min) 15.00 45.00
Storage Coeff. (min)= 13.95 (ii) 33.87 (ii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.07 0.03
*TOTALS*
PEAK FLOW (cms)= 6.78 5.80 9.169 (iii)
TIME TO PEAK (hrs)= 6.00 6.50 6.00
RUNOFF VOLUME (mm)= 78.80 19.31 27.04
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.24 0.33
-----

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANDHYD (3401) | Area (ha)= 51.20
| ID= 1 DT=15.0 min | Total Imp(%)= 46.00 Dir. Conn.(%)= 21.00
-----

```

```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 23.55 27.65
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 584.24 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 106.66 49.39
over (min) 15.00 30.00
Storage Coeff. (min)= 8.84 (ii) 23.02 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.09 0.04
*TOTALS*
PEAK FLOW (cms)= 2.78 1.79 3.900 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 78.80 22.56 34.37
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.28 0.43
-----

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| RESERVOIR (5340) |
| IN= 2--> OUT= 1 |
-----
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
0.0000 0.0000 | 5.3360 1.7877
0.1730 0.6348 | 7.2780 2.2185
1.8360 0.8250 | 9.1880 2.6486
2.1540 1.3051 | 9.5880 2.6586
3.3950 1.3506 | 0.0000 0.0000
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (3401) 51.200 3.900 6.00 34.37
OUTFLOW : ID= 1 (5340) 51.200 1.657 6.50 34.35
-----

```

```

PEAK FLOW REDUCTION [Qout/Qin](%)= 42.48
TIME SHIFT OF PEAK FLOW (min)= 30.00
MAXIMUM STORAGE USED (ha.m.)= 0.8094
-----

```

```

-----
| ADD HYD (8232) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
+ ID1= 1 (3402): 234.60 9.169 6.00 27.04
+ ID2= 2 (5340): 51.20 1.657 6.50 34.35
=====
ID = 3 (8232): 285.80 9.337 6.50 28.35
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHVD (0342) Area (ha)=1004.58 Curve Number (CN)= 54.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 2.57

Unit Hyd Qpeak (cms)= 6.686
PEAK FLOW (cms)= 4.420 (i)
TIME TO PEAK (hrs)= 10.000
RUNOFF VOLUME (mm)= 19.616
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.243

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0422) Area (ha)= 780.20 Curve Number (CN)= 54.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.95

Unit Hyd Qpeak (cms)= 6.838
PEAK FLOW (cms)= 4.215 (i)
TIME TO PEAK (hrs)= 9.000
RUNOFF VOLUME (mm)= 19.591
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.242

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0410) Area (ha)= 572.01 Curve Number (CN)= 48.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.46

Unit Hyd Qpeak (cms)= 6.698
PEAK FLOW (cms)= 3.210 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 16.276
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.201

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0408) Area (ha)= 231.62 Curve Number (CN)= 58.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.64

Unit Hyd Qpeak (cms)= 6.198
PEAK FLOW (cms)= 3.331 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 21.683
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.268

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0376) Area (ha)= 463.85 Curve Number (CN)= 74.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 7.380
PEAK FLOW (cms)= 7.184 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 34.493
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.427

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0374) Area (ha)= 545.70 Curve Number (CN)= 61.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.51

Unit Hyd Qpeak (cms)= 6.158

PEAK FLOW (cms)= 4.432 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 23.989
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.297

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0372) Area (ha)= 110.42 Curve Number (CN)= 37.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 1.954
PEAK FLOW (cms)= 0.576 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 11.182
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.138

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0370) Area (ha)= 191.85 Curve Number (CN)= 63.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.67

Unit Hyd Qpeak (cms)= 4.860
PEAK FLOW (cms)= 3.080 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 25.072
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.310

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0368) Area (ha)= 159.48 Curve Number (CN)= 46.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.12

Unit Hyd Qpeak (cms)= 2.433
PEAK FLOW (cms)= 1.020 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 15.231
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.189

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0366) Area (ha)= 462.62 Curve Number (CN)= 62.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.06

Unit Hyd Qpeak (cms)= 7.451
PEAK FLOW (cms)= 5.064 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 24.589
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.304

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0364) Area (ha)= 155.27 Curve Number (CN)= 55.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.70

Unit Hyd Qpeak (cms)= 3.782
PEAK FLOW (cms)= 1.891 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 19.909
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.246

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8302) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
|-----|
| ID1= 1 (0364): 155.27 1.891 6.75 19.31
| + ID2= 2 (0366): 462.62 5.064 7.50 24.59
|-----|
| ID = 3 (8302): 617.89 6.881 7.25 23.41
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| STANHYD (0362) | Area (ha)= 118.78
| ID= 1 DT=15.0 min | Total Imp(%)= 22.00 Dir. Conn.(%)= 8.00
|-----|

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```

Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
26.13 92.65
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 889.87 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 106.66 18.31
cover (min)= 15.00 45.00
Storage Coeff. (min)= 11.38 (ii) 32.47 (ii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.08 0.03

*TOTALS*
PEAK FLOW (cms)= 2.28 2.82 3.434 (iii)
TIME TO PEAK (hrs)= 6.00 6.50 6.00
RUNOFF VOLUME (mm)= 78.80 17.25 22.18
TOTAL RAINFALL (mm)= 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.21 0.27

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0358) | Area (ha)= 429.87 Curve Number (CN)= 35.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.03
|-----|

```

```

Unit Hyd Qpeak (cms)= 7.091

PEAK FLOW (cms)= 1.973 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 10.392
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.129

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0360) | Area (ha)= 138.37 Curve Number (CN)= 46.0
| ID= 1 DT=10 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.60
|-----|

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```

Unit Hyd Qpeak (cms)= 3.957

PEAK FLOW (cms)= 1.427 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 15.028
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.186

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8306) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
|-----|
| ID1= 1 (0358): 429.87 1.973 7.50 10.39
| + ID2= 2 (0360): 138.37 1.427 6.75 15.03
|-----|
| ID = 3 (8306): 568.24 3.321 7.00 11.52
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6362) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00
|-----|

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-----
| DATA FOR SECTION (3621.0) -----
|-----|
| Distance Elevation Manning |
|-----|
| 0.00 261.46 0.0550 |
| 27.86 254.23 0.0550 |
| 51.07 251.96 0.0550 |
| 74.29 250.77 0.0550 |
| 97.50 249.91 0.0550 |
| 125.36 249.40 0.0550 |
| 150.93 247.40 0.0550 |
| 155.93 247.33 0.0550 / 0.0350 Main Channel |
| 157.93 246.85 0.0350 Main Channel |
| 159.18 246.65 0.0350 Main Channel |
| 160.18 246.63 0.0350 Main Channel |
| 160.93 246.85 0.0350 Main Channel |
| 161.93 247.18 0.0350 / 0.0550 Main Channel |
| 163.18 248.03 0.0550 |
| 168.18 248.58 0.0550 |
| 183.18 250.18 0.0550 |
| 201.97 252.59 0.0550 |
| 213.57 256.02 0.0550 |
| 225.18 260.31 0.0550 |
| 229.82 261.00 0.0550 |
|-----|

```

```

-----
| TRAVEL TIME TABLE -----
|-----|
| DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME |
| (m) (m) (cu.m.) (cms) (m/s) (min) |
|-----|
| 0.55 247.18 .558E+04 1.9 1.03 49.95 |
| 1.28 247.91 .338E+05 16.7 1.53 33.61 |
| 2.00 248.63 .913E+05 53.6 1.81 28.40 |
| 2.73 249.36 .185E+06 122.3 2.05 25.18 |
| 3.46 250.09 .348E+06 224.6 2.02 25.52 |
| 4.19 250.82 .574E+06 417.9 2.25 22.91 |
| 4.91 251.54 .855E+06 699.1 2.53 20.38 |
| 5.64 252.27 .118E+07 1075.3 2.82 18.26 |
| 6.37 253.00 .153E+07 1559.9 3.15 16.38 |
| 7.10 253.73 .191E+07 2137.6 3.46 14.89 |
| 7.82 254.45 .231E+07 2815.2 3.77 13.67 |
| 8.55 255.18 .272E+07 3608.5 4.10 12.58 |
| 9.28 255.91 .315E+07 4488.2 4.41 11.69 |
| 10.01 256.64 .358E+07 5458.0 4.71 10.95 |
| 10.73 257.36 .403E+07 6512.4 4.99 10.32 |
| 11.46 258.09 .449E+07 7649.8 5.27 9.78 |
| 12.19 258.82 .496E+07 8869.5 5.53 9.32 |
| 12.92 259.55 .544E+07 10171.2 5.78 8.91 |
| 13.64 260.27 .593E+07 11554.8 6.02 8.55 |
|-----|

```

```

-----
| hydrograph -----
|-----|
| AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL |
| (ha) (cms) (hrs) (mm) (m) (m/s) |
|-----|
| INFLOW : ID= 2 (8306) 568.24 3.32 7.00 11.52 0.62 1.07 |
| OUTFLOW : ID= 1 (6362) 568.24 2.94 8.25 11.52 0.60 1.06 |
|-----|

```

```

-----
| ADD HYD (8304) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
|-----|
| ID1= 1 (0362): 118.78 3.434 6.00 22.18
| + ID2= 2 (6362): 568.24 2.938 8.25 11.52
|-----|
| ID = 3 (8304): 687.02 4.850 6.50 13.36
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8300) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
|-----|
| ID1= 1 (8302): 617.89 6.881 7.25 23.41
| + ID2= 2 (8304): 687.02 4.850 6.50 13.36
|-----|
| ID = 3 (8300): 1304.91 11.147 7.00 18.12
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6368) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00
|-----|

```

```

-----
| DATA FOR SECTION (3681.0) -----
|-----|
| Distance Elevation Manning |
|-----|
| 0.00 230.00 0.0370 |
| 18.48 223.26 0.0370 |
| 36.96 223.05 0.0370 |
| 64.67 222.94 0.0370 |
| 110.87 222.86 0.0370 |
|-----|

```


133.96	222.74	0.0370	
147.82	222.65	0.0370	
170.92	222.31	0.0370	
174.79	222.26	0.0370 / 0.0300	Main Channel
174.89	221.86	0.0300	Main Channel
175.54	221.86	0.0300	Main Channel
176.19	221.86	0.0300	Main Channel
176.29	222.26	0.0300 / 0.0370	Main Channel
180.16	222.25	0.0370	
184.78	222.28	0.0370	
189.40	222.31	0.0370	
332.60	222.37	0.0370	
450.00	230.00	0.0370	

731.59	219.46	0.0390
762.39	225.00	0.0390

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.40	222.26	.771E+03	0.3	0.52	41.74
0.81	222.67	.784E+05	30.3	0.51	43.17
1.21	223.07	.210E+06	123.2	0.77	28.42
1.62	223.48	.384E+06	301.0	1.03	21.25
2.03	223.89	.563E+06	550.2	1.28	17.06
2.44	224.30	.747E+06	860.7	1.51	14.46
2.84	224.70	.934E+06	1228.1	1.73	12.68
3.25	225.11	.113E+07	1649.3	1.92	11.37
3.66	225.52	.132E+07	2122.3	2.11	10.37
4.07	225.93	.152E+07	2645.5	2.29	9.58
4.47	226.33	.172E+07	3217.8	2.45	8.92
4.88	226.74	.193E+07	3838.3	2.61	8.38
5.29	227.15	.214E+07	4506.3	2.76	7.92
5.70	227.56	.236E+07	5221.3	2.91	7.52
6.10	227.96	.258E+07	5983.0	3.05	7.17
6.51	228.37	.280E+07	6790.9	3.19	6.87
6.92	228.78	.303E+07	7645.0	3.32	6.60
7.33	229.19	.326E+07	8545.1	3.45	6.35
7.73	229.59	.349E+07	9491.0	3.57	6.13

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8300)	1304.91	11.15	7.00	18.12	0.55
OUTFLOW : ID= 1 (6368)	1304.91	10.16	8.00	18.12	0.53

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.29	218.80	.431E+04	1.7	0.58	43.48
0.58	219.09	.867E+04	5.1	0.89	28.32
0.93	219.44	.200E+05	58.2	0.44	57.35
1.28	219.79	.568E+05	278.6	0.74	33.99
1.62	220.13	.939E+05	630.2	1.01	24.84
1.97	220.48	1.31E+06	1090.4	1.25	20.05
2.32	220.83	1.69E+06	1647.7	1.47	17.06
2.67	221.18	2.06E+06	2294.6	1.68	14.99
3.01	221.52	2.44E+06	3025.6	1.87	13.46
3.36	221.87	2.82E+06	3836.5	2.05	12.27
3.71	222.22	3.21E+06	4723.7	2.22	11.31
4.06	222.57	3.59E+06	5684.6	2.39	10.53
4.40	222.91	3.98E+06	6716.6	2.55	9.87
4.75	223.26	4.37E+06	7817.7	2.70	9.31
5.10	223.61	4.76E+06	8986.2	2.85	8.83
5.45	223.96	5.15E+06	10220.4	2.99	8.40
5.79	224.30	5.55E+06	11519.1	3.13	8.03
6.14	224.65	5.94E+06	12880.8	3.27	7.69
6.49	225.00	6.34E+06	14304.5	3.40	7.39

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8296)	1656.24	13.71	7.75	18.65	0.64
OUTFLOW : ID= 1 (6372)	1656.24	13.13	8.50	18.65	0.63

ADD HYD (8294)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0372)	110.42	0.576	7.25	11.18
+ ID2= 2 (6372)	1656.24	13.134	8.50	18.65
ID = 3 (8294)	1766.66	13.657	8.50	18.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8298)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0368)	159.48	1.020	7.75	15.23
+ ID2= 2 (6368)	1304.91	10.162	8.00	18.12
ID = 3 (8298)	1464.39	11.178	8.00	17.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8296)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0370)	191.85	3.080	6.75	25.07
+ ID2= 2 (8296)	1464.39	11.178	8.00	17.81
ID = 3 (8296)	1656.24	13.707	7.75	18.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6372)	ROUTING time step (min)='
IN= 2--> OUT= 1	15.00

DATA FOR SECTION (3721.0) ----->

Distance	Elevation	Manning
0.00	225.00	0.0390
30.80	219.38	0.0390
61.61	219.30	0.0390
77.01	219.27	0.0390
469.76	219.14	0.0390
477.46	219.13	0.0390
485.16	219.10	0.0390
492.86	219.09	0.0390
495.56	219.09	0.0390 / 0.0310
495.66	218.51	0.0310
500.56	218.51	0.0310
505.46	218.51	0.0310
505.56	219.09	0.0310 / 0.0390
508.26	219.09	0.0390
515.96	219.10	0.0390
523.67	219.21	0.0390
562.17	219.32	0.0390
654.58	219.43	0.0390

RESERVOIR (5505)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1				
DT= 15.0 min				
	0.0000	0.0000	65.1290	345.3784
	25.4850	24.6697	84.9510	456.3890
	31.1490	98.6787	*****	838.7689
	39.6440	*****	*****	838.7789
	48.1390	*****	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8292)	2312.360	18.064	8.50
OUTFLOW : ID= 1 (5505)	2312.360	13.246	11.25

PEAK FLOW REDUCTION [Qout/Qin](%)= 73.32
TIME SHIFT OF PEAK FLOW (min)=165.00
MAXIMUM STORAGE USED (ha.m.)= 12.8339

ADD HYD (8272)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0376)	463.85	7.184	7.25	34.49
+ ID2= 2 (5505)	2312.36	13.246	11.25	19.55
ID = 3 (8272)	2776.21	17.737	10.25	22.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB
NASHYD (0396) Area (ha)= 305.21 Curve Number (CN)= 69.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.08

```

```

Unit Hyd Qpeak (cms)= 4.811
PEAK FLOW (cms)= 4.047 (1)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 29.979
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.371

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0394) Area (ha)= 325.45 Curve Number (CN)= 53.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.92

```

```

Unit Hyd Qpeak (cms)= 6.013
PEAK FLOW (cms)= 3.012 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 18.867
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.233

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0390) Area (ha)= 420.00 Curve Number (CN)= 55.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.07

```

```

Unit Hyd Qpeak (cms)= 6.683
PEAK FLOW (cms)= 3.688 (1)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 20.072
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.248

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0388) Area (ha)= 220.77 Curve Number (CN)= 58.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.99

```

```

Unit Hyd Qpeak (cms)= 3.819
PEAK FLOW (cms)= 2.265 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 21.891
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.271

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0386) Area (ha)= 241.27 Curve Number (CN)= 61.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.90

```

```

Unit Hyd Qpeak (cms)= 4.562
PEAK FLOW (cms)= 2.901 (1)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 23.835
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.295

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8286)
1 2 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0386): 241.27 2.901 7.00 23.84
+ ID2= 2 (0388): 220.77 2.265 7.25 21.89
=====
ID = 3 (8286): 462.04 5.164 7.25 22.91

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB
NASHYD (0384) Area (ha)= 199.07 Curve Number (CN)= 44.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.96

```

```

Unit Hyd Qpeak (cms)= 3.537
PEAK FLOW (cms)= 1.336 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 14.242
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.176

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0380) Area (ha)= 182.01 Curve Number (CN)= 40.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.55

```

```

Unit Hyd Qpeak (cms)= 5.609
PEAK FLOW (cms)= 1.618 (1)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 12.271
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.152

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0382) Area (ha)= 216.59 Curve Number (CN)= 53.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.64

```

```

Unit Hyd Qpeak (cms)= 5.733
PEAK FLOW (cms)= 2.645 (1)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 18.713
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.232

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8290)
1 + 2 = 3 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0380): 182.01 1.618 6.50 12.27
+ ID2= 2 (0382): 216.59 2.645 6.75 18.71
=====
ID = 3 (8290): 398.60 4.252 6.75 15.77

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6384)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

```

```

<----- DATA FOR SECTION (3841.0) ----->
Distance Elevation Manning
0.00 294.40 0.0380
10.59 291.93 0.0380
21.17 289.19 0.0380
26.46 287.99 0.0380
31.76 286.79 0.0380
71.45 279.97 0.0380
74.10 279.79 0.0380
76.74 279.71 0.0380
78.99 279.64 0.0380 / 0.0300 Main Channel
79.14 279.30 0.0300 Main Channel
79.39 279.30 0.0300 Main Channel
79.64 279.30 0.0300 Main Channel
79.79 279.64 0.0300 / 0.0380 Main Channel
89.98 279.78 0.0380
119.09 281.86 0.0380
145.55 282.87 0.0380
198.48 284.85 0.0380
211.71 286.31 0.0380
230.23 287.59 0.0380
261.99 294.00 0.0380

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME

```

(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.34	279.64	.715E+01	0.2	0.85	63.42
1.10	280.40	.507E+05	21.9	1.40	38.54
1.85	281.15	.141E+06	91.8	2.10	25.69
2.61	281.91	.269E+06	215.9	2.63	20.46
3.36	282.66	.445E+06	407.1	2.96	18.23
4.12	283.42	.681E+06	698.3	3.31	16.26
4.87	284.17	.977E+06	1111.8	3.68	14.65
5.63	284.93	.133E+07	1674.3	4.06	13.27
6.39	285.69	.173E+07	2457.8	4.59	11.73
7.14	286.44	.215E+07	3378.8	5.07	10.63
7.90	287.20	.261E+07	4416.4	5.47	9.85
8.65	287.95	.310E+07	5676.3	5.92	9.11
9.41	288.71	.361E+07	7150.5	6.40	8.42
10.17	289.47	.414E+07	8783.1	6.86	7.86
10.92	290.22	.469E+07	10575.1	7.30	7.38
11.68	290.98	.528E+07	12520.4	7.72	6.98
12.43	291.73	.582E+07	14619.0	8.12	6.64
13.19	292.49	.642E+07	16863.2	8.50	6.34
13.94	293.24	.703E+07	19259.8	8.86	6.08

```

<---- hydrograph ----> <-pipe / channel->
      AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
      (ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
INFLOW : ID= 2 (8290) 398.60 4.25 6.75 15.77 0.48 0.92
OUTFLOW: ID= 1 (6384) 398.60 3.34 8.00 15.77 0.45 0.90

```

```

| ADD HYD (8288) |
| 1 + 2 = 3 |
-----
ID1= 1 (0384): 398.07 3.336 7.25 14.24
+ ID2= 2 (6384): 398.60 3.339 8.00 15.77
=====
ID = 3 (8288): 597.67 4.648 7.75 15.26

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ADD HYD (8284) |
| 1 + 2 = 3 |
-----
ID1= 1 (8286): 462.04 5.164 7.25 22.91
+ ID2= 2 (8288): 597.67 4.648 7.75 15.26
=====
ID = 3 (8284): 1059.71 9.705 7.50 18.59

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ADD HYD (8280) |
| 1 + 2 = 3 |
-----
ID1= 1 (0390): 420.00 3.688 7.50 20.07
+ ID2= 2 (8284): 1059.71 9.705 7.50 18.59
=====
ID = 3 (8280): 1479.71 13.393 7.50 19.01

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| CALIB (0392) | Area (ha)= 167.22 Curve Number (CN)= 62.0
| NASHYD (0392) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.74

```

Unit Hyd Qpeak (cms) = 3.837

PEAK FLOW (cms) = 2.409 (i)

TIME TO PEAK (hrs) = 7.000

RUNOFF VOLUME (mm) = 24.429

TOTAL RAINFALL (mm) = 80.800

RUNOFF COEFFICIENT = 0.302

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

| CALIB (0378) | Area (ha)= 606.72 Curve Number (CN)= 55.0
| NASHYD (0378) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.18

```

Unit Hyd Qpeak (cms) = 8.771

PEAK FLOW (cms) = 4.960 (i)

TIME TO PEAK (hrs) = 7.750

RUNOFF VOLUME (mm) = 20.097
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.249

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

| ADD HYD (8282) |
| 1 + 2 = 3 |
-----
ID1= 1 (0378): 606.72 4.960 7.75 20.10
+ ID2= 2 (0392): 167.22 2.409 7.00 24.43
=====
ID = 3 (8282): 773.94 7.226 7.25 21.03

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ADD HYD (8278) |
| 1 + 2 = 3 |
-----
ID1= 1 (8280): 1479.71 13.393 7.50 19.01
+ ID2= 2 (8282): 773.94 7.226 7.25 21.03
=====
ID = 3 (8278): 2253.65 20.600 7.50 19.71

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ROUTE CWN (6394) |
| IN= 2----> OUT= 1 | Routing time step (min) = 15.00

```

----- DATA FOR SECTION (3941.0) -----

Distance	Elevation	Manning	
0.00	283.00	0.0380	
13.48	282.87	0.0380	
53.92	280.08	0.0380	
74.13	276.62	0.0380	
97.72	265.45	0.0380	
114.57	256.93	0.0380	
131.42	253.04	0.0380	
134.79	252.58	0.0380	
138.53	251.74	0.0380 / 0.0300	Main Channel
139.03	251.20	0.0300	Main Channel
141.53	251.20	0.0300	Main Channel
144.03	251.20	0.0300	Main Channel
144.53	251.74	0.0300 / 0.0380	Main Channel
148.27	252.69	0.0380	
151.64	252.97	0.0380	
185.34	255.08	0.0380	
219.03	257.54	0.0380	
262.84	259.43	0.0380	
310.02	262.80	0.0380	
333.60	283.00	0.0380	

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.54	251.74	.827E+04	4.5	1.51	30.71
2.19	251.39	.758E+05	73.2	2.69	17.27
3.83	255.03	.282E+06	331.8	3.27	14.17
5.48	256.68	.633E+06	917.3	4.03	11.51
7.12	258.32	.112E+07	1889.5	4.69	9.90
8.77	259.97	.179E+07	3410.2	5.31	8.73
10.41	261.61	.259E+07	5627.6	6.05	7.66
12.06	263.26	.351E+07	8605.1	6.83	6.79
13.70	264.90	.447E+07	12544.9	7.81	5.94
15.35	266.55	.548E+07	17103.4	8.71	5.32
16.99	268.19	.648E+07	22251.7	9.56	4.85
18.64	269.84	.752E+07	27979.0	10.36	4.48
20.28	271.48	.858E+07	34269.0	11.11	4.17
21.93	273.13	.967E+07	41110.8	11.83	3.92
23.57	274.77	.108E+08	48495.9	12.51	3.71
25.22	276.42	.119E+08	56418.1	13.17	3.52
26.86	278.06	.131E+08	64410.6	13.69	3.39
28.51	279.71	.143E+08	73002.1	14.18	3.27
30.15	281.35	.156E+08	81379.3	14.50	3.20

```

<---- hydrograph ----> <-pipe / channel->
      AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
      (ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
INFLOW : ID= 2 (8278) 2253.65 20.60 7.50 19.71 0.93 1.68
OUTFLOW: ID= 1 (6394) 2253.65 20.08 8.00 19.71 0.91 1.68

```

```

| ADD HYD (8276) |
| 1 + 2 = 3 |
-----

```

```

-----
      (ha)  (cms)  (hrs)  (mm)
ID1= 1 (0394): 325.45 3.012 7.25 18.87
+ ID2= 2 (6394): 2253.65 20.077 8.00 19.71
=====
ID = 3 (8276): 2579.10 22.946 8.00 19.60

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6396) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00
-----

```

```

<----- DATA FOR SECTION (3961.0) ----->
Distance  Elevation  Manning
0.00      263.00      0.0410
11.75     257.14      0.0410
23.50     253.97      0.0410
41.13     247.83      0.0410
76.38     232.09      0.0410
135.13    229.07      0.0410
149.82    228.97      0.0410
152.75    228.96      0.0410 /0.0300 Main Channel
154.19    228.73      0.0300 Main Channel
154.69    228.20      0.0300 Main Channel
155.69    228.20      0.0300 Main Channel
156.69    228.20      0.0300 Main Channel
157.19    228.73      0.0300 Main Channel
158.63    228.95      0.0300 /0.0410 Main Channel
161.57    228.96      0.0410
164.51    229.71      0.0410
196.82    241.70      0.0410
223.26    249.21      0.0410
246.76    255.13      0.0410
290.82    263.51      0.0410

```

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.75	228.95	780E+04	2.2	0.98	58.06
2.54	230.74	277E+06	145.1	1.78	31.76
4.33	232.53	786E+06	609.1	2.63	21.50
6.13	234.33	140E+07	1481.5	3.59	15.77
7.92	236.12	207E+07	2677.3	4.39	12.90
9.71	237.91	280E+07	4184.8	5.09	11.14
11.50	239.70	357E+07	6001.4	5.71	9.93
13.29	241.49	441E+07	8129.1	6.27	9.03
15.09	243.29	530E+07	10549.4	6.77	8.37
16.88	245.08	625E+07	13300.6	7.24	7.83
18.67	246.87	726E+07	16396.2	7.68	7.38
20.46	248.66	834E+07	19809.8	8.07	7.02
22.26	250.46	949E+07	23532.5	8.43	6.72
24.05	252.25	107E+08	27641.2	8.77	6.46
25.84	254.04	120E+08	32155.2	9.10	6.22
27.63	255.83	134E+08	36901.0	9.37	6.05
29.42	257.62	149E+08	42147.7	9.64	5.88
31.22	259.42	164E+08	48149.1	9.97	5.68
33.01	261.21	181E+08	54644.8	10.29	5.51

<---- hydrograph ----> <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8276)	2579.10	22.95	8.00	19.60	1.01	1.04
OUTFLOW: ID= 1 (6396)	2579.10	20.73	9.00	19.60	0.98	1.04

```

-----
| ADD HYD (8274) |
| 1 + 2 = 3 |
-----
ID1= 1 (0396): 305.21 4.047 7.50 29.98
+ ID2= 2 (6396): 2579.10 20.733 9.00 19.60
=====
ID = 3 (8274): 2884.31 24.211 9.00 20.70

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8270) |
| 1 + 2 = 3 |
-----
ID1= 1 (8272): 2776.21 17.737 10.25 22.05
+ ID2= 2 (8274): 2884.31 24.211 9.00 20.70
=====
ID = 3 (8270): 5660.52 41.092 9.25 21.36

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| RESERVOIR (5506) |

```

-----
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
      OUTFLOW  STORAGE  OUTFLOW  STORAGE
      (cms)   (ha.m.)  (cms)   (ha.m.)
0.0000    0.0000    60.8810  135.6832
31.1490   24.6597   96.2770  900.4431
36.8120   37.0045   96.6770  900.4531
45.3070   86.3439   0.0000   0.0000

```

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8270)	5660.520	41.092	9.25	21.36
OUTFLOW: ID= 1 (5506)	5660.520	32.437	12.00	21.36

PEAK FLOW REDUCTION [Qout/Qin](%)= 78.94
TIME SHIFT OF PEAK FLOW (min)=165.00
MAXIMUM STORAGE USED (ha.m.)= 27.4808

```

-----
| CALIB |
| NASHYD (0406) | Area (ha)= 142.65 Curve Number (CN)= 66.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.59
-----

```

Unit Hyd Qpeak (cms)= 4.135

PEAK FLOW (cms)= 2.779 (1)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 27.183
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.336

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0404) | Area (ha)= 246.46 Curve Number (CN)= 47.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.98
-----

```

Unit Hyd Qpeak (cms)= 4.280

PEAK FLOW (cms)= 1.795 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 15.696
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.194

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0402) | Area (ha)= 244.00 Curve Number (CN)= 61.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.07
-----

```

Unit Hyd Qpeak (cms)= 3.879

PEAK FLOW (cms)= 2.567 (1)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 23.900
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.296

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0400) | Area (ha)= 93.97 Curve Number (CN)= 52.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.44
-----

```

Unit Hyd Qpeak (cms)= 3.630

PEAK FLOW (cms)= 1.483 (1)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 17.989
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.221

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0398) | Area (ha)= 328.19 Curve Number (CN)= 55.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.83
-----

```

Unit Hyd Qpeak (cms)= 6.759

PEAK FLOW (cms)= 3.522 (1)

TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 19.985
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.247

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8268) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0398): 328.19 3.522 7.00 19.99
+ ID2= 2 (0400): 93.97 1.483 6.50 17.89
=====
ID = 3 (8268): 422.16 4.864 6.75 19.52
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6402) |
| IM= 2--> OUT= 1 |
-----
Routing time step (min)= 15.00
  
```

<----- DATA FOR SECTION (4021.0) ----->

Distance	Elevation	Manning	
0.00	238.50	0.0360	
11.50	238.00	0.0360	
23.00	237.93	0.0360	
34.49	236.39	0.0360	
63.24	233.98	0.0360	
97.73	228.15	0.0360	
123.60	227.08	0.0360	
126.48	226.61	0.0360	
127.60	226.47	0.0360 / 0.0330	Main Channel
127.85	225.25	0.0330	Main Channel
129.35	225.25	0.0330	Main Channel
130.85	225.25	0.0330	Main Channel
131.10	226.47	0.0330 / 0.0360	Main Channel
132.22	226.59	0.0360	
143.72	227.42	0.0360	
169.59	227.88	0.0360	
192.59	231.19	0.0360	
218.46	233.02	0.0360	
241.45	235.50	0.0360	
264.57	236.43	0.0360	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELRV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.41	225.66	.391E+04	1.7	1.36	38.31
0.81	226.06	.804E+04	5.0	1.93	27.02
1.22	226.47	1.248E+05	9.1	2.30	22.61
1.84	227.09	3.08E+05	23.3	2.36	22.03
2.46	227.71	9.04E+05	62.0	2.14	24.31
3.09	228.34	2.23E+06	188.3	2.63	19.75
3.71	228.96	3.79E+06	410.2	3.38	15.39
4.33	229.58	5.50E+06	711.5	4.04	12.89
4.95	230.20	7.37E+06	1091.3	4.62	11.25
5.58	230.83	9.39E+06	1550.6	5.15	10.10
6.20	231.45	1.18E+07	2072.4	5.59	9.31
6.82	232.07	1.40E+07	2659.2	5.93	8.77
7.44	232.69	1.67E+07	3345.8	6.27	8.30
8.07	233.32	1.95E+07	4159.0	6.64	7.83
8.69	233.94	2.26E+07	5100.8	7.03	7.39
9.31	234.56	2.59E+07	6081.3	7.32	7.11
9.93	235.18	2.95E+07	7183.1	7.60	6.84
10.56	235.81	3.34E+07	8345.2	7.62	6.83
11.18	236.43	3.79E+07	9116.0	7.51	6.92

<----- hydrograph ----> <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8268)	422.16	4.86	6.75	19.52	0.80	1.90
OUTFLOW: ID= 1 (6402)	422.16	4.63	7.25	19.52	0.77	1.84

```

-----
| ADD HYD (8266) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0402): 244.00 2.567 7.50 23.90
+ ID2= 2 (6402): 422.16 4.628 7.25 19.52
=====
ID = 3 (8266): 666.16 7.186 7.25 21.12
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8264) |
  
```

```

-----
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0404): 246.46 1.795 7.25 15.70
+ ID2= 2 (8266): 666.16 7.186 7.25 21.12
=====
ID = 3 (8264): 912.62 8.981 7.25 19.66
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8262) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0406): 342.65 2.779 6.50 27.18
+ ID2= 2 (8264): 912.62 8.981 7.25 19.66
=====
ID = 3 (8262): 1055.27 11.503 7.25 20.68
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8260) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (5506): 5660.52 32.437 12.00 21.36
+ ID2= 2 (8262): 1055.27 11.503 7.25 20.68
=====
ID = 3 (8260): 6715.79 37.738 10.25 21.25
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8258) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0408): 231.62 3.331 6.75 21.68
+ ID2= 2 (8260): 6715.79 37.738 10.25 21.25
=====
ID = 3 (8258): 6947.41 39.031 10.25 21.27
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8256) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0410): 572.01 3.210 8.25 16.28
+ ID2= 2 (8258): 6947.41 39.031 10.25 21.27
=====
ID = 3 (8256): 7519.42 41.844 10.25 20.89
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| RESERVOIR (5507) |
| IM= 2--> OUT= 1 |
| DT= 15.0 min |
-----
OUTFLOW STORAGE OUTFLOW STORAGE
(cms) (ha.m.) (cms) (ha.m.)
0.0000 0.0000 | 90.6140 67.8416
39.6440 12.3348 | ***** 160.3529
48.1390 18.5023 | ***** 160.3629
67.9600 37.0045 | 0.0000 0.0000
  
```

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8256)	7519.421	41.844	10.25	20.89
OUTFLOW: ID= 1 (5507)	7519.421	40.140	11.75	20.89

PEAK FLOW REDUCTION [Qout/qin](%)= 95.93
 TIME SHIFT OF PEAK FLOW (min)= 90.00
 MAXIMUM STORAGE USED (ha.m.)= 12.6987

```

-----
| CALIB (0420) |
| ID= 1 DT=15.0 min |
-----
Area (ha)= 175.82 Curve Number (CN)= 53.0
Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.81
  
```

Unit Hyd Qpeak (cms)= 3.692

PEAK FLOW (cms)= 1.800 (l)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 18.821
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.233

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

0.0000 0.0000 | 3.4720 1.9177
0.3820 0.3152 | 3.8720 1.9277
1.8280 0.4690 | 0.0000 0.0000

CALIB
NASHYD (0418) Area (ha)= 182.79 Curve Number (CN)= 64.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.05

Unit Hyd Opeak (cms)= 2.966

PEAK FLOW (cms)= 2.136 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 26.026
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.322

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0416) Area (ha)= 439.30 Curve Number (CN)= 64.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.29

Unit Hyd Opeak (cms)= 5.832

PEAK FLOW (cms)= 4.419 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 26.090
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.323

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0412) Area (ha)= 238.70 Curve Number (CN)= 54.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.80

Unit Hyd Opeak (cms)= 5.088

PEAK FLOW (cms)= 2.551 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 19.387
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.240

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (4141) Area (ha)= 43.70 Dir. Conn.(%)= 17.00
ID= 1 DT=15.0 min Total Imp(%)= 36.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 15.73 27.97
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 539.75 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 106.66 62.65
over (min) 15.00 30.00
Storage Coeff. (min)= 8.43 (ii) 21.33 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.09 0.05

TOTALS

PEAK FLOW (cms)= 1.95 2.39 3.467 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 78.80 31.64 39.66
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.39 0.49

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5414)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (4141) 43.700 3.467 6.00 39.66
OUTFLOW: ID= 1 (5414) 43.700 1.932 6.50 39.66

PEAK FLOW REDUCTION [Qout/Qin](%)= 55.72
TIME SHIFT OF PEAK FLOW (min)= 30.00
MAXIMUM STORAGE USED (ha.m.)= 0.5610

CALIB
STANDHYD (4142) Area (ha)= 144.30 Dir. Conn.(%)= 10.00
ID= 1 DT=15.0 min Total Imp(%)= 21.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 30.30 114.00
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 980.82 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 106.66 50.20
over (min) 15.00 30.00
Storage Coeff. (min)= 12.06 (ii) 26.15 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04

TOTALS

PEAK FLOW (cms)= 3.39 7.05 8.447 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.25
RUNOFF VOLUME (mm)= 78.80 25.93 33.92
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.36 0.42

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8254)
1 + 2 = 3 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (4142): 144.30 8.447 6.25 33.92
+ ID2= 1 (5414): 43.70 1.932 6.50 39.65
=====

ID = 3 (8254): 188.00 10.322 6.25 35.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8252)
1 + 2 = 3 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0412): 238.70 2.551 7.00 19.39
+ ID2= 2 (8254): 188.00 10.322 6.25 35.25
=====

ID = 3 (8252): 426.70 12.418 6.25 26.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6416) Routing time step (min)= 15.00
IN= 2--> OUT= 1

<----- DATA FOR SECTION (4161.0) ----->

Distance Elevation Manning
0.00 270.07 0.0340
20.67 267.91 0.0340
62.01 264.33 0.0340
113.69 259.75 0.0340
165.37 253.30 0.0340
227.38 246.29 0.0340
232.55 246.03 0.0340
237.72 246.16 0.0340
241.39 246.02 0.0340 / 0.0300 Main Channel
241.64 245.75 0.0300 Main Channel
242.89 245.75 0.0300 Main Channel
244.14 245.75 0.0300 Main Channel
244.39 246.02 0.0300 / 0.0340 Main Channel
248.06 246.20 0.0340

253.22	246.28	0.0340
258.39	246.63	0.0340
346.25	252.57	0.0340
413.43	257.77	0.0340
465.11	261.78	0.0340
511.62	270.00	0.0340

TRAVEL TIME TABLE						
DEPTH	SEGV	VOLUME	FLOW RATE	VELOCITY	TRAV TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.27	246.02	.295E+04	0.6	0.75	88.26	
1.53	247.28	.168E+06	69.7	1.65	40.13	
2.79	248.54	.490E+06	299.5	2.42	27.28	
4.06	249.81	.962E+06	740.1	3.05	21.67	
5.32	251.07	.158E+07	1440.0	3.61	18.33	
6.58	252.33	.235E+07	2443.8	4.12	16.06	
7.84	253.59	.327E+07	3811.2	4.63	14.30	
9.10	254.85	.432E+07	5565.1	5.11	12.94	
10.37	256.12	.550E+07	7721.3	5.57	11.88	
11.63	257.38	.682E+07	10309.9	6.00	11.02	
12.89	258.64	.827E+07	13360.6	6.42	10.31	
14.15	259.90	.985E+07	16884.5	6.81	9.72	
15.42	261.17	.116E+08	20816.4	7.14	9.26	
16.68	262.43	.134E+08	25510.4	7.53	8.78	
17.94	263.69	.154E+08	30958.0	7.97	8.30	
19.20	264.95	.175E+08	36988.3	8.38	7.89	
20.46	266.21	.197E+08	43617.1	8.78	7.53	
21.73	267.48	.220E+08	50868.1	9.17	7.22	
22.99	268.74	.244E+08	58854.0	9.56	6.92	

<---- hydrograph ----> <-pipe / channel->						
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8252)	426.70	12.42	6.25	26.38	0.49	0.83
OUTFLOW : ID= 1 (6416)	426.70	5.93	7.25	26.38	0.37	0.78

ADD HYD (8250)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0416):	439.30	4.419	8.00	26.09	
+ ID2= 2 (6416):	426.70	5.929	7.25	26.38	
ID = 3 (8250):	866.00	10.218	7.25	26.23	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8248)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0418):	182.79	2.136	7.50	26.03	
+ ID2= 2 (8250):	866.00	10.218	7.25	26.23	
ID = 3 (8248):	1048.79	12.352	7.25	26.20	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8246)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0420):	175.82	1.800	7.00	18.82	
+ ID2= 2 (8248):	1048.79	12.352	7.25	26.20	
ID = 3 (8246):	1224.61	14.132	7.25	25.14	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8244)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (5507):	7519.42	40.140	11.75	20.89	
+ ID2= 2 (8246):	1224.61	14.132	7.25	25.14	
ID = 3 (8244):	8744.03	48.352	10.50	21.48	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8242)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	

ID1= 1 (0422): 780.20 4.215 9.00 19.59
+ ID2= 2 (8244): 8744.03 48.352 10.50 21.48
=====

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5508)				
IN= 2-->	OUT= 1			
DT= 15.0 min				
OUTFLOW	STORAGE	OUTFLOW	STORAGE	
(cms)	(ha.m.)	(cms)	(mm)	
0.0000	0.0000	*****	197.3574	
76.4550	30.8371	*****	394.7148	
*****	61.6742	*****	394.7248	
*****	*****	0.0000	0.0000	

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8242)	9524.231	52.389	10.50	21.33
OUTFLOW : ID= 1 (5508)	9524.231	50.303	12.00	21.33

PEAK FLOW REDUCTION [Qout/Qin](%)= 96.02	
TIME SHIFT OF PEAK FLOW	(min)= 90.00
MAXIMUM STORAGE USED	(ha.m.)= 20.2951

CALIB (0336)			
NASHYD	Area	Curve Number	(CN)= 72.0
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50
U.H. Tp	(hrs)= 15.39		

Unit Hyd Qpeak (cms)= 3.090

PEAK FLOW (cms)= 3.992 (i)
TIME TO PEAK (hrs)= 22.250
RUNOFF VOLUME (mm)= 32.865
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.407

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3382)			
Area	(ha)= 525.90	Total Imp	(%)= 37.00
Dir. Conn.	(%)= 17.00		

IMPERVIOUS PERVIOUS (i)		
Surface Area	(ha)= 194.58	331.32
Dep. Storage	(mm)= 2.00	5.00
Average Slope	(%)= 0.50	0.50
Length	(m)= 1872.43	40.00
Mannings n	= 0.013	0.250

Max. Ref. Inten. (mm/hr)=	106.66	67.85
over (min)	15.00	45.00
Storage Coeff. (min)=	17.78 (ii)	30.27 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.06	0.03

TOTALS		
PEAK FLOW (cms)=	17.87	23.62
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	78.80	33.63
TOTAL RAINFALL (mm)=	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.42
		0.51

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 66.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3381)			
Area	(ha)= 73.00	Total Imp	(%)= 23.00
Dir. Conn.	(%)= 17.00		

IMPERVIOUS PERVIOUS (i)		
Surface Area	(ha)= 35.77	37.23
Dep. Storage	(mm)= 2.00	5.00
Average Slope	(%)= 0.50	0.50
Length	(m)= 697.61	40.00
Mannings n	= 0.013	0.250

Max. Ref. Inten. (mm/hr)=	106.66	84.70
over (min)	15.00	30.00
Storage Coeff. (min)=	9.83 (ii)	21.26 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00

Unit Hyd. peak (cms)= 0.09 0.05 *TOTALS*
 PEAK FLOW (cms)= 4.22 4.33 7.010 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 78.80 36.58 46.29
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.45 0.57

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5338)
 ID= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.3350	1.2400
0.3230	0.3074	4.3560	1.3520
0.9360	0.5374	5.3290	1.4590
1.7590	0.8302	5.7290	1.4690
2.4110	1.0550	0.0000	0.0000

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (3381) 73.000 7.010 6.00 46.29
 OUTFLOW: ID= 1 (5338) 73.000 3.356 6.50 46.29

PEAK FLOW REDUCTION [Qout/Qin](%)= 47.87
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 1.2583

ADD HYD (8310)
 1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
525.90	29.867	6.50	41.31
73.00	3.356	6.50	46.29

ID= 1 (3382): 525.90 29.867 6.50 41.31
 + ID2= 2 (5338): 73.00 3.356 6.50 46.29
 ID = 3 (8310): 598.90 33.223 6.50 41.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (3342) Area (ha)= 587.10 Curve Number (CN)= 56.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 8.19

Unit Hyd Qpeak (cms)= 1.224

PEAK FLOW (cms)= 0.993 (i)
 TIME TO PEAK (hrs)= 15.250
 RUNOFF VOLUME (mm)= 20.856
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.258

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (3341) Area (ha)= 33.00 Dir. Conn.(%)= 31.00
 ID= 1 DT=15.0 min Total Imp(%)= 51.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)= 16.83	16.17
Dep. Storage (mm)= 2.00	5.00
Average Slope (%)= 0.50	0.50
Length (m)= 469.04	40.00
Mannings n = 0.013	0.250

Max. Eff. Inten.(mm/hr)= 106.66 51.83
 over (min) 15.00 30.00
 Storage Coeff. (min)= 7.75 (ii) 21.66 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.10 0.05

PEAK FLOW (cms)= 2.74 1.13 3.444 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 78.80 24.48 41.32
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.30 0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

CN* = 52.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5334)
 ID= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.6540	0.4604
0.0390	0.1659	0.8330	0.5249
0.1860	0.2610	1.0130	0.5896
0.3750	0.3534	1.4130	0.5996
0.4340	0.3813	0.0000	0.0000

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (3341) 33.000 3.444 6.00 41.32
 OUTFLOW: ID= 1 (5334) 33.000 1.579 6.50 41.29

PEAK FLOW REDUCTION [Qout/Qin](%)= 45.84
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 0.6194

ADD HYD (8314)
 1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
587.10	0.993	15.25	20.86
33.00	1.579	6.50	41.29

ID= 1 (3342): 587.10 0.993 15.25 20.86
 + ID2= 2 (5334): 33.00 1.579 6.50 41.29
 ID = 3 (8314): 620.10 1.884 6.50 21.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0306) Area (ha)= 283.97 Curve Number (CN)= 52.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 6.44

Unit Hyd Qpeak (cms)= 0.753

PEAK FLOW (cms)= 0.538 (i)
 TIME TO PEAK (hrs)= 13.750
 RUNOFF VOLUME (mm)= 18.507
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.229

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0286) Area (ha)= 260.51 Curve Number (CN)= 84.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.16

Unit Hyd Qpeak (cms)= 3.834

PEAK FLOW (cms)= 5.156 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 45.890
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.568

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0282) Area (ha)= 449.38 Curve Number (CN)= 77.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.47

Unit Hyd Qpeak (cms)= 5.226

PEAK FLOW (cms)= 5.992 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 37.664
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.466

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0284) Area (ha)= 78.93 Curve Number (CN)= 84.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50

U.H. Tp(hrs)= 0.57

Unit Hyd Qpeak (cms)= 2.344

PEAK FLOW (cms)= 2.746 (i)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 45.199
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.559

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 5 columns: ADD HYD (8388), AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Rows include ID1=1 (0282), ID2=2 (0284), and ID=3 (8388).

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CEN (6286) Routing time step (min)'= 15.00
IN= 2--> OUT= 1

Table with 4 columns: Distance, Elevation, Manning, and a descriptive column. Lists data points from 0.00 to 509.44.

Table with 6 columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV.TIME. Lists data points from 0.31 to 13.82.

hydrograph table with 6 columns: AREA, QPEAK, TPEAK, R.V., MAX DEPTH, MAX VEL. Includes INFLOW and OUTFLOW data.

Table with 5 columns: ADD HYD (8386), AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Rows include ID1=1 (0286), ID2=2 (6286), and ID=3 (8386).

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with 4 columns: CALIB, NASHYD (0302), Area (ha), Curve Number (CN). Includes ID=1 DT=15.0 min and U.H. Tp(hrs)= 1.66.

Unit Hyd Qpeak (cms)= 4.874

PEAK FLOW (cms)= 3.272 (i)
TIME TO PEAK (hrs)= 8.500
RUNOFF VOLUME (mm)= 22.015
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.272

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 4 columns: CALIB, NASHYD (0300), Area (ha), Curve Number (CN). Includes ID=1 DT=15.0 min and U.H. Tp(hrs)= 1.03.

Unit Hyd Qpeak (cms)= 4.292

PEAK FLOW (cms)= 2.189 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 18.764
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.232

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 4 columns: CALIB, NASHYD (0298), Area (ha), Curve Number (CN). Includes ID=1 DT=15.0 min and U.H. Tp(hrs)= 1.26.

Unit Hyd Qpeak (cms)= 4.461

PEAK FLOW (cms)= 1.875 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 14.769
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.183

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 5 columns: ADD HYD (8395), AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Rows include ID1=1 (0298), ID2=2 (0300), and ID=3 (8395).

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with 4 columns: CALIB, NASHYD (0288), Area (ha), Curve Number (CN). Includes ID=1 DT=15.0 min and U.H. Tp(hrs)= 2.21.

Unit Hyd Qpeak (cms)= 2.629

PEAK FLOW (cms)= 3.362 (i)
TIME TO PEAK (hrs)= 9.250
RUNOFF VOLUME (mm)= 38.847
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.481

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 4 columns: CALIB, NASHYD (0290), Area (ha), Curve Number (CN). Includes ID=1 DT=15.0 min and U.H. Tp(hrs)= 1.07.

Unit Hyd Qpeak (cms)= 4.279

PEAK FLOW (cms)= 4.704 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 38.611
TOTAL RAINFALL (mm)= 80.800

RUNOFF COEFFICIENT = 0.478

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8397) |
| 1 + 2 = 3 |
|-----|
| ID1= 1 (0288): 340.83 3.362 9.25 38.85 |
| + ID2= 2 (0290): 269.18 4.704 7.25 38.61 |
|=====|
| ID = 3 (8397): 610.01 7.793 8.00 38.74 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB (0296) | Area (ha)= 293.65 Curve Number (CN)= 76.0 |
| NASHYD (0296) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.13 |

```

```

Unit Hyd Qpeak (cms)= 4.437
PEAK FLOW (cms)= 4.643 (1)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 36.515
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.452

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0292) | Area (ha)= 738.49 Curve Number (CN)= 68.0 |
| NASHYD (0292) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.52 |

```

```

Unit Hyd Qpeak (cms)= 8.289
PEAK FLOW (cms)= 7.336 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 29.254
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.362

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0294) | Area (ha)= 274.15 Curve Number (CN)= 57.0 |
| NASHYD (0294) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.87 |

```

```

Unit Hyd Qpeak (cms)= 5.367
PEAK FLOW (cms)= 3.004 (1)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 21.217
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.263

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8398) |
| 1 + 2 = 3 |
|-----|
| ID1= 1 (0292): 738.49 7.336 8.00 29.25 |
| + ID2= 2 (0294): 274.15 3.004 7.00 21.22 |
|=====|
| ID = 3 (8398): 1012.64 10.132 8.00 27.08 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6296) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00

```

```

<----- DATA FOR SECTION (2961.0) ----->
Distance Elevation Manning
0.00 243.98 0.0400
42.59 243.18 0.0400
85.17 241.81 0.0400
120.66 240.50 0.0400
156.15 239.56 0.0400
198.74 236.15 0.0400
237.78 234.01 0.0400

```

```

241.33 233.82 0.0400
248.77 233.12 0.0400 /0.0400 Main Channel
249.87 232.32 0.0400 Main Channel
250.37 231.80 0.0400 Main Channel
250.87 232.23 0.0400 Main Channel
251.97 233.10 0.0400 /0.0400 Main Channel
255.37 233.22 0.0400
259.07 233.87 0.0400
262.62 234.12 0.0400
266.17 234.23 0.0400
283.91 234.73 0.0400
337.15 241.75 0.0400
351.34 244.00 0.0400

```

```

-----
| TRAVEL TIME TABLE |
|-----|
| DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME |
| (m) (m) (cu.m.) (cms) (m/s) (min) |
0.43 232.23 .510E+03 0.1 0.53 81.07
0.87 232.67 .213E+04 0.7 0.83 51.49
1.30 233.10 .501E+04 2.2 1.10 38.75
1.98 233.78 .242E+05 12.6 1.34 31.93
2.66 234.46 .738E+05 42.1 1.46 29.21
3.34 235.14 .177E+06 127.9 1.84 23.13
4.02 235.82 .314E+06 281.4 2.29 18.61
4.70 236.50 .481E+06 509.4 2.71 15.73
5.38 237.18 .672E+06 815.6 3.11 13.73
6.06 237.86 .886E+06 1200.7 3.46 12.30
6.74 238.54 .112E+07 1669.4 3.80 11.23
7.42 239.22 .139E+07 2226.9 4.11 10.38
8.10 239.90 .168E+07 2808.4 4.28 9.95
8.78 240.58 .202E+07 3470.4 4.40 9.68
9.46 241.26 .240E+07 4331.7 4.61 9.24
10.14 241.94 .283E+07 5335.2 4.83 8.84
10.82 242.62 .330E+07 6482.6 5.03 8.48
11.50 243.30 .381E+07 7764.1 5.21 8.18
12.18 243.98 .439E+07 9101.2 5.31 8.04

```

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8398) 1012.64 10.13 8.00 27.08 1.82 1.27
OUTFLOW: ID= 1 (6296) 1012.64 9.86 8.50 27.08 1.80 1.26

```

```

-----
| ADD HYD (8396) |
| 1 + 2 = 3 |
|-----|
| ID1= 1 (0296): 293.65 4.643 7.50 36.52 |
| + ID2= 2 (6296): 1012.64 9.865 8.50 27.08 |
|=====|
| ID = 3 (8396): 1306.29 14.310 8.25 29.20 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8394) |
| 1 + 2 = 3 |
|-----|
| ID1= 1 (8396): 1306.29 14.310 8.25 29.20 |
| + ID2= 2 (8397): 610.01 7.793 8.00 38.74 |
|=====|
| ID = 3 (8394): 1916.30 22.076 8.00 32.24 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8392) |
| 1 + 2 = 3 |
|-----|
| ID1= 1 (8394): 1916.30 22.076 8.00 32.24 |
| + ID2= 2 (8395): 589.44 4.038 7.75 16.52 |
|=====|
| ID = 3 (8392): 2505.74 26.095 8.00 28.54 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6302) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00

```

```

<----- DATA FOR SECTION (3021.0) ----->
Distance Elevation Manning
0.00 228.10 0.0400
18.47 227.12 0.0400
36.95 226.12 0.0400
46.18 225.84 0.0400
55.42 225.58 0.0400

```

272.47	222.88	0.0400	
277.09	222.76	0.0400	
281.71	222.58	0.0400	
288.54	222.18	0.0400 / 0.0300	Main Channel
288.64	221.00	0.0300	Main Channel
290.94	221.00	0.0300	Main Channel
291.04	221.00	0.0300	Main Channel
291.54	222.75	0.0300 / 0.0400	Main Channel
300.18	222.83	0.0400	
304.80	223.04	0.0400	
309.42	223.25	0.0400	
318.65	223.69	0.0400	
360.22	225.57	0.0400	
397.16	227.60	0.0400	
457.20	228.35	0.0400	

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.29	221.29	228E+04	0.2	0.32	165.63
0.59	221.59	465E+04	0.7	0.45	116.15
0.88	221.88	713E+04	1.2	0.54	96.87
1.18	222.18	971E+04	1.9	0.61	86.20
1.57	222.57	115E+05	3.3	0.59	88.86
1.97	222.97	427E+05	6.4	0.47	111.39
2.36	223.36	115E+06	16.1	0.44	119.14
2.76	223.76	237E+06	37.1	0.49	106.49
3.15	224.15	409E+06	72.9	0.56	93.40
3.55	224.55	631E+06	127.0	0.63	82.76
3.94	224.94	903E+06	202.4	0.70	74.37
4.34	225.34	123E+07	301.8	0.77	67.68
4.73	225.73	160E+07	433.5	0.85	61.37
5.13	226.13	200E+07	602.2	0.95	55.29
5.52	226.52	242E+07	804.7	1.05	50.13
5.92	226.92	286E+07	1034.2	1.14	46.11
6.31	227.31	332E+07	1290.5	1.22	42.88
6.71	227.71	380E+07	1560.9	1.29	40.55
7.10	228.10	432E+07	1835.6	1.34	39.19

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8392) 2505.74	26.10	8.00	28.54	2.55	0.46
OUTFLOW: ID= 1 (6302) 2505.74	20.61	10.25	28.54	2.45	0.45

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0302):	473.90	3.272	8.50	22.02
+ ID2= 2 (6302):	2505.74	20.608	10.25	28.54
ID = 3 (8390):	2979.64	23.639	10.00	27.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8386):	788.82	11.552	8.50	41.13
+ ID2= 2 (8390):	2979.64	23.639	10.00	27.50
ID = 3 (8348):	3768.46	34.216	9.50	30.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	NASHYD (0304)	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res.(N)
		292.27	5.00	63.0	1.50
			2.78		

Unit Hyd Qpeak (cms)= 1.793
 PEAK FLOW (cms)= 1.571 (i)
 TIME TO PEAK (hrs)= 10.250
 RUNOFF VOLUME (mm)= 25.482
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.315

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD (0280)	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res.(N)
		299.86	5.00	82.0	1.50
			0.85		

Unit Hyd Qpeak (cms)= 6.009

PEAK FLOW (cms)= 7.151 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 43.109
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.534

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD (0278)	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res.(N)
		485.49	5.00	82.0	1.50
			1.52		

Unit Hyd Qpeak (cms)= 5.453

PEAK FLOW (cms)= 7.318 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 43.435
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.538

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD (0276)	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res.(N)
		90.89	5.00	79.0	1.50
			0.67		

Unit Hyd Qpeak (cms)= 2.302

PEAK FLOW (cms)= 2.373 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 39.356
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.487

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD (0274)	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res.(N)
		392.49	5.00	79.0	1.50
			1.08		

Unit Hyd Qpeak (cms)= 6.182

PEAK FLOW (cms)= 6.216 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 35.482
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.439

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0274):	392.49	6.216	7.50	35.48
+ ID2= 2 (0276):	90.89	2.373	6.75	39.36
ID = 3 (8360):	483.38	8.455	7.00	36.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0278):	485.49	7.318	8.00	43.44
+ ID2= 2 (8360):	483.38	8.455	7.00	36.21
ID = 3 (8358):	968.87	15.469	7.50	39.83

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6280)	Routing time step (min)
	15.00

----- DATA FOR SECTION (2801.0) -----
 Distance Elevation Manning

0.00	241.14	0.0500	
13.32	240.80	0.0500	
39.95	240.07	0.0500	
96.54	236.21	0.0500	
113.19	234.15	0.0500	
123.18	232.35	0.0500	
143.15	225.80	0.0500	
149.81	225.62	0.0500	
153.14	225.40	0.0500	
157.30	224.76	0.0500 / 0.0300	Main Channel
159.80	224.26	0.0300	Main Channel
162.30	224.85	0.0300	Main Channel
162.55	224.97	0.0300 / 0.0500	Main Channel
167.80	225.05	0.0500	
186.43	229.14	0.0500	
213.06	234.75	0.0500	
236.37	237.09	0.0500	
266.33	237.31	0.0500	
292.96	237.83	0.0500	
329.58	241.50	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.50	224.76	.263E+04	0.8	0.65	58.31
1.36	225.62	.278E+05	13.4	1.10	34.54
2.22	226.48	.855E+05	52.6	1.40	27.09
3.09	227.35	.157E+06	120.4	1.74	21.78
3.95	228.21	.242E+06	216.3	2.04	18.64
4.81	229.07	.340E+06	341.8	2.29	16.56
5.67	229.93	.450E+06	498.3	2.52	15.06
6.53	230.79	.574E+06	688.3	2.73	13.90
7.40	231.66	.711E+06	913.8	2.93	12.97
8.26	232.52	.861E+06	1173.4	3.11	12.23
9.12	233.38	.103E+07	1461.3	3.24	11.72
9.98	234.24	.121E+07	1793.3	3.37	11.26
10.85	235.11	.142E+07	2141.0	3.45	11.02
11.71	235.97	.165E+07	2530.4	3.50	10.86
12.57	236.83	.192E+07	2954.0	3.51	10.81
13.43	237.69	.226E+07	3160.3	3.18	11.93
14.29	238.55	.271E+07	3869.4	3.26	11.67
15.16	239.42	.320E+07	4740.3	3.38	11.24
16.02	240.28	.373E+07	5688.4	3.48	10.92

<---- hydrograph ----> <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8358)	968.87	15.47	7.50	39.83	1.41
OUTFLOW : ID= 1 (6280)	968.87	15.03	8.25	39.83	1.40

ADD HYD (8354)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID= 1 (0280):	299.86	7.151	7.00	43.11
+ ID= 2 (6280):	968.87	15.029	8.25	39.83
ID = 3 (8354):	1268.73	21.461	7.75	40.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	NASHVD (0272)	Area (ha)	(ha)	Curve Number (CN)
ID= 1 DT=15.0 min	Ia (mm)= 5.00	157.38	215.76	75.0
	U.H. Tp(hrs)= 1.09			# of Linear Res.(N)= 1.50
Unit Hyd Opeak (cms)= 2.456				
PEAK FLOW (cms)= 2.475 (1)				
TIME TO PEAK (hrs)= 7.500				
RUNOFF VOLUME (mm)= 35.486				
TOTAL RAINFALL (mm)= 80.800				
RUNOFF COEFFICIENT = 0.439				

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHVD (0270)	Area (ha)	(ha)	Curve Number (CN)
ID= 1 DT=15.0 min	Ia (mm)= 5.00	243.61	243.61	81.0
	U.H. Tp(hrs)= 0.87			# of Linear Res.(N)= 1.30
Unit Hyd Opeak (cms)= 3.429				
PEAK FLOW (cms)= 4.097 (1)				
TIME TO PEAK (hrs)= 7.250				
RUNOFF VOLUME (mm)= 41.870				
TOTAL RAINFALL (mm)= 80.800				
RUNOFF COEFFICIENT = 0.518				

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHVD (0268)	Area (ha)	(ha)	Curve Number (CN)
ID= 1 DT=15.0 min	Ia (mm)= 5.00	215.76	215.76	75.0
	U.H. Tp(hrs)= 0.69			# of Linear Res.(N)= 1.30
Unit Hyd Opeak (cms)= 3.807				
PEAK FLOW (cms)= 3.621 (1)				
TIME TO PEAK (hrs)= 7.000				
RUNOFF VOLUME (mm)= 35.162				
TOTAL RAINFALL (mm)= 80.800				
RUNOFF COEFFICIENT = 0.435				

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHVD (0264)	Area (ha)	(ha)	Curve Number (CN)
ID= 1 DT=15.0 min	Ia (mm)= 5.00	253.96	253.96	69.0
	U.H. Tp(hrs)= 1.30			# of Linear Res.(N)= 1.30
Unit Hyd Opeak (cms)= 3.313				
PEAK FLOW (cms)= 3.033 (1)				
TIME TO PEAK (hrs)= 8.000				
RUNOFF VOLUME (mm)= 30.013				
TOTAL RAINFALL (mm)= 80.800				
RUNOFF COEFFICIENT = 0.371				

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHVD (0266)	Area (ha)	(ha)	Curve Number (CN)
ID= 1 DT=15.0 min	Ia (mm)= 5.00	508.09	508.09	64.0
	U.H. Tp(hrs)= 1.63			# of Linear Res.(N)= 1.30
Unit Hyd Opeak (cms)= 3.786				
PEAK FLOW (cms)= 3.140 (1)				
TIME TO PEAK (hrs)= 9.000				
RUNOFF VOLUME (mm)= 26.119				
TOTAL RAINFALL (mm)= 80.800				
RUNOFF COEFFICIENT = 0.323				

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8380)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID= 1 (0264):	353.96	3.033	8.00	30.01
+ ID= 2 (0266):	508.09	3.140	9.00	26.12
ID = 3 (8380):	862.05	6.146	8.50	27.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6268)	Routing time step (min)= 15.00
IN= 2--> OUT= 1	

<----- DATA FOR SECTION (2681.0) ----->

Distance	Elevation	Manning
0.00	274.03	0.0500
10.55	272.73	0.0500
21.10	271.28	0.0500
50.12	266.83	0.0500
87.04	260.36	0.0500
92.32	260.06	0.0500
94.95	259.93	0.0500
101.00	256.87	0.0500
103.00	256.58	0.0500 / 0.0350
104.00	256.32	0.0350
106.00	256.05	0.0350
108.00	256.25	0.0350
110.78	256.65	0.0350 / 0.0500
113.42	256.81	0.0500
116.06	257.00	0.0500
155.62	260.24	0.0500
187.27	263.35	0.0500
211.01	267.22	0.0500
224.20	269.60	0.0500
261.13	272.68	0.0500

----- TRAVEL TIME TABLE ----->

Table with 7 columns: DEPTH (m), ELEV (m), VOLUME (cu.m.), FLOW RATE (cms), VELOCITY (m/s), TRAV.TIME (min). Rows show data for various depths from 0.53 to 15.78 meters.

<---- hydrograph ----> <-pipe / channel-->

Summary hydrograph table with 7 columns: AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm), MAX DEPTH (m), MAX VEL (m/s). Includes INFLOW and OUTFLOW data for ID=2 and ID=1.

ADD HYD (8382) summary table with 5 columns: AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes sub-summaries for ID1, ID2, and ID3.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6270) Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2701.0) ----->

Table with 5 columns: Distance, Elevation, Manning, Main Channel. Lists data points for a section, including channel identification.

----- TRAVEL TIME TABLE ----->

Table with 7 columns: DEPTH (m), ELEV (m), VOLUME (cu.m.), FLOW RATE (cms), VELOCITY (m/s), TRAV.TIME (min). Rows show data for various depths from 0.45 to 9.94 meters.

<---- hydrograph ----> <-pipe / channel-->

Summary hydrograph table with 7 columns: AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm), MAX DEPTH (m), MAX VEL (m/s). Includes INFLOW and OUTFLOW data for ID=2 and ID=1.

Summary hydrograph table with 7 columns: AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm), MAX DEPTH (m), MAX VEL (m/s). Includes INFLOW and OUTFLOW data for ID=2 and ID=1.

ADD HYD (8384) summary table with 5 columns: AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Includes sub-summaries for ID1 and ID2.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0262) summary table with 7 columns: Area (ha), Ia (mm), Curve Number (CN), # of Linear Res. (N), U.H. Tp (hrs). Includes Unit Hyd Qpeak.

Unit Hyd Qpeak (cms)= 4.094

Summary table for NASHYD (0262) with 4 columns: PEAK FLOW (cms), TIME TO PEAK (hrs), RUNOFF VOLUME (mm), TOTAL RAINFALL (mm), RUNOFF COEFFICIENT.

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0260) summary table with 7 columns: Area (ha), Ia (mm), Curve Number (CN), # of Linear Res. (N), U.H. Tp (hrs). Includes Unit Hyd Qpeak.

Unit Hyd Qpeak (cms)= 4.360

Summary table for NASHYD (0260) with 4 columns: PEAK FLOW (cms), TIME TO PEAK (hrs), RUNOFF VOLUME (mm), TOTAL RAINFALL (mm), RUNOFF COEFFICIENT.

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0258) summary table with 7 columns: Area (ha), Ia (mm), Curve Number (CN), # of Linear Res. (N), U.H. Tp (hrs). Includes Unit Hyd Qpeak.

Unit Hyd Qpeak (cms)= 1.881

Summary table for NASHYD (0258) with 4 columns: PEAK FLOW (cms), TIME TO PEAK (hrs), RUNOFF VOLUME (mm), TOTAL RAINFALL (mm), RUNOFF COEFFICIENT.

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0252) summary table with 7 columns: Area (ha), Ia (mm), Curve Number (CN), # of Linear Res. (N), U.H. Tp (hrs). Includes Unit Hyd Qpeak.

Unit Hyd Qpeak (cms)= 3.761

Summary table for NASHYD (0252) with 4 columns: PEAK FLOW (cms), TIME TO PEAK (hrs), RUNOFF VOLUME (mm), TOTAL RAINFALL (mm), RUNOFF COEFFICIENT.

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0256) summary table with 7 columns: Area (ha), Ia (mm), Curve Number (CN), # of Linear Res. (N), U.H. Tp (hrs). Includes Unit Hyd Qpeak.

Unit Hyd Qpeak (cms)= 1.707

PEAK FLOW (cms)= 1.403 (1)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 28.295
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.350

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----
| CALIB |
| NASHVD (0254) | Area (ha)= 403.00 Curve Number (CN)= 55.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
|-----|
| U.H. Tp(hrs)= 1.22
```

Unit Hyd Qpeak (cms)= 4.028

PEAK FLOW (cms)= 2.406 (1)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 20.083
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.249

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----
| ROUTE CHN (6256) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
```

----- DATA FOR SECTION (2561.0) -----			
Distance	Elevation	Manning	
0.00	276.07	0.0400	
11.68	273.71	0.0400	
23.36	271.35	0.0400	
29.19	270.30	0.0400	
35.03	269.44	0.0400	
45.47	267.90	0.0400	
78.82	266.24	0.0400	
90.50	265.63	0.0400	
102.18	265.40	0.0400	
105.10	264.95	0.0400 /0.0350	Main Channel
108.02	264.39	0.0350	Main Channel
110.94	264.72	0.0350	Main Channel
113.86	265.19	0.0350 /0.0400	Main Channel
116.78	265.49	0.0400	
143.05	268.24	0.0400	
172.25	270.53	0.0400	
207.28	271.95	0.0400	
233.55	273.72	0.0400	
256.91	274.98	0.0400	
289.02	275.97	0.0400	

----- TRAVEL TIME TABLE -----					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.56	264.95	.723E+04	1.9	0.90	62.51
1.14	265.53	.307E+05	13.5	1.49	37.92
1.72	266.11	.951E+05	48.6	1.73	32.62
2.30	266.69	.191E+06	117.8	2.09	27.03
2.88	267.27	.314E+06	225.2	2.43	23.24
3.46	267.85	.464E+06	375.5	2.74	20.59
4.04	268.43	.641E+06	573.2	3.03	18.62
4.62	269.01	.846E+06	821.9	3.29	17.15
5.20	269.59	.108E+07	1135.0	3.56	15.87
5.78	270.17	.134E+07	1525.1	3.86	14.64
6.36	270.75	.162E+07	1971.1	4.12	13.71
6.94	271.33	.194E+07	2474.0	4.33	13.04
7.52	271.91	.228E+07	3065.0	4.55	12.42
8.10	272.49	.266E+07	3773.4	4.80	11.75
8.68	273.07	.308E+07	4566.3	5.05	11.17
9.26	273.65	.348E+07	5443.7	5.30	10.66
9.84	274.23	.393E+07	6389.6	5.51	10.25
10.42	274.81	.440E+07	7426.2	5.72	9.88
11.00	275.39	.490E+07	8490.1	5.86	9.63

```
----- hydrograph ----- <-pipe / channel-->
| AREA | QPEAK | TPEAK | R.V. | MAX DEPTH | MAX VEL |
| (ha) | (cms) | (hrs) | (mm) | (m) | (m/s) |
INFLOW : ID= 2 (0254) 403.00 2.41 8.00 20.08 0.58 0.82
OUTFLOW : ID= 1 (6256) 403.00 2.26 9.75 20.08 0.58 0.91
```

```
-----
| ADD HYD (8370) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm) |
+ ID1= 1 (0256): 145.79 1.403 8.00 28.30
+ ID2= 2 (6256): 403.00 2.263 9.75 20.08
-----
ID = 3 (8370): 548.79 3.538 9.00 22.26
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
-----
| CALIB |
| NASHVD (0250) | Area (ha)= 192.88 Curve Number (CN)= 70.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
|-----|
| U.H. Tp(hrs)= 1.22
```

Unit Hyd Qpeak (cms)= 1.930

PEAK FLOW (cms)= 1.797 (1)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 30.845
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.382

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----
| CALIB |
| NASHVD (0246) | Area (ha)= 759.61 Curve Number (CN)= 55.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
|-----|
| U.H. Tp(hrs)= 1.81
```

Unit Hyd Qpeak (cms)= 5.121

PEAK FLOW (cms)= 3.349 (1)
 TIME TO PEAK (hrs)= 10.000
 RUNOFF VOLUME (mm)= 20.153
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.249

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----
| CALIB |
| NASHVD (0248) | Area (ha)= 146.04 Curve Number (CN)= 64.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
|-----|
| U.H. Tp(hrs)= 0.78
```

Unit Hyd Qpeak (cms)= 2.271

PEAK FLOW (cms)= 1.592 (1)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 25.873
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.320

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----
| ADD HYD (8364) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm) |
+ ID1= 1 (0246): 759.61 3.349 10.00 20.15
+ ID2= 2 (0248): 146.04 1.592 7.25 25.87
-----
ID = 3 (8364): 905.65 4.741 8.25 21.08
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
-----
| ROUTE CHN (6250) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
```

----- DATA FOR SECTION (2501.0) -----			
Distance	Elevation	Manning	
0.00	269.96	0.0500	
8.56	268.55	0.0500	
17.13	266.91	0.0500	
21.41	266.13	0.0500	
40.68	263.15	0.0500	
62.09	260.75	0.0500	
85.64	258.02	0.0500	
88.20	257.69	0.0500	
93.20	257.05	0.0500 /0.0350	Main Channel
93.45	256.88	0.0350	Main Channel
94.20	256.56	0.0350	Main Channel
94.95	256.83	0.0350	Main Channel
95.20	257.08	0.0350 /0.0500	Main Channel
100.62	257.45	0.0500	
115.61	258.57	0.0500	
139.16	260.43	0.0500	
152.01	261.95	0.0500	
171.27	264.63	0.0500	
188.40	267.90	0.0500	
211.95	274.18	0.0500	

----- TRAVEL TIME TABLE -----

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.49	257.05	.205E+04	0.4	0.68	89.65
1.17	257.73	.248E+05	5.7	0.85	72.16
1.85	258.41	.841E+05	25.2	1.10	55.60
2.53	259.09	.180E+06	66.5	1.36	45.14
3.21	259.77	.312E+06	135.7	1.60	38.36
3.89	260.45	.481E+06	238.3	1.82	33.61
4.57	261.13	.693E+06	383.5	2.07	29.60
5.25	261.81	.911E+06	568.9	2.29	26.70
5.93	262.49	.117E+07	800.5	2.51	24.36
6.61	263.17	.146E+07	1079.0	2.72	22.49
7.29	263.84	.177E+07	1413.4	2.94	20.85
7.96	264.52	.210E+07	1797.0	3.14	19.50
8.64	265.20	.246E+07	2241.6	3.35	18.28
9.32	265.88	.283E+07	2739.9	3.55	17.25
10.00	266.56	.323E+07	3295.7	3.75	16.34
10.68	267.24	.364E+07	3908.9	3.94	15.54
11.36	267.92	.408E+07	4578.5	4.12	14.84
12.04	268.60	.453E+07	5320.1	4.32	14.18
12.72	269.28	.499E+07	6109.1	4.50	13.61

<---- hydrograph ---->				<-pipe / channel->			
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (8364)	905.65	4.74	8.25	21.08	1.04	0.81	
OUTFLOW : ID= 1 (6250)	905.65	4.43	10.25	21.08	1.00	0.80	

ADD HYD (8366)						
1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0250):			192.88	1.797	8.00	30.85
+ ID2= 2 (6250):			905.65	4.429	10.25	21.08
=====						
ID = 3 (8366):			1098.53	6.064	9.75	22.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8368)						
1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8366):			1098.53	6.064	9.75	22.79
+ ID2= 2 (8370):			548.79	3.538	9.00	22.26
=====						
ID = 3 (8368):			1647.32	9.574	9.50	22.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8372)						
1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0252):			318.99	3.678	8.00	33.49
+ ID2= 2 (8368):			1647.32	9.574	9.50	22.62
=====						
ID = 3 (8372):			1967.31	12.931	9.00	24.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6258) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (2581.0) ----->					
Distance	Elevation	Manning			
0.00	252.88	0.0500			
15.47	251.89	0.0500			
46.41	248.45	0.0500			
73.48	245.81	0.0500			
88.95	243.75	0.0500			
112.15	242.00	0.0500			
135.35	240.23	0.0500			
162.42	239.76	0.0500			
170.97	239.52	0.0500 / 0.0350	Main Channel		
171.58	239.03	0.0350	Main Channel		
174.02	239.03	0.0350	Main Channel		
176.46	239.03	0.0350	Main Channel		
177.07	239.52	0.0350 / 0.0500	Main Channel		
185.63	239.67	0.0500			
208.83	239.87	0.0500			
239.77	240.14	0.0500			
274.57	244.83	0.0500			
336.45	249.51	0.0500			
363.52	249.77	0.0500			
362.85	251.78	0.0500			

<----- TRAVEL TIME TABLE ----->						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.49	257.05	.135E+05	2.5	0.92	91.32	
1.14	240.17	.175E+06	25.6	0.73	114.22	
1.78	240.81	.532E+06	123.5	1.17	71.85	
2.43	241.46	.932E+06	285.3	1.54	54.48	
3.07	242.10	.138E+07	506.5	1.85	45.25	
3.72	242.75	.186E+07	786.8	2.12	39.42	
4.36	243.39	.239E+07	1127.3	2.37	35.33	
5.01	244.04	.296E+07	1535.8	2.61	32.12	
5.65	244.68	.358E+07	2014.3	2.84	29.48	
6.30	245.33	.420E+07	2535.9	3.03	27.60	
6.94	245.97	.488E+07	3108.4	3.20	26.16	
7.59	246.62	.561E+07	3740.6	3.35	24.98	
8.23	247.26	.638E+07	4446.7	3.50	23.93	
8.88	247.91	.721E+07	5229.7	3.64	22.98	
9.52	248.55	.809E+07	6094.3	3.78	22.12	
10.17	249.20	.901E+07	7050.4	3.93	21.31	
10.81	249.84	.100E+08	7773.0	3.90	21.46	
11.46	250.49	.111E+08	8998.8	4.07	20.56	
12.10	251.13	.122E+08	10318.2	4.24	19.75	

<---- hydrograph ---->				<-pipe / channel->			
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (8372)	1967.31	12.93	9.00	24.38	0.78	0.82	
OUTFLOW : ID= 1 (6258)	1967.31	11.62	11.25	24.38	0.75	0.83	

ADD HYD (8374)						
1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0258):			381.99	2.269	8.00	39.73
+ ID2= 2 (6258):			1967.31	11.623	11.25	24.38
=====						
ID = 3 (8374):			2149.30	13.409	11.00	25.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8376)						
1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0260):			476.24	5.896	8.00	43.34
+ ID2= 2 (8374):			2149.30	13.409	11.00	25.68
=====						
ID = 3 (8376):			2625.54	18.584	10.25	28.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8378)						
1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0262):			341.31	5.213	7.75	43.20
+ ID2= 2 (8376):			2625.54	18.584	10.25	28.89
=====						
ID = 3 (8378):			2966.85	22.865	10.00	30.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8362)						
1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8378):			2966.85	22.865	10.00	30.53
+ ID2= 2 (8384):			1321.42	12.139	8.75	31.54
=====						
ID = 3 (8362):			4288.27	34.648	9.50	30.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6272) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (2721.0) ----->			
Distance	Elevation	Manning	
0.00	231.01	0.0450	
23.01	223.65	0.0450	
34.51	222.46	0.0450	
51.77	222.11	0.0450	
69.02	221.87	0.0450	

161.06	221.92	0.0450	
166.81	221.31	0.0450	
172.56	221.89	0.0450	
180.57	221.40	0.0450 / 0.0300	Main Channel
181.57	220.60	0.0300	Main Channel
184.07	220.16	0.0300	Main Channel
195.57	221.85	0.0300 / 0.0450	Main Channel
201.32	221.82	0.0450	
207.07	221.77	0.0450	
212.83	221.72	0.0450	
253.09	222.52	0.0450	
408.40	222.65	0.0450	
460.17	223.20	0.0450	
517.69	224.84	0.0450	
569.46	232.57	0.0450	

375.38	225.71	0.0500
403.79	229.37	0.0500
432.29	230.43	0.0500
470.29	232.00	0.0500

```

-----
<<----- TRAVEL TIME TABLE ----->>
DEPTH   ELEV   VOLUME   FLOW RATE   VELOCITY   TRAV.TIME
(m)     (m)     (cu.m.)   (cms)       (m/s)      (min)
-----
0.41    220.57   .189E+04   0.5         0.49       60.18
0.83    220.99   .697E+04   3.2         0.82       35.79
1.24    221.40   .145E+05   8.8         1.07       27.45
1.84    222.00   .643E+05   29.3        0.81       36.61
2.44    222.60   .282E+06   132.9       0.83       35.41
3.04    223.20   .707E+06   413.7       1.04       28.50
3.64    223.80   .118E+07   876.8       1.31       22.46
4.24    224.40   .168E+07   1494.8      1.57       18.75
4.84    225.00   .221E+07   2264.1      1.82       16.23
5.44    225.60   .274E+07   3196.3      2.06       14.29
6.04    226.20   .328E+07   4258.6      2.30       12.84
6.65    226.81   .383E+07   5444.7      2.52       11.72
7.25    227.41   .438E+07   6749.7      2.73       10.82
7.85    228.01   .494E+07   8169.4      2.93       10.08
8.45    228.61   .551E+07   9700.7      3.12       9.46
9.05    229.21   .608E+07   11340.8     3.30       8.94
9.65    229.81   .666E+07   13087.4     3.48       8.48
10.25   230.41   .724E+07   14938.5     3.65       8.08
10.85   231.01   .783E+07   16892.4     3.82       7.73
-----

```

```

-----
<<----- hydrograph ----->> <-pipe / channel->
AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
(ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
-----
INFLOW : ID= 2 (8362) 4288.27 34.18 10.25 30.84 1.87 0.81
OUTFLOW: ID= 1 (6272) 4288.27 34.18 10.25 30.84 1.87 0.81
-----

```

```

-----
| ADD HYD (8356) |
| 1 + 2 = 3 |
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
-----
ID1= 1 (0272): 157.38 2.475 7.50 35.49
+ ID2= 2 (6272): 4288.27 34.180 10.25 30.84
=====
ID = 3 (8356): 4445.65 35.965 10.00 31.01
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
-----

```

```

-----
| ADD HYD (8352) |
| 1 + 2 = 3 |
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
-----
ID1= 1 (8354): 1268.73 21.461 7.75 40.61
+ ID2= 2 (8356): 4445.65 35.965 10.00 31.01
=====
ID = 3 (8352): 5714.38 54.080 9.00 33.14
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
-----

```

```

-----
| ROUTE CHN (6304) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)= 15.00
-----

```

```

-----
<<----- DATA FOR SECTION (3041.0) ----->>
Distance   Elevation   Manning
-----
0.00       232.08      0.0500
19.00      231.87      0.0500
38.00      231.33      0.0500
66.51      230.44      0.0500
104.51     228.25      0.0500
118.76     225.17      0.0500
128.26     219.86      0.0500
175.77     219.17      0.0500
185.27     218.90      0.0500 / 0.0300 Main Channel
185.52     218.65      0.0300 Main Channel
190.02     218.37      0.0300 Main Channel
194.52     218.60      0.0300 Main Channel
194.77     218.85      0.0300 / 0.0500 Main Channel
204.27     219.60      0.0500
299.28     220.91      0.0500
327.78     222.36      0.0500
-----

```

```

-----
<<----- TRAVEL TIME TABLE ----->>
DEPTH   ELEV   VOLUME   FLOW RATE   VELOCITY   TRAV.TIME
(m)     (m)     (cu.m.)   (cms)       (m/s)      (min)
-----
0.48    218.85   .121E+05   2.2         0.36       173.94
1.17    219.54   .839E+05   9.8         0.44       142.60
1.86    220.23   .323E+06   41.0        0.47       131.43
2.56    220.93   .708E+06   109.5       0.58       107.67
3.25    221.62   .118E+07   224.3       0.71       87.36
3.94    222.31   .168E+07   376.5       0.84       74.48
4.63    223.00   .222E+07   567.9       0.96       65.26
5.32    223.69   .279E+07   794.7       1.07       58.58
6.02    224.39   .339E+07   1054.2      1.17       53.52
6.71    225.08   .402E+07   1352.4      1.26       49.52
7.40    225.77   .468E+07   1677.3      1.34       46.46
8.09    226.46   .536E+07   2050.0      1.43       43.58
8.79    227.16   .607E+07   2457.9      1.52       41.14
9.48    227.85   .680E+07   2901.0      1.60       39.05
10.17   228.54   .755E+07   3352.7      1.66       37.53
10.86   229.23   .834E+07   3813.5      1.71       36.48
11.55   229.92   .919E+07   4241.8      1.73       36.13
12.25   230.62   .101E+08  4697.0      1.74       35.93
12.94   231.31   .111E+08  5186.3      1.74       35.83
-----

```

```

-----
<<----- hydrograph ----->> <-pipe / channel->
AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
(ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
-----
INFLOW : ID= 2 (8352) 5714.38 54.08 9.00 33.14 2.00 0.49
OUTFLOW: ID= 1 (6304) 5714.38 44.75 11.50 33.14 1.90 0.48
-----

```

```

-----
| ADD HYD (8350) |
| 1 + 2 = 3 |
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
-----
ID1= 1 (0304): 292.37 1.571 10.25 25.48
+ ID2= 2 (6304): 5714.38 54.08 9.00 33.14
=====
ID = 3 (8350): 6006.75 46.284 11.50 32.77
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
-----

```

```

-----
| ADD HYD (8346) |
| 1 + 2 = 3 |
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
-----
ID1= 1 (8348): 3768.46 34.216 9.50 30.36
+ ID2= 2 (8350): 6006.75 46.284 11.50 32.77
=====
ID = 3 (8346): 9775.21 78.525 10.50 31.84
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
-----

```

```

-----
| ADD HYD (8344) |
| 1 + 2 = 3 |
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
-----
ID1= 1 (0306): 283.97 0.538 13.75 18.51
+ ID2= 2 (8346): 9775.21 78.525 10.50 31.84
=====
ID = 3 (8344): 10059.18 79.003 10.50 31.46
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
-----

```

```

-----
| RESERVOIR (5509) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
OUTFLOW   STORAGE   |   OUTFLOW   STORAGE
(cms)     (ha.m.)   |   (cms)     (ha.m.)
-----
0.0000    0.0000   |   *****   370.0451
41.0590   56.7403   |   *****   863.4386
48.1390   86.3439   |   *****   *****
56.6340   *****   |   *****   *****
67.9600   *****   |   0.0000    0.0000
-----
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
-----
INFLOW : ID= 2 (8344) ***** 79.003 10.50 31.46
OUTFLOW: ID= 1 (5509) ***** 50.565 15.50 31.46
-----

```

```

-----
PEAK FLOW REDUCTION [Qout/Qin](%)= 64.00
TIME SHIFT OF PEAK FLOW (min)=300.00
MAXIMUM STORAGE USED (ha.m.)= 96.9336
-----

```



```

-----
| CALIB |
| NASHYD (0332) | Area (ha)= 393.44 Curve Number (CN)= 75.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.32
-----

```

```

Unit Hyd Qpeak (cms)= 2.894

PEAK FLOW (cms)= 3.428 (1)
TIME TO PEAK (hrs)= 9.500
RUNOFF VOLUME (mm)= 35.702
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.442

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0330) | Area (ha)= 468.30 Curve Number (CN)= 80.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.09
-----

```

```

Unit Hyd Qpeak (cms)= 7.335

PEAK FLOW (cms)= 8.597 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 40.876
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.506

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ROUTE CHN (6332) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
-----

```

----- DATA FOR SECTION (3321.0) -----			
Distance	Elevation	Manning	
0.00	234.00	0.0380	
25.85	227.20	0.0380	
96.94	226.44	0.0380	
168.03	227.38	0.0380	
219.73	225.62	0.0380	
342.52	221.57	0.0380	
368.37	221.42	0.0380	
374.83	221.23	0.0380	
379.79	220.98	0.0380 / 0.0300	Main Channel
380.29	220.47	0.0300	Main Channel
381.29	220.47	0.0300	Main Channel
382.79	220.47	0.0300	Main Channel
383.29	220.98	0.0300 / 0.0380	Main Channel
394.22	221.22	0.0380	
400.68	221.33	0.0380	
407.14	221.44	0.0380	
491.16	225.70	0.0380	
568.71	227.55	0.0380	
607.49	230.14	0.0380	
639.80	234.08	0.0380	

```

<----- TRAVEL TIME TABLE ----->

```

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.51	220.98	.554E+04	0.9	0.56	107.27
1.20	221.67	.885E+05	12.1	0.49	122.32
1.88	222.35	.309E+06	64.7	0.76	79.70
2.57	223.04	.615E+06	165.4	0.97	61.99
3.25	223.72	.101E+07	322.1	1.16	52.06
3.94	224.41	.148E+07	542.9	1.33	45.51
4.62	225.09	.204E+07	835.4	1.48	40.77
5.31	225.78	.269E+07	1202.8	1.62	37.27
5.99	226.46	.344E+07	1618.0	1.70	35.46
6.68	227.15	.447E+07	1932.0	1.56	38.56
7.36	227.83	.580E+07	2763.9	1.72	34.96
8.05	228.52	.718E+07	3861.9	1.95	30.97
8.73	229.20	.859E+07	5115.4	2.16	27.98
9.42	229.89	.100E+08	6519.2	2.35	25.64
10.10	230.57	.115E+08	8092.3	2.55	23.69
10.79	231.26	.130E+08	9821.5	2.73	22.05
11.47	231.94	.145E+08	11690.0	2.92	20.68
12.16	232.63	.160E+08	13694.9	3.09	19.52
12.84	233.31	.176E+08	15833.9	3.26	18.52

```

<----- hydrograph -----> <-pipe / channel->

```

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0330)	468.30	8.60	7.25	40.88	0.98	0.51
OUTFLOW: ID= 1 (6332)	468.30	6.50	9.50	40.88	0.85	0.53

```

-----
| ADD HYD (8322) |
| 1 + 2 = 3 |
|-----|
| ID= 1 (0332): | Area (ha)= 393.44 QPEAK (cms)= 3.428 TPEAK (hrs)= 9.50 R.V. (mm)= 35.70
| ID= 2 (6332): | Area (ha)= 468.30 QPEAK (cms)= 6.501 TPEAK (hrs)= 9.50 R.V. (mm)= 40.88
|-----|
| ID = 3 (8322): | Area (ha)= 861.74 QPEAK (cms)= 9.928 TPEAK (hrs)= 9.50 R.V. (mm)= 38.51
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0328) | Area (ha)= 492.92 Curve Number (CN)= 77.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.91
-----

```

```

Unit Hyd Qpeak (cms)= 4.411

PEAK FLOW (cms)= 5.314 (1)
TIME TO PEAK (hrs)= 8.750
RUNOFF VOLUME (mm)= 37.735
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.467

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0326) | Area (ha)= 678.91 Curve Number (CN)= 80.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.95
-----

```

```

Unit Hyd Qpeak (cms)= 5.941

PEAK FLOW (cms)= 7.858 (1)
TIME TO PEAK (hrs)= 8.750
RUNOFF VOLUME (mm)= 41.091
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.509

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0324) | Area (ha)= 615.64 Curve Number (CN)= 79.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.90
-----

```

```

Unit Hyd Qpeak (cms)= 5.544

PEAK FLOW (cms)= 7.077 (1)
TIME TO PEAK (hrs)= 8.500
RUNOFF VOLUME (mm)= 39.932
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.494

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0320) | Area (ha)= 278.74 Curve Number (CN)= 81.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.36
-----

```

```

Unit Hyd Qpeak (cms)= 3.499

PEAK FLOW (cms)= 4.443 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 42.167
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.522

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (3182) | Area (ha)= 457.40 Curve Number (CN)= 78.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.49
-----

```

```

Unit Hyd Qpeak (cms)= 5.259

PEAK FLOW (cms)= 6.226 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 38.748
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.480

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANHYD (3181) | Area (ha)= 26.00
| ID= 1 DT=15.0 min | Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00
-----
| IMPERVIOUS | PERVIOUS (I) |
| Surface Area (ha)= 21.06 | 4.94 |
| Dep. Storage (mm)= 2.00 | 5.00 |
| Average Slope (%)= 0.50 | 0.50 |
| Length (m)= 416.33 | 40.00 |
| Mannings n = 0.013 | 0.250 |
|
| Max.Eff.Inten.(mm/hr)= 106.66 | 58.84 |
| over (min) 15.00 | 30.00 |
| Storage Coeff. (min)= 7.21 (iii) | 20.44 (iii) |
| Unit Hyd. Tpeak (min)= 15.00 | 30.00 |
| Unit Hyd. peak (cms)= 0.10 | 0.05 |
|
| PEAK FLOW (cms)= 5.72 | 0.41 | *TOTALS*
| TIME TO PEAK (hrs)= 6.00 | 6.25 | 5.977 (iii)
| RUNOFF VOLUME (mm)= 78.80 | 37.88 | 71.03
| TOTAL RAINFALL (mm)= 80.80 | 80.80 | 80.80
| RUNOFF COEFFICIENT = 0.98 | 0.47 | 0.68

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 77.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| RESERVOIR (5318) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
| OUTFLOW | STORAGE | OUTFLOW | STORAGE |
| (cms) | (ha.m.) | (cms) | (ha.m.) | |
|---|---|---|---|---|
| 0.0000 | 0.0000 | 3.3800 | 0.7654 |
| 0.0510 | 0.4423 | 4.1500 | 0.8231 |
| 0.8700 | 0.5378 | 4.8750 | 0.8778 |
| 1.9590 | 0.5105 | 5.2750 | 0.8878 |
| 2.5620 | 0.6796 | 0.0000 | 0.0000 |
|-----|-----|-----|-----|
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
| INFLOW : ID= 2 (3181) | 26.000 | 5.977 | 6.00 | 71.03 |
| OUTFLOW : ID= 1 (5318) | 26.000 | 3.426 | 6.25 | 70.94 |
|-----|-----|-----|-----|
| PEAK FLOW REDUCTION [Qout/Qin](%)= 57.33 |
| TIME SHIFT OF PEAK FLOW (min)= 15.00 |
| MAXIMUM STORAGE USED (ha.m.)= 0.8392 |

```

```

-----
| ADD HYD (8334) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) |
| ID= 1 (3182): | 457.40 | 6.226 | 8.00 | 38.75 |
| + ID= 2 (5318): | 26.00 | 3.426 | 6.25 | 70.94 |
|-----|-----|-----|-----|
| ID = 3 (8334): | 483.40 | 7.520 | 6.25 | 40.48 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6320) |
| IN= 2--> OUT= 1 |
-----
| ROUTING time step (min)= 15.00 |
-----
|<----- DATA FOR SECTION (3201.0) ----->|
| Distance | Elevation | Manning | |
|---|---|---|---|
| 0.00 | 249.00 | 0.0380 |
| 22.92 | 245.86 | 0.0380 |
| 45.83 | 244.87 | 0.0380 |
| 91.66 | 243.11 | 0.0380 |
| 126.03 | 239.53 | 0.0380 |
| 160.41 | 237.17 | 0.0380 |
| 166.14 | 237.06 | 0.0380 |
| 177.59 | 237.13 | 0.0380 |
| 183.32 | 237.20 | 0.0380 |
| 189.05 | 236.70 | 0.0380 / 0.0350 | Main Channel |
| 193.05 | 235.89 | 0.0350 | Main Channel |
| 197.05 | 236.64 | 0.0350 / 0.0380 | Main Channel |
| 200.51 | 236.74 | 0.0380 |
| 206.24 | 237.03 | 0.0380 |
| 246.34 | 238.82 | 0.0380 |
| 263.53 | 243.87 | 0.0380 |
| 389.56 | 247.64 | 0.0380 |
| 452.58 | 247.74 | 0.0380 |
| 498.41 | 248.60 | 0.0380 |
| 567.16 | 249.84 | 0.0380 |

```

```

-----
|<----- TRAVEL TIME TABLE ----->|
| DEPTH | ELEV | VOLUME | FLOW RATE | VELOCITY | TRAV.TIME |
| (m) | (m) | (cu.m.) | (cms) | (m/s) | (min) |
|-----|-----|-----|-----|-----|-----|
| 0.37 | 236.26 | .270E+04 | 0.4 | 0.58 | 107.96 |
| 0.75 | 236.64 | .108E+05 | 2.6 | 0.92 | 68.01 |
| 1.48 | 237.37 | .877E+05 | 27.4 | 1.17 | 53.35 |
| 2.20 | 238.09 | .277E+06 | 122.1 | 1.65 | 37.83 |
| 2.93 | 238.82 | .540E+06 | 299.7 | 2.08 | 30.01 |
| 3.66 | 239.55 | .807E+06 | 587.8 | 2.57 | 24.29 |
| 4.39 | 240.28 | .120E+07 | 977.8 | 3.04 | 20.52 |
| 5.11 | 241.00 | .158E+07 | 1458.1 | 3.46 | 18.02 |
| 5.84 | 241.73 | .198E+07 | 2028.6 | 3.84 | 16.23 |
| 6.57 | 242.46 | .240E+07 | 2690.5 | 4.19 | 14.86 |
| 7.29 | 243.18 | .285E+07 | 3431.8 | 4.50 | 13.84 |
| 8.02 | 243.91 | .335E+07 | 4149.1 | 4.64 | 13.44 |
| 8.75 | 244.64 | .393E+07 | 4746.6 | 4.51 | 13.80 |
| 9.47 | 245.36 | .463E+07 | 5518.6 | 4.53 | 13.75 |
| 10.20 | 246.09 | .545E+07 | 6781.8 | 4.66 | 13.39 |
| 10.93 | 246.82 | .635E+07 | 8274.5 | 4.87 | 12.79 |
| 11.66 | 247.55 | .733E+07 | 9988.0 | 5.10 | 12.23 |
| 12.38 | 248.27 | .853E+07 | 11422.0 | 5.01 | 12.45 |
| 13.11 | 249.00 | .989E+07 | 13749.7 | 5.20 | 11.98 |

```

```

-----
|<---- hydrograph ----> |<-pipe / channel->| | | | | |
| AREA | QPEAK | TPEAK | R.V. | MAX DEPTH | MAX VEL |
| (ha) | (cms) | (hrs) | (mm) | (m) | (m/s) |
| INFLOW : ID= 2 (8334) | 483.40 | 7.52 | 6.25 | 40.48 | 0.89 | 0.96 |
| OUTFLOW : ID= 1 (6320) | 483.40 | 6.12 | 9.00 | 40.48 | 0.85 | 0.95 |

```

```

-----
| ADD HYD (8332) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) |
| ID= 1 (0320): | 278.74 | 4.443 | 8.00 | 42.17 |
| + ID= 2 (6320): | 483.40 | 6.122 | 9.00 | 40.48 |
|-----|-----|-----|-----|
| ID = 3 (8332): | 762.14 | 10.364 | 8.50 | 41.10 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0314) | Area (ha)= 165.20 Curve Number (CN)= 78.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.71 |
-----
| Unit Hyd Qpeak (cms)= 3.978 |
| PEAK FLOW (cms)= 4.019 (i) |
| TIME TO PEAK (hrs)= 6.750 |
| RUNOFF VOLUME (mm)= 38.307 |
| TOTAL RAINFALL (mm)= 80.800 |
| RUNOFF COEFFICIENT = 0.474 |
| (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY. |

```

```

-----
| ROUTE CHN (6316) |
| IN= 2--> OUT= 1 |
-----
| Routing time step (min)= 15.00 |
-----
|<----- DATA FOR SECTION (3161.0) ----->|
| Distance | Elevation | Manning | |
|---|---|---|---|
| 0.00 | 248.54 | 0.0350 |
| 27.93 | 248.34 | 0.0350 |
| 41.89 | 246.61 | 0.0350 |
| 62.83 | 243.09 | 0.0350 |
| 132.65 | 239.00 | 0.0350 |
| 188.50 | 236.74 | 0.0350 |
| 195.48 | 236.54 | 0.0350 |
| 202.46 | 236.32 | 0.0350 |
| 205.48 | 236.14 | 0.0350 / 0.0310 | Main Channel |
| 205.98 | 235.61 | 0.0310 | Main Channel |
| 207.98 | 235.25 | 0.0310 | Main Channel |
| 209.98 | 235.53 | 0.0310 | Main Channel |
| 210.48 | 236.00 | 0.0310 / 0.0350 | Main Channel |
| 216.42 | 236.73 | 0.0350 |
| 258.31 | 239.09 | 0.0350 |
| 328.12 | 239.84 | 0.0350 |
| 439.83 | 241.47 | 0.0350 |
| 530.58 | 242.08 | 0.0350 |
| 586.43 | 242.93 | 0.0350 |
| 691.16 | 248.00 | 0.0350 |

```

```

-----
|<----- TRAVEL TIME TABLE ----->|
| DEPTH | ELEV | VOLUME | FLOW RATE | VELOCITY | TRAV.TIME |
| (m) | (m) | (cu.m.) | (cms) | (m/s) | (min) |
|-----|-----|-----|-----|-----|-----|
| 0.37 | 236.62 | .215E+04 | 0.7 | 0.77 | 53.88 |
| 0.75 | 236.00 | .634E+04 | 3.5 | 1.36 | 30.45 |
| 1.46 | 236.71 | .297E+05 | 19.3 | 1.62 | 25.66 |
| 2.16 | 237.41 | .103E+06 | 75.8 | 1.84 | 22.60 |

```

2.87	238.12	.228E+06	202.8	2.21	18.79
3.57	238.82	.407E+06	423.3	2.59	16.03
4.28	239.53	.654E+06	727.7	2.77	14.97
4.99	240.24	.103E+07	1242.0	3.01	13.76
5.59	240.94	.153E+07	2002.3	3.31	12.54
6.40	241.65	.210E+07	2987.9	3.55	11.70
7.10	242.35	.286E+07	4302.8	3.75	11.07
7.81	243.06	.374E+07	6165.3	4.10	10.11
8.51	243.76	.468E+07	8642.4	4.60	9.03
9.22	244.47	.565E+07	11496.3	5.06	8.20
9.93	245.18	.666E+07	14714.1	5.50	7.54
10.63	245.88	.770E+07	18290.3	5.91	7.02
11.34	246.59	.877E+07	22221.9	6.30	6.58
12.04	247.29	.988E+07	26464.4	6.67	6.22
12.75	248.00	.110E+08	31061.3	7.01	5.91

<---- hydrograph ----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (0314)	165.20	4.02	6.75	38.31	0.77	1.37
OUTFLOW: ID= 1 (6316)	165.20	3.84	7.25	38.31	0.77	1.37

PEAK FLOW (cms) = 2.144 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 26.616
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.329

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8342)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID= 1 (0308):	529.30	4.194	8.25	24.70
+ ID= 2 (0310):	118.28	2.144	7.00	26.62
ID = 3 (8342):	667.58	6.044	7.75	25.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6312) Routing time step (min) = 15.00
 IN= 2 -> OUT= 1

Distance	Elevation	Manning
0.00	265.94	0.0360
38.07	265.43	0.0360
59.82	263.98	0.0360
103.32	254.59	0.0360
157.70	252.16	0.0360
217.52	250.45	0.0360
233.84	247.69	0.0360
234.71	247.27	0.0360
239.71	246.38	0.0360
244.71	246.12	0.0360 / 0.0330
247.71	245.17	0.0330
249.71	245.19	0.0330
251.71	245.64	0.0330 / 0.0360
259.71	246.67	0.0360
282.78	247.12	0.0360
315.41	251.60	0.0360
424.17	256.13	0.0360
478.55	257.04	0.0360
516.62	259.37	0.0360
538.37	266.00	0.0360

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.47	245.64	.111E+04	3.1	1.78	6.07
1.54	246.71	.103E+05	53.9	3.39	3.19
2.61	247.78	.414E+05	281.3	4.40	2.45
3.68	248.85	.839E+05	749.1	5.79	1.87
4.74	249.91	.135E+06	1463.2	6.97	1.55
5.81	250.98	.201E+06	2354.8	7.61	1.42
6.88	252.05	.295E+06	3533.0	7.76	1.39
7.95	253.12	.427E+06	5303.9	8.05	1.34
9.02	254.19	.594E+06	7854.5	8.58	1.26
10.09	255.26	.792E+06	11449.2	9.37	1.15
11.15	256.32	.101E+07	15862.4	10.15	1.06
12.22	257.39	.127E+07	21074.5	10.75	1.01
13.29	258.46	.155E+07	28184.1	11.78	0.92
14.36	259.53	.185E+07	36386.2	12.77	0.85
15.43	260.60	.213E+07	46110.6	13.89	0.78
16.50	261.67	.248E+07	56822.5	14.95	0.72
17.56	262.73	.278E+07	68498.1	15.97	0.68
18.63	263.80	.310E+07	81118.8	16.94	0.64
19.70	264.87	.344E+07	93603.2	17.66	0.61

<---- hydrograph ----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (8342)	667.58	6.04	7.75	25.10	0.53	1.83
OUTFLOW: ID= 1 (6312)	667.58	6.04	8.00	25.10	0.53	1.83

CALIB |
 NASHYD (0316) | Area (ha) = 232.34 Curve Number (CN) = 82.0
 ID= 1 DT=15.0 min | Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.01

Unit Hyd Qpeak (cms) = 3.923
 PEAK FLOW (cms) = 4.823 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 43.236
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.535

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8338) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID= 1 (0316): 232.34 4.823 7.25 43.24
 + ID= 2 (6316): 165.20 3.837 7.25 38.31
 ID = 3 (8338): 397.54 8.660 7.25 41.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB |
 NASHYD (0312) | Area (ha) = 359.44 Curve Number (CN) = 80.0
 ID= 1 DT=15.0 min | Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.36

Unit Hyd Qpeak (cms) = 4.529
 PEAK FLOW (cms) = 5.575 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 40.979
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.507

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHYD (0308) | Area (ha) = 529.30 Curve Number (CN) = 62.0
 ID= 1 DT=15.0 min | Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.62

Unit Hyd Qpeak (cms) = 5.575
 PEAK FLOW (cms) = 4.194 (i)
 TIME TO PEAK (hrs) = 8.250
 RUNOFF VOLUME (mm) = 24.698
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.306

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHYD (0310) | Area (ha) = 138.28 Curve Number (CN) = 65.0
 ID= 1 DT=15.0 min | Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 0.76

Unit Hyd Qpeak (cms) = 3.102

ADD HYD (8340) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID= 1 (0312): 359.44 5.575 8.00 40.98
 + ID= 2 (6312): 667.58 6.040 8.00 25.10
 ID = 3 (8340): 1027.02 11.615 8.00 30.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8336)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8336):	397.54	8.660	7.25	41.19
+ ID2= 2 (8340):	1027.02	11.615	8.00	30.65
=====				
ID = 3 (8336):	1424.56	20.099	7.50	33.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8330)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8332):	762.14	10.364	8.50	41.10
+ ID2= 2 (8336):	1424.56	20.099	7.50	33.59
=====				
ID = 3 (8330):	2186.70	29.999	7.75	36.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6324) |
| IIS= 2 -> OPT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3241.0) ----->

Distance	Elevation	Manning	
0.00	234.38	0.0360	
33.57	232.48	0.0360	
67.14	230.14	0.0360	
83.93	228.80	0.0360	
134.29	227.62	0.0360	
209.82	225.10	0.0360	
218.21	224.86	0.0360	
226.61	224.47	0.0360	
234.00	223.86	0.0360 / 0.0300	Main Channel
234.10	223.66	0.0300	Main Channel
235.00	223.66	0.0300	Main Channel
235.90	223.66	0.0300	Main Channel
236.00	223.86	0.0300 / 0.0360	Main Channel
243.39	224.92	0.0360	
251.78	224.89	0.0360	
335.71	225.64	0.0360	
562.32	226.53	0.0360	
637.85	228.36	0.0360	
705.00	229.80	0.0360	
830.89	234.00	0.0360	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.20	223.86	.145E+04	0.2	0.43	147.27
0.73	224.39	.159E+05	2.8	0.68	93.11
1.27	224.93	.561E+05	11.0	0.75	85.19
1.80	225.46	.217E+06	45.6	0.80	79.20
2.33	225.99	.566E+06	134.4	0.91	70.20
2.87	226.53	.122E+07	336.1	1.06	59.97
3.40	227.06	.208E+07	744.3	1.38	46.16
3.94	227.60	.298E+07	1295.9	1.66	38.38
4.47	228.13	.399E+07	1979.9	1.90	33.59
5.00	228.66	.509E+07	2807.5	2.11	30.21
5.54	229.20	.627E+07	3815.1	2.32	27.41
6.07	229.73	.753E+07	4984.1	2.53	25.17
6.60	230.26	.884E+07	6346.3	2.74	23.20
7.14	230.80	.102E+08	7861.2	2.95	21.61
7.67	231.33	.116E+08	9526.4	3.14	20.29
8.21	231.87	.131E+08	11342.2	3.32	19.18
8.74	232.40	.146E+08	13309.6	3.49	18.22
9.27	232.93	.161E+08	15414.0	3.66	17.41
9.81	233.47	.177E+08	17671.0	3.81	16.70

<----- hydrograph -----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8330) 2186.70	30.00	7.75	36.21	1.56	0.78
OUTFLOW : ID= 1 (6324) 2186.70	25.63	9.50	36.21	1.49	0.77

ADD HYD (8328)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0324):	615.64	7.077	8.50	39.93
+ ID2= 2 (6324):	2186.70	25.633	9.50	36.21
=====				
ID = 3 (8328):	2802.34	32.579	9.50	37.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB			
NASHVD (0322)	Area (ha)	Curve Number (CN)=	# of Linear Res.(N)=
ID= 1 DT=15.0 min	513.13	5.00	1.50
U.H. Tp(hrs)= 1.75			

Unit Hyd Qpeak (cms)= 5.020

PEAK FLOW (cms)= 6.502 (1)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 41.063
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.508

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8326)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0322):	513.13	6.502	8.25	41.06
+ ID2= 2 (8328):	2802.34	32.579	9.50	37.03
=====				
ID = 3 (8326):	3315.47	38.885	9.25	37.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8324)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0326):	678.91	7.858	8.75	41.09
+ ID2= 2 (8326):	3315.47	38.885	9.25	37.65
=====				
ID = 3 (8324):	3994.38	46.684	9.25	38.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6328) |
| IIS= 2 -> OPT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3281.0) ----->

Distance	Elevation	Manning	
0.00	228.00	0.0380	
18.58	224.97	0.0380	
78.98	223.52	0.0380	
125.44	223.28	0.0380	
171.90	221.71	0.0380	
213.72	219.65	0.0380	
218.36	219.40	0.0380	
223.01	219.19	0.0380	
225.95	219.14	0.0380 / 0.0320	Main Channel
226.45	218.14	0.0320	Main Channel
236.95	218.14	0.0320	Main Channel
245.85	218.14	0.0320	Main Channel
245.95	219.14	0.0320 / 0.0380	Main Channel
246.24	219.16	0.0380	
250.88	219.24	0.0380	
255.53	219.39	0.0380	
325.22	221.47	0.0380	
367.03	223.14	0.0380	
404.20	225.17	0.0380	
459.95	228.04	0.0380	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.50	218.64	.373E+05	3.2	0.33	192.37
1.00	219.14	.752E+05	10.1	0.51	124.71
1.52	219.66	.149E+06	22.2	0.57	112.06
2.04	220.18	.279E+06	43.4	0.59	107.06
2.56	220.70	.464E+06	77.0	0.63	100.52
3.08	221.22	.705E+06	125.7	0.68	93.56
3.61	221.75	.100E+07	192.7	0.73	86.62
4.13	222.27	.135E+07	278.1	0.79	80.92
4.65	222.79	.176E+07	385.9	0.84	75.86
5.17	223.31	.222E+07	514.9	0.89	71.82
5.69	223.83	.280E+07	662.7	0.90	70.40
6.21	224.35	.345E+07	867.7	0.96	66.35
6.73	224.87	.417E+07	1107.0	1.01	62.81
7.25	225.39	.494E+07	1400.0	1.08	58.80
7.78	225.92	.573E+07	1732.2	1.15	55.17
8.30	226.44	.655E+07	2099.9	1.22	52.02
8.82	226.96	.740E+07	2502.8	1.29	49.29
9.34	227.48	.828E+07	2940.9	1.36	46.90
9.86	228.00	.918E+07	3414.3	1.42	44.79

<----- hydrograph -----> <-pipe / channel->

	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8324)	3994.38	46.68	9.25	38.24	2.09	0.60
OUTFLOW: ID= 1 (6328)	3994.38	39.91	11.00	38.24	1.96	0.59

ADD HYD (8320)						
1 + 2 = 3						
	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (8328):	492.92	5.314	8.75	37.73		
+ ID2= 2 (6328):	3994.38	39.913	11.00	38.24		
ID = 3 (8320):	4487.30	44.685	10.75	38.18		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8318)						
1 + 2 = 3						
	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (8320):	4487.30	44.685	10.75	38.18		
+ ID2= 2 (8322):	861.74	9.928	9.50	38.51		
ID = 3 (8318):	5349.04	54.210	10.75	38.23		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8316)						
1 + 2 = 3						
	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (8509):	10059.30	50.223	6.50	41.92		
+ ID2= 2 (8318):	5349.04	54.210	10.75	38.23		
ID = 3 (8316):	15408.22	97.899	11.75	33.81		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8312)						
1 + 2 = 3						
	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (8314):	620.10	1.884	6.50	21.94		
+ ID2= 2 (8316):	15408.22	97.899	11.75	33.81		
ID = 3 (8312):	16028.32	98.974	11.75	33.35		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8308)						
1 + 2 = 3						
	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (8310):	598.90	33.223	6.50	41.92		
+ ID2= 2 (8312):	16028.32	98.974	11.75	33.35		
ID = 3 (8308):	16627.22	101.128	11.50	33.66		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5510)						
IN= 2--> OUT= 1						
DT= 15.0 min						
	OUTFLOW	STORAGE	OUTFLOW	STORAGE		
	(cms)	(ha.m.)	(cms)	(ha.m.)		
	0.0000	0.0000	*****	74.0090		
	66.5450	18.5023	*****	*****		
	98.5430	37.0045	*****	*****		

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8308)	*****	101.128	11.50	33.66
OUTFLOW: ID= 1 (5510)	*****	96.575	13.25	33.66

PEAK FLOW REDUCTION [Qout/Qin] (%) = 95.50
 TIME SHIFT OF PEAK FLOW (min) = 105.00
 MAXIMUM STORAGE USED (ha.m.) = 35.8687

ADD HYD (8240)						
1 + 2 = 3						
	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0336):	2785.00	3.992	22.25	32.86
+ ID2= 2 (5510):	16627.22	96.575	13.25	33.66
ID = 3 (8240):	19412.22	100.014	13.25	33.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8238)						
1 + 2 = 3						
	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (5508):	9524.23	50.303	12.00	21.33		
+ ID2= 2 (8240):	19412.22	100.014	13.25	33.55		
ID = 3 (8238):	28936.45	149.326	12.75	29.53		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8236)						
1 + 2 = 3						
	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (0342):	1004.58	4.420	10.00	19.62		
+ ID2= 2 (8238):	28936.45	149.326	12.75	29.53		
ID = 3 (8236):	29941.03	153.277	12.75	29.20		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8234)						
1 + 2 = 3						
	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (8232):	285.80	9.337	6.50	28.35		
+ ID2= 2 (8236):	29941.03	153.277	12.75	29.20		
ID = 3 (8234):	30226.83	153.639	12.75	29.19		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8230)						
1 + 2 = 3						
	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (0344):	344.00	4.034	7.00	22.48		
+ ID2= 2 (8234):	30226.83	153.639	12.75	29.19		
ID = 3 (8230):	30570.83	154.850	12.50	29.11		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8228)						
1 + 2 = 3						
	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (8226):	1952.06	13.442	8.75	18.81		
+ ID2= 2 (8230):	30570.83	154.850	12.50	29.11		
ID = 3 (8228):	32522.89	163.469	12.25	28.49		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8190)						
1 + 2 = 3						
	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (0356):	1015.22	5.609	8.00	15.27		
+ ID2= 2 (8228):	32522.89	163.469	12.25	28.49		
ID = 3 (8190):	33538.11	166.987	12.25	28.09		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ** SIMULATION NUMBER: 12 **

READ STORM	Filename:
	C:\Users\jascott\AppData\Local\Temp\

```

-----
| ADD HYD (8190) |
| 1 + 2 3 |
|-----|
| AREA OPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0356): 1015.22 2.534 8.00 6.98 |
| + ID2= 2 (8228): 32522.89 87.695 12.50 14.27 |
|-----|
| ID = 3 (8190): 33538.11 89.249 12.50 14.05 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

*****
** SIMULATION NUMBER: 6 **
*****

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```

-----
| READ STORM |
|-----|
| Filename: C:\Users\jscott\AppData |
| Local\Temp\ |
| 3e280798-92ee-4282-809c-79f5caed0add\2687e965 |
| Comments: 100-Year 12-Hour SCS II Design Storm |
-----

```

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	2.21	3.25	3.54	6.25	15.93	9.25	3.10
0.50	2.21	3.50	3.54	6.50	15.93	9.50	3.10
0.75	2.21	3.75	3.54	6.75	7.08	9.75	3.10
1.00	2.21	4.00	3.54	7.00	7.08	10.00	3.10
1.25	2.21	4.25	5.31	7.25	5.31	10.25	1.77
1.50	2.21	4.50	5.31	7.50	5.31	10.50	1.77
1.75	2.21	4.75	7.08	7.75	5.31	10.75	1.77
2.00	2.21	5.00	7.08	8.00	5.31	11.00	1.77
2.25	2.65	5.25	10.62	8.25	3.10	11.25	1.77
2.50	2.65	5.50	10.62	8.50	3.10	11.50	1.77
2.75	2.65	5.75	42.48	8.75	3.10	11.75	1.77
3.00	2.65	6.00	116.82	9.00	3.10	12.00	1.77

```

-----
| CALIB |
| NASHYD (0356) | Area (ha)=1015.22 Curve Number (CN)= 46.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 1.37 |
-----

```

```

Unit Hyd Qpeak (cms)= 12.651

PEAK FLOW (cms)= 6.685 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 18.151
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.205

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0354) | Area (ha)= 262.68 Curve Number (CN)= 37.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 1.38 |
-----

```

```

Unit Hyd Qpeak (cms)= 3.252

PEAK FLOW (cms)= 1.265 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 13.427
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.152

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0352) | Area (ha)= 381.43 Curve Number (CN)= 54.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 0.82 |
-----

```

```

Unit Hyd Qpeak (cms)= 7.980

PEAK FLOW (cms)= 4.770 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 22.930
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.259

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
|-----|

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```

-----
| NASHYD (0346) | Area (ha)= 350.93 Curve Number (CN)= 70.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 0.96 |
-----

```

```

Unit Hyd Qpeak (cms)= 6.254

```

```

PEAK FLOW (cms)= 6.173 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 35.852
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.405

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0350) | Area (ha)= 366.84 Curve Number (CN)= 48.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 1.07 |
-----

```

```

Unit Hyd Qpeak (cms)= 5.831

```

```

PEAK FLOW (cms)= 3.076 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 19.261
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.218

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0348) | Area (ha)= 590.18 Curve Number (CN)= 48.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 1.91 |
-----

```

```

Unit Hyd Qpeak (cms)= 5.267

```

```

PEAK FLOW (cms)= 3.191 (i)
TIME TO PEAK (hrs)= 9.000
RUNOFF VOLUME (mm)= 19.364
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.219

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ROUTE CHN (6350) |
| IN= 2--> OUT= 1 |
|-----|

```

<----- DATA FOR SECTION (3501.0) ----->

Distance	Elevation	Manning
0.00	287.00	0.0500
23.66	283.72	0.0500
70.98	280.94	0.0500
112.38	280.23	0.0500
171.52	276.80	0.0500
260.24	274.46	0.0500
266.16	274.26	0.0500
272.07	274.12	0.0500
276.49	274.12	0.0500 / 0.0300
276.99	273.82	0.0300
277.99	273.82	0.0300
279.49	273.82	0.0300
280.49	274.27	0.0300 / 0.0500
283.90	274.27	0.0500
289.81	274.57	0.0500
325.30	275.75	0.0500
396.28	278.98	0.0500
449.51	280.97	0.0500
496.83	283.90	0.0500
585.54	287.92	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.30	274.12	273E+04	0.9	0.99	49.81
0.98	274.80	583E+05	20.8	1.05	46.71
1.66	275.48	203E+06	95.2	1.38	35.53
2.33	276.15	438E+06	256.0	1.73	28.50
3.01	276.83	755E+06	524.8	2.05	23.97
3.69	277.51	114E+07	935.8	2.43	20.26
4.37	278.19	157E+07	1472.0	2.76	17.82
5.05	278.87	206E+07	2140.7	3.06	16.06
5.72	279.54	261E+07	2938.6	3.33	14.79
6.40	280.22	321E+07	3887.6	3.57	13.77
7.08	280.90	390E+07	4795.4	3.63	13.56
7.76	281.58	468E+07	6196.2	3.91	12.58
8.43	282.25	550E+07	7801.9	4.19	11.74
9.11	282.93	637E+07	9601.9	4.45	11.05
9.79	283.61	728E+07	11594.8	4.70	10.46
10.47	284.29	823E+07	13845.3	4.96	9.91

11.15 284.97 .922E+07 16293.4 5.21 9.43
 11.82 285.64 .103E+08 18952.0 5.45 9.02
 12.50 286.32 .113E+08 21824.3 5.69 8.65

```

<----- hydrograph -----> <-pipe / channel->
AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (0348) 590.18 3.19 9.00 19.36 0.38 0.99
OUTFLOW: ID= 1 (6350) 590.18 3.11 10.25 19.36 0.37 0.99
  
```

```

| ADD HYD (8220) |
| 1 + 2 = 3 |
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0350): 366.84 3.076 7.50 19.26
+ ID2= 2 (6350): 590.18 3.111 10.25 19.36
=====
ID = 3 (8220): 957.02 5.697 8.75 19.32
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ADD HYD (8222) |
| 1 + 2 = 3 |
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0346): 350.93 6.173 7.25 35.85
+ ID2= 2 (8220): 957.02 5.697 8.75 19.32
=====
ID = 3 (8222): 1307.95 11.432 8.00 23.76
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ROUTE CHN (6352) |
| IN= 2--> OUT= 1 |
Routing time step (min)'= 15.00
  
```

```

<----- DATA FOR SECTION (3521.0) ----->
Distance Elevation Manning
0.00 257.95 0.0500
7.63 257.34 0.0500
15.66 256.19 0.0500
21.53 254.01 0.0500
41.11 244.06 0.0500
76.35 241.38 0.0500
111.58 239.74 0.0500
113.54 239.61 0.0500
113.75 239.41 0.0500 /0.0300 Main Channel
113.85 238.81 0.0300 Main Channel
115.50 238.81 0.0300 Main Channel
116.15 238.81 0.0300 Main Channel
116.25 239.41 0.0300 Main Channel
117.46 239.52 0.0500
119.41 239.72 0.0500
121.37 240.04 0.0500
131.16 241.84 0.0500
156.61 247.03 0.0500
176.19 251.46 0.0500
193.80 258.79 0.0500
  
```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.60 239.41 .224E+04 2.2 1.52 17.12
1.58 240.39 .216E+05 22.3 1.61 16.16
2.55 241.36 .804E+05 101.0 1.96 13.27
3.53 242.34 .173E+06 275.6 2.48 10.46
4.50 243.31 .293E+06 553.9 2.95 8.90
5.48 244.29 .438E+06 961.8 3.42 7.60
6.45 245.26 .599E+06 1534.4 3.99 6.51
7.43 246.24 .770E+06 2228.4 4.51 5.76
8.41 247.22 .951E+06 3043.6 4.98 5.21
9.38 248.19 .114E+07 3982.1 5.43 4.78
10.36 249.17 .134E+07 5040.1 5.85 4.44
11.33 250.14 .155E+07 6210.5 6.24 4.16
12.31 251.12 .177E+07 7518.6 6.61 3.93
13.29 252.10 .200E+07 8969.0 6.98 3.72
14.26 253.07 .224E+07 10554.1 7.35 3.53
15.24 254.05 .248E+07 12257.1 7.71 3.37
16.21 255.02 .272E+07 14045.1 8.03 3.23
17.19 256.00 .298E+07 15954.8 8.34 3.11
18.16 256.97 .325E+07 17780.4 8.53 3.04
  
```

```

<----- hydrograph -----> <-pipe / channel->
AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8222) 1307.95 11.43 8.00 23.76 1.05 1.56
OUTFLOW: ID= 1 (6352) 1307.95 11.35 8.25 23.76 1.05 1.56
  
```

```

| ADD HYD (8224) |
| 1 + 2 = 3 |
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0352): 381.43 4.770 7.00 22.93
+ ID2= 2 (6352): 1307.95 11.354 8.25 23.76
=====
ID = 3 (8224): 1689.38 15.685 7.75 23.57
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ROUTE CHN (6354) |
| IN= 2--> OUT= 1 |
Routing time step (min)'= 15.00
  
```

```

<----- DATA FOR SECTION (3541.0) ----->
Distance Elevation Manning
0.00 253.92 0.0500
7.95 251.83 0.0500
15.89 249.97 0.0500
19.87 249.05 0.0500
47.68 242.00 0.0500
67.55 237.10 0.0500
83.44 231.11 0.0500
85.43 230.75 0.0500
85.66 230.61 0.0500 /0.0300 Main Channel
85.76 230.01 0.0300 Main Channel
87.41 230.01 0.0300 Main Channel
88.06 230.01 0.0300 Main Channel
88.16 230.61 0.0300 /0.0500 Main Channel
89.40 230.72 0.0500
91.39 230.88 0.0500
93.37 231.03 0.0500
133.11 233.96 0.0500
150.99 235.39 0.0500
172.84 239.03 0.0500
196.68 250.00 0.0500
  
```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.60 230.61 .451E+04 1.5 1.06 49.40
1.62 231.63 .394E+05 14.6 1.16 45.13
2.64 232.65 .128E+06 58.8 1.44 36.30
3.66 233.67 .269E+06 149.7 1.74 30.00
4.68 234.69 .463E+06 301.5 2.04 25.58
5.70 235.71 .705E+06 530.5 2.36 22.14
6.72 236.73 .980E+06 848.4 2.71 19.25
7.74 237.75 .128E+07 1237.9 3.02 17.29
8.76 238.77 .162E+07 1706.7 3.30 15.83
9.78 239.79 .199E+07 2291.9 3.61 14.45
10.81 240.82 .237E+07 2973.7 3.92 13.30
11.83 241.84 .278E+07 3742.2 4.21 12.38
12.85 242.86 .321E+07 4599.1 4.49 11.62
13.87 243.88 .365E+07 5544.6 4.75 10.98
14.89 244.90 .412E+07 6579.7 5.00 10.44
15.91 245.92 .461E+07 7705.9 5.23 9.97
16.93 246.94 .512E+07 8924.6 5.46 9.55
17.95 247.96 .564E+07 10237.5 5.68 9.19
18.97 248.98 .619E+07 11646.2 5.89 8.86
  
```

```

<----- hydrograph -----> <-pipe / channel->
AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8224) 1689.38 15.68 7.75 23.57 1.65 1.16
OUTFLOW: ID= 1 (6354) 1689.38 14.70 8.75 23.57 1.62 1.16
  
```

```

| ADD HYD (8226) |
| 1 + 2 = 3 |
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0354): 262.68 1.265 8.00 13.43
+ ID2= 2 (6354): 1689.38 14.695 8.75 23.57
=====
ID = 3 (8226): 1952.06 15.927 8.75 22.21
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| CALIB |
| NASHYD (0344) |
| ID= 1 DT=15.0 min |
Area (ha)= 344.00 Curve Number (CN)= 59.0
Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.87
  
```

```

Unit Hyd Qpeak (cms)= 6.790
PEAK FLOW (cms)= 4.771 (i)
TIME TO PEAK (hrs)= 7.000
  
```

RUNOFF VOLUME (mm)= 26.476
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.299

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANHYD (3402) | Area (ha)= 234.60
| ID= 1 DT=15.0 min | Total Imp(%)= 28.00 Dir. Conn.(%)= 13.00
-----
| IMPERVIOUS | PERVIOUS (i)
| Surface Area (ha)= 65.69 | 168.91
| Dep. Storage (mm)= 2.00 | 5.00
| Average Slope (%)= 0.50 | 0.50
| Length (m)= 1250.60 | 40.00
| Mannings n = 0.013 | 0.250
-----
| Max.Eff.Inten.(mm/hr)= 116.82 | 25.11
| over (min) | 15.00 | 45.00
| Storage Coeff. (min)= 13.46 (ii) | 32.05 (ii)
| Unit Hyd. Tpeak (min)= 15.00 | 45.00
| Unit Hyd. peak (cms)= 0.08 | 0.03
-----
| *TOTALS*
| PEAK FLOW (cms)= 7.54 | 7.07 | 10.471 (iii)
| TIME TO PEAK (hrs)= 6.00 | 6.50 | 6.00
| RUNOFF VOLUME (mm)= 86.50 | 22.80 | 31.08
| TOTAL RAINFALL (mm)= 88.50 | 88.50 | 88.50
| RUNOFF COEFFICIENT = 0.98 | 0.26 | 0.35
-----

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANHYD (3401) | Area (ha)= 51.20
| ID= 1 DT=15.0 min | Total Imp(%)= 46.00 Dir. Conn.(%)= 21.00
-----
| IMPERVIOUS | PERVIOUS (i)
| Surface Area (ha)= 23.55 | 27.65
| Dep. Storage (mm)= 2.00 | 5.00
| Average Slope (%)= 0.50 | 0.50
| Length (m)= 584.24 | 40.00
| Mannings n = 0.013 | 0.250
-----
| Max.Eff.Inten.(mm/hr)= 116.82 | 58.14
| over (min) | 15.00 | 30.00
| Storage Coeff. (min)= 8.52 (ii) | 21.81 (ii)
| Unit Hyd. Tpeak (min)= 15.00 | 30.00
| Unit Hyd. peak (cms)= 0.09 | 0.05
-----
| *TOTALS*
| PEAK FLOW (cms)= 3.08 | 2.17 | 4.437 (iii)
| TIME TO PEAK (hrs)= 6.00 | 6.25 | 6.00
| RUNOFF VOLUME (mm)= 86.50 | 26.50 | 39.10
| TOTAL RAINFALL (mm)= 88.50 | 88.50 | 88.50
| RUNOFF COEFFICIENT = 0.98 | 0.30 | 0.44
-----

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| RESERVOIR (5340) |
| IN= 2 -> OUT= 1 |
| DT= 15.0 min |
-----
| OUTFLOW | STORAGE | OUTFLOW | STORAGE
| (cms) | (ha.m.) | (cms) | (ha.m.)
|-----|-----|-----|-----|
| 0.0000 | 0.0000 | 5.3360 | 1.7877
| 0.1730 | 0.6348 | 7.2780 | 2.2185
| 1.8360 | 0.8250 | 9.1880 | 2.6486
| 2.1540 | 1.3051 | 9.5880 | 2.6586
| 3.3950 | 1.3506 | 0.0000 | 0.0000
-----
| AREA | QPEAK | TPEAK | R.V.
| (ha) | (cms) | (hrs) | (mm)
|-----|-----|-----|-----|
| INFLOW : ID= 2 (3401) | 51.200 | 4.437 | 6.00 | 39.10
| OUTFLOW : ID= 1 (5340) | 51.200 | 1.889 | 6.50 | 39.08
-----
| PEAK FLOW REDUCTION [Qout/Qin](%)= 42.58
| TIME SHIFT OF PEAK FLOW (min)= 30.00
| MAXIMUM STORAGE USED (ha.m.)= 0.8780
-----

```

```

-----
| ADD HYD (8232) |
| 1 + 2 + 3 |
-----
| AREA | QPEAK | TPEAK | R.V.
| (ha) | (cms) | (hrs) | (mm)
|-----|-----|-----|-----|
| ID= 1 (3402): | 234.60 | 10.471 | 6.00 | 31.08
| + ID= 2 (5340): | 51.20 | 1.889 | 6.50 | 39.08
| -----
| ID= 3 (8232): | 285.80 | 10.972 | 6.50 | 32.52
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0342) | Area (ha)=1004.58 Curve Number (CN)= 54.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 2.57
-----
| Unit Hyd Qpeak (cms)= 6.686
| PEAK FLOW (cms)= 5.230 (i)
| TIME TO PEAK (hrs)= 10.000
| RUNOFF VOLUME (mm)= 23.193
| TOTAL RAINFALL (mm)= 88.500
| RUNOFF COEFFICIENT = 0.262
-----

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0422) | Area (ha)= 780.20 Curve Number (CN)= 54.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.95
-----
| Unit Hyd Qpeak (cms)= 6.838
| PEAK FLOW (cms)= 4.992 (i)
| TIME TO PEAK (hrs)= 9.000
| RUNOFF VOLUME (mm)= 23.163
| TOTAL RAINFALL (mm)= 88.500
| RUNOFF COEFFICIENT = 0.262
-----

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0410) | Area (ha)= 572.01 Curve Number (CN)= 48.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.46
-----
| Unit Hyd Qpeak (cms)= 6.698
| PEAK FLOW (cms)= 3.821 (i)
| TIME TO PEAK (hrs)= 8.000
| RUNOFF VOLUME (mm)= 19.326
| TOTAL RAINFALL (mm)= 88.500
| RUNOFF COEFFICIENT = 0.218
-----

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0408) | Area (ha)= 231.62 Curve Number (CN)= 58.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.64
-----
| Unit Hyd Qpeak (cms)= 6.198
| PEAK FLOW (cms)= 3.942 (i)
| TIME TO PEAK (hrs)= 6.750
| RUNOFF VOLUME (mm)= 25.554
| TOTAL RAINFALL (mm)= 88.500
| RUNOFF COEFFICIENT = 0.289
-----

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0376) | Area (ha)= 463.85 Curve Number (CN)= 74.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.07
-----
| Unit Hyd Qpeak (cms)= 7.380
| PEAK FLOW (cms)= 8.365 (i)
| TIME TO PEAK (hrs)= 7.250
| RUNOFF VOLUME (mm)= 39.991
| TOTAL RAINFALL (mm)= 88.500
| RUNOFF COEFFICIENT = 0.452
-----

```


(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0374) Area (ha)= 545.70 Curve Number (CN)= 61.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.51

Unit Hyd Qpeak (cms)= 6.158

PEAK FLOW (cms)= 5.223 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 28.198
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.319

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0372) Area (ha)= 110.42 Curve Number (CN)= 37.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 1.954

PEAK FLOW (cms)= 0.691 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 13.367
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.151

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0370) Area (ha)= 191.85 Curve Number (CN)= 63.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.67

Unit Hyd Qpeak (cms)= 4.860

PEAK FLOW (cms)= 3.631 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 29.417
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.332

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0368) Area (ha)= 159.48 Curve Number (CN)= 46.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.12

Unit Hyd Qpeak (cms)= 2.433

PEAK FLOW (cms)= 1.216 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 18.110
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.205

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0366) Area (ha)= 462.62 Curve Number (CN)= 62.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.06

Unit Hyd Qpeak (cms)= 7.451

PEAK FLOW (cms)= 5.968 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 28.878
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.326

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0364) Area (ha)= 155.27 Curve Number (CN)= 55.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.70

Unit Hyd Qpeak (cms)= 3.782

PEAK FLOW (cms)= 2.245 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 23.520
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.266

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8302)
1 + 2 = 3 AREA QPEAK TPEAK R.V.
ID= 1 (0364): 155.27 2.245 6.75 23.52
+ ID= 2 (0366): 462.62 5.968 7.50 28.88
=====

ID = 3 (8302): 617.89 8.124 7.25 27.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (0362) Area (ha)= 118.78
ID= 1 DT=15.0 min Total Imp(%)= 22.00 Dir. Conn.(%)= 8.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 26.13 92.65
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 889.87 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.82 21.80
over (min)= 15.00 45.00
Storage Coeff. (min)= 10.97 (ii) 30.65 (ii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.08 0.03

PEAK FLOW (cms)= 2.53 3.45 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.50 4.005 (iii)
RUNOFF VOLUME (mm)= 86.50 20.43 25.72
TOTAL RAINFALL (mm)= 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.23 0.29

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 45.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0358) Area (ha)= 429.87 Curve Number (CN)= 35.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.03

Unit Hyd Qpeak (cms)= 7.091

PEAK FLOW (cms)= 2.367 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 12.436
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.141

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0360) Area (ha)= 138.37 Curve Number (CN)= 46.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.60

Unit Hyd Qpeak (cms)= 3.957

PEAK FLOW (cms)= 1.703 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 17.868
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.202

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8306)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0358): 429.87 2.367 7.50 12.44
+ ID2= 2 (0360): 138.37 1.703 6.75 17.87
=====
ID = 3 (8306): 568.24 3.976 7.00 13.76

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (8362)
IN= 2--> OUT= 1
Routing time step (min)'= 15.00

```

```

----- DATA FOR SECTION (3621.0) -----
Distance Elevation Manning
0.00 261.46 0.0550
27.86 254.23 0.0550
51.07 251.96 0.0550
74.29 250.77 0.0550
97.50 249.91 0.0550
125.36 249.40 0.0550
150.93 247.40 0.0550
155.93 247.33 0.0550 /0.0350 Main Channel
157.93 246.85 0.0350 Main Channel
159.18 246.65 0.0350 Main Channel
160.18 246.63 0.0350 Main Channel
160.93 246.85 0.0350 Main Channel
161.93 247.18 0.0350 /0.0550 Main Channel
163.18 248.03 0.0550
168.18 248.58 0.0550
183.18 250.18 0.0550
201.97 252.59 0.0550
213.57 256.02 0.0550
225.18 280.31 0.0550
229.82 261.00 0.0550

```

```

----- TRAVEL TIME TABLE -----
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.55 247.18 .550E+04 1.9 1.03 49.95
1.28 247.91 .336E+05 16.7 1.53 33.61
2.00 248.63 .913E+05 53.6 1.81 28.40
2.73 249.36 .185E+06 122.3 2.05 25.18
3.46 250.09 .344E+06 224.6 2.02 25.52
4.19 250.82 .574E+06 417.9 2.25 22.91
4.91 251.54 .855E+06 699.1 2.53 20.38
5.64 252.27 .118E+07 1075.3 2.82 18.26
6.37 253.00 .153E+07 1559.9 3.15 16.38
7.10 253.73 .191E+07 2137.6 3.46 14.89
7.82 254.45 .231E+07 2815.2 3.77 13.67
8.55 255.18 .272E+07 3608.5 4.10 12.58
9.28 255.91 .315E+07 4488.2 4.41 11.69
10.01 256.64 .358E+07 5458.0 4.71 10.95
10.73 257.36 .403E+07 6512.4 4.99 10.32
11.46 258.09 .449E+07 7649.8 5.27 9.78
12.19 258.82 .496E+07 8869.5 5.53 9.32
12.92 259.55 .544E+07 10171.2 5.78 8.91
13.64 260.27 .593E+07 11554.8 6.02 8.55

```

```

----- hydrograph ----- <-pipe / channel-->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8306) 568.24 3.98 7.00 13.76 0.65 1.08
OUTFLOW: ID= 1 (8362) 568.24 3.52 8.25 13.76 0.63 1.07

```

```

ADD HYD (8304)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0362): 118.78 4.005 6.50 25.72
+ ID2= 2 (8362): 568.24 3.522 8.25 13.76
=====
ID = 3 (8304): 687.02 5.831 6.50 15.83

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8300)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8302): 617.89 8.124 7.25 27.53
+ ID2= 2 (8304): 687.02 5.831 6.50 15.83
=====
ID = 3 (8300): 1304.91 13.263 6.50 21.37

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (8368)
IN= 2--> OUT= 1
Routing time step (min)'= 15.00

```

```

----- DATA FOR SECTION (3681.0) -----
Distance Elevation Manning
0.00 230.00 0.0370
18.48 223.26 0.0370
36.96 223.05 0.0370
64.67 222.94 0.0370
110.87 222.86 0.0370
133.96 222.74 0.0370
147.82 222.65 0.0370
170.92 222.31 0.0370
174.79 222.26 0.0370 /0.0300 Main Channel
174.89 221.86 0.0300 Main Channel
180.16 222.25 0.0370 Main Channel
176.19 221.86 0.0300 Main Channel
176.29 222.26 0.0300 /0.0370 Main Channel
184.78 222.28 0.0370
189.40 222.31 0.0370
332.60 222.37 0.0370
450.00 230.00 0.0370

```

```

----- TRAVEL TIME TABLE -----
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.40 222.26 .771E+03 0.3 0.52 41.74
0.81 222.67 .784E+05 30.3 0.51 43.17
1.21 223.07 .210E+06 123.2 0.77 28.42
1.62 223.48 .384E+06 301.0 1.03 21.25
2.03 223.89 .563E+06 550.2 1.28 17.06
2.44 224.30 .747E+06 860.7 1.51 14.46
2.84 224.70 .934E+06 1228.1 1.73 12.68
3.25 225.11 .113E+07 1649.3 1.92 11.37
3.66 225.52 .132E+07 2122.3 2.11 10.37
4.07 225.93 .152E+07 2645.5 2.29 9.58
4.47 226.33 .172E+07 3217.8 2.45 8.92
4.88 226.74 .193E+07 3838.3 2.61 8.38
5.29 227.15 .214E+07 4506.3 2.76 7.92
5.70 227.56 .236E+07 5221.3 2.91 7.52
6.10 227.96 .258E+07 5983.0 3.05 7.17
6.51 228.37 .280E+07 6790.9 3.19 6.87
6.92 228.78 .303E+07 7645.0 3.32 6.60
7.33 229.19 .326E+07 8545.1 3.45 6.35
7.73 229.59 .349E+07 9491.0 3.57 6.13

```

```

----- hydrograph ----- <-pipe / channel-->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8300) 1304.91 13.26 6.50 21.37 0.58 0.52
OUTFLOW: ID= 1 (8368) 1304.91 12.01 8.00 21.37 0.56 0.52

```

```

ADD HYD (8298)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0368): 159.48 1.216 7.75 18.11
+ ID2= 2 (8368): 1304.91 12.014 8.00 21.37
=====
ID = 3 (8298): 1464.39 13.223 8.00 21.01

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8296)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0370): 191.85 3.631 6.75 29.42
+ ID2= 2 (8298): 1464.39 13.223 8.00 21.01
=====
ID = 3 (8296): 1656.24 16.201 7.75 21.99

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (8372)
IN= 2--> OUT= 1
Routing time step (min)'= 15.00

```

```

----- DATA FOR SECTION (3721.0) -----
Distance Elevation Manning
0.00 225.00 0.0390
30.80 219.38 0.0390
61.61 219.30 0.0390
77.01 219.27 0.0390
469.76 219.14 0.0390
477.46 219.13 0.0390
485.16 219.10 0.0390

```

492.86	219.09	0.0390	
495.56	219.09	0.0390 / 0.0310	Main Channel
495.66	218.51	0.0310	Main Channel
500.56	218.51	0.0310	Main Channel
505.46	218.51	0.0310	Main Channel
505.56	219.09	0.0310 / 0.0390	Main Channel
508.26	219.09	0.0390	
515.96	219.10	0.0390	
523.67	219.21	0.0390	
562.17	219.32	0.0390	
654.58	219.43	0.0390	
731.59	219.46	0.0390	
762.39	225.00	0.0390	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.29	218.80	.431E+04	1.7	0.58	43.48
0.58	219.09	.867E+04	5.1	0.89	28.32
0.93	219.44	.200E+06	58.2	0.44	57.35
1.28	219.79	.568E+06	278.6	0.74	33.99
1.62	220.13	.939E+06	630.2	1.01	24.84
1.97	220.48	.131E+07	1090.4	1.25	20.05
2.32	220.83	.169E+07	1647.7	1.47	17.06
2.67	221.18	.206E+07	2294.6	1.68	14.99
3.01	221.52	.244E+07	3025.6	1.87	13.46
3.36	221.87	.282E+07	3836.5	2.05	12.27
3.71	222.22	.321E+07	4723.7	2.22	11.31
4.06	222.57	.359E+07	5684.6	2.39	10.53
4.40	222.91	.398E+07	6716.6	2.55	9.87
4.75	223.26	.437E+07	7817.7	2.70	9.31
5.10	223.61	.476E+07	8986.2	2.85	8.83
5.45	223.96	.515E+07	10220.4	2.99	8.40
5.79	224.30	.555E+07	11519.1	3.13	8.03
6.14	224.65	.594E+07	12880.8	3.27	7.69
6.49	225.00	.634E+07	14304.5	3.40	7.39

<---- hydrograph ----> <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8296) 1656.24	16.20	7.75	21.99	0.65	0.73
OUTFLOW: ID= 1 (6372) 1656.24	15.45	8.50	21.99	0.65	0.74

----- ADD HYD (8294) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0372):	110.42	0.691	7.25	13.37
+ ID2= 2 (6372):	1656.24	15.450	8.50	21.99
=====				
ID = 3 (8294):	1766.66	16.075	8.50	21.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ADD HYD (8292) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0374):	545.70	5.223	8.25	28.20
+ ID2= 2 (8294):	1766.66	16.075	8.50	21.45
=====				
ID = 3 (8292):	2312.36	21.266	8.50	23.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- RESERVOIR (5505) -----

IN= 2--> OUT= 1	DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
		0.0000	0.0000	65.1290	345.3754
		25.4850	24.6697	84.9510	456.3890
		31.1490	98.6787	*****	838.7689
		39.6440	*****	*****	838.7789
		48.1390	*****	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8292) 2312.360	21.266	8.50	23.04
OUTFLOW: ID= 1 (5505) 2312.360	15.623	11.25	23.04

PEAK FLOW REDUCTION [Qout/Qin](%)= 73.47
 TIME SHIFT OF PEAK FLOW (min)=165.00
 MAXIMUM STORAGE USED (ha.m.)= 15.1370

----- ADD HYD (8272) -----

----- 1 + 2 = 3 -----

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
ID1= 1 (0376):	463.85	8.365	7.25	39.99
+ ID2= 2 (5505):	2312.36	15.623	11.25	23.04
=====				
ID = 3 (8272):	2776.21	20.788	10.25	25.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- CALIB -----

NASHVD (0396)	Area (ha)	= 305.21	Curve Number (CN)= 69.0
ID= 1 DT=15.0 min	Ia (mm)	= 5.00	# of Linear Res.(N)= 1.50
-----	U.H. Tp(hrs)	= 1.08	

Unit Hyd Qpeak (cms)= 4.811

PEAK FLOW (cms)= 4.736 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 34.962
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.395

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB -----

NASHVD (0394)	Area (ha)	= 325.45	Curve Number (CN)= 53.0
ID= 1 DT=15.0 min	Ia (mm)	= 5.00	# of Linear Res.(N)= 1.50
-----	U.H. Tp(hrs)	= 0.92	

Unit Hyd Qpeak (cms)= 6.013

PEAK FLOW (cms)= 3.577 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 22.323
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.252

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB -----

NASHVD (0390)	Area (ha)	= 420.00	Curve Number (CN)= 55.0
ID= 1 DT=15.0 min	Ia (mm)	= 5.00	# of Linear Res.(N)= 1.50
-----	U.H. Tp(hrs)	= 1.07	

Unit Hyd Qpeak (cms)= 6.683

PEAK FLOW (cms)= 4.371 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 23.713
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.268

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB -----

NASHVD (0388)	Area (ha)	= 220.77	Curve Number (CN)= 58.0
ID= 1 DT=15.0 min	Ia (mm)	= 5.00	# of Linear Res.(N)= 1.50
-----	U.H. Tp(hrs)	= 0.99	

Unit Hyd Qpeak (cms)= 3.819

PEAK FLOW (cms)= 2.679 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 25.800
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.292

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB -----

NASHVD (0386)	Area (ha)	= 241.27	Curve Number (CN)= 61.0
ID= 1 DT=15.0 min	Ia (mm)	= 5.00	# of Linear Res.(N)= 1.50
-----	U.H. Tp(hrs)	= 0.90	

Unit Hyd Qpeak (cms)= 4.562

PEAK FLOW (cms)= 3.426 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 28.018
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.317

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8286) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
ID1= 1 (0386): 241.27 3.426 7.00 28.02
+ ID2= 2 (0388): 220.77 2.679 7.25 25.80
=====
ID = 3 (8286): 462.04 6.100 7.25 26.96
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0384) | Area (ha)= 199.07 Curve Number (CN)= 44.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.96
-----

```

```

Unit Hyd Qpeak (cms)= 3.537
PEAK FLOW (cms)= 1.596 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 16.955
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.192

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0380) | Area (ha)= 182.01 Curve Number (CN)= 40.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.55
-----

```

```

Unit Hyd Qpeak (cms)= 5.609
PEAK FLOW (cms)= 1.939 (i)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 14.644
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.165

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0382) | Area (ha)= 216.59 Curve Number (CN)= 53.0
| ID= 1 DT=15.0 min | Ia (mm)= 6.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.64
-----

```

```

Unit Hyd Qpeak (cms)= 5.733
PEAK FLOW (cms)= 3.143 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 22.142
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.250

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8290) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
ID1= 1 (0380): 182.01 1.939 6.50 14.64
+ ID2= 2 (0382): 216.59 3.143 6.75 22.14
=====
ID = 3 (8290): 398.60 5.066 6.75 18.72
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6384) |
| IN= 2--> OUF= 1 | Routing time step (min)= 15.00
-----

```

```

<----- DATA FOR SECTION (3841.0) ----->
Distance Elevation Manning
0.00 294.40 0.0380
10.59 291.93 0.0380
21.17 289.19 0.0380
26.46 287.99 0.0380
31.76 286.79 0.0380
71.45 279.97 0.0380
74.10 279.79 0.0380
76.74 279.71 0.0380
78.99 279.64 0.0380 / 0.0300 Main Channel
79.14 279.30 0.0300 Main Channel
79.39 279.30 0.0300 Main Channel
79.64 279.30 0.0300 Main Channel

```

```

79.79 279.64 0.0300 / 0.0380 Main Channel
89.98 279.78 0.0380
119.09 281.86 0.0380
145.55 282.87 0.0380
198.48 284.85 0.0380
211.71 286.31 0.0380
230.23 287.59 0.0380
261.99 294.00 0.0380

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.34 279.64 .715E+03 0.2 0.85 63.42
1.10 280.40 .507E+05 21.9 1.40 38.54
1.85 281.15 .141E+06 91.8 2.10 25.69
2.61 281.91 .269E+06 218.9 2.63 20.46
3.36 282.66 .448E+06 407.1 2.96 18.23
4.12 283.42 .681E+06 698.3 3.31 16.26
4.87 284.17 .977E+06 1111.8 3.68 14.65
5.63 284.93 .133E+07 1674.3 4.06 13.27
6.39 285.69 .173E+07 2457.8 4.59 11.73
7.14 286.44 .215E+07 3378.8 5.07 10.63
7.90 287.20 .261E+07 4418.4 5.47 9.85
8.65 287.95 .310E+07 5676.3 5.92 9.11
9.41 288.71 .361E+07 7150.5 6.40 8.42
10.17 289.47 .414E+07 8783.1 6.86 7.86
10.92 290.22 .469E+07 10575.1 7.30 7.38
11.68 290.98 .525E+07 12520.4 7.72 6.98
12.43 291.73 .582E+07 14619.0 8.12 6.64
13.19 292.49 .642E+07 16863.2 8.50 6.34
13.94 293.24 .703E+07 19259.8 8.86 6.08

```

```

<---- hydrograph ----> <-pipe / channel-->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8290) 398.60 5.07 6.75 18.72 0.51 0.93
OUTFLOW : ID= 1 (6384) 398.60 3.99 7.75 18.72 0.47 0.91

```

```

-----
| ADD HYD (8288) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
ID1= 1 (0384): 199.07 1.596 7.25 16.95
+ ID2= 2 (6384): 398.60 3.985 7.75 18.92
=====
ID = 3 (8288): 597.67 5.554 7.75 18.13
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8284) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
ID1= 1 (8286): 462.04 6.100 7.25 26.96
+ ID2= 2 (8288): 597.67 5.554 7.75 18.13
=====
ID = 3 (8284): 1059.71 11.532 7.50 21.98
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8280) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
ID1= 1 (0390): 420.00 4.371 7.50 23.71
+ ID2= 2 (8284): 1059.71 11.532 7.50 21.98
=====
ID = 3 (8280): 1479.71 15.903 7.50 22.47
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0392) | Area (ha)= 167.22 Curve Number (CN)= 62.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.74
-----

```

```

Unit Hyd Qpeak (cms)= 3.837
PEAK FLOW (cms)= 2.840 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 28.690
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.324

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0378) | Area (ha)= 606.72 Curve Number (CN)= 55.0
| ID= 1 DT=15.0 min | Is (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.18

```

```

Unit Hyd Qpeak (cms)= 8.771

PEAK FLOW (cms)= 5.877 (1)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 23.743
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.268

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8282) |
| 1 + 2 = 3 |
|-----|
| ID1= 1 (0378): 606.72 5.877 7.75 23.74
+ ID2= 2 (0392): 167.22 2.840 7.00 28.69
|-----|
| ID = 3 (8282): 773.94 8.554 7.25 24.81

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8278) |
| 1 + 2 = 3 |
|-----|
| ID1= 1 (8280): 1479.71 15.903 7.50 22.47
+ ID2= 2 (8282): 773.94 8.554 7.25 24.81
|-----|
| ID = 3 (8278): 2253.65 24.428 7.50 23.28

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6394) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
|-----|

```

```

<----- DATA FOR SECTION (3941.0) ----->
Distance Elevation Manning
0.00 283.00 0.0380
13.48 282.87 0.0380
53.92 280.08 0.0380
74.13 276.62 0.0380
97.72 265.45 0.0380
114.57 256.93 0.0380
131.42 253.04 0.0380
134.79 252.58 0.0380
138.53 251.74 0.0380 /0.0300 Main Channel
139.03 251.20 0.0300 Main Channel
141.53 251.20 0.0300 Main Channel
144.03 251.20 0.0300 Main Channel
144.53 251.74 0.0300 /0.0380 Main Channel
148.27 252.69 0.0380
151.64 252.97 0.0380
185.34 255.08 0.0380
219.03 257.54 0.0380
262.84 259.43 0.0380
310.02 262.80 0.0380
333.60 283.00 0.0380

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.54 251.74 .827E+04 4.5 1.51 30.71
2.19 253.39 .758E+05 73.2 2.69 17.27
3.83 255.03 .282E+06 331.8 3.27 14.17
5.48 256.68 .633E+06 917.3 4.03 11.51
7.12 258.32 .112E+07 1889.5 4.69 9.90
8.77 259.97 .179E+07 3410.2 5.31 8.73
10.41 261.61 .259E+07 5627.6 6.05 7.66
12.06 263.26 .351E+07 8605.1 6.83 6.79
13.70 264.90 .447E+07 12544.9 7.81 5.94
15.35 266.55 .548E+07 17103.4 8.71 5.32
16.99 268.19 .648E+07 22251.7 9.56 4.85
18.64 269.84 .752E+07 27979.0 10.36 4.48
20.28 271.48 .858E+07 34269.0 11.11 4.17
21.93 273.13 .967E+07 41210.8 11.83 3.92
23.57 274.77 .108E+08 48495.9 12.51 3.71
25.22 276.42 .119E+08 56418.1 13.17 3.52
26.86 278.06 .131E+08 64410.6 13.69 3.39
28.51 279.71 .143E+08 73002.1 14.18 3.27
30.15 281.35 .156E+08 81379.3 14.50 3.20

```

<---- hydrograph ----> <-pipe / channel->

```

AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8278) 2253.65 24.43 7.50 23.28 1.02 1.73
OUTFLOW: ID= 1 (6394) 2253.65 23.85 8.00 23.28 1.00 1.72

```

```

-----
| ADD HYD (8276) |
| 1 + 2 = 3 |
|-----|
| ID1= 1 (0394): 325.45 3.577 7.25 23.32
+ ID2= 2 (6394): 2253.65 23.853 8.00 23.28
|-----|
| ID = 3 (8276): 2579.10 27.252 8.00 23.16

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6396) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
|-----|

```

```

<----- DATA FOR SECTION (3961.0) ----->
Distance Elevation Manning
0.00 283.00 0.0410
11.75 287.14 0.0410
23.50 283.97 0.0410
41.13 247.83 0.0410
76.38 232.09 0.0410
135.13 229.07 0.0410
149.82 228.97 0.0410
152.75 228.96 0.0410 /0.0300 Main Channel
154.19 228.73 0.0300 Main Channel
154.69 228.20 0.0300 Main Channel
155.69 228.20 0.0300 Main Channel
156.69 228.20 0.0300 Main Channel
157.19 228.73 0.0300 Main Channel
158.63 228.95 0.0300 /0.0410 Main Channel
161.57 228.96 0.0410
164.51 229.71 0.0410
196.82 241.70 0.0410
223.26 249.21 0.0410
246.76 255.13 0.0410
290.82 263.51 0.0410

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.75 228.95 .780E+04 2.2 0.88 58.06
2.54 230.74 .277E+06 145.1 1.78 31.76
4.33 232.53 .786E+06 609.1 2.63 21.50
6.13 234.33 .140E+07 1483.5 3.59 15.77
7.92 236.12 .207E+07 2677.3 4.39 12.90
9.71 237.91 .280E+07 4184.8 5.09 11.14
11.50 239.70 .357E+07 6001.4 5.71 9.93
13.29 241.49 .441E+07 8123.1 6.27 9.03
15.09 243.29 .530E+07 10549.4 6.77 8.37
16.88 245.08 .625E+07 13300.6 7.24 7.83
18.67 246.87 .726E+07 16396.2 7.68 7.38
20.46 248.66 .834E+07 19808.8 8.07 7.02
22.26 250.46 .949E+07 23532.5 8.43 6.72
24.05 252.25 .107E+08 27641.2 8.77 6.46
25.84 254.04 .120E+08 32155.2 9.10 6.22
27.63 255.83 .134E+08 36901.0 9.37 6.05
29.42 257.62 .149E+08 42147.7 9.64 5.88
31.22 259.42 .164E+08 48149.1 9.97 5.68
33.01 261.21 .181E+08 54644.8 10.29 5.51

```

```

<---- hydrograph ----> <-pipe / channel->
AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8276) 2579.10 27.25 8.00 23.16 1.06 1.06
OUTFLOW: ID= 1 (6396) 2579.10 24.64 9.00 23.16 1.03 1.05

```

```

-----
| ADD HYD (8274) |
| 1 + 2 = 3 |
|-----|
| ID1= 1 (0396): 305.21 4.736 7.50 34.96
+ ID2= 2 (6396): 2579.10 24.639 9.00 23.16
|-----|
| ID = 3 (8274): 2884.31 28.747 8.75 24.40

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8270) |
| 1 + 2 = 3 |
|-----|

```

```

-----
      (ha)  (cms)  (hrs)  (mm)
ID1= 1 (8272): 2776.21 20.788 10.25 25.87
+ ID2= 2 (8274): 2884.31 28.747  8.75 24.40
=====
ID = 3 (8270): 5660.52 48.391  9.25 25.12

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
RESERVOIR (5506)
IN= 2--> OUT= 1
DT= 15.0 min
-----
      OUTFLOW STORAGE | OUTFLOW STORAGE
      (cms) (ha.m.) | (cms) (ha.m.)
-----
0.0000 0.0000 | 60.8810 135.6832
31.1490 24.6697 | 96.2770 900.4431
36.8120 37.0045 | 96.4770 900.4531
45.3070 86.3439 | 0.0000 0.0000
-----
      AREA QPEAK TPEAK R.V.
      (ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (8270) 5660.520 48.391 9.25 25.12
OUTFLOW: ID= 1 (5506) 5660.520 35.821 12.25 25.12
-----
      PEAK FLOW REDUCTION [Qout/qin](%)= 74.02
      TIME SHIFT OF PEAK FLOW (min)=180.00
      MAXIMUM STORAGE USED (ha.m.)= 34.8735

```

```

-----
CALIB (0406) | Area (ha)= 142.65 Curve Number (CN)= 66.0
NASHYD (0406) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.59
-----

```

Unit Hyd Qpeak (cms)= 4.135

```

PEAK FLOW (cms)= 3.269 (i)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 31.802
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.359

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB (0404) | Area (ha)= 246.46 Curve Number (CN)= 47.0
NASHYD (0404) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.98
-----

```

Unit Hyd Qpeak (cms)= 4.280

```

PEAK FLOW (cms)= 2.141 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 18.650
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.211

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB (0402) | Area (ha)= 244.00 Curve Number (CN)= 61.0
NASHYD (0402) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.07
-----

```

Unit Hyd Qpeak (cms)= 3.879

```

PEAK FLOW (cms)= 3.028 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 28.095
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.317

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB (0400) | Area (ha)= 93.97 Curve Number (CN)= 52.0
NASHYD (0400) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.44
-----

```

Unit Hyd Qpeak (cms)= 3.630

```

PEAK FLOW (cms)= 1.763 (i)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 21.182
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.239

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB (0398) | Area (ha)= 328.19 Curve Number (CN)= 55.0
NASHYD (0398) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.83
-----

```

Unit Hyd Qpeak (cms)= 6.759

```

PEAK FLOW (cms)= 4.178 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 23.611
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.267

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
ADD HYD (8268) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
      (ha) (cms) (hrs) (mm)
-----
ID1= 1 (0398): 328.19 4.178 7.00 23.61
+ ID2= 2 (0400): 93.97 1.763 6.50 21.18
=====
ID = 3 (8268): 422.16 5.776 6.75 23.07

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ROUTE CHN (6402) |
IN= 2--> OUT= 1 | Routing time step (min)= 15.00
-----

```

<----- DATA FOR SECTION (4021.0) ----->

Distance	Elevation	Manning
0.00	138.50	0.0360
11.50	238.00	0.0360
23.00	237.93	0.0360
34.49	236.39	0.0360
63.24	233.98	0.0360
97.73	228.15	0.0360
123.60	227.08	0.0360
126.48	226.61	0.0360
127.60	226.47	0.0360 / 0.0330
127.85	225.25	0.0330
129.35	225.25	0.0330
130.85	225.25	0.0330
131.10	226.47	0.0330 / 0.0360
132.22	226.59	0.0360
143.72	227.42	0.0360
169.59	227.88	0.0360
192.59	231.19	0.0360
218.46	233.02	0.0360
241.45	235.50	0.0360
284.57	236.43	0.0360

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.41	225.66	.391E+04	1.7	1.36	38.31
0.81	226.06	.804E+04	5.0	1.93	27.02
1.22	226.47	1.248E+05	9.1	2.30	22.61
1.84	227.09	3.08E+05	23.3	2.36	22.03
2.46	227.71	9.04E+05	62.0	2.14	24.31
3.09	228.34	2.23E+06	186.3	2.63	19.75
3.71	228.96	3.79E+06	410.2	3.38	15.39
4.33	229.58	5.50E+06	711.5	4.04	12.89
4.95	230.20	7.37E+06	1091.3	4.62	11.25
5.58	230.83	9.39E+06	1550.6	5.15	10.10
6.20	231.45	1.16E+07	2070.4	5.59	9.31
6.82	232.07	1.40E+07	2659.2	5.93	8.77
7.44	232.69	1.67E+07	3345.8	6.27	8.30
8.07	233.32	1.95E+07	4159.0	6.64	7.83
8.69	233.94	2.26E+07	5100.8	7.03	7.39
9.31	234.56	2.59E+07	6081.3	7.32	7.11
9.93	235.18	2.95E+07	7183.1	7.60	6.84
10.56	235.81	3.34E+07	8445.2	7.82	6.63
11.18	236.43	3.79E+07	9115.0	7.51	6.92

```

-----
hydrograph <----> <-pipe / channel->
      AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
      (ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8268) 422.16 5.78 6.75 23.07 0.89 1.99
OUTFLOW: ID= 1 (6402) 422.16 5.53 7.25 23.07 0.87 1.97

```

```

-----
ADD HYD (8266) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
      (ha) (cms) (hrs) (mm)
-----

```

ID1= 1 (0402): 244.00 3.028 7.50 28.09
 + ID2= 2 (6402): 422.16 8.530 7.25 23.07
 =====
 ID = 3 (8266): 666.16 8.550 7.25 24.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8264) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0404): 246.46 2.141 7.25 18.65
 + ID2= 2 (8266): 666.16 8.550 7.25 24.91
 =====
 ID = 3 (8264): 912.62 10.690 7.25 23.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8262) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0406): 142.65 3.269 6.50 31.80
 + ID2= 2 (8264): 912.62 10.690 7.25 23.22
 =====
 ID = 3 (8262): 1055.27 13.662 7.00 24.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8260) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (5506): 5660.52 35.821 12.25 25.12
 + ID2= 2 (8262): 1055.27 13.662 7.00 24.38
 =====
 ID = 3 (8260): 6715.79 41.240 10.75 25.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8258) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0408): 231.62 3.942 6.75 25.55
 + ID2= 2 (8260): 6715.79 41.240 10.75 25.01
 =====
 ID = 3 (8258): 6947.41 42.741 10.00 25.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8256) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0410): 572.01 3.821 8.00 19.33
 + ID2= 2 (8258): 6947.41 42.741 10.00 25.03
 =====
 ID = 3 (8256): 7519.42 46.169 10.00 24.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5507) |
 IN= 2--> OUT= 1 |
 DT= 15.0 min |

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	90.6140	67.8416
39.6440	12.3348	*****	160.3529
48.1390	18.5023	*****	160.3629
67.9600	37.0045	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8256)	7519.421	46.169	10.00
OUTFLOW : ID= 1 (5507)	7519.421	43.949	12.25

PEAK FLOW REDUCTION [Qout/qin](%)= 95.19
 TIME SHIFT OF PEAK FLOW (min)=135.00
 MAXIMUM STORAGE USED (ha.m.)= 15.4685

CALIB |

NASHYD (0420) | Area (ha)= 175.82 Curve Number (CN)= 53.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.81

Unit Hyd Qpeak (cms)= 3.692

PEAK FLOW (cms)= 2.139 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 22.269
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.252

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHYD (0418) | Area (ha)= 182.79 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.05

Unit Hyd Qpeak (cms)= 2.966

PEAK FLOW (cms)= 2.512 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 30.508
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.345

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHYD (0416) | Area (ha)= 439.30 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.29

Unit Hyd Qpeak (cms)= 5.832

PEAK FLOW (cms)= 5.193 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 30.583
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.346

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHYD (0412) | Area (ha)= 238.70 Curve Number (CN)= 54.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.80

Unit Hyd Qpeak (cms)= 5.088

PEAK FLOW (cms)= 3.028 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 22.921
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.259

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 STANHYD (4141) | Area (ha)= 43.70
 ID= 1 DT=15.0 min | Total Imp(%)= 36.00 Dir. Conn.(%)= 17.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	15.73	27.97
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	539.75	40.00
Manning's n	0.013	0.250

Max. Eff. Inten. (mm/hr)	116.82	72.91
over (min)	15.00	30.00
Storage Coeff. (min)	8.13 (ii)	20.27 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.10	0.05

	TOTALS
PEAK FLOW (cms)	2.15 2.85 3.979 (iii)
TIME TO PEAK (hrs)	6.00 6.25 6.00
RUNOFF VOLUME (mm)	86.50 36.78 45.23
TOTAL RAINFALL (mm)	88.50 88.50 88.50
RUNOFF COEFFICIENT	0.98 0.42 0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

CN* = 64.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5414)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	3.4720	1.9177
	0.3820	0.3152	3.8720	1.9277
	1.8280	0.4690	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (4141)	43.700	3.979	6.00	45.23
OUTFLOW: ID= 1 (5414)	43.700	2.035	6.50	45.23

PEAK FLOW REDUCTION [Qout/Qin](%)= 51.13
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 0.6598

CALIB			
STANDHYD (4142)			
ID= 1 DT=15.0 min			
	Area	(ha)	Dir. Conn.(%)
		144.30	10.00
	Total Imp(%)	21.00	

	IMPERVIOUS	PERVIOUS (i)
	(mm)	(mm)
Surface Area	30.30	114.00
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	980.82	40.00
Mannings n	0.013	0.250

	Max. Eff. Inten. (mm/hr)	116.82	58.69
	cover (min)	15.00	30.00
	Storage Coeff. (min)	11.63 (ii)	24.87 (ii)
	Unit Hyd. Tpeak (min)	15.00	30.00
	Unit Hyd. peak (cms)	0.08	0.04
			TOTALS
PEAK FLOW	(cms)	3.76	8.48
TIME TO PEAK	(hrs)	6.00	6.25
RUNOFF VOLUME	(mm)	86.50	39.04
TOTAL RAINFALL	(mm)	88.50	88.50
RUNOFF COEFFICIENT		0.98	0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 64.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8254)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (4142):	144.30	9.979	6.25	39.04
+ ID2= 2 (5414):	43.70	2.035	6.50	45.23
ID = 3 (8254):	188.00	11.926	6.25	40.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8252)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1 = 1 (0412):	238.70	3.028	7.00	22.92
+ ID2 = 2 (8254):	188.00	11.926	6.25	40.48
ID = 3 (8252):	426.70	14.425	6.25	30.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (4161)			
IN= 2--> OUT= 1			
Routing time step (min)= 15.00			
<----- DATA FOR SECTION (4161.0) ----->			
Distance	Elevation	Manning	
0.00	270.07	0.0340	
20.67	267.91	0.0340	
62.01	264.33	0.0340	

113.69	259.75	0.0340
165.37	253.30	0.0340
227.38	246.29	0.0340
232.55	246.03	0.0340
237.72	246.16	0.0340
241.39	246.02	0.0340 / 0.0300 Main Channel
241.64	245.75	0.0300 Main Channel
242.89	245.75	0.0300 Main Channel
244.14	245.75	0.0300 Main Channel
244.39	246.02	0.0300 / 0.0340 Main Channel
248.06	246.20	0.0340
253.22	246.28	0.0340
258.39	246.43	0.0340
346.25	252.57	0.0340
413.43	257.77	0.0340
465.11	261.78	0.0340
511.62	270.00	0.0340

TRAVEL TIME TABLE						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.27	246.02	.295E+04	0.6	0.75	88.26	
1.53	247.28	.168E+06	69.7	1.65	40.13	
2.79	248.54	.450E+06	299.5	2.42	27.28	
4.06	249.81	.962E+06	740.1	3.05	21.67	
5.32	251.07	.158E+07	1440.0	3.61	18.33	
6.58	252.33	.235E+07	2443.8	4.12	16.06	
7.84	253.59	.327E+07	3811.2	4.63	14.30	
9.10	254.85	.432E+07	5565.1	5.11	12.94	
10.37	256.12	.550E+07	7721.3	5.57	11.88	
11.63	257.38	.682E+07	10309.9	6.00	11.02	
12.89	258.64	.827E+07	13369.6	6.42	10.31	
14.15	259.90	.985E+07	16884.5	6.81	9.72	
15.42	261.17	.116E+08	20818.4	7.14	9.26	
16.68	262.43	.134E+08	25510.4	7.53	8.78	
17.94	263.69	.154E+08	30958.0	7.97	8.30	
19.20	264.95	.175E+08	36988.3	8.38	7.89	
20.46	266.21	.197E+08	43617.1	8.78	7.53	
21.73	267.48	.220E+08	50865.1	9.17	7.22	
22.99	268.74	.244E+08	58854.0	9.56	6.92	

<---- hydrograph ---->						
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8252)	426.70	14.42	6.25	30.66	0.52	0.84
OUTFLOW: ID= 1 (6416)	426.70	6.92	7.25	30.66	0.39	0.79

ADD HYD (8250)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0416):	439.30	5.193	8.00	30.58
+ ID2= 2 (6416):	426.70	6.920	7.25	30.66
ID = 3 (8250):	866.00	11.994	7.50	30.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8248)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1 = 1 (0418):	182.79	2.512	7.50	30.51
+ ID2= 2 (8250):	866.00	11.994	7.50	30.62
ID = 3 (8248):	1048.79	14.506	7.50	30.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8246)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1 = 1 (0420):	175.82	2.139	7.00	22.27
+ ID2= 2 (8248):	1048.79	14.506	7.50	30.60
ID = 3 (8246):	1224.61	16.598	7.25	29.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8244)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1 = 1 (5507):	7519.42	43.949	12.25	24.59
+ ID2= 2 (8246):	1224.61	16.598	7.25	29.40

ID = 3 (8244): 8744.03 53.161 10.00 25.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ADD HYD (8242) |
1 + 2 = 3
AREA
(ha)
ID1= 1 (0422):
+ ID2= 2 (8244):

ID = 3 (8242):

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| RESERVOIR (5508) |
| IN= 2--> OUT= 1 |
DT= 15.0 min
OUTFLOW
(cms)
0.0000
76.4550

AREA	QPEAK	TPEAK	R.V.	
(ha)	(cms)	(hrs)	(mm)	
INFLOW : ID= 2 (8242)	9524.231	58.066	10.00	25.09
OUTFLOW : ID= 1 (5508)	9524.231	56.167	11.75	25.09

PEAK FLOW REDUCTION [Qout/Qin](%)= 96.73
TIME SHIFT OF PEAK FLOW (min)=105.00
MAXIMUM STORAGE USED (ha.m.)= 22.6576

| CALIB |
| NASHYD (0336) |
ID= 1 DT=15.0 min
Area (ha)=2785.00
Ia (mm)= 5.00
U.H. Tp(hrs)= 15.39

Unit Hyd Qpeak (cms)= 3.090

PEAK FLOW (cms)= 4.639 (i)
TIME TO PEAK (hrs)= 22.250
RUNOFF VOLUME (mm)= 38.196
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.432

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| STANDHYD (3382) |
ID= 1 DT=15.0 min
Area (ha)= 525.90
Total Imp(%)= 37.00

Surface Area (ha)= 194.58
Dep. Storage (mm)= 2.00
Average Slope (%)= 0.50
Length (m)= 1874.43
Mannings n = 0.013

Max.Eff.Inten.(mm/hr)= 116.82
over (min)= 15.00
Storage Coeff. (min)= 17.14 (ii)
Unit Hyd. Tpeak (min)= 15.00
Unit Hyd. peak (cms)= 0.07

PEAK FLOW (cms)= 19.91
TIME TO PEAK (hrs)= 6.00
RUNOFF VOLUME (mm)= 86.50
TOTAL RAINFALL (mm)= 88.50
RUNOFF COEFFICIENT = 0.98

TOTALS
41.342 (iii)
6.25
47.07
88.50
0.53

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 66.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| STANDHYD (3381) |
ID= 1 DT=15.0 min
Area (ha)= 73.00
Total Imp(%)= 49.00

Surface Area (ha)= 35.77
Dep. Storage (mm)= 2.00
Average Slope (%)= 0.50
Length (m)= 697.61
Mannings n = 0.013

Max.Eff.Inten.(mm/hr)= 116.82
over (min)= 15.00
Storage Coeff. (min)= 9.48 (ii)
Unit Hyd. Tpeak (min)= 15.00
Unit Hyd. peak (cms)= 0.09

PEAK FLOW (cms)= 4.67
TIME TO PEAK (hrs)= 6.00
RUNOFF VOLUME (mm)= 86.50
TOTAL RAINFALL (mm)= 88.50
RUNOFF COEFFICIENT = 0.98

PERVIOUS (i)
37.23
5.00
0.50
40.00
0.250
97.85
30.00
20.27 (ii)
30.00
0.05
TOTALS
7.996 (iii)
6.00
52.41
88.50
0.59

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 66.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| RESERVOIR (5338) |
| IN= 2--> OUT= 1 |
DT= 15.0 min
OUTFLOW
(cms)
0.0000
0.3230
0.9360
1.7590
2.4110

Area (ha)= 73.00
QPEAK (cms)= 7.996
TPEAK (hrs)= 6.00
R.V. (mm)= 52.41

INFLOW : ID= 2 (3381)
OUTFLOW : ID= 1 (5338)

PEAK FLOW REDUCTION [Qout/Qin](%)= 55.24
TIME SHIFT OF PEAK FLOW (min)= 30.00
MAXIMUM STORAGE USED (ha.m.)= 1.3680

| ADD HYD (8310) |
1 + 2 = 3
AREA
(ha)
ID1= 1 (3382):
+ ID2= 2 (5338):

ID = 3 (8310):

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| CALIB |
| NASHYD (3342) |
ID= 1 DT=15.0 min
Area (ha)= 587.10
Ia (mm)= 5.00
U.H. Tp(hrs)= 8.19

Unit Hyd Qpeak (cms)= 1.224

PEAK FLOW (cms)= 1.173 (i)
TIME TO PEAK (hrs)= 15.250
RUNOFF VOLUME (mm)= 24.620
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.278

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| STANDHYD (3341) |
ID= 1 DT=15.0 min
Area (ha)= 33.00
Total Imp(%)= 51.00

Surface Area (ha)= 16.83
Dep. Storage (mm)= 2.00
Average Slope (%)= 0.50
Length (m)= 469.04
Mannings n = 0.013

Max.Eff.Inten.(mm/hr)= 116.82
over (min)= 15.00
Storage Coeff. (min)= 7.47 (ii)
Unit Hyd. Tpeak (min)= 15.00

PERVIOUS (i)
16.17
5.00
0.50
40.00
0.250
60.88
30.00
20.52 (ii)
30.00

Unit Hyd. peak (cms)= 0.10 0.05 *TOTALS*
 PEAK FLOW (cms)= 3.02 1.37 3.881 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 86.50 28.70 46.62
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.32 0.53

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 52.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5334)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.6540	0.4604
0.0390	0.1699	0.8330	0.5249
0.1860	0.2610	1.0130	0.5896
0.3750	0.3534	1.4130	0.5996
0.4340	0.3813	0.0000	0.0000

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (3341) 33.000 3.881 6.00 46.62
 OUTFLOW: ID= 1 (5334) 33.000 1.849 6.25 46.58

PEAK FLOW REDUCTION [Qout/Qin](%)= 47.65
 TIME SHIFT OF PEAK FLOW (min)= 15.00
 MAXIMUM STORAGE USED (ha.m.)= 0.6246

ADD HYD (8314)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (3342):	587.10	1.173	15.25	24.62
+ ID2= 2 (5334):	33.00	1.849	6.25	46.58
ID = 3 (8314):	620.10	2.136	6.25	25.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0306)
 NASHYD (0306) Area (ha)= 283.97 Curve Number (CN)= 52.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 6.44

Unit Hyd Qpeak (cms)= 0.753

PEAK FLOW (cms)= 0.637 (i)
 TIME TO PEAK (hrs)= 13.750
 RUNOFF VOLUME (mm)= 21.914
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.248

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0286)
 NASHYD (0286) Area (ha)= 260.51 Curve Number (CN)= 84.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.16

Unit Hyd Qpeak (cms)= 3.834

PEAK FLOW (cms)= 5.908 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 52.435
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.592

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0282)
 NASHYD (0282) Area (ha)= 449.38 Curve Number (CN)= 77.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.47

Unit Hyd Qpeak (cms)= 5.226

PEAK FLOW (cms)= 6.939 (i)
 TIME TO PEAK (hrs)= 8.000

RUNOFF VOLUME (mm)= 43.497
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.491

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0284)
 NASHYD (0284) Area (ha)= 78.93 Curve Number (CN)= 84.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.57

Unit Hyd Qpeak (cms)= 2.344

PEAK FLOW (cms)= 3.150 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 51.645
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.584

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8388)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0282):	449.38	6.939	8.00	43.50
+ ID2= 2 (0284):	78.93	3.150	6.50	51.65
ID = 3 (8388):	528.31	9.369	7.25	44.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (5286)
 IN= 2--> OUT= 1 Routing time step (min)= 15.00

DATA FOR SECTION (2861.0) ----->

Distance	Elevation	Manning
0.00	233.00	0.0450
20.58	228.51	0.0450
41.17	227.74	0.0450
61.46	227.41	0.0450
81.77	225.96	0.0450
102.09	223.94	0.0450
122.42	220.84	0.0450
142.77	220.66	0.0450
163.11	220.22	0.0450
183.46	220.01	0.0450 / 0.0350
203.81	219.70	0.0350
224.15	219.72	0.0350
244.50	220.06	0.0350
264.85	220.23	0.0350
285.20	221.64	0.0350
305.55	224.98	0.0450
325.90	432.25	0.0450
346.25	457.98	0.0450
366.60	483.71	0.0450
386.95	509.44	0.0450

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.31	220.01	.382E+04	0.8	0.57	82.89
1.06	220.76	.525E+05	12.1	0.66	72.51
1.81	221.51	.240E+06	88.6	1.05	45.21
2.56	222.26	.510E+06	261.7	1.46	32.47
3.31	223.01	.843E+06	533.8	1.81	26.34
4.06	223.76	.124E+07	933.6	2.10	22.64
4.81	224.51	.171E+07	1399.0	2.34	20.33
5.56	225.26	.225E+07	2051.0	2.60	18.27
6.31	226.01	.285E+07	2907.0	2.91	16.35
7.06	226.76	.352E+07	3902.5	3.16	15.04
7.82	227.52	.426E+07	5069.9	3.39	14.01
8.57	228.27	.507E+07	6433.0	3.62	13.13
9.32	229.02	.593E+07	8069.2	3.89	12.24
10.07	229.77	.681E+07	9926.7	4.16	11.44
10.82	230.52	.772E+07	11962.8	4.42	10.75
11.57	231.27	.864E+07	14167.1	4.68	10.17
12.32	232.02	.959E+07	16537.1	4.92	9.66
13.07	232.77	.105E+08	19070.7	5.16	9.22
13.82	233.52	.115E+08	21790.4	5.39	8.82

hydrograph ----> <-pipe / channel-->

INFLOW : ID= 2 (8388)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW: ID= 1 (6286)	528.31	9.37	7.25	44.71	0.88	0.63
	528.31	7.94	9.00	44.71	0.78	0.62

```

-----
| ADD HYD (8386) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
|-----|
| ID1= 1 (0286): 260.51 5.908 7.50 52.44 |
| + ID2= 2 (6286): 528.31 7.939 9.00 44.71 |
|-----|
| ID = 3 (8386): 788.82 13.331 8.25 47.26 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0302) | Area (ha)= 473.90 Curve Number (CN)= 58.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 1.66 |
-----

```

Unit Hyd Qpeak (cms)= 4.874

```

PEAK FLOW (cms)= 3.864 (i)
TIME TO PEAK (hrs)= 8.500
RUNOFF VOLUME (mm)= 25.946
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.293

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0300) | Area (ha)= 258.93 Curve Number (CN)= 52.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 4.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 1.03 |
-----

```

Unit Hyd Qpeak (cms)= 4.292

```

PEAK FLOW (cms)= 2.593 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 22.167
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.250

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0298) | Area (ha)= 330.51 Curve Number (CN)= 45.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 1.26 |
-----

```

Unit Hyd Qpeak (cms)= 4.461

```

PEAK FLOW (cms)= 2.236 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 17.571
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.199

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8395) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
|-----|
| ID1= 1 (0298): 330.51 2.236 8.00 17.57 |
| + ID2= 2 (0300): 258.93 2.593 7.50 22.17 |
|-----|
| ID = 3 (8395): 589.44 4.798 7.75 19.59 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0288) | Area (ha)= 340.83 Curve Number (CN)= 78.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 2.21 |
-----

```

Unit Hyd Qpeak (cms)= 2.629

```

PEAK FLOW (cms)= 3.883 (i)
TIME TO PEAK (hrs)= 9.250
RUNOFF VOLUME (mm)= 44.801
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.506

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0290) | Area (ha)= 269.18 Curve Number (CN)= 78.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 1.07 |
-----

```

Unit Hyd Qpeak (cms)= 4.279

```

PEAK FLOW (cms)= 5.447 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 44.529
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.503

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8397) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
|-----|
| ID1= 1 (0288): 340.83 3.883 9.25 44.80 |
| + ID2= 2 (0290): 269.18 5.447 7.25 44.53 |
|-----|
| ID = 3 (8397): 610.01 9.010 8.00 44.68 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0296) | Area (ha)= 293.65 Curve Number (CN)= 76.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 1.13 |
-----

```

Unit Hyd Qpeak (cms)= 4.437

```

PEAK FLOW (cms)= 5.388 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 42.226
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.477

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0292) | Area (ha)= 738.49 Curve Number (CN)= 68.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 1.82 |
-----

```

Unit Hyd Qpeak (cms)= 8.289

```

PEAK FLOW (cms)= 8.591 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 34.153
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.386

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0294) | Area (ha)= 274.15 Curve Number (CN)= 57.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 |
|-----|
| U.H. Tp(hrs)= 0.87 |
-----

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Unit Hyd Qpeak (cms)= 5.367

```

PEAK FLOW (cms)= 3.559 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 25.026
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.283

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8398) |
| 1 + 2 = 3 |
-----
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
|-----|
| ID1= 1 (0292): 738.49 8.591 8.00 34.15 |
| + ID2= 2 (0294): 274.15 3.559 7.00 25.03 |
|-----|
| ID = 3 (8398): 1012.64 11.892 8.00 31.68 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ROUTE CN (6296) |

| IN= 2--> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (2961.0) -----				
Distance	Elevation	Manning		
0.00	243.98	0.0400		
42.59	243.18	0.0400		
85.17	241.81	0.0400		
120.66	240.50	0.0400		
156.15	239.56	0.0400		
198.74	236.15	0.0400		
237.78	234.01	0.0400		
241.33	233.82	0.0400		
248.77	233.12	0.0400 / 0.0400	Main Channel	
249.87	232.32	0.0400	Main Channel	
250.37	231.80	0.0400	Main Channel	
250.87	232.23	0.0400	Main Channel	
251.87	233.10	0.0400 / 0.0400	Main Channel	
255.37	233.22	0.0400		
259.07	233.87	0.0400		
262.62	234.12	0.0400		
266.17	234.23	0.0400		
283.91	234.73	0.0400		
337.15	241.75	0.0400		
351.34	244.00	0.0400		

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.43	232.23	.510E+03	0.1	0.53	81.07
0.87	232.67	.213E+04	0.7	0.83	51.49
1.30	233.10	.501E+04	2.2	1.10	38.75
1.98	233.78	.242E+05	13.6	1.34	31.93
2.66	234.46	.738E+05	42.1	1.46	29.21
3.34	235.14	.177E+06	127.9	1.84	23.13
4.02	235.82	.314E+06	281.4	2.29	18.61
4.70	236.50	.481E+06	509.4	2.71	15.73
5.38	237.18	.672E+06	815.6	3.11	13.73
6.06	237.86	.886E+06	1200.7	3.46	12.30
6.74	238.54	.112E+07	1669.4	3.80	11.23
7.42	239.22	.139E+07	2226.9	4.11	10.38
8.10	239.90	.168E+07	2808.4	4.28	9.95
8.78	240.58	.202E+07	3470.4	4.40	9.68
9.46	241.26	.240E+07	4331.7	4.61	9.24
10.14	241.94	.283E+07	5335.2	4.83	8.84
10.82	242.62	.330E+07	6482.6	5.03	8.48
11.50	243.30	.381E+07	7764.1	5.21	8.18
12.18	243.98	.439E+07	9101.2	5.31	8.04

		----- hydrograph -----				<-pipe / channel-->			
INFLW : ID= 2 (8398)	1012.64	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)		
OUTFLOW : ID= 1 (6296)	1012.64	11.62	8.25	31.68	1.91	1.31			

----- ADD HYD (8396) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0296):	293.65	5.388	7.50	42.23
+ ID2= 2 (6296):	1012.64	11.616	8.25	31.68
ID = 3 (8396):	1306.29	16.780	8.00	34.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ADD HYD (8394) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8396):	1306.29	16.780	8.00	34.05
+ ID2= 2 (8397):	610.01	9.010	8.00	44.68
ID = 3 (8394):	1916.30	25.790	8.00	37.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ADD HYD (8392) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8394):	1916.30	25.790	8.00	37.44
+ ID2= 2 (8395):	589.44	4.798	7.75	19.59
ID = 3 (8392):	2505.74	30.563	8.00	33.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ROUTE CHN (6302) -----

| IN= 2--> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (3021.0) -----				
Distance	Elevation	Manning		
0.00	228.10	0.0400		
18.47	227.12	0.0400		
36.95	226.12	0.0400		
46.18	225.84	0.0400		
55.42	225.58	0.0400		
272.47	222.88	0.0400		
277.09	222.76	0.0400		
281.71	222.58	0.0400		
288.54	222.18	0.0400 / 0.0300	Main Channel	
288.64	221.00	0.0300	Main Channel	
290.94	221.00	0.0300	Main Channel	
291.04	221.00	0.0300	Main Channel	
291.54	222.75	0.0300 / 0.0400	Main Channel	
300.18	222.83	0.0400		
304.80	223.04	0.0400		
309.42	223.25	0.0400		
318.65	223.69	0.0400		
360.22	225.57	0.0400		
397.16	227.60	0.0400		
457.20	228.35	0.0400		

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.29	221.29	.228E+04	0.2	0.32	165.63
0.59	221.59	.463E+04	0.7	0.45	116.15
0.88	221.88	.713E+04	1.2	0.54	96.87
1.18	222.18	.971E+04	1.9	0.61	86.20
1.57	222.57	.175E+05	3.3	0.59	88.86
1.97	222.97	.427E+05	6.4	0.47	111.38
2.36	223.36	.115E+06	16.1	0.44	119.14
2.76	223.76	.237E+06	37.1	0.49	106.49
3.15	224.15	.409E+06	73.9	0.56	93.40
3.55	224.55	.631E+06	127.0	0.63	82.76
3.94	224.94	.903E+06	202.4	0.70	74.37
4.34	225.34	.123E+07	301.8	0.77	67.68
4.73	225.73	.160E+07	433.5	0.85	61.37
5.13	226.13	.200E+07	602.2	0.95	55.29
5.52	226.52	.242E+07	804.7	1.05	50.13
5.92	226.92	.286E+07	1034.2	1.14	46.11
6.31	227.31	.332E+07	1290.5	1.22	42.88
6.71	227.71	.380E+07	1560.9	1.29	40.55
7.10	228.10	.432E+07	1835.6	1.34	39.19

		----- hydrograph -----				<-pipe / channel-->			
INFLW : ID= 2 (8392)	2505.74	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)		
OUTFLOW : ID= 1 (6302)	2505.74	24.15	10.00	33.24	2.64	0.47			

----- ADD HYD (8390) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0302):	473.90	3.864	8.50	25.95
+ ID2= 2 (6302):	2505.74	24.155	10.00	33.24
ID = 3 (8390):	2979.64	27.780	10.00	32.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ADD HYD (8348) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8386):	786.82	13.331	8.25	47.26
+ ID2= 2 (8390):	2979.64	27.780	10.00	32.08
ID = 3 (8348):	3768.46	40.043	9.50	35.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- CALIB -----

NASHYD (0304)	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
ID= 1 DT=15.0 min	292.37	5.00	1.50
	U.H. Tp (hrs)	2.78	

Unit Hyd Qpeak (cms) = 1.793
 PEAK FLOW (cms) = 1.844 (1)
 TIME TO PEAK (hrs) = 10.250
 RUNOFF VOLUME (mm) = 29.899

TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.338

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
| NASHYD (0280) | Area (ha)= 299.86 Curve Number (CN)= 82.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.85

Unit Hyd Qpeak (cms) = 6.009

PEAK FLOW (cms) = 8.228 (1)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 49.420
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.558

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
| NASHYD (0278) | Area (ha)= 485.49 Curve Number (CN)= 82.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.52

Unit Hyd Qpeak (cms) = 5.453

PEAK FLOW (cms) = 8.411 (1)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 49.794
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.563

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
| NASHYD (0276) | Area (ha)= 90.89 Curve Number (CN)= 79.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.67

Unit Hyd Qpeak (cms) = 2.302

PEAK FLOW (cms) = 2.744 (1)
TIME TO PEAK (hrs) = 6.750
RUNOFF VOLUME (mm) = 45.323
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.512

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
| NASHYD (0274) | Area (ha)= 392.49 Curve Number (CN)= 75.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.08

Unit Hyd Qpeak (cms) = 6.182

PEAK FLOW (cms) = 7.225 (1)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 41.085
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.464

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8360) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
| ID1= 1 (0274): 392.49 7.225 7.25 41.09
+ ID2= 2 (0276): 90.89 2.744 6.75 45.32
=====

ID = 3 (8360): 483.38 9.821 7.00 41.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8358) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
| ID1= 1 (0278): 485.49 8.411 8.00 49.79
+ ID2= 2 (8360): 483.38 9.821 7.00 41.88
=====

ID = 3 (8358): 968.87 17.878 7.50 45.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6280) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00

Distance	Elevation	Manning
0.00	241.14	0.0500
13.32	240.80	0.0500
39.95	240.07	0.0500
96.54	236.21	0.0500
113.19	234.15	0.0500
123.18	232.35	0.0500
143.15	225.80	0.0500
149.81	225.62	0.0500
153.14	225.40	0.0500
157.30	224.76	0.0500 / 0.0300
159.80	224.26	0.0300
162.30	224.85	0.0300
162.55	224.97	0.0300 / 0.0500
167.80	225.05	0.0500
186.43	229.14	0.0500
213.06	234.75	0.0500
236.37	237.09	0.0500
266.33	237.31	0.0500
292.96	237.83	0.0500
329.58	241.50	0.0500

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	224.76	.263E+04	0.8	0.65	58.31
1.36	225.62	.278E+05	13.4	1.10	34.54
2.22	226.48	.855E+05	52.6	1.40	27.09
3.09	227.35	.157E+06	120.4	1.74	21.78
3.95	228.21	.242E+06	216.3	2.04	18.64
4.81	229.07	.340E+06	341.8	2.29	16.56
5.67	229.93	.450E+06	498.3	2.52	15.06
6.53	230.79	.574E+06	688.3	2.73	13.90
7.40	231.66	.711E+06	913.8	2.93	12.97
8.26	232.52	.861E+06	1173.4	3.11	12.23
9.12	233.38	.103E+07	1461.3	3.24	11.72
9.98	234.24	.121E+07	1793.3	3.37	11.26
10.85	235.11	.142E+07	2141.0	3.45	11.02
11.71	235.97	.165E+07	2530.4	3.50	10.86
12.57	236.83	.192E+07	2954.0	3.51	10.81
13.43	237.69	.226E+07	3165.3	3.18	11.93
14.29	238.55	.271E+07	3869.4	3.26	11.67
15.16	239.42	.320E+07	4740.3	3.38	11.24
16.02	240.28	.373E+07	5688.4	3.48	10.92

hydrograph <---> <--- pipe / channel --->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
INFLOW : ID= 2 (8358) 968.87 17.88 7.50 45.85 1.46 1.13
OUTFLOW: ID= 1 (6280) 968.87 17.31 8.25 45.85 1.45 1.12

ADD HYD (8354) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
+ ID1= 1 (0280): 299.86 8.228 7.00 49.42
+ ID2= 2 (6280): 968.87 17.313 8.25 45.85
=====

ID = 3 (8354): 1268.73 24.661 7.75 46.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB |
| NASHYD (0272) | Area (ha)= 157.38 Curve Number (CN)= 75.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.09

Unit Hyd Qpeak (cms) = 2.456

PEAK FLOW (cms) = 2.876 (1)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 41.090
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.464

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |

NASHYD (0270) Area (ha)= 243.61 Curve Number (CN)= 81.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 0.87

Unit Hyd Qpeak (cms)= 3.429
 PEAK FLOW (cms)= 4.722 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 48.074
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.543

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0268) Area (ha)= 215.76 Curve Number (CN)= 75.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 0.69

Unit Hyd Qpeak (cms)= 3.807
 PEAK FLOW (cms)= 4.211 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 40.714
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.460

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0264) Area (ha)= 353.96 Curve Number (CN)= 69.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.30

Unit Hyd Qpeak (cms)= 3.313
 PEAK FLOW (cms)= 3.548 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 35.001
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.395

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0266) Area (ha)= 508.09 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.63

Unit Hyd Qpeak (cms)= 3.786
 PEAK FLOW (cms)= 3.687 (i)
 TIME TO PEAK (hrs)= 9.000
 RUNOFF VOLUME (mm)= 30.617
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.346

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8380)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0264): 353.96 3.548 8.00 35.00
 + ID2= 2 (0266): 508.09 3.687 9.00 30.62
 =====
 ID = 3 (8380): 862.05 7.203 8.50 32.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6270)
 IN= 2--> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2681.0) ----->

Distance	Elevation	Manning
0.00	274.03	0.0500
10.55	272.73	0.0500
21.10	271.28	0.0500
50.12	266.83	0.0500
87.04	260.36	0.0500
92.32	260.06	0.0500
94.85	259.83	0.0500
101.00	256.87	0.0500
103.00	256.58	0.0500 / 0.0350 Main Channel
104.00	256.32	0.0350 Main Channel

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
106.00	256.05	0.0350			
108.00	256.25	0.0350			
110.78	256.65	0.0350 / 0.0500			
113.42	256.81	0.0500			
116.06	257.00	0.0500			
155.62	260.34	0.0500			
187.27	263.35	0.0500			
211.01	267.22	0.0500			
224.20	269.60	0.0500			
261.13	272.68	0.0500			

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.53	256.58	.697E+04	2.1	0.97	55.64
1.38	257.43	.477E+05	25.9	1.76	30.66
2.22	258.27	.133E+06	84.3	2.22	24.26
3.07	259.12	.231E+06	185.4	2.60	20.72
3.92	259.97	.371E+06	335.8	2.92	18.42
4.77	260.82	.560E+06	547.6	3.15	17.06
5.61	261.66	.790E+06	845.6	3.46	15.57
6.46	262.51	.106E+07	1225.0	3.75	14.37
7.31	263.36	.136E+07	1693.3	4.02	13.37
8.16	264.21	.169E+07	2283.2	4.35	12.36
9.00	265.05	.206E+07	2959.1	4.66	11.54
9.85	265.90	.245E+07	3754.4	4.96	10.86
10.70	266.75	.286E+07	4643.0	5.24	10.28
11.55	267.60	.331E+07	5636.1	5.50	9.78
12.39	268.44	.378E+07	6746.2	5.76	9.34
13.24	269.29	.428E+07	7972.2	6.01	8.96
14.09	270.14	.482E+07	9218.9	6.18	8.71
14.94	270.99	.539E+07	10550.5	6.32	8.52
15.78	271.83	.601E+07	12024.7	6.46	8.33

<---- hydrograph ----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8380)	862.05	7.20	8.50	32.42	0.71
OUTFLOW: ID= 1 (6268)	862.05	7.01	9.75	32.42	0.70

ADD HYD (8382)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0268): 215.76 4.211 7.00 40.71
 + ID2= 2 (6268): 862.05 7.011 9.75 32.42
 =====
 ID = 3 (8382): 1077.81 10.204 8.50 34.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6270)
 IN= 2--> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2701.0) ----->

Distance	Elevation	Manning
0.00	245.98	0.0500
8.14	245.66	0.0500
16.28	245.16	0.0500
20.35	244.84	0.0500
38.66	242.98	0.0500
48.83	240.65	0.0500
63.07	235.91	0.0500
65.11	235.18	0.0500
68.43	234.34	0.0500 / 0.0300 Main Channel
68.68	233.95	0.0300 Main Channel
69.18	233.89	0.0300 Main Channel
69.68	233.95	0.0300 Main Channel
71.21	234.48	0.0300 / 0.0500 Main Channel
81.38	236.44	0.0500
95.63	236.66	0.0500
120.04	237.00	0.0500
148.53	241.77	0.0500
158.70	242.34	0.0500
187.18	244.03	0.0500
201.42	244.36	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.45	234.34	.217E+04	0.7	0.99	52.83
0.98	234.87	.966E+04	4.6	1.49	35.20
1.50	235.39	.250E+05	13.7	1.73	30.41
2.03	235.92	.475E+05	29.5	1.95	26.87
2.56	236.45	.772E+05	52.5	2.14	24.50
3.09	236.98	.142E+06	82.4	1.83	28.73
3.61	237.50	.246E+06	158.1	2.03	25.91
4.14	238.03	.357E+06	262.0	2.31	22.73
4.67	238.56	.477E+06	392.1	2.59	20.27
5.20	239.09	.604E+06	547.8	2.86	18.38

5.72	239.61	.739E+06	729.2	3.11	16.90
6.25	240.14	.882E+06	936.5	3.34	15.70
6.78	240.67	.103E+07	1169.7	3.57	14.72
7.31	241.20	.119E+07	1425.9	3.77	13.94
7.83	241.72	.136E+07	1710.4	3.96	13.26
8.36	242.25	.154E+07	1967.9	4.02	13.07
8.89	242.78	.174E+07	2264.9	4.09	12.83
9.42	243.31	.196E+07	2589.3	4.15	12.65
9.94	243.83	.221E+07	2953.5	4.22	12.46

<---- hydrograph ----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8382) 1077.81	10.20	8.50	34.08	1.30	1.63
OUTFLOW: ID= 1 (6270) 1077.81	10.06	9.25	34.08	1.29	1.62

ADD HYD (8384)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID= 1 (0270):	243.61	4.722	7.25	48.07
+ ID= 2 (6270):	1077.81	10.057	9.25	34.08
=====	=====	=====	=====	=====
ID = 3 (8384):	1321.42	14.172	8.75	36.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 82.0
NASHYD (0262)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.30	
ID= 1 DT=15.0 min	U.H. Tp(hrs)=	1.01		

Unit Hyd Qpeak (cms)= 4.094

PEAK FLOW	(cms)= 5.995 (1)
TIME TO PEAK	(hrs)= 7.500
RUNOFF VOLUME	(mm)= 49.519
TOTAL RAINFALL	(mm)= 88.500
RUNOFF COEFFICIENT	= 0.560

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 82.0
NASHYD (0260)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.30	
ID= 1 DT=15.0 min	U.H. Tp(hrs)=	1.33		

Unit Hyd Qpeak (cms)= 4.360

PEAK FLOW	(cms)= 6.777 (1)
TIME TO PEAK	(hrs)= 8.000
RUNOFF VOLUME	(mm)= 49.681
TOTAL RAINFALL	(mm)= 88.500
RUNOFF COEFFICIENT	= 0.561

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 79.0
NASHYD (0258)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.30	
ID= 1 DT=15.0 min	U.H. Tp(hrs)=	1.18		

Unit Hyd Qpeak (cms)= 1.881

PEAK FLOW	(cms)= 2.620 (1)
TIME TO PEAK	(hrs)= 8.000
RUNOFF VOLUME	(mm)= 45.751
TOTAL RAINFALL	(mm)= 88.500
RUNOFF COEFFICIENT	= 0.517

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 73.0
NASHYD (0252)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.30	
ID= 1 DT=15.0 min	U.H. Tp(hrs)=	1.04		

Unit Hyd Qpeak (cms)= 3.761

PEAK FLOW	(cms)= 4.280 (1)
TIME TO PEAK	(hrs)= 8.000
RUNOFF VOLUME	(mm)= 38.873
TOTAL RAINFALL	(mm)= 88.500
RUNOFF COEFFICIENT	= 0.439

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 67.0
NASHYD (0256)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.30	
ID= 1 DT=15.0 min	U.H. Tp(hrs)=	1.04		

Unit Hyd Qpeak (cms)= 1.707

PEAK FLOW	(cms)= 1.643 (1)
TIME TO PEAK	(hrs)= 8.000
RUNOFF VOLUME	(mm)= 33.068
TOTAL RAINFALL	(mm)= 88.500
RUNOFF COEFFICIENT	= 0.374

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 55.0
NASHYD (0254)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.30	
ID= 1 DT=15.0 min	U.H. Tp(hrs)=	1.22		

Unit Hyd Qpeak (cms)= 4.028

PEAK FLOW	(cms)= 2.850 (1)
TIME TO PEAK	(hrs)= 8.000
RUNOFF VOLUME	(mm)= 33.726
TOTAL RAINFALL	(mm)= 88.500
RUNOFF COEFFICIENT	= 0.268

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6256)	Routing time step (min)= 15.00
------------------	--------------------------------

DATA FOR SECTION (2561.0) ----->			
Distance	Elevation	Manning	
0.00	276.07	0.0400	
11.68	273.71	0.0400	
23.36	271.35	0.0400	
29.19	270.30	0.0400	
35.03	269.44	0.0400	
55.47	267.30	0.0400	
78.82	266.24	0.0400	
90.50	265.63	0.0400	
102.18	265.40	0.0400	
105.10	264.95	0.0400 / 0.0350	Main Channel
108.02	264.39	0.0350	Main Channel
110.94	264.72	0.0350	Main Channel
113.86	265.19	0.0350 / 0.0400	Main Channel
116.78	265.49	0.0400	
143.05	268.24	0.0400	
172.25	270.53	0.0400	
207.28	271.95	0.0400	
233.55	273.72	0.0400	
256.91	274.98	0.0400	
289.02	275.97	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.56	264.95	.723E+04	1.9	0.90	62.51
1.14	265.53	.307E+05	13.5	1.49	37.92
1.72	266.11	.951E+05	48.6	1.73	32.62
2.30	266.69	.191E+06	117.8	2.09	27.03
2.88	267.27	.314E+06	225.2	2.43	23.24
3.46	267.85	.464E+06	375.5	2.74	20.59
4.04	268.43	.641E+06	573.2	3.03	18.62
4.62	269.01	.846E+06	821.9	3.29	17.15
5.20	269.59	.108E+07	1135.0	3.56	15.87
5.78	270.17	.134E+07	1525.1	3.86	14.64
6.36	270.75	.162E+07	1971.1	4.12	13.71
6.94	271.33	.194E+07	2474.0	4.33	13.04
7.52	271.91	.228E+07	3065.0	4.55	12.42
8.10	272.49	.266E+07	3773.4	4.80	11.75
8.68	273.07	.306E+07	4566.3	5.05	11.17
9.26	273.65	.348E+07	5443.7	5.30	10.66
9.84	274.23	.393E+07	6389.6	5.51	10.25
10.42	274.81	.440E+07	7426.2	5.72	9.88
11.00	275.39	.490E+07	8490.1	5.86	9.63

<---- hydrograph ----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (0254) 403.00	2.85	8.00	23.73	0.61	0.93
OUTFLOW: ID= 1 (6256) 403.00	2.68	9.75	23.73	0.60	0.93

```

-----
| ADD HYD (8370) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0256): 145.79 1.643 8.00 33.07 |
| + ID2= 2 (6256): 403.00 2.683 9.75 23.73 |
|-----|
| ID = 3 (8370): 548.79 4.183 9.00 26.21 |

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0250) | Area (ha)= 192.88 Curve Number (CN)= 70.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30 |
|-----|
| U.H. Tp(hrs)= 1.22 |
|
| Unit Hyd Qpeak (cms)= 1.930 |
|
| PEAK FLOW (cms)= 2.099 (1) |
| TIME TO PEAK (hrs)= 8.000 |
| RUNOFF VOLUME (mm)= 35.932 |
| TOTAL RAINFALL (mm)= 88.500 |
| RUNOFF COEFFICIENT = 0.406 |

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| CALIB |
| NASHYD (0246) | Area (ha)= 759.61 Curve Number (CN)= 55.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30 |
|-----|
| U.H. Tp(hrs)= 1.81 |
|
| Unit Hyd Qpeak (cms)= 5.121 |
|
| PEAK FLOW (cms)= 3.958 (1) |
| TIME TO PEAK (hrs)= 10.000 |
| RUNOFF VOLUME (mm)= 23.809 |
| TOTAL RAINFALL (mm)= 88.500 |
| RUNOFF COEFFICIENT = 0.269 |

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0248) | Area (ha)= 146.04 Curve Number (CN)= 64.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30 |
|-----|
| U.H. Tp(hrs)= 0.78 |
|
| Unit Hyd Qpeak (cms)= 2.271 |
|
| PEAK FLOW (cms)= 1.874 (1) |
| TIME TO PEAK (hrs)= 7.250 |
| RUNOFF VOLUME (mm)= 30.328 |
| TOTAL RAINFALL (mm)= 88.500 |
| RUNOFF COEFFICIENT = 0.343 |

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8364) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0246): 759.61 3.958 10.00 23.81 |
| + ID2= 2 (0248): 146.04 1.874 7.25 30.33 |
|-----|
| ID = 3 (8364): 905.65 5.601 8.25 24.86 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| ROUTE CHN (6250) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00 |

```

<----- DATA FOR SECTION (2501.0) ----->

Distance	Elevation	Manning
0.00	269.96	0.0500
8.56	268.55	0.0500
17.13	266.91	0.0500
21.41	266.13	0.0500
40.68	263.15	0.0500
62.09	260.75	0.0500
85.64	258.02	0.0500
88.20	257.69	0.0500
93.20	257.05	0.0500 /0.0350 Main Channel
93.45	256.88	0.0350 Main Channel
94.20	256.56	0.0350 Main Channel

Distance	Elevation	Manning
94.95	256.83	0.0350 Main Channel
95.20	257.08	0.0350 /0.0500 Main Channel
100.62	257.45	0.0500
115.61	258.57	0.0500
139.16	260.43	0.0500
152.01	261.95	0.0500
171.27	264.63	0.0500
188.40	267.90	0.0500
211.95	274.18	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.49	257.05	.205E+04	0.4	0.68	89.65
1.17	257.73	.248E+05	5.7	0.85	72.16
1.85	258.41	.841E+05	25.2	1.10	55.60
2.53	259.09	.180E+06	66.5	1.36	45.14
3.21	259.77	.312E+06	135.7	1.60	38.36
3.89	260.45	.481E+06	238.3	1.82	33.61
4.57	261.13	.681E+06	383.5	2.07	29.60
5.25	261.81	.911E+06	568.9	2.29	26.70
5.93	262.49	.117E+07	800.5	2.51	24.36
6.61	263.17	.146E+07	1079.0	2.72	22.49
7.28	263.84	.177E+07	1413.4	2.94	20.85
7.96	264.52	.210E+07	1797.0	3.14	19.50
8.64	265.20	.246E+07	2241.6	3.35	18.28
9.32	265.88	.283E+07	2739.9	3.55	17.25
10.00	266.56	.323E+07	3295.7	3.75	16.34
10.68	267.24	.364E+07	3908.9	3.94	15.54
11.36	267.92	.408E+07	4578.5	4.12	14.84
12.04	268.60	.453E+07	5320.1	4.32	14.18
12.72	269.28	.499E+07	6109.1	4.50	13.61

<---- hydrograph ----> <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (8364)	905.65	5.60	8.25	24.86	1.15	0.84
OUTFLOW: ID= 1 (6250)	905.65	5.25	10.25	24.86	1.11	0.83

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-----
| ADD HYD (8366) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0250): 192.88 2.099 8.00 35.93 |
| + ID2= 2 (6250): 905.65 5.247 10.25 24.86 |
|-----|
| ID = 3 (8366): 1098.53 7.167 9.75 26.80 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| ADD HYD (8368) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (8366): 1098.53 7.167 9.75 26.80 |
| + ID2= 2 (8370): 548.79 4.183 9.00 26.21 |
|-----|
| ID = 3 (8368): 1647.32 11.318 9.50 26.61 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8372) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0252): 319.99 4.280 8.00 38.87 |
| + ID2= 2 (8368): 1647.32 11.318 9.50 26.61 |
|-----|
| ID = 3 (8372): 1967.31 15.243 9.00 28.60 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| ROUTE CHN (6258) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00 |

```

<----- DATA FOR SECTION (2581.0) ----->

Distance	Elevation	Manning
0.00	252.88	0.0500
15.47	251.89	0.0500
46.41	248.45	0.0500
73.48	245.81	0.0500
88.95	243.75	0.0500
112.15	242.00	0.0500
135.35	240.23	0.0500
162.42	239.76	0.0500
170.97	239.52	0.0500 /0.0350 Main Channel

171.58	239.03	0.0350	Main Channel
174.02	239.03	0.0350	Main Channel
176.46	239.03	0.0350	Main Channel
177.07	239.52	0.0350 / 0.0500	Main Channel
185.63	239.67	0.0500	
208.83	239.87	0.0500	
239.77	240.14	0.0500	
274.57	244.93	0.0500	
336.45	249.51	0.0500	
363.52	249.77	0.0500	
382.85	251.78	0.0500	

TRAVEL TIME TABLE					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.49	239.52	.138E+05	2.5	0.92	91.32
1.14	240.17	.175E+06	25.6	0.73	114.22
1.78	240.81	.532E+06	123.5	1.17	71.85
2.43	241.46	.932E+06	285.3	1.54	54.48
3.07	242.10	1.38E+07	505.5	1.85	45.25
3.72	242.75	1.86E+07	786.8	2.12	39.42
4.36	243.39	2.39E+07	1127.3	2.37	35.33
5.01	244.04	2.96E+07	1535.8	2.61	32.12
5.65	244.68	3.58E+07	2014.3	2.84	29.48
6.30	245.33	4.20E+07	2535.9	3.03	27.60
6.94	245.97	4.88E+07	3108.4	3.20	26.16
7.59	246.62	5.61E+07	3740.6	3.35	24.98
8.23	247.26	6.38E+07	4446.7	3.50	23.93
8.88	247.91	7.21E+07	5229.7	3.64	22.98
9.52	248.55	8.09E+07	6094.3	3.78	22.12
10.17	249.20	9.01E+07	7050.4	3.93	21.31
10.81	249.84	1.00E+08	7773.0	3.90	21.46
11.46	250.49	1.11E+08	8998.8	4.07	20.56
12.10	251.13	1.22E+08	10318.2	4.24	19.75

<---- hydrograph ---->						<-pipe / channel->	
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL		
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW : ID= 2 (8372)	1967.31	15.24	9.00	28.60	0.85	0.81	
OUTFLOW : ID= 1 (6258)	1967.31	13.61	11.25	28.60	0.80	0.82	

ADD HYD (8374)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0258)	181.99	2.620	8.00	45.75
+ ID2= 2 (6258)	1967.31	13.612	11.25	28.60
=====				
ID = 3 (8374)	2149.30	15.662	11.00	30.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8376)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0260)	476.24	6.777	8.00	49.68
+ ID2= 2 (8374)	2149.30	15.662	11.00	30.05
=====				
ID = 3 (8376)	2625.54	21.578	10.25	33.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8378)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0262)	341.31	5.995	7.50	49.52
+ ID2= 2 (8376)	2625.54	21.578	10.25	33.61
=====				
ID = 3 (8378)	2966.85	26.472	10.00	35.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8362)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8378)	2966.85	26.472	10.00	35.44
+ ID2= 2 (8384)	1321.42	14.172	8.75	36.66
=====				
ID = 3 (8362)	4288.27	40.199	9.25	35.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6272) |
IN= 2--> OUT= 1 | Routing time step (min)= 15.00

DATA FOR SECTION (2721.0)			
Distance	Elevation	Manning	
0.00	231.01	0.0450	
23.01	223.65	0.0450	
34.51	222.46	0.0450	
51.77	222.11	0.0450	
69.02	221.87	0.0450	
161.06	221.92	0.0450	
166.81	221.91	0.0450	
172.56	221.89	0.0450	
180.57	221.40	0.0450 / 0.0300	Main Channel
181.57	220.60	0.0300	Main Channel
184.07	220.16	0.0300	Main Channel
195.57	221.85	0.0300 / 0.0450	Main Channel
201.32	221.82	0.0450	
207.07	221.77	0.0450	
212.83	221.72	0.0450	
253.09	222.52	0.0450	
408.40	222.65	0.0450	
460.17	223.20	0.0450	
517.69	224.84	0.0450	
569.46	232.57	0.0450	

TRAVEL TIME TABLE					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.41	220.57	.189E+04	0.5	0.49	60.18
0.83	220.99	.697E+04	3.2	0.82	35.79
1.24	221.40	1.45E+05	8.8	1.07	27.45
1.84	222.00	6.43E+05	29.3	0.81	36.61
2.44	222.60	2.82E+06	132.9	0.83	35.41
3.04	223.20	7.07E+06	413.7	1.04	28.50
3.64	223.80	1.18E+07	876.8	1.31	22.46
4.24	224.40	1.68E+07	1494.8	1.57	18.75
4.84	225.00	2.21E+07	2261.1	1.82	16.23
5.44	225.60	2.74E+07	3196.3	2.06	14.29
6.04	226.20	3.28E+07	4258.6	2.30	12.84
6.65	226.81	3.83E+07	5444.7	2.52	11.72
7.25	227.41	4.38E+07	6749.7	2.73	10.82
7.85	228.01	4.94E+07	8169.4	2.93	10.08
8.45	228.61	5.51E+07	9700.7	3.12	9.46
9.05	229.21	6.08E+07	11340.8	3.30	8.94
9.65	229.81	6.66E+07	13087.4	3.48	8.48
10.25	230.41	7.24E+07	14938.5	3.65	8.08
10.85	231.01	7.83E+07	16892.4	3.82	7.73

<---- hydrograph ---->						<-pipe / channel->	
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL		
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW : ID= 2 (8362)	4288.27	40.20	9.25	35.82	1.90	0.81	
OUTFLOW : ID= 1 (6272)	4288.27	39.68	10.00	35.82	1.90	0.81	

ADD HYD (8356)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0272)	157.38	2.876	7.50	41.09
+ ID2= 2 (6272)	4288.27	39.678	10.00	35.82
=====				
ID = 3 (8356)	4445.65	41.748	10.00	36.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8352)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8354)	1268.73	24.661	7.75	46.69
+ ID2= 2 (8356)	4445.65	41.748	10.00	36.00
=====				
ID = 3 (8352)	5714.38	62.779	9.00	38.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6304) |
IN= 2--> OUT= 1 | Routing time step (min)= 15.00

DATA FOR SECTION (3041.0)		
Distance	Elevation	Manning
0.00	232.08	0.0500
19.00	231.87	0.0500
38.00	231.33	0.0500
66.51	230.44	0.0500
104.51	228.25	0.0500

118.76	225.17	0.0500	
128.26	219.86	0.0500	
175.77	219.17	0.0500	
185.27	218.90	0.0500 / 0.0300	Main Channel
185.52	218.65	0.0300	Main Channel
190.02	218.37	0.0300	Main Channel
194.52	218.60	0.0300	Main Channel
194.77	218.85	0.0300 / 0.0500	Main Channel
204.27	219.60	0.0500	
299.28	220.91	0.0500	
327.78	222.36	0.0500	
375.28	225.71	0.0500	
403.79	229.37	0.0500	
432.29	230.43	0.0500	
470.29	232.00	0.0500	

----->>> TRAVEL TIME TABLE <<<-----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.48	218.85	.121E+05	1.2	0.36	173.94
1.17	219.54	.839E+05	9.8	0.44	142.60
1.86	220.23	.323E+06	41.0	0.47	131.43
2.56	220.93	.708E+06	109.5	0.58	107.67
3.25	221.62	.118E+07	224.3	0.71	87.36
3.94	222.31	.168E+07	376.5	0.84	74.48
4.63	223.00	.222E+07	567.9	0.96	65.26
5.32	223.69	.279E+07	794.7	1.07	58.58
6.02	224.39	.339E+07	1056.2	1.17	53.52
6.71	225.08	.402E+07	1352.4	1.26	49.52
7.40	225.77	.468E+07	1677.3	1.34	46.46
8.09	226.46	.536E+07	2050.0	1.43	43.58
8.79	227.16	.607E+07	2457.9	1.52	41.14
9.48	227.85	.680E+07	2901.0	1.60	39.05
10.17	228.54	.755E+07	3352.7	1.66	37.53
10.86	229.23	.834E+07	3812.5	1.71	36.48
11.55	229.92	.919E+07	4241.8	1.73	36.11
12.25	230.62	.101E+08	4697.0	1.74	35.93
12.94	231.31	.111E+08	5186.3	1.74	35.83

----->>> hydrograph <<<----- <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8352) 5714.38	62.78	9.00	38.38	2.08	0.50
OUTFLOW : ID= 1 (6304) 5714.38	52.13	11.25	38.38	1.98	0.49

----->>> ADD HYD (8350) <<<-----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0304):	292.37	1.844	10.25	29.90
+ ID2= 2 (6304):	5714.38	52.127	11.25	38.38
ID = 3 (8350):	6006.75	53.944	11.25	37.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----->>> ADD HYD (8346) <<<-----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8348):	3768.46	40.043	9.50	35.26
+ ID2= 2 (8350):	6006.75	53.944	11.25	37.96
ID = 3 (8346):	9775.21	91.832	10.50	36.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----->>> ADD HYD (8344) <<<-----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0306):	283.97	0.637	13.75	21.91
+ ID2= 2 (8346):	9775.21	91.832	10.50	36.92
ID = 3 (8344):	10059.18	92.399	10.50	36.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----->>> RESERVOIR (5509) <<<-----

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	*****	370.0451
41.0590	56.7403	*****	863.4386
48.1390	86.3439	*****	*****
56.6340	*****	*****	*****

67.9600 ***** | 0.0000 0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8344) *****	92.399	10.50	36.50
OUTFLOW : ID= 1 (5509) *****	55.794	15.75	36.50

PEAK FLOW REDUCTION [Qout/Qin](%)= 60.38
TIME SHIFT OF PEAK FLOW (min)=315.00
MAXIMUM STORAGE USED (ha.m.)=119.7052

----->>> CALIB (0332) <<<-----

Area (ha)	Ia (mm)	U.H. Tp(hrs)	Curve Number (CN)	# of Linear Res.(N)
NASHYD (0332)	393.44	5.00	75.0	1.50
ID= 1 DT=15.0 min	393.44	5.00	75.0	1.50

Unit Hyd Qpeak (cms)= 2.894

PEAK FLOW (cms)= 3.974 (1)
TIME TO PEAK (hrs)= 9.500
RUNOFF VOLUME (mm)= 41.340
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.467

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----->>> CALIB (0330) <<<-----

Area (ha)	Ia (mm)	U.H. Tp(hrs)	Curve Number (CN)	# of Linear Res.(N)
NASHYD (0330)	468.30	5.00	80.0	1.50
ID= 1 DT=15.0 min	468.30	5.00	80.0	1.50

Unit Hyd Qpeak (cms)= 7.335

PEAK FLOW (cms)= 9.926 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 47.004
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.531

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----->>> ROUTE CHN (6332) <<<-----

In= 2-->	Out= 1	Routing time step (min)*= 15.00
----------	--------	---------------------------------

----->>> DATA FOR SECTION (3321.0) <<<-----

Distance	Elevation	Manning
0.00	234.00	0.0380
25.85	227.20	0.0380
96.94	226.44	0.0380
168.03	227.38	0.0380
219.73	225.62	0.0380
342.52	221.57	0.0380
368.37	221.42	0.0380
374.83	221.23	0.0380
379.79	220.98	0.0380 / 0.0300
380.29	220.47	0.0300
381.29	220.47	0.0300
382.79	220.47	0.0300
383.29	220.98	0.0300 / 0.0380
394.22	221.22	0.0380
400.68	221.33	0.0380
407.14	221.44	0.0380
491.16	225.70	0.0380
568.71	227.55	0.0380
607.49	230.14	0.0380
639.80	234.08	0.0380

----->>> TRAVEL TIME TABLE <<<-----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.51	220.98	.554E+04	0.9	0.56	107.27
1.20	221.67	.885E+05	12.1	0.49	122.32
1.88	222.35	.309E+06	64.7	0.76	79.70
2.57	223.04	.615E+06	165.4	0.97	61.99
3.25	223.72	.101E+07	322.1	1.16	52.06
3.94	224.41	.148E+07	542.9	1.33	45.51
4.62	225.09	.204E+07	835.4	1.48	40.77
5.31	225.78	.269E+07	1202.8	1.62	37.27
5.99	226.46	.344E+07	1618.0	1.70	35.46
6.68	227.15	.447E+07	1932.0	1.56	38.56
7.36	227.83	.580E+07	2763.9	1.72	34.96
8.05	228.52	.718E+07	3861.9	1.95	30.97
8.73	229.20	.859E+07	5115.4	2.16	27.98
9.42	229.89	.100E+08	6519.2	2.35	25.64
10.10	230.57	.115E+08	8092.3	2.55	23.69
10.79	231.26	.130E+08	9921.5	2.73	22.05
11.47	231.94	.145E+08	11690.0	2.92	20.68
12.16	232.63	.160E+08	13694.9	3.09	19.52
12.84	233.31	.176E+08	15833.9	3.26	18.52

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (0330) 468.30 9.93 7.25 47.00 1.06 0.50
OUTFLOW: ID= 1 (6332) 468.30 7.46 9.50 47.00 0.91 0.52

```

```

-----
| ADD HYD (8322) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID= 1 (0332): 393.44 3.974 9.50 41.34 |
| + ID= 2 (6332): 468.30 7.455 9.50 47.00 |
|=====|
| ID = 3 (8322): 861.74 11.429 9.50 44.42 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB (0328) | Area (ha)= 492.92 Curve Number (CN)= 77.0
| NASHYD (0328) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.91
|-----|
Unit Hyd Qpeak (cms)= 4.411
PEAK FLOW (cms)= 6.150 (i)
TIME TO PEAK (hrs)= 8.750
RUNOFF VOLUME (mm)= 43.578
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.492

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0326) | Area (ha)= 678.91 Curve Number (CN)= 80.0
| NASHYD (0326) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.95
|-----|
Unit Hyd Qpeak (cms)= 5.941
PEAK FLOW (cms)= 9.054 (i)
TIME TO PEAK (hrs)= 8.750
RUNOFF VOLUME (mm)= 47.251
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.534

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0324) | Area (ha)= 615.64 Curve Number (CN)= 79.0
| NASHYD (0324) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.90
|-----|
Unit Hyd Qpeak (cms)= 5.544
PEAK FLOW (cms)= 8.170 (i)
TIME TO PEAK (hrs)= 8.500
RUNOFF VOLUME (mm)= 45.986
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.520

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0320) | Area (ha)= 278.74 Curve Number (CN)= 81.0
| NASHYD (0320) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.36
|-----|
Unit Hyd Qpeak (cms)= 3.499
PEAK FLOW (cms)= 5.113 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 48.415
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.547

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (3182) | Area (ha)= 457.40 Curve Number (CN)= 78.0
| NASHYD (3182) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.49
|-----|

```

```

Unit Hyd Qpeak (cms)= 5.259
PEAK FLOW (cms)= 7.201 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 44.687
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.505

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (3181) | Area (ha)= 26.00
| STANDHYD (3181) | Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00
| ID= 1 DT=15.0 min |
|-----|

```

```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 21.06 4.94
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 416.33 40.00
Mannings n = 0.013 0.250

```

```

Max.Eff.Inten.(mm/hr)= 116.82 67.96
over (min)= 15.00 30.00
Storage Coeff. (min)= 6.96 (ii) 19.44 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.10 0.05

```

TOTALS

```

PEAK FLOW (cms)= 6.31 0.48 6.617 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 86.50 43.75 78.38
TOTAL RAINFALL (mm)= 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.49 0.89

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

```

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 77.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

```

-----
| RESERVOIR (5318) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
|-----|
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
0.0000 0.0000 | 3.3800 0.7654
0.0510 0.4423 | 4.1500 0.8231
0.8700 0.5378 | 4.8750 0.8778
1.9590 0.6105 | 5.2750 0.8878
2.5620 0.6796 | 0.0000 0.0000

```

```

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (3181) 26.000 6.617 6.00 78.38
OUTFLOW: ID= 1 (5318) 26.000 3.934 6.25 78.29

```

```

PEAK FLOW REDUCTION [Qout/Qin](%)= 59.45
TIME SHIFT OF PEAK FLOW (min)= 15.00
MAXIMUM STORAGE USED (ha.m.)= 0.8924

```

```

-----
| ADD HYD (8334) |
| 1 + 2 = 3 |
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID= 1 (3182): 457.40 7.201 8.00 44.69 |
| + ID= 2 (5318): 26.00 3.934 6.25 78.29 |
|=====|
| ID = 3 (8334): 483.40 8.715 6.25 46.49 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CN (6320) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
|-----|

```

```

<----- DATA FOR SECTION (3201.0) ----->
Distance Elevation Manning
0.00 249.00 0.0380
22.92 245.86 0.0380
45.83 244.87 0.0380
91.66 243.11 0.0380
126.03 239.53 0.0380
160.41 237.17 0.0380
166.14 237.06 0.0380
177.59 237.13 0.0380
183.32 237.20 0.0380
189.05 236.70 0.0380 /0.0350 Main Channel

```

193.05	235.89	0.0350	Main Channel
197.05	236.64	0.0350 / 0.0380	Main Channel
200.51	236.74	0.0380	
206.24	237.03	0.0380	
246.34	238.82	0.0380	
263.53	243.87	0.0380	
389.56	247.64	0.0380	
452.58	247.74	0.0380	
498.41	248.60	0.0380	
567.16	249.84	0.0380	

530.58	242.08	0.0350
586.43	242.93	0.0350
691.16	248.00	0.0350

TRAVEL TIME TABLE						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.37	235.62	.215E+04	0.7	0.77	53.88	
0.75	236.00	.634E+04	3.5	1.36	30.45	
1.46	236.71	.297E+05	19.3	1.62	25.66	
2.16	237.41	.103E+06	75.8	1.84	22.60	
2.87	238.12	.229E+06	202.8	2.21	18.79	
3.57	238.82	.407E+06	423.3	2.59	16.03	
4.28	239.53	.654E+06	727.7	2.77	14.97	
4.99	240.24	.103E+07	1242.0	3.01	13.76	
5.69	240.94	.151E+07	2002.3	3.31	12.54	
6.40	241.65	.230E+07	2987.9	3.55	11.70	
7.10	242.35	.286E+07	4302.8	3.75	11.07	
7.81	243.06	.374E+07	6165.3	4.10	10.11	
8.52	243.76	.468E+07	8642.4	4.60	9.03	
9.22	244.47	.565E+07	11496.3	5.06	8.00	
9.93	245.18	.666E+07	14714.1	5.50	7.54	
10.63	245.88	.770E+07	18290.3	5.91	7.02	
11.34	246.59	.877E+07	22221.9	6.30	6.58	
12.04	247.29	.988E+07	26464.4	6.67	6.22	
12.75	248.00	.110E+08	31061.3	7.01	5.91	

TRAVEL TIME TABLE						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.37	236.26	.270E+04	0.4	0.58	107.96	
0.75	236.64	.108E+05	2.6	0.92	68.01	
1.48	237.37	.497E+05	27.4	1.17	53.35	
2.20	238.09	.277E+06	122.1	1.65	37.83	
2.93	238.82	.540E+06	299.7	2.08	30.01	
3.66	239.55	.857E+06	587.8	2.57	24.29	
4.39	240.28	.120E+07	977.8	3.04	20.52	
5.11	241.00	.158E+07	1458.1	3.46	18.02	
5.84	241.73	.198E+07	2028.6	3.84	16.23	
6.57	242.46	.240E+07	2690.5	4.19	14.86	
7.29	243.18	.285E+07	3431.8	4.50	13.84	
8.02	243.91	.335E+07	4149.1	4.64	13.44	
8.75	244.64	.393E+07	4746.6	4.51	13.80	
9.47	245.36	.463E+07	5618.6	4.53	13.75	
10.20	246.09	.545E+07	6781.8	4.66	13.39	
10.93	246.82	.635E+07	8274.5	4.87	12.79	
11.66	247.55	.733E+07	9980.0	5.10	12.23	
12.38	248.27	.853E+07	11823.0	5.01	12.45	
13.11	249.00	.989E+07	13749.7	5.20	11.98	

hydrograph						
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW: ID= 2 (8334)	483.40	8.72	6.25	46.49	0.93	0.97
OUTFLOW: ID= 1 (6320)	483.40	7.07	9.00	46.49	0.88	0.95

hydrograph						
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW: ID= 2 (8334)	483.40	8.72	6.25	46.49	0.93	0.97
OUTFLOW: ID= 1 (6320)	483.40	7.07	9.00	46.49	0.88	0.95

CALIB						
NASHYD	Area	(ha)	(cms)	Curve Number	(CN)=	82.0
ID= 1 DT=15.0 min	Ia	(mm)=	5.00	# of Linear Res.(N)=	1.50	
U.H. Tp(hrs)=	1.01					

Unit Hyd Qpeak (cms) = 3.923
 PEAK FLOW (cms) = 5.548 (l)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 49.565
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.560

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8338)						
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (0316):	232.34	5.548	7.25	49.57		
+ ID2= 2 (6316):	165.20	4.376	7.25	44.18		
ID = 3 (8338):	397.54	9.924	7.25	47.33		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB						
NASHYD	Area	(ha)	(cms)	Curve Number	(CN)=	80.0
ID= 1 DT=15.0 min	Ia	(mm)=	5.00	# of Linear Res.(N)=	1.50	
U.H. Tp(hrs)=	1.36					

Unit Hyd Qpeak (cms) = 4.529
 PEAK FLOW (cms) = 6.425 (l)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 47.122
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.532

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB						
NASHYD	Area	(ha)	(cms)	Curve Number	(CN)=	62.0
ID= 1 DT=15.0 min	Ia	(mm)=	5.00	# of Linear Res.(N)=	1.50	
U.H. Tp(hrs)=	1.62					

Unit Hyd Qpeak (cms) = 5.575
 PEAK FLOW (cms) = 4.939 (l)
 TIME TO PEAK (hrs) = 8.250
 RUNOFF VOLUME (mm) = 29.006
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.328

ROUTING TIME STEP (min) = 15.00

DATA FOR SECTION (3161.0)			
Distance	Elevation	Manning	
0.00	248.54	0.0350	
27.93	248.34	0.0350	
41.89	246.61	0.0350	
62.83	243.09	0.0350	
132.65	239.00	0.0350	
188.50	236.74	0.0350	
195.48	236.54	0.0350	
202.46	236.32	0.0350	
205.48	236.14	0.0350 / 0.0310	Main Channel
205.98	235.61	0.0310	Main Channel
207.98	235.25	0.0310	Main Channel
209.98	235.53	0.0310	Main Channel
210.48	236.00	0.0310 / 0.0350	Main Channel
216.42	236.73	0.0350	
258.31	239.09	0.0350	
328.12	239.84	0.0350	
439.83	241.47	0.0350	

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0310) Area (ha)= 138.28 Curve Number (CN)= 65.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.76

Unit Hyd Opeak (cms)= 3.102

PEAK FLOW (cms)= 2.521 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 31.169
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.352

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8342)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
ID1= 1 (0308): 529.30 4.939 8.25 29.01
+ ID2= 2 (0312): 138.28 2.521 7.00 31.17
ID = 3 (8342): 667.58 7.113 7.75 29.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6312)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

Table with columns: Distance, Elevation, Manning. Rows show channel data for section 3121.0.

TRAVEL TIME TABLE
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)

hydrograph
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
INFLOW : ID= 2 (8342) 667.58 7.11 7.75 29.45 0.56 1.85
OUTFLOW: ID= 1 (6312) 667.58 7.11 7.75 29.45 0.55 1.85

ADD HYD (8340)

1 + 2 = 3
AREA QPEAK TPEAK R.V.
ID1= 1 (0312): 359.44 6.425 8.00 47.12
+ ID2= 2 (6312): 667.58 7.109 7.75 29.45
ID = 3 (8340): 1027.02 13.531 8.00 35.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8336)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
ID1= 1 (8338): 397.54 9.924 7.25 47.33
+ ID2= 2 (8340): 1027.02 13.531 8.00 35.64
ID = 3 (8336): 1424.56 23.290 7.50 38.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8330)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
ID1= 1 (8332): 762.14 11.950 8.50 47.20
+ ID2= 2 (8336): 1424.56 23.290 7.50 38.90
ID = 3 (8330): 2186.70 34.722 7.75 41.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6324)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

Table with columns: Distance, Elevation, Manning. Rows show channel data for section 3241.0.

TRAVEL TIME TABLE
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)

hydrograph
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
INFLOW : ID= 2 (8330) 2186.70 34.72 7.75 41.79 1.63 0.79
OUTFLOW: ID= 1 (6324) 2186.70 29.73 9.50 41.79 1.56 0.78

ADD HYD (8328)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8324):	615.64	8.170	8.50	45.99
+ ID2= 2 (6324):	2186.70	29.727	9.50	41.79
=====				
ID = 3 (8328):	2802.34	37.743	9.25	42.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	Curve Number (CN)= 80.0
NASHYD (0322)	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min	U.H. Tp(hrs)=	1.75	

Unit Hyd Qpeak (cms)= 5.020

PEAK FLOW (cms)= 7.498 (1)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 47.219
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.534

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8326)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0322):	513.13	7.498	8.25	47.22
+ ID2= 2 (8328):	2802.34	37.743	9.25	42.71
=====				
ID = 3 (8326):	3315.47	45.039	9.25	43.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8324)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0326):	678.91	9.054	8.75	47.25
+ ID2= 2 (8326):	3315.47	45.039	9.25	43.41
=====				
ID = 3 (8324):	3994.38	54.016	9.25	44.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6328)	Routing time step (min)= 15.00
------------------	--------------------------------

<----- DATA FOR SECTION (3281.0) ----->

Distance	Elevation	Manning	
0.00	228.00	0.0380	
18.58	224.97	0.0380	
78.98	223.52	0.0380	
125.44	223.28	0.0380	
171.90	221.71	0.0380	
213.72	219.65	0.0380	
218.36	219.40	0.0380	
223.01	219.19	0.0380	
225.95	219.14	0.0380 / 0.0320	Main Channel
226.45	218.14	0.0320	Main Channel
236.95	218.14	0.0320	Main Channel
245.95	218.14	0.0320	Main Channel
245.95	219.14	0.0320 / 0.0380	Main Channel
246.24	219.16	0.0380	
250.88	219.24	0.0380	
255.53	219.39	0.0380	
325.22	221.47	0.0380	
367.03	223.14	0.0380	
404.20	225.17	0.0380	
459.95	228.04	0.0380	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELRV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	218.64	.373E+05	3.2	0.33	192.37
1.00	219.14	.752E+05	10.1	0.51	124.71
1.52	219.66	.149E+06	22.2	0.57	112.06
2.04	220.18	.279E+06	43.4	0.59	107.06
2.56	220.70	.464E+06	77.0	0.63	100.52
3.08	221.22	.705E+06	125.7	0.68	93.96
3.61	221.75	.100E+07	192.7	0.73	86.62
4.13	222.27	.135E+07	278.1	0.79	80.92
4.65	222.79	.176E+07	385.9	0.84	75.86
5.17	223.31	.222E+07	514.9	0.89	71.82

5.69	223.83	.280E+07	662.7	0.90	70.40
6.21	224.35	.345E+07	867.7	0.96	66.35
6.73	224.87	.417E+07	1107.0	1.01	62.81
7.25	225.39	.494E+07	1400.0	1.08	58.80
7.78	225.92	.573E+07	1732.2	1.15	55.17
8.30	226.44	.655E+07	2099.9	1.22	52.02
8.82	226.96	.740E+07	2502.8	1.29	49.29
9.34	227.48	.828E+07	2940.9	1.36	46.90
9.86	228.00	.918E+07	3414.3	1.42	44.79

<---- hydrograph ----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH (m)	MAX VEL (m/s)
(ha)	(cms)	(hrs)	(mm)		
INFLOW : ID= 2 (8324) 3994.38	54.02	9.25	44.06	2.21	0.61
OUTFLOW: ID= 1 (6328) 3994.38	46.11	11.00	44.06	2.08	0.60

ADD HYD (8320)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0328):	492.92	6.150	8.75	43.58
+ ID2= 2 (6328):	3994.38	46.106	11.00	44.06
=====				
ID = 3 (8320):	4487.30	51.569	11.00	44.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8318)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8320):	4487.30	51.569	11.00	44.01
+ ID2= 2 (8322):	861.74	11.429	9.50	44.42
=====				
ID = 3 (8318):	5349.04	62.499	10.75	44.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8316)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8509):	10059.38	55.794	15.75	36.50
+ ID2= 2 (8318):	5349.04	62.499	10.75	44.08
=====				
ID = 3 (8316):	15408.22	109.731	11.75	39.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8312)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8314):	620.10	2.136	6.25	25.79
+ ID2= 2 (8316):	15408.22	109.731	11.75	39.13
=====				
ID = 3 (8312):	16028.32	110.987	11.75	38.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8308)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8310):	598.90	44.535	6.25	47.72
+ ID2= 2 (8312):	16028.32	110.987	11.75	38.61
=====				
ID = 3 (8308):	16627.22	113.364	11.75	38.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5510)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1				
DT= 15.0 min	0.0000	0.0000	*****	74.0090
	66.5450	18.5023	*****	*****
	98.5430	37.0045	*****	*****

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8308)	*****	113.364	11.75
			38.94

OUTFLOW: ID= 1 (5510) ***** 105.320 13.75 38.94

PEAK FLOW REDUCTION [Qout/Qin](%)= 92.90
TIME SHIFT OF PEAK FLOW (min)=120.00
MAXIMUM STORAGE USED (ha.m.)= 44.9262

ADD HYD (8240)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0336): 2785.00 4.639 22.25 38.20
+ ID2= 2 (5510): 16627.22 105.320 13.75 38.94
ID = 3 (8240): 19412.22 109.428 14.00 38.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8238)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (5508): 9524.23 56.167 11.75 25.09
+ ID2= 2 (8240): 19412.22 109.428 14.00 38.84
ID = 3 (8238): 28936.45 163.719 13.00 34.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8236)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0344): 1004.58 5.230 10.00 23.19
+ ID2= 2 (8238): 28936.45 163.719 13.00 34.32
ID = 3 (8236): 29941.03 168.365 12.75 33.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8234)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8232): 285.80 10.972 6.50 32.52
+ ID2= 2 (8236): 29941.03 168.365 12.75 33.94
ID = 3 (8234): 30226.83 168.774 12.50 33.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8230)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0344): 344.00 4.771 7.00 26.48
+ ID2= 2 (8234): 30226.83 168.774 12.50 33.93
ID = 3 (8230): 30570.83 170.215 12.50 33.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8228)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8226): 1952.06 15.927 8.75 22.21
+ ID2= 2 (8230): 30570.83 170.215 12.50 33.84
ID = 3 (8228): 32522.89 180.663 12.00 33.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8190)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0356): 1015.22 6.685 8.00 18.15
+ ID2= 2 (8228): 32522.89 180.663 12.00 33.15
ID = 3 (8190): 33538.11 185.041 12.00 32.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM
Filename: C:\Users\jsscott\AppData
Local\Temp\
3e280798-92ee-4282-809c-79f5caed0add\9255d239
Total= 80.80 mm
Comments: 50-Year 12-Hour SCS II Design Storm

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity over time for different durations.

CALIB (0356)
NASHYD (0356) Area (ha)=1015.22 Curve Number (CN)= 46.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.37

Unit Hyd Qpeak (cms)= 12.651
PEAK FLOW (cms)= 5.609 (l)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 15.266
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.189

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0354)
NASHYD (0354) Area (ha)= 262.68 Curve Number (CN)= 37.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.38

Unit Hyd Qpeak (cms)= 3.252
PEAK FLOW (cms)= 1.056 (l)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 11.232
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.139

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0352)
NASHYD (0352) Area (ha)= 381.43 Curve Number (CN)= 54.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.82

Unit Hyd Qpeak (cms)= 7.980
PEAK FLOW (cms)= 4.018 (l)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 19.394
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.240

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0346)
NASHYD (0346) Area (ha)= 350.93 Curve Number (CN)= 70.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 6.254
PEAK FLOW (cms)= 5.278 (l)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 30.777
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.381

EXISTING

WEST HOLLAND RIVER REGIONAL STORM (RF1000 - RF927)

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V V I SSSS U U A L
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
WV I SSSS UUUU A A LLLL

OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y M M O O
O O T T H H Y M O O
OOO T T H H Y M M OOO Company Serial
    
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***** DETAILED OUTPUT *****

Input filename: C:\Program Files\Visual Gchymo 2.4r\VO2\voim.dat
 Output filename: C:\Users\jscott\AppData\Local\Temp\96b8d007-9cc9-4be4-b3c8-096f6a228b82\Scenario.out
 Summary filename: C:\Users\jscott\AppData\Local\Temp\96b8d007-9cc9-4be4-b3c8-096f6a228b82\Scenario.sum

DATE: 08/22/2012 TIME: 01:35:05

USER:

COMMENTS: _____

** SIMULATION NUMBER: 1 **

READ STORM Filename: C:\Users\jscott\AppData\Local\Temp\96b8d007-9cc9-4be4-b3c8-096f6a228b82\487f093f
 Ptotal=212.00 mm Comments: 12-Hour Hurricane Hazel

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	6.00	3.25	13.00	6.25	23.00	9.25	53.00
0.50	6.00	3.50	13.00	6.50	23.00	9.50	53.00
0.75	6.00	3.75	13.00	6.75	23.00	9.75	53.00
1.00	6.00	4.00	13.00	7.00	23.00	10.00	53.00
1.25	4.00	4.25	17.00	7.25	13.00	10.25	38.00
1.50	4.00	4.50	17.00	7.50	13.00	10.50	38.00
1.75	4.00	4.75	17.00	7.75	13.00	10.75	38.00
2.00	4.00	5.00	17.00	8.00	13.00	11.00	38.00
2.25	6.00	5.25	13.00	8.25	13.00	11.25	13.00
2.50	6.00	5.50	13.00	8.50	13.00	11.50	13.00
2.75	6.00	5.75	13.00	8.75	13.00	11.75	13.00
3.00	6.00	6.00	13.00	9.00	13.00	12.00	13.00

CALIB (0356) Area (ha)=1015.22 Curve Number (CN)= 66.4
 NASHVD (0356) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.37

Unit Hyd Qpeak (cms)= 12.651

PEAK FLOW (cms)= 47.576 (i)
 TIME TO PEAK (hrs)= 11.750
 RUNOFF VOLUME (mm)= 126.889
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.599

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0354) Area (ha)= 262.68 Curve Number (CN)= 57.0
 NASHVD (0354) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.38

Unit Hyd Qpeak (cms)= 3.252

PEAK FLOW (cms)= 10.497 (i)
 TIME TO PEAK (hrs)= 12.000
 RUNOFF VOLUME (mm)= 106.815

TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.504

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0352) Area (ha)= 381.43 Curve Number (CN)= 72.9
 NASHVD (0352) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.82

Unit Hyd Qpeak (cms)= 7.980

PEAK FLOW (cms)= 26.072 (i)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 140.195
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.661

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0346) Area (ha)= 350.93 Curve Number (CN)= 85.4
 NASHVD (0346) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 6.254

PEAK FLOW (cms)= 25.363 (i)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 169.245
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.798

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0350) Area (ha)= 366.84 Curve Number (CN)= 69.3
 NASHVD (0350) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 5.831

PEAK FLOW (cms)= 20.648 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 132.873
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.627

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0348) Area (ha)= 590.18 Curve Number (CN)= 68.0
 NASHVD (0348) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.91

Unit Hyd Qpeak (cms)= 5.267

PEAK FLOW (cms)= 22.856 (i)
 TIME TO PEAK (hrs)= 12.000
 RUNOFF VOLUME (mm)= 130.716
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.617

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6350) Routing time step (min)= 15.00

<----- DATA FOR SECTION (3501.0) ----->

Distance	Elevation	Manning
0.00	287.00	0.0500
23.66	283.72	0.0500
70.98	280.94	0.0500
112.38	280.23	0.0500
171.52	276.80	0.0500
260.24	274.46	0.0500
266.16	274.26	0.0500
272.07	274.12	0.0500
276.49	274.12	0.0500 / 0.0300 Main Channel
276.99	273.82	0.0300 Main Channel
277.99	273.82	0.0300 Main Channel
279.49	273.82	0.0300 Main Channel
280.49	274.27	0.0300 / 0.0500 Main Channel
283.90	274.27	0.0500
289.81	274.57	0.0500

325.30	275.75	0.0500
396.28	278.98	0.0500
449.51	280.97	0.0500
496.83	283.90	0.0500
585.54	287.92	0.0500

```

----- TRAVEL TIME TABLE -----
DEPTH   ELEV   VOLUME   FLOW RATE   VELOCITY   TRAV.TIME
(m)      (m)      (cu.m.)   (cms)        (m/s)      (min)
0.30    274.12   .273E+04   0.9          0.99       49.81
0.98    274.80   .583E+05   20.8         1.05       46.71
1.66    275.48   .203E+06   95.2         1.38       35.53
2.33    276.15   .438E+06   256.0        1.73       28.50
3.01    276.83   .755E+06   524.8        2.05       23.97
3.69    277.51   .114E+07   935.8        2.43       20.26
4.37    278.19   .157E+07   1472.0       2.76       17.82
5.05    278.87   .206E+07   2140.7       3.06       16.06
5.72    279.54   .261E+07   2938.6       3.33       14.79
6.40    280.22   .321E+07   3887.6       3.57       13.77
7.08    280.90   .390E+07   4795.4       3.63       13.56
7.76    281.58   .468E+07   5716.2       3.91       12.58
8.43    282.25   .550E+07   6783.9       4.19       11.74
9.11    282.93   .637E+07   7901.9       4.45       11.05
9.79    283.61   .728E+07   9154.8       4.70       10.46
10.47   284.29   .823E+07   10445.3      4.96       9.91
11.15   284.97   .922E+07   11879.4      5.21       9.43
11.82   285.64   .103E+08   13452.0     5.45       9.02
12.50   286.32   .113E+08   15174.3     5.69       8.65

```

```

----- hydrograph ----- <-pipe / channel->
AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
(ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
INFLW : ID= 2 (0348) 590.18  22.86  12.00  130.72  1.00  1.06
OUTFLOW: ID= 1 (6350) 590.18  21.81  12.75  130.72  0.99  1.06

```

```

ADD HYD (8220)
1 + 2 = 3
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0350): 366.84  20.648  11.25  132.87
+ ID2= 2 (6350): 590.18  21.810  12.75  130.72
=====
ID = 3 (8220): 957.02  40.682  12.00  131.54

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8222)
1 + 2 = 3
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0346): 350.93  25.363  11.00  169.25
+ ID2= 2 (8220): 957.02  40.682  12.00  131.54
=====
ID = 3 (8222): 1307.95  64.525  11.75  141.66

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6352)
IN= 2----> OUT= 1 | Routing time step (min)'= 15.00

```

```

----- DATA FOR SECTION (3521.0) -----
Distance   Elevation   Manning
0.00       257.95      0.0500
7.83       257.34      0.0500
15.66      256.19      0.0500
21.53      254.01      0.0500
41.11      244.06      0.0500
76.35      241.38      0.0500
111.58     239.74      0.0500
113.54     239.61      0.0500
113.75     239.41      0.0500 /0.0300 Main Channel
113.85     238.81      0.0300 Main Channel
115.50     238.81      0.0300 Main Channel
116.15     238.81      0.0300 Main Channel
116.25     239.41      0.0300 Main Channel
117.46     239.52      0.0500
119.41     239.72      0.0500
121.37     240.04      0.0500
131.16     241.84      0.0500
156.61     247.03      0.0500
176.19     251.46      0.0500
193.80     258.79      0.0500

```

```

----- TRAVEL TIME TABLE -----
DEPTH   ELEV   VOLUME   FLOW RATE   VELOCITY   TRAV.TIME
(m)      (m)      (cu.m.)   (cms)        (m/s)      (min)
0.60    239.41   .224E+04   2.2          1.52       17.12
1.58    240.39   .216E+05   22.3         1.61       16.16

```

2.55	241.36	.804E+05	101.0	1.96	13.27
3.53	242.34	.173E+06	275.6	2.48	10.46
4.50	243.31	.293E+06	553.9	2.95	8.80
5.48	244.29	.438E+06	961.8	3.42	7.60
6.45	245.26	.599E+06	1534.4	3.99	6.51
7.43	246.24	.770E+06	2228.4	4.51	5.76
8.41	247.22	.951E+06	3043.6	4.98	5.21
9.38	248.19	.114E+07	3982.1	5.43	4.78
10.36	249.17	.134E+07	5040.1	5.85	4.44
11.33	250.14	.155E+07	6218.5	6.24	4.16
12.31	251.12	.177E+07	7518.6	6.61	3.93
13.29	252.10	.200E+07	8969.0	6.98	3.72
14.26	253.07	.224E+07	10554.1	7.35	3.53
15.24	254.05	.248E+07	12257.1	7.71	3.37
16.21	255.02	.272E+07	14045.1	8.03	3.23
17.19	256.00	.298E+07	15954.8	8.34	3.11
18.16	256.97	.325E+07	17780.4	8.53	3.04

```

----- hydrograph ----- <-pipe / channel->
AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
(ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
INFLW : ID= 2 (8224) 1307.95  64.52  11.75  141.66  2.10  1.78
OUTFLOW: ID= 1 (6352) 1307.95  64.34  12.00  141.66  2.10  1.78

```

```

ADD HYD (8224)
1 + 2 = 3
AREA   QPEAK   TPEAK   R.V.
(ha)   (cms)   (hrs)   (mm)
ID1= 1 (0352): 381.43  26.072  11.00  140.20
+ ID2= 2 (6352): 1307.95  64.338  12.00  141.66
=====
ID = 3 (8224): 1689.38  88.700  11.50  141.33

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6354)
IN= 2----> OUT= 1 | Routing time step (min)'= 15.00

```

```

----- DATA FOR SECTION (3541.0) -----
Distance   Elevation   Manning
0.00       253.92      0.0500
7.95       251.83      0.0500
15.89      249.97      0.0500
19.87      249.05      0.0500
47.68      242.00      0.0500
67.55      237.10      0.0500
83.44      231.11      0.0500
85.43      230.75      0.0500
85.66      230.61      0.0500 /0.0300 Main Channel
85.76      230.01      0.0300 Main Channel
87.41      230.01      0.0300 Main Channel
88.06      230.01      0.0300 Main Channel
88.16      230.61      0.0300 /0.0500 Main Channel
89.40      230.72      0.0500
91.39      230.88      0.0500
93.37      231.03      0.0500
133.11     233.96      0.0500
150.99     235.39      0.0500
172.84     239.03      0.0500
196.68     250.00      0.0500

```

```

----- TRAVEL TIME TABLE -----
DEPTH   ELEV   VOLUME   FLOW RATE   VELOCITY   TRAV.TIME
(m)      (m)      (cu.m.)   (cms)        (m/s)      (min)
0.60    230.61   .451E+04   1.5          1.06       49.40
1.62    231.63   .394E+05   14.6         1.16       45.13
2.64    232.65   .128E+06   58.8         1.44       36.30
3.66    233.67   .269E+06   149.7        1.74       30.00
4.68    234.69   .463E+06   301.5        2.04       25.58
5.70    235.71   .705E+06   530.5        2.36       22.14
6.72    236.73   .980E+06   848.4        2.71       19.25
7.74    237.75   .128E+07   1237.9       3.02       17.29
8.76    238.77   .162E+07   1706.7       3.30       15.83
9.78    239.79   .199E+07   2291.9       3.61       14.45
10.81   240.82   .237E+07   2973.7       3.92       13.30
11.83   241.84   .278E+07   3742.2       4.21       12.38
12.85   242.86   .321E+07   4599.1       4.49       11.62
13.87   243.88   .365E+07   5544.6       4.75       10.98
14.89   244.90   .412E+07   6579.7       5.00       10.44
15.91   245.92   .461E+07   7705.9       5.23       9.97
16.93   246.94   .512E+07   8924.6       5.46       9.55
17.95   247.96   .564E+07   10237.5      5.68       9.19
18.97   248.98   .619E+07   11646.2      5.89       8.86

```

```

----- hydrograph ----- <-pipe / channel->
AREA   QPEAK   TPEAK   R.V.   MAX DEPTH   MAX VEL
(ha)   (cms)   (hrs)   (mm)   (m)         (m/s)
INFLW : ID= 2 (8224) 1689.38  88.70  11.50  141.33  2.98  1.52
OUTFLOW: ID= 1 (6354) 1689.38  85.97  12.25  141.33  2.95  1.52

```

THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8226) |
| 1 + 2 = 3 |
-----
ID1= 1 (0354): 262.68 10.497 12.00 106.81
+ ID2= 2 (6354): 168.38 85.967 12.25 141.33
=====
ID = 3 (8226): 1952.06 96.397 12.00 136.68
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0344) | Area (ha)= 344.00 Curve Number (CN)= 77.3
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.87
  
```

Unit Hyd Qpeak (cms)= 6.790

```

PEAK FLOW (cms)= 24.075 (i)
TIME TO PEAK (hrs)= 11.000
RUNOFF VOLUME (mm)= 150.243
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.709
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANBYD (3402) | Area (ha)= 234.60
| ID= 1 DT=15.0 min | Total Imp(%)= 28.00 Dir. Conn.(%)= 13.00
|-----|
  
```

```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 65.69 168.91
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1250.60 40.00
Mannings n = 0.013 0.250
  
```

```

Max.Eff.Inten.(mm/hr)= 53.00 54.11
over (min)= 15.00 45.00
Storage Coeff. (min)= 18.46 (ii) 32.14 (ii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.06 0.03
  
```

TOTALS

```

PEAK FLOW (cms)= 4.36 20.95 24.397 (iii)
TIME TO PEAK (hrs)= 10.00 10.50 10.50
RUNOFF VOLUME (mm)= 210.00 141.46 150.37
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.67 0.71
  
```

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 68.3 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANBYD (3401) | Area (ha)= 51.20
| ID= 1 DT=15.0 min | Total Imp(%)= 46.00 Dir. Conn.(%)= 21.00
|-----|
  
```

```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 23.55 27.65
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 584.24 40.00
Mannings n = 0.013 0.250
  
```

```

Max.Eff.Inten.(mm/hr)= 53.00 68.14
over (min)= 15.00 30.00
Storage Coeff. (min)= 11.69 (ii) 24.16 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04
  
```

TOTALS

```

PEAK FLOW (cms)= 1.58 4.63 6.187 (iii)
TIME TO PEAK (hrs)= 10.00 10.25 10.00
RUNOFF VOLUME (mm)= 210.00 150.46 162.96
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.71 0.77
  
```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 68.3 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

```

-----
| ADD HYD (8232) |
| 1 + 2 = 3 |
-----
ID1= 1 (3401): 51.20 6.187 10.00 162.96
+ ID2= 2 (3402): 234.60 24.397 10.50 150.37
=====
ID = 3 (8232): 285.80 30.260 10.25 152.63
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0342) | Area (ha)=1004.58 Curve Number (CN)= 73.3
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 2.57
  
```

Unit Hyd Qpeak (cms)= 6.686

```

PEAK FLOW (cms)= 33.547 (i)
TIME TO PEAK (hrs)= 12.250
RUNOFF VOLUME (mm)= 142.700
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.673
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0422) | Area (ha)= 780.20 Curve Number (CN)= 73.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.95
  
```

Unit Hyd Qpeak (cms)= 6.838

```

PEAK FLOW (cms)= 32.113 (i)
TIME TO PEAK (hrs)= 12.000
RUNOFF VOLUME (mm)= 141.843
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.669
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0410) | Area (ha)= 572.01 Curve Number (CN)= 68.7
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.46
  
```

Unit Hyd Qpeak (cms)= 6.698

```

PEAK FLOW (cms)= 26.726 (i)
TIME TO PEAK (hrs)= 12.000
RUNOFF VOLUME (mm)= 132.000
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.623
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0408) | Area (ha)= 231.62 Curve Number (CN)= 76.4
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.64
  
```

Unit Hyd Qpeak (cms)= 6.198

```

PEAK FLOW (cms)= 18.415 (i)
TIME TO PEAK (hrs)= 11.000
RUNOFF VOLUME (mm)= 147.127
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.694
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0376) | Area (ha)= 463.85 Curve Number (CN)= 87.6
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.07
  
```

Unit Hyd Qpeak (cms)= 7.380

```

PEAK FLOW (cms)= 32.317 (i)
TIME TO PEAK (hrs)= 11.250
  
```

RUNOFF VOLUME (mm)= 174.746
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.824

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0374) Area (ha)= 545.70 Curve Number (CN)= 78.0
NASHYD (0374) | Area (ha)= 545.70 Curve Number (CN)= 78.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.51

Unit Hyd Qpeak (cms)= 6.158

PEAK FLOW (cms)= 28.262 (i)
TIME TO PEAK (hrs)= 11.750
RUNOFF VOLUME (mm)= 152.931
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.721

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0372) Area (ha)= 110.42 Curve Number (CN)= 57.4
NASHYD (0372) | Area (ha)= 110.42 Curve Number (CN)= 57.4
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 1.954

PEAK FLOW (cms)= 5.467 (i)
TIME TO PEAK (hrs)= 11.250
RUNOFF VOLUME (mm)= 107.173
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.506

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0370) Area (ha)= 191.85 Curve Number (CN)= 80.3
NASHYD (0370) | Area (ha)= 191.85 Curve Number (CN)= 80.3
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.67

Unit Hyd Qpeak (cms)= 4.860

PEAK FLOW (cms)= 15.542 (i)
TIME TO PEAK (hrs)= 11.000
RUNOFF VOLUME (mm)= 156.193
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.737

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0368) Area (ha)= 159.48 Curve Number (CN)= 66.3
NASHYD (0368) | Area (ha)= 159.48 Curve Number (CN)= 66.3
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.12

Unit Hyd Qpeak (cms)= 2.433

PEAK FLOW (cms)= 8.386 (i)
TIME TO PEAK (hrs)= 11.500
RUNOFF VOLUME (mm)= 126.385
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.596

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0366) Area (ha)= 462.62 Curve Number (CN)= 79.1
NASHYD (0366) | Area (ha)= 462.62 Curve Number (CN)= 79.1
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.06

Unit Hyd Qpeak (cms)= 7.451

PEAK FLOW (cms)= 29.747 (i)
TIME TO PEAK (hrs)= 11.250
RUNOFF VOLUME (mm)= 154.856
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.730

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0364) Area (ha)= 155.27 Curve Number (CN)= 73.6
NASHYD (0364) | Area (ha)= 155.27 Curve Number (CN)= 73.6

ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.70

Unit Hyd Qpeak (cms)= 3.782

PEAK FLOW (cms)= 11.501 (i)
TIME TO PEAK (hrs)= 11.000
RUNOFF VOLUME (mm)= 141.255
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.666

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8302) | AREA QPEAK TPEAK R.V.
1 + 2 = 3 | (ha) (cms) (hrs) (mm)
ID= 1 (0364): 155.27 11.501 11.00 141.25
+ ID= 2 (0366): 462.62 29.747 11.25 154.86
=====

ID = 3 (8302): 617.89 41.038 11.00 151.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0362) Area (ha)= 118.78
STANDHYD (0362) | Area (ha)= 118.78
ID= 1 DT=15.0 min | Total Imp(%)= 22.00 Dir. Conn.(%)= 8.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 26.13 92.65
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 889.87 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten.(mm/hr)= 53.00 50.34
over (min) 15.00 30.00
Storage Coeff. (min)= 15.05 (ii) 29.13 (iii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.07 0.04

PEAK FLOW (cms)= 1.38 11.05 *TOTALS*
TIME TO PEAK (hrs)= 10.00 10.25 12.195 (iii)
RUNOFF VOLUME (mm)= 210.00 132.29 138.51
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.62 0.65

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.5 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0358) Area (ha)= 429.87 Curve Number (CN)= 55.3
NASHYD (0358) | Area (ha)= 429.87 Curve Number (CN)= 55.3
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.03

Unit Hyd Qpeak (cms)= 7.091

PEAK FLOW (cms)= 19.682 (i)
TIME TO PEAK (hrs)= 11.250
RUNOFF VOLUME (mm)= 102.916
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.485

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0360) Area (ha)= 138.37 Curve Number (CN)= 66.3
NASHYD (0360) | Area (ha)= 138.37 Curve Number (CN)= 66.3
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.60

Unit Hyd Qpeak (cms)= 3.957

PEAK FLOW (cms)= 9.965 (i)
TIME TO PEAK (hrs)= 11.000
RUNOFF VOLUME (mm)= 124.697
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.588

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8306) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0358): 429.87 19.682 11.25 102.92
+ ID2= 2 (0360): 138.37 9.965 11.00 124.70
-----
ID = 3 (8306): 568.24 29.364 11.00 108.22
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6362) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)= 15.00
-----

```

```

<----- DATA FOR SECTION (3621.0) ----->
Distance Elevation Manning
0.00 251.46 0.0550
27.86 254.23 0.0550
51.07 251.96 0.0550
74.29 250.77 0.0550
97.50 249.91 0.0550
125.36 249.40 0.0550
150.93 247.40 0.0550
155.93 247.33 0.0550 / 0.0350 Main Channel
157.93 246.85 0.0350 Main Channel
159.18 246.65 0.0350 Main Channel
160.18 246.63 0.0350 Main Channel
160.93 246.85 0.0350 Main Channel
161.93 247.18 0.0350 / 0.0550 Main Channel
163.18 248.03 0.0550
168.18 248.58 0.0550
183.18 250.18 0.0550
201.97 252.59 0.0550
213.57 256.02 0.0550
225.18 260.31 0.0550
229.82 261.00 0.0550

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.55 247.18 .556E+04 1.9 1.03 49.95
1.28 247.91 .336E+05 16.7 1.53 33.61
2.00 248.63 .913E+05 53.6 1.81 28.40
2.73 249.36 .185E+06 122.3 2.05 25.18
3.46 250.09 .344E+06 224.6 2.02 25.52
4.19 250.82 .574E+06 417.9 2.25 22.91
4.91 251.54 .855E+06 699.1 2.53 20.38
5.64 252.27 .118E+07 1075.3 2.82 18.26
6.37 253.00 .153E+07 1559.9 3.15 16.38
7.10 253.73 .191E+07 2137.6 3.46 14.89
7.82 254.45 .231E+07 2815.2 3.77 13.67
8.55 255.18 .272E+07 3608.5 4.10 12.58
9.28 255.91 .315E+07 4488.2 4.41 11.69
10.01 256.64 .358E+07 5458.0 4.71 10.95
10.73 257.36 .403E+07 6512.4 4.99 10.32
11.46 258.09 .449E+07 7649.8 5.27 9.78
12.19 258.82 .496E+07 8869.5 5.53 9.32
12.92 259.55 .544E+07 10171.2 5.78 8.91
13.64 260.27 .593E+07 11554.8 6.02 8.55

```

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8306) 568.24 29.36 11.00 108.22 1.53 1.62
OUTFLOW: ID= 1 (6362) 568.24 28.03 11.75 108.22 1.50 1.61

```

```

-----
| ADD HYD (8304) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0362): 118.78 12.195 10.25 138.53
+ ID2= 2 (6362): 568.24 28.029 11.75 108.22
-----
ID = 3 (8304): 687.02 36.680 11.00 113.46
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8300) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8302): 617.89 41.038 11.00 151.44
+ ID2= 2 (8304): 687.02 36.680 11.00 113.46
-----
ID = 3 (8300): 1304.91 77.718 11.00 131.44
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6368) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)= 15.00
-----

```

```

<----- DATA FOR SECTION (3681.0) ----->
Distance Elevation Manning
0.00 230.00 0.0370
18.48 223.26 0.0370
36.96 223.05 0.0370
64.67 222.94 0.0370
110.87 222.86 0.0370
133.96 222.74 0.0370
147.82 222.65 0.0370
170.92 222.31 0.0370
174.79 222.26 0.0370 / 0.0300 Main Channel
174.89 221.86 0.0300 Main Channel
175.54 221.86 0.0300 Main Channel
176.19 221.86 0.0300 Main Channel
176.29 222.26 0.0300 / 0.0370 Main Channel
180.16 222.25 0.0370
184.78 222.28 0.0370
189.40 222.31 0.0370
332.60 222.37 0.0370
450.00 230.00 0.0370

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.40 222.26 .771E+03 0.3 0.52 41.74
0.81 222.67 .784E+05 30.3 0.51 43.17
1.21 223.07 .210E+06 123.2 0.77 28.42
1.62 223.48 .384E+06 301.0 1.03 21.25
2.03 223.89 .563E+06 550.2 1.28 17.06
2.44 224.30 .747E+06 860.7 1.51 14.46
2.84 224.70 .934E+06 1228.1 1.73 12.68
3.25 225.11 1.13E+07 1649.3 1.92 11.37
3.66 225.52 1.32E+07 2122.3 2.11 10.37
4.07 225.93 1.52E+07 2645.5 2.29 9.58
4.47 226.33 1.72E+07 3217.8 2.45 8.92
4.88 226.74 1.93E+07 3838.3 2.61 8.38
5.29 227.15 2.14E+07 4506.3 2.76 7.92
5.70 227.56 2.36E+07 5221.3 2.91 7.52
6.10 227.96 2.58E+07 5983.0 3.05 7.17
6.51 228.37 2.80E+07 6790.9 3.19 6.87
6.92 228.78 3.03E+07 7645.0 3.32 6.60
7.33 229.19 3.26E+07 8545.1 3.45 6.35
7.73 229.59 3.49E+07 9491.0 3.57 6.13

```

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8300) 1304.91 77.72 11.00 131.44 1.02 0.61
OUTFLOW: ID= 1 (6368) 1304.91 73.77 11.50 131.44 1.00 0.60

```

```

-----
| ADD HYD (8298) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0368): 159.48 8.386 11.50 126.39
+ ID2= 2 (6368): 1304.91 73.772 11.50 131.44
-----
ID = 3 (8298): 1464.39 82.157 11.50 130.89
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8296) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0370): 191.85 15.542 11.00 156.19
+ ID2= 2 (8298): 1464.39 82.157 11.50 130.89
-----
ID = 3 (8296): 1656.24 96.562 11.50 133.82
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6372) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)= 15.00
-----

```

```

<----- DATA FOR SECTION (3721.0) ----->
Distance Elevation Manning
0.00 225.00 0.0390
30.80 219.38 0.0390
61.61 219.30 0.0390
77.01 219.27 0.0390
469.76 219.14 0.0390

```

477.46	219.13	0.0390	
485.16	219.10	0.0390	
492.86	219.09	0.0390	
495.56	219.09	0.0390 / 0.0310	Main Channel
495.66	218.51	0.0310	Main Channel
500.56	218.51	0.0310	Main Channel
505.46	218.51	0.0310	Main Channel
505.56	219.09	0.0310 / 0.0390	Main Channel
508.26	219.09	0.0390	
515.96	219.10	0.0390	
523.67	219.21	0.0390	
562.17	219.32	0.0390	
654.58	219.43	0.0390	
731.59	219.46	0.0390	
762.39	225.00	0.0390	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.29	218.80	431E+04	1.7	0.58	43.48
0.58	219.09	867E+04	5.1	0.89	28.32
0.93	219.44	200E+06	58.2	0.44	57.35
1.28	219.79	568E+06	278.6	0.74	33.99
1.62	220.13	939E+06	630.2	1.01	24.94
1.97	220.48	131E+07	1090.4	1.25	20.05
2.32	220.83	169E+07	1647.7	1.47	17.06
2.67	221.18	206E+07	2294.6	1.68	14.99
3.01	221.52	244E+07	3025.6	1.87	13.46
3.36	221.87	282E+07	3836.5	2.05	12.27
3.71	222.22	321E+07	4723.7	2.22	11.31
4.06	222.57	359E+07	5684.6	2.39	10.53
4.40	222.91	398E+07	6716.6	2.55	9.87
4.75	223.26	437E+07	7817.7	2.70	9.31
5.10	223.61	476E+07	8986.2	2.85	8.83
5.45	223.96	515E+07	10220.4	2.99	8.40
5.79	224.30	555E+07	11519.1	3.13	8.03
6.14	224.65	594E+07	12880.8	3.27	7.69
6.49	225.00	634E+07	14304.5	3.40	7.39

----- hydrograph ----- <-pipe / channel-->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8296)	1656.24	96.56	11.50	133.82	0.29	0.47
OUTFLOW : ID= 1 (6372)	1656.24	87.59	12.25	133.82	0.97	0.46

----- ADD HYD (8294) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0372)	110.42	5.467	11.25	107.17
+ ID2= 2 (6372)	1656.24	87.590	12.25	133.82
ID = 3 (8294)	1766.66	92.535	12.25	132.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ADD HYD (8292) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0374)	545.70	28.262	11.75	152.93
+ ID2= 2 (8294)	1766.66	92.535	12.25	132.16
ID = 3 (8292)	2312.36	120.216	12.25	137.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- RESERVOIR (5505) -----

IN= 2 --> OUT= 1	DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
		0.0000	0.0000	65.1290	345.3754
		25.4850	24.6597	84.9510	456.3890
		31.1490	98.6787	*****	838.7689
		39.6440	*****	*****	838.7789
		48.1390	*****	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8292)	2312.360	120.216	12.25	137.06
OUTFLOW : ID= 1 (5505)	2312.360	39.146	17.00	137.06

PEAK FLOW REDUCTION [Qout/Qin](%)= 32.56
 TIME SHIFT OF PEAK FLOW (min)=285.00
 MAXIMUM STORAGE USED (ha.m.)=168.3679

----- ADD HYD (8272) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0376)	463.85	32.317	11.25	174.75
+ ID2= 2 (5505)	2312.36	39.146	17.00	137.06
ID = 3 (8272)	2776.21	60.930	11.75	143.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- CALIB NASHVD (0396) -----

ID= 1 DT=15.0 min	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res.(N)
	305.21	5.00	84.1	1.50
			U.H. Tp(hrs)=	1.08

Unit Hyd Qpeak (cms)= 4.811

PEAK FLOW (cms)= 20.475 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 166.498
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.785

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB NASHVD (0394) -----

ID= 1 DT=15.0 min	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res.(N)
	325.45	5.00	71.5	1.50
			U.H. Tp(hrs)=	0.92

Unit Hyd Qpeak (cms)= 6.013

PEAK FLOW (cms)= 20.454 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 137.415
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.648

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB NASHVD (0390) -----

ID= 1 DT=15.0 min	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res.(N)
	420.00	5.00	73.6	1.50
			U.H. Tp(hrs)=	1.07

Unit Hyd Qpeak (cms)= 6.683

PEAK FLOW (cms)= 25.081 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 142.415
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.672

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB NASHVD (0388) -----

ID= 1 DT=15.0 min	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res.(N)
	220.77	5.00	76.3	1.50
			U.H. Tp(hrs)=	0.99

Unit Hyd Qpeak (cms)= 3.819

PEAK FLOW (cms)= 14.258 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 148.317
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.700

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB NASHVD (0386) -----

ID= 1 DT=15.0 min	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res.(N)
	241.27	5.00	78.0	1.50
			U.H. Tp(hrs)=	0.90

Unit Hyd Qpeak (cms)= 4.562

PEAK FLOW (cms)= 16.644 (i)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 151.953
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.717

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

79.39	279.30	0.0300	Main Channel
79.64	279.30	0.0300	Main Channel
79.79	279.64	0.0300 / 0.0380	Main Channel
89.98	279.78	0.0380	
119.09	281.86	0.0380	
145.55	282.87	0.0380	
198.48	284.85	0.0380	
211.71	286.31	0.0380	
230.23	287.59	0.0380	
261.99	294.00	0.0380	

```

-----
| ADD HYD (8286) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
ID1= 1 (0386): 241.27 16.644 11.00 151.95
+ ID2= 2 (0388): 220.77 14.258 11.25 148.32
=====
ID = 3 (8286): 462.04 30.850 11.00 150.22

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0384) | Area (ha)= 199.07 Curve Number (CN)= 64.1
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.96
-----

```

Unit Hyd Qpeak (cms) = 3.537

PEAK FLOW (cms) = 11.033 (i)
 TIME TO PEAK (hrs) = 11.250
 RUNOFF VOLUME (mm) = 121.359
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.572

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0380) | Area (ha)= 182.01 Curve Number (CN)= 59.9
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.55
-----

```

Unit Hyd Qpeak (cms) = 5.609

PEAK FLOW (cms) = 12.262 (i)
 TIME TO PEAK (hrs) = 11.000
 RUNOFF VOLUME (mm) = 110.871
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.523

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0382) | Area (ha)= 216.59 Curve Number (CN)= 71.5
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.64
-----

```

Unit Hyd Qpeak (cms) = 5.733

PEAK FLOW (cms) = 16.195 (i)
 TIME TO PEAK (hrs) = 11.000
 RUNOFF VOLUME (mm) = 136.296
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.643

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8290) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
ID1= 1 (0380): 182.01 12.262 11.00 110.87
+ ID2= 2 (0382): 216.59 16.195 11.00 136.30
=====
ID = 3 (8290): 398.60 28.457 11.00 124.69

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6384) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
-----

```

<----- DATA FOR SECTION (3841.0) ----->

Distance	Elevation	Manning
0.00	294.40	0.0380
10.59	291.93	0.0380
21.17	289.19	0.0380
26.46	287.99	0.0380
31.76	286.79	0.0380
71.45	279.97	0.0380
74.10	279.79	0.0380
76.74	279.71	0.0380
78.99	279.64	0.0380 / 0.0300
79.14	279.30	0.0300

```

-----
| TRAVEL TIME TABLE |
| DEPTH | ELEV | VOLUME | FLOW RATE | VELOCITY | TRAV TIME |
| (m) | (m) | (cu.m.) | (cms) | (m/s) | (min) |
0.34 279.64 .715E+03 0.2 0.85 63.42
1.10 280.40 .507E+05 21.9 1.40 38.54
1.85 281.15 .141E+06 91.8 2.10 25.69
2.61 281.91 .269E+06 218.9 2.63 20.46
3.36 282.66 .445E+06 407.1 2.96 18.23
4.12 283.42 .681E+06 698.3 3.31 16.26
4.87 284.17 .977E+06 1111.8 3.68 14.65
5.63 284.93 .133E+07 1674.3 4.06 13.27
6.39 285.69 .173E+07 2457.8 4.59 11.73
7.14 286.44 .215E+07 3378.8 5.07 10.63
7.90 287.20 .261E+07 4418.4 5.47 9.85
8.65 287.95 .310E+07 5676.3 5.92 9.11
9.41 288.71 .361E+07 7150.5 6.40 8.42
10.17 289.47 .414E+07 8783.1 6.86 7.86
10.92 290.22 .469E+07 10575.1 7.30 7.38
11.68 290.98 .525E+07 12520.4 7.72 6.98
12.43 291.73 .582E+07 14619.0 8.12 6.64
13.19 292.49 .642E+07 16863.2 8.50 6.34
13.94 293.24 .703E+07 19259.8 8.86 6.08

```

```

-----
| <---- hydrograph ----> | <-pipe / channel-> | | | | |
| AREA | QPEAK | TPEAK | R.V. | MAX DEPTH | MAX VEL |
| (ha) | (cms) | (hrs) | (mm) | (m) | (m/s) |
INFLOW : ID= 2 (8290) 398.60 28.46 11.00 124.69 1.17 1.44
OUTFLOW : ID= 1 (6384) 398.60 26.39 11.50 124.69 1.14 1.43

```

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-----
| ADD HYD (8288) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
ID1= 1 (0384): 199.07 11.033 11.25 121.36
+ ID2= 2 (6384): 398.60 26.395 11.50 124.69
=====
ID = 3 (8288): 597.67 37.357 11.25 123.58

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8284) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
ID1= 1 (8286): 462.04 30.850 11.00 150.22
+ ID2= 2 (8288): 597.67 37.357 11.25 123.58
=====
ID = 3 (8284): 1059.71 68.206 11.25 135.19

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| ADD HYD (8280) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
ID1= 1 (0390): 420.00 25.081 11.25 142.42
+ ID2= 2 (8284): 1059.71 68.206 11.25 135.19
=====
ID = 3 (8280): 1479.71 93.286 11.25 137.24

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0392) | Area (ha)= 167.22 Curve Number (CN)= 79.2
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.74
-----

```

Unit Hyd Qpeak (cms) = 3.837

PEAK FLOW (cms) = 12.849 (i)
 TIME TO PEAK (hrs) = 11.000
 RUNOFF VOLUME (mm) = 154.075
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.727

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHLID (0378) Area (ha) = 606.72 Curve Number (CN) = 74.3
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res.(N) = 1.50
U.H. Tp(hrs) = 1.18

Unit Hyd Qpeak (cms) = 8.771
PEAK FLOW (cms) = 34.617 (1)
TIME TO PEAK (hrs) = 11.500
RUNOFF VOLUME (mm) = 144.163
TOTAL RAINFALL (mm) = 212.000
RUNOFF COEFFICIENT = 0.680

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8282)
1 + 2 = 3
ID1= 1 (0378): 606.72 34.617 11.50 144.16
+ ID2= 2 (0392): 167.22 12.849 11.00 154.08
ID = 3 (8282): 773.94 47.183 11.25 146.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8278)
1 + 2 = 3
ID1= 1 (8280): 1479.71 93.286 11.25 137.24
+ ID2= 2 (8282): 773.94 47.183 11.25 146.30
ID = 3 (8278): 2253.65 140.470 11.25 140.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6394)
IN= 2--> OUT= 1 Routing time step (min)' = 15.00

Table with columns: Distance, Elevation, Manning. Rows show data for various distances from 0.00 to 333.60.

TRAVEL TIME TABLE with columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV.TIME. Rows show data for depths from 0.54 to 30.15.

hydrograph
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
INFLOW : ID= 2 (8278) 2253.65 140.47 11.25 140.35 2.61 2.82
OUTFLOW: ID= 1 (6394) 2253.65 138.80 11.50 140.35 2.60 2.81

ADD HYD (8276)
1 + 2 = 3
ID1= 1 (0394): 325.45 20.454 11.25 137.41
+ ID2= 2 (6394): 2253.65 138.801 11.50 140.35
ID = 3 (8276): 2579.10 158.950 11.50 139.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6396)
IN= 2--> OUT= 1 Routing time step (min)' = 15.00

DATA FOR SECTION (3961.0) with columns: Distance, Elevation, Manning. Rows show data for various distances from 0.00 to 290.82.

TRAVEL TIME TABLE with columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV.TIME. Rows show data for depths from 0.75 to 33.01.

hydrograph
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
INFLOW : ID= 2 (8276) 2579.10 158.95 11.50 139.98 2.60 1.80
OUTFLOW: ID= 1 (6396) 2579.10 155.83 12.00 139.98 2.58 1.80

ADD HYD (8274)
1 + 2 = 3
ID1= 1 (0396): 305.21 20.475 11.25 166.50
+ ID2= 2 (6396): 2579.10 155.832 12.00 139.98
ID = 3 (8274): 2884.31 175.498 11.75 142.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8270)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8272):	2776.21	60.930	11.75	143.36
+ ID2= 2 (8274):	2884.31	175.498	11.75	142.79
=====				
ID = 3 (8270):	5660.52	236.428	11.75	143.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5506)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min	0.0000	0.0000	60.8810	135.6832
	31.1490	24.6697	96.2770	900.4431
	36.8120	37.0045	96.6770	900.4531
	45.3070	86.3439	0.0000	0.0000

INFLOW : ID= 2 (8270)	AREA	QPEAK	TPEAK	R.V.
OUTFLOW: ID= 1 (5506)	(ha)	(cms)	(hrs)	(mm)
	5660.520	236.428	11.75	143.07
	5660.520	70.731	18.00	143.07

PEAK FLOW REDUCTION (Qout/Qin)(%)= 29.92
 TIME SHIFT OF PEAK FLOW (min)=375.00
 MAXIMUM STORAGE USED (ha.m.)=348.4974

CALIB	Area	(ha)	Curve Number	(CN)= 82.0
NASHYD (0406)	142.65			
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50	
	U.H. Tp(hrs)=	0.59		

Unit Hyd Qpeak (cms)= 4.135

PEAK FLOW (cms)= 12.335 (1)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 159.436
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.752

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 66.6
NASHYD (0404)	246.46			
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50	
	U.H. Tp(hrs)=	0.98		

Unit Hyd Qpeak (cms)= 4.280

PEAK FLOW (cms)= 14.014 (1)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 126.803
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.598

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 78.0
NASHYD (0402)	244.00			
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50	
	U.H. Tp(hrs)=	1.07		

Unit Hyd Qpeak (cms)= 3.879

PEAK FLOW (cms)= 15.381 (1)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 152.367
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.719

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 71.1
NASHYD (0400)	93.97			
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50	
	U.H. Tp(hrs)=	0.44		

Unit Hyd Qpeak (cms)= 3.630

PEAK FLOW (cms)= 7.874 (1)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 133.418
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.629

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)= 73.9
NASHYD (0398)	328.19			
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50	
	U.H. Tp(hrs)=	0.83		

Unit Hyd Qpeak (cms)= 6.759

PEAK FLOW (cms)= 22.535 (1)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 142.469
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.672

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8268)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0398):	328.19	22.535	11.00	142.47
+ ID2= 2 (0400):	93.97	7.874	11.00	133.42
=====				
ID = 3 (8268):	422.16	30.408	11.00	140.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6402)	Routing time step (min)=' 15.00
IN= 2--> OUT= 1	

Distance	Elevation	Manning
0.00	238.50	0.0360
11.50	238.00	0.0360
23.00	237.33	0.0360
34.49	236.39	0.0360
63.24	233.98	0.0360
97.73	228.15	0.0360
123.60	227.08	0.0360
126.48	226.61	0.0360
127.60	226.47	0.0360 / 0.0330
127.85	225.25	0.0330
129.35	225.25	0.0330
130.85	225.25	0.0330
131.10	226.47	0.0330 / 0.0360
132.22	226.59	0.0360
143.72	227.42	0.0360
169.59	227.88	0.0360
192.59	231.19	0.0360
218.46	233.02	0.0360
241.45	235.50	0.0360
284.57	236.43	0.0360

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.41	225.66	.391E+04	1.7	1.36	38.31
0.81	226.06	.804E+04	5.0	1.93	27.02
1.22	226.47	.124E+05	9.1	2.30	22.61
1.84	227.09	.308E+05	23.3	2.36	22.03
2.46	227.71	.904E+05	62.0	2.14	24.31
3.09	228.34	.223E+06	188.3	2.63	19.75
3.71	228.96	.379E+06	410.2	3.38	15.39
4.33	229.58	.550E+06	711.5	4.04	12.89
4.95	230.20	.737E+06	1091.3	4.62	11.25
5.58	230.83	.939E+06	1550.6	5.15	10.10
6.20	231.45	.116E+07	2072.4	5.59	9.31
6.82	232.07	.140E+07	2659.2	5.93	8.77
7.44	232.69	.167E+07	3345.8	6.27	8.30
8.07	233.32	.195E+07	4159.0	6.64	7.83
8.69	233.94	.226E+07	5100.8	7.03	7.39
9.31	234.56	.259E+07	6081.3	7.32	7.11
9.93	235.18	.295E+07	7183.1	7.60	6.84
10.56	235.81	.334E+07	8145.2	7.62	6.83
11.18	236.43	.379E+07	9116.0	7.51	6.92

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8268)	422.16	30.41	11.00	140.45	1.96
OUTFLOW: ID= 1 (6402)	422.16	28.99	11.50	140.45	1.93

ADD HYD (8266)

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0402):	244.00	15.381	11.25	152.37
+ ID2= 2 (6402):	422.16	28.995	11.50	140.45
=====				
ID = 3 (8266):	666.16	44.353	11.25	144.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8264)	1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0404):		246.46	14.014	11.25	126.80
+ ID2= 2 (8264):		666.16	44.353	11.25	144.82
=====					
ID = 3 (8264):		912.62	58.366	11.25	139.95

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8262)	1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0406):		142.65	12.335	11.00	159.44
+ ID2= 2 (8264):		912.62	58.366	11.25	139.95
=====					
ID = 3 (8262):		1055.27	70.165	11.25	142.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8260)	1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (5506):		5660.52	70.731	18.00	143.07
+ ID2= 2 (8260):		1055.27	70.165	11.25	142.59
=====					
ID = 3 (8260):		6715.79	131.675	11.25	142.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8258)	1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0408):		231.62	18.415	11.00	147.13
+ ID2= 2 (8260):		6715.79	131.675	11.25	142.99
=====					
ID = 3 (8258):		6947.41	149.479	11.25	143.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8256)	1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0410):		572.01	26.726	12.00	132.00
+ ID2= 2 (8256):		6947.41	149.479	11.25	143.13
=====					
ID = 3 (8256):		7519.42	175.654	11.25	142.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5507)	DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
		0.0000	0.0000	90.6140	67.8416
		39.6440	12.3348	*****	160.3529
		48.1390	18.5023	*****	160.3629
		67.9600	37.0045	0.0000	0.0000

INFLOW : ID= 2 (8256)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	7519.421	175.654	11.25	142.28

OUTFLOW: ID= 1 (5507)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	7519.421	118.016	14.25	142.28

PEAK FLOW REDUCTION [Qout/Qin](%)= 67.19
 TIME SHIFT OF PEAK FLOW (min)=180.00
 MAXIMUM STORAGE USED (ha.m.)=120.5190

CALIB	NASHYD (0420)	Area (ha)	(ha)= 175.82	Curve Number (CN)= 71.8
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50	
	U.H. Tp(hrs)=	0.81		

Unit Hyd Qpeak (cms)= 3.692

PEAK FLOW (cms)= 11.871 (i)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 137.745
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.650

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD (0418)	Area (ha)	(ha)= 182.79	Curve Number (CN)= 80.5
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50	
	U.H. Tp(hrs)=	1.05		

Unit Hyd Qpeak (cms)= 2.966

PEAK FLOW (cms)= 11.988 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 158.060
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.746

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD (0416)	Area (ha)	(ha)= 439.30	Curve Number (CN)= 81.0
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50	
	U.H. Tp(hrs)=	1.29		

Unit Hyd Qpeak (cms)= 5.832

PEAK FLOW (cms)= 25.927 (i)
 TIME TO PEAK (hrs)= 11.500
 RUNOFF VOLUME (mm)= 159.607
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.753

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD (0412)	Area (ha)	(ha)= 238.70	Curve Number (CN)= 73.4
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50	
	U.H. Tp(hrs)=	0.80		

Unit Hyd Qpeak (cms)= 5.088

PEAK FLOW (cms)= 16.572 (i)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 141.253
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.666

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (4141)	Area (ha)	(ha)= 43.70	Dir. Conn.(%)= 17.00
ID= 1 DT=15.0 min	Total Imp(%)=	36.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	15.73	27.97
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	539.75	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten.(mm/hr)= 53.00 65.12
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 11.15 (ii) 23.85 (iii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

TOTALS
 PEAK FLOW (cms)= 1.09 4.53 5.621 (iiii)
 TIME TO PEAK (hrs)= 10.00 10.00 10.00
 RUNOFF VOLUME (mm)= 210.00 171.31 177.89
 TOTAL RAINFALL (mm)= 212.00 212.00 212.00
 RUNOFF COEFFICIENT = 0.99 0.81 0.84

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 81.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (4142) | Area (ha)= 144.30
ID= 1 DT=15.0 min | Total Imp(%)= 21.00 Dir. Conn.(%)= 10.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	30.30	114.00
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	980.82	40.00
Mannings n =	0.013	0.250
Max.Eff.Inten.(mm/hr)=	53.00	56.49
over (min)=	15.00	30.00
Storage Coeff. (min)=	15.95 (ii)	29.40 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.07	0.04
PEAK FLOW (cms)=	2.09	15.44
TIME TO PEAK (hrs)=	10.00	10.25
RUNOFF VOLUME (mm)=	210.00	166.70
TOTAL RAINFALL (mm)=	212.00	212.00
RUNOFF COEFFICIENT =	0.99	0.79

TOTALS
17.281 (iii)

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 81.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8254) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (4141): 43.70 5.621 10.00 177.89
+ ID2= 2 (4142): 144.30 17.281 10.00 171.03
ID = 3 (8254): 188.00 22.901 10.00 172.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8252) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0412): 238.70 16.572 11.00 141.25
+ ID2= 2 (8254): 188.00 22.901 10.00 172.63
ID = 3 (8252): 426.70 36.423 10.25 155.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6416) |
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (4161.0) ----->

Distance	Elevation	Manning	
0.00	270.07	0.0340	
20.67	267.91	0.0340	
62.01	264.33	0.0340	
113.69	259.75	0.0340	
165.37	253.30	0.0340	
227.38	246.29	0.0340	
232.55	246.03	0.0340	
237.72	246.16	0.0340	
241.39	246.02	0.0340 / 0.0300	Main Channel
241.64	245.75	0.0300	Main Channel
242.89	245.75	0.0300	Main Channel
244.14	245.75	0.0300	Main Channel
244.39	246.02	0.0300 / 0.0340	Main Channel
248.06	246.20	0.0340	
253.22	246.28	0.0340	
258.39	246.63	0.0340	
346.25	252.57	0.0340	
413.43	257.77	0.0340	
465.11	261.78	0.0340	
511.62	270.00	0.0340	

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.27	246.02	.295E+04	0.6	0.75	88.26
1.53	247.28	.168E+06	69.7	1.65	40.13
2.79	248.54	.490E+06	299.5	2.42	27.28
4.06	249.81	.962E+06	740.1	3.05	21.67
5.32	251.07	.158E+07	1440.0	3.61	18.33
6.58	252.33	.235E+07	2443.8	4.12	16.06
7.84	253.59	.327E+07	3811.2	4.63	14.30
9.10	254.85	.432E+07	5565.1	5.11	12.94
10.37	256.12	.550E+07	7721.3	5.57	11.88
11.63	257.38	.682E+07	10309.9	6.00	11.02
12.89	258.64	.827E+07	13360.6	6.42	10.31
14.15	259.90	.985E+07	16884.5	6.81	9.72
15.42	261.17	.116E+08	20818.4	7.14	9.26
16.68	262.43	.134E+08	25510.4	7.53	8.78
17.94	263.69	.154E+08	30958.0	7.97	8.30
19.20	264.95	.175E+08	36988.3	8.38	7.89
20.46	266.21	.197E+08	43617.1	8.78	7.53
21.73	267.48	.220E+08	50866.1	9.17	7.22
22.99	268.74	.244E+08	58854.0	9.56	6.92

<---- hydrograph ----> <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8252)	426.70	36.42	10.25	155.08	0.92
OUTFLOW: ID= 1 (6416)	426.70	31.82	11.25	155.08	0.84

ADD HYD (8250) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0416): 439.30 25.927 11.50 159.61
+ ID2= 2 (6416): 426.70 31.818 11.25 155.08
ID = 3 (8250): 866.00 57.651 11.25 157.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8248) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0418): 182.79 11.988 11.25 158.06
+ ID2= 2 (8250): 866.00 57.651 11.25 157.37
ID = 3 (8248): 1048.79 69.639 11.25 157.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8246) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0420): 175.82 11.871 11.00 137.75
+ ID2= 2 (8248): 1048.79 69.639 11.25 157.49
ID = 3 (8246): 1224.61 81.403 11.25 154.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8244) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (5507): 7519.42 118.016 14.25 142.28
+ ID2= 2 (8246): 1224.61 81.403 11.25 154.66
ID = 3 (8244): 8744.03 181.100 12.00 144.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8242) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0422): 780.20 32.113 12.00 141.84
+ ID2= 2 (8244): 8744.03 181.100 12.00 144.02
ID = 3 (8242): 9524.23 213.214 12.00 143.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
RESERVOIR (5508)
IN= 2--> OUT= 1
DT= 15.0 min
-----

```

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	*****	197.3574
	76.4550	30.8371	*****	394.7148
	*****	61.6742	*****	394.7248
	*****	*****	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8242)	9524.231	213.214	12.00	143.84
OUTFLOW: ID= 1 (5508)	9524.231	162.331	15.50	143.84


```

PEAK FLOW REDUCTION [Qout/Qin](%)= 76.14
TIME SHIFT OF PEAK FLOW (min)=210.00
MAXIMUM STORAGE USED (ha.m.)=142.1958
-----

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```

-----
CALIB
NASHVD (0336)
ID= 1 DT=15.0 min
-----

```

Area	(ha)=	2785.00	Curve Number (CN)=	86.4
Ia	(mm)=	5.00	# of Linear Res.(N)=	1.50
U.H. Tp	(hrs)=	15.39		


```

Unit Hyd Qpeak (cms)= 3.090
PEAK FLOW (cms)= 20.993 (i)
TIME TO PEAK (hrs)= 24.000
RUNOFF VOLUME (mm)= 173.232
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.817
-----

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB
STANDHYD (3382)
ID= 1 DT=15.0 min
-----

```

Area	(ha)=	525.90	Dir. Conn.(%)=	17.00
Total Imp	(%)=	37.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	194.58	331.32
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1872.43	40.00
Mannings n =	0.013	0.250

Max.Eff.Inten.(mm/hr)=	53.00	66.50
over (min)	30.00	45.00
Storage Coeff. (min)=	23.52 (ii)	36.11 (ii)
Unit Hyd. Tpeak (min)=	30.00	45.00
Unit Hyd. peak (cms)=	0.04	0.03

	PEAK FLOW (cms)=	11.98	49.95	61.079 (iii)
TIME TO PEAK (hrs)=	10.00	10.50	10.25	
RUNOFF VOLUME (mm)=	210.00	173.23	179.48	
TOTAL RAINFALL (mm)=	212.00	212.00	212.00	
RUNOFF COEFFICIENT =	0.99	0.82	0.85	

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 82.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB
STANDHYD (3381)
ID= 1 DT=15.0 min
-----

```

Area	(ha)=	73.00	Dir. Conn.(%)=	23.00
Total Imp	(%)=	49.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	35.77	37.23
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	697.61	40.00
Mannings n =	0.013	0.250

Max.Eff.Inten.(mm/hr)=	53.00	76.96
over (min)	15.00	30.00
Storage Coeff. (min)=	13.00 (ii)	24.88 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04

	PEAK FLOW (cms)=	2.45	7.09	9.539 (iii)
TIME TO PEAK (hrs)=	10.00	10.00	10.00	
RUNOFF VOLUME (mm)=	210.00	177.49	184.97	
TOTAL RAINFALL (mm)=	212.00	212.00	212.00	
RUNOFF COEFFICIENT =	0.99	0.84	0.87	

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 82.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
ADD HYD (8310)
1 + 2 = 3
-----

```

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (3381):	73.00	9.539	10.00	184.97
+ ID= 2 (3382):	525.90	61.079	10.25	179.48
ID = 3 (8310):	598.90	70.136	10.25	180.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
CALIB
NASHVD (3342)
ID= 1 DT=15.0 min
-----

```

Area	(ha)=	587.10	Curve Number (CN)=	75.0
Ia	(mm)=	5.00	# of Linear Res.(N)=	1.50
U.H. Tp	(hrs)=	8.19		


```

Unit Hyd Qpeak (cms)= 1.224
PEAK FLOW (cms)= 6.972 (i)
TIME TO PEAK (hrs)= 17.250
RUNOFF VOLUME (mm)= 146.846
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.693
-----

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB
STANDHYD (3341)
ID= 1 DT=15.0 min
-----

```

Area	(ha)=	33.00	Dir. Conn.(%)=	31.00
Total Imp	(%)=	51.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	16.83	16.17
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	469.04	40.00
Mannings n =	0.013	0.250

Max.Eff.Inten.(mm/hr)=	53.00	66.54
over (min)	15.00	30.00
Storage Coeff. (min)=	10.25 (ii)	22.84 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.04

	PEAK FLOW (cms)=	1.50	2.67	4.174 (iii)
TIME TO PEAK (hrs)=	10.00	10.00	10.00	
RUNOFF VOLUME (mm)=	210.00	153.82	171.23	
TOTAL RAINFALL (mm)=	212.00	212.00	212.00	
RUNOFF COEFFICIENT =	0.99	0.73	0.81	

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
ADD HYD (8314)
1 + 2 = 3
-----

```

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (3341):	33.00	4.174	10.00	171.23
+ ID= 2 (3342):	587.10	6.972	17.25	146.85
ID = 3 (8314):	620.10	7.956	11.00	148.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
CALIB
NASHVD (0306)
ID= 1 DT=15.0 min
-----

```

Area	(ha)=	283.97	Curve Number (CN)=	71.2
Ia	(mm)=	5.00	# of Linear Res.(N)=	1.50
U.H. Tp	(hrs)=	6.44		


```

Unit Hyd Qpeak (cms)= 0.753
PEAK FLOW (cms)= 4.007 (i)
-----

```

TIME TO PEAK (hrs)= 15.750
 RUNOFF VOLUME (mm)= 138.250
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.652

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHVD (0286) | Area (ha)= 260.51 | Curve Number (CN)= 93.4
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.16

Unit Hyd Qpeak (cms)= 3.834

PEAK FLOW (cms)= 18.264 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 188.924
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.891

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHVD (0282) | Area (ha)= 449.38 | Curve Number (CN)= 89.1
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.47

Unit Hyd Qpeak (cms)= 5.226

PEAK FLOW (cms)= 26.730 (i)
 TIME TO PEAK (hrs)= 11.750
 RUNOFF VOLUME (mm)= 178.945
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.844

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHVD (0284) | Area (ha)= 78.93 | Curve Number (CN)= 93.3
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.57

Unit Hyd Qpeak (cms)= 2.344

PEAK FLOW (cms)= 7.418 (i)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 185.839
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.877

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8388) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 ID1= 1 (0282): 449.38 26.730 11.75 178.94
 + ID2= 2 (0284): 78.93 7.418 11.00 185.84
 ID = 3 (8388): 528.31 33.585 11.25 179.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6286) |
 IN= 2 -> OUT= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (2861.0) -----

Distance	Elevation	Manning
0.00	233.00	0.0450
20.58	228.51	0.0450
41.17	227.74	0.0450
51.46	227.41	0.0450
97.77	225.96	0.0450
149.23	223.94	0.0450
200.69	220.84	0.0450
226.42	220.66	0.0450
238.85	220.22	0.0450
241.35	220.01	0.0450 / 0.0350 Main Channel
241.85	219.70	0.0350 Main Channel
245.85	219.72	0.0350 Main Channel
246.35	220.06	0.0350 Main Channel
248.85	220.23	0.0350 Main Channel
303.60	221.64	0.0350 Main Channel
380.79	224.98	0.0450
432.25	229.54	0.0450

457.98 233.33 0.0450
 483.71 234.27 0.0450
 509.44 233.81 0.0450

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.31	220.01	382E+04	0.8	0.57	82.89
1.06	220.76	525E+05	12.1	0.66	72.51
1.81	221.51	240E+06	88.6	1.05	45.21
2.56	222.26	510E+06	261.7	1.46	32.47
3.31	223.01	843E+06	533.8	1.81	26.34
4.06	223.76	124E+07	933.6	2.10	22.64
4.81	224.51	171E+07	1399.0	2.34	20.33
5.56	225.26	225E+07	2051.0	2.60	18.27
6.31	226.01	285E+07	2907.0	2.91	16.35
7.06	226.76	352E+07	3903.5	3.16	15.04
7.82	227.52	426E+07	5069.9	3.39	14.01
8.57	228.27	507E+07	6433.0	3.62	13.13
9.32	229.02	593E+07	8069.2	3.89	12.24
10.07	229.77	681E+07	9926.7	4.16	11.44
10.82	230.52	772E+07	11962.8	4.42	10.75
11.57	231.27	864E+07	14167.1	4.68	10.17
12.32	232.02	959E+07	16537.1	4.92	9.66
13.07	232.77	105E+08	19070.7	5.16	9.22
13.82	233.52	115E+08	21790.4	5.39	8.82

----- hydrograph -----<- pipe / channel ->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW: ID= 2 (8388) 528.31 33.59 11.25 179.97 1.27 0.73
 OUTFLOW: ID= 1 (6286) 528.31 30.63 12.25 179.97 1.24 0.72

ADD HYD (8386) |
 1 2 = 3 | AREA QPEAK TPEAK R.V.
 ID1= 1 (0286): 260.51 18.264 11.25 188.92
 + ID2= 2 (6286): 528.31 30.634 12.25 179.97
 ID = 3 (8386): 788.82 48.029 12.00 182.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHVD (0302) | Area (ha)= 473.90 | Curve Number (CN)= 75.7
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.66

Unit Hyd Qpeak (cms)= 4.874

PEAK FLOW (cms)= 22.555 (i)
 TIME TO PEAK (hrs)= 12.000
 RUNOFF VOLUME (mm)= 147.792
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.697

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHVD (0300) | Area (ha)= 258.93 | Curve Number (CN)= 70.8
 ID= 1 DT=15.0 min | Ia (mm)= 4.00 | # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.03

Unit Hyd Qpeak (cms)= 4.292

PEAK FLOW (cms)= 15.281 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 136.980
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.646

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHVD (0298) | Area (ha)= 330.51 | Curve Number (CN)= 64.6
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.26

Unit Hyd Qpeak (cms)= 4.461

PEAK FLOW (cms)= 15.779 (i)
 TIME TO PEAK (hrs)= 11.500
 RUNOFF VOLUME (mm)= 122.883
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.580

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8395) |
| 1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
-----
| ID1= 1 (0298): 330.51 15.779 11.50 122.88
+ ID2= 2 (0300): 258.93 15.281 11.25 136.98
=====
| ID = 3 (8395): 589.44 30.954 11.50 129.08
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB (0288) | Area (ha)= 340.83 Curve Number (CN)= 89.9
| NASHYD (0288) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 2.21
-----

```

Unit Hyd Qpeak (cms)= 2.629

```

PEAK FLOW (cms)= 15.781 (1)
TIME TO PEAK (hrs)= 12.000
RUNOFF VOLUME (mm)= 161.352
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.855

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0290) | Area (ha)= 269.18 Curve Number (CN)= 90.9
| NASHYD (0290) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 2.07
-----

```

Unit Hyd Qpeak (cms)= 4.279

```

PEAK FLOW (cms)= 19.260 (1)
TIME TO PEAK (hrs)= 11.250
RUNOFF VOLUME (mm)= 182.662
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.862

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8397) |
| 1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
-----
| ID1= 1 (0288): 340.83 15.781 12.00 181.35
+ ID2= 2 (0290): 269.18 19.260 11.25 182.66
=====
| ID = 3 (8397): 610.01 34.406 11.50 181.93
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB (0296) | Area (ha)= 293.65 Curve Number (CN)= 88.9
| NASHYD (0296) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.13
-----

```

Unit Hyd Qpeak (cms)= 4.437

```

PEAK FLOW (cms)= 20.151 (1)
TIME TO PEAK (hrs)= 11.250
RUNOFF VOLUME (mm)= 177.972
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.839

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0292) | Area (ha)= 738.49 Curve Number (CN)= 83.9
| NASHYD (0292) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.52
-----

```

Unit Hyd Qpeak (cms)= 8.289

```

PEAK FLOW (cms)= 40.850 (1)
TIME TO PEAK (hrs)= 11.750
RUNOFF VOLUME (mm)= 166.632
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.786

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0294) | Area (ha)= 274.15 Curve Number (CN)= 75.3
| NASHYD (0294) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.87
-----

```

Unit Hyd Qpeak (cms)= 5.367

```

PEAK FLOW (cms)= 18.657 (1)
TIME TO PEAK (hrs)= 11.000
RUNOFF VOLUME (mm)= 145.749
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.687

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8398) |
| 1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
-----
| ID1= 1 (0292): 738.49 40.850 11.75 166.63
+ ID2= 2 (0294): 274.15 18.657 11.00 145.75
=====
| ID = 3 (8398): 1012.64 58.892 11.50 160.98
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CEN (8296) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
-----

```

<----- DATA FOR SECTION (2961.0) ----->

Distance	Elevation	Manning
0.00	243.98	0.0400
42.59	243.18	0.0400
85.17	241.81	0.0400
120.66	240.50	0.0400
156.15	239.56	0.0400
198.74	236.15	0.0400
237.78	234.01	0.0400
241.33	233.82	0.0400
248.77	233.12	0.0400 /0.0400
249.87	232.32	0.0400
250.37	231.80	0.0400
250.87	232.23	0.0400
251.97	233.10	0.0400 /0.0400
255.37	233.22	0.0400
259.07	233.87	0.0400
262.62	234.12	0.0400
266.17	234.23	0.0400
283.91	234.73	0.0400
337.15	241.75	0.0400
351.34	244.00	0.0400

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.43	232.23	.510E+03	0.1	0.53	81.07
0.87	232.67	.213E+04	0.7	0.83	51.49
1.30	233.10	.501E+04	2.2	1.10	38.75
1.98	233.78	.242E+05	12.6	1.34	31.93
2.66	234.46	.738E+05	42.1	1.46	29.21
3.34	235.14	.177E+06	127.9	1.64	23.13
4.02	235.82	.314E+06	281.4	2.29	18.61
4.70	236.50	.481E+06	509.4	2.71	15.73
5.38	237.18	.672E+06	815.6	3.11	13.73
6.06	237.86	.895E+06	1200.7	3.46	12.30
6.74	238.54	.112E+07	1669.4	3.80	11.23
7.42	239.22	.139E+07	2226.9	4.11	10.38
8.10	239.90	.168E+07	2808.4	4.28	9.95
8.78	240.58	.202E+07	3470.4	4.40	9.68
9.46	241.26	.240E+07	4331.7	4.61	9.24
10.14	241.94	.283E+07	5335.2	4.83	8.84
10.82	242.62	.330E+07	6492.6	5.03	8.48
11.50	243.30	.381E+07	7764.1	5.21	8.18
12.18	243.98	.439E+07	9101.2	5.31	8.04

<---- hydrograph ----> <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8398)	1012.64	58.89	11.50	160.98	2.79	1.52
OUTFLOW: ID= 1 (8296)	1012.64	57.83	12.00	160.98	2.78	1.52

```

-----
| ADD HYD (8396) |
| 1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
-----

```

```

-----
      (ha)   (cms)  (hrs)  (mm)
ID1= 1 (0296): 293.65 20.151 11.25 177.97
+ ID2= 2 (6296): 1012.64 57.830 12.00 160.98
=====
ID = 3 (8396): 1306.29 77.347 11.75 164.80
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8394) |
| 1 + 2 = 3 |
      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)  (hrs)  (mm)
ID1= 1 (8396): 1306.29 77.347 11.75 164.80
+ ID2= 2 (8397): 610.01 34.406 11.50 181.93
=====
ID = 3 (8394): 1916.30 111.656 11.75 170.25
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8392) |
| 1 + 2 = 3 |
      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)  (hrs)  (mm)
ID1= 1 (8394): 1916.30 111.656 11.75 170.25
+ ID2= 2 (8395): 589.44 30.954 11.50 129.08
=====
ID = 3 (8392): 2505.74 142.366 11.75 160.57
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6302) |
| IN= 2--> OUT= 1 |
      Routing time step (min) = 15.00
-----

```

```

-----
<----- DATA FOR SECTION (3021.0) ----->
Distance   Elevation   Manning
-----
0.00       228.10       0.0400
18.47     227.12       0.0400
36.95     226.12       0.0400
46.18     225.84       0.0400
55.42     225.58       0.0400
272.47    222.88       0.0400
277.09    222.76       0.0400
281.71    222.58       0.0400
288.54    222.18       0.0400 / 0.0300 Main Channel
288.64    221.00       0.0300 Main Channel
290.94    221.00       0.0300 Main Channel
291.04    221.00       0.0300 Main Channel
291.54    222.75       0.0300 / 0.0400 Main Channel
300.18    222.83       0.0400
304.80    223.04       0.0400
309.42    223.25       0.0400
318.65    223.69       0.0400
360.22    225.57       0.0400
397.16    227.60       0.0400
457.20    228.35       0.0400
-----

```

```

-----
<----- TRAVEL TIME TABLE ----->
DEPTH   ELEV   VOLUME   FLOW RATE   VELOCITY   TRAV.TIME
(m)     (m)     (cu.m.)  (cms)        (m/s)      (min)
-----
0.29    221.29  .228E+04  0.2          0.32       165.63
0.59    221.59  .465E+04  0.7          0.45       116.15
0.88    221.88  .713E+04  1.2          0.54       96.87
1.18    222.18  .971E+04  1.9          0.61       86.20
1.57    222.57  .175E+05  3.3          0.59       88.86
1.97    222.97  .427E+05  6.4          0.47       111.39
2.36    223.36  .115E+06  16.1         0.44       119.14
2.76    223.76  .237E+06  37.1         0.49       106.49
3.15    224.15  .409E+06  72.9         0.56       93.40
3.55    224.55  .631E+06  127.0        0.63       82.76
3.94    224.94  .903E+06  202.4        0.70       74.37
4.34    225.34  .123E+07  301.8        0.77       67.68
4.73    225.73  .160E+07  433.5        0.85       61.37
5.13    226.13  .200E+07  602.2        0.95       55.29
5.52    226.52  .242E+07  804.7        1.05       50.13
5.92    226.92  .286E+07  1034.2       1.14       46.11
6.31    227.31  .332E+07  1290.5       1.22       42.88
6.71    227.71  .380E+07  1560.9       1.29       40.55
7.10    228.10  .432E+07  1835.6       1.34       39.19
-----

```

```

-----
      AREA   QPEAK   TPEAK   R.V.   <-pipe / channel->
      (ha)   (cms)  (hrs)  (mm)   (m)   (m/s)
INFLOW : ID= 2 (8392) 2505.74 142.37 11.75 160.57 3.63 0.65
OUTFLOW : ID= 1 (6302) 2505.74 125.22 12.50 160.57 3.53 0.63
-----

```

```

-----
| ADD HYD (8390) |
| 1 + 2 = 3 |
      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)  (hrs)  (mm)
ID1= 1 (0302): 473.90 22.555 12.00 147.79
+ ID2= 2 (6302): 2505.74 125.217 12.50 160.57
=====
ID = 3 (8390): 2979.64 146.953 12.50 158.53
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8348) |
| 1 + 2 = 3 |
      AREA   QPEAK   TPEAK   R.V.
      (ha)   (cms)  (hrs)  (mm)
ID1= 1 (8388): 788.82 48.039 12.00 182.93
+ ID2= 2 (8390): 2979.64 146.953 12.50 158.53
=====
ID = 3 (8348): 3768.46 194.008 12.25 163.64
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB
| NASHYD (0304) | Area (ha)= 292.37 Curve Number (CN)= 80.3
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 2.78
-----

```

Unit Hyd Qpeak (cms) = 1.793

```

PEAK FLOW (cms) = 10.078 (i)
TIME TO PEAK (hrs) = 12.500
RUNOFF VOLUME (mm) = 158.751
TOTAL RAINFALL (mm) = 212.000
RUNOFF COEFFICIENT = 0.749

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB
| NASHYD (0280) | Area (ha)= 299.86 Curve Number (CN)= 92.3
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.85
-----

```

Unit Hyd Qpeak (cms) = 6.009

```

PEAK FLOW (cms) = 24.229 (i)
TIME TO PEAK (hrs) = 11.000
RUNOFF VOLUME (mm) = 185.349
TOTAL RAINFALL (mm) = 212.000
RUNOFF COEFFICIENT = 0.874

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB
| NASHYD (0278) | Area (ha)= 485.49 Curve Number (CN)= 92.3
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.52
-----

```

Unit Hyd Qpeak (cms) = 5.453

```

PEAK FLOW (cms) = 29.133 (i)
TIME TO PEAK (hrs) = 11.750
RUNOFF VOLUME (mm) = 186.750
TOTAL RAINFALL (mm) = 212.000
RUNOFF COEFFICIENT = 0.881

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB
| NASHYD (0276) | Area (ha)= 90.89 Curve Number (CN)= 90.9
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.67
-----

```

Unit Hyd Qpeak (cms) = 2.302

```

PEAK FLOW (cms) = 8.011 (i)
TIME TO PEAK (hrs) = 11.000
RUNOFF VOLUME (mm) = 180.981
TOTAL RAINFALL (mm) = 212.000
RUNOFF COEFFICIENT = 0.854

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB
| NASHYD (0274) | Area (ha)= 392.49 Curve Number (CN)= 88.3
-----

```

|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.08

Unit Hyd Qpeak (cms) = 6.182

PEAK FLOW (cms) = 27.372 (i)
 TIME TO PEAK (hrs) = 11.250
 RUNOFF VOLUME (mm) = 176.439
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.832

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | ADD HYD (8360) |
 | 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0274): 392.49 27.372 11.25 176.44
 + ID2= 2 (0276): 90.89 8.011 11.00 180.98
 =====
 ID = 3 (8360): 483.38 35.253 11.00 177.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | ADD HYD (8358) |
 | 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0278): 485.49 29.133 11.75 186.75
 + ID2= 2 (8360): 483.38 35.253 11.00 177.29
 =====
 ID = 3 (8358): 968.87 64.013 11.25 182.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | ROUTE CHN (6280) |
 | ID= 2--> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2801.0) ----->

Distance	Elevation	Manning
0.00	241.14	0.0500
13.32	240.80	0.0500
39.95	240.07	0.0500
96.54	236.21	0.0500
113.19	234.15	0.0500
123.18	232.35	0.0500
143.15	225.80	0.0500
149.81	225.62	0.0500
153.14	225.40	0.0500
157.30	224.76	0.0500 /0.0300 Main Channel
159.80	224.26	0.0300 Main Channel
162.30	224.85	0.0300 Main Channel
162.55	224.97	0.0300 /0.0500 Main Channel
167.80	225.05	0.0500
186.43	229.14	0.0500
213.06	234.75	0.0500
236.37	237.09	0.0500
266.33	237.31	0.0500
292.96	237.83	0.0500
329.58	241.50	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.50	224.76	.263E+04	0.8	0.65	58.31
1.36	225.62	.278E+05	13.4	1.10	34.54
2.22	226.48	.855E+05	52.6	1.40	27.09
3.09	227.35	.157E+06	120.4	1.74	21.78
3.95	228.21	.242E+06	216.3	2.04	18.64
4.81	229.07	.340E+06	341.8	2.29	16.56
5.67	229.93	.450E+06	498.3	2.52	15.06
6.53	230.79	.574E+06	688.3	2.73	13.90
7.40	231.66	.711E+06	913.8	2.93	12.97
8.26	232.52	.861E+06	1173.4	3.11	12.23
9.12	233.38	.103E+07	1461.3	3.24	11.72
9.98	234.24	.121E+07	1793.3	3.37	11.26
10.85	235.11	.142E+07	2141.0	3.45	11.02
11.71	235.97	.165E+07	2530.4	3.50	10.86
12.57	236.83	.192E+07	2954.0	3.51	10.81
13.43	237.69	.226E+07	3160.3	3.18	11.93
14.29	238.55	.271E+07	3869.4	3.26	11.67
15.16	239.42	.320E+07	4740.3	3.38	11.24
16.02	240.28	.373E+07	5688.4	3.48	10.92

<----- hydrograph ----> <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8358)	968.87	64.01	11.25	182.03	2.37	1.45
OUTFLOW: ID= 1 (6280)	968.87	62.71	11.75	182.03	2.35	1.44

 | ADD HYD (8354) |
 | 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0280): 299.86 24.228 11.00 185.35
 + ID2= 2 (6280): 968.87 62.714 11.75 182.03
 =====
 ID = 3 (8354): 1268.73 85.722 11.50 182.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | CALIB (0272) |
 | NASHVD (0272) | Area (ha)= 157.38 Curve Number (CN)= 88.3
 |ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.09

Unit Hyd Qpeak (cms) = 2.456

PEAK FLOW (cms) = 10.925 (i)
 TIME TO PEAK (hrs) = 11.250
 RUNOFF VOLUME (mm) = 176.461
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.832

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | CALIB (0270) |
 | NASHVD (0270) | Area (ha)= 243.61 Curve Number (CN)= 91.6
 |ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 0.87

Unit Hyd Qpeak (cms) = 3.429

PEAK FLOW (cms) = 16.128 (i)
 TIME TO PEAK (hrs) = 11.000
 RUNOFF VOLUME (mm) = 183.560
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.866

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | CALIB (0268) |
 | NASHVD (0268) | Area (ha)= 215.76 Curve Number (CN)= 88.3
 |ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 0.69

Unit Hyd Qpeak (cms) = 3.807

PEAK FLOW (cms) = 15.631 (i)
 TIME TO PEAK (hrs) = 11.000
 RUNOFF VOLUME (mm) = 174.848
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.825

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | CALIB (0264) |
 | NASHVD (0264) | Area (ha)= 353.96 Curve Number (CN)= 84.1
 |ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.30

Unit Hyd Qpeak (cms) = 3.313

PEAK FLOW (cms) = 17.065 (i)
 TIME TO PEAK (hrs) = 11.750
 RUNOFF VOLUME (mm) = 166.685
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.786

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | CALIB (0266) |
 | NASHVD (0266) | Area (ha)= 508.09 Curve Number (CN)= 81.1
 |ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.63

Unit Hyd Qpeak (cms) = 3.786

PEAK FLOW (cms) = 20.326 (i)
 TIME TO PEAK (hrs) = 12.000
 RUNOFF VOLUME (mm) = 160.016

TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.755

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8380)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1 = 1 (0264):	353.96	17.065	11.75	166.69
+ ID2 = 2 (0265):	508.09	20.326	12.00	160.02
ID = 3 (8380):	862.05	37.389	12.00	162.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6268)	ROUTING TIME STEP (min)
IN= 2 -> OUT= 1	15.00

DATA FOR SECTION (2681.0) ----->

Distance (m)	Elevation (m)	Manning (cu.m.)	Channel
0.00	274.03	0.0500	
10.55	272.73	0.0500	
21.10	271.28	0.0500	
50.12	266.83	0.0500	
87.04	260.36	0.0500	
92.32	260.06	0.0500	
94.95	259.93	0.0500	
101.00	256.87	0.0500	
103.00	256.58	0.0500 / 0.0350	Main Channel
104.00	256.32	0.0350	Main Channel
106.00	256.05	0.0350	Main Channel
108.00	256.25	0.0350	Main Channel
110.78	256.65	0.0350 / 0.0500	Main Channel
113.42	256.81	0.0500	
116.06	257.00	0.0500	
155.62	260.24	0.0500	
187.27	263.35	0.0500	
211.01	267.22	0.0500	
224.20	269.60	0.0500	
261.13	272.68	0.0500	

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.53	256.58	697E+04	2.1	0.97	55.64
1.38	257.43	477E+05	25.9	1.76	30.66
2.22	258.27	123E+06	84.3	2.22	24.26
3.07	259.12	231E+06	185.4	2.60	20.72
3.92	259.97	371E+06	335.8	2.92	18.42
4.77	260.82	560E+06	547.6	3.15	17.06
5.61	261.66	790E+06	845.6	3.46	15.57
6.46	262.51	106E+07	1225.0	3.75	14.37
7.31	263.36	136E+07	1693.3	4.02	13.37
8.16	264.21	169E+07	2283.2	4.35	12.36
9.00	265.05	206E+07	2969.1	4.66	11.54
9.85	265.90	245E+07	3754.4	4.96	10.86
10.70	266.75	286E+07	4643.0	5.24	10.28
11.55	267.60	331E+07	5636.1	5.50	9.78
12.39	268.44	378E+07	6746.2	5.76	9.34
13.24	269.29	428E+07	7972.2	6.01	8.96
14.09	270.14	482E+07	9218.9	6.18	8.71
14.94	270.99	539E+07	10550.5	6.32	8.52
15.78	271.83	601E+07	12024.7	6.46	8.33

INFLOW	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
ID= 2 (8380)	862.05	37.39	12.00	162.75	1.54	1.83
OUTFLOW: ID= 1 (6268)	862.05	36.80	12.25	162.75	1.53	1.82

ADD HYD (8382)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1 = 1 (0268):	215.76	15.631	11.00	174.85
+ ID2 = 2 (6268):	862.05	36.804	12.25	162.75
ID = 3 (8382):	1077.81	50.727	12.00	165.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6270)	ROUTING TIME STEP (min)
IN= 2 -> OUT= 1	15.00

DATA FOR SECTION (2701.0) ----->

Distance	Elevation	Manning	Channel
0.00	245.89	0.0500	
8.14	245.66	0.0500	
16.28	245.16	0.0500	
20.35	244.84	0.0500	
38.66	242.98	0.0500	
48.83	240.65	0.0500	
63.07	235.91	0.0500	
65.11	235.18	0.0500	
68.43	234.34	0.0500 / 0.0300	Main Channel
68.68	233.95	0.0300	Main Channel
69.18	233.89	0.0300	Main Channel
69.68	233.95	0.0300	Main Channel
71.21	234.48	0.0300 / 0.0500	Main Channel
81.38	236.44	0.0500	
95.63	236.66	0.0500	
120.04	237.00	0.0500	
148.53	241.77	0.0500	
158.70	242.34	0.0500	
187.18	244.03	0.0500	
201.42	244.36	0.0500	

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.45	234.34	217E+04	0.7	0.99	52.83
0.98	234.87	966E+04	4.6	1.49	35.20
1.50	235.39	250E+05	13.7	1.73	30.41
2.03	235.92	475E+05	29.5	1.95	26.87
2.56	236.45	772E+05	52.5	2.14	24.50
3.09	236.98	142E+06	82.4	1.83	28.73
3.61	237.50	246E+06	158.1	2.03	25.91
4.14	238.03	357E+06	262.0	2.31	22.73
4.67	238.56	477E+06	392.1	2.59	20.27
5.20	239.09	604E+06	547.8	2.86	18.38
5.72	239.61	739E+06	729.2	3.11	16.90
6.25	240.14	882E+06	936.5	3.34	15.70
6.78	240.67	103E+07	1169.7	3.57	14.72
7.31	241.20	119E+07	1425.9	3.77	13.94
7.83	241.72	136E+07	1710.4	3.96	13.26
8.36	242.25	154E+07	1967.9	4.02	13.07
8.89	242.78	174E+07	2264.9	4.09	12.83
9.42	243.31	196E+07	2589.3	4.15	12.65
9.94	243.83	221E+07	2953.5	4.22	12.46

INFLOW	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
ID= 2 (8382)	1077.81	50.73	12.00	165.18	2.52	2.13
OUTFLOW: ID= 1 (6270)	1077.81	50.21	12.25	165.18	2.50	2.12

ADD HYD (8384)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1 = 1 (0270):	243.61	16.128	11.00	183.56
+ ID2 = 2 (6270):	1077.81	50.214	12.25	165.18
ID = 3 (8384):	1321.42	65.392	12.00	168.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0262)	341.31	92.3
ID= 1 DT=15.0 min	5.00	# of Linear Res. (N)= 1.30
U.H. Tp (hrs)	1.01	
Unit Hyd Qpeak (cms)	4.094	

PEAK FLOW (cms)= 20.776 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 185.722
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.876

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0260)	476.24	92.3
ID= 1 DT=15.0 min	5.00	# of Linear Res. (N)= 1.30
U.H. Tp (hrs)	1.33	
Unit Hyd Qpeak (cms)	4.360	

PEAK FLOW (cms)= 24.646 (i)
 TIME TO PEAK (hrs)= 11.750
 RUNOFF VOLUME (mm)= 186.327
 TOTAL RAINFALL (mm)= 212.000

RUNOFF COEFFICIENT = 0.879

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

207.28	271.95	0.0400
233.55	273.72	0.0400
256.91	274.98	0.0400
289.02	275.97	0.0400

CALIB (0258) | Area (ha)= 181.99 | Curve Number (CN)= 91.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.18

Unit Hyd Qpeak (cms)= 1.881

PEAK FLOW (cms)= 10.024 (i)
 TIME TO PEAK (hrs)= 11.500
 RUNOFF VOLUME (mm)= 182.929
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.863

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0252) | Area (ha)= 319.99 | Curve Number (CN)= 86.9
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.04

Unit Hyd Qpeak (cms)= 3.761

PEAK FLOW (cms)= 18.292 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 172.824
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.815

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0256) | Area (ha)= 145.79 | Curve Number (CN)= 82.9
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.04

Unit Hyd Qpeak (cms)= 1.707

PEAK FLOW (cms)= 7.959 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 163.436
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.771

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0254) | Area (ha)= 403.00 | Curve Number (CN)= 74.4
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.22

Unit Hyd Qpeak (cms)= 4.028

PEAK FLOW (cms)= 17.953 (i)
 TIME TO PEAK (hrs)= 11.750
 RUNOFF VOLUME (mm)= 144.288
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.681

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6256) | Routing time step (min)'= 15.00
 IN= 2--> OUT= 1

Distance	Elevation	Manning
0.00	276.07	0.0400
11.68	273.71	0.0400
23.36	271.35	0.0400
29.19	270.30	0.0400
35.03	269.44	0.0400
55.47	267.90	0.0400
78.82	266.24	0.0400
90.50	265.63	0.0400
102.18	265.40	0.0400
105.10	264.95	0.0400 / 0.0350 Main Channel
108.02	264.39	0.0350 Main Channel
110.94	264.72	0.0350 Main Channel
113.86	265.19	0.0350 / 0.0400 Main Channel
116.78	265.49	0.0400
143.05	268.24	0.0400
172.25	270.53	0.0400

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.56	264.95	723E+04	1.9	0.90	62.51
1.14	265.53	307E+05	13.5	1.49	37.92
1.72	266.11	951E+05	48.6	1.73	32.62
2.30	266.69	191E+06	117.8	2.09	27.03
2.88	267.27	314E+06	225.2	2.43	23.24
3.46	267.85	464E+06	375.5	2.74	20.59
4.04	268.43	641E+06	573.2	3.03	18.62
4.62	269.01	846E+06	821.9	3.29	17.15
5.20	269.59	108E+07	1135.0	3.56	15.87
5.78	270.17	134E+07	1525.1	3.86	14.64
6.36	270.75	162E+07	1971.1	4.12	13.71
6.94	271.33	194E+07	2474.0	4.33	13.04
7.52	271.91	228E+07	3065.0	4.55	12.42
8.10	272.49	266E+07	3773.4	4.80	11.75
8.68	273.07	306E+07	4566.3	5.05	11.17
9.26	273.65	348E+07	5443.7	5.30	10.66
9.84	274.23	393E+07	6389.6	5.51	10.25
10.42	274.81	440E+07	7426.2	5.72	9.88
11.00	275.39	490E+07	8490.1	5.86	9.63

AREA	OPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (0254)	403.00	17.95	11.75	144.29	1.21
OUTFLOW : ID= 1 (6256)	403.00	17.53	12.25	144.29	1.20

ADD HYD (8370)	AREA	OPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID= 1 (0256):	145.79	7.959	11.25	163.44
+ ID= 2 (6256):	403.00	17.537	12.25	144.29
ID = 3 (8370):	548.79	25.263	12.00	149.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0250) | Area (ha)= 192.88 | Curve Number (CN)= 85.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.22

Unit Hyd Qpeak (cms)= 1.930

PEAK FLOW (cms)= 9.790 (i)
 TIME TO PEAK (hrs)= 11.750
 RUNOFF VOLUME (mm)= 168.680
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.796

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0246) | Area (ha)= 759.61 | Curve Number (CN)= 74.2
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 1.81

Unit Hyd Qpeak (cms)= 5.121

PEAK FLOW (cms)= 25.684 (i)
 TIME TO PEAK (hrs)= 12.000
 RUNOFF VOLUME (mm)= 144.339
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.681

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0248) | Area (ha)= 146.04 | Curve Number (CN)= 81.1
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.30
 U.H. Tp(hrs)= 0.78

Unit Hyd Qpeak (cms)= 2.271

PEAK FLOW (cms)= 9.220 (i)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 158.507
 TOTAL RAINFALL (mm)= 212.000

RUNOFF COEFFICIENT = 0.748

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8364)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0246):	759.61	25.684	12.00	144.34
+ ID2= 2 (0248):	146.04	9.220	11.00	158.51
ID = 3 (8364):	905.65	34.343	12.00	146.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6250) Routing time step (min)'= 15.00

Distance	Elevation	Manning
0.00	269.96	0.0500
8.56	268.55	0.0500
17.13	266.91	0.0500
21.41	266.13	0.0500
40.68	263.15	0.0500
62.09	260.75	0.0500
85.64	258.02	0.0500
88.00	257.69	0.0500
93.20	257.05	0.0500 / 0.0350
93.45	256.88	0.0350
94.20	256.56	0.0350
94.95	256.83	0.0350
95.20	257.08	0.0350 / 0.0500
100.62	257.45	0.0500
115.61	258.57	0.0500
139.16	260.43	0.0500
152.01	261.95	0.0500
171.27	264.63	0.0500
188.40	267.90	0.0500
211.95	274.18	0.0500

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.49	257.05	.205E+04	0.4	0.68	89.65
1.17	257.73	.248E+05	5.7	0.85	72.16
1.85	258.41	.841E+05	25.2	1.10	55.60
2.53	259.09	.180E+06	66.5	1.36	45.14
3.21	259.77	.312E+06	135.7	1.60	38.36
3.89	260.45	.481E+06	236.3	1.82	33.61
4.57	261.13	.681E+06	383.5	2.07	29.60
5.25	261.81	.911E+06	568.9	2.29	26.70
5.93	262.49	.117E+07	800.5	2.51	24.36
6.61	263.17	.146E+07	1079.0	2.72	22.49
7.28	263.84	.177E+07	1413.4	2.94	20.85
7.96	264.52	.210E+07	1797.0	3.14	19.50
8.64	265.20	.246E+07	2241.6	3.35	18.28
9.32	265.88	.283E+07	2739.9	3.55	17.25
10.00	266.56	.323E+07	3295.7	3.75	16.34
10.68	267.24	.364E+07	3908.9	3.94	15.54
11.36	267.92	.408E+07	4578.5	4.12	14.84
12.04	268.60	.453E+07	5320.1	4.32	14.18
12.72	269.28	.499E+07	6109.1	4.50	13.61

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (8364)	905.65	34.34	12.00	146.62	2.00	1.15
OUTFLOW : ID= 1 (6250)	905.65	32.61	12.50	146.62	1.97	1.14

ADD HYD (8366)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0250):	192.88	9.790	11.75	168.68
+ ID2= 2 (6250):	905.65	32.614	12.50	146.62
ID = 3 (8366):	1098.53	41.960	12.25	150.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8368)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8366):	1098.53	41.960	12.25	150.50

+ ID2= 2 (8370): 548.79 25.263 12.00 149.37
ID = 3 (8368): 1647.32 67.068 12.25 150.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8372)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0252):	319.99	18.292	11.25	172.82
+ ID2= 2 (8368):	1647.32	67.068	12.25	150.12
ID = 3 (8372):	1967.31	84.865	12.00	153.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6258) Routing time step (min)'= 15.00

Distance	Elevation	Manning
0.00	252.88	0.0500
15.47	251.89	0.0500
46.41	248.45	0.0500
73.48	245.81	0.0500
88.95	243.75	0.0500
112.15	242.00	0.0500
135.35	240.23	0.0500
162.42	239.76	0.0500
170.97	239.52	0.0500 / 0.0350
171.58	239.03	0.0350
174.02	239.03	0.0350
176.46	239.03	0.0350
177.07	239.52	0.0350 / 0.0500
185.63	239.67	0.0500
208.83	239.87	0.0500
239.77	240.14	0.0500
274.57	244.93	0.0500
336.45	249.51	0.0500
363.52	249.77	0.0500
382.85	251.78	0.0500

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.49	239.52	.135E+05	2.5	0.92	91.32
1.14	240.17	.175E+06	25.6	0.73	114.22
1.78	240.81	.532E+06	123.5	1.17	71.85
2.43	241.46	.932E+06	285.3	1.54	54.48
3.07	242.10	.138E+07	506.5	1.85	45.25
3.72	242.75	.186E+07	786.8	2.12	39.42
4.36	243.39	.239E+07	1127.3	2.37	35.33
5.01	244.04	.296E+07	1535.8	2.61	32.12
5.65	244.68	.356E+07	2014.3	2.84	29.48
6.30	245.33	.420E+07	2535.9	3.03	27.60
6.94	245.97	.488E+07	3108.4	3.20	26.16
7.59	246.62	.561E+07	3740.6	3.35	24.98
8.23	247.26	.638E+07	4446.7	3.50	23.93
8.88	247.91	.721E+07	5229.7	3.64	22.98
9.52	248.55	.809E+07	6094.3	3.78	22.12
10.17	249.20	.901E+07	7050.4	3.93	21.31
10.81	249.84	.100E+08	7773.0	3.90	21.46
11.46	250.49	.111E+08	8998.8	4.07	20.56
12.10	251.13	.122E+08	10318.2	4.24	19.75

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (8372)	1967.31	84.87	12.00	153.82	1.53	0.95
OUTFLOW : ID= 1 (6258)	1967.31	75.75	13.25	153.82	1.46	0.90

ADD HYD (8374)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0258):	181.99	10.024	11.50	182.93
+ ID2= 2 (6258):	1967.31	75.748	13.25	153.82
ID = 3 (8374):	2149.30	84.408	13.00	156.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8376)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				

	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0260):	476.24	24.646	11.75	186.33
+ ID2= 2 (8374):	2149.30	84.408	13.00	156.28
=====				
ID = 3 (8376):	2625.54	106.930	12.75	161.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8378)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0262):	341.31	20.776	11.25	185.72
+ ID2= 2 (8376):	2625.54	106.930	12.75	161.73
=====				
ID = 3 (8378):	2966.85	125.359	12.50	164.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8362)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8378):	2966.85	125.359	12.50	164.49
+ ID2= 2 (8384):	1321.42	65.392	12.00	168.56
=====				
ID = 3 (8362):	4288.27	189.854	12.25	165.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6272)		Routing time step (min) = 15.00
IN= 2-->	OUT= 1	

----- DATA FOR SECTION (2721.0) -----

Distance	Elevation	Manning
0.00	231.01	0.0450
23.01	223.65	0.0450
34.51	222.46	0.0450
51.77	222.11	0.0450
69.02	221.87	0.0450
161.06	221.92	0.0450
166.81	221.91	0.0450
172.56	221.89	0.0450
180.57	221.40	0.0450 / 0.0300
181.57	220.60	0.0300
184.07	220.16	0.0300
195.57	221.85	0.0300 / 0.0450
201.32	221.82	0.0450
207.07	221.77	0.0450
212.83	221.72	0.0450
253.09	222.52	0.0450
408.40	222.65	0.0450
460.17	223.20	0.0450
517.69	224.84	0.0450
569.46	232.57	0.0450

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.41	220.57	189E+04	0.5	0.49	60.18
0.83	220.99	697E+04	3.2	0.82	35.79
1.24	221.40	145E+05	8.8	1.07	27.45
1.84	222.00	643E+05	29.3	0.81	36.61
2.44	222.60	282E+06	132.9	0.83	35.41
3.04	223.20	707E+06	413.7	1.04	28.50
3.64	223.80	118E+07	876.8	1.31	22.46
4.24	224.40	168E+07	1494.8	1.57	18.75
4.84	225.00	221E+07	2264.1	1.82	16.23
5.44	225.60	274E+07	3196.3	2.06	14.29
6.04	226.20	328E+07	4258.6	2.30	12.84
6.65	226.81	383E+07	5444.7	2.52	11.72
7.25	227.41	438E+07	6749.7	2.73	10.82
7.85	228.01	494E+07	8169.4	2.93	10.08
8.45	228.61	551E+07	9700.7	3.12	9.46
9.05	229.21	608E+07	11340.8	3.30	8.94
9.65	229.81	666E+07	13087.4	3.48	8.48
10.25	230.41	724E+07	14938.5	3.65	8.08
10.85	231.01	783E+07	16892.4	3.82	7.73

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8362)	4288.27	189.85	12.25	165.75	2.56
OUTFLOW : ID= 1 (6272)	4288.27	185.57	12.75	165.75	2.55

ADD HYD (8356)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0272):	357.38	10.925	11.25	176.46
+ ID2= 2 (6272):	4288.27	185.571	12.75	165.75
=====				
ID = 3 (8356):	4445.65	194.398	12.75	166.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8352)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8354):	1268.73	85.722	11.50	182.82
+ ID2= 2 (8356):	4445.65	194.398	12.75	166.12
=====				
ID = 3 (8352):	5714.38	272.950	12.25	169.83

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6304)		Routing time step (min) = 15.00
IN= 2-->	OUT= 1	

----- DATA FOR SECTION (3041.0) -----

Distance	Elevation	Manning
0.00	232.08	0.0500
19.00	231.87	0.0500
38.00	231.33	0.0500
66.51	230.44	0.0500
104.51	228.25	0.0500
118.76	225.17	0.0500
128.26	219.86	0.0500
175.77	219.17	0.0500
185.27	218.90	0.0500 / 0.0300
185.52	218.65	0.0300
190.02	218.37	0.0300
194.52	218.60	0.0300
194.77	218.85	0.0300 / 0.0500
204.27	219.60	0.0500
299.28	220.91	0.0500
327.78	222.36	0.0500
375.28	225.71	0.0500
403.79	229.37	0.0500
432.29	230.43	0.0500
470.29	232.00	0.0500

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.48	218.85	121E+05	1.2	0.36	173.94
1.17	219.54	839E+05	9.8	0.44	142.60
1.86	220.23	323E+06	41.0	0.47	131.43
2.56	220.93	708E+06	109.5	0.58	107.67
3.25	221.62	118E+07	224.3	0.71	87.36
3.94	222.31	168E+07	376.5	0.84	74.48
4.63	223.00	222E+07	567.9	0.96	65.26
5.32	223.69	279E+07	794.7	1.07	58.58
6.02	224.39	339E+07	1056.2	1.17	53.52
6.71	225.08	402E+07	1352.4	1.26	49.52
7.40	225.77	468E+07	1677.3	1.34	46.46
8.09	226.46	536E+07	2050.0	1.43	43.58
8.79	227.16	607E+07	2457.9	1.52	41.14
9.48	227.85	680E+07	2901.0	1.60	39.05
10.17	228.54	755E+07	3352.7	1.66	37.53
10.86	229.23	834E+07	3812.5	1.71	36.48
11.55	229.92	919E+07	4241.8	1.73	36.13
12.25	230.62	1018E+08	4697.0	1.74	35.93
12.94	231.31	1118E+08	5186.3	1.74	35.83

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8352)	5714.38	272.95	12.25	169.83	3.47
OUTFLOW : ID= 1 (6304)	5714.38	244.13	13.25	169.83	3.33

ADD HYD (8350)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0304):	292.37	10.078	12.50	158.75
+ ID2= 2 (6304):	5714.38	244.133	13.25	169.83
=====				
ID = 3 (8350):	6006.75	254.018	13.25	169.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ADD HYD (8346)
1 + 2 = 3
AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 1 (8348): 3768.46 194.008 12.25 163.64
+ ID2= 2 (8350): 6006.75 254.018 13.25 169.29
=====
ID = 3 (8346): 9775.21 442.837 12.75 167.11
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ADD HYD (8344)
1 + 2 = 3
AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 1 (0306): 283.97 4.007 15.75 138.25
+ ID2= 2 (8346): 9775.21 442.837 12.75 167.11
=====
ID = 3 (8344): 10059.18 446.536 12.75 166.30
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
RESERVOIR (5509)
IN= 2--> OUT= 1
DT= 15.0 min
OUTFLOW STORAGE OUTFLOW STORAGE
(cms) (ha.m.) (cms) (ha.m.)
0.0000 0.0000 ***** 370.0451
41.0590 56.7403 ***** 863.4386
48.1390 86.3439 ***** *****
56.6340 ***** *****
67.9600 ***** 0.0000 0.0000
-----

```

```

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (8344) ***** 446.536 12.75 166.30
OUTFLOW: ID= 1 (5509) ***** 122.099 21.25 166.30
-----

```

```

PEAK FLOW REDUCTION [Qout/Qin](%)= 27.34
TIME SHIFT OF PEAK FLOW (min)=510.00
MAXIMUM STORAGE USED (ha.m.)=909.2796
-----

```

```

-----
CALIB
NASHYD (0332) Area (ha)= 393.44 Curve Number (CN)= 68.4
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 2.32
-----

```

```

Unit Hyd Qpeak (cms)= 2.894
PEAK FLOW (cms)= 17.291 (i)
TIME TO PEAK (hrs)= 12.000
RUNOFF VOLUME (mm)= 177.773
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.839
-----

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB
NASHYD (0330) Area (ha)= 468.30 Curve Number (CN)= 91.4
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.09
-----

```

```

Unit Hyd Qpeak (cms)= 7.335
PEAK FLOW (cms)= 33.391 (i)
TIME TO PEAK (hrs)= 11.250
RUNOFF VOLUME (mm)= 183.908
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.867
-----

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
ROUTE CHN (6332)
IN= 2--> OUT= 1
Routing time step (min)'= 15.00
-----

```

```

<----- DATA FOR SECTION (3321.0) ----->
Distance Elevation Manning
0.00 234.00 0.0380
25.85 227.20 0.0380
96.94 226.44 0.0380
168.03 227.38 0.0380
219.73 225.62 0.0380
342.52 221.57 0.0380
368.37 221.42 0.0380
374.83 221.23 0.0380
-----

```

```

379.79 220.98 0.0380 /0.0380 Main Channel
380.29 220.47 0.0300 Main Channel
381.29 220.47 0.0300 Main Channel
382.79 220.47 0.0300 Main Channel
383.29 220.98 0.0300 /0.0380 Main Channel
394.22 221.22 0.0380
400.68 221.33 0.0380
407.14 221.44 0.0380
491.16 225.70 0.0380
568.71 227.55 0.0380
607.49 230.14 0.0380
639.80 234.08 0.0380
-----

```

```

-----
<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.51 220.98 554E+04 0.9 0.56 107.27
1.20 221.67 885E+05 12.1 0.49 122.32
1.88 222.35 309E+06 64.7 0.76 79.70
2.57 223.04 615E+06 165.4 0.97 61.99
3.25 223.72 101E+07 322.1 1.16 52.06
3.94 224.41 148E+07 542.9 1.33 45.51
4.62 225.09 204E+07 835.4 1.48 40.77
5.31 225.78 269E+07 1202.8 1.62 37.27
5.99 226.46 344E+07 1618.0 1.70 35.46
6.68 227.15 447E+07 1932.0 1.56 38.56
7.36 227.83 580E+07 2763.9 1.72 34.96
8.05 228.52 718E+07 3861.9 1.95 30.97
8.73 229.20 859E+07 5115.4 2.16 27.98
9.42 229.89 100E+08 6519.2 2.35 25.64
10.10 230.57 115E+08 8092.3 2.55 23.69
10.79 231.26 130E+08 9821.5 2.73 22.05
11.47 231.94 145E+08 11690.0 2.92 20.68
12.16 232.63 160E+08 13694.9 3.09 19.52
12.84 233.31 176E+08 15833.9 3.26 18.52
-----

```

```

-----
<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (0330) 468.30 33.39 11.25 183.91 1.47 0.57
OUTFLOW: ID= 1 (6332) 468.30 27.64 12.50 183.91 1.40 0.55
-----

```

```

-----
ADD HYD (8322)
1 + 2 = 3
AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 1 (0332): 393.44 17.291 12.00 177.77
+ ID2= 2 (6332): 468.30 27.640 12.50 183.91
=====
ID = 3 (8322): 861.74 44.899 12.25 181.11
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
CALIB
NASHYD (0328) Area (ha)= 492.92 Curve Number (CN)= 89.1
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.91
-----

```

```

Unit Hyd Qpeak (cms)= 4.411
PEAK FLOW (cms)= 25.084 (i)
TIME TO PEAK (hrs)= 12.000
RUNOFF VOLUME (mm)= 179.281
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.846
-----

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB
NASHYD (0326) Area (ha)= 678.91 Curve Number (CN)= 91.3
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.95
-----

```

```

Unit Hyd Qpeak (cms)= 5.941
PEAK FLOW (cms)= 34.798 (i)
TIME TO PEAK (hrs)= 12.000
RUNOFF VOLUME (mm)= 184.631
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.871
-----

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB
NASHYD (0324) Area (ha)= 615.64 Curve Number (CN)= 91.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
-----

```

U.H. Tp(hrs)= 1.90

Unit Hyd Qpeak (cms)= 5.544

PEAK FLOW (cms)= 32.052 (i)
TIME TO PEAK (hrs)= 12.000
RUNOFF VOLUME (mm)= 183.871
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.867

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (8320) Area (ha)= 278.74 Curve Number (CN)= 92.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.36

Unit Hyd Qpeak (cms)= 3.499

PEAK FLOW (cms)= 17.763 (i)
TIME TO PEAK (hrs)= 11.500
RUNOFF VOLUME (mm)= 185.834
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.877

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (3182) Area (ha)= 457.40 Curve Number (CN)= 90.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.49

Unit Hyd Qpeak (cms)= 5.259

PEAK FLOW (cms)= 27.258 (i)
TIME TO PEAK (hrs)= 11.750
RUNOFF VOLUME (mm)= 181.131
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.854

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANBYD (3181) Area (ha)= 26.00
ID= 1 DT=15.0 min Total Imp(%)= 81.00 Dir. Conn.(%)= 81.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 21.06 4.94
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 416.33 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 53.00 51.40
over (min)= 15.00 30.00
Storage Coeff. (min)= 9.54 (ii) 23.50 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.09 0.04

TOTALS*
PEAK FLOW (cms)= 3.10 0.64 3.733 (iii)
TIME TO PEAK (hrs)= 10.00 10.00
RUNOFF VOLUME (mm)= 210.00 179.74 204.25
TOTAL RAINFALL (mm)= 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.85 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 89.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
TO THE STORAGE COEFFICIENT
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8334)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (3181): 26.00 3.733 10.00 204.25
+ ID2= 2 (3182): 457.40 27.258 11.75 181.13
=====
ID = 3 (8334): 483.40 29.279 11.00 182.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6320)

IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (3201.0)
Distance Elevation Manning
0.00 249.00 0.0380
22.92 245.86 0.0380
45.83 244.87 0.0380
91.66 243.11 0.0380
126.03 239.53 0.0380
160.41 237.17 0.0380
166.14 237.06 0.0380
177.59 237.13 0.0380
183.32 237.20 0.0380
189.05 236.70 0.0380 / 0.0350 Main Channel
193.05 235.89 0.0350 Main Channel
197.05 236.64 0.0350 / 0.0380 Main Channel
200.81 236.74 0.0380
206.24 237.03 0.0380
246.34 238.82 0.0380
263.53 243.87 0.0380
389.56 247.64 0.0380
452.58 247.74 0.0380
498.41 248.60 0.0380
567.16 249.84 0.0380

TRAVEL TIME TABLE
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV. TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.37 236.26 .270E+04 0.4 0.58 107.96
0.75 236.64 .108E+05 2.6 0.92 68.01
1.48 237.37 .877E+05 27.4 1.17 53.35
2.20 238.09 .277E+06 123.1 1.65 37.83
2.93 238.82 .540E+06 299.7 2.08 30.01
3.66 239.55 .857E+06 587.8 2.57 24.29
4.39 240.28 .120E+07 977.8 3.04 20.52
5.11 241.00 .158E+07 1458.1 3.46 18.02
5.84 241.73 .198E+07 2028.6 3.84 16.23
6.57 242.46 .240E+07 2690.5 4.19 14.86
7.29 243.18 .285E+07 3431.8 4.50 13.84
8.02 243.91 .335E+07 4149.1 4.64 13.44
8.75 244.64 .393E+07 4746.6 4.51 13.80
9.47 245.36 .463E+07 5618.6 4.53 13.75
10.20 246.09 .545E+07 6783.8 4.66 13.39
10.93 246.82 .635E+07 8274.5 4.87 12.79
11.66 247.55 .733E+07 9988.0 5.10 12.23
12.38 248.27 .853E+07 11422.0 5.01 12.45
13.11 249.00 .989E+07 13749.7 5.20 11.98

hydrograph
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8334) 483.40 29.28 11.00 182.37 1.49 1.17
OUTFLOW: ID= 1 (6320) 483.40 27.43 12.00 182.37 1.48 1.17

ADD HYD (8332)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8320): 278.74 17.763 11.50 185.83
+ ID2= 2 (6320): 483.40 27.435 12.00 182.37
=====
ID = 3 (8332): 762.14 44.940 12.00 183.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHYD (0314) Area (ha)= 165.20 Curve Number (CN)= 89.9
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.71

Unit Hyd Qpeak (cms)= 3.978

PEAK FLOW (cms)= 14.198 (i)
TIME TO PEAK (hrs)= 11.000
RUNOFF VOLUME (mm)= 178.932
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.844

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (5316)
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (3161.0)
Distance Elevation Manning
0.00 248.54 0.0350
27.93 248.34 0.0350
41.89 246.61 0.0350

62.83	243.09	0.0350
132.65	239.00	0.0350
188.50	236.74	0.0350
195.48	236.54	0.0350
202.46	236.32	0.0350
205.48	236.14	0.0350 / 0.0310
205.98	235.61	0.0310
207.98	235.25	0.0310
209.98	235.53	0.0310
210.48	236.00	0.0310 / 0.0350
216.42	236.73	0.0350
258.31	239.09	0.0350
328.12	239.84	0.0350
439.83	241.47	0.0350
530.58	242.08	0.0350
586.43	242.93	0.0350
691.16	248.00	0.0350

----- TRAVEL TIME TABLE -----

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.37	235.62	.215E+04	0.7	0.77	53.88
0.75	236.00	.634E+04	3.5	1.36	30.45
1.46	236.71	.297E+05	19.3	1.62	25.66
2.16	237.41	.103E+06	75.8	1.84	22.60
2.87	238.12	.229E+06	202.8	2.21	18.79
3.57	238.82	.407E+06	423.3	2.59	16.03
4.28	239.53	.654E+06	727.7	2.77	14.97
4.99	240.24	.103E+07	1242.0	3.01	13.76
5.69	240.94	.151E+07	2002.3	3.31	12.54
6.40	241.65	.210E+07	2987.9	3.55	11.70
7.10	242.35	.286E+07	4303.8	3.75	11.07
7.81	243.06	.374E+07	6165.3	4.10	10.11
8.51	243.76	.468E+07	8642.4	4.60	9.03
9.22	244.47	.565E+07	11495.3	5.06	8.20
9.93	245.18	.666E+07	14714.1	5.50	7.54
10.63	245.88	.770E+07	18290.3	5.91	7.02
11.34	246.59	.877E+07	22221.9	6.30	6.58
12.04	247.29	.988E+07	26464.4	6.67	6.22
12.75	248.00	.110E+08	31061.3	7.01	5.91

----- hydrograph ----- <-pipe / channel-->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (0314)	165.20	14.20	11.00	178.83	1.23	1.52
OUTFLOW : ID= 1 (6316)	165.20	13.54	11.25	178.83	1.20	1.51

----- CALIB -----

NASHVD (0316)	Area (ha) = 232.34	Curve Number (CN) = 92.3
ID= 1 DT=15.0 min	Ia (mm) = 5.00	# of Linear Res.(N) = 1.50
U.H. Tp(hrs) = 1.01		
Unit Hyd Qpeak (cms) = 3.923		
PEAK FLOW (cms) = 17.290 (i)		
TIME TO PEAK (hrs) = 11.000		
RUNOFF VOLUME (mm) = 185.893		
TOTAL RAINFALL (mm) = 212.000		
RUNOFF COEFFICIENT = 0.877		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- ADD HYD (8338) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (0316):	232.34	17.290	11.00	185.89
+ ID= 2 (6316):	165.20	13.544	11.25	178.83
=====				
ID = 3 (8338):	397.54	30.818	11.25	182.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- CALIB -----

NASHVD (0312)	Area (ha) = 359.44	Curve Number (CN) = 91.1
ID= 1 DT=15.0 min	Ia (mm) = 5.00	# of Linear Res.(N) = 1.50
U.H. Tp(hrs) = 1.36		
Unit Hyd Qpeak (cms) = 4.529		
PEAK FLOW (cms) = 22.783 (i)		
TIME TO PEAK (hrs) = 11.500		
RUNOFF VOLUME (mm) = 183.641		
TOTAL RAINFALL (mm) = 212.000		
RUNOFF COEFFICIENT = 0.866		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB -----

NASHVD (0308)	Area (ha) = 529.30	Curve Number (CN) = 79.2
ID= 1 DT=15.0 min	Ia (mm) = 5.00	# of Linear Res.(N) = 1.50
U.H. Tp(hrs) = 1.62		

Unit Hyd Qpeak (cms) = 5.575

PEAK FLOW (cms) = 26.714 (i)
TIME TO PEAK (hrs) = 12.000
RUNOFF VOLUME (mm) = 155.772
TOTAL RAINFALL (mm) = 212.000
RUNOFF COEFFICIENT = 0.735

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB -----

NASHVD (0310)	Area (ha) = 138.28	Curve Number (CN) = 81.9
ID= 1 DT=15.0 min	Ia (mm) = 5.00	# of Linear Res.(N) = 1.50
U.H. Tp(hrs) = 0.76		

Unit Hyd Qpeak (cms) = 3.102

PEAK FLOW (cms) = 10.810 (i)
TIME TO PEAK (hrs) = 12.000
RUNOFF VOLUME (mm) = 160.351
TOTAL RAINFALL (mm) = 212.000
RUNOFF COEFFICIENT = 0.756

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- ADD HYD (8342) -----

1	2	3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (0308):	529.30	26.714	12.00	155.77		
+ ID= 2 (0310):	138.28	10.810	11.00	160.35		
=====						
ID = 3 (8342):	667.58	36.681	11.50	156.72		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ROUTE CWN (6312) -----

IN= 2 ---> OUT= 1 | Routing time step (min) = 15.00

----- DATA FOR SECTION (3121.0) -----

Distance	Elevation	Manning
0.00	265.94	0.0360
38.07	265.43	0.0360
59.82	263.98	0.0360
103.32	254.59	0.0360
157.70	252.16	0.0360
217.52	250.45	0.0360
233.84	247.69	0.0360
234.71	247.27	0.0360
239.71	246.38	0.0360
244.71	246.12	0.0360 / 0.0330
247.71	245.17	0.0330
249.71	245.19	0.0330
251.71	245.64	0.0330 / 0.0360
259.71	246.67	0.0360
282.78	247.12	0.0360
315.41	251.60	0.0360
424.17	256.13	0.0360
478.55	257.04	0.0360
516.62	259.37	0.0360
538.37	266.00	0.0360

----- TRAVEL TIME TABLE -----

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.47	245.64	.111E+04	3.1	1.78	6.07
1.54	246.71	.103E+05	53.9	3.39	3.19
2.61	247.78	.414E+05	281.3	4.40	2.45
3.68	248.85	.839E+05	749.1	5.79	1.87
4.74	249.91	.136E+06	1463.2	6.97	1.55
5.81	250.98	.201E+06	2354.8	7.61	1.42
6.88	252.05	.295E+06	3533.0	7.76	1.39
7.95	253.12	.427E+06	5303.9	8.05	1.34
9.02	254.19	.594E+06	7856.5	8.58	1.26
10.09	255.26	.792E+06	11449.2	9.37	1.15
11.15	256.32	.101E+07	15862.4	10.15	1.06
12.22	257.39	.127E+07	21074.5	10.75	1.01
13.29	258.46	.155E+07	28184.1	11.78	0.92
14.36	259.53	.185E+07	36386.2	12.77	0.85
15.43	260.60	.215E+07	46110.6	13.89	0.78
16.50	261.67	.246E+07	56822.5	14.95	0.72
17.56	262.73	.278E+07	68498.1	15.97	0.68

18.63 263.80 .310E+07 81118.8 16.94 0.64
19.70 264.87 .344E+07 93603.2 17.66 0.61

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8342) 667.58 36.68 11.50 156.72 1.18 2.60
OUTFLOW : ID= 1 (6312) 667.58 36.77 11.25 156.72 1.18 2.60

| ADD HYD (8340) |
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0312): 358.44 22.783 11.50 183.64
+ ID2= 2 (6312): 667.58 36.772 11.25 156.72
=====

ID = 3 (8340): 1027.02 59.491 11.25 166.14
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ADD HYD (8336) |
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8338): 397.54 30.818 11.25 182.96
+ ID2= 2 (8340): 1027.02 59.491 11.25 166.14
=====

ID = 3 (8336): 1424.56 90.309 11.25 170.84
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ADD HYD (8330) |
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8332): 762.14 44.940 12.00 183.64
+ ID2= 2 (8336): 1424.56 90.309 11.25 170.84
=====

ID = 3 (8330): 2186.70 134.425 11.50 175.30
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ROUTE CHN (6324) |
IN= 2 -> OUT= 1
Routing time step (min) = 15.00

<----- DATA FOR SECTION (3241.0) ----->
Distance Elevation Manning
0.00 234.38 0.0360
33.57 232.48 0.0360
67.14 230.14 0.0360
83.93 228.80 0.0360
134.29 227.62 0.0360
209.82 225.10 0.0360
218.21 224.86 0.0360
226.61 224.47 0.0360
234.00 223.86 0.0360 / 0.0300 Main Channel
234.10 223.66 0.0300 Main Channel
235.00 223.66 0.0300 Main Channel
235.90 223.66 0.0300 Main Channel
236.00 223.86 0.0300 / 0.0360 Main Channel
243.39 224.92 0.0360
251.78 224.89 0.0360
335.71 225.64 0.0360
562.32 226.53 0.0360
637.85 228.36 0.0360
705.00 229.80 0.0360
830.89 234.00 0.0360

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.20 223.86 .145E+04 0.2 0.43 147.07
0.73 224.39 .159E+05 2.8 0.68 93.11
1.27 224.93 .561E+05 11.0 0.75 85.19
1.80 225.46 .217E+06 45.6 0.80 79.20
2.33 225.99 .566E+06 134.4 0.91 70.20
2.87 226.53 .122E+07 338.1 1.06 59.97
3.40 227.06 .208E+07 744.3 1.38 46.16
3.94 227.60 .298E+07 1295.9 1.66 38.38
4.47 228.13 .399E+07 1979.9 1.90 33.59
5.00 228.66 .509E+07 2807.5 2.11 30.21
5.54 229.20 .627E+07 3815.1 2.32 27.41
6.07 229.73 .753E+07 4984.1 2.53 25.17
6.60 230.26 .884E+07 6346.3 2.74 23.20
7.14 230.80 .102E+08 7861.2 2.95 21.61
7.67 231.33 .116E+08 9526.4 3.14 20.29

8.21 231.87 .131E+08 11342.2 3.32 19.18
8.74 232.40 .146E+08 13309.6 3.49 18.22
9.27 232.93 .161E+08 15414.0 3.66 17.41
9.81 233.47 .177E+08 17671.0 3.81 16.70

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8330) 2186.70 134.42 11.50 175.30 2.33 0.91
OUTFLOW : ID= 1 (6324) 2186.70 121.79 12.50 175.30 2.26 0.89

| ADD HYD (8328) |
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0324): 615.64 32.052 12.00 183.87
+ ID2= 2 (6324): 2186.70 121.791 12.50 175.30
=====

ID = 3 (8328): 2802.34 153.169 12.25 177.18
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| CALIB (0322) |
IN= 1 DT=15.0 min
Area (ha) = 513.13 Curve Number (CN) = 91.4
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U.N. Tp (hrs) = 1.75

Unit Hyd Qpeak (cms) = 5.020

PEAK FLOW (cms) = 28.218 (1)
TIME TO PEAK (hrs) = 12.000
RUNOFF VOLUME (mm) = 184.747
TOTAL RAINFALL (mm) = 212.000
RUNOFF COEFFICIENT = 0.871

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| ADD HYD (8326) |
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0322): 513.13 28.218 12.00 184.75
+ ID2= 2 (8328): 2802.34 153.169 12.25 177.18
=====

ID = 3 (8326): 3315.47 180.979 12.25 178.35
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ADD HYD (8324) |
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0326): 678.91 34.798 12.00 184.63
+ ID2= 2 (8326): 3315.47 180.979 12.25 178.35
=====

ID = 3 (8324): 3994.38 215.469 12.25 179.42
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ROUTE CHN (6328) |
IN= 2 -> OUT= 1
Routing time step (min) = 15.00

<----- DATA FOR SECTION (3281.0) ----->
Distance Elevation Manning
0.00 228.00 0.0380
18.58 224.97 0.0380
78.98 223.52 0.0380
125.44 223.28 0.0380
171.90 221.71 0.0380
213.72 219.65 0.0380
218.36 219.40 0.0380
223.01 219.19 0.0380
225.95 219.14 0.0380 / 0.0320 Main Channel
226.45 218.14 0.0320 Main Channel
236.95 218.14 0.0320 Main Channel
245.85 218.14 0.0320 Main Channel
245.95 219.14 0.0320 / 0.0380 Main Channel
246.24 219.16 0.0380
250.88 219.24 0.0380
255.53 219.39 0.0380
325.22 221.47 0.0380
367.03 223.14 0.0380
404.20 225.17 0.0380
459.95 228.04 0.0380

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.50	216.64	373E+05	3.2	0.33	192.37
1.00	219.14	752E+05	10.1	0.51	124.71
1.52	219.66	149E+06	22.2	0.57	112.06
2.04	220.18	279E+06	43.4	0.59	107.06
2.56	220.70	464E+06	77.0	0.63	100.52
3.08	221.22	705E+06	125.7	0.68	93.56
3.61	221.75	100E+07	192.7	0.73	86.62
4.13	222.27	135E+07	278.1	0.79	80.92
4.65	222.79	176E+07	385.9	0.84	75.86
5.17	223.31	222E+07	514.9	0.89	71.82
5.69	223.83	280E+07	662.7	0.90	70.40
6.21	224.35	345E+07	867.7	0.96	66.35
6.73	224.87	417E+07	1107.0	1.01	62.81
7.25	225.39	494E+07	1400.0	1.08	58.80
7.78	225.92	573E+07	1732.2	1.15	55.17
8.30	226.44	655E+07	2099.9	1.22	52.02
8.82	226.96	740E+07	2502.8	1.29	49.09
9.34	227.48	828E+07	2940.9	1.36	46.90
9.86	228.00	918E+07	3414.3	1.42	44.79

<----- hydrograph -----> <-pipe / channel-->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8324) 3994.38	215.47	12.25	179.42	3.74	0.75
OUTFLOW : ID= 1 (6328) 3994.38	189.16	13.25	179.42	3.57	0.73

ADD HYD (8320)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0328):	492.92	25.084	12.00	179.28
+ ID2= 2 (6328):	3994.38	189.161	13.25	179.42
ID = 3 (8320):	4487.30	212.288	13.00	179.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8318)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8320):	4487.30	212.288	13.00	179.40
+ ID2= 2 (8322):	861.74	44.899	12.25	181.11
ID = 3 (8318):	5349.04	259.187	13.00	179.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8316)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (5509):	10059.18	122.099	21.25	166.30
+ ID2= 2 (8318):	5349.04	259.187	13.00	179.68
ID = 3 (8316):	15408.22	381.286	13.00	170.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8312)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8314):	620.10	7.956	11.00	148.14
+ ID2= 2 (8316):	15408.22	381.286	13.00	170.94
ID = 3 (8312):	16028.32	389.242	13.00	170.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8308)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8310):	598.90	70.136	10.25	180.15
+ ID2= 2 (8312):	16028.32	389.242	13.00	170.06
ID = 3 (8308):	16627.22	459.378	12.25	170.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5510)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IR= 2--> OUT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min	0.0000	0.0000	*****	74.0090
	66.5450	18.5023	*****	*****
	98.5430	37.0045	*****	*****

AREA	QPEAK	TPEAK	R.V.	
(ha)	(cms)	(hrs)	(mm)	
INFLOW : ID= 2 (8308)	*****	385.156	12.25	170.42
OUTFLOW : ID= 1 (5510)	*****	136.187	28.75	170.42

PEAK FLOW REDUCTION [Qout/Qin](%)= 35.36
 TIME SHIFT OF PEAK FLOW (min)=990.00
 MAXIMUM STORAGE USED (ha.m.)=858.5957

ADD HYD (8240)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0336):	2785.00	20.993	24.00	173.23
+ ID2= 2 (5510):	16627.22	136.187	28.75	170.42
ID = 3 (8240):	19412.22	157.058	25.25	170.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8238)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (5508):	9524.23	162.331	15.50	143.84
+ ID2= 2 (8240):	19412.22	157.058	25.25	170.86
ID = 3 (8238):	28936.45	319.389	16.00	161.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8236)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0342):	1004.58	33.547	12.25	142.70
+ ID2= 2 (8238):	28936.45	319.389	16.00	161.97
ID = 3 (8236):	29941.03	352.936	14.75	161.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8234)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8232):	285.80	30.260	10.25	152.63
+ ID2= 2 (8236):	29941.03	352.936	14.75	161.32
ID = 3 (8234):	30226.83	383.196	14.75	161.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8230)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0344):	344.00	24.075	11.00	150.24
+ ID2= 2 (8234):	30226.83	383.196	14.75	161.34
ID = 3 (8230):	30570.83	407.271	12.25	161.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8228)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8226):	1952.06	96.397	12.00	136.68
+ ID2= 2 (8230):	30570.83	407.271	12.25	161.11
ID = 3 (8228):	32522.89	503.668	12.25	159.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

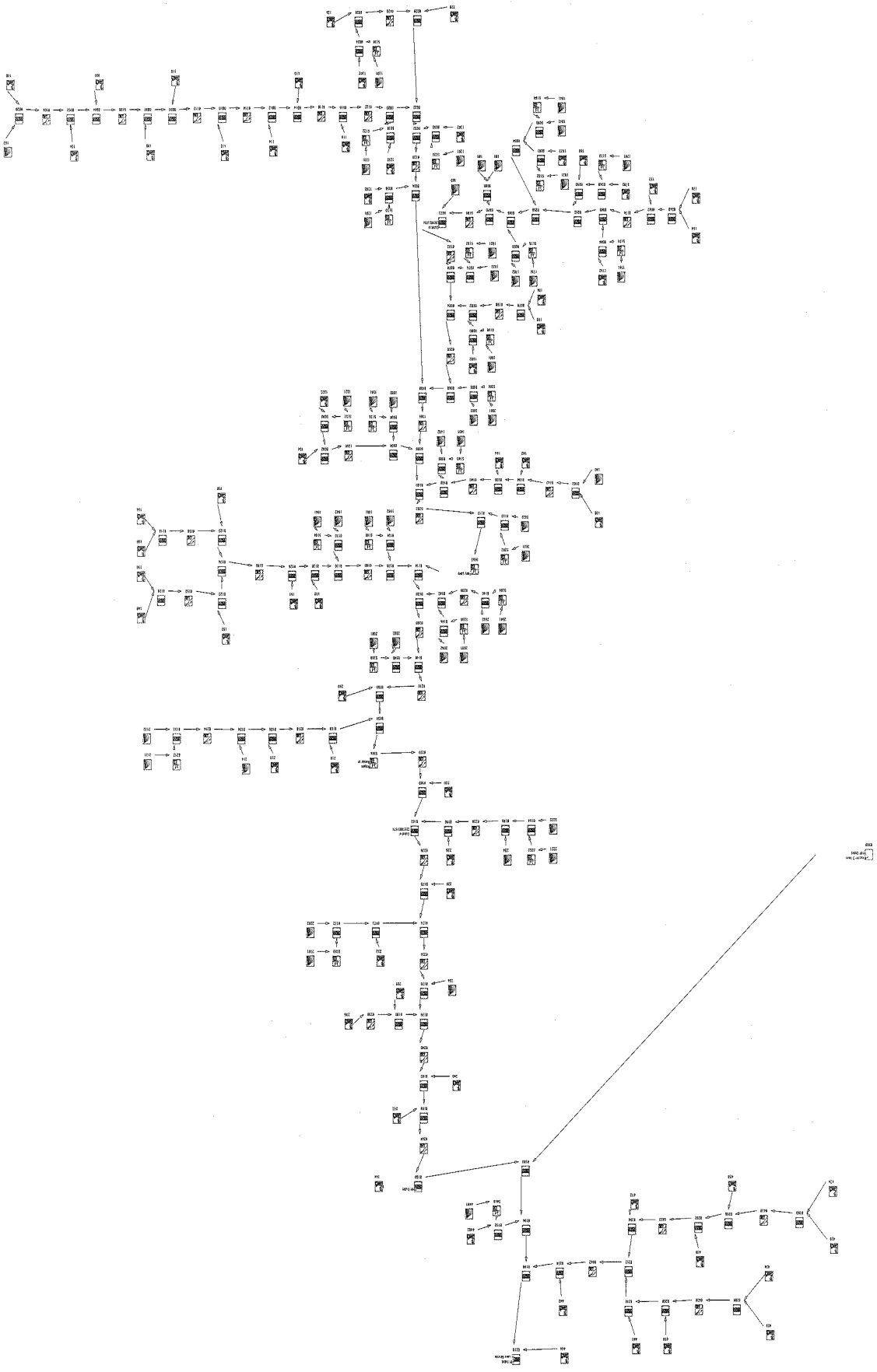
| ADD HYD (8190) |
1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
ID1= 1 (0356): 1015.22 47.576 11.75 126.89
+ ID2= 2 (8228): 32522.89 454.139 12.25 159.65
=====

ID = 3 (8190): 33538.11 501.128 12.00 158.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH
=====

EAST HOLLAND RIVER SUBWATERSHED



**EXISTING
EAST HOLLAND RIVER 2-YEAR STORM**

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V V I SSSS U U A L
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
W V I SSSS UUUU A A LLLL
OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y MM MM O O
O O T T H H Y Y M O O
OOO T T H H Y Y M M OOO Company Serial
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***** DETAILED OUTPUT *****

Input filename: C:\Program Files\Visual Ortymo 2.4r\VO2\voin.dat
Output filename: C:\Users\jscott\AppData\Local\Temp\547c57b8-560f-4803-b24a-bd376f2963f2\Scenario.out
Summary filename: C:\Users\jscott\AppData\Local\Temp\547c57b8-560f-4803-b24a-bd376f2963f2\Scenario.sum

DATE: 08/22/2012 TIME: 01:19:58

USER:

COMMENTS: _____

** SIMULATION NUMBER: 1 **

READ STORM Filename: C:\Users\jscott\AppData\Local\Temp\547c57b8-560f-4803-b24a-bd376f2963f2\75768f8e
Total= 42.00 mm Comments: 2-Year 12-Hour SCS II Design Storm

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.05	3.25	1.68	6.25	7.56	9.25	1.47
0.50	1.05	3.50	1.68	6.50	7.56	9.50	1.47
0.75	1.05	3.75	1.68	6.75	7.56	9.75	1.47
1.00	1.05	4.00	1.68	7.00	7.56	10.00	1.47
1.25	1.05	4.25	1.68	7.25	7.56	10.25	1.47
1.50	1.05	4.50	1.68	7.50	7.56	10.50	1.47
1.75	1.05	4.75	1.68	7.75	7.56	10.75	1.47
2.00	1.05	5.00	1.68	8.00	7.56	11.00	1.47
2.25	1.26	5.25	3.36	8.25	14.71	11.25	2.94
2.50	1.26	5.50	3.36	8.50	14.71	11.50	2.94
2.75	1.26	5.75	3.36	8.75	14.71	11.75	2.94
3.00	1.26	6.00	3.36	9.00	14.71	12.00	2.94

CALIB STANDBYD (1662) Area (ha)= 158.10
ID= 1 DT=15.0 min Total Imp(%)= 58.00 Dir. Conn.(%)= 30.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	91.70	66.40
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1026.65	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	55.44	25.13
over (min)	15.00	45.00
Storage Coeff. (min)=	16.11 (ii)	34.69 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.07	0.03
PEAK FLOW (cms)=	5.16	2.65
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	40.00	15.95
TOTAL RAINFALL (mm)=	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.38

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (1661) Area (ha)= 276.60
ID= 1 DT=15.0 min Total Imp(%)= 40.00 Dir. Conn.(%)= 21.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	110.64	165.96
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1357.94	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	55.44	16.29
over (min)	15.00	45.00
Storage Coeff. (min)=	19.05 (ii)	41.16 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.06	0.03
PEAK FLOW (cms)=	5.84	3.94
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	40.00	13.32
TOTAL RAINFALL (mm)=	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.32

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5166)
IR= 2 -> OUTF= 1
DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	6.0870	6.1196
	0.3790	3.2753	7.7230	7.0042
	1.3370	3.4678	9.3080	7.8876
	2.9800	4.4433	9.7080	7.8976
	4.4520	5.2716	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (1661)	276.600	7.426	6.00	18.92
OUTFLOW: ID= 1 (5166)	276.600	1.437	8.25	18.92

PEAK FLOW REDUCTION [Qout/Qin](%)= 19.35
TIME SHIFT OF PEAK FLOW (min)=135.00
MAXIMUM STORAGE USED (ha.m.)= 3.5273

ADD HYD (8134)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1662):	158.10	6.269	6.00	23.16
+ ID2= 2 (5166):	276.60	1.437	8.25	18.92
ID= 3 (8134):	434.70	6.384	6.00	20.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDBYD (1642) Area (ha)= 132.50
ID= 1 DT=15.0 min Total Imp(%)= 20.00 Dir. Conn.(%)= 15.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	26.50	106.00
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	939.86	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	55.44	10.29
over (min)	15.00	45.00
Storage Coeff. (min)=	15.27 (ii)	41.84 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.07	0.03
PEAK FLOW (cms)=	2.21	1.58
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	40.00	10.58
TOTAL RAINFALL (mm)=	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.25

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%

YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (1641) | Area (ha)= 101.70 | Dir. Conn.(%)= 52.00
|ID= 1 DT=15.0 min | Total Imp(%)= 70.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	71.19	30.51
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	823.41	40.00
Manning's n	0.013	0.250
Max.Eff.Inten.(mm/hr)	55.44	22.58
over (min)	15.00	45.00
Storage Coeff. (min)	14.11 (ii)	33.51 (ii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.07	0.03
PEAK FLOW (cms)	6.09	1.12
TIME TO PEAK (hrs)	6.00	6.50
RUNOFF VOLUME (mm)	40.00	15.02
TOTAL RAINFALL (mm)	42.00	42.00
RUNOFF COEFFICIENT	0.95	0.36

***** TOTALS*
PEAK FLOW (cms)= 6.550 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5164)
IN= 2--> OUT= 1
DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	7.6100	4.1638
	0.3440	1.7056	8.2090	4.5053
	3.1340	1.9730	8.8070	4.8468
	4.1310	3.0607	9.2070	4.8568
	5.1070	3.7142	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (1641)	101.700	6.550	6.00	28.01
OUTFLOW: ID= 1 (5164)	101.700	1.275	7.00	28.00

PEAK FLOW REDUCTION [Qout/Qin](%)= 19.46
TIME SHIFT OF PEAK FLOW (min)= 60.00
MAXIMUM STORAGE USED (ha.m.)= 1.7977

ADD HYD (8132)
1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (1641):	132.50	2.826	6.00	15.08
+ ID= 2 (5164):	101.70	1.275	7.00	28.00
ID = 3 (8132):	234.20	3.004	6.00	20.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHYD (0162) | Area (ha)= 190.14 | Curve Number (CN)= 64.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.79

Unit Hyd Qpeak (cms)= 4.104

PEAK FLOW (cms)= 0.776 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 7.501
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.179

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

NASHYD (0160) | Area (ha)= 318.82 | Curve Number (CN)= 61.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.45

Unit Hyd Qpeak (cms)= 3.754

PEAK FLOW (cms)= 0.745 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 6.825
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.163

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0152) | Area (ha)= 124.37 | Curve Number (CN)= 59.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.89

Unit Hyd Qpeak (cms)= 2.372

PEAK FLOW (cms)= 0.386 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 6.335
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.151

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0148) | Area (ha)= 417.89 | Curve Number (CN)= 26.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.18

Unit Hyd Qpeak (cms)= 6.067

PEAK FLOW (cms)= 0.295 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 1.787
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.043

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0150) | Area (ha)= 105.88 | Curve Number (CN)= 17.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.58

Unit Hyd Qpeak (cms)= 3.138

PEAK FLOW (cms)= 0.074 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 1.047
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.025

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8116)
1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (0148):	417.89	0.295	8.00	1.79
+ ID= 2 (0150):	105.88	0.074	6.75	1.05
ID = 3 (8116):	523.77	0.355	7.50	1.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6152)
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1521.0) ----->

Distance	Elevation	Manning
0.00	290.00	0.0500
15.34	286.87	0.0500
46.03	284.84	0.0500
88.22	283.07	0.0500
126.57	282.11	0.0500
153.42	280.34	0.0500
157.26	280.07	0.0500
161.09	279.72	0.0500
167.76	278.94	0.0500 / 0.0300
168.01	278.60	0.0300

168.76	278.60	0.0300	Main Channel
169.51	278.60	0.0300	Main Channel
169.76	278.90	0.0300 / 0.0500	Main Channel
173.76	279.40	0.0500	
180.27	279.69	0.0500	
184.11	279.93	0.0500	
218.62	281.86	0.0500	
260.82	283.20	0.0500	
360.54	289.25	0.0500	
379.72	289.80	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.30	278.90	.114E+04	0.4	0.80	45.70
0.87	279.47	.932E+04	4.2	0.98	37.22
1.45	280.05	.353E+05	17.5	1.09	33.68
2.02	280.62	.831E+05	47.8	1.26	29.01
2.59	281.19	.155E+06	102.9	1.46	25.06
3.17	281.77	.250E+06	189.3	1.66	22.03
3.74	282.34	.375E+06	298.5	1.75	20.91
4.32	282.92	.547E+06	456.5	1.83	19.98
4.89	283.49	.767E+06	705.7	2.02	18.12
5.46	284.06	.102E+07	1038.0	2.24	16.34
6.04	284.64	.130E+07	1444.1	2.45	14.98
6.61	285.21	.161E+07	1940.1	2.66	13.79
7.18	285.78	.194E+07	2522.6	2.86	12.80
7.76	286.36	.229E+07	3187.4	3.06	11.98
8.33	286.93	.267E+07	3942.3	3.25	11.28
8.91	287.51	.306E+07	4824.9	3.46	10.58
9.48	288.08	.347E+07	5795.7	3.67	9.99
10.05	288.65	.390E+07	6855.4	3.86	9.48
10.63	289.23	.434E+07	8005.0	4.05	9.04

<---- hydrograph ----> <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (8116)	523.77	0.36	7.50	1.64	0.26	0.80
OUTFLOW : ID= 1 (6152)	523.77	0.33	8.50	1.64	0.24	0.80

ADD HYD (8122)

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0152):	124.37	0.386	7.25	6.33
+ ID2= 2 (6152):	523.77	0.333	8.50	1.64
ID = 3 (8122):	648.14	0.693	8.00	2.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0158)

Area (ha)	178.59	Curve Number (CN)	61.0
U.H. Tp (hrs)	0.94	# of Linear Res. (N)	1.50

Unit Hyd Qpeak (cms) = 3.230

PEAK FLOW (cms) = 0.571 (i)

TIME TO PEAK (hrs) = 7.250

RUNOFF VOL (mm) = 6.789

TOTAL RAINFALL (mm) = 42.000

RUNOFF COEFFICIENT = 0.162

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0156)

Area (ha)	83.49	Curve Number (CN)	45.0
U.H. Tp (hrs)	0.62	# of Linear Res. (N)	1.50

Unit Hyd Qpeak (cms) = 2.306

PEAK FLOW (cms) = 0.208 (i)

TIME TO PEAK (hrs) = 6.750

RUNOFF VOL (mm) = 3.858

TOTAL RAINFALL (mm) = 42.000

RUNOFF COEFFICIENT = 0.092

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0154)

Area (ha)	200.32	Curve Number (CN)	37.0
U.H. Tp (hrs)	0.97	# of Linear Res. (N)	1.50

Unit Hyd Qpeak (cms) = 3.537

PEAK FLOW (cms) = 0.263 (i)

TIME TO PEAK (hrs) = 7.500

RUNOFF VOL (mm) = 2.985

TOTAL RAINFALL (mm) = 42.000

RUNOFF COEFFICIENT = 0.069

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8118)

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0154):	200.32	0.263	7.50	2.88
+ ID2= 2 (0156):	83.49	0.208	6.75	3.86
ID = 3 (8118):	283.81	0.463	7.00	3.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6158)

IN= 2 -> OUT= 1

Routing time step (min) = 15.00

DATA FOR SECTION (1581.0)

Distance	Elevation	Manning
0.00	290.00	0.0400
19.20	288.88	0.0400
48.00	288.26	0.0400
119.99	283.68	0.0400
167.99	282.07	0.0400
196.79	281.32	0.0400
210.79	280.90	0.0400
219.79	280.38	0.0400 / 0.0300
220.04	279.65	0.0300
220.79	279.65	0.0300
221.54	279.65	0.0300
221.79	280.39	0.0300 / 0.0400
225.79	280.59	0.0400
235.19	280.88	0.0400
254.39	281.50	0.0400
273.58	282.56	0.0400
302.38	283.42	0.0400
359.98	284.03	0.0400
436.77	289.37	0.0400
475.17	290.37	0.0400

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.37	280.01	.973E+03	0.5	0.86	31.78
0.73	280.38	.210E+04	1.5	1.18	23.16
1.30	280.95	.149E+05	8.6	0.95	28.91
1.86	281.51	.579E+05	40.0	1.13	24.12
2.43	282.08	.133E+06	118.5	1.46	18.75
2.99	282.64	.236E+06	254.6	1.77	15.46
3.56	283.21	.369E+06	450.3	2.00	13.67
4.13	283.78	.542E+06	702.7	2.13	12.86
4.69	284.34	.767E+06	1125.2	2.41	11.36
5.26	284.91	.101E+07	1697.1	2.75	9.93
5.82	285.47	.127E+07	2379.1	3.07	8.91
6.39	286.04	.155E+07	3171.4	3.36	8.14
6.95	286.60	.184E+07	4075.5	3.64	7.53
7.52	287.17	.215E+07	5093.1	3.89	7.03
8.09	287.74	.247E+07	6226.5	4.13	6.62
8.65	288.30	.281E+07	7461.2	4.36	6.28
9.22	288.87	.318E+07	8635.1	4.46	6.13
9.78	289.43	.357E+07	10143.6	4.67	5.86
10.35	290.00	.398E+07	11608.8	4.79	5.71

<---- hydrograph ----> <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (8118)	283.81	0.46	7.00	3.17	0.33	0.86
OUTFLOW : ID= 1 (6158)	283.81	0.43	7.75	3.17	0.31	0.86

ADD HYD (8120)

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0158):	178.59	0.571	7.25	6.79
+ ID2= 2 (6158):	283.81	0.434	7.75	3.17
ID = 3 (8120):	462.40	0.999	7.75	4.57

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ADD HYD (8124)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8120): 462.40 0.999 7.75 4.57
+ ID2= 2 (8122): 648.14 0.693 8.00 2.54
=====
ID = 3 (8124): 1110.54 1.686 7.75 3.38

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
ROUTE CHN (6160)
IN# 2--> OTR# 1 Routing time step (min)'= 15.00

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-----
<----- DATA FOR SECTION (1601.0) ----->
Distance Elevation Manning
0.00 251.02 0.0400
29.89 254.30 0.0400
40.75 252.58 0.0400
62.49 251.30 0.0400
114.11 250.94 0.0400
130.41 248.58 0.0400
141.28 247.50 0.0400
154.86 246.87 0.0400
155.20 246.70 0.0400
160.20 246.30 0.0400 /0.0300 Main Channel
160.30 245.30 0.0300 Main Channel
165.20 245.30 0.0300 Main Channel
165.30 246.30 0.0300 /0.0400 Main Channel
168.45 246.96 0.0400
195.62 249.20 0.0400
203.77 250.82 0.0400
225.50 256.78 0.0400
244.52 261.14 0.0400
255.39 261.95 0.0400
268.97 261.92 0.0400

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-----
<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.50 245.80 .120E+05 2.9 1.18 68.31
1.00 246.30 .242E+05 8.5 1.70 47.47
1.92 247.22 .825E+05 31.7 1.86 43.34
2.84 248.14 .248E+06 102.5 2.00 40.39
3.76 249.06 .506E+06 247.7 2.37 34.03
4.68 249.98 .832E+06 486.8 2.83 28.49
5.60 250.90 .121E+07 814.9 3.27 24.71
6.52 251.82 .180E+07 1157.1 3.11 25.97
7.44 252.74 .253E+07 1840.2 3.52 22.92
8.36 253.66 .331E+07 2727.1 3.98 20.25
9.29 254.59 .418E+07 3792.6 4.43 18.22
10.21 255.51 .499E+07 5010.7 4.86 16.61
11.13 256.43 .589E+07 6397.4 5.26 15.33
12.05 257.35 .681E+07 7932.8 5.64 14.31
12.97 258.27 .777E+07 9621.2 5.99 13.46
13.89 259.19 .877E+07 11469.4 6.33 12.74
14.81 260.11 .980E+07 13478.4 6.66 12.12
15.73 261.03 .109E+08 15651.7 6.97 11.58
16.65 261.95 .120E+08 17492.5 7.07 11.42

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-----
<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8124) 1110.54 1.69 7.75 3.38 0.29 1.18
OUTFLOW: ID= 1 (6160) 1110.54 1.47 9.25 3.38 0.25 1.18

```

```

-----
ADD HYD (8126)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0160): 318.82 0.745 8.25 6.83
+ ID2= 2 (6160): 1110.54 1.470 9.25 3.38
=====
ID = 3 (8126): 1429.36 2.196 9.00 4.15

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ADD HYD (8128)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0162): 190.14 0.776 7.00 7.50
+ ID2= 2 (8126): 1429.36 2.196 9.00 4.15
=====
ID = 3 (8128): 1619.50 2.802 8.50 4.54

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ADD HYD (8130)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8128): 1619.50 2.802 8.50 4.54
+ ID2= 2 (8132): 234.20 3.004 6.00 20.69
=====
ID = 3 (8130): 1853.70 4.877 7.00 6.58

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ROUTE CHN (6166)
IN# 2--> OTR# 1 Routing time step (min)'= 15.00

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-----
<----- DATA FOR SECTION (1661.0) ----->
Distance Elevation Manning
0.00 248.36 0.0500
41.67 246.98 0.0500
95.82 244.93 0.0500
149.98 242.42 0.0500
191.64 241.88 0.0500
224.97 239.90 0.0500
229.13 239.35 0.0500
233.30 237.77 0.0500
237.47 236.57 0.0500
240.63 235.60 0.0500 /0.0300 Main Channel
241.63 234.10 0.0300 Main Channel
247.13 234.10 0.0300 Main Channel
247.53 235.60 0.0300 /0.0500 Main Channel
254.13 236.25 0.0500
266.63 236.77 0.0500
283.29 237.84 0.0500
291.63 238.89 0.0500
329.12 244.66 0.0500
370.78 249.55 0.0500
412.44 253.17 0.0500

```

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-----
<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.75 234.85 .130E+05 6.0 1.36 36.45
1.50 235.60 .276E+05 18.1 1.95 25.41
2.25 236.35 .544E+05 40.0 2.18 22.69
3.00 237.10 .117E+06 80.1 2.04 24.32
3.75 237.85 .213E+06 148.9 2.07 23.89
4.50 238.60 .334E+06 255.1 2.27 21.85
5.25 239.35 .473E+06 395.8 2.49 19.90
6.00 240.10 .631E+06 563.8 2.65 18.66
6.75 240.85 .824E+06 773.2 2.79 17.77
7.50 241.60 .106E+07 1037.9 2.92 16.97
8.26 242.36 .135E+07 1326.7 2.92 16.94
9.01 243.11 .173E+07 1758.5 3.01 16.44
9.76 243.86 .217E+07 2285.9 3.13 15.81
10.51 244.61 .265E+07 2909.5 3.26 15.17
11.26 245.36 .318E+07 3610.1 3.37 14.67
12.01 246.11 .377E+07 4417.4 3.48 14.21
12.76 246.86 .441E+07 5348.0 3.60 13.75
13.51 247.61 .512E+07 6394.0 3.71 13.34
14.26 248.36 .589E+07 7580.3 3.82 12.95

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-----
<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8130) 1853.70 4.88 7.00 6.58 0.61 1.36
OUTFLOW: ID= 1 (6166) 1853.70 4.30 7.75 6.58 0.54 1.36

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-----
ADD HYD (8136)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (6166): 1853.70 4.303 7.75 6.58
+ ID2= 2 (8134): 434.70 6.384 6.00 20.46
=====
ID = 3 (8136): 2288.40 7.588 6.00 9.22

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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CALIB
STANDHYD (2021) Area (ha)= 91.20
ID= 1 DT=15.0 min Total Imp(%)= 40.00 Dir. Conn.(%)= 24.00

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IMPERVIOUS PERVIOUS (I)
Surface Area (ha)= 36.48 54.72
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50

```

Length (m)= 779.74 40.00
 Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 55.44 15.13
 over (min) 15.00 45.00
 Storage Coeff. (min)= 13.65 (ii) 36.43 (iii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03

TOTALS
 PEAK FLOW (cms)= 2.55 1.29 3.071 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 40.00 12.90 19.41
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.31 0.46

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 73.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5202)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	4.0850	2.2999
	0.3100	0.9668	5.0850	2.6924
	1.2890	1.1373	5.9090	3.1012
	2.2140	1.5358	6.3090	3.1112
	3.0620	1.8918	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (2021)	91.200	3.071	6.00	19.41
OUTFLOW: ID= 1 (5202)	91.200	0.687	7.50	19.40

PEAK FLOW REDUCTION [Qout/qin](%)= 22.36
 TIME SHIFT OF PEAK FLOW (min)= 90.00
 MAXIMUM STORAGE USED (ha.m.)= 1.0352

CALIB
 STANDHYD (2022) Area (ha)= 377.30
 ID= 1 DT=15.0 min Total Imp(%)= 57.00 Dir. Conn.(%)= 34.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 215.06 162.24
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1585.98 40.00
 Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 37.80 20.23
 over (min) 30.00 45.00
 Storage Coeff. (min)= 24.37 (ii) 44.64 (iii)
 Unit Hyd. Tpeak (min)= 30.00 45.00
 Unit Hyd. peak (cms)= 0.04 0.03

TOTALS
 PEAK FLOW (cms)= 9.72 4.56 13.042 (iii)
 TIME TO PEAK (hrs)= 6.25 6.50 6.25
 RUNOFF VOLUME (mm)= 40.00 14.12 22.92
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.34 0.55

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 71.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8110)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID= 1 (2022):	377.30	13.042	6.25	22.92
+ ID= 2 (5202):	91.20	0.687	7.50	19.40
ID = 3 (8110):	468.50	13.235	6.25	22.23

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (0102) Area (ha)= 466.00
 ID= 1 DT=15.0 min Total Imp(%)= 23.00 Dir. Conn.(%)= 9.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 107.18 358.82
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1762.57 40.00
 Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 37.80 4.37
 over (min) 30.00 75.00
 Storage Coeff. (min)= 25.96 (ii) 63.38 (iii)
 Unit Hyd. Tpeak (min)= 30.00 75.00
 Unit Hyd. peak (cms)= 0.04 0.02

TOTALS
 PEAK FLOW (cms)= 3.09 2.11 3.952 (iii)
 TIME TO PEAK (hrs)= 6.25 7.00 6.25
 RUNOFF VOLUME (mm)= 40.00 5.46 8.56
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.13 0.20

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 49.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHVD (0100) Area (ha)= 693.84 Curve Number (CN)= 40.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.95

Unit Hyd Qpeak (cms)= 6.087

PEAK FLOW (cms)= 0.615 (i)
 TIME TO PEAK (hrs)= 9.50
 RUNOFF VOLUME (mm)= 3.263
 TOTAL RAINFALL (mm)= 42.00
 RUNOFF COEFFICIENT = 0.078

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8000)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID= 1 (0100): 693.84 0.615 9.50 3.26
 + ID= 2 (0102): 466.00 3.952 6.25 8.56
 ID = 3 (8000): 1159.84 4.251 6.25 5.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6104)
 IN= 2--> OUT= 1 Routing time step (min)= 15.00

DATA FOR SECTION (1041.0)				
Distance	Elevation	Manning		
0.00	330.16	0.0450		
50.03	328.36	0.0450		
89.34	326.33	0.0450		
117.94	324.30	0.0450		
160.82	322.75	0.0450		
178.34	319.96	0.0450		
183.34	319.81	0.0450		
184.34	319.38	0.0450 / 0.0300	Main Channel	
185.09	318.78	0.0300	Main Channel	
185.84	318.72	0.0300	Main Channel	
186.84	319.32	0.0300 / 0.0450	Main Channel	
193.84	319.70	0.0450		
198.84	320.38	0.0450		
200.13	320.81	0.0450		
218.00	322.49	0.0450		
239.44	323.49	0.0450		
250.17	323.96	0.0450		
275.18	325.20	0.0450		
310.92	326.47	0.0450		
353.81	330.00	0.0450		

TRAVEL TIME TABLE					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.30	319.02	.624E+03	0.1	0.45	73.00
0.60	319.32	1.81E+04	0.6	0.67	49.46
1.23	319.85	1.08E+05	4.9	0.77	43.38
1.86	320.58	3.96E+05	19.1	0.96	34.50
2.48	321.20	7.57E+05	43.5	1.15	28.98
3.11	321.83	1.25E+06	81.2	1.30	25.58

3.74	322.46	.187E+06	135.9	1.45	22.92
4.37	323.09	.269E+06	195.6	1.45	22.90
5.00	323.72	.387E+06	287.8	1.48	22.44
5.63	324.35	.545E+06	428.4	1.56	21.22
6.25	324.97	.735E+06	630.5	1.71	19.42
6.88	325.60	.952E+06	876.1	1.83	18.12
7.51	326.23	.120E+07	1177.0	1.95	17.03
8.14	326.86	.149E+07	1554.4	2.08	15.93
8.77	327.49	.179E+07	2009.7	2.23	14.88
9.40	328.12	.213E+07	2533.9	2.37	13.99
10.02	328.74	.249E+07	3112.6	2.49	13.31
10.65	329.37	.288E+07	3760.9	2.60	12.75
11.28	330.00	.330E+07	4494.8	2.71	12.23

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(cms) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8000) 1159.84 4.25 6.25 5.39 1.13 0.75
OUTFLOW: ID= 1 (6104) 1159.84 3.09 7.25 5.39 0.96 0.72

```

```

-----
| CALIB |
| NASHVD (0104) | Area (ha)= 527.81 Curve Number (CN)= 43.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.34
-----

```

```

Unit Hyd Qpeak (cms)= 6.735
PEAK FLOW (cms)= 0.693 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 3.639
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.087

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8002) |
| 1 + 2 = 3 |
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0104): 527.81 0.693 8.00 3.64
+ ID2= 2 (6104): 1159.84 3.088 7.25 5.39
=====
ID = 3 (8002): 1687.65 3.736 7.25 4.84
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHVD (0106) | Area (ha)= 259.52 Curve Number (CN)= 55.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.23
-----

```

```

Unit Hyd Qpeak (cms)= 3.614
PEAK FLOW (cms)= 0.559 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 5.550
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.132

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8004) |
| 1 + 2 = 3 |
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0106): 259.52 0.559 8.00 5.55
+ ID2= 2 (8002): 1687.65 3.736 7.25 4.84
=====
ID = 3 (8004): 1947.17 4.271 7.25 4.94
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6108) |
| IN= 2----> OUT= 1 | Routing time step (min)'= 15.00
-----

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```

<----- DATA FOR SECTION (1081.0) ----->
Distance Elevation Manning
0.00 326.06 0.0400
12.06 325.74 0.0400
30.15 324.28 0.0400
59.27 320.35 0.0400
72.37 317.60 0.0400
87.45 314.52 0.0400
93.48 313.91 0.0400

```

105.54	313.55	0.0400
106.17	313.40	0.0400 / 0.0300 Main Channel
110.62	313.04	0.0300 Main Channel
110.82	312.97	0.0300 Main Channel
111.57	313.08	0.0300 Main Channel
111.67	313.48	0.0300 / 0.0400 Main Channel
129.66	316.62	0.0400
150.77	318.95	0.0400
180.92	319.61	0.0400
205.04	321.23	0.0400
232.18	322.09	0.0400
268.37	322.31	0.0400
298.52	326.00	0.0400

```

-----
<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m) (cms) (m/s) (min)
0.43 313.40 .244E+04 0.7 0.59 58.83
1.09 314.06 .214E+05 10.4 1.01 34.24
1.76 314.73 .600E+05 40.2 1.39 24.89
2.42 315.39 .109E+06 92.1 1.75 19.80
3.08 316.05 .169E+06 166.5 2.05 16.88
3.75 316.72 .237E+06 264.4 2.31 14.97
4.41 317.38 .318E+06 386.3 2.52 13.72
5.07 318.04 .411E+06 539.3 2.72 12.71
5.74 318.71 .517E+06 725.5 2.91 11.88
6.40 319.37 .642E+06 909.2 2.94 11.77
7.06 320.03 .806E+06 1179.3 3.04 11.38
7.72 320.69 .989E+06 1515.9 3.18 10.87
8.39 321.36 .119E+07 1895.5 3.30 10.49
9.05 322.02 .143E+07 2310.2 3.36 10.30
9.71 322.68 .172E+07 2763.2 3.33 10.39
10.38 323.35 .205E+07 3477.6 3.53 9.81
11.04 324.01 .238E+07 4284.3 3.73 9.27
11.70 324.67 .274E+07 5154.0 3.91 8.85
12.37 325.34 .311E+07 6104.0 4.08 8.48
-----

```

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8004) 1947.17 4.27 7.25 4.94 0.67 0.70
OUTFLOW: ID= 1 (6108) 1947.17 3.74 8.25 4.94 0.64 0.68

```

```

-----
| CALIB |
| NASHVD (0108) | Area (ha)= 207.05 Curve Number (CN)= 27.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.03
-----

```

```

Unit Hyd Qpeak (cms)= 3.432
PEAK FLOW (cms)= 0.168 (1)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 1.873
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.045

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8006) |
| 1 + 2 = 3 |
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0108): 207.05 0.168 7.75 1.87
+ ID2= 2 (6108): 1947.17 3.740 8.25 4.94
=====
ID = 3 (8006): 2154.22 3.903 8.25 4.64
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHVD (0110) | Area (ha)= 323.92 Curve Number (CN)= 52.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.35
-----

```

```

Unit Hyd Qpeak (cms)= 4.087
PEAK FLOW (cms)= 0.583 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 5.010
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.119

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8008) |
-----

```


1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0110):	323.92	0.583	8.00	5.01
+ ID2= 2 (8006):	2154.22	3.903	8.25	4.64
=====				
ID = 3 (8008):	2478.14	4.486	8.25	4.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6112) |
| ID= 2 -> OUT= 1 |
Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1121.0) ----->

Distance	Elevation	Manning			
0.00	320.59	0.0400			
13.48	317.93	0.0400			
40.45	311.52	0.0400			
64.04	307.68	0.0400			
74.15	307.01	0.0400			
91.00	307.22	0.0400			
107.86	305.21	0.0400			
111.23	304.86	0.0400			
113.32	304.60	0.0400 /0.0300	Main Channel		
117.77	304.24	0.0300	Main Channel		
117.97	304.17	0.0300	Main Channel		
118.72	304.28	0.0300	Main Channel		
118.82	304.68	0.0300 /0.0400	Main Channel		
124.71	304.76	0.0400			
171.90	306.30	0.0400			
205.60	307.24	0.0400			
239.31	308.24	0.0400			
262.90	310.64	0.0400			
289.86	314.34	0.0400			
333.68	317.66	0.0400			

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.43	304.60	.372E+04	0.6	0.54	97.65
1.12	305.29	.463E+05	12.5	0.85	61.89
1.80	305.97	.149E+06	53.0	1.13	46.85
2.49	306.66	.311E+06	134.2	1.37	38.62
3.18	307.35	.554E+06	261.6	1.50	35.27
3.87	308.04	.894E+06	490.7	1.74	30.37
4.55	308.72	.139E+07	831.0	2.05	25.75
5.24	309.41	.171E+07	1269.8	2.36	22.39
5.93	310.10	.215E+07	1789.0	2.64	20.01
6.62	310.79	.261E+07	2392.7	2.90	18.21
7.30	311.47	.310E+07	3087.8	3.16	16.74
7.99	312.16	.361E+07	3873.6	3.40	15.52
8.68	312.85	.413E+07	4741.0	3.64	14.52
9.37	313.54	.467E+07	5689.3	3.86	13.69
10.05	314.22	.523E+07	6718.4	4.07	12.97
10.74	314.91	.581E+07	7770.7	4.24	12.46
11.43	315.60	.641E+07	8897.5	4.40	12.01
12.12	316.29	.704E+07	10113.2	4.55	11.61
12.80	316.97	.770E+07	11420.1	4.70	11.24

<----- hydrograph -----> <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8008) 2478.14	4.49	8.25	4.69	0.65	0.61
OUTFLOW: ID= 1 (6112) 2478.14	3.74	10.00	4.69	0.61	0.60

CALIB
NASHVD (0112) | Area (ha)= 414.89 Curve Number (CN)= 30.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.91

Unit Hyd Qpeak (cms)= 3.699

PEAK FLOW (cms)= 0.247 (i)
TIME TO PEAK (hrs)= 9.500
RUNOFF VOLUME (mm)= 2.166
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.052

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8010) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0112): 414.89 0.247 9.50 2.17
+ ID2= 2 (6112): 2478.14 3.736 10.00 4.69
=====
ID = 3 (8010): 2893.03 3.981 10.00 4.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6114) |
ID= 2 -> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1141.0) ----->

Distance	Elevation	Manning			
0.00	312.27	0.0500			
14.07	309.70	0.0500			
28.14	306.51	0.0500			
35.17	305.81	0.0500			
42.20	304.95	0.0500			
52.75	304.24	0.0500			
77.37	303.61	0.0500			
179.36	302.20	0.0500			
198.47	301.33	0.0500			
131.76	300.40	0.0500 /0.0300	Main Channel		
136.21	300.04	0.0300	Main Channel		
136.41	299.97	0.0300	Main Channel		
137.16	300.08	0.0300	Main Channel		
137.26	300.48	0.0300 /0.0500	Main Channel		
140.68	300.53	0.0500			
179.36	302.20	0.0500			
214.53	303.82	0.0500			
242.66	305.27	0.0500			
288.38	307.66	0.0500			
316.52	311.76	0.0500			
348.17	313.48	0.0500			

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.43	300.40	.366E+04	0.7	0.59	88.31
1.05	301.02	.505E+05	12.1	0.75	69.49
1.68	301.65	.165E+06	52.7	0.99	52.28
2.30	302.27	.324E+06	130.3	1.26	41.40
2.93	302.90	.520E+06	247.1	1.48	35.08
3.55	303.52	.754E+06	406.8	1.68	30.91
4.18	304.15	.104E+07	586.1	1.76	29.55
4.80	304.77	.138E+07	856.0	1.93	26.96
5.43	305.40	.177E+07	1201.1	2.12	24.56
6.05	306.02	.219E+07	1612.2	2.30	22.63
6.68	306.65	.264E+07	2090.6	2.47	21.07
7.30	307.27	.313E+07	2653.0	2.64	19.65
7.93	307.90	.364E+07	3303.9	2.83	18.36
8.55	308.52	.417E+07	4053.9	3.03	17.14
9.18	309.15	.471E+07	4871.9	3.22	16.12
9.80	309.77	.527E+07	5756.2	3.41	15.25
10.43	310.40	.584E+07	6699.8	3.58	14.52
11.05	311.02	.642E+07	7710.5	3.74	13.89
11.68	311.65	.702E+07	8788.5	3.90	13.32

<----- hydrograph -----> <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8010) 2893.03	3.98	10.00	4.33	0.61	0.63
OUTFLOW: ID= 1 (6114) 2893.03	3.60	11.50	4.33	0.59	0.62

CALIB
NASHVD (0114) | Area (ha)= 287.65 Curve Number (CN)= 49.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.12

Unit Hyd Qpeak (cms)= 4.389

PEAK FLOW (cms)= 0.534 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 4.503
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.107

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8012) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0114): 287.65 0.534 8.00 4.50
+ ID2= 2 (6114): 2893.03 3.600 11.50 4.33
=====
ID = 3 (8012): 3180.68 3.935 11.25 4.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHVD (0116) | Area (ha)= 206.43 Curve Number (CN)= 64.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50

U.H. Tp(hrs)= 0.80
 Unit Hyd Qpeak (cms)= 4.394
 PEAK FLOW (cms)= 0.833 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 7.503
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.179

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8014)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0116):	206.43	0.833	7.00	7.50
+ ID2= 2 (8012):	3180.68	3.935	11.25	4.35
ID = 3 (8014):	3387.11	4.302	11.00	4.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6118) | Routing time step (min)'= 15.00
 IN= 2--> OUT= 1 |

Distance	Elevation	Manning
0.00	307.18	0.0450
24.89	299.75	0.0450
37.34	296.39	0.0450
71.57	292.75	0.0450
96.46	291.12	0.0450
108.90	289.43	0.0450
112.02	288.86	0.0450
115.13	288.46	0.0450
117.24	288.32	0.0450 / 0.0350 Main Channel
117.49	288.15	0.0350 Main Channel
118.24	288.00	0.0350 Main Channel
118.99	288.20	0.0350 Main Channel
119.24	288.36	0.0350 / 0.0450 Main Channel
121.35	288.40	0.0450
149.35	289.36	0.0450
171.14	290.47	0.0450
208.47	293.14	0.0450
236.48	295.17	0.0450
280.04	299.48	0.0450
308.04	307.00	0.0450

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.32	288.32	1.16E+04	0.2	0.62	81.92
1.30	289.30	6.11E+05	22.2	1.07	47.43
2.29	290.29	2.18E+06	117.5	1.64	30.95
3.27	291.27	4.48E+06	314.6	2.15	23.72
4.25	292.25	7.56E+06	625.6	2.53	20.13
5.24	293.24	1.15E+07	1093.2	2.91	17.50
6.22	294.22	1.61E+07	1731.6	3.28	15.50
7.20	295.20	2.14E+07	2547.4	3.63	14.01
8.19	296.19	2.74E+07	3584.9	4.00	12.72
9.17	297.17	3.38E+07	4859.1	4.39	11.60
10.15	298.15	4.07E+07	6341.5	4.76	10.70
11.13	299.13	4.80E+07	8024.6	5.11	9.97
12.12	300.12	5.56E+07	10006.6	5.49	9.27
13.10	301.10	6.35E+07	12244.6	5.89	8.64
14.08	302.08	7.16E+07	14681.7	6.26	8.13
15.07	303.07	7.99E+07	17316.5	6.62	7.69
16.05	304.05	8.84E+07	20145.1	6.96	7.31
17.03	305.03	9.71E+07	23176.2	7.29	6.98
18.02	306.02	1.06E+08	26400.8	7.61	6.69

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8014) 3387.11	4.30	11.00	4.54	0.50	0.67
OUTFLOW: ID= 1 (6118) 3387.11	4.07	12.25	4.54	0.49	0.67

CALIB (0118)	Area (ha)	Curve Number (CN)= 45.0
NASHYD (0118)	340.53	
ID= 1 DT=15.0 min	1a (mm)= 5.00	# of Linear Res. (N)= 1.50
U.H. Tp(hrs)= 1.12		

Unit Hyd Qpeak (cms)= 5.200
 PEAK FLOW (cms)= 0.548 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 3.906

TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.093

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8016)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0118):	340.53	0.548	8.00	3.91
+ ID2= 2 (6118):	3387.11	4.066	12.25	4.54
ID = 3 (8016):	3727.64	4.365	12.00	4.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6122) | Routing time step (min)'= 15.00
 IN= 2--> OUT= 1 |

Distance	Elevation	Manning
0.00	277.00	0.0500
52.18	272.82	0.0500
86.97	269.38	0.0500
134.80	267.35	0.0500
160.89	263.85	0.0500
169.58	262.88	0.0500
178.28	262.45	0.0500
195.67	262.07	0.0500
213.07	260.99	0.0500
226.01	259.80	0.0500 / 0.0300 Main Channel
226.11	259.30	0.0300 Main Channel
230.11	259.30	0.0300 Main Channel
230.21	259.80	0.0300 / 0.0500 Main Channel
235.01	250.20	0.0500
239.15	260.57	0.0500
243.50	260.93	0.0500
300.03	264.43	0.0500
339.16	268.34	0.0500
373.95	272.36	0.0500
430.48	277.46	0.0500

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.50	259.80	6.05E+04	2.5	1.23	41.35
1.41	260.71	4.62E+05	21.8	1.44	35.07
2.31	261.61	1.49E+06	79.6	1.63	31.19
3.22	262.52	3.40E+06	200.1	1.80	28.34
4.12	263.42	6.05E+06	446.8	2.15	23.71
5.03	264.33	9.95E+06	819.9	2.52	20.23
5.93	265.23	1.41E+07	1337.0	2.90	17.56
6.84	266.14	1.87E+07	1984.8	3.25	15.67
7.74	267.04	2.37E+07	2765.0	3.57	14.26
8.65	267.95	2.92E+07	3596.9	3.76	13.53
9.55	268.85	3.55E+07	4575.4	3.93	12.95
10.46	269.76	4.27E+07	5814.9	4.16	12.23
11.36	270.66	5.03E+07	7319.0	4.44	11.46
12.27	271.57	5.84E+07	9009.7	4.71	10.81
13.17	272.47	6.70E+07	10886.3	4.96	10.26
14.08	273.38	7.61E+07	12893.2	5.17	9.84
14.98	274.28	8.58E+07	15098.1	5.37	9.48
15.89	275.19	9.61E+07	17527.6	5.57	9.14
16.79	276.09	1.07E+08	20190.3	5.76	8.83

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8016) 3727.64	4.37	12.00	4.48	0.59	1.25
OUTFLOW: ID= 1 (6122) 3727.64	4.29	12.75	4.48	0.58	1.25

CALIB (1221)	Area (ha)	Imperious (%)	Pervious (%)
STANDHYD (1221)	17.70		
ID= 1 DT=15.0 min	Total Imp(%)= 36.00	Dir. Com.(%)= 19.00	

Surface Area (ha)	Imperious (%)	Pervious (%)
6.37		11.33
Dep. Storage (mm)= 2.00		5.00
Average Slope (%)= 0.50		0.50
Length (m)= 343.51		40.00
Mannings n = 0.013		0.250
Max. Eff. Inten. (mm/hr)= 55.44		3.00
over (min)= 15.00		60.00
Storage Coeff. (min)= 8.35 (ii)		51.86 (ii)
Unit Hyd. Tpeak (min)= 15.00		60.00
Unit Hyd. peak (cms)= 0.09		0.02

TOTALS

PEAK FLOW (cms)= 0.46 0.05 0.474 (iii)
 TIME TO PEAK (hrs)= 6.00 6.75 6.00
 RUNOFF VOLUME (mm)= 40.00 3.52 10.45
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.08 0.25

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 35.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5122)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	1.1840	0.4294
	0.0600	0.1855	1.4870	0.5014
	0.3430	0.2103	1.7310	0.5718
	0.6390	0.2878	2.1310	0.5818
	0.8860	0.3509	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (1221)	17.700	0.474	6.00	10.45
OUTFLOW: ID= 1 (5122)	17.700	0.038	8.25	10.41

PEAK FLOW REDUCTION [Qout/Qin](%)= 8.00
 TIME SHIFT OF PEAK FLOW (min)=135.00
 MAXIMUM STORAGE USED (ha.m.)= 0.1174

CALIB				
NASHVD (1222)				
ID= 1 DT=15.0 min				
	Area	(ha)	Curve Number	(CN)= 41.0
	Ia	(mm)=	5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)=	1.52		

Unit Hyd Qpeak (cms)= 3.887

PEAK FLOW (cms)= 0.383 (i)
 TIME TO PEAK (hrs)= 8.500
 RUNOFF VOLUME (mm)= 3.383
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.081

- (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8018)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1222):	347.20	0.383	8.50	3.38
+ ID2= 2 (5122):	17.70	0.038	8.25	10.41
ID = 3 (8018):	364.90	0.421	8.50	3.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8020)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6122):	3727.64	4.294	12.75	4.48
+ ID2= 2 (8018):	364.90	0.421	8.50	3.72
ID = 3 (8020):	4092.54	4.584	12.50	4.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB				
STANDHYD (1261)				
ID= 1 DT=15.0 min				
	Area	(ha)=	50.00	
	Total Imp(%)=	36.00	Dir. Conn.(%)=	26.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	18.00	32.00
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	577.35	40.00
Mannings n =	0.013	0.250
Max.Eff.Inten.(mm/hr)=	55.44	4.03
over (min)	15.00	60.00

Storage Coeff. (min)= 11.40 (ii) 50.04 (ii)
 Unit Hyd. Tpeak (min)= 15.00 60.00
 Unit Hyd. peak (cms)= 0.08 0.02

PEAK FLOW (cms)= 1.62 0.21 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.75 1.679 (iii)
 RUNOFF VOLUME (mm)= 40.00 5.15 6.00
 TOTAL RAINFALL (mm)= 42.00 42.00 14.21
 RUNOFF COEFFICIENT = 0.95 0.12 42.00
 0.34

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5126)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	3.3450	1.2129
	0.1690	0.5421	4.2000	1.4164
	0.9690	0.5941	4.8890	1.6153
	1.8040	0.8130	5.2890	1.6253
	2.5020	0.9912	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (1261)	50.000	1.679	6.00	14.21
OUTFLOW: ID= 1 (5126)	50.000	0.142	8.50	14.19

PEAK FLOW REDUCTION [Qout/Qin](%)= 8.48
 TIME SHIFT OF PEAK FLOW (min)=150.00
 MAXIMUM STORAGE USED (ha.m.)= 0.4572

CALIB				
NASHVD (1262)				
ID= 1 DT=15.0 min				
	Area	(ha)=	83.50	Curve Number (CN)= 58.0
	Ia	(mm)=	5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)=	0.56		

Unit Hyd Qpeak (cms)= 2.523

PEAK FLOW (cms)= 0.354 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 6.049
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.144

- (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8024)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1262):	83.50	0.354	6.75	6.05
+ ID2= 2 (5126):	50.00	0.142	8.50	14.19
ID = 3 (8024):	133.50	0.477	6.75	9.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB				
NASHVD (0124)				
ID= 1 DT=15.0 min				
	Area	(ha)=	121.43	Curve Number (CN)= 43.0
	Ia	(mm)=	5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)=	0.51		

Unit Hyd Qpeak (cms)= 4.049

PEAK FLOW (cms)= 0.324 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 3.563
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.085

- (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8026)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0124):	121.43	0.324	6.50	3.56
+ ID2= 2 (8024):	133.50	0.477	6.75	9.10

ID = 3 (8026): 254.93 0.797 6.75 6.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6128) | IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

Table with columns: Distance, Elevation, Manning, and Main Channel. Rows show data points from 0.00 to 190.92.

TRAVEL TIME TABLE

Table with columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV. TIME. Rows show depth and travel time data from 0.65 to 16.30.

hydrograph table with columns: AREA, QPEAK, TPEAK, R.V., MAX DEPTH, MAX VEL. Includes INFLOW and OUTFLOW data.

CALIB (0128) | Area (ha)= 240.24 | Curve Number (CN)= 62.0 | IN= 1 DT=15.0 min | U.H. Tp(hrs)= 1.00

Unit Hyd Qpeak (cms) = 4.118 | PEAK FLOW (cms) = 0.765 (i) | TIME TO PEAK (hrs) = 7.500 | RUNOFF VOLUME (mm) = 7.032 | TOTAL RAINFALL (mm) = 42.000 | RUNOFF COEFFICIENT = 0.167

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8028) | 1 + 2 = 3 | AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm) | ID1= 1 (0128): 240.24 0.765 7.50 7.03 | ID2= 2 (6128): 254.93 0.703 7.50 6.46 | ID = 3 (8028): 495.17 1.468 7.50 6.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8022) | 1 + 2 = 3 | AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm) | ID1= 1 (8020): 4092.54 4.584 12.50 4.41 | ID2= 2 (8028): 495.17 1.468 7.50 6.74 | ID = 3 (8022): 4587.71 5.203 12.00 4.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (1201) | Area (ha)= 86.00 | Total Imp(%)= 36.00 | Dir. Conn.(%)= 14.00 | ID= 1 DT=15.0 min

IMPERVIOUS PERVIOUS (i) | Surface Area (ha)= 30.96 55.04 | Dep. Storage (mm)= 2.00 5.00 | Average Slope (ft)= 0.50 0.50 | Length (ft)= 757.19 40.00 | Mannings n = 0.013 0.250 | Max. Eff. Inten. (mm/hr)= 55.44 5.48 | over (min)= 15.00 60.00 | Storage Coeff. (min)= 13.42 (ii) 47.59 (ii) | Unit Hyd. Tpeak (min)= 15.00 60.00 | Unit Hyd. peak (cms)= 0.08 0.02

PEAK FLOW (cms)= 1.41 0.50 1.560 (iii) | TIME TO PEAK (hrs)= 6.00 6.75 6.00 | RUNOFF VOLUME (mm)= 40.00 6.03 10.78 | TOTAL RAINFALL (mm)= 42.00 42.00 42.00 | RUNOFF COEFFICIENT = 0.95 0.14 0.26

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP! ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 48.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5120) | IN= 2--> OUT= 1 | DT= 15.0 min | OUTFLOW STORAGE OUTFLOW STORAGE (cms) (ha.m.) (cms) (ha.m.) | 0.0000 0.0000 5.7530 2.0863 | 0.2910 0.9015 7.2230 2.4361 | 1.6660 1.0219 8.4090 2.7784 | 3.1030 1.3984 8.8090 2.7884 | 4.3040 1.7049 0.0000 0.0000

AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm) | INFLOW : ID= 2 (1201) 86.000 1.560 6.00 10.78 | OUTFLOW: ID= 1 (5120) 86.000 0.187 9.00 10.78

PEAK FLOW REDUCTION [Qout/Qin](%)= 12.00 | TIME SHIFT OF PEAK FLOW (min)=180.00 | MAXIMUM STORAGE USED (ha.m.)= 0.5800

CALIB (1202) | Area (ha)= 161.00 | Curve Number (CN)= 58.0 | IN= 1 DT=15.0 min | U.H. Tp(hrs)= 0.70

Unit Hyd Qpeak (cms) = 3.932 | PEAK FLOW (cms) = 0.582 (i) | TIME TO PEAK (hrs) = 7.000 | RUNOFF VOLUME (mm) = 6.089 | TOTAL RAINFALL (mm) = 42.000 | RUNOFF COEFFICIENT = 0.145

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8030) | 1 + 2 = 3 | AREA QPEAK TPEAK R.V. (ha) (cms) (hrs) (mm) | ID1= 1 (1202): 161.00 0.582 7.00 6.09 | ID2= 2 (5120): 86.00 0.187 9.00 10.78 | ID = 3 (8030): 247.00 0.733 7.00 7.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RUNOFF COEFFICIENT = 0.95 0.37 0.64

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CN = 74.0 Is = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8032)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8022):	4587.71	5.203	12.00	4.66
+ ID2= 2 (8030):	247.00	0.733	7.00	7.72
=====				
ID = 3 (8032):	4834.71	5.546	12.00	4.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTER CHN (6130)
IN= 2--> OUTF= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (1301.0) -----

Distance	Elevation	Manning	AREA	QPEAK	TPEAK	R.V.
(m)	(m)		(ha)	(cms)	(hrs)	(mm)
0.00	257.18	0.0350				
62.45	254.70	0.0350				
109.39	252.74	0.0350				
150.92	251.55	0.0350				
192.56	248.96	0.0350				
239.39	247.16	0.0350				
291.84	246.00	0.0350				
296.84	245.00	0.0350				
298.84	245.00	0.0350 / 0.0300	Main Channel			
299.24	244.60	0.0300	Main Channel			
301.84	244.60	0.0300	Main Channel			
303.44	244.60	0.0300	Main Channel			
303.84	245.00	0.0300 / 0.0350	Main Channel			
306.84	245.00	0.0350				
309.84	246.00	0.0350				
317.46	247.22	0.0350				
348.68	254.83	0.0350				
359.09	256.70	0.0350				
452.76	258.23	0.0350				
515.22	258.33	0.0350				

----- TRAVEL TIME TABLE -----

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.40	245.00	.870E+04	1.2	0.64	124.12
1.04	245.64	4.68E+05	9.5	0.96	81.81
1.68	246.28	1.09E+06	25.6	1.11	70.86
2.32	246.92	2.57E+06	63.5	1.17	67.51
2.96	247.56	4.98E+06	142.7	1.36	58.18
3.61	248.21	8.00E+06	266.7	1.58	50.01
4.25	248.85	1.16E+07	437.4	1.78	44.24
4.89	249.49	1.57E+07	669.1	2.01	39.20
5.53	250.13	2.03E+07	954.9	2.23	35.36
6.17	250.77	2.52E+07	1294.4	2.43	32.42
6.81	251.41	3.05E+07	1689.6	2.62	30.07
7.45	252.05	3.63E+07	2090.7	2.73	28.93
8.09	252.69	4.29E+07	2553.1	2.82	27.98
8.73	253.33	5.01E+07	3138.4	2.96	26.60
9.37	253.97	5.79E+07	3803.8	3.11	25.36
10.02	254.62	6.62E+07	4549.3	3.25	24.25
10.66	255.26	7.51E+07	5364.9	3.38	23.32
11.30	255.90	8.45E+07	6265.7	3.51	22.49
11.94	256.54	9.46E+07	7260.1	3.63	21.72

----- hydrograph ----- <-pipe / channel-->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8032) 4834.71	5.55	12.00	4.82	0.74	0.77
OUTFLOW : ID= 1 (6130) 4834.71	5.21	13.00	4.82	0.71	0.76

CALIB
STANDHYD (1301)
ID= 1 DT=15.0 min | Area (ha)= 228.20
Total Imp(%)= 65.00 Dir. Conn.(%)= 47.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	148.39	79.91
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1233.69	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	55.44	21.89
over (min)=	15.00	45.00
Storage Coeff. (min)=	17.98 (ii)	37.63 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.06	0.03
PEAK FLOW (cms)=	11.09	2.67
TIME TO PEAK (hrs)=	6.00	6.00
RUNOFF VOLUME (mm)=	40.00	15.34
TOTAL RAINFALL (mm)=	42.00	42.00

TOTALS

12.193 (iii)
6.00
26.93
42.00

RESERVOIR (5130)
ID= 2--> OUTF= 1 | DT= 15.0 min

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	12.4560	7.0142
0.8210	3.9074	14.9550	7.8554
4.8550	4.4209	22.4790	8.9550
7.0620	5.2214	22.8790	8.9650
9.5970	6.0904	0.0000	0.0000

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (1301) 228.300	12.193	6.00	26.93
OUTFLOW : ID= 1 (5130) 228.300	1.989	7.50	26.92

PEAK FLOW REDUCTION [Qout/Qin](%)= 16.31
TIME SHIFT OF PEAK FLOW (min)= 90.00
MAXIMUM STORAGE USED (ha.m.)= 4.0574

CALIB
NASHYD (1302) | Area (ha)= 157.80 Curve Number (CN)= 76.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U.H. Tp (hrs)= 1.82

Unit Hyd Qpeak (cms)= 1.479

PEAK FLOW (cms)= 0.533 (i)
TIME TO PEAK (hrs)= 8.750
RUNOFF VOLUME (mm)= 11.631
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.277

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8036) | AREA QPEAK TPEAK R.V.
1 + 2 = 3 | (ha) (cms) (hrs) (mm)
ID1= 1 (1302): 157.80 0.533 8.75 11.63
+ ID2= 2 (5130): 228.30 1.989 7.50 26.92
=====
ID = 3 (8036): 386.10 2.476 7.50 20.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8034) | AREA QPEAK TPEAK R.V.
1 + 2 = 3 | (ha) (cms) (hrs) (mm)
ID1= 1 (6130): 4834.71 5.211 13.00 4.82
+ ID2= 2 (8036): 386.10 2.476 7.50 20.67
=====
ID = 3 (8034): 5220.81 6.344 12.75 5.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (0188) | Area (ha)= 308.53
ID= 1 DT=15.0 min | Total Imp(%)= 41.00 Dir. Conn.(%)= 25.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	126.50	182.03
Dep. Storage (mm)=	3.00	5.00
Average Slope (%)=	0.10	0.10
Length (m)=	1434.18	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	37.80	6.09
over (min)=	30.00	105.00
Storage Coeff. (min)=	37.18 (ii)	90.28 (ii)
Unit Hyd. Tpeak (min)=	30.00	105.00
Unit Hyd. peak (cms)=	0.03	0.01
PEAK FLOW (cms)=	4.76	1.34
TIME TO PEAK (hrs)=	6.25	7.50
RUNOFF VOLUME (mm)=	39.00	8.19

TOTALS

5.148 (iii)
6.25
15.89

TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.93 0.20 0.38

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0186) | Area (ha)= 351.79 | Dir. Conn.(%)= 22.00
 ID= 1 DT=15.0 min | Total Imp(%)= 42.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	147.75	204.04	
Dep. Storage (mm)=	3.00	5.00	
Average Slope (%)=	0.10	0.10	
Length (m)=	1531.43	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	37.80	5.57	
over (min)	30.00	105.00	
Storage Coeff. (min)=	38.67 (ii)	93.70 (ii)	
Unit Hyd. Tpeak (min)=	30.00	105.00	
Unit Hyd. peak (cms)=	0.03	0.01	
			TOTALS
PEAK FLOW (cms)=	4.68	1.35	5.066 (iii)
TIME TO PEAK (hrs)=	6.25	7.50	6.25
RUNOFF VOLUME (mm)=	39.00	7.12	14.14
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.93	0.17	0.34

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 53.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8068) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0186): 351.79 5.066 6.25 14.14
 + ID2= 2 (0186): 308.53 5.148 6.25 15.89

 ID = 3 (8068): 660.32 10.213 6.25 14.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1821) | Area (ha)= 111.80 | Dir. Conn.(%)= 17.00
 ID= 1 DT=15.0 min | Total Imp(%)= 40.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	44.72	67.08	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	863.33	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	55.44	8.68	
over (min)	15.00	45.00	
Storage Coeff. (min)=	14.52 (ii)	42.96 (ii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.07	0.03	
			TOTALS
PEAK FLOW (cms)=	2.16	0.84	2.488 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	40.00	7.09	12.68
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.17	0.30

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 52.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5182) |
 IN= 2--> OUT= 1 |
 DT= 15.0 min | OUTFLOW STORAGE | OUTFLOW STORAGE

	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	5.7800	2.5656
	0.3790	1.0207	7.3720	3.0829
	1.6240	1.1939	8.7710	3.5956
	3.0930	1.6631	9.1710	3.6056
	4.3530	2.1032	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (1821)	111.800	2.488	6.00	12.68
OUTFLOW: ID= 1 (5182)	111.800	0.323	8.50	12.68

PEAK FLOW REDUCTION [Qout/Qin](%)= 12.96
 TIME SHIFT OF PEAK FLOW (min)=150.00
 MAXIMUM STORAGE USED (ha.m.)= 0.8691

CALIB
 NASHYD (1822) | Area (ha)= 44.40 | Curve Number (CN)= 57.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.79

Unit Hyd Qpeak (cms)= 0.966

PEAK FLOW (cms)= 0.142 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 5.901
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.140

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8062) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (1822): 44.40 0.142 7.00 5.90
 + ID2= 2 (5182): 111.80 0.323 8.50 12.68

 ID = 3 (8062): 156.20 0.450 8.00 10.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1841) | Area (ha)= 144.40 | Dir. Conn.(%)= 19.00
 ID= 1 DT=15.0 min | Total Imp(%)= 48.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	69.31	75.09	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	981.16	40.00	
Mannings n =	0.013	0.250	

Max.Eff.Inten.(mm/hr)=	55.44	12.63	
over (min)	15.00	45.00	
Storage Coeff. (min)=	15.67 (ii)	40.15 (ii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.07	0.03	
			TOTALS
PEAK FLOW (cms)=	3.02	1.41	3.582 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	40.00	9.01	14.90
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.21	0.35

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 56.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5184) |
 IN= 2--> OUT= 1 |
 DT= 15.0 min | OUTFLOW STORAGE | OUTFLOW STORAGE
 (cms) (ha.m.) (cms) (ha.m.)
 0.0000 0.0000 | 5.8820 2.3298
 0.2960 1.0006 | 7.4070 2.7386
 1.6530 1.1680 | 8.6830 3.1418
 3.0860 1.4687 | 9.0830 3.1518
 4.3760 1.8752 | 0.0000 0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (1841)	144.400	3.582	6.00	14.90

OUTFLOW: ID= 1 (5184) 144.400 1.183 7.25 14.90

PEAK FLOW REDUCTION [Qout/Qin](%)= 33.02
TIME SHIFT OF PEAK FLOW (min)= 75.00
MAXIMUM STORAGE USED (ha.m.)= 1.1160

CALIB	STANDHYD (1842)	Area (ha)= 118.10	Dir. Conn.(%)= 17.00
ID= 1 DT=15.0 min	Total Imp(%)= 42.00		
	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	49.60	68.50	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	887.32	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	55.44	10.66	
over (min)=	15.00	45.00	
Storage Coeff. (min)=	14.76 (ii)	40.95 (ii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.07	0.03	
	TOTALS		
PEAK FLOW (cms)=	2.27	1.08	2.693 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	40.00	8.33	13.71
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.20	0.33

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 56.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8060)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1842):	118.10	2.693	6.00	13.71
+ ID2= 2 (5184):	144.40	1.183	7.25	14.90
ID = 3 (8060):	262.50	2.829	6.00	14.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8064)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8060):	262.50	2.829	6.00	14.36
+ ID2= 2 (8062):	156.20	0.450	8.00	10.75
ID = 3 (8064):	418.70	3.032	6.00	13.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	STANDHYD (1781)	Area (ha)= 14.00	Dir. Conn.(%)= 30.00
ID= 1 DT=15.0 min	Total Imp(%)= 50.00		

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	7.00	7.00	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	305.51	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	55.44	20.38	
over (min)=	15.00	30.00	
Storage Coeff. (min)=	7.78 (ii)	27.99 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.10	0.04	
	TOTALS		
PEAK FLOW (cms)=	0.58	0.28	0.753 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	40.00	15.43	22.80
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.37	0.54

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

- CN* = 76.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5178)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IR= 2 -> OUTF= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min	0.0000	0.0000	0.9360	0.3780
	0.0470	0.1841	1.1760	0.4277
	0.2710	0.2148	1.3690	0.4763
	0.5050	0.2704	1.7690	0.4863
	0.7010	0.3197	0.0000	0.0000
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (1781)	14.000	0.753	6.00	22.80
OUTFLOW: ID= 1 (5178)	14.000	0.131	7.25	22.73

PEAK FLOW REDUCTION [Qout/Qin](%)= 17.43
TIME SHIFT OF PEAK FLOW (min)= 75.00
MAXIMUM STORAGE USED (ha.m.)= 0.1963

CALIB	NASHYD (1782)	Area (ha)= 261.40	Curve Number (CN)= 78.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50	
	U.H. Tp(hrs)= 1.15		

Unit Hyd Qpeak (cms)= 3.867

PEAK FLOW (cms)= 1.354 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 12.497
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.298

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8046)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1782):	261.40	1.354	7.75	12.50
+ ID2= 2 (5178):	14.00	0.131	7.25	22.73
ID = 3 (8046):	275.40	1.468	7.50	13.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	NASHYD (0180)	Area (ha)= 226.65	Curve Number (CN)= 65.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50	
	U.H. Tp(hrs)= 1.17		

Unit Hyd Qpeak (cms)= 3.299

PEAK FLOW (cms)= 0.714 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 7.815
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.186

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8050)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0180):	226.65	0.714	8.00	7.81
+ ID2= 2 (8046):	275.40	1.468	7.50	13.02
ID = 3 (8050):	502.05	2.174	7.50	10.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	STANDHYD (1741)	Area (ha)= 56.30	Dir. Conn.(%)= 18.00
ID= 1 DT=15.0 min	Total Imp(%)= 35.00		

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	19.70	36.60	
Dep. Storage (mm)=	2.00	5.00	

Average Slope (%) = 0.50 0.50
 Length (m) = 612.64 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr) = 55.44 10.47
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 11.82 (ii) 37.80 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.08 0.03
 TOTALS
 PEAK FLOW (cms) = 1.25 0.61 1.489 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 40.00 9.56 15.04
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.23 0.36

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 64.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5174)
 IN= 2--> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	1.1370	1.3070
	0.0580	0.5427	1.4280	1.5354
	0.3290	0.6196	1.6620	1.7593
	0.6130	0.8627	2.0620	1.7693
	0.8510	1.0621	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1741)	56.300	1.489	6.00	15.04
OUTFLOW: ID= 1 (5174)	56.300	0.213	8.25	15.00

PEAK FLOW REDUCTION [Qout/Qin](%) = 14.28
 TIME SHIFT OF PEAK FLOW (min)=135.00
 MAXIMUM STORAGE USED (ha.m.) = 0.5868

CALIB (1742) | Area (ha) = 82.80 Curve Number (CN) = 71.0
 NASHVD (1742) | Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs) = 0.69

Unit Hyd Qpeak (cms) = 2.055

PEAK FLOW (cms) = 0.484 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 9.554
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.227

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8044)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1742):	82.80	0.484	6.75	9.55
+ ID2= 2 (5174):	56.30	0.213	8.25	15.00
=====				
ID = 3 (8044):	139.10	0.614	8.00	11.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0172) | Area (ha) = 202.23 Curve Number (CN) = 78.0
 NASHVD (0172) | Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs) = 0.89

Unit Hyd Qpeak (cms) = 3.875

PEAK FLOW (cms) = 1.274 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 12.449
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.296

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0168) | Area (ha) = 247.13 Curve Number (CN) = 52.0
 NASHVD (0168) | Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs) = 0.75

Unit Hyd Qpeak (cms) = 5.611

PEAK FLOW (cms) = 0.686 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 4.965
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.118

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0170) | Area (ha) = 166.91 Curve Number (CN) = 65.0
 NASHVD (0170) | Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs) = 0.80

Unit Hyd Qpeak (cms) = 3.553

PEAK FLOW (cms) = 0.698 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 7.767
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.185

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8040)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0168):	247.13	0.686	7.00	4.96
+ ID2= 2 (0170):	166.91	0.698	7.00	7.77
=====				
ID = 3 (8040):	414.04	1.384	7.00	6.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8042)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0172):	202.23	1.274	7.25	12.45
+ ID2= 2 (8040):	414.04	1.384	7.00	6.09
=====				
ID = 3 (8042):	616.27	2.656	7.00	8.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6174) | Routing time step (min) = 15.00
 IN= 2--> OUT= 1 |

<----- DATA FOR SECTION (1741.0) ----->

Distance	Elevation	Manning
0.00	296.00	0.0450
13.36	288.94	0.0450
26.71	288.64	0.0450
51.19	287.96	0.0450
73.45	287.32	0.0450
86.81	286.48	0.0450
102.39	285.08	0.0450
110.73	282.76	0.0450
115.73	281.06	0.0450 / 0.0350 Main Channel
115.74	280.35	0.0350 Main Channel
120.73	280.35	0.0350 Main Channel
120.74	281.10	0.0350 / 0.0450 Main Channel
125.74	282.64	0.0450
135.78	286.87	0.0450
149.13	289.58	0.0450
162.49	292.29	0.0450
175.84	293.95	0.0450
195.88	295.13	0.0450
213.68	296.02	0.0450
220.36	296.09	0.0450

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.71	281.06	136E+05	6.0	1.69	37.89
1.50	281.85	358E+05	23.5	2.42	26.43
2.28	282.63	726E+05	53.8	2.85	22.47
3.07	283.42	124E+06	104.1	3.24	19.79
3.86	284.21	189E+06	176.7	3.60	17.81

4.64	284.99	.268E+06	274.9	3.94	16.26
5.43	285.78	.369E+06	399.8	4.07	15.73
6.21	286.56	.501E+06	551.6	4.23	15.15
7.00	287.35	.674E+06	750.5	4.28	14.97
7.79	288.14	.920E+06	993.3	4.15	15.44
8.57	288.92	1.26E+07	1361.5	4.14	15.47
9.36	289.71	1.67E+07	1986.0	4.56	14.04
10.15	290.50	2.10E+07	2739.7	5.02	12.76
10.93	291.28	2.54E+07	3615.0	5.47	11.70
11.72	292.07	3.00E+07	4608.8	5.91	10.83
12.50	292.85	3.47E+07	5693.3	6.30	10.16
13.29	293.64	3.97E+07	6892.9	6.67	9.60
14.08	294.43	4.50E+07	8150.4	6.97	9.19
14.86	295.21	5.06E+07	9525.0	7.23	8.86

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8042) 616.27 2.66 7.00 8.18 0.31 1.69
OUTFLOW : ID= 1 (6174) 616.27 2.45 8.00 8.18 0.29 1.69

```

```

ADD HYD (8048)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (6174): 616.27 2.452 8.00 8.18
+ ID2= 2 (8044): 139.10 0.614 8.00 11.76
=====
ID = 3 (8048): 755.37 3.066 8.00 8.84

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8052)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8048): 755.37 3.066 8.00 8.84
+ ID2= 2 (8050): 502.05 2.174 7.50 10.67
=====
ID = 3 (8052): 1257.42 5.226 8.00 9.57

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8058)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8052): 1257.42 5.226 8.00 9.57
+ ID2= 2 (8064): 418.70 3.032 6.00 13.02
=====
ID = 3 (8058): 1676.12 7.333 7.25 10.43

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB
STANDHYD (1761) Area (ha)= 162.00
ID= 1 DT=15.0 min Total Imp(%)= 26.00 Dir. Conn.(%)= 11.00

```

```

Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1039.23 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 55.44 8.28
over (min)= 15.00 60.00
Storage Coeff. (min)= 15.22 (ii) 45.20 (ii)
Unit Hyd. Tpeak (min)= 15.00 60.00
Unit Hyd. Tpeak (cms)= 0.07 0.02

**TOTALS*
PEAK FLOW (cms)= 1.93 1.31 2.311 (iii)
TIME TO PEAK (hrs)= 6.00 6.75 6.00
RUNOFF VOLUME (mm)= 40.00 7.76 11.30
TOTAL RAINFALL (mm)= 42.00 42.00
RUNOFF COEFFICIENT = 0.95 0.18 0.27

```

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

```

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 59.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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```

CALIB
STANDHYD (1761) Area (ha)= 48.80
ID= 1 DT=15.0 min Total Imp(%)= 50.00 Dir. Conn.(%)= 21.00

```

```

Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 570.38 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 55.44 14.34
over (min)= 15.00 45.00
Storage Coeff. (min)= 11.32 (ii) 34.58 (ii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. Tpeak (cms)= 0.08 0.03

**TOTALS*
PEAK FLOW (cms)= 1.28 0.56 1.508 (iii)
TIME TO PEAK (hrs)= 6.00 6.50 6.00
RUNOFF VOLUME (mm)= 40.00 10.02 16.31
TOTAL RAINFALL (mm)= 42.00 42.00 42.00
RUNOFF COEFFICIENT = 0.95 0.24 0.39

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

```

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 59.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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RESERVOIR (5176)
IN= 2--> OUT= 1
DT= 15.0 min

```

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	3.2640	1.3175
	0.1650	0.6418	4.0090	1.4908
	0.9450	0.7489	4.7720	1.6604
	1.7610	0.9426	5.1720	1.6704
	2.4420	1.1144	0.0000	0.0000

```

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (1761) 48.800 1.508 6.00 16.31
OUTFLOW : ID= 1 (5176) 48.800 0.139 8.75 16.29

```

```

PEAK FLOW REDUCTION [Qout/Qin](%)= 9.20
TIME SHIFT OF PEAK FLOW (min)=165.00
MAXIMUM STORAGE USED (ha.m.)= 0.5398

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```

ADD HYD (8056)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (1762): 162.00 2.311 6.00 11.30
+ ID2= 2 (5176): 48.80 0.139 8.75 16.29
=====
ID = 3 (8056): 210.80 2.360 6.00 12.46

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8066)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8056): 210.80 2.360 6.00 12.46
+ ID2= 2 (8058): 1676.12 7.333 7.25 10.43
=====
ID = 3 (8066): 1886.92 8.748 7.00 10.66

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8070)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8066): 1886.92 8.748 7.00 10.66
+ ID2= 2 (8068): 660.32 10.213 6.25 14.96
=====
ID = 3 (8070): 2547.24 17.105 6.25 11.77

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6190) |
 IN= 2----> OUT= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (1901.0) -----

Distance	Elevation	Manning
0.00	261.34	0.0600
19.76	261.64	0.0600
59.28	261.29	0.0600
79.04	261.09	0.0600
103.74	255.99	0.0600
153.13	253.42	0.0600
192.65	252.78	0.0600
212.29	249.80	0.0600
217.29	249.50	0.0600 /0.0350 Main Channel
217.30	249.05	0.0350 Main Channel
222.29	249.05	0.0350 Main Channel
222.30	249.50	0.0350 /0.0600 Main Channel
227.30	249.80	0.0600
232.17	252.31	0.0600
276.63	253.53	0.0600
326.03	256.97	0.0600
355.67	257.30	0.0600
400.12	260.08	0.0600
469.28	261.25	0.0600
489.04	262.53	0.0600

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.45	249.50	.436E+04	1.0	0.43	75.97
1.07	250.12	.205E+05	5.4	0.51	62.96
1.70	250.75	.451E+05	14.8	0.64	50.78
2.32	251.37	.762E+05	29.2	0.74	43.46
2.94	251.99	.114E+06	49.1	0.84	38.61
3.57	252.62	.161E+06	70.2	0.85	38.18
4.19	253.24	.248E+06	93.4	0.73	44.29
4.81	253.86	.398E+06	154.9	0.76	42.79
5.44	254.49	.576E+06	246.1	0.83	38.99
6.06	255.11	.779E+06	362.7	0.90	35.79
6.68	255.73	.101E+07	506.5	0.98	33.15
7.30	256.35	.126E+07	687.6	1.06	30.52
7.93	256.98	.153E+07	902.8	1.15	28.18
8.55	257.60	.183E+07	1114.3	1.18	27.39
9.17	258.22	.216E+07	1407.8	1.26	25.55
9.80	258.85	.250E+07	1735.4	1.35	24.03
10.42	259.47	.286E+07	2097.5	1.42	22.73
11.04	260.09	.324E+07	2492.3	1.49	21.63
11.67	260.72	.364E+07	2837.8	1.51	21.39

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8070) 2547.24	17.10	6.25	11.77	1.80	0.65
OUTFLOW : ID= 1 (6190) 2547.24	12.88	7.50	11.77	1.57	0.61

CALIB STANDHYD (0190) |
 ID= 1 DT=15.0 min | Area (ha)= 287.99 | Total Imp(%)= 51.00 | Dir. Conn.(%)= 27.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	146.87	141.12
Dep. Storage (mm)=	3.00	5.00
Average Slope (%)=	0.10	0.10
Length (m)=	1385.62	40.00
Mannings n =	0.013	0.250
Max.Eff.Inten.(mm/hr)=	37.80	14.57
over (min)=	30.00	75.00
Storage Coeff. (min)=	36.42 (ii)	73.89 (ii)
Unit Hyd. Tpeak (min)=	30.00	75.00
Unit Hyd. Tpeak (cms)=	0.03	0.02

PEAK FLOW (cms)= 4.86 | TIME TO PEAK (hrs)= 6.25 | RUNOFF VOLUME (mm)= 39.00 | TOTAL RAINFALL (mm)= 42.00 | RUNOFF COEFFICIENT = 0.93

TOTALS
 PEAK FLOW (cms)= 2.54 | TIME TO PEAK (hrs)= 7.00 | RUNOFF VOLUME (mm)= 13.79 | TOTAL RAINFALL (mm)= 42.00 | RUNOFF COEFFICIENT = 0.49

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 71.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8072) |
 1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)

ID1= 1 (0190): 287.99 5.953 6.25 20.60

+ ID2= 2 (6190): 2547.24 12.881 7.50 11.77
 ID = 3 (8072): 2835.23 17.173 7.00 12.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6192) |
 IN= 2----> OUT= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (1921.0) -----

Distance	Elevation	Manning
0.00	264.40	0.0500
3.64	263.94	0.0500
14.54	261.05	0.0500
29.08	256.78	0.0500
36.36	254.96	0.0500
50.90	252.20	0.0500
65.44	249.78	0.0500
76.35	246.86	0.0500
77.24	245.90	0.0500
82.24	245.45	0.0500 /0.0350 Main Channel
82.25	244.90	0.0350 Main Channel
87.25	244.90	0.0350 Main Channel
87.26	245.45	0.0350 /0.0500 Main Channel
94.53	246.89	0.0500
170.87	247.16	0.0500
218.13	248.69	0.0500
265.40	249.22	0.0500
301.75	250.78	0.0500
338.11	253.47	0.0500
359.92	264.00	0.0500

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.55	245.45	.345E+04	2.0	0.71	29.20
1.53	246.43	.174E+05	14.6	1.05	19.52
2.50	247.40	.767E+05	52.8	0.86	24.20
3.48	248.38	.224E+06	199.8	1.11	18.71
4.46	249.36	.429E+06	441.2	1.29	16.19
5.43	250.33	.691E+06	874.3	1.58	13.18
6.41	251.31	.987E+06	1468.3	1.86	11.20
7.38	252.28	.131E+07	2224.8	2.13	9.79
8.36	253.26	.165E+07	3132.4	2.37	8.78
9.34	254.24	.201E+07	4265.4	2.65	7.87
10.31	255.21	.238E+07	5551.0	2.92	7.15
11.29	256.19	.276E+07	6996.8	3.17	6.57
12.27	257.17	.314E+07	8585.9	3.42	6.10
13.24	258.14	.353E+07	10315.9	3.65	5.71
14.22	259.12	.393E+07	12180.7	3.87	5.38
15.19	260.09	.434E+07	14178.2	4.09	5.10
16.17	261.07	.475E+07	16306.5	4.29	4.85
17.15	262.05	.517E+07	18558.6	4.49	4.64
18.12	263.02	.559E+07	20939.7	4.68	4.45

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8072) 2835.23	17.17	7.00	12.67	1.59	1.03
OUTFLOW : ID= 1 (6192) 2835.23	16.46	7.50	12.67	1.57	1.04

CALIB STANDHYD (1921) |
 ID= 1 DT=15.0 min | Area (ha)= 30.70 | Total Imp(%)= 48.00 | Dir. Conn.(%)= 24.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	14.74	15.96
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	452.40	40.00
Mannings n =	0.013	0.250
Max.Eff.Inten.(mm/hr)=	55.44	10.39
over (min)=	15.00	45.00
Storage Coeff. (min)=	9.85 (ii)	36.31 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. Tpeak (cms)=	0.09	0.03

PEAK FLOW (cms)= 0.96 | TIME TO PEAK (hrs)= 6.00 | RUNOFF VOLUME (mm)= 40.00 | TOTAL RAINFALL (mm)= 42.00 | RUNOFF COEFFICIENT = 0.95

TOTALS
 PEAK FLOW (cms)= 0.26 | TIME TO PEAK (hrs)= 6.50 | RUNOFF VOLUME (mm)= 7.98 | TOTAL RAINFALL (mm)= 42.00 | RUNOFF COEFFICIENT = 0.37

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 54.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5192)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	2.0560	0.7051
	0.1040	0.3429	2.5820	0.7960
	0.5960	0.4035	3.2900	0.8880
	1.1090	0.5056	3.6900	0.8980
	1.5380	0.5974	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (1921)	30.700	1.066	6.00	15.66
OUTFLOW: ID= 1 (5192)	30.700	0.095	8.50	15.63

PEAK FLOW REDUCTION [Qout/Qin](%)= 8.95
 TIME SHIFT OF PEAK FLOW (min)=150.00
 MAXIMUM STORAGE USED (ha.m.)= 0.3149

CALIB STANDBYD (1922)			
ID= 1 DT=15.0 min			
Area	(ha)	= 215.90	
Total Imp(%)		= 27.00	Dir. Com.(%) = 14.00
IMPERVIOUS PERVIOUS (i)			
Surface Area (ha)	=	58.29	157.61
Dep. Storage (mm)	=	2.00	5.00
Average Slope (%)	=	0.50	0.50
Length (m)	=	1199.72	40.00
Mannings n	=	0.013	0.250
Max. Eff. Inten. (mm/hr)	=	55.44	5.16
over (min)	=	15.00	60.00
Storage Coeff. (min)	=	17.68 (ii)	52.00 (ii)
Unit Hyd. Tpeak (min)	=	15.00	60.00
Unit Hyd. peak (cms)	=	0.06	0.02
*****TOTALS*****			
PEAK FLOW (cms)	=	3.15	1.27
TIME TO PEAK (hrs)	=	6.00	6.75
RUNOFF VOLUME (mm)	=	40.00	6.44
TOTAL RAINFALL (mm)	=	42.00	42.00
RUNOFF COEFFICIENT	=	0.95	0.15

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 54.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8074)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1922):	215.90	3.512	6.00	11.14
+ ID2= 2 (5192):	30.70	0.095	8.50	15.63
*****TOTALS*****				
ID = 3 (8074):	246.60	3.552	6.00	11.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8076)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6192):	2835.23	16.455	7.50	12.67
+ ID2= 2 (8074):	246.60	3.552	6.00	11.70
*****TOTALS*****				
ID = 3 (8076):	3081.83	17.794	7.25	12.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0196)				
ID= 1 DT=15.0 min				
Area	(ha)	= 41.47	Curve Number (CN)= 70.0	
Ia	(mm)	= 5.00	# of Linear Res. (N)= 1.50	
U.H. Tp (hrs)		= 0.24		
Unit Hyd Qpeak (cms)	=	2.950		
PEAK FLOW (cms)	=	0.489 (i)		
TIME TO PEAK (hrs)	=	6.000		
RUNOFF VOLUME (mm)	=	8.615		

TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.205

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0194)				
ID= 1 DT=15.0 min				
Area	(ha)	= 256.09	Curve Number (CN)= 69.0	
Ia	(mm)	= 5.00	# of Linear Res. (N)= 1.50	
U.H. Tp (hrs)		= 0.68		
Unit Hyd Qpeak (cms)	=	6.458		
PEAK FLOW (cms)	=	1.406 (i)		
TIME TO PEAK (hrs)	=	6.750		
RUNOFF VOLUME (mm)	=	8.895		
TOTAL RAINFALL (mm)	=	42.000		
RUNOFF COEFFICIENT	=	0.212		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8078)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0194):	256.09	1.406	6.75	8.89
+ ID2= 2 (0196):	41.47	0.489	6.00	8.62
*****TOTALS*****				
ID = 3 (8078):	297.56	1.797	6.50	8.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6198)			
ID= 2 OUT= 1			
Routing time step (min)'= 15.00			

----- DATA FOR SECTION (1981.0) -----

Distance	Elevation	Manning	
0.00	267.15	0.0500	
22.99	265.94	0.0500	
51.73	261.39	0.0500	
74.72	258.75	0.0500	
94.83	257.79	0.0500	
114.95	254.36	0.0500	
126.44	254.06	0.0500	
132.19	253.68	0.0500	
135.06	253.35	0.0500	
137.94	252.93	0.0500 / 0.0300	Main Channel
140.81	252.41	0.0300	Main Channel
143.69	251.89	0.0300	Main Channel
146.56	252.51	0.0300	Main Channel
149.43	253.36	0.0300 / 0.0500	Main Channel
158.05	255.89	0.0500	
172.42	256.25	0.0500	
204.03	259.15	0.0500	
229.90	260.36	0.0500	
252.89	260.46	0.0500	
264.50	260.45	0.0500	

----- TRAVEL TIME TABLE -----

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.35	252.24	.111E+04	0.6	0.97	31.07
0.69	252.58	.442E+04	3.8	1.55	19.42
1.04	252.93	.974E+04	11.1	2.06	14.65
1.51	253.40	.203E+05	31.2	2.77	10.87
1.98	253.87	.357E+05	65.0	3.30	9.14
2.45	254.34	.596E+05	112.9	3.43	8.80
2.92	254.81	.935E+05	186.8	3.62	8.34
3.39	255.28	.131E+06	282.1	3.89	7.75
3.86	255.75	.172E+06	398.2	4.18	7.22
4.33	256.22	.221E+06	527.0	4.31	6.99
4.80	256.69	.282E+06	697.3	4.47	6.75
5.27	257.16	.350E+06	898.5	4.64	6.50
5.74	257.63	.425E+06	1131.4	4.82	6.26
6.21	258.10	.508E+06	1367.4	4.88	6.19
6.68	258.57	.603E+06	1635.6	4.91	6.14
7.15	259.04	.709E+06	1977.2	5.04	5.98
7.62	259.51	.826E+06	2360.5	5.17	5.83
8.09	259.98	.954E+06	2790.7	5.29	5.70
8.56	260.45	.110E+07	3179.6	5.25	5.74

hydrograph					<-pipe / channel->	
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8078)	297.56	1.80	6.50	8.86	0.48	1.13
OUTFLOW: ID= 1 (6198)	297.56	1.61	7.25	8.86	0.46	1.10

CALIB
STANDHYD (1981) Area (ha)= 104.70
ID= 1 DT=15.0 min Total Imp(%)= 44.00 Dir. Conn.(%)= 20.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	46.07	58.63
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	835.46	40.00
Manning's n	0.013	0.250

Max.Eff.Inten.(mm/hr)=	55.44	11.36	
over (min)	15.00	45.00	
Storage Coeff. (min)=	14.23 (ii)	39.77 (ii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.07	0.03	
TOTALS			
PEAK FLOW (cms)=	2.40	1.00	2.796 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	40.00	8.86	15.09
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT	0.95	0.21	0.36

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING:FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 58.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5198)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min	0.0000	0.0000	7.9640	2.3173
	0.2850	1.2270	9.5520	2.6039
	1.7410	1.4052	10.9290	2.8853
	3.5350	1.7675	11.3290	2.8953
	5.6580	2.0183	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (1981)	104.700	2.796	6.00	15.09
OUTFLOW: ID= 1 (5198)	104.700	0.253	9.25	15.08

PEAK FLOW REDUCTION [Qout/Qin](%)= 9.05
TIME SHIFT OF PEAK FLOW (min)=195.00
MAXIMUM STORAGE USED (ha.m.)= 1.0898

CALIB
NASHHYD (1982) Area (ha)= 285.90 Curve Number (CN)= 65.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.81

Unit Hyd Qpeak (cms)=	6.048
PEAK FLOW (cms)=	1.190 (i)
TIME TO PEAK (hrs)=	7.000
RUNOFF VOLUME (mm)=	7.768
TOTAL RAINFALL (mm)=	42.000
RUNOFF COEFFICIENT	0.185

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8080)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1982):	285.90	1.190	7.00	7.77
+ ID2= 2 (5198):	104.70	0.253	9.25	15.08
ID= 3 (8080):	390.60	1.405	7.25	9.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8082)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6198):	287.56	1.614	7.25	8.86
+ ID2= 2 (8080):	390.60	1.405	7.25	9.73
ID= 3 (8082):	688.16	3.019	7.25	9.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8084)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8076):	3081.83	17.794	7.25	12.59
+ ID2= 2 (8082):	688.16	3.019	7.25	9.35
ID= 3 (8084):	3769.99	20.814	7.25	12.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6200) Routing time step (min)= 15.00
IN= 2--> OUT= 1

DATA FOR SECTION (2001.0)			
Distance	Elevation	Manning	
0.00	252.47	0.0500	
10.38	252.29	0.0500	
20.76	252.28	0.0500	
41.52	247.66	0.0500	
62.28	247.47	0.0500	
83.05	247.34	0.0500	
90.83	246.85	0.0500	
103.81	245.51	0.0500	
106.00	244.50	0.0500	
108.99	244.05	0.0500 / 0.0300	Main Channel
109.00	243.50	0.0300	Main Channel
114.00	243.50	0.0300	Main Channel
114.01	244.05	0.0300 / 0.0500	Main Channel
118.99	244.50	0.0500	
132.36	245.09	0.0500	
150.52	245.83	0.0500	
199.83	249.24	0.0500	
207.62	250.16	0.0500	
236.16	253.11	0.0500	
256.92	253.83	0.0500	

TRAVEL TIME TABLE					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.28	243.77	365E+04	0.7	0.54	81.42
0.55	244.05	730E+04	2.2	0.81	54.59
1.05	244.55	197E+05	7.2	0.97	45.51
1.54	245.04	463E+05	17.2	0.99	44.76
2.04	245.54	895E+05	34.8	1.03	42.81
2.53	246.03	152E+06	62.8	1.09	40.43
3.03	246.53	232E+06	104.7	1.20	36.91
3.52	247.02	328E+06	159.7	1.29	34.19
4.02	247.52	450E+06	223.4	1.32	33.56
4.51	248.01	627E+06	326.9	1.38	31.99
5.01	248.51	820E+06	463.0	1.50	29.52
5.50	249.00	103E+07	626.0	1.62	27.30
6.00	249.50	124E+07	820.1	1.75	25.24
6.49	249.99	147E+07	1044.4	1.89	23.42
6.99	250.49	170E+07	1292.6	2.01	21.95
7.48	250.98	195E+07	1565.6	2.13	20.71
7.98	251.48	220E+07	1864.5	2.25	19.65
8.47	251.97	246E+07	2189.4	2.36	18.73
8.97	252.47	274E+07	2427.5	2.35	18.80

hydrograph						
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW: ID= 2 (8084)	3769.99	20.81	7.25	12.00	1.64	1.00
OUTFLOW: ID= 1 (6200)	3769.99	18.55	8.25	12.00	1.58	0.99

CALIB
STANDHYD (2001) Area (ha)= 6.00 Dir. Conn.(%)= 23.00
ID= 1 DT=15.0 min Total Imp(%)= 50.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	3.00	3.00
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	200.00	40.00
Manning's n	0.013	0.250

Max.Eff.Inten.(mm/hr)=	55.44	17.79	
over (min)	15.00	30.00	
Storage Coeff. (min)=	6.04 (ii)	27.38 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.10	0.04	
TOTALS			
PEAK FLOW (cms)=	0.20	0.10	0.265 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	40.00	12.52	18.84

TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.30 0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 67.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5200)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.4010	0.1620
0.0200	0.0789	0.5040	0.1833
0.1160	0.0921	0.5870	0.2041
0.2170	0.1159	0.9870	0.2141
0.3000	0.1370	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
6.000	0.265	6.00	18.84
6.000	0.020	8.50	18.67

INFLW : ID= 2 (2001) 6.000 0.265 6.00 18.84
 OUTFLOW: ID= 1 (5200) 6.000 0.020 8.50 18.67

PEAK FLOW REDUCTION [Qout/Qin](%)= 7.43
 TIME SHIFT OF PEAK FLOW (min)=150.00
 MAXIMUM STORAGE USED (ha.m.)= 0.0777

CALIB
 STANDHYD (2002)
 ID= 1 DT=15.0 min

Area (ha)= 109.30
 Total Imp(%)= 22.00 Dir. Conn.(%)= 10.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)= 24.05	85.25
Dep. Storage (mm)= 2.00	5.00
Average Slope (%)= 0.50	0.50
Length (m)= 853.62	40.00
Mannings n = 0.013	0.250

Max. Eff. Inten. (mm/hr)= 55.44	10.10
cover (min)= 18.00	45.00
Storage Coeff. (min)= 14.42 (ii)	41.19 (iii)
Unit Hyd. Tpeak (min)= 15.00	45.00
Unit Hyd. peak (cms)= 0.07	0.03

***** TOTALS*

PEAK FLOW (cms)= 1.25	1.26	1.738 (iii)
TIME TO PEAK (hrs)= 6.00	6.50	6.00
RUNOFF VOLUME (mm)= 40.00	9.71	12.74
TOTAL RAINFALL (mm)= 42.00	42.00	42.00
RUNOFF COEFFICIENT = 0.95	0.23	0.30

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 67.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8086)
 1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (2002): 109.30	1.738	6.00	12.74
+ ID2= 2 (5200): 6.00	0.020	8.50	18.67
ID = 3 (8086): 115.30	1.746	6.00	13.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8088)
 1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6200): 3769.99	18.550	8.25	12.00
+ ID2= 2 (8086): 115.30	1.746	6.00	13.05
ID = 3 (8088): 3885.29	19.003	8.25	12.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8038)
 1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8034): 5220.81	6.344	12.75	5.99
+ ID2= 2 (8088): 3885.29	19.003	8.25	12.03
ID = 3 (8038): 9106.10	23.596	8.25	8.57

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (1365)
 IN= 2--> OUT= 1

Routing time step (min)'= 15.00

DATA FOR SECTION (1362.0) ----->

Distance	Elevation	Manning
0.00	252.00	0.0400
2.35	244.60	0.0400
39.94	243.14	0.0400
58.74	242.65	0.0400
91.63	242.03	0.0400
101.03	241.69	0.0400
103.38	241.66	0.0400
105.73	240.76	0.0400
105.93	240.59	0.0400
109.18	240.28	0.0400 / 0.0300
110.43	239.81	0.0300
111.68	239.81	0.0300
113.43	240.09	0.0300 / 0.0400
118.43	240.50	0.0400
138.62	241.68	0.0400
152.72	241.60	0.0400
166.82	242.68	0.0400
185.62	249.09	0.0400
197.36	251.00	0.0400
232.61	252.28	0.0400

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.28	240.09	1.18E+04	0.1	0.220	142.09
0.91	240.72	1.14E+05	2.4	0.35	79.92
1.53	241.34	3.52E+05	9.4	0.45	62.17
2.16	241.97	8.31E+05	22.8	0.46	60.69
2.79	242.60	1.72E+06	54.2	0.53	52.74
3.41	243.22	2.97E+06	111.4	0.63	44.49
4.04	243.85	4.45E+06	194.1	0.74	38.22
4.67	244.48	6.12E+06	301.1	0.83	33.88
5.29	245.10	7.93E+06	443.0	0.94	29.82
5.92	245.73	9.97E+06	611.3	1.06	26.60
6.55	246.36	1.16E+07	802.0	1.17	24.12
7.18	246.99	1.35E+07	1014.0	1.27	22.16
7.80	247.61	1.54E+07	1246.2	1.37	20.56
8.43	248.24	1.73E+07	1498.0	1.46	19.24
9.06	248.87	1.92E+07	1768.8	1.55	18.12
9.68	249.49	2.12E+07	2049.6	1.63	17.24
10.31	250.12	2.32E+07	2344.7	1.71	16.49
10.94	250.75	2.52E+07	2658.5	1.78	15.83
11.56	251.37	2.74E+07	2931.2	1.81	15.56

hydrograph -----> <- pipe / channel ->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8038) 9106.10	23.60	8.25	8.57	2.18	0.47
OUTFLOW: ID= 1 (1365) 9106.10	20.22	9.25	8.57	2.04	0.46

CALIB
 NASHYD (1322)
 ID= 1 DT=15.0 min

Area (ha)= 249.10 Curve Number (CN)= 73.0
 Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.83

Unit Hyd Qpeak (cms)= 5.130
 PEAK FLOW (cms)= 1.365 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 10.314
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.246

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (1321)
 ID= 1 DT=15.0 min

Area (ha)= 41.70
 Total Imp(%)= 55.00 Dir. Conn.(%)= 35.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)= 22.94	18.77
Dep. Storage (mm)= 2.00	5.00

Average Slope (%) = 0.50 0.50
 Length (m) = 527.26 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 55.44 17.45
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 10.80 (ii) 32.30 (iii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.09 0.03

TOTALS
 PEAK FLOW (cms) = 1.85 0.54 2.074 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 40.00 13.04 22.48
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.31 0.54

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
 CN* = 70.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5132)				
ID= 2 -> OUTF- 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	0.9630	1.4050
	0.1780	1.0280	1.4870	1.5525
	0.4300	1.0400	1.9900	1.7000
	0.8150	1.0500	2.3900	1.7100
	0.8990	1.2275	0.0000	0.0000
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (1321)	41.700	2.074	6.00	22.48
OUTFLOW : ID= 1 (5132)	41.700	0.120	10.00	22.44
PEAK FLOW REDUCTION [Qout/Qin](%) = 5.80				
TIME SHIFT OF PEAK FLOW (min) = 240.00				
MAXIMUM STORAGE USED (ha.m.) = 0.6954				

ADD HYD (8090)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1322):	249.10	1.365	7.00	10.31
+ ID2= 2 (5132):	41.70	0.120	10.00	22.44

ID = 3 (8090):	290.80	1.468	7.00	12.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0134)
 Area (ha) = 155.95 Curve Number (CN) = 64.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp (hrs) = 0.64

Unit Hyd Qpeak (cms) = 4.167

PEAK FLOW (cms) = 0.747 (i)
 TIME TO PEAK (hrs) = 6.750 (min) = 240.00
 RUNOFF VOLUME (mm) = 7.460
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.178

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8092)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0134):	155.95	0.747	6.75	7.46
+ ID2= 2 (8090):	290.80	1.468	7.00	12.05

ID = 3 (8092):	446.75	2.208	7.00	10.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (1366)
 ID= 2 -> OUTF- 1
 Routing time step (min) = 15.00

----- DATA FOR SECTION (1361.0) -----
 Distance Elevation Manning

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.47	248.18	.881E+04	1.3	0.76	114.66
1.19	248.90	.742E+05	17.3	1.22	71.44
1.92	249.63	.278E+06	78.3	1.48	59.26
2.64	250.35	.589E+06	210.3	1.87	46.69
3.36	251.07	.974E+06	414.3	2.23	39.19
4.08	251.79	1.43E+07	696.2	2.55	34.32
4.81	252.52	1.97E+07	1063.0	2.83	30.86
5.53	253.24	2.59E+07	1505.7	3.06	28.62
6.25	253.96	3.31E+07	2051.3	3.26	26.85
6.97	254.68	4.13E+07	2720.9	3.46	25.30
7.70	255.41	5.04E+07	3584.5	3.73	23.43
8.42	256.13	6.01E+07	4580.9	4.00	21.87
9.14	256.85	7.04E+07	5707.9	4.25	20.57
9.86	257.57	8.18E+07	6636.1	4.26	20.53
10.59	258.29	9.45E+07	8115.0	4.50	19.42
11.31	259.02	1.08E+08	9759.2	4.74	18.45
12.03	259.74	1.22E+08	11569.5	4.96	17.61
12.75	260.46	1.37E+08	13550.6	5.18	16.87
13.48	261.19	1.53E+08	15636.4	5.37	16.29

----- TRAVEL TIME TABLE -----

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.47	248.18	.881E+04	1.3	0.76	114.66
1.19	248.90	.742E+05	17.3	1.22	71.44
1.92	249.63	.278E+06	78.3	1.48	59.26
2.64	250.35	.589E+06	210.3	1.87	46.69
3.36	251.07	.974E+06	414.3	2.23	39.19
4.08	251.79	1.43E+07	696.2	2.55	34.32
4.81	252.52	1.97E+07	1063.0	2.83	30.86
5.53	253.24	2.59E+07	1505.7	3.06	28.62
6.25	253.96	3.31E+07	2051.3	3.26	26.85
6.97	254.68	4.13E+07	2720.9	3.46	25.30
7.70	255.41	5.04E+07	3584.5	3.73	23.43
8.42	256.13	6.01E+07	4580.9	4.00	21.87
9.14	256.85	7.04E+07	5707.9	4.25	20.57
9.86	257.57	8.18E+07	6636.1	4.26	20.53
10.59	258.29	9.45E+07	8115.0	4.50	19.42
11.31	259.02	1.08E+08	9759.2	4.74	18.45
12.03	259.74	1.22E+08	11569.5	4.96	17.61
12.75	260.46	1.37E+08	13550.6	5.18	16.87
13.48	261.19	1.53E+08	15636.4	5.37	16.29

----- hydrograph ----- <-pipe / channel->

	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8092)	446.75	2.21	7.00	10.45	0.51	0.78
OUTFLOW : ID= 1 (1366)	446.75	1.55	9.00	10.45	0.48	0.77

CALIB (1362)
 Area (ha) = 410.10
 ID= 1 DT=15.0 min Total Imp(%) = 21.00 Dir. Conn.(%) = 11.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha) = 86.12 323.98
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 1653.48 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 37.00 8.94
 over (min) = 30.00 60.00
 Storage Coeff. (min) = 24.99 (ii) 53.09 (iii)
 Unit Hyd. Tpeak (min) = 30.00 60.00
 Unit Hyd. peak (cms) = 0.04 0.02

TOTALS
 PEAK FLOW (cms) = 3.38 3.48 5.209 (iii)
 TIME TO PEAK (hrs) = 6.25 6.75 6.25
 RUNOFF VOLUME (mm) = 40.00 8.88 12.30
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.21 0.29

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.
 (i) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
 CN* = 65.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (1361)
 Area (ha) = 102.30
 ID= 1 DT=15.0 min Total Imp(%) = 55.00 Dir. Conn.(%) = 27.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha) = 56.27 46.03
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50

Length (m)= 825.83 40.00
 Mannings n = 0.013 0.250
 Max.Eff.Inten.(mm/hr)= 55.44 18.37
 over (min) 15.00 45.00
 Storage Coeff. (min)= 14.13 (ii) 35.20 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.07 0.03
 TOTALS
 PEAK FLOW (cms)= 3.18 1.34 3.725 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 40.00 12.29 19.77
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.29 0.47

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5136)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.4000	3.2690
0.1600	1.6363	2.6450	3.8640
0.9600	1.7470	2.8900	4.4590
1.8400	1.9320	3.2900	4.4690
2.1200	2.6005	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
102.300	3.725	6.00	19.77
102.300	0.222	10.50	19.75

PEAK FLOW REDUCTION [Qout/qin](%)= 5.95
 TIME SHIFT OF PEAK FLOW (min)=270.00
 MAXIMUM STORAGE USED (ha.m.)= 1.6452

ADD HYD (8096)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1362):	410.10	5.209	6.25	12.30
+ ID2= 2 (5136):	102.30	0.222	10.50	19.75
ID = 3 (8096):	512.40	5.285	6.25	13.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8094)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1366):	446.75	1.548	9.00	10.45
+ ID2= 2 (8096):	512.40	5.285	6.25	13.79
ID = 3 (8094):	959.15	5.906	6.75	12.23

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8098)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1365):	9106.10	20.222	9.25	8.57
+ ID2= 2 (8094):	959.15	5.906	6.75	12.23
ID = 3 (8098):	10065.25	23.147	9.00	8.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0142)
 ID= 1 DT=15.0 min
 U.H. Tp(hrs)= 1.32

Unit Hyd Qpeak (cms)= 5.133

PEAK FLOW (cms)= 1.069 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 7.304

TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.174

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0138)
 NASHYD (0138)
 ID= 1 DT=15.0 min
 U.H. Tp(hrs)= 1.39

Area (ha)= 414.79
 Curve Number (CN)= 24.0
 Ia (mm)= 5.00
 # of Linear Res.(N)= 1.50

Unit Hyd Qpeak (cms)= 5.087

PEAK FLOW (cms)= 0.233 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 1.617
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.039

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0140)
 STANDHYD (0140)
 ID= 1 DT=15.0 min

Area (ha)= 158.58
 Total Imp(%)= 24.00
 Dir. Com.(%)= 21.00

Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
38.06	120.52	
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1028.20	40.00
Mannings n =	0.013	0.250

Max.Eff.Inten.(mm/hr)= 55.44 2.11
 over (min) 15.00 75.00
 Storage Coeff. (min)= 16.12 (ii) 66.21 (ii)
 Unit Hyd. Tpeak (min)= 15.00 75.00
 Unit Hyd. peak (cms)= 0.07 0.02

PEAK FLOW (cms)= 3.62 0.39 3.703 (iii)
 TIME TO PEAK (hrs)= 6.00 7.00 6.00
 RUNOFF VOLUME (mm)= 40.00 3.56 11.21
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.08 0.27

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 41.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8102)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0138):	414.79	0.233	8.25	1.62
+ ID2= 2 (0140):	158.58	3.703	6.00	11.21
ID = 3 (8102):	573.37	3.801	6.00	4.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6142)
 IN= 2--> OUT= 1
 Routing time step (min)= 15.00

DATA FOR SECTION (1421.0) ----->

Distance	Elevation	Manning
0.00	295.50	0.0400
7.25	295.36	0.0400
14.50	295.22	0.0400
21.75	295.22	0.0400
29.00	294.23	0.0400
52.56	286.45	0.0400
67.06	281.44	0.0400
77.94	278.28	0.0400
82.81	277.20	0.0400 / 0.0300
83.21	276.80	0.0300
88.81	276.80	0.0300
96.41	276.80	0.0300
96.81	277.20	0.0300 / 0.0400
103.31	278.96	0.0400
108.75	280.16	0.0400
117.81	282.73	0.0400
135.94	290.30	0.0400
159.50	292.14	0.0400
174.00	293.07	0.0400
179.44	293.37	0.0400

----- TRAVEL TIME TABLE -----

Table with columns: DEPTH (m), ELEV (m), VOLUME (cu.m.), FLOW RATE (cms), VELOCITY (m/s), TRAV TIME (min). Rows show data for various depths from 0.40 to 15.72 meters.

Summary table with columns: DEPTH (m), ELEV (m), VOLUME (cu.m.), FLOW RATE (cms), VELOCITY (m/s), TRAV TIME (min). Rows show values for 331.50, 380.98, 445.30, 455.19, and 489.83.

----- TRAVEL TIME TABLE -----

Table with columns: DEPTH (m), ELEV (m), VOLUME (cu.m.), FLOW RATE (cms), VELOCITY (m/s), TRAV TIME (min). Rows show data for various depths from 0.39 to 12.20 meters.

----- hydrograph ----- <-pipe / channel->

Summary table for hydrograph with columns: AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm), MAX DEPTH (m), MAX VEL (m/s). Rows show data for INFLOW and OUTFLOW.

----- hydrograph ----- <-pipe / channel->

Summary table for hydrograph with columns: AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm), MAX DEPTH (m), MAX VEL (m/s). Rows show data for INFLOW and OUTFLOW.

Table for ADD HYD (8104) with columns: ID, AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Rows show data for ID1, ID2, and ID3.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table for CALIB (0144) with columns: Area (ha), Curve Number (CN), U.H. Tp (hrs). Rows show data for NASHVD and routing parameters.

Table for Unit Hyd Qpeak (cms) = 3.240. Rows show PEAK FLOW, TIME TO PEAK, RUNOFF VOLUME, TOTAL RAINFALL, and RUNOFF COEFFICIENT.

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table for ADD HYD (8106) with columns: ID, AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm). Rows show data for ID1, ID2, and ID3.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table for ROUTE CHN (6146) with columns: ID, Routing time step (min). Rows show data for ID=2 and ID=1.

Table for DATA FOR SECTION (1461.0) with columns: Distance, Elevation, Manning. Rows show data for various distances from 0.00 to 286.97.

Table for STANDHYD (1462) with columns: Area (ha), Total Imp(%), Dir. Conn.(%). Rows show data for CALIB and DT=15.0 min.

Table for IMPERVIOUS and PERVIOUS (i) with columns: Surface Area, Dep. Storage, Average Slope, Length, Mannings n, Max. Eff. Inten., Storage Coeff., Unit Hyd. Tpeak, Unit Hyd. peak, PEAK FLOW, TIME TO PEAK, RUNOFF VOLUME, TOTAL RAINFALL, RUNOFF COEFFICIENT.

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 73.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table for STANDHYD (1461) with columns: Area (ha), Total Imp(%), Dir. Conn.(%). Rows show data for CALIB and DT=15.0 min.

Table for IMPERVIOUS and PERVIOUS (i) with columns: Surface Area, Dep. Storage, Average Slope, Length, Mannings n, Max. Eff. Inten., Storage Coeff., Unit Hyd. Tpeak, Unit Hyd. peak, PEAK FLOW, TIME TO PEAK, RUNOFF VOLUME, TOTAL RAINFALL, RUNOFF COEFFICIENT.

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

704.86 242.71 0.0400
 734.54 242.62 0.0400

- (i) CN PROCEDURE SELECTED FOR PervIOUS LOSSES:
 CN* = 73.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5146)					
IN= 2--> OUT= 1					
DT= 15.0 min					
	OUTFLOW	STORAGE	OUTFLOW	STORAGE	
	(cms)	(ha.m.)	(cms)	(ha.m.)	
	0.0000	0.0000	8.0640	2.2844	
	0.2390	0.7085	9.9830	2.6409	
	2.2880	0.8391	11.7350	3.0631	
	4.3270	1.4661	12.1350	3.0731	
	6.1170	1.8197	0.0000	0.0000	

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (1461)	70.500	1.756	6.00	17.40
OUTFLOW: ID= 1 (5146)	70.500	0.485	7.75	17.39

PEAK FLOW REDUCTION [Qout/Qin](%)= 27.64
TIME SHIFT OF PEAK FLOW (min)=105.00
MAXIMUM STORAGE USED (ha.m.)= 0.7254

----- TRAVEL TIME TABLE ----->

DEPTH	RELV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.50	231.90	.221E+05	4.7	0.85	78.67
1.00	232.40	.462E+05	15.3	1.28	52.40
1.50	233.00	.913E+05	38.3	1.68	39.77
2.20	233.60	.154E+06	72.9	1.90	35.24
2.80	234.20	.278E+06	124.2	1.79	37.30
3.40	234.80	.599E+06	214.3	1.44	46.59
4.01	235.41	.125E+07	445.0	1.43	46.90
4.61	236.01	.205E+07	838.4	1.64	40.81
5.21	236.61	.293E+07	1361.6	1.86	35.89
5.81	237.21	.389E+07	2011.2	2.08	32.23
6.41	237.81	.494E+07	2753.5	2.24	29.91
7.01	238.41	.612E+07	3635.0	2.39	28.04
7.61	239.01	.741E+07	4672.5	2.53	26.43
8.21	239.61	.880E+07	5999.3	2.74	24.44
8.82	240.22	.102E+08	7510.0	2.94	22.72
9.42	240.82	.117E+08	9201.9	3.15	21.23
10.02	241.42	.132E+08	11059.7	3.35	19.95
10.62	242.02	.148E+08	13073.4	3.54	18.87
11.22	242.62	.164E+08	15242.1	3.73	17.93

<---- hydrograph ----> <-pipe / channel-->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW: ID= 2 (8100) *****	26.53	8.75	8.78	1.29	1.45
OUTFLOW: ID= 1 (6202) *****	25.63	9.50	8.78	1.27	1.43

ADD HYD (8099)					
1 + 2 = 3					
ID#	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID# 1 (1462):	125.00	2.466	6.00	15.95	
+ ID# 2 (5146):	70.50	0.485	7.75	17.39	
=====					
ID = 3 (8099):	195.50	2.616	6.50	16.47	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8112)					
1 + 2 = 3					
ID#	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID# 1 (6202):	11366.35	25.630	9.50	8.78	
+ ID# 2 (8110):	468.50	13.235	6.25	22.23	
=====					
ID = 3 (8112):	11834.85	27.245	9.25	9.31	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8108)					
1 + 2 = 3					
ID#	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID# 1 (6146):	1105.60	2.763	8.25	6.14	
+ ID# 2 (8099):	195.50	2.616	6.50	16.47	
=====					
ID = 3 (8108):	1301.10	4.120	6.50	7.69	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5555)					
IN= 2--> OUT= 1					
DT= 15.0 min					
	OUTFLOW	STORAGE	OUTFLOW	STORAGE	
	(cms)	(ha.m.)	(cms)	(ha.m.)	
	0.0000	0.0000	*****	19.2100	
	2.8500	0.5200	*****	24.7300	
	8.1800	1.2600	*****	30.8600	
	15.0900	2.2100	*****	37.6100	
	23.2700	3.3900	*****	41.2200	
	32.5300	4.7800	*****	48.9000	
	36.5100	5.3900	*****	66.4700	
	42.7600	6.3700	*****	100.1200	
	53.8500	8.1300	*****	140.4100	
	91.7300	14.3200	*****	140.4200	

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (8112) *****	27.245	9.25	9.31	
OUTFLOW: ID= 1 (5555) *****	27.040	10.00	9.31	

PEAK FLOW REDUCTION [Qout/Qin](%)= 99.25
TIME SHIFT OF PEAK FLOW (min)= 45.00
MAXIMUM STORAGE USED (ha.m.)= 3.9590

ADD HYD (8100)					
1 + 2 = 3					
ID#	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID# 1 (8098):	10065.25	23.147	9.00	8.92	
+ ID# 2 (8108):	1301.10	4.120	6.50	7.69	
=====					
ID = 3 (8100):	11366.35	26.528	8.75	8.78	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8114)					
1 + 2 = 3					
ID#	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID# 1 (5555):	11834.85	27.040	10.00	9.31	
+ ID# 2 (8136):	2288.40	7.588	6.00	9.22	
=====					
ID = 3 (8114):	14123.25	32.133	9.50	9.30	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6202)					
IN= 2--> OUT= 1					
Routing time step (min)'= 15.00					
----- DATA FOR SECTION (2021.0) ----->					
Distance	Elevation	Manning			
0.00	245.48	0.0400			
29.68	242.80	0.0400			
74.20	240.39	0.0400			
155.81	237.12	0.0400			
200.33	234.48	0.0400			
244.85	234.67	0.0400			
259.69	233.96	0.0400			
274.53	233.84	0.0400			
281.68	233.40	0.0400			
285.78	232.40	0.0400 / 0.0300	Main Channel		
286.78	231.40	0.0300	Main Channel		
296.78	231.40	0.0300	Main Channel		
299.78	232.40	0.0300 / 0.0400	Main Channel		
311.62	233.74	0.0400			
333.88	234.34	0.0400			
497.11	235.32	0.0400			
571.31	237.32	0.0400			
675.18	239.10	0.0400			

CALIB					
STANDHYD (2061)					
ID#	Area	(ha)	Total Imp(%)	Dir. Conn.(%)	
ID= 1 DT=15.0 min	25.00	93.00		60.00	

Surface Area	(ha)	IMPERVIOUS	PERVIOUS (i)
Dep. Storage	(mm)		
Average Slope	(%)		
	23.25	1.75	
	2.00	5.00	
	0.50	0.50	

Length (m) = 408.25 40.00
 Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr) = 55.44 262.37
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 9.26 (ii) 16.53 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.09 0.05

*****TOTALS*
 PEAK FLOW (cms) = 1.99 0.71 2.490 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 40.00 30.23 36.09
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.72 0.86

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5206)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	1.8990	1.7295
	0.0850	0.5280	2.1560	1.9661
	0.8990	0.4577	2.3740	2.2023
	1.3460	1.1601	2.7740	2.2123
	1.6060	1.4451	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (2061)	25.000	2.490	6.00	36.09
OUTFLOW : ID= 1 (5206)	25.000	0.303	7.00	36.03

PEAK FLOW REDUCTION [Qout/qin](%) = 12.16
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 0.6163

CALIB			
STANDHYD (2062)			
ID= 1 DT=15.0 min			
	Area	(ha) =	250.50
	Total Imp(%) =		62.00
	Dir. Conn.(%) =		39.00

Surface Area (ha) = 155.31 95.19
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 1292.28 40.00
 Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr) = 55.44 25.16
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 18.49 (ii) 37.07 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.06 0.03

*****TOTALS*
 PEAK FLOW (cms) = 9.96 3.67 11.500 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 40.00 16.50 25.67
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.39 0.61

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8144)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (2062):	250.50	11.500	6.00	25.67
+ ID2= 2 (5206):	25.00	0.303	7.00	36.03

ID = 3 (8144):	275.50	11.553	6.00	26.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB			
STANDHYD (2041)			
ID= 1 DT=15.0 min			
	Area	(ha) =	201.50
	Total Imp(%) =		36.00
	Dir. Conn.(%) =		20.00

Surface Area (ha) = 72.54 128.96
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 1159.02 40.00
 Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr) = 55.44 9.61
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 17.32 (ii) 44.62 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.07 0.03

*****TOTALS*
 PEAK FLOW (cms) = 4.24 1.74 4.916 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 40.00 8.60 14.88
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.20 0.35

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5204)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	12.4660	4.9468
	0.5050	1.9141	15.4770	5.3726
	2.8390	2.2056	18.2610	6.4806
	5.2410	3.6606	18.6610	6.4906
	7.9920	4.3236	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (2041)	201.500	4.216	6.00	14.88
OUTFLOW : ID= 1 (5204)	201.500	0.755	8.25	14.87

PEAK FLOW REDUCTION [Qout/qin](%) = 15.35
 TIME SHIFT OF PEAK FLOW (min) = 135.00
 MAXIMUM STORAGE USED (ha.m.) = 1.9467

CALIB			
STANDHYD (2042)			
ID= 1 DT=15.0 min			
	Area	(ha) =	122.90
	Total Imp(%) =		60.00
	Dir. Conn.(%) =		33.00

Surface Area (ha) = 73.74 49.16
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 905.17 40.00
 Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr) = 55.44 17.14
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 14.93 (ii) 36.60 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.07 0.03

*****TOTALS*
 PEAK FLOW (cms) = 4.56 1.31 5.092 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 40.00 11.20 20.70
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.27 0.49

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8140)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (2042):	122.90	5.092	6.00	20.70
+ ID2= 2 (5204):	201.50	0.755	8.25	14.87

ID = 3 (8140):	324.40	5.263	6.00	17.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
ROUTE CHN (6206) |
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
-----
<----- DATA FOR SECTION (2061.0) ----->
Distance Elevation Manning
0.00 253.07 0.0350
13.98 252.89 0.0350
27.95 251.99 0.0350
34.94 252.18 0.0350
41.93 252.02 0.0350
73.38 250.35 0.0350
94.34 247.09 0.0350
111.81 244.79 0.0350
115.31 244.24 0.0350
117.55 244.20 0.0350 /0.0300 Main Channel
118.80 243.60 0.0300 Main Channel
122.30 243.60 0.0300 Main Channel
122.55 244.60 0.0300 /0.0350 Main Channel
146.76 244.74 0.0350
164.23 247.26 0.0350
213.15 248.92 0.0350
241.10 249.51 0.0350
272.55 250.87 0.0350
300.50 252.26 0.0350
345.93 253.67 0.0350

```

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-----
<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.30 243.90 .310E+04 1.4 1.23 36.25
0.60 244.20 .676E+04 4.6 1.81 24.64
1.12 244.72 .221E+05 17.1 2.07 21.60
1.64 245.24 .754E+05 64.1 2.28 19.60
2.17 245.77 .139E+06 149.8 2.88 15.50
2.69 246.29 .214E+06 273.3 3.43 13.04
3.21 246.81 .299E+06 436.1 3.91 11.43
3.73 247.33 .395E+06 632.6 4.30 10.40
4.25 247.85 .511E+06 836.4 4.39 10.18
4.77 248.37 .653E+06 1114.9 4.58 9.76
5.30 248.90 .822E+06 1472.1 4.81 9.30
5.82 249.42 .102E+07 1872.2 4.91 9.10
6.34 249.94 .126E+07 2444.6 5.22 8.56
6.86 250.46 .151E+07 3113.1 5.52 8.09
7.38 250.98 .180E+07 3843.5 5.74 7.79
7.90 251.50 .211E+07 4717.8 6.00 7.45
8.43 252.03 .245E+07 5701.4 6.24 7.16
8.95 252.55 .284E+07 6727.5 6.36 7.03
9.47 253.07 .326E+07 7885.3 6.48 6.90

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-----
<----- hydrograph -----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8140) 324.40 5.26 6.00 17.08 0.63 1.83
OUTFLOW : ID= 1 (6206) 324.40 3.07 6.50 17.08 0.46 1.48

```

```

-----
ADD HYD (8142) |
1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
+ ID1= 1 (6206): 324.40 3.066 6.50 17.08
+ ID2= 2 (8144): 275.50 11.553 6.00 26.61
=====
ID = 3 (8142): 599.90 13.439 6.00 21.46

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ADD HYD (8138) |
1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
+ ID1= 1 (8114): 14123.25 32.133 9.50 9.30
+ ID2= 2 (8142): 599.90 13.439 6.00 21.46
=====
ID = 3 (8138): 14723.15 34.292 9.25 9.79

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ROUTE CHN (6208) |
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
-----

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```

<----- DATA FOR SECTION (2081.0) ----->
Distance Elevation Manning
0.00 249.19 0.1000
73.11 244.13 0.1000
121.86 243.39 0.1000
243.71 240.01 0.1000

```

```

353.38 238.75 0.1000
450.87 235.40 0.1000
499.61 231.40 0.1000
511.80 230.96 0.1000
523.98 230.12 0.1000
534.50 227.80 0.1000 /0.0500 Main Channel
536.50 226.70 0.0500 Main Channel
546.50 226.70 0.0500 Main Channel
548.50 227.80 0.0500 /0.1000 Main Channel
572.72 230.25 0.1000
682.40 230.50 0.1000
804.25 233.95 0.1000
926.11 235.83 0.1000
1047.96 240.70 0.1000
1145.45 243.26 0.1000
1206.38 247.03 0.1000

```

```

-----
<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.55 227.25 .111E+05 2.0 0.33 92.44
1.10 227.80 .242E+05 6.5 0.50 61.56
2.23 228.93 .700E+05 26.0 0.68 44.91
3.36 230.06 .150E+06 60.8 0.74 41.04
4.49 231.19 .446E+06 130.8 0.54 56.83
5.62 232.32 .925E+06 287.4 0.57 53.66
6.76 233.46 .152E+07 522.9 0.63 48.32
7.89 234.59 .223E+07 827.2 0.68 44.91
9.02 235.72 .312E+07 1219.5 0.72 42.61
10.15 236.85 .418E+07 1780.6 0.78 39.11
11.28 237.98 .537E+07 2479.2 0.85 36.07
12.41 239.11 .669E+07 3283.9 0.90 33.95
13.54 240.24 .824E+07 4228.6 0.94 32.47
14.67 241.37 .997E+07 5384.6 0.99 30.87
15.81 242.51 .119E+08 6698.4 1.03 29.56
16.94 243.64 .140E+08 8235.2 1.08 28.25
18.07 244.77 .162E+08 10104.2 1.14 26.73
19.20 245.90 .185E+08 12251.5 1.21 25.21
20.33 247.03 .209E+08 14597.0 1.28 23.89

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-----
<----- hydrograph -----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8138) ***** 34.29 9.25 9.79 2.50 0.69
OUTFLOW : ID= 1 (6208) ***** 33.35 10.00 9.79 2.47 0.69

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-----
CALIB |
STANDHYD (2082) | Area (ha)= 426.60
| ID= 1 DT=15.0 min | Total Imp(%)= 64.00 Dir. Conn.(%)= 37.00
-----

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-----
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 273.02 153.58
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1686.42 40.00
Mannings n = 0.013 0.250

```

```

Max.Eff.Inten.(mm/hr)= 37.80 24.86
over (min) 30.00 45.00
Storage Coeff. (min)= 25.28 (ii) 43.95 (ii)
Unit Hyd. Tpeak (min)= 30.00 45.00
Unit Hyd. peak (cms)= 0.04 0.03
=====
*TOTALS*
PEAK FLOW (cms)= 11.76 5.34 15.690 (iii)
TIME TO PEAK (hrs)= 6.25 6.50 6.25
RUNOFF VOLUME (mm)= 40.00 15.12 24.32
TOTAL RAINFALL (mm)= 42.00 42.00 42.00
RUNOFF COEFFICIENT = 0.95 0.36 0.58

```

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB |
STANDHYD (2081) | Area (ha)= 265.70
| ID= 1 DT=15.0 min | Total Imp(%)= 35.00 Dir. Conn.(%)= 20.00
-----

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-----
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 93.00 172.71
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1330.91 40.00
Mannings n = 0.013 0.250

```

```

Max.Eff.Inten.(mm/hr)= 55.44 12.81
over (min) 15.00 45.00
Storage Coeff. (min)= 18.82 (ii) 43.15 (ii)

```

Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.06 0.03

TOTALS
 PEAK FLOW (cms)= 5.37 3.15 6.618 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 40.00 11.39 17.11
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.27 0.41

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5208)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	16.8530	6.3371
0.8360	2.7353	21.1630	7.3975
4.7810	3.1070	25.1140	8.4406
8.9050	4.2484	25.5140	8.4506
12.6070	5.1796	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (2081)	265.700	6.618	6.00
OUTFLOW: ID= 1 (5208)	265.700	1.452	8.00

PEAK FLOW REDUCTION [Qout/Qin](%)= 21.94
 TIME SHIFT OF PEAK FLOW (min)=120.00
 MAXIMUM STORAGE USED (ha.m.)= 2.7954

ADD HYD (8148)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (2082):	426.60	15.690	6.25	24.32
+ ID2= 2 (5208):	265.70	1.452	8.00	17.11
ID = 3 (8148):	692.30	16.107	6.25	21.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8146)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6208):	14723.15	33.345	10.00	9.79
+ ID2= 2 (8148):	692.30	16.107	6.25	21.55
ID = 3 (8146):	15415.45	35.611	9.75	10.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6210)
 IN= 2--> OUT= 1
 Routing time step (min)'= 15.00

----- DATA FOR SECTION (2101.0) -----

Distance	Elevation	Manning
0.00	241.81	0.0800
28.43	241.78	0.0800
85.30	237.04	0.0800
149.27	231.32	0.0800
170.60	230.10	0.0800
255.89	229.37	0.0800
277.22	228.68	0.0800
284.33	228.42	0.0800
291.43	228.16	0.0800
297.76	227.00	0.0800 / 0.0350 Main Channel
298.76	225.00	0.0350 Main Channel
312.76	225.00	0.0350 Main Channel
313.76	227.00	0.0350 / 0.0800 Main Channel
326.97	229.50	0.0800
348.30	230.29	0.0800
376.73	229.76	0.0800
490.46	235.75	0.0800
568.65	236.24	0.0800
661.05	241.85	0.0800
703.71	246.04	0.0800

----- TRAVEL TIME TABLE -----
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME

(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.67	226.67	141E+05	6.6	0.69	35.86
1.33	226.33	289E+05	20.4	1.04	23.65
2.00	227.00	443E+05	39.3	1.31	18.81
2.93	227.93	730E+05	77.8	1.57	15.64
3.85	228.85	123E+06	131.1	1.57	15.65
4.78	229.78	228E+06	205.6	1.33	18.51
5.70	230.70	503E+06	356.7	1.05	23.49
6.63	231.63	846E+06	606.7	1.06	23.17
7.55	232.55	123E+07	946.4	1.14	21.65
8.48	233.48	165E+07	1367.0	1.22	20.13
9.41	234.41	211E+07	1871.8	1.31	18.80
10.33	235.33	261E+07	2463.2	1.40	17.65
11.26	236.26	317E+07	2999.4	1.40	17.63
12.18	237.18	384E+07	3848.3	1.48	16.63
13.11	238.11	454E+07	4806.0	1.56	15.75
14.03	239.03	528E+07	5876.9	1.64	14.97
14.96	239.96	605E+07	7062.7	1.72	14.29
15.88	240.88	687E+07	8365.0	1.80	13.68
16.81	241.81	771E+07	9506.9	1.82	13.52

----- hydrograph ----- <-pipe / channel-->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8146) ***** 35.61 9.75 10.32 1.87 1.25
 OUTFLOW: ID= 1 (6210) ***** 35.57 10.00 10.32 1.86 1.24

CALIB (0210)
 NASHVD (0210) Area (ha)= 218.27 Curve Number (CN)= 80.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.08

Unit Hyd Qpeak (cms)= 3.454
 PEAK FLOW (cms)= 1.289 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 13.498
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.321

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8150)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0210):	218.27	1.289	7.50	13.50
+ ID2= 2 (6210):	15415.45	35.568	10.00	10.32
ID = 3 (8150):	15633.72	36.539	10.00	10.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0218)
 NASHVD (0218) Area (ha)= 152.25 Curve Number (CN)= 69.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.79

Unit Hyd Qpeak (cms)= 3.278
 PEAK FLOW (cms)= 0.743 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 8.929
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.213

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0216)
 NASHVD (0216) Area (ha)= 145.77 Curve Number (CN)= 75.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.52

Unit Hyd Qpeak (cms)= 4.768
 PEAK FLOW (cms)= 1.235 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 10.952
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.261

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

STANDHYD (2121) | Area (ha)= 56.00
 ID= 1 DT=15.0 min | Total Imp(%)= 93.00 Dir. Conn.(%)= 75.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	52.08	3.92	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	611.01	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	55.44	124.85	
over (min)	15.00	30.00	
Storage Coeff. (min)=	11.80 (ii)	11.59 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.08	0.05	
			**TOTALS*
PEAK FLOW (cms)=	5.17	0.67	5.615 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	40.00	22.72	35.68
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.54	0.85

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5212)

IN= 2--> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.7460	1.9720	
0.1900	1.1850	4.7040	2.0838	
1.0850	1.4413	5.4760	2.1940	
2.0210	1.5949	5.8760	2.2040	
2.8030	1.7848	0.0000	0.0000	

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (2121)	56.000	5.615	6.00	35.68
OUTFLOW: ID= 1 (5212)	56.000	0.752	7.00	35.65

PEAK FLOW REDUCTION [Qout/Qin](%)=	13.39
TIME SHIFT OF PEAK FLOW (min)=	60.00
MAXIMUM STORAGE USED (ha.m.)=	1.3500

CALIB (2122) | Area (ha)= 272.40
 ID= 1 DT=15.0 min | Total Imp(%)= 29.00 Dir. Conn.(%)= 23.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	79.00	193.40	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	1347.59	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	55.44	9.57	
over (min)	15.00	60.00	
Storage Coeff. (min)=	18.96 (ii)	46.30 (ii)	
Unit Hyd. Tpeak (min)=	15.00	60.00	
Unit Hyd. peak (cms)=	0.06	0.02	
			**TOTALS*
PEAK FLOW (cms)=	6.31	2.40	7.003 (iii)
TIME TO PEAK (hrs)=	6.00	6.75	6.00
RUNOFF VOLUME (mm)=	40.00	9.80	16.75
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.23	0.40

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8152)

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (2122):	272.40	7.003	6.00	16.75
+ ID2= 2 (5212):	56.00	0.752	7.00	35.65
=====				
ID = 3 (8152):	328.40	7.123	6.00	19.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6214) |
 IN= 2--> OUT= 1 | Routing time step (min)= 15.00

	Distance	Elevation	Manning	
0.00	259.29	259.29	0.0500	
15.33	258.91	258.91	0.0500	
38.33	258.57	258.57	0.0500	
65.16	257.93	257.93	0.0500	
95.82	253.44	253.44	0.0500	
111.15	251.21	251.21	0.0500	
130.32	249.67	249.67	0.0500	
149.48	248.51	248.51	0.0500	
150.66	248.50	248.50	0.0500	0.0370 Main Channel
150.81	247.50	247.50	0.0370	Main Channel
154.66	247.50	247.50	0.0370	Main Channel
155.66	248.50	248.50	0.0370	0.0500 Main Channel
160.98	248.50	248.50	0.0500	
187.81	249.78	249.78	0.0500	
199.31	252.13	252.13	0.0500	
245.30	254.04	254.04	0.0500	
264.46	253.99	253.99	0.0500	
298.96	255.23	255.23	0.0500	
329.62	257.02	257.02	0.0500	
379.45	258.82	258.82	0.0500	

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.50	248.00	6.48E+04	2.1	1.00	52.30
1.00	248.50	1.39E+05	6.4	1.44	36.35
1.61	249.11	5.70E+05	22.6	1.24	42.01
2.21	249.71	1.43E+06	63.6	1.39	37.61
2.82	250.32	2.63E+06	142.9	1.70	30.69
3.43	250.93	4.03E+06	257.2	2.00	26.11
4.04	251.54	5.62E+06	410.4	2.29	22.82
4.64	252.14	7.35E+06	601.3	2.56	20.37
5.25	252.75	9.93E+06	791.0	2.66	19.67
5.86	253.36	1.17E+07	1038.0	2.79	18.74
6.46	253.96	1.44E+07	1344.4	2.93	17.82
7.07	254.57	1.78E+07	1664.1	2.93	17.83
7.68	255.18	2.17E+07	2140.2	3.10	16.87
8.28	255.78	2.59E+07	2722.9	3.30	15.83
8.89	256.39	3.04E+07	3387.1	3.50	14.94
9.50	257.00	3.51E+07	4132.7	3.69	14.16
10.11	257.61	4.02E+07	4913.0	3.83	13.64
10.71	258.21	4.58E+07	5648.3	3.87	13.50
11.32	258.82	5.21E+07	6343.1	3.82	13.68

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8152)	328.40	7.12	6.00	19.97	1.03	1.43
OUTFLOW: ID= 1 (6214)	328.40	4.04	6.75	19.97	0.73	1.16

CALIB (0214) | Area (ha)= 316.95
 ID= 1 DT=15.0 min | Total Imp(%)= 25.00 Dir. Conn.(%)= 9.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	79.24	237.71	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	1453.62	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	37.80	9.36	
over (min)	30.00	60.00	
Storage Coeff. (min)=	23.13 (ii)	50.71 (ii)	
Unit Hyd. Tpeak (min)=	30.00	60.00	
Unit Hyd. peak (cms)=	0.04	0.02	
			**TOTALS*
PEAK FLOW (cms)=	2.21	2.75	3.724 (iii)
TIME TO PEAK (hrs)=	6.25	6.75	6.75
RUNOFF VOLUME (mm)=	40.00	8.63	11.46
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.21	0.27

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 62.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8154)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0214):	316.95	3.724	6.75	11.46	
+ ID2= 2 (6214):	328.40	4.038	6.75	10.97	
=====					
ID = 3 (8154):	645.35	7.762	6.75	15.79	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8156)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0216):	145.77	1.235	6.50	10.95	
+ ID2= 2 (8154):	645.35	7.762	6.75	15.79	
=====					
ID = 3 (8156):	791.12	8.976	6.75	14.90	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6218) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2181.0) ----->

Distance	Elevation	Manning	
0.00	256.38	0.0450	
7.70	254.85	0.0450	
15.40	253.20	0.0450	
19.25	252.29	0.0450	
23.10	251.17	0.0450	
78.94	236.16	0.0450	
80.87	235.80	0.0450	
82.79	235.45	0.0450	
84.57	234.60	0.0450 /0.0300	Main Channel
85.07	233.60	0.0300	Main Channel
88.57	233.60	0.0300	Main Channel
93.57	233.60	0.0300	Main Channel
94.07	234.60	0.0300 /0.0450	Main Channel
96.27	235.13	0.0450	
98.19	235.20	0.0450	
100.12	235.32	0.0450	
138.63	239.95	0.0450	
180.98	244.80	0.0450	
188.69	245.00	0.0450	
190.61	256.00	0.0450	

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
1.00	234.60	.181E+05	6.0	0.66	50.64
2.13	235.73	.518E+05	23.8	0.93	36.27
3.25	236.85	.119E+06	60.0	1.02	33.03
4.38	237.98	.217E+06	120.6	1.12	29.99
5.51	239.11	.346E+06	210.5	1.23	27.39
6.63	240.23	.506E+06	334.1	1.33	25.22
7.76	241.36	.697E+06	495.6	1.43	23.44
8.88	242.48	.920E+06	699.8	1.53	21.91
10.01	243.61	.117E+07	950.8	1.63	20.59
11.14	244.74	.146E+07	1252.5	1.73	19.45
12.26	245.86	.178E+07	1619.6	1.83	18.37
13.39	246.99	.212E+07	2078.5	1.97	17.01
14.52	248.12	.247E+07	2589.0	2.11	15.88
15.64	249.24	.282E+07	3149.7	2.25	14.94
16.77	250.37	.319E+07	3759.8	2.37	14.14
17.89	251.49	.356E+07	4419.5	2.50	13.44
19.02	252.62	.395E+07	5126.4	2.61	12.84
20.15	253.75	.435E+07	5871.6	2.72	12.34
21.27	254.87	.476E+07	6663.2	2.82	11.89

<----- hydrograph -----> <-pipe / channel->

INFLOW : ID= 2 (8156)	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
791.12	8.98	6.75	14.90	1.19	0.70	
OUTFLOW : ID= 1 (6218)	791.12	6.54	7.25	14.90	1.04	0.67

ADD HYD (8158)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0218):	152.25	0.743	7.00	8.93	
+ ID2= 2 (6218):	791.12	6.536	7.25	14.90	
=====					
ID = 3 (8158):	943.37	7.273	7.25	13.93	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8151)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (8150):	15633.72	36.539	10.00	10.36	
+ ID2= 2 (8158):	943.37	7.273	7.25	13.93	
=====					
ID = 3 (8151):	16577.09	41.190	8.50	10.57	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5504)					
IN= 2--> OUT= 1	DT= 15.0 min	OUTFLOW	STORAGE	OUTFLOW	STORAGE
		(cms)	(ha.m.)	(cms)	(ha.m.)
		0.0000	0.0000	33.0100	0.6700
		2.6400	0.0500	37.4800	0.7800
		4.8300	0.0900	47.5400	1.0100
		7.4100	0.1300	60.2300	1.2800
		10.3300	0.1900	71.9300	1.5258
		13.5300	0.2500	*****	3.0529
		16.9900	0.3200	*****	5.3299
		20.6800	0.3900	*****	8.4148
		24.6000	0.4800	*****	11.7637
		28.7100	0.5700	*****	11.7737

INFLOW : ID= 2 (8151)	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
*****	41.190	8.50	10.57		
OUTFLOW : ID= 1 (5504)	*****	41.193	8.50	10.57	

PEAK FLOW REDUCTION [Qout/Qin](%)=100.01
TIME SHIFT OF PEAK FLOW (min)= 0.00
MAXIMUM STORAGE USED (ha.m.)= 0.8650

**** WARNING : HYDROGRAPH PEAK WAS NOT REDUCED.
CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.

ROUTE CHN (6220) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2201.0) ----->

Distance	Elevation	Manning	
0.00	245.59	0.0600	
5.76	245.45	0.0600	
23.03	242.27	0.0600	
57.58	232.35	0.0600	
86.37	226.18	0.0600	
109.41	224.31	0.0600	
115.17	223.86	0.0600	
120.92	222.78	0.0600	
126.68	221.93	0.0600	
128.06	221.50	0.0600 /0.0300	Main Channel
129.56	220.00	0.0300	Main Channel
141.56	220.00	0.0300	Main Channel
143.06	221.50	0.0300 /0.0600	Main Channel
149.72	223.77	0.0600	
155.47	225.09	0.0600	
253.36	229.21	0.0600	
333.98	227.50	0.0600	
454.90	229.25	0.0600	
500.97	233.11	0.0600	
570.07	237.42	0.0600	

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.75	220.75	.262E+05	5.5	0.57	79.44
1.50	221.50	.555E+05	17.5	0.86	52.89
2.44	222.44	.103E+06	42.7	1.14	40.03
3.37	223.37	.171E+06	79.4	1.27	35.98
4.31	224.31	.263E+06	128.1	1.33	34.22
5.25	225.25	.393E+06	191.7	1.34	34.19
6.18	226.18	.594E+06	276.7	1.28	35.77
7.12	227.12	.872E+06	395.9	1.26	36.33
8.06	228.06	.127E+07	542.7	1.17	38.92
8.99	228.99	.199E+07	770.0	1.06	43.01
9.93	229.93	.297E+07	1151.8	1.06	42.95
10.86	230.86	.400E+07	1652.9	1.13	40.33
11.80	231.80	.507E+07	2251.8	1.22	37.53
12.74	232.74	.618E+07	2946.5	1.31	34.97
13.67	233.67	.733E+07	3726.8	1.39	32.79
14.61	234.61	.853E+07	4591.0	1.48	30.96
15.55	235.55	.977E+07	5544.1	1.55	29.37
16.48	236.48	.111E+08	6586.7	1.63	27.99
17.42	237.42	.124E+08	7719.2	1.71	26.77

<----- hydrograph -----> <-pipe / channel->

INFLOW : ID= 2 (8156)	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
791.12	8.98	6.75	14.90	1.19	0.70	
OUTFLOW : ID= 1 (6220)	791.12	6.54	7.25	14.90	1.04	0.67

INFLOW : ID= 2 (5504) ***** 41.19 8.50 10.57 2.38 1.12
 OUTFLOW: ID= 1 (6220) ***** 40.31 9.75 10.57 2.35 1.11

5.0230 3.2496 | 0.0000 0.0000

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (2221) 145.200 3.967 6.00 18.77
 OUTFLOW: ID= 1 (5222) 145.200 0.436 9.50 18.77

PEAK FLOW REDUCTION [Qout/Qin](%) = 10.99
 TIME SHIFT OF PEAK FLOW (min) = 210.00
 MAXIMUM STORAGE USED (ha.m.) = 1.8882

CALIB
 NASHVD (0220) Area (ha) = 169.10 Curve Number (CN) = 73.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.50

Unit Hyd Qpeak (cms) = 1.923

PEAK FLOW (cms) = 0.592 (i)
 TIME TO PEAK (hrs) = 8.250
 RUNOFF VOLUME (mm) = 10.396
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.248

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8160)
 1 + 2 = 3 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0220): 169.10 0.592 8.25 10.40
 + ID2= 2 (6220): 16577.09 40.310 9.75 10.57
 ID = 3 (8160): 16746.19 40.864 9.75 10.57

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHVD (0226) Area (ha) = 237.58 Curve Number (CN) = 75.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.05

Unit Hyd Qpeak (cms) = 3.852

PEAK FLOW (cms) = 1.169 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 11.146
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.265

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (2221) Area (ha) = 145.20 Curve Number (CN) = 75.0
 ID= 1 DT=15.0 min Total Imp(%) = 36.00 Dir. Conn.(%) = 19.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	52.27	92.93
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	983.87	40.00
Mannings n	0.013	0.250
Max.Eff.Inten.(mm/hr)	55.44	16.27
over (min)	15.00	45.00
Storage Coeff. (min)	15.70 (ii)	37.81 (iii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.07	0.03

TOTALS
 PEAK FLOW (cms) = 3.03 2.31 3.967 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 40.00 13.80 18.77
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.33 0.45

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5222)
 IN= 2 -> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	6.4430	3.7677
0.4620	2.0014	8.2830	4.2576
2.0160	2.2850	14.1970	4.6909
3.6250	2.7987	14.5970	4.7009

CALIB
 STANDHYD (2222) Area (ha) = 74.30 Curve Number (CN) = 75.0
 ID= 1 DT=15.0 min Total Imp(%) = 78.00 Dir. Conn.(%) = 41.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	57.95	16.35
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	703.80	40.00
Mannings n	0.013	0.250
Max.Eff.Inten.(mm/hr)	55.44	93.27
over (min)	15.00	30.00
Storage Coeff. (min)	12.84 (ii)	23.84 (iii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04

TOTALS
 PEAK FLOW (cms) = 3.64 1.99 4.938 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 40.00 22.46 29.65
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.53 0.71

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8164)
 1 + 2 = 3 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (2222): 74.30 4.938 6.00 29.65
 + ID2= 2 (5222): 145.20 0.436 9.50 18.77
 ID = 3 (8164): 219.50 5.054 6.00 22.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (0224) Area (ha) = 140.45 Curve Number (CN) = 75.0
 ID= 1 DT=15.0 min Total Imp(%) = 34.00 Dir. Conn.(%) = 26.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	47.75	92.70
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	967.64	40.00
Mannings n	0.013	0.250
Max.Eff.Inten.(mm/hr)	55.44	15.14
over (min)	15.00	45.00
Storage Coeff. (min)	15.54 (ii)	38.50 (iii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.07	0.03

TOTALS
 PEAK FLOW (cms) = 4.03 2.13 4.885 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 40.00 14.42 21.07
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.34 0.50

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8166)
 1 + 2 = 3 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)

ID1= 1 (0224): 140.45 4.885 6.00 21.07
 + ID2= 2 (8164): 219.50 5.054 6.00 22.45
 =====
 ID = 3 (8166): 359.95 9.939 6.00 21.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6226) |
 IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (2261.0) -----

Distance	Elevation	Manning
0.00	245.35	0.0600
8.27	242.72	0.0600
16.54	240.01	0.0600
31.02	235.31	0.0600
41.36	233.14	0.0600
90.99	227.73	0.0600
93.06	227.50	0.0600
95.13	227.27	0.0600
95.83	227.00	0.0600
100.83	226.50	0.0600 / 0.0350 Main Channel
101.33	225.50	0.0350 Main Channel
101.83	226.50	0.0350 / 0.0600 Main Channel
105.83	227.00	0.0600
107.54	227.39	0.0600
109.60	227.62	0.0600
111.67	227.86	0.0600
146.83	231.76	0.0600
173.71	234.77	0.0600
186.12	238.23	0.0600
202.67	238.35	0.0600

----- TRAVEL TIME TABLE -----

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.50	226.00	403E+03	0.0	0.16	330.59
1.00	226.50	161E+04	0.1	0.26	208.26
1.70	227.20	173E+05	1.5	0.27	195.72
2.39	227.89	541E+05	5.9	0.35	152.27
3.09	228.59	119E+06	16.0	0.43	124.08
3.79	229.29	212E+06	33.8	0.51	104.65
4.49	229.99	334E+06	61.2	0.59	90.97
5.18	230.68	484E+06	99.7	0.66	80.88
5.88	231.38	663E+06	151.0	0.73	73.13
6.58	232.08	870E+06	216.5	0.80	66.95
7.27	232.77	111E+07	297.5	0.87	61.91
7.97	233.47	137E+07	398.1	0.94	57.29
8.67	234.17	165E+07	518.9	1.01	53.12
9.36	234.86	196E+07	658.9	1.08	49.59
10.06	235.56	228E+07	827.0	1.17	46.02
10.76	236.26	262E+07	1015.4	1.25	42.96
11.46	236.96	296E+07	1221.2	1.33	40.41
12.15	237.65	332E+07	1444.6	1.40	38.26
12.85	238.35	368E+07	1598.2	1.40	38.42

----- hydrograph ----- <-pipe / channel-->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8166)	359.95	9.94	6.00	21.91	2.67
OUTFLOW : ID= 1 (6226)	359.95	2.49	6.75	21.91	1.86

----- ADD HYD (8168) -----

1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0226):	237.58	1.169	7.50	11.15
+ ID2= 2 (6226):	359.95	2.486	6.75	21.91
ID = 3 (8168):	597.53	3.572	6.75	17.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ADD HYD (8162) -----

1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8160):	16746.19	40.864	9.75	10.57
+ ID2= 2 (8168):	597.53	3.572	6.75	17.63
ID = 3 (8162):	17343.72	43.595	9.25	10.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6228) |
 IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (2281.0) -----

Distance	Elevation	Manning
0.00	245.00	0.0600
18.60	244.74	0.0600
37.20	242.47	0.0600
93.01	233.71	0.0600
125.57	226.95	0.0600
213.93	221.63	0.0600
218.58	221.15	0.0600
223.23	220.45	0.0600
225.03	220.55	0.0600 / 0.0300 Main Channel
225.53	218.05	0.0300 Main Channel
232.53	218.05	0.0300 Main Channel
240.03	218.05	0.0300 Main Channel
241.03	220.55	0.0300 / 0.0600 Main Channel
246.48	221.00	0.0600
251.13	221.27	0.0600
255.78	221.22	0.0600
325.54	225.25	0.0600
367.40	228.91	0.0600
404.60	231.75	0.0600
460.41	245.00	0.0600

----- TRAVEL TIME TABLE -----

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
1.25	219.30	338E+05	16.0	0.86	35.15
2.50	220.55	696E+05	48.3	1.26	24.02
3.94	221.99	168E+06	116.0	1.26	24.14
5.38	223.43	392E+06	247.5	1.15	26.37
6.81	224.86	743E+06	473.6	1.16	26.20
8.25	226.30	122E+07	821.6	1.23	24.67
9.69	227.74	179E+07	1320.1	1.34	22.59
11.13	229.18	243E+07	1965.7	1.47	20.58
12.57	230.62	313E+07	2744.2	1.59	19.02
14.01	232.06	390E+07	3683.4	1.72	17.66
15.44	233.49	472E+07	4824.1	1.86	16.30
16.88	234.93	557E+07	6092.4	1.99	15.23
18.32	236.37	646E+07	7503.3	2.11	14.35
19.76	237.81	739E+07	9061.2	2.23	13.59
21.20	239.25	836E+07	10767.6	2.34	12.94
22.64	240.69	937E+07	12624.8	2.45	12.37
24.07	242.12	104E+08	14634.9	2.56	11.87
25.51	243.56	115E+08	16762.8	2.65	11.45
26.95	245.00	127E+08	18699.9	2.69	11.28

----- hydrograph ----- <-pipe / channel-->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8162)	43.59	9.25	10.81	2.32	1.18
OUTFLOW : ID= 1 (6228)	43.24	9.50	10.81	2.31	1.18

CALIB (0228) | Area (ha)= 310.54 Curve Number (CN)= 61.0
 | HASHYD | Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 | ID= 1 DT=15.0 min | U.H. Tp (hrs)= 1.12

 Unit Hyd Qpeak (cms)= 4.751
 PEAK FLOW (cms)= 0.879 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 6.806
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.162

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- ADD HYD (8170) -----

1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0228):	310.54	0.879	7.75	6.81
+ ID2= 2 (6228):	17343.72	43.238	9.50	10.81
ID = 3 (8170):	17654.26	43.987	9.50	10.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (2302) | Area (ha)= 126.70
 | STANHYD | Total Imp(%)= 23.00 Dir. Conn.(%)= 9.00
 | ID= 1 DT=15.0 min |

Surface Area (ha)= 28.14 IMMERSED PERVIOUS (i)
 Dep. Storage (mm)= 2.00 97.56
 Average Slope (%)= 0.50 5.00
 Length (m)= 919.06 40.00

Mannings n = 0.013 0.250
 Max.Eff.Inten.(mm/hr)= 55.44 11.40
 over (min) 15.00 45.00
 Storage Coeff. (min)= 15.07 (ii) 40.57 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.07 0.03
 PEAK FLOW (cms)- 1.28 1.64 2.023 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.50
 RUNOFF VOLUME (mm)= 40.00 10.62 13.26
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.25 0.32

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (2301) Area (ha)= 78.80 Dir. Conn.(%)= 19.00
 Total Imp(%)= 50.00
 IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 39.40 39.40
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 724.80 40.00
 Mannings n = 0.013 0.250
 Max.Eff.Inten.(mm/hr)= 55.44 20.90
 over (min) 15.00 45.00
 Storage Coeff. (min)= 13.07 (ii) 33.08 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03
 PEAK FLOW (cms)- 1.78 1.35 2.332 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 40.00 13.85 18.82
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.33 0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5230)
 IN= 2--> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	5.6350	1.5914
	0.3160	0.7624	6.9200	1.8001
	1.7040	0.8931	7.9010	2.0070
	3.5140	1.1218	8.3010	2.0170
	4.5020	1.3405	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (2301)	78.800	2.332	6.00	18.82
OUTFLOW : ID= 1 (5230)	78.800	0.752	7.25	18.81

PEAK FLOW REDUCTION [Qout/Qin](%)= 32.25
 TIME SHIFT OF PEAK FLOW (min)= 75.00
 MAXIMUM STORAGE USED (ha.m.)= 0.8042

ADD HYD (8172)
 1 + 2 = 3
 ID1= 1 (2302): 126.70 2.023 6.50 13.26
 + ID2= 2 (5230): 78.80 0.752 7.25 18.81
 ID = 3 (8172): 205.50 2.269 6.50 15.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB

NASHYD (8232) Area (ha)= 314.80 Curve Number (CN)= 58.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 150
 U.H. Tp(hrs)= 1.00

Unit Hyd Qpeak (cms)= 5.391
 PEAK FLOW (cms)= 0.870 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 6.133
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.146

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8173)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0232): 314.80 0.870 7.50 6.13
 + ID2= 2 (8172): 205.50 2.269 6.50 15.39
 ID = 3 (8173): 520.30 3.018 6.50 9.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8174)
 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8170): 17654.26 43.987 9.50 10.74
 + ID2= 2 (8173): 520.30 3.018 6.50 9.79
 ID = 3 (8174): 18174.56 45.297 9.50 10.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6234)
 IN= 2--> OUT= 1 Routing time step (min)'= 15.00

DATA FOR SECTION (2341.0) ----->
 Distance Elevation Manning
 0.00 228.00 0.0550
 42.86 223.21 0.0550
 85.72 219.56 0.0550
 107.15 219.42 0.0550
 128.58 219.39 0.0550
 214.30 219.30 0.0550
 225.02 219.26 0.0550
 235.73 219.24 0.0550
 255.16 219.20 0.0550 /0.0350 Main Channel
 257.16 217.20 0.0350 Main Channel
 303.16 217.20 0.0350 Main Channel
 305.16 219.20 0.0350 /0.0550 Main Channel
 310.74 219.28 0.0550
 321.45 219.27 0.0550
 332.17 219.26 0.0550
 396.46 219.29 0.0550
 782.20 222.91 0.0550
 878.64 227.43 0.0550
 964.36 227.80 0.0550
 1060.79 228.00 0.0550

TRAVEL TIME TABLE ----->
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
 (m) (m) (cu.m.) (cms) (m/s) (min)
 0.50 217.70 .887E+05 7.2 0.31 206.54
 1.00 218.20 .179E+06 22.7 0.48 131.74
 1.50 218.70 .272E+06 44.5 0.62 101.75
 2.00 219.20 .366E+06 71.8 0.75 84.97
 2.59 219.79 .993E+06 136.1 0.52 121.59
 3.17 220.37 .169E+07 258.6 0.52 121.80
 3.76 220.96 .294E+07 434.7 0.56 112.82
 4.35 221.55 .415E+07 665.9 0.61 103.88
 4.93 222.13 .551E+07 955.0 0.66 96.22
 5.52 222.72 .703E+07 1305.1 0.71 89.80
 6.11 223.31 .868E+07 1744.9 0.77 82.91
 6.69 223.89 .104E+08 2261.4 0.83 76.46
 7.28 224.48 .121E+08 2838.0 0.89 71.12
 7.87 225.07 .139E+08 3472.9 0.95 66.63
 8.45 225.65 .157E+08 4165.0 1.01 62.82
 9.04 226.24 .176E+08 4913.5 1.07 59.54
 9.63 226.83 .194E+08 5717.7 1.12 56.68
 10.21 227.41 .214E+08 6577.1 1.17 54.18
 10.80 228.00 .235E+08 6919.5 1.12 56.61

hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8174) ***** 45.30 9.50 10.71 1.51 0.63

OUTFLOW: ID= 1 (6234) ***** 39.99 11.25 10.71 1.40 0.59

CALIB	
STANDHYD (0234)	Area (ha)= 267.16
ID= 1 DT=15.0 min	Total Imp(%)= 22.00 Dir. Conn.(%)= 8.00
IMPERVIOUS PERVIOUS (i)	
Surface Area (ha)=	58.78 208.38
Dep. Storage (mm)=	2.00 5.00
Average Slope (%)=	0.50 0.50
Length (m)=	1334.57 40.00
Mannings n =	0.013 0.250
Max. Ref. Inten. (mm/hr)=	55.44 3.02
over (min)=	15.00 75.00
Storage Coeff. (min)=	18.85 (ii) 62.20 (iii)
Unit Hyd. Tpeak (min)=	15.00 75.00
Unit Hyd. peak (cms)=	0.06 0.02
TOTALS	
PEAK FLOW (cms)=	2.16 0.86 2.347 (iii)
TIME TO PEAK (hrs)=	6.00 7.00 6.00
RUNOFF VOLUME (mm)=	40.00 3.81 6.70
TOTAL RAINFALL (mm)=	42.00 42.00 42.00
RUNOFF COEFFICIENT =	0.95 0.09 0.16

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 39.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8176)	
1 + 2 = 3	AREA QPEAK TPEAK R.V.
	(ha) (cms) (hrs) (mm)
ID1= 1 (0234):	267.16 2.347 6.00 6.70
+ ID2= 2 (6234):	18174.56 39.994 11.25 10.71
=====	
ID = 3 (8176):	18441.72 40.228 11.00 10.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	
NASHYD (0238)	Area (ha)= 311.70 Curve Number (CN)= 53.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.56	
Unit Hyd Qpeak (cms)=	3.405
PEAK FLOW (cms)=	0.520 (i)
TIME TO PEAK (hrs)=	8.500
RUNOFF VOLUME (mm)=	5.193
TOTAL RAINFALL (mm)=	42.000
RUNOFF COEFFICIENT =	0.124
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.	

CALIB	
NASHYD (0236)	Area (ha)= 494.49 Curve Number (CN)= 54.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.24	
Unit Hyd Qpeak (cms)=	6.830
PEAK FLOW (cms)=	1.022 (i)
TIME TO PEAK (hrs)=	8.000
RUNOFF VOLUME (mm)=	5.363
TOTAL RAINFALL (mm)=	42.000
RUNOFF COEFFICIENT =	0.128
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.	

ROUTE CHN (6238)	
ID= 2 -> OUTF 1	Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2381.0) ----->		
Distance	Elevation	Manning
0.00	241.54	0.0380
602.55	227.00	0.0380
1702.00	224.50	0.0380
1721.25	223.00	0.0380

1725.10	222.60	0.0380
1780.94	222.50	0.0380
1782.87	222.45	0.0380
1784.79	222.40	0.0380 / 0.0300
1786.57	221.75	0.0300
1787.07	220.75	0.0300
1787.57	220.75	0.0300
1791.57	220.75	0.0300
1794.07	221.75	0.0300
1798.27	222.00	0.0300
1800.19	222.35	0.0300 / 0.0380
1802.12	222.47	0.0380
1840.63	223.00	0.0380
1900.00	225.00	0.0380
1950.00	226.00	0.0380
2242.61	240.00	0.0380

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.80	221.55	1.01E+05	5.2	1.14	32.36
1.60	222.35	2.96E+05	18.7	1.40	26.36
2.64	223.39	2.49E+06	151.8	1.35	27.33
3.68	224.43	6.13E+06	512.2	1.85	19.94
4.71	225.46	1.52E+07	1066.8	1.55	23.80
5.75	226.50	3.59E+07	2824.9	1.75	21.16
6.79	227.54	6.63E+07	6528.0	2.18	16.93
7.83	228.58	9.94E+07	12158.6	2.71	13.62
8.87	229.62	1.34E+08	19268.8	3.19	11.58
9.91	230.66	1.70E+08	27785.3	3.63	10.19
10.94	231.69	2.07E+08	37667.1	4.03	9.18
11.98	232.73	2.46E+08	48991.7	4.40	8.40
13.02	233.77	2.87E+08	61448.0	4.75	7.78
14.06	234.81	3.29E+08	75332.7	5.08	7.28
15.10	235.85	3.72E+08	90547.8	5.39	6.85
16.14	236.89	4.17E+08	*****	5.69	6.49
17.17	237.92	4.64E+08	*****	5.98	6.18
18.21	238.96	5.12E+08	*****	6.25	5.91
19.25	240.00	5.61E+08	*****	6.52	5.67

<----- hydrograph ----->					
<-pipe / channel-->					
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH
	(ha)	(cms)	(hrs)	(mm)	(m)
INFLOW : ID= 2 (0236)	494.49	1.02	8.00	5.36	0.16
OUTFLOW: ID= 1 (6238)	494.49	0.99	8.50	5.36	0.15

ADD HYD (8180)	
1 + 2 = 3	AREA QPEAK TPEAK R.V.
	(ha) (cms) (hrs) (mm)
ID1= 1 (0238):	311.70 0.520 8.50 5.19
+ ID2= 2 (6238):	494.49 0.991 8.50 5.36
=====	
ID = 3 (8180):	806.19 1.511 8.50 5.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8178)	
1 + 2 = 3	AREA QPEAK TPEAK R.V.
	(ha) (cms) (hrs) (mm)
ID1= 1 (8176):	18441.72 40.228 11.00 10.65
+ ID2= 2 (8180):	806.19 1.511 8.50 5.30
=====	
ID = 3 (8178):	19247.91 41.487 11.00 10.43

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6240)	
ID= 2 -> OUTF 1	Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2401.0) ----->		
Distance	Elevation	Manning
0.00	222.00	0.0450
11.46	221.00	0.0450
208.98	221.00	0.0450
404.04	220.97	0.0450
808.08	220.83	0.0450
905.60	220.17	0.0450
919.53	219.43	0.0450
933.47	219.22	0.0450
945.26	219.21	0.0450 / 0.0300
946.26	217.81	0.0300
975.26	217.81	0.0300
1003.26	217.81	0.0300
1005.26	219.21	0.0300 / 0.0450
1017.06	219.28	0.0450
1030.99	219.26	0.0450

1044.92 219.23 0.0450
 1058.86 219.23 0.0450
 1253.91 219.22 0.0450
 1323.57 221.05 0.0450
 1379.30 222.00 0.0450

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.20	218.01	307E+05	2.2	0.20	227.52
0.40	218.21	617E+05	7.1	0.31	143.95
0.60	218.41	928E+05	14.0	0.41	110.33
0.80	218.61	124E+06	22.6	0.49	91.46
1.00	218.81	156E+06	32.8	0.57	79.15
1.20	219.01	188E+06	44.5	0.64	70.38
1.40	219.21	220E+06	57.5	0.70	63.77
1.63	219.44	413E+06	83.5	0.54	83.52
1.87	219.68	632E+06	120.9	0.51	87.03
2.10	219.91	858E+06	170.0	0.53	84.11
2.33	220.14	109E+07	228.7	0.56	79.62
2.56	220.37	134E+07	294.4	0.59	76.03
2.80	220.61	162E+07	371.1	0.61	72.75
3.03	220.84	192E+07	456.8	0.64	70.18
3.26	221.07	251E+07	552.1	0.59	75.74
3.49	221.30	333E+07	711.8	0.57	78.07
3.73	221.54	417E+07	907.0	0.58	76.62
3.96	221.77	502E+07	1134.1	0.61	73.70
4.19	222.00	587E+07	1390.9	0.64	70.34

hydrograph

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
41.49	11.00	10.43	1.15	0.62	
39.02	12.25	10.43	1.11	0.60	

CALIB (0240) Area (ha)= 434.37 Curve Number (CN)= 46.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 3.60

Unit Hyd Qpeak (cms)= 2.058
 PEAK FLOW (cms)= 0.304 (l)
 TIME TO PEAK (hrs)= 12.000
 RUNOFF VOLUME (mm)= 4.078
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.097

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8182)
 1 + 2 = 3
 ID1= 1 (0240): 434.37 0.304 12.00 4.08
 + ID2= 2 (6240): 19247.91 39.020 12.25 10.43
 ID = 3 (8182): 19682.28 39.323 12.25 10.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0242) Area (ha)= 657.88 Curve Number (CN)= 52.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 5.37

Unit Hyd Qpeak (cms)= 2.090
 PEAK FLOW (cms)= 0.402 (l)
 TIME TO PEAK (hrs)= 13.000
 RUNOFF VOLUME (mm)= 5.039
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.120

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8184)
 1 + 2 = 3
 ID1= 1 (0242): 657.88 0.402 13.00 5.04
 + ID2= 2 (8182): 19682.28 39.323 12.25 10.29
 ID = 3 (8184): 20340.16 39.722 12.25 10.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6244)
 IN= 2--> OUT= 1
 Routing time step (min)= 15.00

DATA FOR SECTION (2441.0)

Distance	Elevation	Manning
0.00	225.00	0.0450
2.50	220.10	0.0450
3.00	220.06	0.0450
3.64	220.07	0.0450
7.28	220.08	0.0450
10.91	220.09	0.0450
14.55	219.81	0.0450
18.19	219.29	0.0450
24.13	219.21	0.0450 / 0.0300 Main Channel
24.63	217.81	0.0300 Main Channel
69.13	217.81	0.0300 Main Channel
114.13	217.81	0.0300 Main Channel
115.13	219.21	0.0300 / 0.0450 Main Channel
120.06	219.25	0.0450
123.70	219.32	0.0450
127.34	219.77	0.0450
130.98	219.88	0.0450
134.61	219.99	0.0450
350.00	220.18	0.0450
360.18	225.00	0.0450

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.35	218.16	1.08E+06	9.0	0.29	200.52
0.70	218.51	2.16E+06	28.4	0.45	126.89
1.05	218.86	3.25E+06	55.7	0.59	97.27
1.40	219.21	4.34E+06	89.7	0.71	80.65
1.79	219.60	5.74E+06	135.9	0.81	70.36
2.17	219.98	7.26E+06	190.8	0.90	63.38
2.56	220.37	1.11E+07	264.3	0.82	69.82
2.94	220.75	1.57E+07	371.7	0.81	70.36
3.33	221.14	2.03E+07	503.4	0.85	67.30
3.72	221.53	2.50E+07	656.6	0.90	63.40
4.10	221.91	2.96E+07	829.6	0.96	59.54
4.49	222.30	3.43E+07	1021.1	1.02	56.00
4.87	222.68	3.90E+07	1230.2	1.08	52.83
5.26	223.07	4.37E+07	1456.2	1.14	50.01
5.65	223.46	4.84E+07	1698.4	1.20	47.51
6.03	223.84	5.31E+07	1956.3	1.26	45.28
6.42	224.23	5.79E+07	2229.3	1.32	43.28
6.80	224.61	6.26E+07	2517.0	1.38	41.48
7.19	225.00	6.74E+07	2819.2	1.44	39.85

hydrograph

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
39.72	12.25	10.12	0.85	0.50	
35.55	14.00	10.12	0.79	0.48	

CALIB (0244) Area (ha)=1908.71 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 2.20

Unit Hyd Qpeak (cms)= 14.812
 PEAK FLOW (cms)= 3.274 (l)
 TIME TO PEAK (hrs)= 10.000
 RUNOFF VOLUME (mm)= 6.844
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.163

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8186)
 1 + 2 = 3
 ID1= 1 (0244): 1908.71 3.274 10.00 6.84
 + ID2= 2 (6244): 20340.16 35.549 14.00 10.12
 ID = 3 (8186): 22248.87 37.982 13.75 9.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ HYD (8190) AREA (ha)=*****
 DT=15.0 min TPEAK (hrs)= 12.50
 VOLUME (mm)= 8.64

46.00	1.312	103.25	0.228	160.50	0.044	217.75	0.008	275.00	0.001
46.25	1.294	103.50	0.226	160.75	0.043	218.00	0.008	275.25	0.001
46.50	1.277	103.75	0.224	161.00	0.043	218.25	0.008	275.50	0.001
46.75	1.260	104.00	0.223	161.25	0.043	218.50	0.008	275.75	0.001
47.00	1.243	104.25	0.221	161.50	0.042	218.75	0.008	276.00	0.001
47.25	1.227	104.50	0.220	161.75	0.042	219.00	0.008	276.25	0.001
47.50	1.212	104.75	0.218	162.00	0.042	219.25	0.008	276.50	0.001
47.75	1.197	105.00	0.217	162.25	0.041	219.50	0.008	276.75	0.001
48.00	1.182	105.25	0.215	162.50	0.041	219.75	0.007	277.00	0.001
48.25	1.168	105.50	0.214	162.75	0.041	220.00	0.007	277.25	0.001
48.50	1.154	105.75	0.212	163.00	0.040	220.25	0.007	277.50	0.001
48.75	1.141	106.00	0.211	163.25	0.040	220.50	0.007	277.75	0.001
49.00	1.128	106.25	0.209	163.50	0.040	220.75	0.007	278.00	0.001
49.25	1.115	106.50	0.208	163.75	0.040	221.00	0.007	278.25	0.001
49.50	1.103	106.75	0.206	164.00	0.039	221.25	0.007	278.50	0.001
49.75	1.091	107.00	0.205	164.25	0.039	221.50	0.007	278.75	0.001
50.00	1.079	107.25	0.203	164.50	0.039	221.75	0.007	279.00	0.001
50.25	1.067	107.50	0.202	164.75	0.038	222.00	0.007	279.25	0.001
50.50	1.056	107.75	0.200	165.00	0.038	222.25	0.007	279.50	0.001
50.75	1.045	108.00	0.199	165.25	0.038	222.50	0.007	279.75	0.001
51.00	1.035	108.25	0.198	165.50	0.038	222.75	0.007	280.00	0.001
51.25	1.024	108.50	0.196	165.75	0.037	223.00	0.007	280.25	0.001
51.50	1.014	108.75	0.195	166.00	0.037	223.25	0.007	280.50	0.001
51.75	1.004	109.00	0.193	166.25	0.037	223.50	0.007	280.75	0.001
52.00	0.994	109.25	0.192	166.50	0.036	223.75	0.007	281.00	0.001
52.25	0.985	109.50	0.191	166.75	0.036	224.00	0.007	281.25	0.001
52.50	0.976	109.75	0.189	167.00	0.036	224.25	0.006	281.50	0.001
52.75	0.966	110.00	0.188	167.25	0.036	224.50	0.006	281.75	0.001
53.00	0.958	110.25	0.187	167.50	0.035	224.75	0.006	282.00	0.001
53.25	0.949	110.50	0.185	167.75	0.035	225.00	0.006	282.25	0.001
53.50	0.940	110.75	0.184	168.00	0.035	225.25	0.006	282.50	0.001
53.75	0.932	111.00	0.183	168.25	0.035	225.50	0.006	282.75	0.001
54.00	0.924	111.25	0.182	168.50	0.034	225.75	0.006	283.00	0.001
54.25	0.916	111.50	0.180	168.75	0.034	226.00	0.006	283.25	0.001
54.50	0.908	111.75	0.179	169.00	0.034	226.25	0.006	283.50	0.001
54.75	0.900	112.00	0.178	169.25	0.034	226.50	0.006	283.75	0.001
55.00	0.892	112.25	0.176	169.50	0.033	226.75	0.006	284.00	0.001
55.25	0.885	112.50	0.175	169.75	0.033	227.00	0.006	284.25	0.001
55.50	0.877	112.75	0.174	170.00	0.033	227.25	0.006	284.50	0.001
55.75	0.870	113.00	0.173	170.25	0.033	227.50	0.006	284.75	0.001
56.00	0.863	113.25	0.171	170.50	0.032	227.75	0.006	285.00	0.001
56.25	0.856	113.50	0.170	170.75	0.032	228.00	0.006	285.25	0.001
56.50	0.849	113.75	0.168	171.00	0.032	228.25	0.006	285.50	0.001
56.75	0.842	114.00	0.168	171.25	0.032	228.50	0.006	285.75	0.001
57.00	0.835	114.25	0.167	171.50	0.031	228.75	0.006		

ADD HYD (8188)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (8186):	22248.87	37.982	13.75	9.84	
+ ID2= 2 (8190):	1908.71	3.274	10.00	6.84	
=====					
ID = 3 (8188):	24157.57	40.477	13.50	9.60	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (4462)			
Area	(ha)=1366.10	Curve Number	(CN)= 61.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50	U.H. Tp(hrs)= 4.22

Unit Hyd Qpeak (cms)= 5.524

PEAK FLOW (cms)= 1.411 (i)
 TIME TO PEAK (hrs)= 12.250
 RUNOFF VOLUME (mm)= 6.858
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.163

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (4461)			
Area	(ha)= 108.90	Dir. Conn.(%)= 30.00	Total Imp(%)= 50.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)	54.45	54.45	
Dep. Storage (mm)	2.00	5.00	
Average Slope (%)	0.50	0.50	
Length (m)	852.06	40.00	
Mannings n	0.013	0.250	

Max.Eff.Inten.(mm/hr)= 55.44 12.08
 over (min) 15.00 45.00
 Storage Coeff. (min)= 14.40 (ii) 39.32 (iii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.07 0.03

PEAK FLOW (cms)= 3.73 0.99 *TOTALS*
 4.121 (iii)

TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 40.00 9.56 18.69
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.23 0.45

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PervIOUS LOSSES:
- CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5446)				
In= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	1.6700	2.9494
	0.3690	1.4413	1.8200	3.3343
	0.5700	1.6830	1.9700	3.7111
	1.0600	2.1139	2.3700	3.7211
	1.3800	2.4971	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (4461):	108.900	4.121	6.00	18.69
OUTFLOW: ID= 1 (5446):	108.900	0.355	8.75	18.69

PEAK FLOW REDUCTION [Qout/Qin](%)= 8.62
 TIME SHIFT OF PEAK FLOW (min)=165.00
 MAXIMUM STORAGE USED (ha.m.)= 1.3873

ADD HYD (8192)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (4462):	1366.10	1.411	12.25	6.86	
+ ID2= 2 (5446):	108.90	0.355	8.75	18.69	
=====					
ID = 3 (8192):	1475.00	1.732	12.00	7.73	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8194)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (8188):	24157.57	40.477	13.50	9.60	
+ ID2= 2 (8192):	1475.00	1.732	12.00	7.73	
=====					
ID = 3 (8194):	25632.58	42.140	13.50	9.49	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0442)			
Area	(ha)= 117.26	Curve Number	(CN)= 43.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50	U.H. Tp(hrs)= 1.17

Unit Hyd Qpeak (cms)= 1.705

PEAK FLOW (cms)= 0.170 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 3.634
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.087

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0440)			
Area	(ha)= 226.35	Curve Number	(CN)= 60.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50	U.H. Tp(hrs)= 1.11

Unit Hyd Qpeak (cms)= 3.481

PEAK FLOW (cms)= 0.621 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 6.577
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.157

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0438) Area (ha)= 130.70 Curve Number (CN)= 51.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.60

Unit Hyd Qpeak (cms)= 3.738
 PEAK FLOW (cms)= 0.416 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 4.765
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.113

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0436) Area (ha)= 187.51 Curve Number (CN)= 55.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.73

Unit Hyd Qpeak (cms)= 4.391
 PEAK FLOW (cms)= 0.592 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 5.501
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.131

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0434) Area (ha)= 56.64 Curve Number (CN)= 46.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.52

Unit Hyd Qpeak (cms)= 1.867
 PEAK FLOW (cms)= 0.167 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 3.974
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.095

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8206)
 1 + 2 = 3
 ID1= 1 (0434): 56.64 0.167 6.50 3.97
 + ID2= 2 (0436): 187.51 0.592 7.00 5.50
 ID = 3 (8206): 244.15 0.751 7.00 5.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6438)
 IN= 2--> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (4381.0) ----->

Distance	Elevation	Manning
0.00	256.00	0.0600
9.23	255.50	0.0600
27.69	255.00	0.0600
50.77	254.90	0.0600
64.62	254.85	0.0600
78.47	252.26	0.0600
96.93	249.44	0.0600
129.24	245.94	0.0600
145.40	245.76	0.0600
146.20	245.80	0.0600 / 0.0350 Main Channel
146.70	245.30	0.0350 Main Channel
147.70	245.30	0.0350 Main Channel
148.20	245.80	0.0350 / 0.0600 Main Channel
150.01	245.78	0.0600
152.32	245.79	0.0600
163.86	246.72	0.0600
175.40	249.07	0.0600
186.94	253.15	0.0600
196.17	255.38	0.0600
228.48	255.55	0.0600

TRAVEL TIME TABLE
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
 (m) (m) (cu.m.) (cms) (m/s) (min)
 0.50 245.80 .221E+04 0.9 1.02 40.44

1.01	246.31	.352E+05	14.0	0.99	41.77
1.53	246.83	.841E+05	47.3	1.39	29.61
2.04	247.34	.144E+06	102.0	1.76	23.49
2.55	247.85	.213E+06	177.3	2.07	19.99
3.07	248.37	.293E+06	274.4	2.34	17.66
3.58	248.88	.378E+06	394.6	2.58	15.97
4.09	249.39	.474E+06	541.4	2.83	14.60
4.61	249.91	.578E+06	719.9	3.08	13.38
5.12	250.42	.688E+06	923.8	3.33	12.40
5.63	250.93	.803E+06	1153.3	3.56	11.61
6.14	251.44	.925E+06	1408.6	3.77	10.95
6.66	251.96	1.05E+07	1690.6	3.98	10.38
7.17	252.47	1.19E+07	2002.3	4.18	9.88
7.68	252.98	1.33E+07	2345.4	4.38	9.43
8.20	253.50	1.47E+07	2710.1	4.56	9.05
8.71	254.01	1.62E+07	3101.0	4.73	8.72
9.22	254.52	1.78E+07	3521.8	4.90	8.43
9.74	255.04	1.95E+07	3467.6	4.39	9.39

<---- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8206) 244.15 0.75 7.00 5.15 0.41 1.02
 OUTFLOW : ID= 1 (6438) 244.15 0.67 7.75 5.15 0.37 1.02

ADD HYD (8208)
 1 + 2 = 3
 ID1= 1 (0438): 130.70 0.416 6.75 4.76
 + ID2= 2 (6438): 244.15 0.666 7.75 5.15
 ID = 3 (8208): 374.85 1.025 7.50 5.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8210)
 1 + 2 = 3
 ID1= 1 (0440): 226.35 0.621 7.75 6.58
 + ID2= 2 (8208): 374.85 1.025 7.50 5.01
 ID = 3 (8210): 601.20 1.644 7.50 5.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0432) Area (ha)= 114.21 Curve Number (CN)= 53.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.21

Unit Hyd Qpeak (cms)= 1.617
 PEAK FLOW (cms)= 0.232 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 5.180
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.123

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0430) Area (ha)= 111.64 Curve Number (CN)= 55.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.52

Unit Hyd Qpeak (cms)= 3.687
 PEAK FLOW (cms)= 0.457 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 5.441
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.130

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0428) Area (ha)= 50.53 Curve Number (CN)= 51.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.50

Unit Hyd Qpeak (cms)= 1.725

PEAK FLOW (cms)= 0.184 (1)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 4.733
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.113

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0426) Area (ha)= 247.17 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.98

Unit Hyd Opeak (cms)= 4.315

PEAK FLOW (cms)= 0.770 (1)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 6.793
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.162

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0424) Area (ha)= 49.57 Curve Number (CN)= 59.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.37

Unit Hyd Opeak (cms)= 2.293

PEAK FLOW (cms)= 0.296 (1)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 6.128
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.146

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8198) 1 + 2 = 3
 ID1= 1 (0424): 49.57 0.296 6.50 6.13
 + ID2= 2 (0426): 247.17 0.770 7.50 6.79
 =====
 ID = 3 (8198): 296.74 0.999 7.00 6.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6428) Routing time step (min)'= 15.00
 IN= 2 -> OUT= 1

Distance	Elevation	Manning	
0.00	246.54	0.0900	
8.80	246.29	0.0900	
30.78	246.03	0.0900	
41.78	243.97	0.0900	
63.77	237.16	0.0900	
85.76	232.53	0.0900	
96.75	230.16	0.0900	
109.94	228.40	0.0900	
112.14	228.28	0.0900 /0.0450	Main Channel
112.84	228.00	0.0450	Main Channel
113.34	227.50	0.0450	Main Channel
114.34	227.50	0.0450	Main Channel
114.84	228.00	0.0450	Main Channel
116.54	228.29	0.0450 /0.0900	Main Channel
118.74	228.40	0.0900	
120.94	228.52	0.0900	
149.52	229.51	0.0900	
169.31	230.69	0.0900	
186.00	234.27	0.0900	
217.69	243.56	0.0900	

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.78	228.28	.205E+04	1.6	0.97	21.60
1.58	229.08	.202E+05	16.6	1.02	20.37
2.39	229.89	.665E+05	64.8	1.22	17.11
3.19	230.69	1.133E+06	158.1	1.49	14.05
4.00	231.50	2.13E+06	308.6	1.82	11.48
4.80	232.30	2.99E+06	504.9	2.11	9.89
5.61	233.11	3.94E+06	746.6	2.37	8.79
6.41	233.91	4.96E+06	1034.5	2.61	7.99
7.21	234.71	6.05E+06	1373.3	2.84	7.35

8.02	235.52	.722E+06	1763.3	3.06	6.82
8.82	236.32	.844E+06	2202.0	3.27	6.39
9.63	237.13	.974E+06	2690.5	3.46	6.03
10.43	237.93	1.11E+07	3239.5	3.66	5.71
11.23	238.73	1.25E+07	3839.0	3.84	5.43
12.04	239.54	1.40E+07	4489.1	4.03	5.18
12.84	240.34	1.55E+07	5190.5	4.20	4.97
13.65	241.15	1.70E+07	5943.8	4.36	4.78
14.45	241.95	1.87E+07	6749.8	4.53	4.61
15.26	242.76	2.04E+07	7609.4	4.68	4.46

hydrograph <---> <-pipe / channel-->
 AREA QPEAK TPEAK R.V. TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8198) 296.74 1.00 7.00 6.68 0.49 0.97
 OUTFLOW : ID= 1 (6428) 296.74 0.96 7.50 6.68 0.47 0.97

ADD HYD (8200) 1 + 2 = 3
 ID1= 1 (0428): 50.53 0.184 6.50 4.73
 + ID2= 2 (6428): 296.74 0.963 7.50 6.68
 =====
 ID = 3 (8200): 347.27 1.118 7.25 6.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8202) 1 + 2 = 3
 ID1= 1 (0430): 111.64 0.457 6.50 5.44
 + ID2= 2 (8200): 347.27 1.118 7.25 6.40
 =====
 ID = 3 (8202): 458.91 1.531 7.00 6.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6432) Routing time step (min)'= 15.00
 IN= 2 -> OUT= 1

Distance	Elevation	Manning	
0.00	221.00	0.0600	
15.29	219.85	0.0600	
45.86	219.86	0.0600	
76.44	219.96	0.0600	
110.84	220.23	0.0600	
112.00	220.15	0.0600	
114.66	220.09	0.0600	
118.48	219.73	0.0600	
120.80	219.70	0.0600 /0.0350	Main Channel
121.30	219.20	0.0350	Main Channel
122.30	219.20	0.0350	Main Channel
122.80	219.70	0.0350 /0.0600	Main Channel
129.95	220.11	0.0600	
175.81	220.24	0.0600	
214.03	220.55	0.0600	
252.25	220.61	0.0600	
290.47	220.70	0.0600	
328.69	221.04	0.0600	
347.80	221.08	0.0600	
378.37	221.04	0.0600	

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.08	219.28	1.85E+03	0.0	0.23	149.38
0.17	219.37	3.99E+03	0.1	0.34	100.62
0.25	219.45	6.41E+03	0.1	0.42	80.98
0.33	219.53	9.11E+03	0.2	0.49	69.79
0.42	219.62	1.21E+04	0.3	0.55	62.35
0.50	219.70	1.54E+04	0.5	0.60	56.91
0.61	219.81	2.67E+04	0.7	0.56	61.28
0.71	219.91	4.01E+04	1.3	0.30	112.96
0.82	220.02	5.49E+05	3.4	0.28	122.71
0.92	220.12	4.52E+05	6.8	0.31	109.98
1.03	220.23	7.47E+05	12.2	0.33	102.05
1.14	220.34	1.12E+06	21.3	0.39	87.59
1.24	220.44	1.53E+06	33.0	0.44	77.01
1.35	220.55	1.96E+06	47.2	0.49	69.22
1.46	220.66	2.48E+06	62.3	0.51	66.48
1.56	220.76	3.10E+06	83.4	0.55	62.06
1.67	220.87	3.76E+06	108.8	0.59	57.56
1.77	220.97	4.44E+06	137.7	0.64	53.77
1.88	221.08	5.17E+06	163.6	0.65	52.70

```

<---- hydrograph ----> <-pipe / channel->
AREA   QPEAK  TPEAK  R.V.  MAX DEPTH  MAX VEL
(ha)   (cms)  (hrs)  (mm)   (m)        (m/s)
INFLOW : ID= 2 (8202) 458.91  1.53  7.00  6.17  0.72  0.30
OUTFLOW : ID= 1 (6432) 458.91  1.14  9.50  6.17  0.68  0.35

```

```

ADD HYD (8204)
1 + 2 = 3 |
AREA   QPEAK  TPEAK  R.V.
(ha)   (cms)  (hrs)  (mm)
ID1= 1 (0432): 114.21  0.232  8.00  5.18
+ ID2= 2 (6432): 458.91  1.138  9.50  6.17
=====
ID = 3 (8204): 573.12  1.344  9.25  5.97

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8212)
1 + 2 = 3 |
AREA   QPEAK  TPEAK  R.V.
(ha)   (cms)  (hrs)  (mm)
ID1= 1 (8204): 573.12  1.344  9.25  5.97
+ ID2= 2 (8210): 601.20  1.644  7.50  5.60
=====
ID = 3 (8212): 1174.32  2.836  8.00  5.78

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6442)
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

```

```

<----- DATA FOR SECTION (4421.0) ----->
Distance  Elevation  Manning
0.00      221.00      0.0350
26.73     220.80      0.0350
53.47     220.60      0.0350
66.83     220.40      0.0350
80.20     220.20      0.0350
347.52    220.00      0.0350
354.21    219.59      0.0350
360.89    219.50      0.0350
367.57    219.08      0.0350
368.76    219.00      0.0350 /0.0300 Main Channel
369.26    218.50      0.0300 Main Channel
374.26    218.50      0.0300 Main Channel
374.76    219.00      0.0300 /0.0350 Main Channel
380.94    220.33      0.0350
387.62    219.62      0.0350
394.31    219.23      0.0350
454.45    219.07      0.0350
514.60    219.09      0.0350
660.00    219.19      0.0350
661.63    221.00      0.0350

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH  ELEV  VOLUME  FLOW RATE  VELOCITY  TRAV.TIME
(m)    (m)    (cu.m.) (cms)      (m/s)     (min)
0.12   218.62  .100E+04  0.2  0.28  92.89
0.25   218.75  .205E+04  0.6  0.43  60.09
0.37   218.87  .315E+04  1.1  0.55  46.99
0.50   219.00  .430E+04  1.8  0.66  39.67
0.63   219.13  .541E+04  2.4  0.77  36.99
0.77   219.27  .652E+04  3.1  0.88  34.14
0.90   219.40  .764E+04  3.8  0.99  32.02
1.03   219.53  .873E+04  4.5  1.09  30.45
1.17   219.67  .985E+04  5.2  1.19  29.30
1.30   219.80  .107E+05  6.0  1.29  28.40
1.43   219.93  .118E+05  6.8  1.38  27.70
1.57   220.07  .130E+05  7.6  1.47  27.20
1.70   220.20  .141E+05  8.4  1.56  26.80
1.83   220.33  .153E+05  9.2  1.65  26.40
1.97   220.47  .164E+05  10.0  1.74  26.10
2.10   220.60  .176E+05  10.8  1.83  25.80
2.23   220.73  .187E+05  11.6  1.91  25.50
2.37   220.87  .199E+05  12.4  2.00  25.20
2.50   221.00  .210E+05  13.2  2.09  25.00

```

```

<---- hydrograph ----> <-pipe / channel->
AREA   QPEAK  TPEAK  R.V.  MAX DEPTH  MAX VEL
(ha)   (cms)  (hrs)  (mm)   (m)        (m/s)
INFLOW : ID= 2 (8212) 1174.32  2.84  8.00  5.78  0.59  0.44
OUTFLOW : ID= 1 (6442) 1174.32  2.60  9.75  5.78  0.57  0.47

```

```

ADD HYD (8214) |

```

```

1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0442): 117.26  0.170  8.00  3.63
+ ID2= 2 (6442): 1174.32  2.599  9.75  5.78
=====
ID = 3 (8214): 1291.58  2.744  9.75  5.59

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8196) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8194): 25632.58  42.140  13.50  9.49
+ ID2= 2 (8214): 1291.58  2.744  9.75  5.59
=====
ID = 3 (8196): 26924.15  43.675  13.25  9.31

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB (0444) | Area (ha)= 221.65 Curve Number (CN)= 56.0
NASHYD (0444) | ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
                | U.H. Tp(hrs)= 1.03

```

Unit Hyd Qpeak (cms)= 3.656

```

PEAK FLOW (cms)= 0.556 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 5.731
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.136

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8216) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0444): 221.65  0.556  7.50  5.73
+ ID2= 2 (8196): 26924.15  43.675  13.25  9.31
=====
ID = 3 (8216): 27145.80  43.879  13.00  9.28

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

**EXISTING
EAST HOLLAND RIVER 5-YEAR STORM**

```
V V I SSSS U U A L
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
WV I SSSS UUUU A A LLLL
```

```
OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y MM MM O O
O O T T H H Y M M O O
OOO T T H H Y M M OOO Company Serial
```

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***** DETAILED OUTPUT *****

Input filename: C:\program files\Visual Ortymo 2.4r\VO2\voim.dat
Output filename: C:\Users\jscott\AppData\Local\Temp\814cdde6-6a8a-4471-bdfd-8b92ff416e4b\Scenario.out
Summary filename: C:\Users\jscott\AppData\Local\Temp\814cdde6-6a8a-4471-bdfd-8b92ff416e4b\Scenario.sum

DATE: 08/22/2012 TIME: 01:21:03

USER:

COMMENTS: _____

** SIMULATION NUMBER: 6 **

```
CHICAGO STORM | IDF curve parameters: A=1770.000
Ptotal= 78.03 mm | B= 4.000
                  | C= 0.820
```

used in: INTENSITY = A / (t + B)^C

Duration of storm = 4.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	4.34	1.17	38.21	2.17	10.60	3.17	5.19
0.33	5.00	1.33	203.31	2.33	8.96	3.33	4.81
0.50	5.92	1.50	50.96	2.50	7.78	3.50	4.48
0.67	7.33	1.67	25.53	2.67	6.90	3.67	4.20
0.83	9.77	1.83	17.18	2.83	6.21	3.83	3.96
1.00	15.10	2.00	13.06	3.00	5.65	4.00	3.74

```
CALIB |
STANDHYD (1662) | Area (ha)= 158.10
ID= 1 DT=15.0 min | Total Imp(%)= 58.00 Dir. Conn.(%)= 30.00
```

```
Surface Area (ha)= 91.70 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1026.65 40.00
Mannings n = 0.013 0.250
```

NOTE: RAINFALL WAS TRANSFORMED TO 15.0 MIN. TIME STEP.

---- TRANSFORMED HYETOGRAPH ----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.250	4.56	1.250	93.24	2.250	10.05	3.25	5.07
0.500	5.61	1.500	101.74	2.500	8.17	3.50	4.59
0.750	8.14	1.750	22.73	2.750	6.67	3.75	4.12
1.000	13.32	2.000	14.44	3.000	5.84	4.00	3.81

```
Max.Eff.Inten.(mm/hr)= 101.74 111.68
over (min) = 15.00 30.00
Storage Coeff. (min)= 12.63 (ii) 22.87 (iii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04
*TOTALS*
PEAK FLOW (cms)= 12.06 11.28 20.444 (iii)
TIME TO PEAK (hrs)= 1.50 1.75 1.50
```

```
RUNOFF VOLUME (mm)= 76.03 42.84 52.80
TOTAL RAINFALL (mm)= 78.03 78.03 78.03
RUNOFF COEFFICIENT = 0.97 0.55 0.68
```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
CALIB |
STANDHYD (1661) | Area (ha)= 276.60
ID= 1 DT=15.0 min | Total Imp(%)= 40.00 Dir. Conn.(%)= 21.00
```

```
Surface Area (ha)= 110.64 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1357.94 40.00
Mannings n = 0.013 0.250
```

```
Max.Eff.Inten.(mm/hr)= 101.74 78.37
over (min) = 15.00 30.00
Storage Coeff. (min)= 14.94 (ii) 25.73 (iii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.07 0.04
*TOTALS*
PEAK FLOW (cms)= 14.12 18.12 26.958 (iii)
TIME TO PEAK (hrs)= 1.50 1.75 1.50
RUNOFF VOLUME (mm)= 76.03 37.85 45.87
TOTAL RAINFALL (mm)= 78.03 78.03 78.03
RUNOFF COEFFICIENT = 0.97 0.49 0.59
```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
RESERVOIR (5166)
IN= 2--> OUT= 1
DT= 15.0 min
```

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	6.0870	6.1196
0.3730	3.2753	7.7230	7.0042
1.3370	3.4678	9.3080	7.8976
2.9800	4.4433	9.7080	7.8976
4.4520	5.2716	0.0000	0.0000

```
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (1661) 276.600 26.958 1.50 45.87
OUTFLOW: ID= 1 (5166) 276.600 8.367 2.50 45.86
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PEAK FLOW REDUCTION [Qout/qin](%)= 31.04
TIME SHIFT OF PEAK FLOW (min)= 60.00
MAXIMUM STORAGE USED (ha.m.)= 7.4077
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ADD HYD (8134)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID= 1 (1662): 158.10 20.444 1.50 52.80
+ ID= 2 (5166): 276.60 8.367 2.50 45.86
=====
ID = 3 (8134): 434.70 20.753 1.75 48.38
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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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CALIB |
STANDHYD (1642) | Area (ha)= 132.50
ID= 1 DT=15.0 min | Total Imp(%)= 20.00 Dir. Conn.(%)= 15.00
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Surface Area (ha)= 26.50 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 939.86 40.00
Mannings n = 0.013 0.250
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Max.Eff.Inten.(mm/hr)= 101.74 54.14
over (min) = 15.00 30.00
Storage Coeff. (min)= 11.98 (ii) 25.65 (iii)
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Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

*****TOTALS*****
 PEAK FLOW (cms)= 5.11 8.03 10.650 (iii)
 TIME TO PEAK (hrs)= 1.50 1.75 1.50
 RUNOFF VOLUME (mm)= 76.03 32.33 38.88
 TOTAL RAINFALL (mm)= 78.03 78.03 78.03
 RUNOFF COEFFICIENT = 0.97 0.41 0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (1641) Area (ha)= 101.70
 ID= 1 DT=15.0 min Total Imp(%)= 70.00 Dir. Conn.(%)= 52.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	71.19	30.51
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	823.41	40.00
Mannings n	0.013	0.250

Max.Eff.Inten.(mm/hr)= 101.74 102.87
 over (min) 15.00 30.00
 Storage Coeff. (min)= 11.07 (ii) 21.64 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.05

*****TOTALS*****
 PEAK FLOW (cms)= 13.83 4.87 17.437 (iii)
 TIME TO PEAK (hrs)= 1.50 1.75 1.50
 RUNOFF VOLUME (mm)= 76.03 41.06 59.24
 TOTAL RAINFALL (mm)= 78.03 78.03 78.03
 RUNOFF COEFFICIENT = 0.97 0.53 0.76

- ***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5164)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min	0.0000	0.0000	7.6100	4.1638
	0.3440	1.7056	8.2090	4.5053
	3.1340	1.9730	8.8070	4.8468
	4.1310	3.9607	9.2070	4.8568
	5.1070	3.7142	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (1641)	101.700	17.437	1.50	59.24
OUTFLOW: ID= 1 (5164)	101.700	4.621	2.25	59.23

PEAK FLOW REDUCTION [Qout/Qin](%)= 26.50
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 3.4059

ADD HYD (8132)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID= 1 (1642):	132.50	10.650	1.50	38.88
+ ID= 2 (5164):	101.70	4.621	2.25	59.23
ID = 3 (8132):	234.20	14.386	1.75	47.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0162) Area (ha)= 190.14 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.79

Unit Hyd Qpeak (cms)= 4.104

PEAK FLOW (cms)= 3.102 (i)
 TIME TO PEAK (hrs)= 2.500
 RUNOFF VOLUME (mm)= 24.344
 TOTAL RAINFALL (mm)= 78.027
 RUNOFF COEFFICIENT = 0.312

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0160) Area (ha)= 318.82 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.45

Unit Hyd Qpeak (cms)= 3.754

PEAK FLOW (cms)= 3.011 (i)
 TIME TO PEAK (hrs)= 3.750
 RUNOFF VOLUME (mm)= 22.520
 TOTAL RAINFALL (mm)= 78.027
 RUNOFF COEFFICIENT = 0.289

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0152) Area (ha)= 124.37 Curve Number (CN)= 59.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.89

Unit Hyd Qpeak (cms)= 2.372

PEAK FLOW (cms)= 1.596 (i)
 TIME TO PEAK (hrs)= 2.750
 RUNOFF VOLUME (mm)= 21.114
 TOTAL RAINFALL (mm)= 78.027
 RUNOFF COEFFICIENT = 0.271

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0148) Area (ha)= 417.89 Curve Number (CN)= 26.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.18

Unit Hyd Qpeak (cms)= 6.067

PEAK FLOW (cms)= 1.360 (i)
 TIME TO PEAK (hrs)= 3.500
 RUNOFF VOLUME (mm)= 6.646
 TOTAL RAINFALL (mm)= 78.027
 RUNOFF COEFFICIENT = 0.085

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0150) Area (ha)= 105.88 Curve Number (CN)= 17.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.58

Unit Hyd Qpeak (cms)= 3.138

PEAK FLOW (cms)= 0.348 (i)
 TIME TO PEAK (hrs)= 2.250
 RUNOFF VOLUME (mm)= 3.968
 TOTAL RAINFALL (mm)= 78.027
 RUNOFF COEFFICIENT = 0.051

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8116)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID= 1 (0148):	417.89	1.360	3.50	6.65
+ ID= 2 (0150):	105.88	0.348	2.25	3.97
ID = 3 (8116):	523.77	1.659	3.00	6.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6152)
 IN= 2--> OUT= 1 Routing time step (min)= 15.00

<----- DATA FOR SECTION (1521.0) ----->

Distance	Elevation	Manning
0.00	290.00	0.0500
15.34	286.87	0.0500
46.03	284.84	0.0500
88.22	283.07	0.0500
126.57	282.11	0.0500
153.42	280.34	0.0500
157.26	280.07	0.0500
161.09	279.72	0.0500
167.76	278.94	0.0500 / 0.0300
168.01	278.60	0.0300
168.76	278.60	0.0300
169.51	278.60	0.0300
169.76	278.90	0.0300 / 0.0500
173.76	279.40	0.0500
180.27	279.69	0.0500
184.11	279.93	0.0500
218.62	281.86	0.0500
260.82	283.20	0.0500
360.54	289.25	0.0500
379.72	289.80	0.0500

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.30	278.90	1.14E+04	0.4	0.80	45.70
0.87	279.47	932E+04	4.2	0.98	37.22
1.45	280.05	353E+05	17.5	1.09	33.68
2.02	280.62	831E+05	47.8	1.26	29.01
2.59	281.19	155E+06	102.9	1.46	25.06
3.17	281.77	250E+06	189.3	1.66	22.03
3.74	282.34	375E+06	298.5	1.75	20.91
4.32	282.92	547E+06	456.5	1.83	19.98
4.89	283.49	767E+06	705.7	2.02	18.12
5.46	284.06	102E+07	1038.0	2.24	16.34
6.04	284.64	130E+07	1444.1	2.45	14.98
6.61	285.21	161E+07	1940.1	2.66	13.79
7.18	285.78	194E+07	2522.6	2.86	12.80
7.76	286.36	239E+07	3187.4	3.06	11.98
8.33	286.93	287E+07	3942.3	3.25	11.28
8.91	287.51	306E+07	4824.9	3.46	10.58
9.48	288.08	347E+07	5795.7	3.67	9.99
10.05	288.65	390E+07	6856.4	3.86	9.48
10.63	289.23	434E+07	8005.0	4.05	9.04

INFLOW : ID= 2 (8116)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
523.77	1.66	3.00	6.10	0.49	0.85	
OUTFLOW : ID= 1 (6152)	523.77	1.56	4.00	6.10	0.47	0.85

ADD HYD (8122)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0152):	124.37	1.596	2.75	21.11
+ ID2= 2 (6152):	523.77	1.558	4.00	6.10
ID = 3 (8122):	648.14	3.031	3.50	8.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	NASHVD	Area (ha)	Curve Number (CN)
(0158)	(0156)	178.59	45.0
ID= 1 DT=15.0 min	Ia (mm)	5.00	# of Linear Res. (N)= 1.50
	U.H. Tp (hrs)	0.94	
Unit Hyd Qpeak (cms)	= 2.320		
PEAK FLOW (cms)	= 2.333 (i)		
TIME TO PEAK (hrs)	= 2.750		
RUNOFF VOLUME (mm)	= 22.401		
TOTAL RAINFALL (mm)	= 78.027		
RUNOFF COEFFICIENT	= 0.287		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHVD	Area (ha)	Curve Number (CN)
(0156)	(0156)	83.49	45.0
ID= 1 DT=15.0 min	Ia (mm)	5.00	# of Linear Res. (N)= 1.50
	U.H. Tp (hrs)	0.62	
Unit Hyd Qpeak (cms)	= 2.306		
PEAK FLOW (cms)	= 0.906 (i)		
TIME TO PEAK (hrs)	= 2.250		
RUNOFF VOLUME (mm)	= 13.618		
TOTAL RAINFALL (mm)	= 78.027		

RUNOFF COEFFICIENT = 0.175

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHVD	Area (ha)	Curve Number (CN)
(0154)	(0154)	200.32	37.0
ID= 1 DT=15.0 min	Ia (mm)	5.00	# of Linear Res. (N)= 1.50
	U.H. Tp (hrs)	0.97	

Unit Hyd Qpeak (cms) = 3.537

PEAK FLOW (cms) = 1.188 (i)

TIME TO PEAK (hrs) = 3.000

RUNOFF VOLUME (mm) = 10.436

TOTAL RAINFALL (mm) = 78.027

RUNOFF COEFFICIENT = 0.134

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8118)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0154):	200.32	1.188	3.00	10.44
+ ID2= 2 (0156):	83.49	0.906	2.25	13.62
ID = 3 (8118):	283.81	2.054	2.75	11.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6158)	ROUTING TIME STEP (min)
IN= 2--> OUT= 1	= 15.00

Distance	Elevation	Manning
0.00	290.00	0.0400
19.20	288.88	0.0400
48.00	288.26	0.0400
119.99	283.68	0.0400
167.99	282.07	0.0400
196.79	281.32	0.0400
210.79	280.90	0.0400
219.79	280.38	0.0400 / 0.0300
220.04	279.65	0.0300
220.79	279.65	0.0300
221.54	279.65	0.0300
221.79	280.39	0.0300 / 0.0400
225.79	280.59	0.0400
235.19	280.88	0.0400
254.39	281.50	0.0400
273.58	282.56	0.0400
302.38	283.42	0.0400
359.98	284.03	0.0400
436.77	289.37	0.0400
475.17	290.37	0.0400

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.37	280.01	973E+03	0.5	0.86	31.78
0.73	280.38	210E+04	1.5	1.18	23.16
1.30	280.95	149E+05	8.6	0.95	28.91
1.86	281.51	579E+05	40.0	1.13	24.12
2.43	282.08	133E+06	118.5	1.46	18.75
2.99	282.64	236E+06	254.6	1.77	15.46
3.56	283.21	369E+06	450.3	2.00	13.67
4.13	283.78	542E+06	702.7	2.13	12.86
4.69	284.34	767E+06	1125.2	2.41	11.36
5.26	284.91	101E+07	1697.1	2.75	9.93
5.82	285.47	127E+07	2379.1	3.07	8.91
6.39	286.04	155E+07	3171.4	3.36	8.14
6.95	286.60	184E+07	4075.5	3.64	7.53
7.52	287.17	215E+07	5093.1	3.89	7.03
8.09	287.74	247E+07	6226.5	4.13	6.52
8.65	288.30	281E+07	7461.2	4.36	6.28
9.22	288.87	318E+07	8635.1	4.46	6.13
9.78	289.43	357E+07	10143.6	4.67	5.86
10.35	290.00	398E+07	11608.8	4.79	5.71

INFLOW : ID= 2 (8118)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
283.81	2.05	2.75	11.37	0.77	1.16	
OUTFLOW : ID= 1 (6158)	283.81	1.99	3.25	11.37	0.77	1.16

ADD HYD (8120)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0158):	178.59	2.333	2.75	22.40
+ ID2= 2 (6158):	283.61	1.990	3.25	11.37
ID = 3 (8120):	462.40	4.318	3.00	15.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8124)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8120):	462.40	4.318	3.00	15.63
+ ID2= 2 (8122):	648.14	3.031	3.50	8.98
ID = 3 (8124):	1110.54	7.308	3.25	11.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6160)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

DATA FOR SECTION (1601.0)			
Distance	Elevation	Manning	
0.00	261.02	0.0400	
29.89	254.30	0.0400	
40.75	252.58	0.0400	
62.49	251.30	0.0400	
114.11	250.94	0.0400	
130.41	248.58	0.0400	
141.28	247.50	0.0400	
154.86	246.87	0.0400	
155.20	246.70	0.0400	
160.20	246.30	0.0400 / 0.0300	Main Channel
160.30	245.30	0.0300	Main Channel
165.20	245.30	0.0300	Main Channel
165.30	246.30	0.0300 / 0.0400	Main Channel
168.45	246.96	0.0400	
195.62	249.20	0.0400	
203.77	250.82	0.0400	
225.50	256.78	0.0400	
244.52	261.14	0.0400	
255.39	261.95	0.0400	
268.97	261.92	0.0400	

TRAVEL TIME TABLE					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.50	245.80	.120E+05	2.9	1.18	68.31
1.00	246.30	.242E+05	8.5	1.70	47.47
1.92	247.22	.825E+05	31.7	1.86	43.34
2.84	248.14	.248E+06	102.5	2.00	40.39
3.76	249.06	.506E+06	247.7	2.37	34.03
4.68	249.98	.832E+06	486.8	2.83	28.49
5.60	250.90	.121E+07	814.9	3.27	24.71
6.52	251.82	.180E+07	1157.1	3.11	25.97
7.44	252.74	.253E+07	1840.2	3.52	22.92
8.36	253.66	.331E+07	2727.1	3.98	20.25
9.29	254.59	.414E+07	3782.6	4.43	18.22
10.21	255.51	.499E+07	5010.7	4.86	16.61
11.13	256.43	.589E+07	6397.4	5.26	15.33
12.05	257.35	.681E+07	7932.8	5.64	14.31
12.97	258.27	.777E+07	9621.2	5.99	13.46
13.89	259.19	.877E+07	11469.4	6.33	12.74
14.81	260.11	.980E+07	13478.4	6.66	12.12
15.73	261.03	.109E+08	15651.7	6.97	11.58
16.65	261.95	.120E+08	17492.5	7.07	11.42

hydrograph				pipe / channel			
INFLOW : ID= 2 (8124)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
1110.54	7.31	3.25	11.75	0.89	1.55		
OUTFLOW : ID= 1 (6160)	1110.54	6.67	4.00	11.75	0.83	1.48	

ADD HYD (8126)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0160):	318.82	3.011	3.75	22.52
+ ID2= 2 (6160):	1110.54	6.668	4.00	11.75
ID = 3 (8126):	1429.36	9.670	4.00	14.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8128)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0162):	190.14	3.102	2.50	24.34
+ ID2= 2 (8126):	1429.36	9.670	4.00	14.15
ID = 3 (8128):	1619.50	12.239	3.75	15.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8130)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8128):	1619.50	12.239	3.75	15.35
+ ID2= 2 (8132):	234.20	14.386	1.75	47.72
ID = 3 (8130):	1853.70	19.676	1.75	19.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6166)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

DATA FOR SECTION (1661.0)			
Distance	Elevation	Manning	
0.00	248.36	0.0500	
41.67	246.98	0.0500	
95.82	244.93	0.0500	
149.98	242.42	0.0500	
191.64	241.88	0.0500	
224.97	239.90	0.0500	
229.12	239.35	0.0500	
233.30	237.77	0.0500	
237.47	236.57	0.0500	
240.63	235.60	0.0500 / 0.0300	Main Channel
241.63	234.10	0.0300	Main Channel
247.13	234.10	0.0300	Main Channel
247.53	235.60	0.0300 / 0.0500	Main Channel
254.13	236.25	0.0500	
266.63	236.77	0.0500	
283.29	237.84	0.0500	
291.63	238.89	0.0500	
329.12	244.66	0.0500	
370.78	249.55	0.0500	
412.44	253.17	0.0500	

TRAVEL TIME TABLE					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.75	234.85	.130E+05	6.0	1.36	36.45
1.50	235.60	.276E+05	18.1	1.95	25.41
2.25	236.35	.544E+05	40.0	2.18	22.69
3.00	237.10	.117E+06	80.1	2.04	24.32
3.75	237.85	.213E+06	148.9	2.07	23.89
4.50	238.60	.334E+06	255.1	2.27	21.85
5.25	239.35	.473E+06	395.8	2.49	19.90
6.00	240.10	.631E+06	563.8	2.65	18.66
6.75	240.85	.804E+06	773.2	2.79	17.77
7.50	241.60	.106E+07	1037.9	2.92	16.97
8.26	242.36	.135E+07	1326.7	2.92	16.94
9.01	243.11	.173E+07	1758.5	3.01	16.44
9.76	243.86	.217E+07	2285.9	3.13	15.81
10.51	244.61	.265E+07	2909.5	3.26	15.17
11.26	245.36	.318E+07	3610.1	3.37	14.67
12.01	246.11	.377E+07	4417.4	3.48	14.21
12.76	246.86	.441E+07	5348.0	3.60	13.75
13.51	247.61	.512E+07	6394.0	3.71	13.34
14.26	248.36	.589E+07	7580.3	3.82	12.95

hydrograph				pipe / channel			
INFLOW : ID= 2 (8130)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
1853.70	17.69	2.75	19.44	1.55	1.96		
OUTFLOW : ID= 1 (6166)	1853.70	17.69	2.75	19.44	1.47	1.91	

ADD HYD (8136)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6166):	1853.70	17.691	2.75	19.44
+ ID2= 2 (8134):	434.70	20.753	1.75	48.38
ID = 3 (8136):	2288.40	32.929	2.00	24.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (2021)
ID= 1 DT=15.0 min

Area (ha)= 91.20
Total Imp(%)= 40.00 Dir. Conn.(%)= 24.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	36.48	54.72	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	779.74	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	101.74	73.79	
over (min)	15.00	30.00	
Storage Coeff. (min)=	10.71 (ii)	22.79 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.09	0.04	
			TOTALS
PEAK FLOW (cms)=	5.76	6.05	10.101 (iii)
TIME TO PEAK (hrs)=	1.50	1.75	1.50
RUNOFF VOLUME (mm)=	76.03	37.02	46.38
TOTAL RAINFALL (mm)=	78.03	78.03	78.03
RUNOFF COEFFICIENT =	0.97	0.47	0.59

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5202)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.0850	2.2999
0.3100	0.9668	5.0850	2.6924
1.2590	1.1373	5.9090	3.1012
2.2140	1.5358	6.3090	3.1112
3.0620	1.8918	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (2021)	91.200	10.101	1.50	46.38
OUTFLOW: ID= 1 (5202)	91.200	3.767	2.25	46.37

PEAK FLOW REDUCTION [Qout/Qin](%)= 37.29
TIME SHIFT OF PEAK FLOW (min)= 45.00
MAXIMUM STORAGE USED (ha.m.)= 2.1767

CALIB
STANDHYD (2022)
ID= 1 DT=15.0 min

Area (ha)= 377.30
Total Imp(%)= 57.00 Dir. Conn.(%)= 34.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	215.06	162.24	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	1585.98	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	101.74	94.45	
over (min)	15.00	30.00	
Storage Coeff. (min)=	16.40 (ii)	27.34 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.07	0.04	
			TOTALS
PEAK FLOW (cms)=	30.29	21.25	45.473 (iii)
TIME TO PEAK (hrs)=	1.50	1.75	1.50
RUNOFF VOLUME (mm)=	76.03	39.27	51.77
TOTAL RAINFALL (mm)=	78.03	78.03	78.03
RUNOFF COEFFICIENT =	0.97	0.50	0.66

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 71.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8110)
1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (2022):	377.30	45.473	1.50	51.77
+ ID2= 2 (5202):	91.20	3.767	2.25	46.37

ID = 3 (8110): 468.50 46.110 1.50 50.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (0102)
ID= 1 DT=15.0 min

Area (ha)= 466.00
Total Imp(%)= 23.00 Dir. Conn.(%)= 9.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	107.18	358.82	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	1762.57	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	101.74	23.39	
over (min)	15.00	45.00	
Storage Coeff. (min)=	17.47 (ii)	36.60 (ii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.07	0.03	
			TOTALS
PEAK FLOW (cms)=	9.69	12.77	16.125 (iii)
TIME TO PEAK (hrs)=	1.50	2.00	2.00
RUNOFF VOLUME (mm)=	76.03	18.31	23.50
TOTAL RAINFALL (mm)=	78.03	78.03	78.03
RUNOFF COEFFICIENT =	0.97	0.23	0.30

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 49.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0100)
ID= 1 DT=15.0 min

Area (ha)= 693.84 Curve Number (CN)= 40.0
Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.95

Unit Hyd Qpeak (cms)= 6.087

PEAK FLOW (cms)= 2.676 (i)
TIME TO PEAK (hrs)= 4.000
RUNOFF VOLUME (mm)= 11.701
TOTAL RAINFALL (mm)= 78.027
RUNOFF COEFFICIENT = 0.150

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8000)
1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0100):	693.84	2.676	4.00	11.70
+ ID2= 2 (0102):	466.00	16.125	2.00	23.50
ID = 3 (8000):	1159.84	17.756	2.00	16.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6104)
ID= 2--> OUT= 1

Routing time step (min)= 15.00

<----- DATA FOR SECTION (1041.0) ----->

Distance	Elevation	Manning	
0.00	350.16	0.0450	
50.03	328.36	0.0450	
89.34	326.33	0.0450	
117.94	324.30	0.0450	
160.82	322.75	0.0450	
178.34	319.96	0.0450	
183.34	319.81	0.0450	
184.34	319.38	0.0450 / 0.0300	Main Channel
185.09	318.78	0.0300	Main Channel
185.84	318.72	0.0300	Main Channel
186.84	319.32	0.0300 / 0.0450	Main Channel
193.84	319.70	0.0450	
198.84	320.38	0.0450	
200.13	320.81	0.0450	
218.00	322.49	0.0450	
239.44	323.49	0.0450	
250.17	323.96	0.0450	
275.18	325.20	0.0450	
310.92	326.47	0.0450	

353.81 330.00 0.0450

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.30	319.02	.624E+03	0.1	0.45	73.00
0.60	319.32	.181E+04	0.6	0.67	49.46
1.23	319.95	.128E+05	4.9	0.77	43.38
1.86	320.58	.395E+05	19.1	0.96	34.50
2.48	321.20	.757E+05	43.5	1.15	28.98
3.11	321.83	.125E+06	81.2	1.30	25.58
3.74	322.46	.187E+06	135.9	1.45	22.92
4.37	323.09	.269E+06	195.6	1.45	22.90
5.00	323.72	.387E+06	287.8	1.48	22.44
5.63	324.35	.545E+06	428.4	1.56	21.22
6.25	324.97	.735E+06	630.5	1.71	19.42
6.88	325.60	.952E+06	876.1	1.83	18.12
7.51	326.23	.120E+07	1177.0	1.95	17.03
8.14	326.86	.149E+07	1554.4	2.08	15.93
8.77	327.49	.179E+07	2009.7	2.23	14.88
9.40	328.12	.213E+07	2533.9	2.37	13.99
10.02	328.74	.249E+07	3112.6	2.49	13.31
10.65	329.37	.288E+07	3760.9	2.60	12.75
11.28	330.00	.330E+07	4494.8	2.71	12.23

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLW : ID= 2 (8000) 1159.84	17.76	2.00	16.44	1.80	0.94
OUTFLOW: ID= 1 (6104) 1159.84	13.44	2.25	16.44	1.60	0.87

 | CALIB |
 | NASHVD (0104) | Area (ha)= 527.81 Curve Number (CN)= 43.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.34

Unit Hyd Qpeak (cms)= 6.735
 PEAK FLOW (cms)= 3.038 (1)
 TIME TO PEAK (hrs)= 3.750
 RUNOFF VOLUME (mm)= 12.930
 TOTAL RAINFALL (mm)= 78.027
 RUNOFF COEFFICIENT = 0.166

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8002)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0104):	527.81	3.038	3.75	12.93
+ ID2= 2 (6104):	1159.84	13.441	2.25	16.44
=====				
ID = 3 (8002):	1687.65	15.947	2.25	15.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | CALIB |
 | NASHVD (0106) | Area (ha)= 259.52 Curve Number (CN)= 55.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.23

Unit Hyd Qpeak (cms)= 3.614
 PEAK FLOW (cms)= 2.330 (1)
 TIME TO PEAK (hrs)= 3.500
 RUNOFF VOLUME (mm)= 18.846
 TOTAL RAINFALL (mm)= 78.027
 RUNOFF COEFFICIENT = 0.242

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8004)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0106):	259.52	2.330	3.50	18.85
+ ID2= 2 (8002):	1687.65	15.947	2.25	15.34
=====				
ID = 3 (8004):	1947.17	18.002	2.50	15.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ROUTE CHN (6108) |

| IN= 2---> OUT= 1 | Routing time step (min)= 15.00

DATA FOR SECTION (1081.0)		
Distance	Elevation	Manning
(m)	(m)	(min)
12.06	325.74	0.0400
30.15	324.28	0.0400
59.27	320.35	0.0400
72.37	317.60	0.0400
87.45	314.52	0.0400
93.48	313.91	0.0400
105.54	313.55	0.0400
106.17	313.40	0.0400 / 0.0300
110.62	313.04	0.0300 Main Channel
110.82	312.97	0.0300 Main Channel
111.57	313.08	0.0300 Main Channel
111.67	313.48	0.0300 / 0.0400
129.66	316.62	0.0400
150.77	318.95	0.0400
180.92	319.61	0.0400
205.04	321.23	0.0400
232.18	322.09	0.0400
268.37	322.31	0.0400
298.52	326.00	0.0400

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.43	313.40	.244E+04	0.7	0.59	58.83
1.09	314.06	.214E+05	10.4	1.01	34.24
1.76	314.73	.600E+05	40.2	1.39	24.89
2.42	315.39	.109E+06	92.1	1.75	19.80
3.08	316.05	.169E+06	166.5	2.05	16.88
3.75	316.72	.237E+06	264.4	2.31	14.97
4.41	317.38	.318E+06	386.3	2.52	13.72
5.07	318.04	.411E+06	539.3	2.72	12.71
5.74	318.71	.517E+06	725.5	2.91	11.88
6.40	319.37	.642E+06	909.2	2.94	11.77
7.06	320.03	.805E+06	1179.3	3.04	11.38
7.72	320.69	.989E+06	1515.9	3.18	10.87
8.39	321.36	.119E+07	1895.5	3.30	10.49
9.05	322.02	.143E+07	2310.2	3.36	10.30
9.71	322.68	.172E+07	2765.2	3.33	10.39
10.38	323.35	.205E+07	3477.6	3.53	9.81
11.04	324.01	.238E+07	4284.3	3.73	9.27
11.70	324.67	.274E+07	5154.0	3.91	8.85
12.37	325.34	.311E+07	6104.0	4.08	8.48

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLW : ID= 2 (8004) 1947.17	18.00	2.50	15.81	1.26	1.09
OUTFLOW: ID= 1 (6108) 1947.17	16.29	3.00	15.81	1.22	1.07

 | CALIB |
 | NASHVD (0108) | Area (ha)= 207.05 Curve Number (CN)= 27.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.03

Unit Hyd Qpeak (cms)= 3.432
 PEAK FLOW (cms)= 0.777 (1)
 TIME TO PEAK (hrs)= 3.250
 RUNOFF VOLUME (mm)= 6.951
 TOTAL RAINFALL (mm)= 78.027
 RUNOFF COEFFICIENT = 0.089

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8006)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0108):	207.05	0.777	3.25	6.95
+ ID2= 2 (6108):	1947.17	16.288	3.00	15.81
=====				
ID = 3 (8006):	2154.22	17.065	3.00	14.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | CALIB |
 | NASHVD (0110) | Area (ha)= 323.92 Curve Number (CN)= 52.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.35

Unit Hyd Qpeak (cms)= 4.087
 PEAK FLOW (cms)= 2.465 (1)

TIME TO PEAK (hrs)= 3.750
 RUNOFF VOLUME (mm)= 17.231
 TOTAL RAINFALL (mm)= 78.027
 RUNOFF COEFFICIENT = 0.221

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8008)
1 + 2 = 3
AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 1 (0110): 323.92 2.465 3.75 17.23
+ ID2= 2 (8006): 2154.22 17.065 3.00 14.96
=====
ID = 3 (8008): 2478.14 19.465 3.00 15.26
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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ROUTE CHN (6112)
IR= 2 -> OUTF= 1
Routing time step (min)'= 15.00
  
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<----- DATA FOR SECTION (1121.0) ----->
Distance Elevation Manning
0.00 320.59 0.0400
13.48 317.93 0.0400
40.45 311.52 0.0400
64.04 307.68 0.0400
74.15 307.01 0.0400
91.00 307.22 0.0400
107.86 305.21 0.0400
111.23 304.86 0.0400
113.32 304.60 0.0400 /0.0300 Main Channel
117.77 304.24 0.0300 Main Channel
117.97 304.17 0.0300 Main Channel
118.72 304.28 0.0300 Main Channel
118.82 304.68 0.0300 /0.0400 Main Channel
124.71 304.76 0.0400
171.90 306.30 0.0400
205.60 307.24 0.0400
239.31 308.24 0.0400
262.90 310.64 0.0400
289.86 314.34 0.0400
333.68 317.66 0.0400
  
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<----- TRAVEL TIME TABLE ----->
DEPTH (m) ELLEV (m) VOLUME (cu.m.) FLOW RATE (cms) VELOCITY (m/s) TRAV. TIME (min)
0.43 304.60 .372E+04 0.6 0.54 97.65
1.12 305.29 .463E+05 12.5 0.85 61.89
1.80 305.97 .149E+06 53.0 1.13 46.85
2.49 306.66 .311E+06 134.2 1.37 38.62
3.18 307.35 .554E+06 261.6 1.50 35.27
3.87 308.04 .894E+06 490.7 1.74 30.37
4.55 308.72 .129E+07 833.0 2.05 25.75
5.24 309.41 .171E+07 1269.8 2.36 22.39
5.93 310.10 .215E+07 1789.0 2.64 20.01
6.62 310.79 .261E+07 2392.7 2.90 18.21
7.30 311.47 .310E+07 3087.8 3.16 16.74
7.99 312.16 .361E+07 3873.6 3.40 15.52
8.68 312.85 .413E+07 4741.0 3.64 14.52
9.37 313.54 .467E+07 5689.3 3.86 13.69
10.05 314.22 .523E+07 6718.4 4.07 12.97
10.74 314.91 .581E+07 7770.7 4.24 12.46
11.43 315.60 .641E+07 8897.5 4.40 12.01
12.12 316.29 .704E+07 10113.2 4.55 11.61
12.80 316.97 .770E+07 11420.1 4.70 11.24
  
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<----- hydrograph -----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8008) 2478.14 19.46 3.00 15.26 1.24 0.89
OUTFLOW: ID= 1 (6112) 2478.14 16.60 4.25 15.26 1.19 0.87
  
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CALIB
NASHYD (0112) | Area (ha)= 414.89 Curve Number (CN)= 30.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.N. Tp(hrs)= 1.91

Unit Hyd Qpeak (cms)= 3.699

PEAK FLOW (cms)= 1.107 (i)
TIME TO PEAK (hrs)= 4.000
RUNOFF VOLUME (mm)= 7.980
TOTAL RAINFALL (mm)= 78.027
RUNOFF COEFFICIENT = 0.102

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
  
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ADD HYD (8010)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0112): 414.89 1.107 4.00 7.98
+ ID2= 2 (6112): 2478.14 16.596 4.25 15.26
=====
ID = 3 (8010): 2893.03 17.700 4.25 14.21
  
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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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ROUTE CHN (6114)
IR= 2 -> OUTF= 1
Routing time step (min)'= 15.00
  
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<----- DATA FOR SECTION (1141.0) ----->
Distance Elevation Manning
0.00 312.27 0.0500
14.07 309.70 0.0500
28.14 306.51 0.0500
35.17 305.81 0.0500
42.20 304.95 0.0500
52.75 304.24 0.0500
77.37 303.61 0.0500
98.47 301.33 0.0500
131.76 300.40 0.0500 /0.0300 Main Channel
136.21 300.04 0.0300 Main Channel
136.41 299.97 0.0300 Main Channel
137.16 300.08 0.0300 Main Channel
137.26 300.48 0.0300 /0.0500 Main Channel
140.68 300.53 0.0500
179.36 302.20 0.0500
214.53 303.82 0.0500
242.66 305.27 0.0500
288.38 307.66 0.0500
316.52 311.76 0.0500
348.17 313.48 0.0500
  
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<----- TRAVEL TIME TABLE ----->
DEPTH (m) ELLEV (m) VOLUME (cu.m.) FLOW RATE (cms) VELOCITY (m/s) TRAV. TIME (min)
0.43 300.40 .366E+04 0.7 0.59 88.31
1.05 301.02 .505E+05 12.1 0.75 69.49
1.68 301.65 .165E+06 52.7 0.99 52.28
2.30 302.27 .324E+06 130.3 1.26 41.40
2.93 302.90 .520E+06 247.1 1.48 35.08
3.55 303.52 .754E+06 406.8 1.68 30.91
4.18 304.15 .104E+07 586.1 1.76 29.55
4.80 304.77 .138E+07 856.0 1.93 26.96
5.43 305.40 .177E+07 1201.1 2.12 24.56
6.05 306.02 .219E+07 1611.2 2.30 22.63
6.68 306.65 .264E+07 2090.6 2.47 21.07
7.30 307.27 .313E+07 2653.0 2.64 19.65
7.93 307.90 .364E+07 3303.9 2.83 18.36
8.55 308.52 .417E+07 4053.9 3.03 17.14
9.18 309.15 .471E+07 4871.9 3.22 16.12
9.80 309.77 .527E+07 5756.2 3.41 15.25
10.43 310.40 .584E+07 6699.8 3.58 14.52
11.05 311.02 .642E+07 7710.5 3.74 13.89
11.68 311.65 .702E+07 8788.5 3.90 13.32
  
```

```

<----- hydrograph -----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8010) 2893.03 17.70 4.25 14.21 1.14 0.77
OUTFLOW: ID= 1 (6114) 2893.03 15.47 5.25 14.21 1.11 0.76
  
```

```

CALIB
NASHYD (0114) | Area (ha)= 287.65 Curve Number (CN)= 49.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.N. Tp(hrs)= 1.12
  
```

```

Unit Hyd Qpeak (cms)= 4.389

PEAK FLOW (cms)= 2.300 (i)
TIME TO PEAK (hrs)= 3.250
RUNOFF VOLUME (mm)= 15.670
TOTAL RAINFALL (mm)= 78.027
RUNOFF COEFFICIENT = 0.201
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8012)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0114): 287.65 2.300 3.25 15.67
  
```

+ ID2= 2 (6114): 2893.03 15.465 5.25 14.21
 =====
 ID = 3 (8012): 3180.68 17.112 5.25 14.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB          | Area (ha)= 206.43  Curve Number (CN)= 64.0
| NASHVD (0116) | Ia (mm)= 5.00      # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.80
-----

```

Unit Hyd Qpeak (cms)= 4.394

PEAK FLOW (cms)= 3.331 (i)
 TIME TO PEAK (hrs)= 2.500
 RUNOFF VOLUME (mm)= 24.351
 TOTAL RAINFALL (mm)= 78.027
 RUNOFF COEFFICIENT = 0.312

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8014) | AREA QPEAK TPEAK R.V.
| 1 2 3 | (ha) (cms) (hrs) (mm)
-----
| ID1= 1 (0116): | 206.43 3.331 2.50 24.35
+ ID2= 2 (8012): | 3180.68 17.112 5.25 14.34
=====
| ID = 3 (8014): | 3387.11 18.875 5.00 14.95
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6118) | Routing time step (min)'= 15.00
| IN= 2--> OUT= 1 |
-----

```

<----- DATA FOR SECTION (1181.0) ----->

Distance	Elevation	Manning
0.00	307.18	0.0450
24.89	299.75	0.0450
37.34	296.39	0.0450
71.57	292.75	0.0450
96.46	291.12	0.0450
108.90	289.43	0.0450
112.02	288.86	0.0450
115.13	288.46	0.0450
117.24	288.32	0.0450 /0.0350 Main Channel
117.49	288.15	0.0350 Main Channel
118.24	288.00	0.0350 Main Channel
118.99	288.20	0.0350 Main Channel
119.24	288.36	0.0350 /0.0450 Main Channel
121.35	288.40	0.0450
149.35	289.36	0.0450
171.14	290.47	0.0450
208.47	293.14	0.0450
236.48	295.17	0.0450
280.04	299.48	0.0450
308.04	307.00	0.0450

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.32	288.32	1.16E+04	0.2	0.62	81.92
1.30	289.30	6.31E+05	22.2	1.07	47.43
2.29	290.29	2.18E+06	117.5	1.64	30.95
3.27	291.27	4.48E+06	314.6	2.15	23.72
4.25	292.25	7.56E+06	625.6	2.53	20.13
5.24	293.24	1.15E+07	1093.2	2.91	17.50
6.22	294.22	1.61E+07	1731.6	3.28	15.50
7.20	295.20	2.14E+07	2547.4	3.63	14.01
8.19	296.19	2.74E+07	3584.9	4.00	12.72
9.17	297.17	3.38E+07	4859.1	4.39	11.60
10.15	298.15	4.07E+07	6341.5	4.76	10.70
11.13	299.13	4.80E+07	8024.6	5.11	9.97
12.12	300.12	5.56E+07	10006.6	5.49	9.27
13.10	301.10	6.35E+07	12244.6	5.89	8.64
14.08	302.08	7.16E+07	14681.7	6.26	8.13
15.07	303.07	7.99E+07	17316.5	6.62	7.69
16.05	304.05	8.84E+07	20148.1	6.96	7.31
17.03	305.03	9.71E+07	23176.2	7.29	6.98
18.02	306.02	1.06E+08	26400.8	7.61	6.69

<----- hydrograph ----> <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (8014)	3387.11	18.87	5.00	14.95	1.16	0.97
OUTFLOW: ID= 1 (6118)	3387.11	17.91	5.75	14.95	1.11	0.94

```

-----
| CALIB          | Area (ha)= 340.53  Curve Number (CN)= 45.0
| NASHVD (0118) | Ia (mm)= 5.00      # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.12
-----

```

Unit Hyd Qpeak (cms)= 5.200

PEAK FLOW (cms)= 2.396 (i)
 TIME TO PEAK (hrs)= 3.250
 RUNOFF VOLUME (mm)= 13.787
 TOTAL RAINFALL (mm)= 78.027
 RUNOFF COEFFICIENT = 0.177

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8016) | AREA QPEAK TPEAK R.V.
| 1 2 3 | (ha) (cms) (hrs) (mm)
-----
| ID1= 1 (0118): | 340.53 2.396 3.25 13.79
+ ID2= 2 (6118): | 3387.11 17.934 5.75 14.95
=====
| ID = 3 (8016): | 3727.64 19.506 5.50 14.85
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6122) | Routing time step (min)'= 15.00
| IN= 2--> OUT= 1 |
-----

```

<----- DATA FOR SECTION (1221.0) ----->

Distance	Elevation	Manning
0.00	277.00	0.0500
52.18	272.82	0.0500
86.97	269.38	0.0500
134.80	267.35	0.0500
160.89	263.85	0.0500
169.58	262.88	0.0500
178.28	262.45	0.0500
195.67	262.07	0.0500
213.07	260.99	0.0500
226.01	259.80	0.0500 /0.0300 Main Channel
226.11	259.30	0.0300 Main Channel
230.11	259.30	0.0300 Main Channel
230.21	259.80	0.0300 /0.0500 Main Channel
235.01	260.20	0.0500
239.15	260.57	0.0500
243.50	260.93	0.0500
300.03	264.43	0.0500
339.16	268.34	0.0500
373.95	272.36	0.0500
430.48	277.46	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.50	259.80	6.26E+04	2.5	1.23	41.35
1.41	260.71	4.62E+05	21.8	1.44	35.27
2.31	261.61	1.49E+06	79.6	1.63	31.19
3.22	262.52	3.40E+06	200.1	1.80	28.34
4.12	263.42	6.35E+06	446.8	2.15	23.71
5.03	264.33	9.95E+06	819.9	2.52	20.23
5.93	265.23	1.41E+07	1337.0	2.90	17.56
6.84	266.14	1.87E+07	1984.8	3.25	15.67
7.74	267.04	2.37E+07	2766.0	3.57	14.26
8.65	267.95	2.92E+07	3596.9	3.76	13.53
9.55	268.85	3.55E+07	4575.4	3.93	12.95
10.46	269.76	4.27E+07	5814.9	4.16	12.23
11.36	270.66	5.03E+07	7319.0	4.44	11.46
12.27	271.57	5.84E+07	9009.7	4.71	10.81
13.17	272.47	6.70E+07	10886.3	4.96	10.26
14.08	273.38	7.61E+07	12893.2	5.17	9.84
14.98	274.28	8.58E+07	15098.1	5.37	9.48
15.89	275.19	9.61E+07	17527.6	5.57	9.14
16.79	276.09	1.07E+08	20190.3	5.76	8.83

<----- hydrograph ----> <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (8016)	3727.64	19.51	5.50	14.85	1.30	1.41
OUTFLOW: ID= 1 (6122)	3727.64	18.83	6.00	14.85	1.26	1.41

```

-----
| CALIB          | Area (ha)= 17.70
| STAMHYD (1221) | Total Imp(%)= 36.00  Dir. Conn.(%)= 19.00
| ID= 1 DT=15.0 min |
-----

```

Surface Area (ha)= IMPERVIOUS 6.37 PERVIOUS (i) 11.33

Dep. Storage (mm)= 2.00 5.00
 Average Slope (ft)= 0.50 0.50
 Length (m)= 343.51 40.00
 Mannings n = 0.013 0.250
 Max.Eff.Inten.(mm/hr)= 101.74 16.42
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 6.55 (ii) 28.58 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.10 0.04

TOTALS
 PEAK FLOW (cms)= 0.93 0.35 1.160 (iii)
 TIME TO PEAK (hrs)= 1.50 1.75 1.50
 RUNOFF VOLUME (mm)= 76.03 12.28 24.39
 TOTAL RAINFALL (mm)= 78.03 78.03 78.03
 RUNOFF COEFFICIENT = 0.97 0.16 0.31

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 35.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5122)			
IN= 2--> OUT= 1			
DT= 15.0 min			
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.1840	0.4294
0.0600	0.1855	1.4870	0.5014
0.3430	0.2103	1.7310	0.5718
0.6390	0.2878	2.1310	0.5818
0.8860	0.3509	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1221)	17.700	1.160	1.50	24.39
OUTFLOW : ID= 1 (5122)	17.700	0.395	2.00	24.35

PEAK FLOW REDUCTION [Qout/Qin](%)= 34.01
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 0.2256

CALIB
 NASHYD (1222) | Area (ha)= 347.20 Curve Number (CN)= 41.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.52
 Unit Hyd Qpeak (cms)= 3.887
 PEAK FLOW (cms)= 1.695 (i)
 TIME TO PEAK (hrs)= 4.000
 RUNOFF VOLUME (mm)= 12.094
 TOTAL RAINFALL (mm)= 78.027
 RUNOFF COEFFICIENT = 0.155

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8018)				
1 + 2 = 3				
ID#	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1222):	347.20	1.695	4.00	12.09
+ ID2= 2 (5122):	17.70	0.395	2.00	24.35
ID = 3 (8018):	364.90	1.822	2.75	12.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8020)				
1 + 2 = 3				
ID#	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6122):	3727.64	18.831	6.00	14.85
+ ID2= 2 (8018):	364.90	1.822	2.75	12.69
ID = 3 (8020):	4092.54	20.141	6.00	14.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1261) | Area (ha)= 50.00

|ID= 1 DT=15.0 min | Total Imp(%)= 36.00 Dir. Conn.(%)= 26.00

IMPERVIOUS			PERVIOUS (i)		
Surface Area (ha)	(mm)	(ft)	Surface Area (ha)	(mm)	(ft)
18.00	2.00	0.50	32.00	5.00	0.50
18.00	0.50	0.50	32.00	40.00	0.250
577.35	0.013	0.250			

Max.Eff.Inten.(mm/hr)= 101.74 21.69
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 8.94 (ii) 28.66 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.04

TOTALS
 PEAK FLOW (cms)= 3.51 1.29 4.358 (iii)
 TIME TO PEAK (hrs)= 1.50 1.75 1.50
 RUNOFF VOLUME (mm)= 76.03 17.43 32.66
 TOTAL RAINFALL (mm)= 78.03 78.03 78.03
 RUNOFF COEFFICIENT = 0.97 0.22 0.42

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5126)			
IN= 2--> OUT= 1			
DT= 15.0 min			
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.4870	0.5014
0.1690	0.5421	4.2000	1.4164
0.9690	0.5941	4.8890	1.6153
1.8040	0.8130	5.2890	1.6253
2.5020	0.9912	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1261)	50.000	4.358	1.50	32.66
OUTFLOW : ID= 1 (5126)	50.000	1.756	2.00	32.65

PEAK FLOW REDUCTION [Qout/Qin](%)= 40.29
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 0.8021

CALIB
 NASHYD (1262) | Area (ha)= 83.50 Curve Number (CN)= 58.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.56

Unit Hyd Qpeak (cms)= 2.523
 PEAK FLOW (cms)= 1.461 (i)
 TIME TO PEAK (hrs)= 2.250
 RUNOFF VOLUME (mm)= 20.262
 TOTAL RAINFALL (mm)= 78.027
 RUNOFF COEFFICIENT = 0.260

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8024)				
1 + 2 = 3				
ID#	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1262):	83.50	1.461	2.25	20.26
+ ID2= 2 (5126):	50.00	1.756	2.00	32.65
ID = 3 (8024):	133.50	3.194	2.00	24.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0124) | Area (ha)= 121.43 Curve Number (CN)= 43.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.51

Unit Hyd Qpeak (cms)= 4.049
 PEAK FLOW (cms)= 1.406 (i)
 TIME TO PEAK (hrs)= 2.250
 RUNOFF VOLUME (mm)= 12.659
 TOTAL RAINFALL (mm)= 78.027
 RUNOFF COEFFICIENT = 0.162

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8026)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0124):	121.43	1.406	2.25	12.66
+ ID2= 2 (8024):	133.50	3.194	2.00	24.90
ID = 3 (8026):	254.93	4.595	2.00	19.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6128)
IN= 2--> OUT= 1

Routing time step (min)= 15.00

<----- DATA FOR SECTION (1281.0) ----->

Distance	Elevation	Manning	
0.00	270.58	0.0400	
7.71	270.36	0.0400	
15.43	269.48	0.0400	
23.14	265.88	0.0400	
42.43	261.83	0.0400	
61.71	258.31	0.0400	
75.21	256.62	0.0400	
90.64	255.35	0.0400	
92.49	255.00	0.0400 / 0.0300	Main Channel
93.49	254.00	0.0300	Main Channel
94.49	253.90	0.0300	Main Channel
94.99	253.80	0.0300	Main Channel
95.49	253.70	0.0300	Main Channel
95.99	253.80	0.0300	Main Channel
98.49	255.00	0.0300 / 0.0400	Main Channel
100.28	255.44	0.0400	
134.99	257.45	0.0400	
148.49	259.67	0.0400	
167.77	263.42	0.0400	
190.92	270.00	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.65	254.35	439E+04	2.0	1.24	36.10
1.30	255.00	131E+05	9.4	1.92	23.39
2.18	255.88	493E+05	37.0	2.30	19.54
3.06	256.76	131E+06	113.3	2.34	19.20
3.95	257.65	273E+06	273.8	2.70	16.64
4.83	258.53	453E+06	538.5	3.21	14.01
5.71	259.41	658E+06	902.9	3.70	12.14
6.59	260.29	885E+06	1368.4	4.16	10.79
7.48	261.18	114E+07	1936.3	4.59	9.79
8.36	262.06	141E+07	2610.9	4.99	9.01
9.24	262.94	171E+07	3398.7	5.37	8.36
10.12	263.82	202E+07	4309.6	5.75	7.81
11.01	264.71	235E+07	5347.5	6.13	7.33
11.89	265.59	270E+07	6501.0	6.48	6.93
12.77	266.47	307E+07	7808.6	6.86	6.55
13.65	267.35	345E+07	9250.5	7.23	6.21
14.54	268.24	384E+07	10808.3	7.59	5.92
15.42	269.12	424E+07	12482.5	7.94	5.66
16.30	270.00	466E+07	14127.4	8.18	5.49

<----- hydrograph -----> <-pipe / channel->

INFLOW : ID= 2 (8026)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW: ID= 1 (6128)	254.93	4.60	2.00	19.07	0.88	1.42
	254.93	3.90	2.75	19.07	0.82	1.37

CALIB (0128)	Area (ha)	Curve Number (CN)= 62.0
NASHVD	240.24	
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 1.00	

Unit Hyd Qpeak (cms)= 4.118

PEAK FLOW (cms)= 3.108 (i)
TIME TO PEAK (hrs)= 3.000
RUNOFF VOLUME (mm)= 23.079
TOTAL RAINFALL (mm)= 78.027
RUNOFF COEFFICIENT = 0.296

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8028)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				

	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0128):	240.24	3.108	3.00	23.08
+ ID2= 2 (6128):	254.93	3.905	2.75	19.07
ID = 3 (8028):	495.17	6.999	2.75	21.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8022)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8020):	4092.54	20.141	6.00	14.65
+ ID2= 2 (8028):	495.17	6.999	2.75	21.01
ID = 3 (8022):	4587.71	22.719	5.75	15.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (1201)	Area (ha)	Dir. Conn.(%)
STANDHYD (1201)	86.00	14.00
ID= 1 DT=15.0 min	Total Imp(%)= 36.00	Dir. Conn.(%)= 14.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	30.96	55.04
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	757.19	40.00
Mannings n	= 0.013	0.250
Max. Eff. Inten.(mm/hr)=	101.74	28.99
over (min)	15.00	30.00
Storage Coeff. (min)=	10.52 (ii)	28.08 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.04
PEAK FLOW (cms)=	3.18	2.98
TIME TO PEAK (hrs)=	1.50	1.75
RUNOFF VOLUME (mm)=	76.03	19.78
TOTAL RAINFALL (mm)=	78.03	78.03
RUNOFF COEFFICIENT =	0.97	0.25
TOTALS		5.157 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5120)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1				
DT= 15.0 min	0.0000	0.0000	5.7530	2.0863
	0.2910	0.9015	7.2230	2.4361
	1.6660	1.0219	8.4090	2.7784
	3.1030	1.3984	8.8090	2.7884
	4.3040	1.7049	0.0000	0.0000
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1201)	86.000	5.157	1.50	27.66
OUTFLOW: ID= 1 (5120)	86.000	2.281	2.25	27.65

PEAK FLOW REDUCTION [Qout/Qin](%)= 44.22
TIME SHIFT OF PEAK FLOW (min)= 45.00
MAXIMUM STORAGE USED (ha.m.)= 1.1840

CALIB (1202)	Area (ha)	Curve Number (CN)= 58.0
NASHVD (1202)	161.00	
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 0.70	

Unit Hyd Qpeak (cms)= 3.932

PEAK FLOW (cms)= 2.409 (i)
TIME TO PEAK (hrs)= 2.500
RUNOFF VOLUME (mm)= 20.394
TOTAL RAINFALL (mm)= 78.027
RUNOFF COEFFICIENT = 0.261

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8030)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1202):	161.00	2.409	2.50	20.39
+ ID2= 2 (5120):	86.00	2.281	2.25	27.65
=====				
ID = 3 (8030):	247.00	4.670	2.25	22.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8032)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8022):	4587.71	22.719	5.75	15.34
+ ID2= 2 (8030):	247.00	4.670	2.25	22.92
=====				
ID = 3 (8032):	4834.71	23.776	5.50	15.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (5130) Routing time step (min) = 15.00
IN= 2--> OUT= 1

Distance	Elevation	Manning	
0.00	257.18	0.0350	
62.45	254.70	0.0350	
109.29	252.74	0.0350	
150.92	251.55	0.0350	
192.56	248.96	0.0350	
239.39	247.16	0.0350	
291.84	246.00	0.0350	
296.84	245.00	0.0350	
298.84	245.00	0.0350 / 0.0300	Main Channel
299.24	244.60	0.0300	Main Channel
301.84	244.60	0.0300	Main Channel
303.44	244.60	0.0300	Main Channel
303.84	245.00	0.0300 / 0.0350	Main Channel
306.84	245.00	0.0350	
309.84	246.00	0.0350	
317.46	247.22	0.0350	
348.68	254.83	0.0350	
359.09	256.70	0.0350	
452.76	258.23	0.0350	
515.22	258.33	0.0350	

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.40	245.00	.870E+04	1.2	0.64	124.12
1.04	245.64	.468E+05	9.5	0.96	81.81
1.68	246.28	.109E+06	25.6	1.11	70.86
2.32	246.92	.257E+06	63.5	1.17	67.51
2.96	247.56	.498E+06	142.7	1.36	58.18
3.61	248.21	.800E+06	266.7	1.58	50.01
4.25	248.85	.116E+07	437.4	1.78	44.24
4.89	249.49	.157E+07	669.1	2.01	39.20
5.53	250.13	.203E+07	954.9	2.23	35.36
6.17	250.77	.252E+07	1294.4	2.43	32.42
6.81	251.41	.305E+07	1689.6	2.62	30.07
7.45	252.05	.363E+07	2090.7	2.73	28.93
8.09	252.69	.429E+07	2553.1	2.82	27.98
8.73	253.33	.501E+07	3138.4	2.96	26.60
9.37	253.97	.579E+07	3803.8	3.11	25.36
10.02	254.62	.662E+07	4549.3	3.25	24.25
10.66	255.26	.751E+07	5364.9	3.38	23.32
11.30	255.90	.845E+07	6265.7	3.51	22.49
11.94	256.54	.946E+07	7260.1	3.63	21.72

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLW : ID= 2 (8032) 4834.71	23.78	5.50	15.73	1.61	1.09
OUTFLOW: ID= 1 (6130) 4834.71	22.16	6.50	15.73	1.55	1.08

CALIB	STANDHYD (1301)	Area	(ha)	= 228.30	Dir. Conn.(%) = 47.00
Total Imp(%) = 65.00					
Surface Area	(ha)	148.39	79.91		
Dep. Storage	(mm)	2.00	5.00		
Average Slope	(%)	0.50	0.50		
Length	(m)	1233.69	40.00		
Manning n		0.013	0.250		

Max. Ref. Inten.(mm/hr)=	101.74	99.20	
over (min)	15.00	30.00	
Storage Coeff. (min)	14.10 (ii)	24.84 (ii)	
Unit Hyd. Tpeak (min)	15.00	30.00	
Unit Hyd. peak (cms)	0.07	0.04	
=====			
PEAK FLOW (cms)	26.52	11.57	*TOTALS*
TIME TO PEAK (hrs)	1.50	1.75	34.968 (iii)
RUNOFF VOLUME (mm)	76.03	41.78	1.50
TOTAL RAINFALL (mm)	78.03	78.03	57.87
RUNOFF COEFFICIENT	0.97	0.54	78.03
			0.74

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CN* = 74.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5130)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min	0.0000	0.0000	12.4560	7.0142
	0.8210	3.9074	14.9550	7.8554
	4.8550	4.4209	22.4790	8.9550
	7.0620	5.2214	22.8790	8.9650
	9.5970	6.0904	0.0000	0.0000
=====				
INFLOW : ID= 2 (1301)	228.300	34.968	1.50	57.87
OUTFLOW: ID= 1 (5130)	228.300	12.646	2.25	57.87
=====				
	PEAK FLOW REDUCTION [Qout/Qin](%)	= 36.16		
	TIME SHIFT OF PEAK FLOW (min)	= 45.00		
	MAXIMUM STORAGE USED (ha.m.)	= 7.1666		

CALIB	STANDHYD (1302)	Area	(ha)	= 157.80	Curve Number (CN)= 76.0
ID= 1 DT=15.0 min		Ia	(mm)	= 5.00	# of Linear Res. (N)= 1.50
		U.H. Tp(hrs)	= 1.82		

Unit Hyd Qpeak (cms) = 1.479

PEAK FLOW (cms)	1.913 (i)
TIME TO PEAK (hrs)	4.000
RUNOFF VOLUME (mm)	34.656
TOTAL RAINFALL (mm)	78.027
RUNOFF COEFFICIENT	= 0.444

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8036)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1302):	157.80	1.913	4.00	34.66
+ ID2= 2 (5130):	228.30	12.646	2.25	57.87
=====				
ID = 3 (8036):	386.10	14.102	2.25	48.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8034)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6130):	4834.71	22.162	6.50	15.73
+ ID2= 2 (8036):	386.10	14.102	2.25	48.38
=====				
ID = 3 (8034):	5220.81	24.380	6.25	18.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	STANDHYD (0188)	Area	(ha)	= 308.53	Dir. Conn.(%) = 25.00
ID= 1 DT=15.0 min		Total Imp(%)	= 41.00		
Surface Area	(ha)	126.50	182.03		
Dep. Storage	(mm)	3.00	5.00		
Average Slope	(%)	0.10	0.10		

Length (m) = 1434.18 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr) = 97.49 36.61
 over (min) = 30.00 60.00
 Storage Coeff. (min) = 25.45 (ii) 51.37 (ii)
 Unit Hyd. Tpeak (min) = 30.00 60.00
 Unit Hyd. peak (cms) = 0.04 0.02

TOTALS
 PEAK FLOW (cms) = 13.27 7.88 17.560 (iii)
 TIME TO PEAK (hrs) = 1.75 2.25 1.75
 RUNOFF VOLUME (mm) = 75.03 25.78 38.09
 TOTAL RAINFALL (mm) = 78.03 78.03 78.03
 RUNOFF COEFFICIENT = 0.96 0.33 0.49

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0186) | Area (ha) = 351.79
 ID= 1 DT=15.0 min | Total Imp(%) = 42.00 Dir. Conn.(%) = 22.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 147.75 204.04
 Dep. Storage (mm) = 3.00 5.00
 Average Slope (%) = 0.10 0.10
 Length (m) = 1531.43 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 97.49 33.92
 over (min) = 30.00 60.00
 Storage Coeff. (min) = 26.47 (ii) 53.19 (ii)
 Unit Hyd. Tpeak (min) = 30.00 60.00
 Unit Hyd. peak (cms) = 0.04 0.02

TOTALS
 PEAK FLOW (cms) = 13.11 8.06 17.473 (iii)
 TIME TO PEAK (hrs) = 1.75 2.25 1.75
 RUNOFF VOLUME (mm) = 75.03 22.84 34.32
 TOTAL RAINFALL (mm) = 78.03 78.03 78.03
 RUNOFF COEFFICIENT = 0.96 0.29 0.44

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 53.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8068) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1 = 1 (0186): 351.79 17.473 1.75 34.32
 + ID2 = 2 (0186): 308.53 17.560 1.75 38.09
 ID = 3 (8068): 660.32 35.033 1.75 36.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1821) | Area (ha) = 111.80
 ID= 1 DT=15.0 min | Total Imp(%) = 40.00 Dir. Conn.(%) = 17.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 44.72 67.08
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 863.33 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 101.74 48.56
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 11.39 (ii) 25.67 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.08 0.04

TOTALS
 PEAK FLOW (cms) = 4.94 4.54 8.008 (iii)
 TIME TO PEAK (hrs) = 1.50 1.75 1.50
 RUNOFF VOLUME (mm) = 76.03 22.70 31.77
 TOTAL RAINFALL (mm) = 78.03 78.03 78.03
 RUNOFF COEFFICIENT = 0.97 0.29 0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 52.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5182) |
 IN= 2--> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	5.7800	2.5656
	0.3790	1.0207	7.3720	3.0829
	1.6240	1.1939	8.7710	3.5956
	3.0930	1.4631	9.1710	3.6056
	4.3530	2.1032	0.0000	0.0000

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (1821) 111.800 8.008 1.50 31.77
 OUTFLOW : ID= 1 (5182) 111.800 3.284 2.25 31.76

PEAK FLOW REDUCTION [Qout/Qin](%) = 41.01
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 1.7312

CALIB
 NASHYD (1822) | Area (ha) = 44.40 Curve Number (CN) = 57.0
 ID= 1 DT=15.0 min | Ia (mm) = 5.00 # of Linear Res.(N) = 1.50
 U.H. Tp(hrs) = 0.79

Unit Hyd Qpeak (cms) = 0.966
 PEAK FLOW (cms) = 0.591 (i)
 TIME TO PEAK (hrs) = 2.500
 RUNOFF VOLUME (mm) = 19.857
 TOTAL RAINFALL (mm) = 78.027
 RUNOFF COEFFICIENT = 0.254

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8062) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1 = 1 (1822): 44.40 0.591 2.50 19.86
 + ID2 = 2 (5182): 111.80 3.284 2.25 31.76
 ID = 3 (8062): 156.20 3.861 2.25 28.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1841) | Area (ha) = 144.40
 ID= 1 DT=15.0 min | Total Imp(%) = 48.00 Dir. Conn.(%) = 19.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 69.31 75.09
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 981.16 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 101.74 66.71
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 12.29 (ii) 24.87 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.08 0.04

TOTALS
 PEAK FLOW (cms) = 7.02 7.14 11.961 (iii)
 TIME TO PEAK (hrs) = 1.50 1.75 1.50
 RUNOFF VOLUME (mm) = 76.03 27.58 36.79
 TOTAL RAINFALL (mm) = 78.03 78.03 78.03
 RUNOFF COEFFICIENT = 0.97 0.35 0.47

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 56.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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RESERVOIR (5184)
IN= 2--> OUT= 1
DT= 15.0 min

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	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	5.8820	2.3298
	0.2960	1.0006	7.4070	2.7386
	1.6530	1.1680	8.6830	3.1418
	3.0860	1.4687	9.0830	3.1518
	4.3760	1.8752	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1841)	144.400	11.961	1.50	36.79
OUTFLOW: ID= 1 (5184)	144.400	5.715	2.25	36.78


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PEAK FLOW REDUCTION [Qout/Qin](%)= 47.78
TIME SHIFT OF PEAK FLOW (min)= 45.00
MAXIMUM STORAGE USED (ha.m.)= 2.3105

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CALIB
STANDHYD (1842)
ID= 1 DT=15.0 min

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	Area (ha)	Total Imp(%)	Dir. Conn.(%)
	118.10	42.00	17.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	49.60	68.50
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	887.32	40.00
Mannings n	0.013	0.250

	Max.Eff.Inten.(mm/hr)	over (min)
	101.74	57.66
	15.00	30.00

	Storage Coeff. (min)	Unit Hyd. Tpeak (min)	Unit Hyd. peak (cms)
	11.57 (ii)	24.91 (iii)	
	15.00	30.00	
	0.08	0.04	

	PEAK FLOW (cms)	TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
	5.20	5.61	25.96	78.03	0.33
	1.50	1.75	34.47	78.03	0.44
	76.03	25.96	34.47	78.03	0.44
	78.03	78.03	78.03	78.03	0.44
	0.97	0.33	0.44	0.44	0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 56.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8060)
1 + 2 = 3

```

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1842):	118.10	9.049	1.50	34.47
+ ID2= 2 (5184):	144.40	5.715	2.25	36.78
ID = 3 (8060):	262.50	12.413	1.75	35.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8064)
1 + 2 = 3

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	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8060):	262.50	12.413	1.75	35.74
+ ID2= 2 (8062):	156.20	3.861	2.25	28.38
ID = 3 (8064):	418.70	15.102	1.75	32.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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CALIB
STANDHYD (1781)
ID= 1 DT=15.0 min

```

	Area (ha)	Total Imp(%)	Dir. Conn.(%)
	14.00	50.00	30.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	7.00	7.00
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	305.51	40.00
Mannings n	0.013	0.250

	Max.Eff.Inten.(mm/hr)	over (min)
	101.74	92.57
	15.00	30.00

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Storage Coeff. (min)= 6.10 (ii) 17.14 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.10 0.05

```

	PEAK FLOW (cms)	TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
	1.17	1.10	42.08	78.03	0.54
	1.50	1.75	42.08	78.03	0.54
	76.03	42.08	52.26	78.03	0.67
	78.03	78.03	52.26	78.03	0.67
	0.97	0.54	0.67	0.67	0.67

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 76.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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RESERVOIR (5178)
IN= 2--> OUT= 1
DT= 15.0 min

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	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	0.9360	0.3780
	0.0470	0.1841	1.1760	0.4277
	0.2710	0.2148	1.3690	0.4763
	0.5050	0.2704	1.7690	0.4863
	0.7010	0.3197	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1781)	14.000	2.012	1.50	52.26
OUTFLOW: ID= 1 (5178)	14.000	0.853	2.00	52.19


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PEAK FLOW REDUCTION [Qout/Qin](%)= 42.39
TIME SHIFT OF PEAK FLOW (min)= 30.00
MAXIMUM STORAGE USED (ha.m.)= 0.3583

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CALIB
NASHYD (1782)
ID= 1 DT=15.0 min

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	Area (ha)	Curve Number (CN)	Ia (mm)	# of Linear Res. (N)
	261.40	78.0	5.00	1.50
			U.H. Tp(hrs)= 1.15	

Unit Hyd Qpeak (cms)= 3.867

```

PEAK FLOW (cms)= 4.813 (i)
TIME TO PEAK (hrs)= 3.000
RUNOFF VOLUME (mm)= 36.559
TOTAL RAINFALL (mm)= 78.027
RUNOFF COEFFICIENT = 0.469

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- (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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ADD HYD (8046)
1 + 2 = 3

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	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1782):	261.40	4.813	3.00	36.56
+ ID2= 2 (5178):	14.00	0.853	2.00	52.19
ID = 3 (8046):	275.40	5.355	2.75	37.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB
NASHYD (0180)
ID= 1 DT=15.0 min

```

	Area (ha)	Curve Number (CN)	Ia (mm)	# of Linear Res. (N)
	226.65	78.0	5.00	1.50
			U.H. Tp(hrs)= 1.17	

Unit Hyd Qpeak (cms)= 3.299

```

PEAK FLOW (cms)= 2.826 (i)
TIME TO PEAK (hrs)= 3.250
RUNOFF VOLUME (mm)= 25.215
TOTAL RAINFALL (mm)= 78.027
RUNOFF COEFFICIENT = 0.323

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- (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8050)
1 + 2 = 3

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	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0180):	226.65	2.826	3.25	25.21
+ ID2= 2 (8046):	275.40	5.355	2.75	37.35

ID = 3 (8050): 502.05 8.126 3.00 31.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| CALIB |
| STANHYD (1741) | Area (ha)= 56.30 | Dir. Conn.(%)= 18.00
| ID= 1 DT=15.0 min | Total Imp(%)= 35.00
-----
| IMPERVIOUS | PERVIOUS (i) |
| Surface Area (ha)= 19.70 | 36.60 |
| Dep. Storage (mm)= 2.00 | 5.00 |
| Average Slope (%)= 0.50 | 0.50 |
| Length (m)= 612.64 | 40.00 |
| Mannings n = 0.013 | 0.250 |
| Max.Eff.Inten.(mm/hr)= 101.74 | 57.75 |
| over (min) 15.00 | 30.00 |
| Storage Coeff. (min)= 9.27 (ii) | 22.59 (ii) |
| Unit Hyd. Tpeak (min)= 15.00 | 30.00 |
| Unit Hyd. peak (cms)= 0.09 | 0.04 |
| PEAK FLOW (cms)= 2.73 | 3.14 | 4.919 (iii)
| TIME TO PEAK (hrs)= 1.50 | 1.75 | 1.50
| RUNOFF VOLUME (mm)= 76.03 | 29.28 | 37.70
| TOTAL RAINFALL (mm)= 78.03 | 78.03 | 78.03
| RUNOFF COEFFICIENT = 0.97 | 0.38 | 0.48
-----

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***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 64.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| RESERVOIR (5174) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
| OUTFLOW | STORAGE | OUTFLOW | STORAGE |
| (cms) | (ha.m.) | (cms) | (ha.m.) |
| 0.0000 | 0.0000 | 1.1370 | 1.3070 |
| 0.0580 | 0.8427 | 1.4280 | 1.5384 |
| 0.3290 | 0.6196 | 1.6620 | 1.7593 |
| 0.6130 | 0.8627 | 2.0620 | 1.7693 |
| 0.8510 | 1.0621 | 0.0000 | 0.0000 |
-----
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) |
| INFLOW : ID= 2 (1741) | 56.300 | 4.919 | 1.50 | 37.70 |
| OUTFLOW: ID= 1 (5174) | 56.300 | 1.160 | 2.75 | 37.66 |
-----
| PEAK FLOW REDUCTION [Qout/Qin](%)= 23.58 |
| TIME SHIFT OF PEAK FLOW (min)= 75.00 |
| MAXIMUM STORAGE USED (ha.m.)= 1.3278 |
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-----
| CALIB |
| NASHYD (1742) | Area (ha)= 82.80 | Curve Number (CN)= 71.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.69
-----
| Unit Hyd Qpeak (cms)= 2.055
| PEAK FLOW (cms)= 1.842 (i)
| TIME TO PEAK (hrs)= 2.500
| RUNOFF VOLUME (mm)= 29.633
| TOTAL RAINFALL (mm)= 78.027
| RUNOFF COEFFICIENT = 0.380
-----
| (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
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-----
| ADD HYD (8044) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
| ID1= 1 (1742): 82.80 1.842 2.50 29.63
| + ID2= 2 (5174): 56.30 1.160 2.75 37.66
| *****
| ID = 3 (8044): 139.10 2.989 2.50 32.88
-----
| NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
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-----
| CALIB |
| NASHYD (0172) | Area (ha)= 202.23 | Curve Number (CN)= 78.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
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-----
| U.H. Tp(hrs)= 0.89
| Unit Hyd Qpeak (cms)= 3.875
| PEAK FLOW (cms)= 4.557 (i)
| TIME TO PEAK (hrs)= 2.750
| RUNOFF VOLUME (mm)= 36.417
| TOTAL RAINFALL (mm)= 78.027
| RUNOFF COEFFICIENT = 0.467
-----
| (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
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-----
| CALIB |
| NASHYD (0168) | Area (ha)= 247.13 | Curve Number (CN)= 52.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.75
-----
| Unit Hyd Qpeak (cms)= 5.611
| PEAK FLOW (cms)= 2.916 (i)
| TIME TO PEAK (hrs)= 2.500
| RUNOFF VOLUME (mm)= 17.074
| TOTAL RAINFALL (mm)= 78.027
| RUNOFF COEFFICIENT = 0.219
-----
| (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
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| CALIB |
| NASHYD (0170) | Area (ha)= 166.91 | Curve Number (CN)= 65.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.80
-----
| Unit Hyd Qpeak (cms)= 3.553
| PEAK FLOW (cms)= 2.775 (i)
| TIME TO PEAK (hrs)= 2.500
| RUNOFF VOLUME (mm)= 25.060
| TOTAL RAINFALL (mm)= 78.027
| RUNOFF COEFFICIENT = 0.321
-----
| (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
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| ADD HYD (8040) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
| ID1= 1 (0168): 247.13 2.916 2.50 17.07
| + ID2= 2 (0170): 166.91 2.775 2.50 25.06
| *****
| ID = 3 (8040): 414.04 5.691 2.50 20.29
-----
| NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
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| ADD HYD (8042) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
| ID1= 1 (0172): 202.23 4.557 2.75 36.42
| + ID2= 2 (8040): 414.04 5.691 2.50 20.29
| *****
| ID = 3 (8042): 616.27 10.231 2.50 25.58
-----
| NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
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| ROUTE CHN (6174) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00
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| <----- DATA FOR SECTION (1741.0) ----->
| Distance | Elevation | Manning
| 0.00 | 296.00 | 0.0450
| 13.36 | 288.94 | 0.0450
| 26.71 | 288.64 | 0.0450
| 51.19 | 287.96 | 0.0450
| 73.45 | 287.32 | 0.0450
| 86.81 | 286.48 | 0.0450
| 102.39 | 285.08 | 0.0450
| 110.73 | 282.76 | 0.0450
| 115.73 | 281.06 | 0.0450 / 0.0350 Main Channel
| 115.74 | 280.35 | 0.0350 Main Channel
| 120.73 | 280.35 | 0.0350 Main Channel
| 120.74 | 281.10 | 0.0350 / 0.0450 Main Channel
| 125.74 | 282.64 | 0.0450
| 135.78 | 286.87 | 0.0450
| 149.13 | 289.58 | 0.0450
| 162.49 | 292.29 | 0.0450
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175.84 293.95 0.0450
 195.88 295.13 0.0450
 213.68 296.02 0.0450
 220.36 296.09 0.0450

TRAVEL TIME TABLE					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.71	281.06	136E+05	6.0	1.69	27.89
1.50	281.85	358E+05	22.5	2.42	26.43
2.28	282.63	726E+05	53.8	2.85	22.47
3.07	283.42	124E+06	104.1	3.24	19.79
3.86	284.21	189E+06	176.7	3.60	17.81
4.64	284.99	268E+06	274.9	3.94	16.26
5.43	285.78	369E+06	390.8	4.07	15.73
6.21	286.56	501E+06	551.6	4.23	15.15
7.00	287.35	674E+06	760.5	4.28	14.97
7.79	288.14	920E+06	993.3	4.15	15.44
8.57	288.92	126E+07	1361.5	4.14	15.47
9.36	289.71	167E+07	1985.0	4.56	14.04
10.15	290.50	210E+07	2739.7	5.02	12.76
10.93	291.28	254E+07	3615.0	5.47	11.70
11.72	292.07	300E+07	4608.8	5.91	10.83
12.50	292.85	347E+07	5693.3	6.30	10.16
13.29	293.64	397E+07	6892.9	6.67	9.60
14.08	294.43	450E+07	8150.4	6.97	9.19
14.86	295.21	506E+07	9525.0	7.23	8.86

<---- hydrograph ---->		<-pipe / channel-->				
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8042)	616.27	10.23	2.50	25.58	0.91	1.83
OUTFLOW : ID= 1 (6174)	616.27	9.54	3.25	25.58	0.88	1.81

ADD HYD (8048)					
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
ID1= 1 (6174):	616.27	9.539	3.25	25.58	
+ ID2= 2 (8044):	139.10	2.959	2.50	25.58	
ID = 3 (8048):	755.37	12.281	3.25	26.93	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8052)					
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
ID1= 1 (8048):	755.37	12.281	3.25	26.93	
+ ID2= 2 (8050):	502.05	8.126	3.00	31.87	
ID = 3 (8052):	1257.42	20.332	3.00	28.90	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8058)					
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
ID1= 1 (8052):	1257.42	20.332	3.00	28.90	
+ ID2= 2 (8064):	418.70	15.102	1.75	32.99	
ID = 3 (8058):	1676.12	31.011	2.50	29.92	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB			
STANDHYD (1762)	Area (ha)	Imp(%)	Dir. Conn.(%)
ID= 1 DT=15.0 min	162.00	26.00	11.00

IMPERVIOUS PERVIOUS (i)			
Surface Area (ha)	Imp(%)	Pervious (i)	
42.12	119.88		
2.00	5.00		
0.50	0.50		
1039.23	40.00		
0.013	0.250		
101.74	46.27		
15.00	30.00		
12.73 (ii)	27.29 (ii)		
15.00	30.00		
0.08	0.04		
4.52	7.48	9.647 (iii)	
1.50	1.75	1.75	

RUNOFF VOLUME (mm)= 76.03 24.73 30.37
 TOTAL RAINFALL (mm)= 78.03 78.03 78.03
 RUNOFF COEFFICIENT = 0.97 0.32 0.39

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB			
STANDHYD (1761)	Area (ha)	Imp(%)	Dir. Conn.(%)
ID= 1 DT=15.0 min	48.80	50.00	21.00

IMPERVIOUS PERVIOUS (i)			
Surface Area (ha)	Imp(%)	Pervious (i)	
24.40	24.40		
2.00	5.00		
0.50	0.50		
570.38	40.00		
0.013	0.250		
101.74	73.87		
15.00	30.00		
8.88 (ii)	20.95 (ii)		
15.00	30.00		
0.09	0.05		
2.77	2.79	4.756 (iii)	
1.50	1.75		
76.03	30.04	39.70	
78.03	78.03	78.03	
0.97	0.38	0.51	

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5176)					
IN= 2--> OUT= 1	DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
		0.0000	0.0000	3.2640	1.3175
		0.1650	0.6418	4.0090	1.4908
		0.9450	0.7489	4.7720	1.6604
		1.7610	0.9426	5.1720	1.6704
		2.4420	1.1144	0.0000	0.0000
INFLOW : ID= 2 (1761)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
OUTFLOW : ID= 1 (5176)	48.800	4.756	1.50	39.70	
	48.800	1.937	2.25	39.68	

PEAK FLOW REDUCTION [Qout/Qin](%)= 40.73
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 0.9979

ADD HYD (8056)					
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
ID1= 1 (1762):	162.00	9.647	1.75	30.37	
+ ID2= 2 (5176):	48.80	1.937	2.25	39.68	
ID = 3 (8056):	210.80	10.922	1.75	32.53	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8066)					
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
ID1= 1 (8056):	210.80	10.922	1.75	32.53	
+ ID2= 2 (8058):	1676.12	31.011	2.50	29.92	
ID = 3 (8066):	1886.92	38.662	2.00	30.22	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8070)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8066):	1886.92	38.662	2.00	30.22
+ ID2= 2 (8068):	660.32	35.033	1.75	36.08
ID = 3 (8070):	2547.24	72.687	1.75	31.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6190)
IN= 2--> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (1901.0) -----				
Distance (m)	Elevation (m)	Manning		
0.00	261.34	0.0600		
19.76	261.64	0.0600		
59.28	261.29	0.0600		
79.04	261.09	0.0600		
103.74	255.99	0.0600		
153.13	253.42	0.0600		
192.65	252.78	0.0600		
212.29	249.80	0.0600		
217.29	249.50	0.0600 / 0.0350	Main Channel	
217.30	249.05	0.0350	Main Channel	
222.29	249.05	0.0350	Main Channel	
222.30	249.50	0.0350 / 0.0600	Main Channel	
227.30	249.80	0.0600		
232.17	252.31	0.0600		
276.63	253.53	0.0600		
326.03	256.97	0.0600		
355.67	257.30	0.0600		
400.12	260.08	0.0600		
469.28	261.25	0.0600		
489.04	262.53	0.0600		

----- TRAVEL TIME TABLE -----					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.45	249.50	436E+04	1.0	0.43	75.97
1.07	250.12	205E+05	5.4	0.51	62.96
1.70	250.75	451E+05	14.8	0.64	50.78
2.32	251.37	762E+05	29.2	0.74	43.46
2.94	251.99	114E+06	49.1	0.84	38.61
3.57	252.62	161E+06	70.2	0.85	38.18
4.19	253.24	248E+06	93.4	0.73	44.29
4.81	253.86	398E+06	154.9	0.76	42.79
5.44	254.49	576E+06	246.1	0.83	38.99
6.06	255.11	779E+06	362.7	0.90	35.79
6.68	255.73	101E+07	506.5	0.98	33.15
7.30	256.35	126E+07	687.6	1.06	30.52
7.93	256.98	153E+07	902.8	1.15	28.18
8.55	257.60	183E+07	1114.3	1.18	27.39
9.17	258.22	216E+07	1407.8	1.26	25.55
9.80	258.85	250E+07	1735.4	1.35	24.03
10.42	259.47	286E+07	2097.5	1.42	22.73
11.04	260.09	324E+07	2492.3	1.49	21.63
11.67	260.72	364E+07	2837.8	1.51	21.39

----- hydrograph -----		<-pipe / channel-->	
AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8070)	2547.24	72.69	1.75 31.74
OUTFLOW : ID= 1 (6190)	2547.24	55.12	2.50 31.74

CALIB	Area (ha)	Imp(%)	Dir. Conn.(%)
STANDHYD (0190)	287.99	51.00	27.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)	146.87	141.12	
Dep. Storage (mm)	3.00	5.00	
Average Slope (%)	0.10	0.10	
Length (m)	1385.62	40.00	
Mannings n	0.013	0.250	
Max. Eff. Inten. (mm/hr)	97.49	68.02	
over (min)	30.00	60.00	
Storage Coeff. (min)	24.93 (ii)	45.16 (ii)	
Unit Hyd. Tpeak (min)	30.00	60.00	
Unit Hyd. Tpeak (cms)	0.04	0.02	
PEAK FLOW (cms)	13.48	11.82	*TOTALS*
TIME TO PEAK (hrs)	1.75	2.25	20.245 (iii)
RUNOFF VOLUME (mm)	75.03	38.44	
TOTAL RAINFALL (mm)	78.03	78.03	
RUNOFF COEFFICIENT	0.96	0.50	0.62

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 71.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8072)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0190):	287.99	20.245	1.75	48.46
+ ID2= 2 (6190):	2547.24	55.122	2.50	31.74
ID = 3 (8072):	2835.23	72.898	2.25	33.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6192)
IN= 2--> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (1921.0) -----				
Distance (m)	Elevation (m)	Manning		
0.00	264.40	0.0500		
3.64	263.94	0.0500		
14.54	261.05	0.0500		
29.08	256.78	0.0500		
36.36	254.96	0.0500		
50.90	252.20	0.0500		
65.44	249.78	0.0500		
76.35	246.86	0.0500		
77.24	245.90	0.0500		
82.24	245.45	0.0500 / 0.0350	Main Channel	
82.25	244.90	0.0350	Main Channel	
87.25	244.90	0.0350	Main Channel	
87.26	245.45	0.0350 / 0.0500	Main Channel	
94.53	246.89	0.0500		
170.87	247.16	0.0500		
218.13	248.59	0.0500		
265.40	249.22	0.0500		
301.75	250.78	0.0500		
338.11	253.47	0.0500		
359.92	264.00	0.0500		

----- TRAVEL TIME TABLE -----					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.55	245.45	345E+04	2.0	0.71	29.20
1.53	246.43	174E+05	14.6	1.05	19.92
2.50	247.40	767E+05	52.8	0.86	24.20
3.48	248.38	224E+06	199.8	1.11	18.71
4.46	249.36	429E+06	441.2	1.29	16.19
5.43	250.33	691E+06	874.3	1.58	13.18
6.41	251.31	987E+06	1468.3	1.86	11.20
7.38	252.28	131E+07	2224.8	2.13	9.79
8.36	253.26	165E+07	3132.4	2.37	8.78
9.34	254.24	201E+07	4256.4	2.65	7.87
10.31	255.21	238E+07	5551.0	2.92	7.15
11.29	256.19	276E+07	6996.8	3.17	6.57
12.27	257.17	314E+07	8585.9	3.42	6.10
13.24	258.14	353E+07	10315.9	3.65	5.71
14.22	259.12	393E+07	12180.7	3.87	5.38
15.19	260.09	434E+07	14178.2	4.09	5.10
16.17	261.07	475E+07	16306.5	4.29	4.85
17.15	262.05	517E+07	18558.6	4.49	4.64
18.12	263.02	559E+07	20939.7	4.68	4.45

----- hydrograph -----		<-pipe / channel-->	
AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8072)	2835.23	72.90	2.25 33.44
OUTFLOW : ID= 1 (6192)	2835.23	64.50	2.75 33.44

CALIB	Area (ha)	Imp(%)	Dir. Conn.(%)
STANDHYD (1921)	30.70	48.00	24.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)	14.74	15.96	
Dep. Storage (mm)	2.00	5.00	
Average Slope (%)	0.50	0.50	
Length (m)	452.40	40.00	
Mannings n	0.013	0.250	
Max. Eff. Inten. (mm/hr)	101.74	56.62	
over (min)	15.00	30.00	
Storage Coeff. (min)	7.73 (ii)	21.16 (ii)	
Unit Hyd. Tpeak (min)	15.00	30.00	

Unit Hyd. peak (cms)= 0.10 0.05 *TOTALS*
 PEAK FLOW (cms)= 2.02 1.38 2.986 (iii)
 TIME TO PEAK (hrs)= 1.50 1.75 1.50
 RUNOFF VOLUME (mm)= 76.03 25.00 37.25
 TOTAL RAINFALL (mm)= 78.03 78.03 78.03
 RUNOFF COEFFICIENT = 0.97 0.32 0.48

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 54.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5192)				
ID= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	2.0560	0.7051
	0.1040	0.3429	2.5820	0.7960
	0.5960	0.4035	3.2900	0.8880
	1.1090	0.5056	3.6900	0.8980
	1.5380	0.5974	0.0000	0.0000
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (1921)	30.700	2.986	1.50	37.25
OUTFLOW: ID= 1 (5192)	30.700	1.328	2.00	37.22
PEAK FLOW REDUCTION [Qout/Qin](%)= 44.49				
TIME SHIFT OF PEAK FLOW (min)= 30.00				
MAXIMUM STORAGE USED (ha.m.)= 0.5530				

CALIB				
STANDHYD (1922)				
ID= 1 DT=15.0 min				
	Area	(ha)=	215.90	Dir. Com. (%)=
	Total Imp (%)=	27.00		14.00
	IMPERVIOUS	PERVIOUS (i)		
Surface Area (ha)=	58.29	157.61		
Dep. Storage (mm)=	2.00	5.00		
Average Slope (%)=	0.50	0.50		
Length (m)=	1199.72	40.00		
Mannings n =	0.013	0.250		
Max. Eff. Inten. (mm/hr)=	101.74	27.25		
over (min)=	15.00	45.00		
Storage Coeff. (min)=	13.87 (ii)	31.87 (ii)		
Unit Hyd. Tpeak (min)=	15.00	45.00		
Unit Hyd. peak (cms)=	0.07	0.03		
TOTALS				
PEAK FLOW (cms)=	7.50	6.98	10.350 (iii)	
TIME TO PEAK (hrs)=	1.50	2.00	1.50	
RUNOFF VOLUME (mm)=	76.03	21.15	28.83	
TOTAL RAINFALL (mm)=	78.03	78.03	78.03	
RUNOFF COEFFICIENT =	0.97	0.27	0.37	

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 54.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8074)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1922):	215.90	10.350	1.50	28.83
+ ID2= 2 (5192):	30.70	1.328	2.00	37.22
=====				
ID = 3 (8074):	246.60	10.453	1.50	29.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8076)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6192):	2835.23	64.503	2.75	33.44
+ ID2= 2 (8074):	246.60	10.453	1.50	29.87
=====				
ID = 3 (8076):	3081.83	69.685	2.75	33.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB				
NASHYD (0196)				
ID= 1 DT=15.0 min				
	Area	(ha)=	41.47	Curve Number (CN)=
	Ia	(mm)=	5.00	# of Linear Res. (N)=
	U.H. Tp (hrs)=	0.24		1.50

Unit Hyd Qpeak (cms)= 2.950
 PEAK FLOW (cms)= 1.851 (i)
 TIME TO PEAK (hrs)= 1.750
 RUNOFF VOLUME (mm)= 26.913
 TOTAL RAINFALL (mm)= 78.027
 RUNOFF COEFFICIENT = 0.345

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB				
NASHYD (0194)				
ID= 1 DT=15.0 min				
	Area	(ha)=	256.09	Curve Number (CN)=
	Ia	(mm)=	5.00	# of Linear Res. (N)=
	U.H. Tp (hrs)=	0.68		69.0

Unit Hyd Qpeak (cms)= 6.458
 PEAK FLOW (cms)= 5.432 (i)
 TIME TO PEAK (hrs)= 2.250
 RUNOFF VOLUME (mm)= 27.979
 TOTAL RAINFALL (mm)= 78.027
 RUNOFF COEFFICIENT = 0.359

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8078)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0194):	256.09	5.432	2.25	27.98
+ ID2= 2 (0196):	41.47	1.851	1.75	26.91
=====				
ID = 3 (8078):	297.56	6.846	2.00	27.83

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6198)				
ID= 2--> OUT= 1				
Routing time step (min)'= 15.00				

----- DATA FOR SECTION (1981.0) -----				
Distance	Elevation	Manning		
0.00	267.15	0.0500		
22.99	265.94	0.0500		
51.73	261.39	0.0500		
74.72	258.75	0.0500		
94.83	257.79	0.0500		
114.95	254.36	0.0500		
126.44	254.06	0.0500		
132.19	253.68	0.0500		
135.06	253.35	0.0500		
137.94	252.93	0.0500 / 0.0300	Main Channel	
140.81	252.41	0.0300	Main Channel	
143.69	251.89	0.0300	Main Channel	
146.56	252.51	0.0300	Main Channel	
149.43	253.36	0.0300 / 0.0500	Main Channel	
158.05	255.89	0.0500		
172.42	256.25	0.0500		
204.03	259.15	0.0500		
229.90	260.36	0.0500		
252.89	260.46	0.0500		
264.50	260.45	0.0500		

----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.35	252.24	.111E+04	0.6	0.97	31.07
0.69	252.58	.442E+04	3.8	1.55	19.42
1.04	252.93	.974E+04	11.1	2.06	14.65
1.51	253.40	.203E+05	31.2	2.77	10.87
1.98	253.87	.357E+05	65.0	3.30	9.14
2.45	254.34	.596E+05	112.9	3.43	8.80
2.92	254.81	.935E+05	186.8	3.62	8.34
3.39	255.28	.131E+06	282.1	3.89	7.75
3.86	255.75	.172E+06	398.2	4.18	7.22
4.33	256.22	.221E+06	527.0	4.31	6.99
4.80	256.69	.282E+06	697.3	4.47	6.75
5.27	257.16	.350E+06	898.5	4.64	6.50
5.74	257.63	.425E+06	1131.4	4.82	6.26
6.21	258.10	.508E+06	1367.4	4.88	6.19

6.68	258.57	.603E+06	1635.6	4.91	6.14
7.15	259.04	.709E+06	1977.2	5.04	5.98
7.62	259.51	.826E+06	2360.5	5.17	5.83
8.09	259.98	.954E+06	2790.7	5.29	5.70
8.56	260.45	.110E+07	3179.6	5.25	5.74

		<---- hydrograph ---->				<-pipe / channel->	
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (8078)	297.56	6.85	2.00	27.83	0.84	1.73	
OUTFLOW: ID= 1 (6198)	297.56	6.64	2.50	27.83	0.83	1.72	

CALIB (1981)	Area (ha)= 104.70	Dir. Conn.(%)= 20.00
ID= 1 DT=15.0 min	Total Imp(%)= 44.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	46.07	58.63
Dep. Storage (mm)=	2.00	5.00
Average Slope (ft)=	0.50	0.50
Length (m)=	835.46	40.00
Manning's n	0.013	0.250
Max.Eff.Inten.(mm/hr)=	101.74	60.70
over (min)	15.00	30.00
Storage Coeff. (min)=	11.16 (ii)	24.23 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04
TOTALS		
PEAK FLOW (cms)=	5.47	5.13
TIME TO PEAK (hrs)=	1.50	1.75
RUNOFF VOLUME (mm)=	76.03	37.06
TOTAL RAINFALL (mm)=	78.03	78.03
RUNOFF COEFFICIENT	0.97	0.35
		0.48

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING:FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 58.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5198)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min	0.0000	0.0000	7.9640	2.3173
	0.2850	1.2270	9.5520	2.6039
	1.7410	1.4052	10.9290	2.8853
	3.5350	1.7675	11.3290	2.8953
	5.6580	2.0183	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (1981)	104.700	9.009	1.50	37.06
OUTFLOW: ID= 1 (5198)	104.700	4.232	2.25	37.05

PEAK FLOW REDUCTION [Qout/Qin](%)= 46.97
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 1.8798

CALIB (1982)	Area (ha)= 285.90	Curve Number (CN)= 65.0
NASHVD (1982)	Area (ha)= 5.00	# of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min	U.H. Tp(hrs)= 0.81	

Unit Hyd Qpeak (cms)=	6.048
PEAK FLOW (cms)=	4.729 (i)
TIME TO PEAK (hrs)=	2.500
RUNOFF VOLUME (mm)=	25.063
TOTAL RAINFALL (mm)=	78.027
RUNOFF COEFFICIENT	0.321

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8080)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1982):	285.90	4.729	2.50	25.06
+ ID2= 2 (5198):	104.70	4.232	2.25	37.05

ID = 3 (8080): 390.60 8.847 2.25 28.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8082)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6198):	297.56	6.636	2.50	27.83
+ ID2= 2 (8080):	390.60	8.847	2.25	28.28
ID = 3 (8082):	688.16	15.481	2.25	28.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8084)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8078):	3081.83	69.685	2.75	33.15
+ ID2= 2 (8082):	688.16	15.481	2.25	28.08
ID = 3 (8084):	3769.99	84.188	2.50	32.23

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6200)	Routing time step (min)= 15.00
IN= 2--> OUT= 1	

<----- DATA FOR SECTION (2001.0) ----->

Distance	Elevation	Manning
0.00	252.47	0.0500
10.38	252.29	0.0500
20.76	252.28	0.0500
41.52	247.66	0.0500
62.28	247.47	0.0500
82.45	247.34	0.0500
90.83	246.85	0.0500
103.81	245.51	0.0500
106.00	244.50	0.0500
108.99	244.05	0.0500 / 0.0300
109.00	243.50	0.0300
114.00	243.50	0.0300
114.01	244.05	0.0300 / 0.0500
118.99	244.50	0.0500
132.36	245.09	0.0500
150.52	245.83	0.0500
199.83	249.24	0.0500
207.62	250.16	0.0500
236.16	253.11	0.0500
256.92	253.83	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.28	243.77	365E+04	0.7	0.54	81.42
0.55	244.05	.730E+04	2.2	0.81	54.59
1.05	244.55	.197E+05	7.2	0.97	45.51
1.54	245.04	.463E+05	17.2	0.99	44.76
2.04	245.54	.895E+05	34.8	1.03	42.81
2.53	246.03	.152E+06	62.8	1.09	40.43
3.03	246.53	.232E+06	104.7	1.20	36.91
3.52	247.02	.328E+06	159.7	1.29	34.19
4.02	247.52	.450E+06	223.4	1.32	33.56
4.51	248.01	.627E+06	326.9	1.38	31.99
5.01	248.51	.820E+06	463.0	1.50	29.52
5.50	249.00	.103E+07	626.0	1.62	27.30
6.00	249.50	.124E+07	820.1	1.75	25.24
6.49	249.99	.147E+07	1044.4	1.89	23.42
6.99	250.49	.170E+07	1292.6	2.01	21.95
7.48	250.98	.195E+07	1565.6	2.13	20.71
7.98	251.48	.220E+07	1864.5	2.25	19.65
8.47	251.97	.246E+07	2189.4	2.36	18.73
8.97	252.47	.274E+07	2427.5	2.35	18.80

		<---- hydrograph ---->				<-pipe / channel->	
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (8084)	3769.99	84.19	2.50	32.23	2.78	1.14	
OUTFLOW: ID= 1 (6200)	3769.99	73.41	3.25	32.23	2.66	1.12	

CALIB (2001)	Area (ha)= 6.00	Dir. Conn.(%)= 23.00
STANDHYD (2001)	Total Imp(%)= 50.00	
ID= 1 DT=15.0 min		

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 3.00 3.00
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 200.00 40.00
 Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 101.74 86.36
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 4.73 (ii) 16.08 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.11 0.05

 PEAK FLOW (cms)= 0.39 0.45 0.723 (iii)
 TIME TO PEAK (hrs)= 1.50 1.75 1.50
 RUNOFF VOLUME (mm)= 76.03 35.84 45.08
 TOTAL RAINFALL (mm)= 78.03 78.03 78.03
 RUNOFF COEFFICIENT = 0.97 0.46 0.58

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 67.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5200)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.4010	0.1620
0.0200	0.0789	0.5040	0.1833
0.1160	0.0921	0.5870	0.2041
0.2170	0.1159	0.9870	0.2141
0.3000	0.1370	0.0000	0.0000

INFLOW : ID= 2 (2001) 6.000 0.723 1.50 45.08
 OUTFLOW: ID= 1 (5200) 6.000 0.304 2.00 44.92

PEAK FLOW REDUCTION [Qout/qin](%)= 42.01
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 0.1392

CALIB (2002)
 Area (ha)= 109.30
 Total Imp(%)= 22.00 Dir. Conn.(%)= 10.00

Surface Area (ha)= 24.05 85.25
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 853.62 40.00
 Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 101.74 53.95
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 11.31 (ii) 25.00 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

 PEAK FLOW (cms)= 2.85 6.51 7.774 (iii)
 TIME TO PEAK (hrs)= 1.50 1.75 1.75
 RUNOFF VOLUME (mm)= 76.03 29.82 34.44
 TOTAL RAINFALL (mm)= 78.03 78.03 78.03
 RUNOFF COEFFICIENT = 0.97 0.38 0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING:FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 67.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8086)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 1 (2002):	109.30	7.774	1.75	34.44
+ ID2 = 2 (5200):	6.00	0.304	2.00	44.92

ID = 3 (8086):	115.30	8.020	1.75	34.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8088)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 1 (6200):	3769.99	73.407	3.25	32.23
+ ID2 = 2 (8086):	115.30	8.020	1.75	34.99

ID = 3 (8088):	3885.29	75.030	3.25	32.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8038)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 1 (8034):	5220.81	24.380	6.25	18.14
+ ID2 = 2 (8088):	3885.29	75.030	3.25	32.31

ID = 3 (8038):	9106.10	97.089	3.25	24.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (1365)
 IN= 2--> OUT= 1
 Routing time step (min)= 15.00

DATA FOR SECTION (1362.0) ----->

Distance	Elevation	Manning
0.00	252.00	0.0400
2.35	244.60	0.0400
39.94	243.14	0.0400
58.74	242.65	0.0400
91.63	242.03	0.0400
101.03	241.69	0.0400
103.38	241.66	0.0400
105.73	240.76	0.0400
105.93	240.59	0.0400
109.18	240.28	0.0400 / 0.0300
110.43	239.81	0.0300
111.68	239.81	0.0300
113.43	240.09	0.0300 / 0.0400
118.43	240.50	0.0400
138.62	241.68	0.0400
152.72	241.60	0.0400
166.82	242.68	0.0400
185.62	249.09	0.0400
197.36	251.00	0.0400
232.61	252.28	0.0400

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.28	240.09	1.18E+04	0.1	0.20	142.09
0.91	240.72	1.14E+05	2.4	0.35	79.92
1.53	241.34	3.52E+05	9.4	0.45	62.17
2.16	241.97	8.31E+05	22.8	0.46	60.69
2.79	242.60	1.72E+06	54.2	0.53	52.74
3.41	243.22	2.97E+06	111.4	0.63	44.49
4.04	243.85	4.45E+06	194.1	0.74	38.22
4.67	244.48	6.12E+06	301.1	0.83	33.88
5.29	245.10	7.93E+06	443.0	0.94	29.82
5.92	245.73	9.97E+06	611.3	1.06	26.60
6.55	246.36	1.16E+07	802.0	1.17	24.12
7.18	246.99	1.35E+07	1014.0	1.27	22.16
7.80	247.61	1.54E+07	1246.2	1.37	20.56
8.43	248.24	1.73E+07	1498.0	1.46	19.24
9.06	248.87	1.92E+07	1768.8	1.55	18.12
9.68	249.49	2.12E+07	2049.6	1.63	17.24
10.31	250.12	2.32E+07	2344.7	1.71	16.49
10.94	250.75	2.52E+07	2658.5	1.78	15.83
11.56	251.37	2.74E+07	2931.2	1.81	15.56

hydrograph ----> <- pipe / channel ->

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8038)	9106.10	97.09	3.25	24.19	3.26	0.61
OUTFLOW: ID= 1 (1365)	9106.10	84.33	4.00	24.19	3.12	0.58

CALIB (1322)
 Area (ha)= 249.10 Curve Number (CN)= 73.0
 ID= 1 DT=15.0 min
 Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.83

Unit Hyd Qpeak (cms)= 5.130
 PEAK FLOW (cms)= 5.111 (i)
 TIME TO PEAK (hrs)= 2.500

RUNOFF VOLUME (mm)= 31.509
 TOTAL RAINFALL (mm) = 78.027
 RUNOFF COEFFICIENT = 0.404

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | CALIB |
 | STANDBYD (1321) | Area (ha)= 41.70
 | ID= 1 DT=15.0 min | Total Imp(%)= 55.00 Dir. Conn.(%)= 35.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	22.94	18.77	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	527.26	40.00	
Manning's n	= 0.013	0.250	
Max.Eff.Inten.(mm/hr)=	101.74	84.00	
over (min)=	15.00	30.00	
Storage Coeff. (min)=	8.47 (ii)	19.94 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.09	0.05	
			TOTALS
PEAK FLOW (cms)=	3.97	2.51	5.811 (iii)
TIME TO PEAK (hrs)=	1.50	1.75	1.50
RUNOFF VOLUME (mm)=	76.03	37.08	50.71
TOTAL RAINFALL (mm)=	78.03	78.03	78.03
RUNOFF COEFFICIENT =	0.97	0.48	0.65

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | RESERVOIR (5132) |
 | ID= 2 --> OUTF= 1 |
DT= 15.0 min

	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	0.9630	1.4050
	0.1780	1.0280	1.4870	1.5525
	0.4300	1.0400	1.9900	1.7000
	0.8150	1.0500	2.3900	1.7100
	0.8990	1.2275	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (1321)	41.700	5.811	1.50	50.71
OUTFLOW: ID= 1 (5132)	41.700	0.985	2.75	50.68

PEAK FLOW REDUCTION [Qout/Qin](%)= 16.95
 TIME SHIFT OF PEAK FLOW (min)= 75.00
 MAXIMUM STORAGE USED (ha.m.)= 1.4157

 | ADD HYD (8090) |
1 + 2 = 3

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1322):	249.10	5.111	2.50	31.51
+ ID2= 2 (5132):	41.70	0.985	2.75	50.68

ID = 3 (8090):	290.80	6.085	2.75	34.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | CALIB |
 | NASHYD (0134) | Area (ha)= 155.95 Curve Number (CN)= 64.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.64

Unit Hyd Qpeak (cms)= 4.167

PEAK FLOW (cms)=	2.983 (i)
TIME TO PEAK (hrs)=	2.250
RUNOFF VOLUME (mm)=	24.212
TOTAL RAINFALL (mm)=	78.027
RUNOFF COEFFICIENT =	0.310

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | ADD HYD (8092) |
1 + 2 = 3

	AREA	QPEAK	TPEAK	R.V.
--	------	-------	-------	------

----- (ha) (cms) (hrs) (mm)
 ID1= 1 (0134): 155.95 2.983 2.25 24.21
 + ID2= 2 (8090): 290.80 6.085 2.75 34.26

 ID = 3 (8092): 446.75 9.045 2.50 30.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | ROUTE CHN (1366) |
 | IN= 2 --> OUF= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (1361.0) ----->			
Distance	Elevation	Manning	
0.00	262.81	0.0400	
37.33	260.54	0.0400	
79.12	256.98	0.0400	
111.70	254.74	0.0400	
172.20	252.76	0.0400	
251.32	248.82	0.0400	
255.97	248.75	0.0400	
260.43	248.76	0.0400	
265.43	248.49	0.0400	
268.68	248.18	0.0400 / 0.0300	Main Channel
269.93	247.71	0.0300	Main Channel
271.18	247.71	0.0300	Main Channel
272.93	247.99	0.0300	Main Channel
277.93	248.40	0.0300 / 0.0400	Main Channel
283.90	248.88	0.0400	
307.17	249.60	0.0400	
321.13	251.59	0.0400	
363.01	257.08	0.0400	
400.25	257.58	0.0400	
460.75	261.91	0.0400	

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.47	248.18	.861E+04	1.3	0.76	114.66
1.19	248.90	.742E+05	17.3	1.22	71.44
1.92	249.63	.278E+06	78.3	1.48	59.26
2.64	250.35	.589E+06	210.3	1.87	46.69
3.36	251.07	.974E+06	414.3	2.23	39.19
4.08	251.79	.143E+07	696.2	2.55	34.32
4.81	252.52	.197E+07	1063.0	2.83	30.86
5.53	253.24	.259E+07	1505.7	3.06	28.62
6.25	253.96	.331E+07	2051.3	3.26	26.85
6.97	254.68	.413E+07	2720.9	3.46	25.30
7.70	255.41	.504E+07	3584.5	3.73	23.43
8.42	256.13	.601E+07	4580.9	4.00	21.87
9.14	256.85	.704E+07	5707.9	4.25	20.57
9.86	257.57	.818E+07	6636.1	4.26	20.53
10.59	258.30	.945E+07	8115.0	4.50	19.42
11.31	259.02	.108E+08	9759.2	4.74	18.45
12.03	259.74	.122E+08	11569.5	4.96	17.61
12.75	260.46	.137E+08	13550.6	5.18	16.87
13.48	261.19	.153E+08	15636.4	5.37	16.29

<----- hydrograph ----->					
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH
	(ha)	(cms)	(hrs)	(mm)	(m)
INFLOW : ID= 2 (8092)	446.75	9.04	2.50	30.75	0.82
OUTFLOW: ID= 1 (1366)	446.75	6.51	4.25	30.75	0.70

 | CALIB |
 | STANDBYD (1362) | Area (ha)= 410.10
 | ID= 1 DT=15.0 min | Total Imp(%)= 21.00 Dir. Conn.(%)= 11.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	86.12	323.98	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	1653.48	40.00	
Manning's n	= 0.013	0.250	
Max.Eff.Inten.(mm/hr)=	101.74	48.96	
over (min)=	15.00	45.00	
Storage Coeff. (min)=	16.81 (ii)	31.05 (ii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.07	0.03	
			TOTALS
PEAK FLOW (cms)=	10.56	18.52	22.049 (iii)
TIME TO PEAK (hrs)=	1.50	2.00	2.00
RUNOFF VOLUME (mm)=	76.03	27.77	33.08
TOTAL RAINFALL (mm)=	78.03	78.03	78.03
RUNOFF COEFFICIENT =	0.97	0.36	0.42

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

CN* = 65.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| CALIB |
| STANHYD (1361) | Area (ha)= 102.30
| ID= 1 DT=15.0 min | Total Imp(%)= 55.00 Dir. Com.(%)= 27.00
-----
| IMPERVIOUS | PERVIOUS (i) |
| Surface Area (ha)= 56.27 | 46.03 |
| Dep. Storage (mm)= 2.00 | 5.00 |
| Average Slope (%)= 0.50 | 0.50 |
| Length (m)= 825.83 | 40.00 |
| Mannings n = 0.013 | 0.250 |
-----
| Max.Eff.Inten.(mm/hr)= 101.74 | 89.38 |
| over (min) 15.00 | 30.00 |
| Storage Coeff. (min)= 11.09 (ii) | 22.27 (ii) |
| Unit Hyd. Tpeak (min)= 15.00 | 30.00 |
| Unit Hyd. peak (cms)= 0.08 | 0.04 |
-----
| *TOTALS* |
| PEAK FLOW (cms)= 7.22 | 6.24 | 11.724 (iii) |
| TIME TO PEAK (hrs)= 1.50 | 1.75 | 1.50 |
| RUNOFF VOLUME (mm)= 76.03 | 35.27 | 46.27 |
| TOTAL RAINFALL (mm)= 78.03 | 78.03 | 78.03 |
| RUNOFF COEFFICIENT = 0.97 | 0.45 | 0.59 |
-----

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***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 65.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| RESERVOIR (5136) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
| OUTFLOW | STORAGE | OUTFLOW | STORAGE |
| (cms) | (ha.m.) | (cms) | (ha.m.) |
|-----|-----|-----|-----|
| 0.0000 | 0.0000 | 2.4000 | 3.2690 |
| 0.1600 | 1.6363 | 2.6450 | 3.8640 |
| 0.9600 | 1.7470 | 2.8900 | 4.4590 |
| 1.8400 | 1.9320 | 3.2900 | 4.4690 |
| 2.1200 | 2.6005 | 0.0000 | 0.0000 |
-----
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) |
| INFLOW: ID= 2 (1361) | 102.300 | 11.724 | 1.50 | 46.27 |
| OUTFLOW: ID= 1 (5136) | 102.300 | 2.305 | 2.75 | 46.25 |
-----
| PEAK FLOW REDUCTION [Qout/Qin](%)= 19.66 |
| TIME SHIFT OF PEAK FLOW (min)= 75.00 |
| MAXIMUM STORAGE USED (ha.m.)= 3.0445 |
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-----
| ADD HYD (8096) |
| 1 + 2 = 3 |
|-----|-----|-----|-----|
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
| ID1= 1 (1362): | 410.10 | 22.049 | 2.00 | 33.08 |
| + ID2= 2 (5136): | 102.30 | 2.305 | 2.75 | 46.25 |
|-----|-----|-----|-----|
| ID = 3 (8096): | 512.40 | 24.184 | 2.00 | 35.71 |
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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| ADD HYD (8094) |
| 1 + 2 = 3 |
|-----|-----|-----|-----|
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
| ID1= 1 (1366): | 446.75 | 6.509 | 4.25 | 30.75 |
| + ID2= 2 (8096): | 512.40 | 24.184 | 2.00 | 35.71 |
|-----|-----|-----|-----|
| ID = 3 (8094): | 959.15 | 26.413 | 2.00 | 33.40 |
-----

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| ADD HYD (8098) |
| 1 + 2 = 3 |
|-----|-----|-----|-----|
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
| ID1= 1 (1365): | 9106.10 | 84.335 | 4.00 | 24.19 |
| + ID2= 2 (8094): | 959.15 | 26.413 | 2.00 | 33.40 |
|-----|-----|-----|-----|
| ID = 3 (8098): | 10065.25 | 97.002 | 4.00 | 25.06 |
-----

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| CALIB |
| NASHYD (0142) | Area (ha)= 396.54 Curve Number (CN)= 63.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|-----|-----|-----|
| U.H. Tp(hrs)= 1.32 |
-----
| Unit Hyd Qpeak (cms)= 5.133 |
-----
| PEAK FLOW (cms)= 4.267 (i) |
| TIME TO PEAK (hrs)= 3.500 |
| RUNOFF VOLUME (mm)= 23.838 |
| TOTAL RAINFALL (mm)= 78.027 |
| RUNOFF COEFFICIENT = 0.306 |
-----
| (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY. |
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-----
| CALIB |
| NASHYD (0138) | Area (ha)= 414.79 Curve Number (CN)= 24.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|-----|-----|-----|
| U.H. Tp(hrs)= 1.39 |
-----
| Unit Hyd Qpeak (cms)= 5.087 |
-----
| PEAK FLOW (cms)= 1.083 (i) |
| TIME TO PEAK (hrs)= 3.750 |
| RUNOFF VOLUME (mm)= 6.040 |
| TOTAL RAINFALL (mm)= 78.027 |
| RUNOFF COEFFICIENT = 0.077 |
-----
| (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY. |
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| CALIB |
| STANHYD (0140) | Area (ha)= 158.58 Dir. Com.(%)= 21.00
| ID= 1 DT=15.0 min | Total Imp(%)= 24.00
-----
| IMPERVIOUS | PERVIOUS (i) |
| Surface Area (ha)= 38.06 | 120.52 |
| Dep. Storage (mm)= 2.00 | 5.00 |
| Average Slope (%)= 0.50 | 0.50 |
| Length (m)= 1028.20 | 40.00 |
| Mannings n = 0.013 | 0.250 |
-----
| Max.Eff.Inten.(mm/hr)= 101.74 | 13.79 |
| over (min) 15.00 | 45.00 |
| Storage Coeff. (min)= 12.64 (ii) | 36.27 (ii) |
| Unit Hyd. Tpeak (min)= 15.00 | 45.00 |
| Unit Hyd. peak (cms)= 0.08 | 0.03 |
-----
| *TOTALS* |
| PEAK FLOW (cms)= 8.46 | 2.57 | 9.465 (iii) |
| TIME TO PEAK (hrs)= 1.50 | 2.00 | 1.50 |
| RUNOFF VOLUME (mm)= 76.03 | 12.62 | 25.93 |
| TOTAL RAINFALL (mm)= 78.03 | 78.03 | 78.03 |
| RUNOFF COEFFICIENT = 0.97 | 0.16 | 0.33 |
-----

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***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 41.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| ADD HYD (8102) |
| 1 + 2 = 3 |
|-----|-----|-----|-----|
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
| ID1= 1 (0138): | 414.79 | 1.083 | 3.75 | 6.04 |
| + ID2= 2 (0140): | 158.58 | 9.465 | 1.50 | 25.93 |
|-----|-----|-----|-----|
| ID = 3 (8102): | 573.37 | 9.895 | 1.50 | 11.54 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| ROUTE CHN (6142) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00
-----
| <----- DATA FOR SECTION (1421.0) -----> |
| Distance | Elevation | Manning |
|-----|-----|-----|
| 0.00 | 295.50 | 0.0400 |
| 7.25 | 295.36 | 0.0400 |
| 14.50 | 295.22 | 0.0400 |
| 21.75 | 295.22 | 0.0400 |
| 29.00 | 294.23 | 0.0400 |
-----

```

52.56	286.45	0.0400	
57.06	281.44	0.0400	
77.94	278.28	0.0400	
82.81	277.20	0.0400 / 0.0300	Main Channel
83.21	276.80	0.0300	Main Channel
88.81	276.80	0.0300	Main Channel
96.41	276.80	0.0300	Main Channel
96.81	277.20	0.0300 / 0.0400	Main Channel
103.31	278.96	0.0400	
108.75	280.16	0.0400	
117.81	282.73	0.0400	
135.94	290.30	0.0400	
159.50	292.14	0.0400	
174.00	293.07	0.0400	
179.44	293.37	0.0400	

Distance	Elevation	Manning
0.00	262.00	0.0350
39.58	257.13	0.0350
123.69	257.51	0.0350
183.07	256.83	0.0350
202.86	255.73	0.0350
212.75	254.73	0.0350
222.65	252.86	0.0350
232.44	250.70	0.0350 / 0.0300
236.44	250.20	0.0300
236.84	249.80	0.0300
242.44	249.80	0.0300
250.04	249.80	0.0300
250.44	250.20	0.0300
252.33	250.57	0.0300 / 0.0350
286.97	253.52	0.0350
331.50	255.80	0.0350
380.98	255.55	0.0350
445.30	258.40	0.0350
455.19	259.82	0.0350
489.83	262.22	0.0350

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.40	277.20	238E+05	6.9	1.27	57.28
1.25	278.05	889E+05	50.9	2.50	29.13
2.10	278.90	179E+06	133.4	3.26	22.36
2.95	279.75	293E+06	256.7	3.83	19.03
3.80	280.60	432E+06	426.0	4.31	16.90
4.66	281.46	593E+06	644.3	4.75	15.35
5.51	282.31	776E+06	915.3	5.16	14.12
6.36	283.16	978E+06	1242.2	5.55	13.12
7.21	284.01	120E+07	1626.2	5.94	12.27
8.06	284.86	143E+07	2065.6	6.30	11.57
8.91	285.71	169E+07	2562.4	6.64	10.97
9.76	286.56	196E+07	3118.0	6.97	10.46
10.61	287.41	224E+07	3733.3	7.28	10.02
11.46	288.26	255E+07	4411.9	7.57	9.62
12.31	289.11	287E+07	5155.6	7.86	9.27
13.17	289.97	321E+07	5966.6	8.14	8.96
14.02	290.82	357E+07	6705.0	8.42	8.67
14.87	291.67	398E+07	7494.3	8.24	8.85
15.72	292.52	444E+07	8405.3	8.28	8.80

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.39	250.19	169E+05	5.4	1.04	52.00
0.77	250.57	373E+05	16.6	1.44	37.46
1.44	251.24	911E+05	60.3	2.14	25.19
2.11	251.91	169E+06	136.3	2.61	20.64
2.79	252.59	270E+06	249.7	2.99	18.05
3.46	253.26	396E+06	405.4	3.31	16.28
4.13	253.93	549E+06	603.3	3.56	15.15
4.80	254.60	737E+06	857.8	3.77	14.31
5.48	255.28	963E+06	1174.6	3.95	13.67
6.15	255.95	128E+07	1449.5	3.66	14.74
6.82	256.62	173E+07	2046.4	3.83	14.07
7.49	257.29	227E+07	2665.2	3.81	14.17
8.17	257.97	310E+07	3705.2	3.87	13.93
8.84	258.64	400E+07	5127.4	4.15	12.99
9.51	259.31	492E+07	6858.6	4.51	11.95
10.18	259.98	587E+07	8815.5	4.86	11.09
10.86	260.66	684E+07	10954.6	5.18	10.41
11.53	261.33	785E+07	13242.3	5.49	9.82
12.20	262.00	890E+07	15923.2	5.79	9.31

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (8102)	573.37	9.90	11.54	0.45	1.32	
OUTFLOW: ID= 1 (6142)	573.37	4.58	2.25	11.54	0.26	1.27

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (8106)	1105.60	11.55	2.50	18.50	0.60	1.23
OUTFLOW: ID= 1 (6146)	1105.60	10.30	3.50	18.50	0.55	1.18

----- ADD HYD (8104) -----

ID#	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1	396.54	4.267	3.50	23.84
2	573.37	4.583	2.25	11.54
3	969.91	8.342	2.50	16.57

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- CALIB -----

NASHVD (0144)	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res. (N)
ID= 1 DT=15.0 min	135.69	5.00	74.0	1.50
U.N. Tp(hrs)	0.71			

Unit Hyd Qpeak (cms) = 3.240

PEAK FLOW (cms) = 3.211 (i)

TIME TO PEAK (hrs) = 2.500

RUNOFF VOLUME (mm) = 32.314

TOTAL RAINFALL (mm) = 78.027

RUNOFF COEFFICIENT = 0.414

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- ADD HYD (8106) -----

ID#	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1	135.69	3.211	2.50	32.31
2	969.91	8.342	2.50	16.57
3	1105.60	11.552	2.50	18.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ROUTE CHN (6146) -----

IN=	OUT=	Routing time step (min)
2	1	15.00

----- DATA FOR SECTION (1461.0) -----

----- CALIB -----

STANDHYD (1462)	Area (ha)	Total Imp(%)	Dir. Com.(%)
ID= 1 DT=15.0 min	125.00	29.00	12.00

Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
36.25		88.75
Dep. Storage (mm)	2.00	5.00
Average Slope (ft)	0.50	0.50
Length (m)	912.87	40.00
Mannings n	0.013	0.250

Max.Eff.Inten.(mm/hr)	101.74	71.32
over (min)	15.00	30.00
Storage Coeff. (min)	11.77 (ii)	24.02 (iii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04

TOTALS

PEAK FLOW (cms)	3.87	9.25	11.014 (iii)
TIME TO PEAK (hrs)	1.50	1.75	1.75
RUNOFF VOLUME (mm)	76.03	36.55	41.29
TOTAL RAINFALL (mm)	78.03	78.03	78.03
RUNOFF COEFFICIENT	0.97	0.47	0.53

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 73.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB -----

STANDHYD (1461)	Area (ha)	Total Imp(%)	Dir. Com.(%)
ID= 1 DT=15.0 min	70.50	36.00	15.00

Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
25.38		45.12

Dep. Storage (mm)= 2.00 5.00
 Average Slope (s)= 0.50 0.50
 Length (m)= 685.57 40.00
 Mannings n = 0.013 0.250
 Max.Eff.Inten.(mm/hr)= 101.74 79.43
 over (min) 15.00 30.00
 Storage Coeff. (min)= 9.91 (ii) 21.64 (iii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.05
 TOTALS
 PEAK FLOW (cms)= 2.82 5.51 6.822 (iii)
 TIME TO PEAK (hrs)= 1.50 1.75 1.50
 RUNOFF VOLUME (mm)= 76.03 38.03 43.73
 TOTAL RAINFALL (mm)= 78.03 78.03 78.03
 RUNOFF COEFFICIENT = 0.97 0.49 0.56

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 73.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5146)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	8.0640	2.2344	
0.2390	0.7085	9.9830	2.6409	
2.2880	0.8391	11.7350	3.0631	
4.3270	1.4661	12.1350	3.0731	
6.1170	1.8197	0.0000	0.0000	

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (1461)	70.500	6.822	1.50	43.73
OUTFLOW : ID= 1 (5146)	70.500	3.746	2.25	43.72

PEAK FLOW REDUCTION [Qout/qin](%)= 54.91
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 1.3388

ADD HYD (8099)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1462):	125.00	11.014	1.75	41.29
+ ID2= 2 (5146):	70.50	3.746	2.25	43.72
ID = 3 (8099):	195.50	14.137	1.75	42.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8108)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6146):	1105.60	10.299	3.50	18.50
+ ID2= 2 (8099):	195.50	14.137	1.75	42.17
ID = 3 (8108):	1301.10	17.360	1.75	22.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8100)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8098):	10065.25	97.002	4.00	25.06
+ ID2= 2 (8108):	1301.10	17.360	1.75	22.06
ID = 3 (8100):	11366.35	109.369	3.75	24.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6202)	
IN= 2--> OUT= 1	
Routing time step (min)= 15.00	

<----- DATA FOR SECTION (2021.0) ----->		
Distance	Elevation	Manning
0.00	245.48	0.0400

29.68	242.80	0.0400
74.20	240.39	0.0400
155.81	237.12	0.0400
200.33	234.48	0.0400
244.85	234.67	0.0400
259.69	233.96	0.0400
274.53	233.84	0.0400
281.68	233.40	0.0400
285.78	232.40	0.0400 / 0.0300
286.78	231.40	0.0300
296.78	231.40	0.0300
299.78	232.40	0.0300 / 0.0400
311.62	233.74	0.0400
333.88	234.34	0.0400
497.11	235.32	0.0400
571.31	237.32	0.0400
675.18	239.10	0.0400
704.86	242.71	0.0400
734.54	242.62	0.0400

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.50	231.90	221E+05	4.7	0.85	78.67
1.00	232.40	482E+05	15.3	1.28	52.40
1.60	233.00	913E+05	38.3	1.68	39.77
2.20	233.60	154E+06	72.9	1.90	35.24
2.80	234.20	278E+06	124.2	1.79	37.30
3.40	234.80	599E+06	214.3	1.44	46.59
4.01	235.41	1.25E+07	445.0	1.43	46.90
4.61	236.01	2.05E+07	838.4	1.64	40.81
5.21	236.61	2.93E+07	1361.6	1.86	35.89
5.81	237.21	3.89E+07	2011.2	2.08	32.23
6.41	237.81	4.94E+07	2753.5	2.24	29.91
7.01	238.41	6.12E+07	3635.0	2.39	28.04
7.61	239.01	7.41E+07	4672.5	2.53	26.43
8.21	239.61	8.80E+07	5999.3	2.74	24.44
8.82	240.22	1.02E+08	7510.0	2.94	22.72
9.42	240.82	1.17E+08	9201.9	3.15	21.23
10.02	241.42	1.32E+08	11059.7	3.35	19.95
10.62	242.02	1.48E+08	13073.4	3.54	18.87
11.22	242.62	1.64E+08	15242.1	3.73	17.93

<----- hydrograph ----->						
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8100)	*****	103.37	2.75	24.72	2.63	1.82
OUTFLOW : ID= 1 (6202)	*****	103.81	4.50	24.72	2.56	1.83

ADD HYD (8112)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6202):	11366.35	103.807	4.50	24.72
+ ID2= 2 (8110):	468.50	46.110	1.50	50.72
ID = 3 (8112):	11834.85	108.055	4.00	25.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5555)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	*****	19.2100	
2.8500	0.5200	*****	24.7300	
8.1800	1.2600	*****	30.8600	
15.0900	2.2100	*****	37.6100	1.82
23.2700	3.3900	*****	41.2200	
32.5300	4.7800	*****	48.9000	
36.5100	5.3900	*****	66.4700	
42.7600	6.3700	*****	100.1200	
53.8500	8.1300	*****	140.4100	
91.7300	14.3200	*****	140.4200	

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8112)	*****	108.055	4.00	25.75
OUTFLOW : ID= 1 (5555)	*****	105.481	4.75	25.75

PEAK FLOW REDUCTION [Qout/qin](%)= 97.62
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 16.7091

ADD HYD (8114)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)

ID1= 1 (5555): 11834.85 105.481 4.75 25.75
 + ID2= 2 (8136): 2288.40 32.929 2.00 24.94
 =====
 ID = 3 (8114): 14123.25 126.744 4.25 25.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| STANDBY (2061) | Area (ha)= 25.00
| ID= 1 DT=15.0 min | Total Imp(%)= 93.00 Dir. Conn.(%)= 60.00
-----
| IMPERVIOUS | PERVIOUS (i) |
| Surface Area (ha)= 23.25 | 1.75 |
| Dep. Storage (mm)= 2.00 | 5.00 |
| Average Slope (%)= 0.50 | 0.50 |
| Length (m)= 408.25 | 40.00 |
| Mannings n = 0.013 | 0.250 |
-----
| Max.Eff.Inten.(mm/hr)= 101.74 | 541.36 |
| over (min)= 15.00 | 15.00 |
| Storage Coeff. (min)= 7.26 (ii) | 12.71 (ii) |
| Unit Hyd. Tpeak (min)= 15.00 | 15.00 |
| Unit Hyd. peak (cms)= 0.10 | 0.08 |
-----
| PEAK FLOW (cms)= 4.14 | 2.26 | *TOTALS*
| TIME TO PEAK (hrs)= 1.50 | 1.50 | 6.405 (iii)
| RUNOFF VOLUME (mm)= 76.03 | 64.72 | 71.50
| TOTAL RAINFALL (mm)= 78.03 | 78.03 | 78.03
| RUNOFF COEFFICIENT = 0.97 | 0.83 | 0.92
  
```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| RESERVOIR (5206) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
| OUTFLOW | STORAGE | OUTFLOW | STORAGE |
| (cms) | (ha.m.) | (cms) | (ha.m.) |
| 0.0000 | 0.0000 | 1.8990 | 1.7295 |
| 0.0850 | 0.5280 | 2.1560 | 1.9661 |
| 0.8990 | 0.8577 | 2.3740 | 2.2023 |
| 1.3460 | 1.1601 | 2.7740 | 2.2123 |
| 1.6060 | 1.4451 | 0.0000 | 0.0000 |
-----
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) |
| INFLOW : ID= 2 (2061) | 25.000 | 6.405 | 1.50 | 71.50 |
| OUTFLOW: ID= 1 (5206) | 25.000 | 1.334 | 2.00 | 71.44 |
-----
| PEAK FLOW REDUCTION [Qout/Qin](%)= 20.84 |
| TIME SHIFT OF PEAK FLOW (min)= 30.00 |
| MAXIMUM STORAGE USED (ha.m.)= 1.1535 |
  
```

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-----
| CALIB |
| STANDBY (2062) | Area (ha)= 250.50
| ID= 1 DT=15.0 min | Total Imp(%)= 62.00 Dir. Conn.(%)= 39.00
-----
| IMPERVIOUS | PERVIOUS (i) |
| Surface Area (ha)= 155.31 | 95.19 |
| Dep. Storage (mm)= 2.00 | 5.00 |
| Average Slope (%)= 0.50 | 0.50 |
| Length (m)= 1292.28 | 40.00 |
| Mannings n = 0.013 | 0.250 |
-----
| Max.Eff.Inten.(mm/hr)= 101.74 | 110.45 |
| over (min)= 15.00 | 30.00 |
| Storage Coeff. (min)= 14.50 (ii) | 24.78 (ii) |
| Unit Hyd. Tpeak (min)= 15.00 | 30.00 |
| Unit Hyd. peak (cms)= 0.07 | 0.04 |
-----
| PEAK FLOW (cms)= 23.96 | 15.46 | *TOTALS*
| TIME TO PEAK (hrs)= 1.50 | 1.75 | 35.379 (iii)
| RUNOFF VOLUME (mm)= 76.03 | 43.96 | 56.47
| TOTAL RAINFALL (mm)= 78.03 | 78.03 | 78.03
| RUNOFF COEFFICIENT = 0.97 | 0.56 | 0.72
  
```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8144) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (2062): 250.50 35.379 1.50 56.47
| + ID2= 2 (5206): 25.00 1.334 2.00 71.44
| =====
| ID = 3 (8144): 275.50 36.109 1.50 57.83
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| STANDBY (2041) | Area (ha)= 201.50
| ID= 1 DT=15.0 min | Total Imp(%)= 36.00 Dir. Conn.(%)= 20.00
-----
| IMPERVIOUS | PERVIOUS (i) |
| Surface Area (ha)= 72.54 | 128.96 |
| Dep. Storage (mm)= 2.00 | 5.00 |
| Average Slope (%)= 0.50 | 0.50 |
| Length (m)= 1159.02 | 40.00 |
| Mannings n = 0.013 | 0.250 |
-----
| Max.Eff.Inten.(mm/hr)= 101.74 | 52.36 |
| over (min)= 15.00 | 30.00 |
| Storage Coeff. (min)= 13.59 (ii) | 27.44 (ii) |
| Unit Hyd. Tpeak (min)= 15.00 | 30.00 |
| Unit Hyd. peak (cms)= 0.08 | 0.04 |
-----
| PEAK FLOW (cms)= 10.06 | 9.11 | *TOTALS*
| TIME TO PEAK (hrs)= 1.50 | 1.75 | 16.230 (iii)
| RUNOFF VOLUME (mm)= 76.03 | 26.87 | 36.70
| TOTAL RAINFALL (mm)= 78.03 | 78.03 | 78.03
| RUNOFF COEFFICIENT = 0.97 | 0.34 | 0.47
  
```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| RESERVOIR (5204) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
| OUTFLOW | STORAGE | OUTFLOW | STORAGE |
| (cms) | (ha.m.) | (cms) | (ha.m.) |
| 0.0000 | 0.0000 | 12.4660 | 4.9468 |
| 0.5050 | 1.9141 | 15.4770 | 5.3726 |
| 2.8390 | 2.2056 | 18.2610 | 6.4806 |
| 5.2410 | 3.6606 | 18.6610 | 6.4906 |
| 7.9920 | 4.3236 | 0.0000 | 0.0000 |
-----
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) |
| INFLOW : ID= 2 (2041) | 201.500 | 16.230 | 1.50 | 36.70 |
| OUTFLOW: ID= 1 (5204) | 201.500 | 5.948 | 2.50 | 36.70 |
-----
| PEAK FLOW REDUCTION [Qout/Qin](%)= 36.65 |
| TIME SHIFT OF PEAK FLOW (min)= 60.00 |
| MAXIMUM STORAGE USED (ha.m.)= 3.8617 |
  
```

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-----
| CALIB |
| STANDBY (2042) | Area (ha)= 122.90
| ID= 1 DT=15.0 min | Total Imp(%)= 60.00 Dir. Conn.(%)= 33.00
-----
| IMPERVIOUS | PERVIOUS (i) |
| Surface Area (ha)= 73.74 | 49.16 |
| Dep. Storage (mm)= 2.00 | 5.00 |
| Average Slope (%)= 0.50 | 0.50 |
| Length (m)= 905.17 | 40.00 |
| Mannings n = 0.013 | 0.250 |
-----
| Max.Eff.Inten.(mm/hr)= 101.74 | 85.46 |
| over (min)= 15.00 | 30.00 |
| Storage Coeff. (min)= 11.71 (ii) | 23.10 (ii) |
| Unit Hyd. Tpeak (min)= 15.00 | 30.00 |
| Unit Hyd. peak (cms)= 0.08 | 0.04 |
-----
| PEAK FLOW (cms)= 10.48 | 6.25 | *TOTALS*
| TIME TO PEAK (hrs)= 1.50 | 1.75 | 14.937 (iii)
| RUNOFF VOLUME (mm)= 76.03 | 32.74 | 47.03
| TOTAL RAINFALL (mm)= 78.03 | 78.03 | 78.03
| RUNOFF COEFFICIENT = 0.97 | 0.42 | 0.60
  
```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERSISTENT LOSSES:
 CN* = 61.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8140) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
-----
+ ID1= 1 (2042): 122.90 14.937 1.50 47.03
+ ID2= 2 (5204): 201.50 5.948 2.50 36.70
-----
ID = 3 (8140): 324.40 15.380 1.50 40.61
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| ROUTE CHN (6206) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min) = 15.00
-----

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<----- DATA FOR SECTION (2061.0) ----->
Distance      Elevation      Manning
0.00          253.07          0.0350
13.98        252.89          0.0350
27.95        251.99          0.0350
34.94        252.18          0.0350
41.93        252.02          0.0350
73.38        250.35          0.0350
94.34        247.09          0.0350
111.81       244.79          0.0350
115.31       244.24          0.0350
117.55       244.20          0.0350 / 0.0300 Main Channel
118.80       243.60          0.0300 Main Channel
122.30       243.60          0.0300 Main Channel
122.55       244.60          0.0300 / 0.0350 Main Channel
146.76       244.74          0.0350
164.23       247.26          0.0350
213.15       248.92          0.0350
241.10       249.51          0.0350
272.55       250.87          0.0350
300.50       252.26          0.0350
345.93       253.67          0.0350
-----

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<----- TRAVEL TIME TABLE ----->
DEPTH      ELEV      VOLUME      FLOW RATE      VELOCITY      TRAV.TIME
(m)         (m)         (cu.m.)      (cms)           (m/s)         (min)
-----
0.30 243.90 .310E+04 1.4 1.23 36.25
0.60 244.20 .676E+04 4.6 1.81 24.64
1.12 244.72 .221E+05 17.1 2.07 21.60
1.54 245.24 .754E+05 64.1 2.28 19.60
2.17 245.77 .139E+06 149.8 2.88 15.50
2.69 246.29 .214E+06 273.3 3.43 13.04
3.21 246.81 .299E+06 436.1 3.91 11.43
3.73 247.33 .395E+06 632.6 4.30 10.40
4.25 247.85 .511E+06 836.4 4.39 10.18
4.77 248.37 .653E+06 1114.9 4.58 9.76
5.30 248.90 .822E+06 1472.1 4.81 9.30
5.82 249.42 .102E+07 1872.2 4.91 9.10
6.34 249.94 .126E+07 2444.6 5.22 8.56
6.86 250.46 .151E+07 3113.1 5.52 8.09
7.38 250.98 .180E+07 3843.5 5.74 7.79
7.90 251.50 .211E+07 4717.8 6.00 7.45
8.43 252.03 .245E+07 5701.4 6.24 7.16
8.95 252.55 .284E+07 6727.5 6.36 7.03
9.47 253.07 .326E+07 7885.3 6.48 6.90
-----

```

```

<----- hydrograph -----> <-pipe / channel->
AREA      QPEAK      TPEAK      R.V.      MAX DEPTH      MAX VEL
(ha)      (cms)      (hrs)      (mm)      (m)            (m/s)
-----
INFLOW : ID= 2 (8140) 324.40 15.38 1.50 40.61 1.05 2.03
OUTFLOW: ID= 1 (6206) 324.40 12.72 2.00 40.61 0.94 1.97
-----

```

```

-----
| ADD HYD (8142) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
-----
+ ID1= 1 (6206): 324.40 12.719 2.00 40.61
+ ID2= 2 (8144): 275.50 36.109 1.50 57.83
-----
ID = 3 (8142): 599.90 43.491 1.50 48.52
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8138) |
| 1 + 2 = 3 |
-----
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
-----

```

```

-----
| (ha) | (cms) | (hrs) | (mm) |
-----
ID1= 1 (8114): 14123.25 126.744 4.25 25.62
+ ID2= 2 (8142): 599.90 43.491 1.50 48.52
-----
ID = 3 (8138): 14723.15 134.961 4.00 26.55
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6208) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min) = 15.00
-----

```

```

<----- DATA FOR SECTION (2081.0) ----->
Distance      Elevation      Manning
0.00          249.19          0.1000
73.11        244.13          0.1000
121.86       243.39          0.1000
243.71       240.01          0.1000
353.38       238.75          0.1000
450.87       235.40          0.1000
499.61       231.40          0.1000
511.80       230.96          0.1000
523.98       230.12          0.1000
534.50       227.80          0.1000 / 0.0500 Main Channel
536.50       226.70          0.0500 Main Channel
546.50       226.70          0.0500 Main Channel
548.50       227.80          0.0500 / 0.1000 Main Channel
572.72       230.25          0.1000
682.40       230.50          0.1000
804.25       233.95          0.1000
926.11       235.83          0.1000
1047.96      240.70          0.1000
1145.45      243.26          0.1000
1206.38      247.03          0.1000
-----

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH      ELEV      VOLUME      FLOW RATE      VELOCITY      TRAV.TIME
(m)         (m)         (cu.m.)      (cms)           (m/s)         (min)
-----
0.55 227.25 .111E+05 2.0 0.33 92.44
1.10 227.80 .242E+05 6.5 0.50 61.56
2.23 228.93 .700E+05 26.0 0.68 44.91
3.36 230.06 .150E+06 60.8 0.74 41.04
4.49 231.19 .446E+06 130.8 0.54 56.83
5.62 232.32 .925E+06 287.4 0.57 53.66
6.76 233.46 .152E+07 522.9 0.63 48.32
7.89 234.59 .233E+07 827.2 0.68 44.91
9.02 235.72 .312E+07 1219.5 0.72 42.61
10.15 236.85 .418E+07 1780.6 0.78 39.11
11.28 237.98 .537E+07 2479.2 0.85 36.07
12.41 239.11 .669E+07 3282.9 0.90 33.95
13.54 240.24 .824E+07 4228.6 0.94 32.47
14.67 241.37 .997E+07 5384.6 0.99 30.87
15.81 242.51 .119E+08 6699.4 1.03 29.56
16.94 243.64 .140E+08 8235.2 1.08 28.25
18.07 244.77 .162E+08 10104.2 1.14 26.73
19.20 245.90 .185E+08 12251.5 1.21 25.21
20.33 247.03 .209E+08 14597.0 1.28 23.89
-----

```

```

<----- hydrograph -----> <-pipe / channel->
AREA      QPEAK      TPEAK      R.V.      MAX DEPTH      MAX VEL
(ha)      (cms)      (hrs)      (mm)      (m)            (m/s)
-----
INFLOW : ID= 2 (8138) ***** 134.96 4.00 26.55 4.52 0.54
OUTFLOW: ID= 1 (6208) ***** 120.19 4.75 26.55 4.33 0.56
-----

```

```

-----
| CALIB |
| STANDBYD (2082) | Area (ha) = 426.60
| ID= 1 DT=15.0 min | Total Imp(%) = 64.00 Dir. Conn.(%) = 37.00
-----

```

```

IMPERVIOUS      PERVIOUS (i)
Surface Area (ha) = 273.02 153.58
Dep. Storage (mm) = 2.00 5.00
Average Slope (%) = 0.50 0.50
Length (m) = 1686.42 40.00
Mannings n = 0.013 0.250
Max. Eff. Inten. (mm/hr) = 101.74 112.47
over (min) = 15.00 30.00
Storage Coeff. (min) = 17.01 (ii) 27.22 (ii)
Unit Hyd. Tpeak (min) = 15.00 30.00
Unit Hyd. peak (cms) = 0.07 0.04
*TOTALS*
PEAK FLOW (cms) = 36.81 24.17 54.287 (iii)
TIME TO PEAK (hrs) = 1.50 1.75 1.50
RUNOFF VOLUME (mm) = 76.03 41.13 54.04
TOTAL RAINFALL (mm) = 78.03 78.03 78.03
RUNOFF COEFFICIENT = 0.97 0.53 0.69
-----

```

(i) CN PROCEDURE SELECTED FOR PERSISTENT LOSSES:
 CN* = 70.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
STANDHYD (2081) | Area (ha)= 265.70
ID= 1 DT=15.0 min | Total Imp(%)= 35.00 Dir. Com.(%)= 20.00
-----
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 93.00 172.71
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1330.91 40.00
Mannings n = 0.013 0.250
Max. Ref. Inten.(mm/hr)= 101.74 65.14
over (min)= 15.00 30.00
Storage Coeff. (min)= 14.76 (ii) 27.46 (iii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.07 0.04
-----
*TOTALS*
PEAK FLOW (cms)= 12.97 15.32 23.581 (iii)
TIME TO PEAK (hrs)= 1.50 1.75 1.50
RUNOFF VOLUME (mm)= 76.03 33.68 42.15
TOTAL RAINFALL (mm)= 78.03 78.03 78.03
RUNOFF COEFFICIENT = 0.97 0.43 0.54
  
```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

RESERVOIR (5208)
IN= 2 -> OUT= 1
DT= 15.0 min
-----
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
0.0000 0.0000 | 16.8530 6.3371
0.8360 2.7353 | 21.1630 7.3975
4.7810 3.1070 | 25.1140 8.4406
8.9050 4.2484 | 25.5140 8.4506
12.6070 5.1796 | 0.0000 0.0000
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (2081) 265.700 23.581 1.50 42.15
OUTFLOW : ID= 1 (5208) 265.700 11.914 2.25 42.15
-----
PEAK FLOW REDUCTION [Qout/qin](%)= 50.52
TIME SHIFT OF PEAK FLOW (min)= 45.00
MAXIMUM STORAGE USED (ha.m.)= 5.0347
  
```

```

ADD HYD (8148)
1 + 2 = 3
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (2082): 426.60 54.287 1.50 54.04
+ ID2= 2 (5208): 265.70 11.914 2.25 42.15
=====
ID = 3 (8148): 692.30 54.997 1.50 49.48
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
  
```

```

ADD HYD (8146)
1 + 2 = 3
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (6208): 14723.15 120.191 4.75 26.55
+ ID2= 2 (8148): 692.30 54.997 1.50 49.48
=====
ID = 3 (8146): 15415.45 126.000 4.00 27.58
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
  
```

```

ROUTE CN (6210)
IN= 2 -> OUT= 1 | Routing time step (min)'= 15.00
-----
<----- DATA FOR SECTION (2101.0) ----->
Distance Elevation Manning
0.00 241.81 0.0800
28.43 241.78 0.0800
85.30 237.04 0.0800
  
```

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
149.27	231.32	0.0800			
170.60	230.10	0.0800			
255.89	229.37	0.0800			
277.22	228.68	0.0800			
284.33	228.42	0.0800			
291.43	228.16	0.0800			
297.76	227.00	0.0800 / 0.0350	Main Channel		
298.76	225.00	0.0350	Main Channel		
312.76	225.00	0.0350	Main Channel		
313.76	227.00	0.0350 / 0.0800	Main Channel		
326.97	229.50	0.0800			
348.30	230.29	0.0800			
376.73	229.76	0.0800			
490.46	235.75	0.0800			
568.65	236.24	0.0800			
661.06	241.85	0.0800			
703.71	246.04	0.0800			

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.67 225.67 .141E+05 6.6 0.69 35.86
1.33 226.33 .289E+05 20.4 1.04 23.65
2.00 227.00 .443E+05 39.3 1.31 18.81
2.93 227.93 .730E+05 77.8 1.57 15.64
3.85 228.85 .123E+06 131.1 1.57 15.65
4.78 229.78 .228E+06 205.6 1.33 18.51
5.70 230.70 .503E+06 355.7 1.05 23.49
6.63 231.63 .846E+06 608.7 1.06 23.17
7.55 232.55 .123E+07 946.4 1.14 21.65
8.48 233.48 .165E+07 1367.0 1.22 20.13
9.41 234.41 .211E+07 1871.8 1.31 18.80
10.33 235.33 .261E+07 2463.2 1.40 17.65
11.26 236.26 .317E+07 2999.4 1.40 17.63
12.18 237.18 .384E+07 3848.3 1.48 16.63
13.11 238.11 .454E+07 4806.0 1.56 15.75
14.03 239.03 .528E+07 5876.9 1.64 14.97
14.96 239.96 .605E+07 7052.7 1.72 14.29
15.88 240.88 .687E+07 8365.0 1.80 13.68
16.81 241.81 .771E+07 9506.9 1.82 13.52
  
```

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8146) ***** 126.00 4.00 27.58 3.76 1.57
OUTFLOW : ID= 1 (6210) ***** 124.77 4.25 27.58 3.73 1.57
  
```

```

CALIB
NASHYD (0210) | Area (ha)= 218.27 Curve Number (CN)= 80.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.N. Tp(hrs)= 1.08
-----
Unit Hyd Qpeak (cms)= 3.454
PEAK FLOW (cms)= 4.500 (i)
TIME TO PEAK (hrs)= 3.000
RUNOFF VOLUME (mm)= 38.705
TOTAL RAINFALL (mm)= 78.027
RUNOFF COEFFICIENT = 0.496
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8150)
1 + 2 = 3
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0210): 218.27 4.500 3.00 38.71
+ ID2= 2 (6210): 15415.45 124.773 4.25 27.58
=====
ID = 3 (8150): 15633.72 128.721 4.25 27.74
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
  
```

```

CALIB
NASHYD (0218) | Area (ha)= 152.25 Curve Number (CN)= 69.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.N. Tp(hrs)= 0.79
  
```

```

Unit Hyd Qpeak (cms)= 3.278
PEAK FLOW (cms)= 2.875 (i)
TIME TO PEAK (hrs)= 2.500
RUNOFF VOLUME (mm)= 28.087
TOTAL RAINFALL (mm)= 78.027
RUNOFF COEFFICIENT = 0.360
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHVD (0216) Area (ha)= 145.77 Curve Number (CN)= 75.0
 ID= 1 DT=15.0 min (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.52

Unit Hyd Qpeak (cms)= 4.768
 PEAK FLOW (cms)= 4.504 (i)
 TIME TO PEAK (hrs)= 2.000
 RUNOFF VOLUME (mm)= 32.917
 TOTAL RAINFALL (mm)= 78.027
 RUNOFF COEFFICIENT = 0.422

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (2121) Area (ha)= 56.00
 ID= 1 DT=15.0 min Total Imp(%)= 93.00 Dir. Conn.(%)= 75.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	52.08	3.92
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	611.01	40.00
Mannings n	0.013	0.250
Max.Eff.Inten.(mm/hr)	101.74	294.50
over (min)	15.00	30.00
Storage Coeff. (min)	9.25 (ii)	16.20 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.05
TOTALS		
PEAK FLOW (cms)	11.30	2.06
TIME TO PEAK (hrs)	1.50	1.75
RUNOFF VOLUME (mm)	76.03	70.54
TOTAL RAINFALL (mm)	78.03	78.03
RUNOFF COEFFICIENT	0.97	0.69

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5212)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
ID= 2 -> OUT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min	0.0000	0.0000	3.7460	1.9720
	0.1900	1.3850	4.7040	2.0838
	1.0850	1.4413	5.4760	2.1940
	2.0210	1.5949	5.8760	2.2040
	2.8030	1.7848	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (2121)	56.000	13.033	1.50	70.54
OUTFLOW: ID= 1 (5212)	56.000	5.061	1.75	70.51

PEAK FLOW REDUCTION [Qout/Qin](%)= 38.83
 TIME SHIFT OF PEAK FLOW (min)= 15.00
 MAXIMUM STORAGE USED (ha.m.)= 2.1958

CALIB
 STANDHYD (2122) Area (ha)= 272.40
 ID= 1 DT=15.0 min Total Imp(%)= 29.00 Dir. Conn.(%)= 23.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	79.00	193.40
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1347.59	40.00
Mannings n	0.013	0.250
Max.Eff.Inten.(mm/hr)	101.74	51.42
over (min)	15.00	30.00
Storage Coeff. (min)	14.87 (ii)	28.83 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.07	0.04
TOTALS		
PEAK FLOW (cms)	15.25	13.07
TIME TO PEAK (hrs)	1.50	1.75
RUNOFF VOLUME (mm)	76.03	30.17
TOTAL RAINFALL (mm)	78.03	78.03
RUNOFF COEFFICIENT	0.97	0.39

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8152)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID= 1 (2122)	272.40	24.099	1.50	40.72
+ ID= 2 (5212)	56.00	5.061	1.75	70.51
ID = 3 (8152)	328.40	26.211	1.75	45.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6214) Routing time step (min)= 15.00
 IN= 2 -> OUT= 1

DATA FOR SECTION (2141.0)			
Distance	Elevation	Manning	
0.00	259.29	0.0500	
15.33	258.91	0.0500	
38.33	258.57	0.0500	
65.16	257.93	0.0500	
95.82	253.44	0.0500	
111.15	251.21	0.0500	
130.32	249.67	0.0500	
149.48	248.51	0.0500	
150.66	248.50	0.0500 / 0.0370	Main Channel
150.81	247.50	0.0370	Main Channel
154.66	247.50	0.0370	Main Channel
155.66	248.50	0.0370 / 0.0500	Main Channel
160.98	248.50	0.0500	
187.61	249.78	0.0500	
199.31	252.13	0.0500	
245.30	254.04	0.0500	
264.46	253.99	0.0500	
288.96	255.23	0.0500	
329.62	257.02	0.0500	
379.45	258.82	0.0500	

TRAVEL TIME TABLE					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.50	248.00	.648E+04	2.1	1.00	52.30
1.00	248.50	.139E+05	6.4	1.44	36.35
1.61	249.11	.570E+05	22.6	1.24	42.01
2.21	249.71	.143E+06	63.6	1.39	37.61
2.82	250.32	.263E+06	142.9	1.70	30.69
3.43	250.93	.403E+06	257.2	2.00	26.11
4.04	251.54	.562E+06	410.4	2.29	22.82
4.64	252.14	.735E+06	601.3	2.56	20.37
5.25	252.75	.933E+06	791.0	2.66	19.67
5.86	253.36	.117E+07	1038.0	2.79	18.74
6.46	253.96	.144E+07	1344.4	2.93	17.82
7.07	254.57	.179E+07	1664.1	2.93	17.83
7.68	255.18	.217E+07	2140.2	3.10	16.87
8.28	255.78	.259E+07	2722.9	3.30	15.83
8.89	256.39	.304E+07	3387.1	3.50	14.94
9.50	257.00	.351E+07	4132.7	3.69	14.16
10.11	257.61	.402E+07	4913.0	3.83	13.64
10.71	258.21	.458E+07	5648.3	3.87	13.50
11.32	258.82	.521E+07	6343.1	3.82	13.68

<---- hydrograph ----> <-pipe / channel-->						
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8152)	328.40	26.21	1.75	45.80	1.66	1.25
OUTFLOW: ID= 1 (6214)	328.40	16.90	2.25	45.80	1.39	1.31

CALIB
 STANDHYD (0214) Area (ha)= 316.95
 ID= 1 DT=15.0 min Total Imp(%)= 25.00 Dir. Conn.(%)= 9.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	79.24	237.71
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1453.62	40.00
Mannings n	0.013	0.250
Max.Eff.Inten.(mm/hr)	101.74	51.13
over (min)	15.00	30.00

Storage Coeff. (min) = 15.56 (ii) 29.55 (iii)
 Unit Hyd. Tpeak (min) = 18.00 30.00
 Unit Hyd. peak (cms) = 0.07 0.04
 TOTALS
 PEAK FLOW (cms) = 6.85 15.75 19.482 (iii)
 TIME TO PEAK (hrs) = 1.50 1.75 1.75
 RUNOFF VOLUME (mm) = 76.03 27.01 31.42
 TOTAL RAINFALL (mm) = 78.03 78.03 78.03
 RUNOFF COEFFICIENT = 0.97 0.35 0.40

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 62.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8154) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
|-----|
| ID1= 1 (0214): 316.95 19.482 1.75 31.42 |
| + ID2= 2 (6214): 328.40 16.903 2.25 45.80 |
|-----|
| ID = 3 (8154): 645.35 33.688 1.75 38.74 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8156) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
|-----|
| ID1= 1 (0216): 145.77 4.504 2.00 32.92 |
| + ID2= 2 (8154): 645.35 33.688 1.75 38.74 |
|-----|
| ID = 3 (8156): 791.12 37.919 1.75 37.67 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6218) |
| IN= 2--> OUT= 1 |
|-----|
| Routing time step (min) = 15.00 |
-----

```

<----- DATA FOR SECTION (2181.0) ----->

Distance	Elevation	Manning	
0.00	256.38	0.0450	
7.70	254.85	0.0450	
15.40	253.20	0.0450	
19.25	252.29	0.0450	
23.10	251.17	0.0450	
78.94	236.16	0.0450	
80.87	235.80	0.0450	
82.79	235.45	0.0450	
84.57	234.60	0.0450 / 0.0300	Main Channel
85.07	233.60	0.0300	Main Channel
88.57	233.60	0.0300	Main Channel
93.57	233.60	0.0300	Main Channel
94.07	234.60	0.0300 / 0.0450	Main Channel
96.27	235.13	0.0450	
98.19	235.20	0.0450	
100.12	235.32	0.0450	
138.63	239.95	0.0450	
180.98	244.80	0.0450	
188.69	245.00	0.0450	
190.61	256.00	0.0450	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
1.00	234.60	181E+05	6.0	0.66	50.64
2.13	235.73	518E+05	23.8	0.93	36.27
3.25	236.85	119E+06	60.0	1.02	33.03
4.38	237.98	217E+06	120.6	1.12	29.99
5.51	239.11	346E+06	210.5	1.23	27.39
6.63	240.23	506E+06	334.1	1.33	25.22
7.76	241.36	697E+06	495.6	1.43	23.44
8.88	242.48	920E+06	699.8	1.53	21.91
10.01	243.61	117E+07	950.8	1.63	20.59
11.14	244.74	146E+07	1252.5	1.73	19.45
12.26	245.86	178E+07	1619.6	1.83	18.37
13.39	246.99	212E+07	2076.5	1.97	17.01
14.52	248.12	247E+07	2589.0	2.11	15.88
15.64	249.24	282E+07	3149.7	2.25	14.94
16.77	250.37	319E+07	3759.8	2.37	14.14
17.89	251.49	356E+07	4419.5	2.50	13.44
19.02	252.62	395E+07	5126.4	2.61	12.84
20.15	253.75	435E+07	5871.6	2.72	12.34
21.27	254.87	476E+07	6663.2	2.82	11.89

```

-----
| ADD HYD (8158) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
|-----|
| ID1= 1 (0218): 152.25 2.875 2.50 28.09 |
| + ID2= 2 (6218): 791.12 27.748 2.25 37.67 |
|-----|
| ID = 3 (8158): 943.37 30.564 2.25 36.12 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8151) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
|-----|
| ID1= 1 (8150): 15633.72 128.721 4.25 27.74 |
| + ID2= 2 (8158): 943.37 30.564 2.25 36.12 |
|-----|
| ID = 3 (8151): 16577.09 146.688 2.75 28.21 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| RESERVOIR (5504) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
|-----|
| OUTFLOW STORAGE | OUTFLOW STORAGE |
| (cms) (ha.m.) | (cms) (ha.m.) | |
|---|---|---|
| 0.0000 0.0000 | 33.0100 0.6700 |
| 2.6400 0.0500 | 37.4800 0.7800 |
| 4.8300 0.0900 | 47.5400 1.0100 |
| 7.4100 0.1300 | 60.2300 1.2800 |
| 10.3300 0.1900 | 71.9530 1.5258 |
| 13.5300 0.2500 | ***** | 3.0529 |
| 16.9800 0.3200 | ***** | 5.3299 |
| 20.6800 0.3900 | ***** | 8.4148 |
| 24.6000 0.4800 | ***** | 11.7637 |
| 28.7100 0.5700 | ***** | 11.7737 |
-----

```

```

-----
| ROUTE CHN (6218) |
| IN= 2--> OUT= 1 |
|-----|
| Routing time step (min) = 15.00 |
-----

```

PEAK FLOW REDUCTION [Qout/Qin] (%) = 99.83

TIME SHIFT OF PEAK FLOW (min) = 0.00

MAXIMUM STORAGE USED (ha.m.) = 3.1488

```

-----
| ROUTE CHN (6220) |
| IN= 2--> OUT= 1 |
|-----|
| Routing time step (min) = 15.00 |
-----

```

<----- DATA FOR SECTION (2201.0) ----->

Distance	Elevation	Manning	
0.00	245.59	0.0600	
5.76	245.45	0.0600	
23.03	242.27	0.0600	
57.58	232.35	0.0600	
86.37	226.18	0.0600	
109.41	224.31	0.0600	
115.17	223.86	0.0600	
120.92	222.78	0.0600	
126.68	221.93	0.0600	
128.06	221.50	0.0600 / 0.0300	Main Channel
129.56	220.00	0.0300	Main Channel
141.56	220.00	0.0300	Main Channel
143.06	221.50	0.0300 / 0.0600	Main Channel
149.72	223.77	0.0600	
155.47	225.09	0.0600	
253.36	229.21	0.0600	
333.98	227.50	0.0600	
454.90	229.25	0.0600	
500.97	233.11	0.0600	
570.07	237.42	0.0600	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.75	220.75	262E+05	5.5	0.57	79.44
1.50	221.50	555E+05	17.5	0.86	52.89
2.44	222.44	103E+06	42.7	1.14	40.03
3.37	223.37	171E+06	79.4	1.27	35.98

4.31	224.31	.263E+06	128.1	1.33	34.22
5.25	225.25	.393E+06	191.7	1.34	34.19
6.18	226.18	.594E+06	276.7	1.28	35.77
7.12	227.12	.872E+06	399.9	1.26	36.33
8.06	228.06	1.27E+07	542.7	1.17	38.92
8.99	228.99	1.99E+07	770.0	1.06	43.01
9.93	229.93	2.97E+07	1151.8	1.06	42.95
10.86	230.86	4.00E+07	1652.9	1.13	40.33
11.80	231.80	5.07E+07	2251.8	1.22	37.53
12.74	232.74	6.18E+07	2946.5	1.31	34.97
13.67	233.67	7.33E+07	3726.8	1.39	32.79
14.61	234.61	8.53E+07	4591.0	1.48	30.96
15.55	235.55	9.77E+07	5544.1	1.55	29.37
16.48	236.48	1.11E+08	6586.7	1.63	27.99
17.42	237.42	1.24E+08	7719.2	1.71	26.77

<---- hydrograph ----> <-pipe / channel-->

	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (5504)	*****	146.44	2.75	28.21	4.58	1.33
OUTFLOW: ID= 1 (6220)	*****	140.19	4.25	28.21	4.50	1.33

```

-----
| CALIB          |
| NASHYD (0220) | Area (ha)= 169.10 Curve Number (CN)= 73.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 250
|-----|
| U.H. Tp(hrs)= 1.50
|-----|
Unit Hyd Qpeak (cms)= 1.923
PEAK FLOW (cms)= 2.194 (i)
TIME TO PEAK (hrs)= 3.750
RUNOFF VOLUME (mm)= 31.761
TOTAL RAINFALL (mm)= 78.027
RUNOFF COEFFICIENT = 0.407

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8160) |
| 1 + 2 = 3      | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (0220): | 169.10 2.194 3.75 31.76
| + ID2= 2 (6220):| 16577.09 140.190 4.25 28.21
|=====|
| ID = 3 (8160): | 16746.19 142.330 4.25 28.25

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB          |
| NASHYD (0226) | Area (ha)= 237.58 Curve Number (CN)= 75.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 150
|-----|
| U.H. Tp(hrs)= 1.05
|-----|
Unit Hyd Qpeak (cms)= 3.852
PEAK FLOW (cms)= 4.306 (i)
TIME TO PEAK (hrs)= 3.000
RUNOFF VOLUME (mm)= 33.499
TOTAL RAINFALL (mm)= 78.027
RUNOFF COEFFICIENT = 0.429

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB          |
| STANDHYD (2221) | Area (ha)= 145.20 Dir. Conn.(%)= 19.00
| ID= 1 DT=15.0 min | Total Imp(%)= 36.00
|-----|

```

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	52.27	92.93
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	983.87	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	101.74	77.60
over (min)	15.00	30.00
Storage Coeff. (min)	12.31 (ii)	24.15 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04
PEAK FLOW (cms)	7.05	10.56
TIME TO PEAK (hrs)	1.50	1.75
RUNOFF VOLUME (mm)	76.03	38.92
TOTAL RAINFALL (mm)	78.03	78.03
RUNOFF COEFFICIENT	0.97	0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 75.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| RESERVOIR (5222) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min     |
|-----|
| OUTFLOW STORAGE | OUTFLOW STORAGE |
| (cms) (ha.m.) | (cms) (ha.m.) |
|-----|
| 0.0000 0.0000 | 6.4640 3.7677 |
| 0.4620 2.0014 | 8.2830 4.2576 |
| 2.0160 2.2850 | 14.1970 4.6909 |
| 3.6250 2.7987 | 14.5970 4.7009 |
| 5.0230 3.2496 | 0.0000 0.0000 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
|-----|
| INFLOW : ID= 2 (2221) | 145.200 14.643 1.50 45.97 |
| OUTFLOW: ID= 1 (5222) | 145.200 5.789 2.25 45.97 |
|-----|

```

PEAK FLOW REDUCTION [Qout/Qin](%)= 39.54
TIME SHIFT OF PEAK FLOW (min)= 45.00
MAXIMUM STORAGE USED (ha.m.)= 3.5319

```

-----
| CALIB          |
| STANDHYD (2222) | Area (ha)= 74.30 Dir. Conn.(%)= 41.00
| ID= 1 DT=15.0 min | Total Imp(%)= 78.00
|-----|

```

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	57.95	16.35
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	703.80	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	101.74	221.17
over (min)	15.00	30.00
Storage Coeff. (min)	10.07 (ii)	17.86 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.05
PEAK FLOW (cms)	8.10	6.24
TIME TO PEAK (hrs)	1.50	1.75
RUNOFF VOLUME (mm)	76.03	53.84
TOTAL RAINFALL (mm)	78.03	78.03
RUNOFF COEFFICIENT	0.97	0.69

TOTALS

13.237 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 75.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8164) |
| 1 + 2 = 3      | AREA QPEAK TPEAK R.V.
|-----|
| ID1= 1 (2222): | 74.30 13.237 1.50 62.94
| + ID2= 2 (5222):| 145.20 5.789 2.25 45.97
|=====|
| ID = 3 (8164): | 219.50 13.565 1.50 51.71

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB          |
| STANDHYD (0224) | Area (ha)= 140.45 Dir. Conn.(%)= 26.00
| ID= 1 DT=15.0 min | Total Imp(%)= 34.00
|-----|

```

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	47.75	92.70
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	967.64	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	101.74	71.77
over (min)	15.00	30.00
Storage Coeff. (min)	12.19 (ii)	24.41 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04

*****TOTALS*
 PEAK FLOW (cms)= 9.36 9.69 16.322 (iii)
 TIME TO PEAK (hrs)= 1.50 1.75 1.50
 RUNOFF VOLUME (mm)= 76.03 40.45 49.70
 TOTAL RAINFALL (mm)= 76.03 76.03 76.03
 RUNOFF COEFFICIENT = 0.97 0.52 0.64

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8166)
 1 + 2 = 3
 AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (0224): 140.45 16.322 1.50 49.70
 + ID2= 2 (8164): 219.50 13.565 1.50 51.71
 ID = 3 (8166): 359.95 29.887 1.50 50.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6226)
 IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (2261.0) -----

Distance	Elevation	Manning
0.00	245.35	0.0600
8.27	242.72	0.0600
16.54	240.01	0.0600
31.02	235.31	0.0600
41.36	233.14	0.0600
90.99	227.73	0.0600
93.06	227.50	0.0600
95.13	227.27	0.0600
95.83	227.00	0.0600
100.83	226.50	0.0600 / 0.0350 Main Channel
101.33	225.50	0.0350 Main Channel
101.83	226.50	0.0350 / 0.0600 Main Channel
105.83	227.00	0.0600
107.54	227.39	0.0600
109.60	227.62	0.0600
111.67	227.86	0.0600
146.83	231.76	0.0600
173.71	234.77	0.0600
186.12	238.23	0.0600
202.67	238.35	0.0600

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.50	226.00	4.03E+03	0.0	0.16	330.59
1.00	226.50	1.61E+04	0.1	0.26	208.26
1.70	227.20	1.73E+05	1.5	0.27	195.72
2.39	227.89	5.41E+05	5.9	0.35	152.27
3.09	228.59	1.19E+06	16.0	0.43	124.08
3.79	229.29	2.12E+06	33.8	0.51	104.65
4.49	229.99	3.34E+06	61.2	0.59	90.97
5.18	230.68	4.84E+06	99.7	0.66	80.88
5.88	231.38	6.63E+06	151.0	0.73	73.13
6.58	232.08	8.70E+06	216.5	0.80	66.95
7.27	232.77	1.11E+07	297.5	0.87	61.91
7.97	233.47	1.37E+07	398.1	0.94	57.29
8.67	234.17	1.65E+07	518.9	1.01	53.12
9.36	234.86	1.96E+07	658.9	1.08	49.59
10.06	235.56	2.28E+07	827.0	1.17	46.02
10.76	236.26	2.62E+07	1015.4	1.25	42.96
11.46	236.96	2.96E+07	1221.2	1.33	40.41
12.15	237.65	3.32E+07	1444.6	1.40	38.26
12.85	238.35	3.68E+07	1598.2	1.40	38.42

----- hydrograph ----- <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8166) 359.95 29.89 1.50 50.93 3.64 0.49
 OUTFLOW: ID= 1 (6226) 359.95 9.86 2.25 50.93 2.67 0.38

ADD HYD (8168)
 1 + 2 = 3
 AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (0226): 237.58 4.306 3.00 33.50
 + ID2= 2 (6226): 359.95 9.864 2.25 50.93

ID = 3 (8168): 597.53 13.964 2.50 44.00
 NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8162)
 1 + 2 = 3
 AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (8160): 16746.19 142.330 4.25 28.25
 + ID2= 2 (8168): 597.53 13.964 2.50 44.00
 ID = 3 (8162): 17343.72 152.950 4.00 28.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6228)
 IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (2281.0) -----

Distance	Elevation	Manning
0.00	245.00	0.0600
18.60	244.74	0.0600
37.20	242.47	0.0600
93.01	233.71	0.0600
125.57	226.95	0.0600
213.93	221.63	0.0600
218.58	221.15	0.0600
223.23	220.45	0.0600
225.03	220.55	0.0600 / 0.0300 Main Channel
225.53	218.05	0.0300 Main Channel
232.53	218.05	0.0300 Main Channel
240.03	218.05	0.0300 Main Channel
241.03	220.55	0.0300 / 0.0600 Main Channel
246.48	221.00	0.0600
251.13	221.27	0.0600
255.78	221.22	0.0600
325.54	225.25	0.0600
367.40	228.91	0.0600
404.60	231.75	0.0600
460.41	245.00	0.0600

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
1.25	219.30	3.38E+05	16.0	0.86	35.15
2.50	220.55	6.96E+05	48.3	1.26	24.02
3.94	221.99	1.68E+06	116.0	1.26	24.14
5.38	223.43	3.92E+06	247.5	1.15	26.37
6.81	224.86	7.43E+06	472.6	1.16	26.20
8.25	226.30	1.22E+07	821.6	1.23	24.67
9.69	227.74	1.79E+07	1320.1	1.34	22.59
11.13	229.18	2.43E+07	1955.7	1.47	20.58
12.57	230.62	3.13E+07	2744.2	1.59	19.02
14.01	232.06	3.90E+07	3683.4	1.72	17.66
15.44	233.49	4.72E+07	4824.1	1.86	16.30
16.88	234.93	5.57E+07	6092.4	1.99	15.23
18.32	236.37	6.46E+07	7503.3	2.11	14.35
19.76	237.81	7.39E+07	9061.2	2.23	13.59
21.20	239.25	8.36E+07	10767.6	2.34	12.94
22.64	240.69	9.37E+07	12624.8	2.45	12.37
24.07	242.12	1.04E+08	14634.9	2.56	11.87
25.51	243.56	1.15E+08	16762.8	2.65	11.45
26.95	245.00	1.27E+08	18699.9	2.69	11.28

----- hydrograph ----- <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8162) ***** 152.95 4.00 28.79 4.34 1.22
 OUTFLOW: ID= 1 (6228) ***** 148.88 4.50 28.79 4.31 1.23

CALIB NASHYD (0228) Area (ha)= 310.54 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.12

Unit Hyd Qpeak (cms)= 4.751
 PEAK FLOW (cms)= 3.577 (i)
 TIME TO PEAK (hrs)= 3.250
 RUNOFF VOLUME (mm)= 22.456
 TOTAL RAINFALL (mm)= 78.027
 RUNOFF COEFFICIENT = 0.288

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8170)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0228):	310.54	3.577	3.25	22.46
+ ID2= 2 (6228):	17343.72	148.877	4.50	28.79
=====	=====	=====	=====	=====
ID = 3 (8170):	17654.26	151.964	4.50	28.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	Dir. Conn.(%)
STANDHYD (2302)	126.70		9.00
ID= 1 DT=15.0 min	Total Imp(%)= 23.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	29.14	97.56
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	919.06	40.00
Mannings n	0.013	0.250
Max.Eff.Inten.(mm/hr)	101.74	59.33
over (min)	15.00	30.00
Storage Coeff. (min)	11.82 (ii)	25.00 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04
		TOTALS
PEAK FLOW (cms)	2.94	8.23
TIME TO PEAK (hrs)	1.50	1.75
RUNOFF VOLUME (mm)	76.03	31.97
TOTAL RAINFALL (mm)	78.03	78.03
RUNOFF COEFFICIENT	0.97	0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Dir. Conn.(%)
STANDHYD (2301)	78.80		19.00
ID= 1 DT=15.0 min	Total Imp(%)= 50.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	39.40	39.40
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	724.80	40.00
Mannings n	0.013	0.250
Max.Eff.Inten.(mm/hr)	101.74	97.92
over (min)	15.00	30.00
Storage Coeff. (min)	10.25 (ii)	21.04 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.05
		TOTALS
PEAK FLOW (cms)	3.97	6.03
TIME TO PEAK (hrs)	1.50	1.50
RUNOFF VOLUME (mm)	76.03	38.63
TOTAL RAINFALL (mm)	78.03	78.03
RUNOFF COEFFICIENT	0.97	0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5230)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min				

0.0000	0.0000	5.6350	1.5914
0.3160	0.7624	6.9200	1.8001
1.7040	0.8931	7.9010	2.0070
3.5140	1.1218	8.3010	2.0170
4.5020	1.3405	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (2301)	78.800	8.411	1.50	45.74

OUTFLOW: ID= 1 (5230) 78.800 4.998 2.00 45.73

PEAK FLOW REDUCTION [Qout/Qin](%)= 59.43
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 1.4515

ADD HYD (8172)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (2302):	126.70	9.572	1.75	35.93
+ ID2= 2 (5230):	78.80	4.998	2.00	45.73
=====	=====	=====	=====	=====
ID = 3 (8172):	205.50	13.880	1.75	39.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	Curve Number (CN)
NASHYD (0232)	314.80		58.0
ID= 1 DT=15.0 min	U.H. Tp(hrs)= 1.00		# of Linear Res. (N)= 1.50

Unit Hyd Qpeak (cms)= 5.391

PEAK FLOW (cms)= 3.616 (i)
 TIME TO PEAK (hrs)= 3.000
 RUNOFF VOLUME (mm)= 20.541
 TOTAL RAINFALL (mm)= 78.027
 RUNOFF COEFFICIENT = 0.263

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8173)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0232):	314.80	3.616	3.00	20.54
+ ID2= 2 (8172):	205.50	13.880	1.75	39.69
=====	=====	=====	=====	=====
ID = 3 (8173):	520.30	16.471	1.75	28.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8174)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8170):	17654.26	151.964	4.50	28.68
+ ID2= 2 (8173):	520.30	16.471	1.75	28.10
=====	=====	=====	=====	=====
ID = 3 (8174):	18174.56	156.195	4.25	28.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6234)	Routing time step (min)
IN= 2--> OUT= 1	= 15.00

<----- DATA FOR SECTION (2341.0) ----->

Distance	Elevation	Manning
0.00	228.00	0.0550
42.86	223.21	0.0550
85.72	219.56	0.0550
107.15	219.42	0.0550
128.58	219.39	0.0550
214.30	219.30	0.0550
225.02	219.26	0.0550
235.73	219.24	0.0550
255.16	219.20	0.0550 / 0.0350
257.16	217.20	0.0350
303.16	217.20	0.0350
305.16	219.20	0.0350 / 0.0550
310.74	219.28	0.0550
321.45	219.27	0.0550
332.17	219.26	0.0550
396.46	219.29	0.0550
782.20	222.91	0.0550
878.64	227.43	0.0550
964.16	227.80	0.0550
1060.79	228.00	0.0550

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.50	217.70	.887E+05	7.2	0.31	206.54
1.00	218.20	.179E+06	22.7	0.48	131.74

1.50	218.70	.272E+06	44.5	0.62	101.75
2.00	219.20	.366E+06	71.8	0.75	84.97
2.59	219.79	.993E+06	136.1	0.52	121.59
3.17	220.37	.189E+07	258.6	0.52	121.80
3.76	220.96	.294E+07	434.7	0.56	112.82
4.35	221.55	.415E+07	665.9	0.61	103.88
4.93	222.13	.551E+07	955.0	0.66	96.22
5.52	222.72	.703E+07	1305.1	0.71	89.80
6.11	223.31	.868E+07	1744.9	0.77	82.91
6.69	223.89	.104E+08	2261.4	0.83	76.46
7.28	224.48	.121E+08	2838.0	0.89	71.12
7.87	225.07	.139E+08	3472.9	0.95	66.63
8.45	225.65	.157E+08	4165.0	1.01	62.92
9.04	226.24	.176E+08	4913.5	1.07	59.54
9.63	226.83	.194E+08	5717.7	1.12	56.68
10.21	227.41	.214E+08	6577.1	1.17	54.18
10.80	228.00	.235E+08	6919.5	1.12	56.61

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8174) ***** 156.19 4.25 28.66 2.68 0.52
OUTFLOW: ID= 1 (6234) ***** 126.16 6.75 28.66 2.50 0.55

```

```

-----
| CALIB |
| STANHYD (0234) | Area (ha)= 267.16
| ID= 1 DT=15.0 min | Total Imp(%)= 22.00 Dir. Conn.(%)= 8.00
-----
| IMPERVIOUS | PERVIOUS (i)
| Surface Area (ha)= 58.78 | 208.38
| Dep. Storage (mm)= 2.00 | 5.00
| Average Slope (%)= 0.50 | 0.50
| Length (m)= 1334.57 | 40.00
| Mannings n = 0.013 | 0.250
-----
| Max. Eff. Inten. (mm/hr)= 101.74 | 16.56
| over (min)= 15.00 | 45.00
| Storage Coeff. (min)= 14.79 (ii) | 36.75 (ii)
| Unit Hyd. Tpeak (min)= 15.00 | 45.00
| Unit Hyd. peak (cms)= 0.07 | 0.03
-----
| PEAK FLOW (cms)= 5.21 | 5.28 | 7.291 (iii)
| TIME TO PEAK (hrs)= 1.50 | 2.00 | 1.50
| RUNOFF VOLUME (mm)= 76.03 | 13.26 | 18.28
| TOTAL RAINFALL (mm)= 78.03 | 78.03 | 78.03
| RUNOFF COEFFICIENT = 0.97 | 0.17 | 0.23
-----

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 39.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8176) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0234): 267.16 7.291 1.50 18.28
+ ID2= 2 (6234): 18174.56 126.159 6.75 28.66
=====
ID = 3 (8176): 18441.72 126.181 6.75 28.51
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0238) | Area (ha)= 311.70 Curve Number (CN)= 53.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp (hrs)= 1.56
-----
| Unit Hyd Qpeak (cms) = 3.405
-----
| PEAK FLOW (cms) = 2.195 (i)
| TIME TO PEAK (hrs) = 4.000
| RUNOFF VOLUME (mm) = 17.786
| TOTAL RAINFALL (mm) = 78.027
| RUNOFF COEFFICIENT = 0.228
-----
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
-----

```

```

-----
| CALIB |
| NASHYD (0236) | Area (ha)= 494.49 Curve Number (CN)= 54.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
-----

```

U.H. Tp (hrs)= 1.24

Unit Hyd Qpeak (cms)= 6.830

PEAK FLOW (cms)= 4.282 (i)
TIME TO PEAK (hrs)= 3.500
RUNOFF VOLUME (mm)= 18.291
TOTAL RAINFALL (mm)= 78.027
RUNOFF COEFFICIENT = 0.234

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ROUTE CHN (6238) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00
-----

```

```

<----- DATA FOR SECTION (2381.0) ----->
Distance Elevation Manning
0.00 241.54 0.0380
602.55 227.00 0.0380
1702.00 224.50 0.0380
1721.25 223.00 0.0380
1725.10 222.60 0.0380
1780.94 222.50 0.0380
1782.87 222.45 0.0380
1784.79 222.40 0.0380 / 0.0300 Main Channel
1786.57 221.75 0.0300 Main Channel
1787.07 220.75 0.0300 Main Channel
1787.57 220.75 0.0300 Main Channel
1791.57 220.75 0.0300 Main Channel
1794.07 221.75 0.0300 Main Channel
1798.27 222.00 0.0300 Main Channel
1800.19 222.35 0.0300 / 0.0380 Main Channel
1802.12 222.47 0.0380
1840.63 223.00 0.0380
1900.00 225.00 0.0380
1950.00 226.00 0.0380
2242.61 240.00 0.0380

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.80 221.55 1.01E+05 5.2 1.14 32.36
1.60 222.35 2.96E+05 18.7 1.40 26.36
2.64 223.39 2.49E+06 151.8 1.35 27.33
3.68 224.43 613E+06 512.2 1.85 19.94
4.71 225.46 1.52E+07 1066.8 1.55 23.80
5.75 226.50 3.99E+07 2824.9 1.75 21.16
6.79 227.54 6.63E+07 6528.0 2.18 16.93
7.83 228.58 9.94E+07 12158.6 2.71 13.62
8.87 229.62 1.34E+08 19268.8 3.19 11.58
9.91 230.66 1.70E+08 27785.3 3.63 10.19
10.94 231.69 2.07E+08 37667.1 4.03 9.18
11.98 232.73 2.46E+08 48891.7 4.40 8.40
13.02 233.77 2.87E+08 61448.0 4.75 7.78
14.06 234.81 3.29E+08 75332.7 5.08 7.28
15.10 235.85 372E+08 90547.8 5.39 6.85
16.14 236.89 417E+08 ***** 5.69 6.49
17.17 237.92 4.64E+08 ***** 5.98 6.18
18.21 238.96 5.12E+08 ***** 6.25 5.91
19.25 240.00 5.61E+08 ***** 6.52 5.67

```

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (0236) 494.49 4.28 3.50 18.29 0.66 1.14
OUTFLOW: ID= 1 (6238) 494.49 4.15 4.00 18.29 0.63 1.14

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8178) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0238): 311.70 2.195 4.00 17.79
+ ID2= 2 (6238): 494.49 4.151 4.00 18.29
=====
ID = 3 (8178): 806.19 6.346 4.00 18.10
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.


```

-----
| ROUTE CHN (6240) |
| IN= 2--> OUT= 1 |
-----
<----- DATA FOR SECTION (2401.0) ----->
Distance      Elevation      Manning
0.00          222.00          0.0450
11.46         221.00          0.0450
208.98        221.00          0.0450
404.04        220.97          0.0450
808.08        220.83          0.0450
905.60        220.17          0.0450
919.53        219.43          0.0450
933.47        219.22          0.0450
945.26        219.21          0.0450 / 0.0300 Main Channel
946.26        217.81          0.0300 Main Channel
975.26        217.81          0.0300 Main Channel
1003.26       217.81          0.0300 Main Channel
1005.26       219.21          0.0300 / 0.0450 Main Channel
1017.06       219.28          0.0450
1030.99       219.26          0.0450
1044.92       219.23          0.0450
1058.86       219.23          0.0450
1253.91       219.22          0.0450
1323.57       221.05          0.0450
1379.30       222.00          0.0450

```

```

-----
<----- TRAVEL TIME TABLE ----->
DEPTH      ELEV      VOLUME      FLOW RATE      VELOCITY      TRAV.TIME
(m)        (m)        (cu.m.)     (cms)          (m/s)         (min)
0.20  218.01  .307E+05    2.2            0.20          227.52
0.40  218.21  .617E+05    7.1            0.31          143.95
0.60  218.41  .938E+05   14.0           0.41          110.33
0.80  218.61  .124E+06   22.6           0.49           91.46
1.00  218.81  .156E+06   32.8           0.57           79.15
1.20  219.01  .188E+06   44.5           0.64           70.38
1.40  219.21  .220E+06   57.5           0.70           63.77
1.63  219.44  .413E+06   82.5           0.54           83.52
1.87  219.68  .632E+06  120.9          0.51           87.03
2.10  219.91  .858E+06  170.0          0.53           84.11
2.33  220.14  .109E+07  228.7          0.56           79.62
2.56  220.37  .134E+07  294.4          0.59           76.03
2.80  220.61  .162E+07  371.1          0.61           72.75
3.03  220.84  .192E+07  456.8          0.64           70.18
3.26  221.07  .251E+07  552.1          0.59           75.74
3.49  221.30  .333E+07  711.8          0.57           78.07
3.73  221.54  .417E+07  907.0          0.58           76.62
3.96  221.77  .502E+07  1134.1         0.61           73.70
4.19  222.00  .587E+07  1390.9         0.64           70.34

```

```

<----- hydrograph -----> <-pipe / channel->
AREA      QPEAK      TPEAK      R.V.      MAX DEPTH      MAX VEL
(ha)      (cms)      (hrs)      (mm)      (m)            (m/s)
INFLOW : ID= 2 (8178) ***** 130.15  6.50  28.08  1.91  0.52
OUTFLOW : ID= 1 (6240) ***** 120.32  8.50  28.08  1.86  0.51

```

```

-----
| CALIB |
| NASHVD (0240) | Area (ha)= 434.37 Curve Number (CN)= 46.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 3.60

```

```

Unit Hyd Qpeak (cms)= 2.058
PEAK FLOW (cms)= 1.150 (i)
TIME TO PEAK (hrs)= 5.500
RUNOFF VOLUME (mm)= 14.345
TOTAL RAINFALL (mm)= 78.027
RUNOFF COEFFICIENT = 0.184

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8184) |
| 1 + 2 = 3 |
-----
AREA      QPEAK      TPEAK      R.V.
(ha)      (cms)      (hrs)      (mm)
ID1= 1 (0240): 434.37  1.150  5.50  14.35
+ ID2= 2 (6240): 19247.91 120.319  8.50  28.08
=====
ID = 3 (8184): 19682.28 121.350  8.25  27.77

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHVD (0242) | Area (ha)= 657.88 Curve Number (CN)= 52.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 5.37

```

Unit Hyd Qpeak (cms)= 2.090

```

PEAK FLOW (cms)= 1.420 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 17.329
TOTAL RAINFALL (mm)= 78.027
RUNOFF COEFFICIENT = 0.222

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8184) |
| 1 + 2 = 3 |
-----
AREA      QPEAK      TPEAK      R.V.
(ha)      (cms)      (hrs)      (mm)
ID1= 1 (0242): 657.88  1.420  7.25  17.33
+ ID2= 2 (8182): 19682.28 121.350  8.25  27.77
=====
ID = 3 (8184): 20340.16 122.756  8.25  27.44

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6244) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)'= 15.00

```

```

-----
<----- DATA FOR SECTION (2441.0) ----->
Distance      Elevation      Manning
0.00          225.00          0.0450
2.50          220.10          0.0450
3.00          220.06          0.0450
3.64          220.07          0.0450
7.28          220.08          0.0450
10.91         220.09          0.0450
14.55         219.81          0.0450
18.19         219.29          0.0450
24.13         219.21          0.0450 / 0.0300 Main Channel
24.63         217.81          0.0300 Main Channel
69.13         217.81          0.0300 Main Channel
127.34        217.81          0.0300 Main Channel
115.13        219.21          0.0300 / 0.0450 Main Channel
120.06        219.25          0.0450
123.70        219.32          0.0450
127.34        219.77          0.0450
130.98        219.88          0.0450
134.61        219.99          0.0450
350.00        220.18          0.0450
360.18        225.00          0.0450

```

```

-----
<----- TRAVEL TIME TABLE ----->
DEPTH      ELEV      VOLUME      FLOW RATE      VELOCITY      TRAV.TIME
(m)        (m)        (cu.m.)     (cms)          (m/s)         (min)
0.35  218.16  .108E+06   9.0            0.29          200.52
0.70  218.51  .216E+06  28.4           0.45          126.89
1.05  218.86  .325E+06  55.7           0.59           97.27
1.40  219.21  .434E+06  89.7           0.71           80.65
1.79  219.60  .574E+06  135.9          0.81           70.36
2.17  219.98  .726E+06  190.8          0.90           63.38
2.56  220.37  .881E+06  264.3          0.82           69.82
2.94  220.75  .157E+07  371.7          0.81           70.36
3.33  221.14  .203E+07  503.4          0.85           67.30
3.72  221.53  .250E+07  656.6          0.90           63.40
4.10  221.91  .296E+07  829.6          0.96           59.54
4.49  222.30  .343E+07  1021.1         1.02           56.00
4.87  222.68  .390E+07  1230.2         1.08           52.83
5.26  223.07  .437E+07  1456.2         1.14           50.01
5.65  223.46  .484E+07  1698.4         1.20           47.51
6.03  223.84  .531E+07  1956.3         1.26           45.28
6.42  224.23  .579E+07  2229.3         1.32           43.28
6.80  224.61  .626E+07  2517.0         1.38           41.48
7.19  225.00  .674E+07  2819.2         1.44           39.85

```

```

<----- hydrograph -----> <-pipe / channel->
AREA      QPEAK      TPEAK      R.V.      MAX DEPTH      MAX VEL
(ha)      (cms)      (hrs)      (mm)      (m)            (m/s)
INFLOW : ID= 2 (8184) ***** 122.76  8.25  27.44  1.68  0.78
OUTFLOW : ID= 1 (6244) ***** 118.66  9.50  27.44  1.64  0.77

```

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-----
| CALIB |
| NASHVD (0244) | Area (ha)=1908.71 Curve Number (CN)= 61.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 2.20

```

Unit Hyd Qpeak (cms)= 14.812

```

PEAK FLOW (cms)= 12.730 (i)
TIME TO PEAK (hrs)= 4.250
RUNOFF VOLUME (mm)= 22.581
TOTAL RAINFALL (mm)= 78.027

```

RUNOFF COEFFICIENT = 0.289

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8186) |
|-----|
| 1 2 3 |
|-----|
| ID1= 1 (0244): 1908.71 12.730 4.25 22.58 |
| + ID2= 2 (6244): 20340.16 118.662 9.50 27.44 |
|-----|
| ID = 3 (8186): 22248.87 125.773 9.25 27.02 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| READ HYD (8190) | AREA (ha)=***** | |
|-----| | TPEAK (hrs)= 12.50 |
| DT=15.0 min | VOLUME (mm)= 14.04 |
|-----| |
| Filename: S:\2010 Projects\DE-EE (E10)\WR\W10-487 East Gwillimbury SWM Master Plan\Calcs\Hydrologic Models\Exis |
| Comments: |
-----

```

HOWEVER THE TABLE BELOW AND HYDROGRAPH FILES DO.

TIME	ACTUAL FLOW	PEAK FLOW	PEAK TIME	BASE FLOW	BASE TIME
hrs	cms	cms	hrs	cms	hrs
0.00	0.000	60.50	1.220	121.00	0.224
0.25	0.000	60.75	1.211	121.25	0.222
0.50	0.000	61.00	1.202	121.50	0.221
0.75	0.000	61.25	1.193	121.75	0.219
1.00	0.000	61.50	1.184	122.00	0.217
1.25	0.000	61.75	1.175	122.25	0.216
1.50	0.009	62.00	1.167	122.50	0.214
1.75	0.086	62.25	1.158	122.75	0.213
2.00	0.151	62.50	1.150	123.00	0.211
2.25	0.225	62.75	1.141	123.25	0.210
2.50	0.308	63.00	1.134	123.50	0.208
2.75	0.376	63.25	1.125	123.75	0.207
3.00	0.434	63.50	1.117	124.00	0.205
3.25	0.489	63.75	1.109	124.25	0.204
3.50	0.587	64.00	1.102	124.50	0.202
3.75	0.659	64.25	1.094	124.75	0.201
4.00	0.733	64.50	1.086	125.00	0.199
4.25	0.809	64.75	1.078	125.25	0.198
4.50	1.035	65.00	1.071	125.50	0.197
4.75	1.292	65.25	1.063	125.75	0.195
5.00	1.580	65.50	1.056	126.00	0.194
5.25	2.088	65.75	1.048	126.25	0.192
5.50	2.676	66.00	1.041	126.50	0.191
5.75	5.159	66.25	1.034	126.75	0.190
6.00	13.985	66.50	1.026	127.00	0.188
6.25	19.434	66.75	1.019	127.25	0.187
6.50	26.698	67.00	1.012	127.50	0.186
6.75	32.148	67.25	1.005	127.75	0.184
7.00	36.907	67.50	0.998	128.00	0.183
7.25	40.996	67.75	0.991	128.25	0.182
7.50	44.902	68.00	0.984	128.50	0.180
7.75	48.789	68.25	0.977	128.75	0.179
8.00	52.548	68.50	0.971	129.00	0.178
8.25	56.278	68.75	0.964	129.25	0.177
8.50	59.755	69.00	0.957	129.50	0.175
8.75	63.043	69.25	0.951	129.75	0.174
9.00	66.178	69.50	0.944	130.00	0.173
9.25	69.181	69.75	0.938	130.25	0.172
9.50	72.043	70.00	0.931	130.50	0.170
9.75	74.753	70.25	0.925	130.75	0.169
10.00	77.303	70.50	0.918	131.00	0.168
10.25	79.561	70.75	0.912	131.25	0.167
10.50	81.583	71.00	0.906	131.50	0.165
10.75	83.345	71.25	0.899	131.75	0.164
11.00	84.880	71.50	0.893	132.00	0.163
11.25	86.196	71.75	0.887	132.25	0.162
11.50	87.285	72.00	0.881	132.50	0.161
11.75	88.155	72.25	0.875	132.75	0.160
12.00	88.824	72.50	0.869	133.00	0.158
12.25	89.135	72.75	0.863	133.25	0.157
12.50	89.186	73.00	0.857	133.50	0.156
12.75	88.956	73.25	0.851	133.75	0.155
13.00	88.498	73.50	0.845	134.00	0.154
13.25	87.846	73.75	0.840	134.25	0.153
13.50	87.025	74.00	0.834	134.50	0.152
13.75	86.054	74.25	0.828	134.75	0.151
14.00	84.951	74.50	0.822	135.00	0.150
14.25	83.731	74.75	0.817	135.25	0.148
14.50	82.407	75.00	0.811	135.50	0.147
14.75	80.992	75.25	0.806	135.75	0.146
15.00	79.495	75.50	0.800	136.00	0.145
15.25	77.926	75.75	0.794	136.25	0.144
15.50	76.295	76.00	0.789	136.50	0.143
15.75	74.614	76.25	0.784	136.75	0.142
16.00	72.873	76.50	0.778	137.00	0.141
16.25	71.053	76.75	0.773	137.25	0.140

16.50	69.168	77.00	0.768	137.50	0.139
16.75	67.246	77.25	0.762	137.75	0.138
17.00	65.304	77.50	0.757	138.00	0.137
17.25	63.353	77.75	0.752	138.25	0.136
17.50	61.400	78.00	0.747	138.50	0.135
17.75	59.456	78.25	0.741	138.75	0.134
18.00	57.532	78.50	0.736	139.00	0.133
18.25	55.640	78.75	0.731	139.25	0.132
18.50	53.784	79.00	0.726	139.50	0.131
18.75	51.966	79.25	0.721	139.75	0.130
19.00	50.189	79.50	0.716	140.00	0.129
19.25	48.456	79.75	0.711	140.25	0.128
19.50	46.768	80.00	0.707	140.50	0.127
19.75	45.128	80.25	0.702	140.75	0.126
20.00	43.534	80.50	0.697	141.00	0.125
20.25	41.988	80.75	0.692	141.25	0.125
20.50	40.488	81.00	0.687	141.50	0.124
20.75	39.034	81.25	0.683	141.75	0.123
21.00	37.622	81.50	0.678	142.00	0.122
21.25	36.254	81.75	0.673	142.25	0.121
21.50	34.929	82.00	0.669	142.50	0.120
21.75	33.648	82.25	0.664	142.75	0.119
22.00	32.409	82.50	0.660	143.00	0.118
22.25	31.212	82.75	0.655	143.25	0.117
22.50	30.057	83.00	0.651	143.50	0.117
22.75	28.941	83.25	0.646	143.75	0.116
23.00	27.865	83.50	0.642	144.00	0.115
23.25	26.828	83.75	0.637	144.25	0.114
23.50	25.829	84.00	0.633	144.50	0.113
23.75	24.868	84.25	0.628	144.75	0.112
24.00	23.948	84.50	0.624	145.00	0.112
24.25	23.066	84.75	0.620	145.25	0.111
24.50	22.220	85.00	0.616	145.50	0.110
24.75	21.407	85.25	0.611	145.75	0.109
25.00	20.625	85.50	0.607	146.00	0.108
25.25	19.873	85.75	0.603	146.25	0.108
25.50	19.150	86.00	0.599	146.50	0.107
25.75	18.455	86.25	0.595	146.75	0.106
26.00	17.785	86.50	0.591	147.00	0.105
26.25	17.142	86.75	0.587	147.25	0.105
26.50	16.523	87.00	0.583	147.50	0.104
26.75	15.927	87.25	0.578	147.75	0.103
27.00	15.352	87.50	0.574	148.00	0.102
27.25	14.804	87.75	0.571	148.25	0.102
27.50	14.275	88.00	0.567	148.50	0.101
27.75	13.766	88.25	0.563	148.75	0.100
28.00	13.276	88.50	0.559	149.00	0.099
28.25	12.807	88.75	0.555	149.25	0.099
28.50	12.355	89.00	0.551	149.50	0.098
28.75	11.920	89.25	0.547	149.75	0.097
29.00	11.502	89.50	0.544	150.00	0.096
29.25	11.101	89.75	0.540	150.25	0.096
29.50	10.715	90.00	0.536	150.50	0.095
29.75	10.345	90.25	0.532	150.75	0.094
30.00	9.989	90.50	0.529	151.00	0.094
30.25	9.647	90.75	0.525	151.25	0.093
30.50	9.320	91.00	0.521	151.50	0.092
30.75	9.005	91.25	0.518	151.75	0.092
31.00	8.703	91.50	0.514	152.00	0.091
31.25	8.414	91.75	0.511	152.25	0.090
31.50	8.136	92.00	0.507	152.50	0.090
31.75	7.869	92.25	0.504	152.75	0.089
32.00	7.614	92.50	0.500	153.00	0.088
32.25	7.369	92.75	0.497	153.25	0.088
32.50	7.135	93.00	0.493	153.50	0.087
32.75	6.910	93.25	0.490	153.75	0.086
33.00	6.695	93.50	0.486	154.00	0.085
33.25	6.488	93.75	0.483	154.25	0.085
33.50	6.293	94.00	0.480	154.50	0.085
33.75	6.101	94.25	0.476	154.75	0.084
34.00	5.919	94.50	0.473	155.00	0.083
34.25	5.745	94.75	0.470	155.25	0.083
34.50	5.576	95.00	0.466	155.50	0.082
34.75	5.418	95.25	0.463	155.75	0.082
35.00	5.264	95.50	0.460	156.00	0.081
35.25	5.117	95.75	0.457	156.25	0.080
35.50	4.975	96.00	0.454	156.50	0.080
35.75	4.840	96.25	0.450	156.75	0.079
36.00	4.710	96.50	0.447	157.00	0.079
36.25	4.585	96.75	0.444	157.25	0.078
36.50	4.465	97.00	0.441	157.50	0.077
36.75	4.349	97.25	0.438	157.75	0.077
37.00	4.239	97.50	0.435	158.00	0.076
37.25	4.133	97.75	0.432	158.25	0.075
37.50	4.031	98.00	0.429	158.50	0.075
37.75	3.933	98.25	0.426	158.75	0.075
38.00	3.838	98.50	0.423	159.00	0.074
38.25	3.748	98.75	0.420	159.25	0.074
38.50	3.661	99.00	0.417	159.50	0.073
38.75	3.577	99.25	0.414	159.75	0.073
39.00	3.497	99.50	0.411	160.00	0.072
39.25	3.419	99.75	0.408	160.25	0.071
39.50	3.345	100.00	0.406	160.50	0.071
39.75	3.273	100.25	0.403	160.75	0.070
40.00	3.204	100.50	0.400	161.00	0.070

Table with columns for time, flow, and various parameters. Includes a note: 'NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.' at the bottom.

Summary table for ADD HYD (8188) with columns: AREA, QPEAK, TPEAK, R.V. Includes a note: 'NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.' at the bottom.

Summary table for CALIB (4462) with columns: Area, Curve Number, Peak Flow, Time to Peak, Runoff Volume, Total Rainfall, Runoff Coefficient. Includes a note: '(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.'

Summary table for CALIB (4461) with columns: Area, Total Imp, Dir. Com, Surface Area, Dep. Storage, etc. Includes a note: '***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!' and a note: '(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.'

Summary table for RESERVOIR (5446) with columns: INFLOW, OUTFLOW, STORAGE. Includes a note: '(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.'

Summary table for ADD HYD (8192) with columns: AREA, QPEAK, TPEAK, R.V.

Summary table for ADD HYD (8194) with columns: AREA, QPEAK, TPEAK, R.V. Includes a note: 'NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.'

ID = 3 (8206): 244.15 3.181 2.50 17.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHYD (0442) Area (ha)= 117.26 Curve Number (CN)= 43.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.17

Unit Hyd Qpeak (cms)= 1.705
PEAK FLOW (cms)= 0.744 (i)
TIME TO PEAK (hrs)= 3.250
RUNOFF VOLUME (mm)= 12.911
TOTAL RAINFALL (mm)= 78.027
RUNOFF COEFFICIENT = 0.165

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0440) Area (ha)= 226.35 Curve Number (CN)= 60.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.11

Unit Hyd Qpeak (cms)= 3.481
PEAK FLOW (cms)= 2.542 (i)
TIME TO PEAK (hrs)= 3.250
RUNOFF VOLUME (mm)= 21.812
TOTAL RAINFALL (mm)= 78.027
RUNOFF COEFFICIENT = 0.280

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0438) Area (ha)= 130.70 Curve Number (CN)= 51.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.60

Unit Hyd Qpeak (cms)= 3.738
PEAK FLOW (cms)= 1.769 (i)
TIME TO PEAK (hrs)= 2.250
RUNOFF VOLUME (mm)= 16.452
TOTAL RAINFALL (mm)= 78.027
RUNOFF COEFFICIENT = 0.211

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0436) Area (ha)= 187.51 Curve Number (CN)= 55.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.73

Unit Hyd Qpeak (cms)= 4.391
PEAK FLOW (cms)= 2.485 (i)
TIME TO PEAK (hrs)= 2.500
RUNOFF VOLUME (mm)= 18.679
TOTAL RAINFALL (mm)= 78.027
RUNOFF COEFFICIENT = 0.239

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0434) Area (ha)= 56.64 Curve Number (CN)= 46.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.52

Unit Hyd Qpeak (cms)= 1.867
PEAK FLOW (cms)= 0.720 (i)
TIME TO PEAK (hrs)= 2.250
RUNOFF VOLUME (mm)= 13.979
TOTAL RAINFALL (mm)= 78.027
RUNOFF COEFFICIENT = 0.179

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8206)
1 + 2 = 3
ID1= 1 (0434): 56.64 0.720 2.25 13.98
+ ID2= 2 (0436): 187.51 2.485 2.50 18.68

ROUTE CHN (6438) |
IN= 2--> OUT= 1 | Routing time step (min)= 15.00

Table with 3 columns: Distance, Elevation, Manning. Rows show data for various points along the route.

TRAVEL TIME TABLE
DEPTH (m), ELEV (m), VOLUME (cu.m.), FLOW RATE (cms), VELOCITY (m/s), TRAV. TIME (min)
0.50 245.80 .221E+04 0.9 3.02 40.44

hydrograph
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
INFLOW: ID= 2 (8206) 244.15 3.18 2.50 17.59 0.59 1.02
OUTFLOW: ID= 1 (6438) 244.15 2.82 3.25 17.59 0.57 1.02

ADD HYD (8208)
1 + 2 = 3
ID1= 1 (0438): 130.70 1.769 2.25 16.45
+ ID2= 2 (6438): 244.15 2.822 3.25 17.59
ID = 3 (8208): 374.85 4.370 3.00 17.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8210)
1 + 2 = 3
ID1= 1 (0440): 226.35 2.542 3.25 21.81
+ ID2= 2 (8208): 374.85 4.370 3.00 17.19
ID = 3 (8210): 601.20 6.909 3.00 18.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHYD (0432) Area (ha)= 114.21 Curve Number (CN)= 53.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.21

Unit Hyd Qpeak (cms) = 1.617

PEAK FLOW (cms) = 0.977 (1)
TIME TO PEAK (hrs) = 3.250
RUNOFF VOLUME (mm) = 17.741
TOTAL RAINFALL (mm) = 78.027
RUNOFF COEFFICIENT = 0.227

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0430) Area (ha) = 111.64 Curve Number (CN) = 55.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U.H. Tp(hrs) = 0.52

Unit Hyd Qpeak (cms) = 3.687

PEAK FLOW (cms) = 1.890 (1)
TIME TO PEAK (hrs) = 2.250
RUNOFF VOLUME (mm) = 18.476
TOTAL RAINFALL (mm) = 78.027
RUNOFF COEFFICIENT = 0.237

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0428) Area (ha) = 50.53 Curve Number (CN) = 51.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U.H. Tp(hrs) = 0.50

Unit Hyd Qpeak (cms) = 1.725

PEAK FLOW (cms) = 0.774 (1)
TIME TO PEAK (hrs) = 2.000
RUNOFF VOLUME (mm) = 15.342
TOTAL RAINFALL (mm) = 78.027
RUNOFF COEFFICIENT = 0.209

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0426) Area (ha) = 247.17 Curve Number (CN) = 61.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U.H. Tp(hrs) = 0.98

Unit Hyd Qpeak (cms) = 4.315

PEAK FLOW (cms) = 3.146 (1)
TIME TO PEAK (hrs) = 3.000
RUNOFF VOLUME (mm) = 22.414
TOTAL RAINFALL (mm) = 78.027
RUNOFF COEFFICIENT = 0.287

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0424) Area (ha) = 49.57 Curve Number (CN) = 59.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U.H. Tp(hrs) = 0.37

Unit Hyd Qpeak (cms) = 2.293

PEAK FLOW (cms) = 1.198 (1)
TIME TO PEAK (hrs) = 2.000
RUNOFF VOLUME (mm) = 20.426
TOTAL RAINFALL (mm) = 78.027
RUNOFF COEFFICIENT = 0.262

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8198) AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0424): 49.57 1.198 2.00 20.43
+ ID2= 2 (0426): 247.17 3.146 3.00 22.41
=====

ID = 3 (8198): 296.74 4.079 2.50 22.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6428) Routing time step (min) = 15.00
IN= 2--> OUT= 1

----- DATA FOR SECTION (4281.0) -----

Distance	Elevation	Manning
0.00	246.54	0.0900
8.80	246.29	0.0900
30.78	246.03	0.0900
41.78	243.97	0.0900
63.77	237.16	0.0900
85.76	232.53	0.0900
96.75	230.16	0.0900
109.94	228.40	0.0900
112.14	228.28	0.0900 / 0.0450
112.84	228.00	0.0450
113.34	227.50	0.0450
114.34	227.50	0.0450
114.84	228.00	0.0450
116.84	228.29	0.0450 / 0.0900
118.74	228.40	0.0900
120.94	228.52	0.0900
149.52	229.51	0.0900
169.31	230.69	0.0900
186.00	234.27	0.0900
217.69	243.56	0.0900

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.78	228.28	205E+04	1.6	0.97	21.60
1.58	229.08	202E+05	16.6	1.02	20.37
2.39	229.89	665E+05	64.8	1.22	17.11
3.19	230.69	133E+06	158.1	1.49	14.05
4.00	231.50	213E+06	308.6	1.82	11.48
4.80	232.30	299E+06	504.9	2.11	9.89
5.61	233.11	394E+06	746.6	2.37	8.79
6.41	233.91	495E+06	1034.5	2.61	7.99
7.21	234.71	605E+06	1373.3	2.84	7.35
8.02	235.52	722E+06	1763.3	3.06	6.82
8.82	236.32	844E+06	2202.0	3.27	6.39
9.63	237.13	974E+06	2690.5	3.46	6.03
10.43	237.93	111E+07	3239.5	3.66	5.71
11.23	238.73	125E+07	3839.0	3.84	5.43
12.04	239.54	140E+07	4489.1	4.03	5.18
12.84	240.34	155E+07	5190.5	4.20	4.97
13.65	241.15	170E+07	5943.8	4.36	4.78
14.45	241.95	187E+07	6749.8	4.53	4.61
15.26	242.76	204E+07	7609.4	4.68	4.46

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8198)	296.74	4.08	2.50	22.08	0.91	0.98
OUTFLOW : ID= 1 (6428)	296.74	3.95	3.00	22.08	0.91	0.97

ADD HYD (8200)
1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0428):	50.53	0.774	2.00	16.34
+ ID2= 2 (6428):	296.74	3.946	3.00	22.08
=====				
ID = 3 (8200):	347.27	4.611	2.75	21.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8202)
1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0430):	111.64	1.890	2.25	18.48
+ ID2= 2 (8200):	347.27	4.611	2.75	21.25
=====				
ID = 3 (8202):	458.91	6.336	2.50	20.57

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6432) Routing time step (min) = 15.00
IN= 2--> OUT= 1

----- DATA FOR SECTION (4321.0) -----

Distance	Elevation	Manning
0.00	221.00	0.0600
15.29	219.85	0.0600
45.86	219.86	0.0600
76.44	219.96	0.0600
110.84	220.23	0.0600
112.00	220.15	0.0600
114.66	220.09	0.0600
118.48	219.73	0.0600

120.80	219.70	0.0600 / 0.0350	Main Channel
121.30	219.20	0.0350	Main Channel
122.30	219.20	0.0350	Main Channel
122.80	219.70	0.0350 / 0.0600	Main Channel
129.95	220.11	0.0600	
175.81	220.24	0.0600	
214.03	220.55	0.0600	
252.25	220.61	0.0600	
290.47	220.70	0.0600	
328.69	221.04	0.0600	
347.80	221.08	0.0600	
378.37	221.04	0.0600	

TRAVEL TIME TABLE					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.08	219.28	185E+01	0.0	0.23	149.38
0.17	219.37	399E+03	0.1	0.34	100.62
0.25	219.45	641E+03	0.1	0.42	80.98
0.33	219.53	911E+03	0.2	0.49	69.79
0.42	219.62	121E+04	0.3	0.55	62.35
0.50	219.70	154E+04	0.5	0.60	56.91
0.61	219.81	267E+04	0.7	0.56	61.28
0.71	219.91	301E+04	1.3	0.30	112.96
0.82	220.02	249E+05	3.4	0.28	122.71
0.92	220.12	452E+05	6.8	0.31	109.98
1.03	220.23	747E+05	12.2	0.33	102.05
1.14	220.34	112E+06	21.3	0.39	87.59
1.24	220.44	153E+06	33.0	0.44	77.01
1.35	220.55	196E+06	47.2	0.49	69.22
1.46	220.66	248E+06	62.3	0.51	66.48
1.56	220.76	310E+06	83.4	0.55	62.06
1.67	220.87	376E+06	108.8	0.59	57.56
1.77	220.97	444E+06	137.7	0.64	53.77
1.88	221.08	517E+06	163.6	0.65	52.70

hydrograph							<-pipe / channel->		
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL				
(ha)	(cms)	(mm)	(m)	(m/s)					
INFLOW : ID= 2 (8202)	458.91	6.34	2.50	20.57	0.91	0.31			
OUTFLOW: ID= 1 (6432)	458.91	4.18	4.50	20.57	0.84	0.29			

ADD HYD (8204)						
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (0432):	114.21	0.977	3.25	17.74		
+ ID2= 2 (6432):	458.91	4.184	4.50	20.57		
ID = 3 (8204):	573.12	5.099	4.25	20.01		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8212)						
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (8204):	573.12	5.099	4.25	20.01		
+ ID2= 2 (8210):	601.20	6.909	3.00	18.93		
ID = 3 (8212):	1174.32	11.471	3.50	19.46		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6442)
 IIR= 2--> ODF= 1
 Routing time step (min)'= 15.00

DATA FOR SECTION (4421.0)			
Distance	Elevation	Manning	
0.00	221.00	0.0350	
26.73	220.80	0.0350	
53.47	220.60	0.0350	
66.83	220.40	0.0350	
80.20	220.20	0.0350	
347.52	220.00	0.0350	
354.21	219.59	0.0350	
360.89	219.50	0.0350	
367.57	219.08	0.0350	
368.76	219.00	0.0350 / 0.0300	Main Channel
369.26	218.50	0.0300	Main Channel
374.26	218.50	0.0300	Main Channel
374.76	219.00	0.0300 / 0.0350	Main Channel
380.94	220.33	0.0350	
387.62	219.62	0.0350	
394.31	219.23	0.0350	
454.45	219.07	0.0350	
514.60	219.09	0.0350	
660.00	219.19	0.0350	

661.63 221.00 0.0350

TRAVEL TIME TABLE					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.12	218.62	100E+04	0.2	0.28	92.89
0.25	218.75	205E+04	0.6	0.43	60.09
0.37	218.87	315E+04	1.1	0.55	46.99
0.50	219.00	430E+04	1.8	0.66	39.67
0.63	219.13	541E+04	3.4	0.37	69.99
0.77	219.27	652E+04	5.1	0.33	79.14
0.90	219.40	763E+04	6.8	0.45	58.02
1.03	219.53	874E+04	8.5	0.56	46.45
1.17	219.67	985E+04	10.3	0.66	39.30
1.30	219.80	1096E+04	12.0	0.76	34.30
1.43	219.93	1207E+04	13.7	0.85	30.65
1.57	220.07	1318E+04	15.4	0.91	28.56
1.70	220.20	1429E+04	17.1	0.94	27.70
1.83	220.33	1540E+04	18.8	0.98	26.62
1.97	220.47	1651E+04	20.5	1.03	25.21
2.10	220.60	1762E+04	22.2	1.09	23.82
2.23	220.73	1873E+04	23.9	1.15	22.62
2.37	220.87	1984E+04	25.6	1.21	21.54
2.50	221.00	2095E+04	27.3	1.27	20.57

hydrograph							<-pipe / channel->		
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL				
(ha)	(cms)	(mm)	(m)	(m/s)					
INFLOW : ID= 2 (8212)	1174.32	11.47	3.50	19.46	0.74	0.34			
OUTFLOW: ID= 1 (6442)	1174.32	9.73	5.00	19.46	0.72	0.34			

ADD HYD (8214)						
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (0442):	117.26	0.744	3.25	12.91		
+ ID2= 2 (6442):	1174.32	9.734	5.00	19.46		
ID = 3 (8214):	1291.58	10.359	4.75	18.86		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8196)						
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (8194):	25632.58	138.238	8.75	26.52		
+ ID2= 2 (8214):	1291.58	10.359	4.75	18.86		
ID = 3 (8196):	26924.15	143.507	8.50	26.16		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0444)				
Area	(ha)	=	221.65	Curve Number (CN)= 56.0
Ia	(mm)	=	5.00	# of Linear Res.(N)= 1.50
U.H. Tp	(hrs)	=	1.03	

Unit Hyd Qpeak (cms)= 3.656

PEAK FLOW (cms)= 2.331 (i)
 TIME TO PEAK (hrs)= 3.000
 RUNOFF VOLUME (mm)= 19.374
 TOTAL RAINFALL (mm)= 78.027
 RUNOFF COEFFICIENT = 0.248

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8216)						
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (0444):	221.65	2.331	3.00	19.37		
+ ID2= 2 (8196):	26924.15	143.507	8.50	26.16		
ID = 3 (8216):	27145.80	143.968	8.50	26.10		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

**EXISTING
EAST HOLLAND RIVER 10-YEAR STORM**

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V V I SSSS U U A L
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
W V I SSSS UUUU A A LLLL

OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y MM MM O O
O O T T H H Y Y M M O O
OOO T T H H Y Y M M OOO Company Serial
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***** DETAILED OUTPUT *****

Input filename: C:\Program Files\Visual Ortymo 2.4r\VO2\voim.dat
Output filename: C:\Users\jscott\AppData\Local\Temp\3371e7f9-0f83-418a-a026-a8db992e563c\Scenario.out
Summary filename: C:\Users\jscott\AppData\Local\Temp\3371e7f9-0f83-418a-a026-a8db992e563c\Scenario.sum

DATE: 08/22/2012 TIME: 01:25:56

USER:

COMMENTS: _____

** SIMULATION NUMBER: 10 **

READ STORM Filename: C:\Users\jscott\AppData\Local\Temp\3371e7f9-0f83-418a-a026-a8db992e563c\4f31470
Total= 62.70 mm Comments: 10-Year 12-Hour SCS II Design Storm

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.57	3.25	2.51	6.25	11.29	9.25	2.19
0.50	1.57	3.50	2.51	6.50	11.29	9.50	2.19
0.75	1.57	3.75	2.51	6.75	5.02	9.75	2.19
1.00	1.57	4.00	2.51	7.00	5.02	10.00	2.19
1.25	1.57	4.25	3.76	7.25	3.76	10.25	1.25
1.50	1.57	4.50	3.76	7.50	3.76	10.50	1.25
1.75	1.57	4.75	5.02	7.75	3.76	10.75	1.25
2.00	1.57	5.00	5.02	8.00	3.76	11.00	1.25
2.25	1.88	5.25	7.52	8.25	2.19	11.25	1.25
2.50	1.88	5.50	7.52	8.50	2.19	11.50	1.25
2.75	1.88	5.75	30.10	8.75	2.19	11.75	1.25
3.00	1.88	6.00	82.76	9.00	2.19	12.00	1.25

CALIB STANDBYD (1662) Area (ha)= 158.10
ID= 1 DT=15.0 min Total Imp(%)= 58.00 Dir. Conn.(%)= 30.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 91.70 66.40
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1026.65 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 82.76 79.08
over (min) 15.00 30.00
Storage Coeff. (min)= 13.72 (ii) 25.47 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04
TOTALS
PEAK FLOW (cms)= 8.24 6.63 12.499 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 60.70 30.71 39.70
TOTAL RAINFALL (mm)= 62.70 62.70
RUNOFF COEFFICIENT = 0.97 0.49 0.63

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (1661) Area (ha)= 276.60
ID= 1 DT=15.0 min Total Imp(%)= 40.00 Dir. Conn.(%)= 21.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 110.64 165.96
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1357.94 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 82.76 54.12
over (min) 15.00 30.00
Storage Coeff. (min)= 16.23 (ii) 29.90 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.07 0.04
TOTALS
PEAK FLOW (cms)= 9.40 10.34 15.889 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 60.70 26.64 33.79
TOTAL RAINFALL (mm)= 62.70 62.70
RUNOFF COEFFICIENT = 0.97 0.42 0.54

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5166)
IN= 2--> OUT= 1
DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	6.0870	6.1196
	0.3730	3.2753	7.7230	7.0042
	1.3370	3.4678	9.3080	7.8876
	2.9800	4.4433	9.7080	7.8976
	4.4520	5.2716	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (1661)	276.600	15.889	6.00	33.79
OUTFLOW: ID= 1 (5166)	276.600	4.354	7.25	33.78

PEAK FLOW REDUCTION [Qout/Qin](%)= 27.41
TIME SHIFT OF PEAK FLOW (min)= 75.00
MAXIMUM STORAGE USED (ha.m.)= 5.2233

ADD HYD (8134)
1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (1662):	158.10	12.499	6.00	39.70
+ ID2= 2 (5166):	276.60	4.354	7.25	33.78
ID = 3 (8134):	434.70	12.732	6.00	35.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDBYD (1642) Area (ha)= 132.50
ID= 1 DT=15.0 min Total Imp(%)= 20.00 Dir. Conn.(%)= 15.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 26.50 106.00
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 939.86 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 82.76 22.23
over (min) 15.00 45.00
Storage Coeff. (min)= 13.01 (ii) 32.53 (ii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.08 0.03
TOTALS
PEAK FLOW (cms)= 3.53 3.88 5.137 (iii)
TIME TO PEAK (hrs)= 6.00 6.50 6.00
RUNOFF VOLUME (mm)= 60.70 22.28 28.04
TOTAL RAINFALL (mm)= 62.70 62.70
RUNOFF COEFFICIENT = 0.97 0.36 0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----
| CALIB          |
| STANDHYD (1641) | Area (ha)= 101.70
| ID= 1 DT=15.0 min | Total Imp(%)= 70.00 Dir. Conn.(%)= 52.00
|-----|
```

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	71.19	30.51
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	823.41	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	82.76	72.24
over (min)	15.00	30.00
Storage Coeff. (min)	12.02 (ii)	24.20 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04
PEAK FLOW (cms)	9.66	2.85
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	60.70	29.26
TOTAL RAINFALL (mm)	62.70	62.70
RUNOFF COEFFICIENT	0.97	0.47

TOTALS
 PEAK FLOW (cms) = 11.484 (iii)
 TIME TO PEAK (hrs) = 6.00
 RUNOFF VOLUME (mm) = 45.61
 TOTAL RAINFALL (mm) = 62.70
 RUNOFF COEFFICIENT = 0.73

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----
| RESERVOIR (5164) |
| IIR 2--> OUT= 1 |
| DT= 15.0 min    |
|-----|
```

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	7.4100	4.1638
0.3440	1.7056	8.2090	4.5053
3.1340	1.9730	8.8070	4.8468
4.1310	3.0607	9.2070	4.8568
5.1070	3.7142	0.0000	0.0000

INFLOW : ID= 2 (1641)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	101.700	11.484	6.00	45.61
OUTFLOW: ID= 1 (5164)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	101.700	3.502	6.50	45.60

PEAK FLOW REDUCTION [Qout/Qin](%) = 30.49
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 2.4106

```
-----
| ADD HYD (8132) |
| 1 + 2 = 3      |
|-----|
```

ID#	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 1 (1642):	132.50	5.137	6.00	28.04
+ ID2 = 2 (5164):	101.70	3.502	6.50	45.60
=====				
ID = 3 (8132):	234.20	8.292	6.50	35.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
-----
| CALIB          |
| NASHYD (0162) | Area (ha)= 190.14 Curve Number (CN)= 64.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.79
```

Unit Hyd Qpeak (cms) = 4.104
 PEAK FLOW (cms) = 1.735 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 16.359
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.261

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----
| CALIB          |
| NASHYD (0160) | Area (ha)= 318.82 Curve Number (CN)= 61.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.45
```

Unit Hyd Qpeak (cms) = 3.754
 PEAK FLOW (cms) = 1.665 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 15.038
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.240

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----
| CALIB          |
| NASHYD (0152) | Area (ha)= 124.37 Curve Number (CN)= 59.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.89
```

Unit Hyd Qpeak (cms) = 2.372
 PEAK FLOW (cms) = 0.875 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 14.044
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.224

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----
| CALIB          |
| NASHYD (0148) | Area (ha)= 417.89 Curve Number (CN)= 26.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.18
```

Unit Hyd Qpeak (cms) = 6.067
 PEAK FLOW (cms) = 0.705 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 4.231
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.067

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----
| CALIB          |
| NASHYD (0150) | Area (ha)= 105.88 Curve Number (CN)= 17.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.58
```

Unit Hyd Qpeak (cms) = 3.138
 PEAK FLOW (cms) = 0.181 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 2.506
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.040

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
-----
| ADD HYD (8116) |
| 1 + 2 = 3      |
|-----|
```

ID#	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 1 (0148):	417.89	0.705	8.00	4.23
+ ID2 = 2 (0150):	105.88	0.181	6.75	2.51
=====				
ID = 3 (8116):	523.77	0.854	7.50	3.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
-----
| ROUTE CHN (6152) |
| IIR 2--> OUT= 1 |
|-----|
| Routing time step (min)'= 15.00
```

```
<----- DATA FOR SECTION (1521.0) ----->
Distance      Elevation      Manning
-----
15.34         286.87          0.0500
46.03         284.84          0.0500
88.22         283.07          0.0500
126.57        282.11          0.0500
153.42        280.34          0.0500
157.26        280.07          0.0500
161.09        279.72          0.0500
```


167.76	278.94	0.0500	/0.0300	Main Channel
168.01	278.60	0.0300		Main Channel
168.76	278.60	0.0300		Main Channel
169.51	278.60	0.0300		Main Channel
169.76	278.90	0.0300	/0.0500	Main Channel
173.76	279.40	0.0500		
180.27	279.69	0.0500		
184.11	279.93	0.0500		
218.62	281.86	0.0500		
260.82	283.20	0.0500		
360.54	289.25	0.0500		
379.72	289.80	0.0500		

TRAVEL TIME TABLE					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.30	278.90	1148+04	0.4	0.80	45.70
0.87	279.47	932E+04	4.2	0.98	37.22
1.45	280.05	353E+05	17.5	1.09	33.68
2.02	280.62	831E+05	47.9	1.26	29.01
2.59	281.19	155E+06	102.9	1.46	25.06
3.17	281.77	250E+06	189.3	1.66	22.03
3.74	282.34	375E+06	298.5	1.75	20.91
4.32	282.92	547E+06	456.5	1.83	19.98
4.89	283.49	767E+06	705.7	2.02	18.12
5.46	284.06	102E+07	1038.0	2.24	16.34
6.04	284.64	130E+07	1444.1	2.45	14.98
6.61	285.21	161E+07	1940.1	2.66	13.79
7.18	285.78	194E+07	2522.6	2.86	12.80
7.76	286.36	229E+07	3187.4	3.06	11.98
8.33	286.93	267E+07	3942.3	3.25	11.28
8.91	287.51	306E+07	4824.9	3.46	10.58
9.48	288.08	347E+07	5795.7	3.67	9.99
10.05	288.65	390E+07	6855.4	3.86	9.48
10.63	289.23	434E+07	8005.0	4.05	9.04

hydrograph						<-pipe / channel-->		
INFLOW	ID= 2 (8116)	523.77	0.85	7.50	3.88	0.37	0.82	
OUTFLOW	ID= 1 (6152)	523.77	0.80	8.50	3.88	0.36	0.82	

ADD HYD (8122)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
	124.37	0.875	7.25	14.04
+ ID2= 2 (6152):	523.77	0.800	8.50	3.88
=====				
ID = 3 (8122):	648.14	1.610	8.00	5.83

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB				
NASHVD	(0158)	Area	(ha)= 178.59	Curve Number (CN)= 61.0
ID= 1 DT=15.0 min		Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50

Unit Hyd Qpeak (cms)= 3.230

PEAK FLOW (cms)= 1.287 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 14.959
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.239

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB				
NASHVD	(0156)	Area	(ha)= 83.49	Curve Number (CN)= 45.0
ID= 1 DT=15.0 min		Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50

Unit Hyd Qpeak (cms)= 2.306

PEAK FLOW (cms)= 0.488 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 8.855
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.141

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB				
NASHVD	(0154)	Area	(ha)= 200.32	Curve Number (CN)= 37.0
ID= 1 DT=15.0 min		Ia	(mm)= 5.00	# of Linear Res.(N)= 1.50

U.H. Tp(hrs)= 0.97

Unit Hyd Qpeak (cms)= 3.537

PEAK FLOW (cms)= 0.622 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 6.719
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.107

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8118)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0154):	200.32	0.622	7.25	6.72
+ ID2= 2 (0156):	83.49	0.488	6.75	8.86
=====				
ID = 3 (8118):	283.81	1.094	7.00	7.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6158)		ROUTING time step (min)'= 15.00
IN= 2-->	OUT= 1	

DATA FOR SECTION (1581.0)			
Distance	Elevation	Manning	
0.00	290.00	0.0400	
19.20	288.88	0.0400	
48.00	288.26	0.0400	
119.99	283.68	0.0400	
167.99	282.07	0.0400	
196.79	281.32	0.0400	
210.79	280.90	0.0400	
219.79	280.38	0.0400 /0.0300	Main Channel
220.04	279.65	0.0300	Main Channel
220.79	279.65	0.0300	Main Channel
221.54	279.65	0.0300	Main Channel
221.79	280.39	0.0300 /0.0400	Main Channel
225.79	280.59	0.0400	
235.19	280.88	0.0400	
254.39	281.50	0.0400	
273.58	282.56	0.0400	
302.38	283.42	0.0400	
359.98	284.03	0.0400	
436.77	289.37	0.0400	
475.17	290.37	0.0400	

TRAVEL TIME TABLE					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.37	280.01	973E+03	0.5	0.86	31.78
0.73	280.38	210E+04	1.5	1.18	23.16
1.30	280.95	149E+05	8.6	0.95	28.91
1.86	281.51	579E+05	40.0	1.13	24.12
2.43	282.08	133E+06	118.5	1.46	18.75
2.99	282.64	236E+06	254.6	1.77	15.46
3.56	283.21	369E+06	450.3	2.00	13.67
4.13	283.78	542E+06	702.7	2.13	12.86
4.69	284.34	767E+06	1125.2	2.41	11.36
5.26	284.91	101E+07	1697.1	2.75	9.93
5.82	285.47	127E+07	2379.1	3.07	8.91
6.39	286.04	155E+07	3171.4	3.36	8.14
6.95	286.60	184E+07	4075.5	3.64	7.53
7.52	287.17	215E+07	5093.1	3.89	7.03
8.09	287.74	247E+07	6226.5	4.13	6.62
8.65	288.30	281E+07	7461.2	4.36	6.28
9.22	288.87	318E+07	8635.1	4.46	6.13
9.78	289.43	357E+07	10143.6	4.67	5.86
10.35	290.00	398E+07	11608.8	4.79	5.71

hydrograph							<-pipe / channel-->		
INFLOW	ID= 2 (8118)	283.81	1.09	7.00	7.35	0.58	1.02		
OUTFLOW	ID= 1 (6158)	283.81	1.05	7.50	7.35	0.56	1.01		

ADD HYD (8120)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0158):	178.59	1.287	7.25	14.96
+ ID2= 2 (6158):	283.81	1.048	7.50	7.35
=====				
ID = 3 (8120):	462.40	2.327	7.50	10.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8124)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8120):	462.40	2.327	7.50	10.29
+ ID2= 2 (8122):	648.14	1.610	8.00	5.83
=====				
ID = 3 (8124):	1110.54	3.906	7.75	7.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6160) | Routing time step (min)'= 15.00
 IN= 2--> OUF= 1 |

----- DATA FOR SECTION (1601.0) -----				
Distance	Elevation	Manning		
0.00	261.02	0.0400		
29.89	254.30	0.0400		
40.75	252.58	0.0400		
62.49	251.30	0.0400		
114.11	250.94	0.0400		
130.41	248.58	0.0400		
141.28	247.50	0.0400		
154.86	246.87	0.0400		
155.20	246.70	0.0400		
160.20	246.30	0.0400 / 0.0300	Main Channel	
160.30	245.30	0.0300	Main Channel	
165.20	245.30	0.0300	Main Channel	
165.30	246.30	0.0300 / 0.0400	Main Channel	
168.45	246.96	0.0400		
195.62	249.20	0.0400		
203.77	250.82	0.0400		
225.50	256.78	0.0400		
244.52	261.14	0.0400		
255.39	261.95	0.0400		
268.97	261.92	0.0400		

----- TRAVEL TIME TABLE -----					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.50	245.80	120E+05	2.9	1.18	68.31
1.00	246.30	242E+05	8.5	1.70	47.47
1.92	247.22	825E+05	31.7	1.86	43.34
2.84	248.14	248E+06	102.5	2.00	40.39
3.76	249.06	505E+06	247.7	2.37	34.03
4.68	249.98	832E+06	486.8	2.83	28.49
5.60	250.90	121E+07	814.9	3.27	24.71
6.52	251.82	180E+07	1157.1	3.11	25.97
7.44	252.74	253E+07	1840.2	3.52	22.92
8.36	253.66	331E+07	2727.1	3.98	20.25
9.29	254.59	414E+07	3782.6	4.43	18.22
10.21	255.51	499E+07	5010.7	4.86	16.61
11.13	256.43	589E+07	6397.4	5.26	15.33
12.05	257.35	681E+07	7932.8	5.64	14.31
12.97	258.27	777E+07	9621.2	5.99	13.46
13.89	259.19	877E+07	11469.4	6.33	12.74
14.81	260.11	900E+07	13478.4	6.66	12.12
15.73	261.03	109E+08	15651.7	6.97	11.58
16.65	261.95	120E+08	17492.5	7.07	11.42

----- hydrograph -----		<-pipe / channel->	
AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8124) 1110.54	3.91	7.75	7.69
OUTFLOW : ID= 1 (6160) 1110.54	3.39	9.00	7.69

ADD HYD (8126)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0160):	318.82	1.665	8.00	15.04
+ ID2= 2 (6160):	1110.54	3.389	9.00	7.69
=====				
ID = 3 (8126):	1429.36	5.020	8.75	9.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8128)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0162):	190.14	1.735	7.00	16.36
+ ID2= 2 (8126):	1429.36	5.020	8.75	9.33
=====				
ID = 3 (8128):	1619.50	6.381	8.50	10.15

ADD HYD (8130)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8128):	1619.50	6.381	8.50	10.15
+ ID2= 2 (8132):	234.20	8.292	6.50	35.66
=====				
ID = 3 (8130):	1853.70	12.213	6.50	13.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6166) | Routing time step (min)'= 15.00
 IN= 2--> OUF= 1 |

----- DATA FOR SECTION (1661.0) -----				
Distance	Elevation	Manning		
0.00	248.36	0.0500		
41.67	246.98	0.0500		
95.82	244.93	0.0500		
149.98	242.42	0.0500		
191.64	241.88	0.0500		
224.97	239.90	0.0500		
229.13	239.35	0.0500		
233.30	237.77	0.0500		
237.47	236.57	0.0500		
240.63	235.60	0.0500 / 0.0300	Main Channel	
241.63	234.10	0.0300	Main Channel	
247.13	234.10	0.0300	Main Channel	
247.53	235.60	0.0300 / 0.0500	Main Channel	
254.13	236.25	0.0500		
266.63	236.77	0.0500		
283.29	237.84	0.0500		
291.63	238.89	0.0500		
329.12	244.66	0.0500		
370.78	249.55	0.0500		
412.44	253.17	0.0500		

----- TRAVEL TIME TABLE -----					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.75	234.85	130E+05	6.0	1.36	36.45
1.50	235.60	276E+05	18.1	1.95	25.41
2.25	236.35	544E+05	40.0	2.18	22.69
3.00	237.10	117E+06	80.1	2.04	24.32
3.75	237.85	213E+06	148.9	2.07	23.89
4.50	238.60	334E+06	255.1	2.27	21.85
5.25	239.35	473E+06	395.8	2.49	19.90
6.00	240.10	631E+06	563.8	2.65	18.66
6.75	240.85	824E+06	773.2	2.79	17.77
7.50	241.60	106E+07	1037.9	2.92	16.97
8.26	242.36	135E+07	1326.7	2.92	16.94
9.01	243.11	173E+07	1758.5	3.01	16.44
9.76	243.86	217E+07	2285.9	3.13	15.81
10.51	244.61	265E+07	2909.5	3.26	15.17
11.26	245.36	318E+07	3610.1	3.37	14.67
12.01	246.11	377E+07	4417.4	3.48	14.21
12.76	246.86	441E+07	5348.0	3.60	13.75
13.51	247.61	512E+07	6394.0	3.71	13.34
14.26	248.36	589E+07	7580.3	3.82	12.95

----- hydrograph -----		<-pipe / channel->	
AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8130) 1853.70	12.21	6.50	13.38
OUTFLOW : ID= 1 (6166) 1853.70	10.51	7.50	13.38

ADD HYD (8136)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6166):	1853.70	10.511	7.50	13.38
+ ID2= 2 (8134):	434.70	12.732	6.00	35.94
=====				
ID = 3 (8136):	2288.40	17.785	6.75	17.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)=	81.20
STANDHYD (2021)	Total Imp(%)=	40.00	Dir. Conn.(%)= 24.00
ID= 1 DT=15.0 min			

Surface Area	(ha)=	IMPERVIOUS	PERVIOUS (i)
		36.48	54.72

Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 779.74 40.00
 Mannings n = 0.013 0.250
 Max.Eff.Inten.(mm/hr)= 82.76 50.74
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 11.63 (ii) 25.67 (iii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

 PEAK FLOW (cms)= 4.05 3.46 6.224 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 60.70 25.97 34.30
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.41 0.55

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR (5202)
 IN= 2--> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	4.0850	2.2999
	0.3100	0.9668	5.0850	2.6924
	1.2590	1.1373	5.9090	3.1012
	2.2140	1.5358	6.3090	3.1112
	3.0620	1.8918	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (2021)	91.200	6.224	6.00	34.30
OUTFLOW: ID= 1 (5202)	91.200	2.110	6.75	34.30

	PEAK FLOW (cms)	REDUCTION [Qout/Qin](%)	TIME SHIFT OF PEAK FLOW (min)	MAXIMUM STORAGE USED (ha.m.)
	3.46	33.90	45.00	1.5022

 CALIB
 STANDHYD (2022)
 ID= 1 DT=15.0 min
 Total Imp(%)= 57.00 Dir. Conn.(%)= 34.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	215.06	162.24
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1585.98	40.00
Mannings n	0.013	0.250

	Max.Eff.Inten.(mm/hr)	over (min)	Storage Coeff. (min)	Unit Hyd. Tpeak (min)	Unit Hyd. peak (cms)
	82.76	65.77	17.81 (ii)	30.46 (ii)	0.06
	15.00	45.00			0.03

	PEAK FLOW (cms)	TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
	19.88	11.18	60.70	62.70	0.97
	6.00	6.00	27.81	62.70	0.44
	39.00		39.00		
	62.70		62.70		
	0.44		0.62		

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 71.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8110)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (2022):	377.30	24.738	6.00	39.00
+ ID2= 2 (5202):	91.20	2.110	6.75	34.30
ID = 3 (8110):	468.50	24.976	6.00	38.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB
 STANDHYD (0102)
 Area (ha)= 466.00

|ID= 1 DT=15.0 min | Total Imp(%)= 23.00 Dir. Conn.(%)= 9.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	107.18	358.82
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1762.57	40.00
Mannings n	0.013	0.250

	Max.Eff.Inten.(mm/hr)	over (min)	Storage Coeff. (min)	Unit Hyd. Tpeak (min)	Unit Hyd. peak (cms)
	82.76	12.80	18.98 (ii)	43.32 (iii)	0.06
	15.00	45.00			0.03

	PEAK FLOW (cms)	TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
	6.30	6.55	60.70	62.70	0.97
	6.00	6.50	12.12	62.70	0.19
	16.49		16.49		
	62.70		62.70		
	0.26		0.26		

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 49.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB
 NASHYD (0100)
 ID= 1 DT=15.0 min | Area (ha)= 693.84 Curve Number (CN)= 40.0
 | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.95

Unit Hyd Qpeak (cms)= 6.087
 PEAK FLOW (cms)= 1.434 (i)
 TIME TO PEAK (hrs)= 9.250
 RUNOFF VOLUME (mm)= 7.560
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.121
 (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8000)

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
+ ID1= 1 (0100):	693.84	1.434	9.25	7.56
+ ID2= 2 (0102):	466.00	8.922	6.00	16.49
ID = 3 (8000):	1159.84	9.798	6.50	11.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6104)
 IN= 2--> OUT= 1
 Routing time step (min)= 15.00

<----- DATA FOR SECTION (1041.0) ----->

Distance	Elevation	Manning
0.00	330.16	0.0450
50.03	328.36	0.0450
89.34	326.33	0.0450
117.94	324.30	0.0450
160.82	322.75	0.0450
178.34	319.96	0.0450
183.34	319.81	0.0450
184.34	319.38	0.0450 /0.0300 Main Channel
185.09	318.78	0.0300 Main Channel
185.84	318.72	0.0300 Main Channel
186.84	319.32	0.0300 /0.0450 Main Channel
193.84	319.70	0.0450
198.84	320.38	0.0450
200.13	320.81	0.0450
218.00	322.49	0.0450
239.44	323.49	0.0450
250.17	323.96	0.0450
275.18	325.20	0.0450
310.92	326.47	0.0450
353.81	330.00	0.0450

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.30	319.02	624E+01	0.1	0.45	73.00
0.60	319.32	.181E+04	0.6	0.67	49.46
1.23	319.95	.128E+05	4.9	0.77	43.38
1.86	320.58	.396E+05	19.1	0.96	34.50

2.48	321.20	.757E+05	43.5	1.15	28.98
3.11	321.83	.125E+06	81.2	1.30	25.58
3.74	322.46	.187E+06	135.9	1.45	22.92
4.37	323.09	.269E+06	195.6	1.45	22.90
5.00	323.72	.387E+06	287.8	1.48	22.44
5.63	324.35	.545E+06	428.4	1.56	21.22
6.25	324.97	.735E+06	630.5	1.71	19.42
6.88	325.60	.952E+06	876.1	1.83	18.12
7.51	326.23	.120E+07	1177.0	1.95	17.03
8.14	326.86	.149E+07	1554.4	2.08	15.93
8.77	327.49	.179E+07	2009.7	2.23	14.88
9.40	328.12	.213E+07	2533.9	2.37	13.99
10.02	328.74	.249E+07	3112.6	2.49	13.31
10.65	329.37	.288E+07	3760.9	2.60	12.75
11.28	330.00	.330E+07	4494.8	2.71	12.23

<---- hydrograph ----> <-pipe / channel-->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8000) 1159.84	9.80	6.50	11.15	1.44	0.82
OUTFLOW: ID= 1 (6104) 1159.84	7.09	6.75	11.15	1.32	0.79

```

-----
| CALIB |
| NASHVD (0104) | Area (ha)= 527.81 Curve Number (CN)= 43.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.34
|-----|

Unit Hyd Qpeak (cms)= 6.735

PEAK FLOW (cms)= 1.618 (l)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 8.386
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.134

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8002) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
| ID1= 1 (0104): 527.81 1.618 8.00 8.39
| + ID2= 2 (6104): 1159.84 7.088 6.75 11.15
|-----|
| ID = 3 (8002): 1687.65 8.549 7.00 10.28
|-----|

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHVD (0106) | Area (ha)= 259.52 Curve Number (CN)= 55.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.23
|-----|

Unit Hyd Qpeak (cms)= 3.614

PEAK FLOW (cms)= 1.270 (l)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 12.444
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.198

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8004) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
| ID1= 1 (0106): 259.52 1.270 8.00 12.44
| + ID2= 2 (8002): 1687.65 8.549 7.00 10.28
|-----|
| ID = 3 (8004): 1947.17 9.740 7.00 10.57
|-----|

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6108) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00
|-----|

```

<----- DATA FOR SECTION (1081.0) ----->

Distance	Elevation	Manning
0.00	326.05	0.0400
12.06	325.74	0.0400
30.15	324.28	0.0400
59.27	320.35	0.0400
72.37	317.60	0.0400

87.45	314.52	0.0400
93.48	313.91	0.0400
105.54	313.55	0.0400
106.17	313.40	0.0400 / 0.0300 Main Channel
110.62	313.04	0.0300 Main Channel
110.82	312.97	0.0300 Main Channel
111.57	313.08	0.0300 Main Channel
111.67	313.48	0.0300 / 0.0400 Main Channel
129.66	316.62	0.0400
150.77	318.95	0.0400
180.92	319.61	0.0400
205.04	321.23	0.0400
232.18	322.09	0.0400
268.37	322.31	0.0400
298.52	326.00	0.0400

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.43	313.40	.244E+04	0.7	0.59	58.83
1.09	314.06	.214E+05	10.4	1.01	34.24
1.76	314.73	.600E+05	40.2	1.39	24.89
2.42	315.39	.109E+06	92.1	1.75	19.80
3.08	316.05	.169E+06	166.5	2.05	16.88
3.75	316.72	.237E+06	264.4	2.31	14.97
4.41	317.38	.318E+06	386.3	2.52	13.72
5.07	318.04	.411E+06	539.3	2.72	12.71
5.74	318.71	.517E+06	725.5	2.91	11.88
6.40	319.37	.642E+06	909.2	2.94	11.77
7.06	320.03	.806E+06	1179.3	3.04	11.38
7.72	320.69	.989E+06	1515.9	3.18	10.87
8.39	321.36	.119E+07	1895.5	3.30	10.49
9.05	322.02	.143E+07	2310.2	3.36	10.30
9.71	322.68	.172E+07	2763.2	3.33	10.39
10.38	323.35	.205E+07	3477.6	3.53	9.81
11.04	324.01	.238E+07	4284.3	3.73	9.27
11.70	324.67	.274E+07	5154.0	3.91	8.85
12.37	325.34	.311E+07	6104.0	4.08	8.48

<---- hydrograph ----> <-pipe / channel-->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8004) 1947.17	9.74	7.00	10.57	1.05	0.96
OUTFLOW: ID= 1 (6108) 1947.17	8.70	7.50	10.57	0.97	0.90

```

-----
| CALIB |
| NASHVD (0108) | Area (ha)= 207.05 Curve Number (CN)= 27.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.03
|-----|

```

Unit Hyd Qpeak (cms)= 3.432

```

PEAK FLOW (cms)= 0.402 (l)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 4.428
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.071

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8006) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
| ID1= 1 (0108): 207.05 0.402 7.50 4.43
| + ID2= 2 (6108): 1947.17 8.702 7.50 10.57
|-----|
| ID = 3 (8006): 2154.22 9.104 7.50 9.98
|-----|

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHVD (0110) | Area (ha)= 323.92 Curve Number (CN)= 52.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.35
|-----|

```

Unit Hyd Qpeak (cms)= 4.087

```

PEAK FLOW (cms)= 1.336 (l)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 11.321
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.181

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8008) |
| 1 + 2 = 3 |
-----
ID1= 1 (0110): 323.92 1.336 8.00 11.32
+ ID2= 2 (8006): 2154.22 9.104 7.50 9.98
=====
ID = 3 (8008): 2478.14 10.403 7.50 10.16

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6112) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)'= 15.00

```

```

-----
|<----- DATA FOR SECTION (1121.0) ----->
| Distance Elevation Manning |
| 0.00 320.59 0.0400 |
| 13.48 317.93 0.0400 |
| 40.45 311.52 0.0400 |
| 64.04 307.68 0.0400 |
| 74.15 307.01 0.0400 |
| 91.00 307.22 0.0400 |
| 107.86 305.21 0.0400 |
| 111.23 304.86 0.0400 |
| 113.32 304.60 0.0400 /0.0300 Main Channel |
| 117.77 304.24 0.0300 Main Channel |
| 117.97 304.17 0.0300 Main Channel |
| 118.72 304.28 0.0300 Main Channel |
| 118.82 304.68 0.0300 /0.0400 Main Channel |
| 124.71 304.76 0.0400 |
| 171.90 306.30 0.0400 |
| 205.60 307.24 0.0400 |
| 239.31 308.24 0.0400 |
| 262.90 310.64 0.0400 |
| 289.86 314.34 0.0400 |
| 333.68 317.66 0.0400 |

```

```

-----
|<----- TRAVEL TIME TABLE ----->
| DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME |
| (m) (m) (cu.m.) (cms) (m/s) (min) |
| 0.43 304.60 372E+04 0.6 0.54 97.65 |
| 1.12 305.29 463E+05 12.5 0.85 61.89 |
| 1.80 305.97 149E+06 53.0 1.13 46.85 |
| 2.49 306.66 311E+06 134.2 1.37 38.62 |
| 3.18 307.35 554E+06 261.6 1.50 35.27 |
| 3.87 308.04 894E+06 490.7 1.74 30.37 |
| 4.55 308.72 129E+07 833.0 2.05 25.75 |
| 5.24 309.41 171E+07 1269.8 2.36 22.39 |
| 5.93 310.10 215E+07 1789.0 2.64 20.01 |
| 6.62 310.79 261E+07 2392.7 2.90 18.21 |
| 7.30 311.47 310E+07 3087.8 3.16 16.74 |
| 7.99 312.16 361E+07 3873.6 3.40 15.52 |
| 8.68 312.85 413E+07 4741.0 3.64 14.52 |
| 9.37 313.54 467E+07 5689.3 3.86 13.69 |
| 10.05 314.22 523E+07 6718.4 4.07 12.97 |
| 10.74 314.91 581E+07 7770.7 4.24 12.46 |
| 11.43 315.60 641E+07 8897.5 4.40 12.01 |
| 12.12 316.29 704E+07 10113.2 4.55 11.61 |
| 12.80 316.97 770E+07 11420.1 4.70 11.24 |

```

```

-----
|<----- hydrograph -----> |<----- pipe / channel----->
| AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL |
| (ha) (cms) (hrs) (mm) (m) (m/s) |
INFLOW : ID= 2 (8008) 2478.14 10.40 7.50 10.16 1.00 0.78
OUTFLOW : ID= 1 (6112) 2478.14 8.45 9.00 10.16 0.88 0.71

```

```

-----
| CALIB (0112) |
| NASHYD (0112) | Area (ha)= 414.89 Curve Number (CN)= 30.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.91 |

```

```

Unit Hyd Qpeak (cms)= 3.699
PEAK FLOW (cms)= 0.584 (i)
TIME TO PEAK (hrs)= 9.250
RUNOFF VOLUME (mm)= 5.099
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.081

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8010) |
| 1 + 2 = 3 |
-----
ID1= 1 (0112): 414.89 0.584 9.25 5.10
+ ID2= 2 (6112): 2478.14 8.447 9.00 10.16
=====

```

ID = 3 (8010): 2893.03 9.029 9.00 9.43

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6114) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)'= 15.00

```

```

-----
|<----- DATA FOR SECTION (1141.0) ----->
| Distance Elevation Manning |
| 0.00 312.27 0.0500 |
| 14.07 309.70 0.0500 |
| 28.14 306.51 0.0500 |
| 35.17 305.81 0.0500 |
| 42.20 304.95 0.0500 |
| 52.75 304.24 0.0500 |
| 77.37 303.61 0.0500 |
| 98.47 301.33 0.0500 |
| 131.76 300.40 0.0500 /0.0300 Main Channel |
| 136.21 300.04 0.0300 Main Channel |
| 136.41 299.97 0.0300 Main Channel |
| 137.16 300.08 0.0300 Main Channel |
| 137.26 300.48 0.0300 /0.0500 Main Channel |
| 140.68 300.53 0.0500 |
| 179.36 302.20 0.0500 |
| 214.53 303.82 0.0500 |
| 242.66 305.27 0.0500 |
| 288.38 307.66 0.0500 |
| 316.52 311.76 0.0500 |
| 348.17 313.48 0.0500 |

```

```

-----
|<----- TRAVEL TIME TABLE ----->
| DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME |
| (m) (m) (cu.m.) (cms) (m/s) (min) |
| 0.43 300.40 366E+04 0.7 0.59 88.31 |
| 1.05 301.02 505E+05 12.1 0.75 69.49 |
| 1.68 301.65 165E+06 52.7 0.99 52.28 |
| 2.30 302.27 324E+06 130.3 1.26 41.40 |
| 2.93 302.90 520E+06 247.1 1.48 35.08 |
| 3.55 303.52 754E+06 406.8 1.68 30.91 |
| 4.18 304.15 104E+07 586.1 1.76 29.55 |
| 4.80 304.77 138E+07 856.0 1.93 26.96 |
| 5.43 305.40 177E+07 1201.1 2.12 24.56 |
| 6.05 306.02 219E+07 1612.2 2.30 22.63 |
| 6.68 306.65 264E+07 2090.6 2.47 21.07 |
| 7.30 307.27 313E+07 2653.0 2.64 19.65 |
| 7.93 307.90 364E+07 3303.9 2.83 18.36 |
| 8.55 308.52 417E+07 4053.9 3.03 17.14 |
| 9.18 309.15 471E+07 4871.9 3.22 16.12 |
| 9.80 309.77 527E+07 5756.2 3.41 15.25 |
| 10.43 310.40 584E+07 6699.8 3.58 14.52 |
| 11.05 311.02 642E+07 7710.5 3.74 13.89 |
| 11.68 311.65 702E+07 8788.5 3.90 13.32 |

```

```

-----
|<----- hydrograph -----> |<----- pipe / channel----->
| AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL |
| (ha) (cms) (hrs) (mm) (m) (m/s) |
INFLOW : ID= 2 (8010) 2893.03 9.03 9.00 9.43 0.89 0.70
OUTFLOW : ID= 1 (6114) 2893.03 8.02 10.50 9.43 0.83 0.68

```

```

-----
| CALIB (0114) | Area (ha)= 287.65 Curve Number (CN)= 49.0
| NASHYD (0114) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.12

```

```

Unit Hyd Qpeak (cms)= 4.389
PEAK FLOW (cms)= 1.232 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 10.248
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.163

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8012) |
| 1 + 2 = 3 |
-----
ID1= 1 (0114): 287.65 1.232 7.75 10.25
+ ID2= 2 (6114): 2893.03 8.016 10.50 9.43
=====
ID = 3 (8012): 3180.68 8.914 10.25 9.50

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |

```

NASHYD (0116) Area (ha)= 206.43 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.80

Unit Hyd Qpeak (cms)= 4.394

PEAK FLOW (cms)= 1.864 (i)
 TIME TO PEAK (hrs)= 7.000
 RINOFF VOLUME (mm)= 16.364
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.261

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8014)
 1 + 2 = 3
 AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (0116): 206.43 1.864 7.00 16.36
 + ID2= 2 (8012): 3180.68 8.914 10.25 9.50
 ID = 3 (8014): 3387.11 9.923 10.00 9.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6118) Routing time step (min)'= 15.00
 IN= 2--> OUT= 1

<----- DATA FOR SECTION (1181.0) ----->				
Distance	Elevation	Manning		
0.00	307.18	0.0450		
24.89	299.75	0.0450		
37.34	296.39	0.0450		
71.57	292.75	0.0450		
95.46	291.12	0.0450		
108.90	289.43	0.0450		
112.02	288.86	0.0450		
115.13	288.46	0.0450		
117.24	288.32	0.0450 / 0.0350	Main Channel	
117.49	288.15	0.0350	Main Channel	
118.24	288.00	0.0350	Main Channel	
118.99	288.20	0.0350	Main Channel	
119.24	288.36	0.0350 / 0.0450	Main Channel	
121.35	288.40	0.0450		
149.35	289.36	0.0450		
171.14	290.47	0.0450		
208.47	293.14	0.0450		
236.48	295.17	0.0450		
280.04	299.48	0.0450		
308.04	307.00	0.0450		

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.32	288.32	.116E+04	0.2	0.62	81.92
1.30	289.30	.631E+05	22.2	1.07	47.43
2.29	290.29	.218E+06	117.5	1.64	30.95
3.27	291.27	.488E+06	314.6	2.15	23.72
4.25	292.25	.756E+06	625.6	2.53	20.13
5.24	293.24	.115E+07	1093.2	2.91	17.50
6.22	294.22	.161E+07	1731.6	3.28	15.50
7.20	295.20	.214E+07	2547.4	3.63	14.01
8.19	296.19	.274E+07	3584.9	4.00	12.72
9.17	297.17	.338E+07	4859.1	4.39	11.60
10.15	298.15	.407E+07	6341.5	4.76	10.70
11.13	299.13	.480E+07	8024.6	5.11	9.97
12.12	300.12	.556E+07	10006.6	5.49	9.27
13.10	301.10	.635E+07	12244.6	5.89	8.64
14.08	302.08	.716E+07	14681.7	6.26	8.13
15.07	303.07	.799E+07	17316.5	6.62	7.69
16.05	304.05	.884E+07	20148.1	6.96	7.31
17.03	305.03	.971E+07	23176.2	7.29	6.98
18.02	306.02	.106E+08	26400.8	7.61	6.69

<---- hydrograph ----> <-pipe / channel-->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8014) 3387.11 9.92 10.00 9.92 0.75 0.76
 OUTFLOW: ID= 1 (6118) 3387.11 9.33 11.25 9.92 0.72 0.75

CALIB
 NASHYD (0118) Area (ha)= 340.53 Curve Number (CN)= 45.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.12

Unit Hyd Qpeak (cms)= 5.200

PEAK FLOW (cms)= 1.273 (i)

TIME TO PEAK (hrs)= 7.750
 RINOFF VOLUME (mm)= 8.965
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.143

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8016)
 1 + 2 = 3
 AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (0118): 340.53 1.273 7.75 8.97
 + ID2= 2 (6118): 3387.11 9.332 11.25 9.92
 ID = 3 (8016): 3727.64 10.159 11.00 9.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6122) Routing time step (min)'= 15.00
 IN= 2--> OUT= 1

<----- DATA FOR SECTION (1221.0) ----->				
Distance	Elevation	Manning		
0.00	277.00	0.0500		
52.18	272.82	0.0500		
86.97	269.38	0.0500		
134.80	267.35	0.0500		
160.89	263.85	0.0500		
169.58	262.88	0.0500		
178.28	262.45	0.0500		
195.67	262.07	0.0500		
213.07	260.99	0.0500		
226.01	259.80	0.0500 / 0.0300	Main Channel	
226.11	259.30	0.0300	Main Channel	
230.11	259.30	0.0300	Main Channel	
230.21	259.80	0.0300 / 0.0500	Main Channel	
235.01	260.20	0.0500		
239.15	260.57	0.0500		
243.50	260.93	0.0500		
300.03	264.43	0.0500		
339.16	268.34	0.0500		
373.95	272.36	0.0500		
430.48	277.46	0.0500		

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	259.80	.626E+04	2.5	1.23	41.35
1.41	260.71	.462E+05	21.8	1.44	35.27
2.31	261.61	.149E+06	79.6	1.63	31.19
3.22	262.52	.340E+06	200.1	1.80	28.34
4.12	263.42	.636E+06	446.8	2.15	23.71
5.03	264.33	.995E+06	819.9	2.52	20.23
5.93	265.23	.141E+07	1337.0	2.90	17.56
6.84	266.14	.187E+07	1984.8	3.25	15.67
7.74	267.04	.237E+07	2766.0	3.57	14.26
8.65	267.95	.292E+07	3596.9	3.76	13.53
9.55	268.85	.355E+07	4575.4	3.93	12.95
10.46	269.76	.427E+07	5814.9	4.16	12.23
11.36	270.66	.503E+07	7319.0	4.44	11.46
12.27	271.57	.584E+07	9009.7	4.71	10.81
13.17	272.47	.670E+07	10886.3	4.96	10.26
14.08	273.38	.761E+07	12893.2	5.17	9.84
14.98	274.28	.858E+07	15098.1	5.37	9.48
15.89	275.19	.961E+07	17527.6	5.57	9.14
16.79	276.09	.107E+08	20190.3	5.76	8.83

<---- hydrograph ----> <-pipe / channel-->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8016) 3727.64 10.16 11.00 9.84 0.86 1.31
 OUTFLOW: ID= 1 (6122) 3727.64 9.97 11.75 9.84 0.84 1.30

CALIB
 STANDHYD (1221) Area (ha)= 17.70
 ID= 1 DT=15.0 min Total Imp(%)= 36.00 Dir. Conn.(%)= 19.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 6.37 11.33
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 343.51 40.00
 Mannings n = 0.013 0.250
 Max. Rff. Inten.(mm/hr)= 82.76 8.88
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 7.11 (ii) 35.29 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00

Unit Hyd. peak (cms)= 0.10 0.03 *TOTALS*
 PEAK FLOW (cms)= 0.71 0.16 0.775 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 60.70 8.00 18.01
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.13 0.29

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 35.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR (5122) |
 IN= 2--> OUT= 1 |
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.1840	0.4294
0.0600	0.1855	1.4870	0.5014
0.3430	0.2103	1.7310	0.5718
0.6390	0.2878	2.1310	0.5818
0.8860	0.3509	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
17.700	0.775	6.00	18.01
17.700	0.121	7.25	17.97

INFLOW : ID= 2 (1221)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	17.700	0.775	6.00	18.01

OUTFLOW: ID= 1 (5122)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	17.700	0.121	7.25	17.97

PEAK FLOW REDUCTION [Qout/Qin](%)	TIME SHIFT OF PEAK FLOW (min)	MAXIMUM STORAGE USED (ha.m.)
15.61	75.00	0.1908

 CALIB (1222) |
 NASHYD (1222) | Area (ha)= 347.20 Curve Number (CN)= 41.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.52

Unit Hyd Qpeak (cms)= 3.887
 PEAK FLOW (cms)= 0.895 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 7.824
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.125

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8018) |
1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
347.20	0.895	8.25	7.82
17.70	0.121	7.25	17.97
364.90	0.983	8.00	8.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8020) |
1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
3727.64	9.970	11.75	9.84
364.90	0.983	8.00	8.32
4092.54	10.712	11.50	9.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB (1261) |
 STANDHYD (1261) | Area (ha)= 50.00
 ID= 1 DT=15.0 min | Total Imp(%)= 36.00 Dir. Conn.(%)= 26.00

IMPERVIOUS (i)	PERVIOUS (i)
18.00	32.00
2.00	5.00
0.50	0.50
577.35	40.00
0.013	0.250

Max.Eff.Inten.(mm/hr)= 82.76 11.85
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 9.71 (ii) 34.82 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.09 0.03 *TOTALS*
 PEAK FLOW (cms)= 2.55 0.61 2.790 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 60.70 11.50 24.29
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.18 0.39

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR (5126) |
 IN= 2--> OUT= 1 |
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.3450	1.2129
0.1690	0.5421	4.2000	1.4164
0.9690	0.5941	4.8890	1.6153
1.8040	0.8130	5.2890	1.6253
2.5020	0.9912	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
50.000	2.790	6.00	24.29
50.000	0.925	6.75	24.27

INFLOW : ID= 2 (1261)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	50.000	2.790	6.00	24.29

OUTFLOW: ID= 1 (5126)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	50.000	0.925	6.75	24.27

PEAK FLOW REDUCTION [Qout/Qin](%)	TIME SHIFT OF PEAK FLOW (min)	MAXIMUM STORAGE USED (ha.m.)
33.16	45.00	0.5993

 CALIB (1262) |
 NASHYD (1262) | Area (ha)= 83.50 Curve Number (CN)= 58.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.56

Unit Hyd Qpeak (cms)= 2.523
 PEAK FLOW (cms)= 0.809 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 13.451
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.215

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8024) |
1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
83.50	0.809	6.50	13.45
50.00	0.925	6.75	24.27
133.50	1.730	6.75	17.51

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB (0124) |
 NASHYD (0124) | Area (ha)= 121.43 Curve Number (CN)= 43.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.51

Unit Hyd Qpeak (cms)= 4.049
 PEAK FLOW (cms)= 0.766 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 8.210
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.131

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8026) |
1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
121.43	0.766	6.50	8.21

+ ID2= 2 (8024): 133.50 1.730 6.75 17.51
 ID = 3 (8026): 254.93 2.482 6.75 13.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN= (6128) |
 IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (1281.0) ----->

Distance	Elevation	Manning	
0.00	270.58	0.0400	
7.71	270.36	0.0400	
15.43	269.48	0.0400	
23.14	265.88	0.0400	
42.43	261.83	0.0400	
61.71	258.31	0.0400	
75.21	256.62	0.0400	
90.64	255.35	0.0400	
92.49	255.00	0.0400 / 0.0300	Main Channel
93.49	254.00	0.0300	Main Channel
94.49	253.90	0.0300	Main Channel
94.99	253.80	0.0300	Main Channel
95.49	253.70	0.0300	Main Channel
95.99	253.80	0.0300	Main Channel
98.49	255.00	0.0300 / 0.0400	Main Channel
100.28	255.44	0.0400	
134.99	257.45	0.0400	
148.49	259.67	0.0400	
167.77	263.42	0.0400	
190.92	270.00	0.0400	

TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.65	254.35	.439E+04	2.0	1.24	36.10
1.30	255.00	1.31E+05	9.4	1.92	23.39
2.18	255.88	4.33E+05	37.0	2.30	19.54
3.06	256.76	1.31E+06	113.3	2.34	19.20
3.95	257.65	2.73E+06	273.8	2.70	16.64
4.83	258.53	4.53E+06	530.5	3.21	14.01
5.71	259.41	6.58E+06	902.9	3.70	12.14
6.59	260.29	8.86E+06	1368.4	4.16	10.79
7.48	261.18	1.14E+07	1936.3	4.59	9.79
8.36	262.06	1.41E+07	2610.9	4.99	9.01
9.24	262.94	1.71E+07	3398.7	5.37	8.36
10.12	263.82	2.02E+07	4309.6	5.75	7.81
11.01	264.71	2.35E+07	5347.5	6.13	7.33
11.89	265.59	2.70E+07	6501.0	6.48	6.93
12.77	266.47	3.07E+07	7808.6	6.86	6.55
13.65	267.35	3.45E+07	9250.5	7.23	6.21
14.54	268.24	3.84E+07	10800.3	7.59	5.92
15.42	269.12	4.24E+07	12482.5	7.94	5.66
16.30	270.00	4.66E+07	14127.4	8.18	5.49

<--- hydrograph ---> <- pipe / channel ->

	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8026)	254.93	2.48	6.75	13.08	0.69	1.27
OUTFLOW : ID= 1 (6128)	254.93	1.94	7.25	13.08	0.62	1.24

CALIB
 NASHYD (0128) | Area (ha)= 240.24 Curve Number (CN)= 62.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.00

Unit Hyd Qpeak (cms)= 4.118
 PEAK FLOW (cms)= 1.714 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 15.443
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.246

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8028) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID= 1 (0128): 240.24 1.714 7.25 15.44
 + ID2= 2 (6128): 254.93 1.941 7.25 13.08
 ID = 3 (8028): 495.17 3.655 7.25 14.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8022) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID= 1 (8020): 4092.84 10.712 11.50 9.70
 + ID2= 2 (8028): 495.17 3.655 7.25 14.22
 ID = 3 (8022): 4587.71 12.209 11.25 10.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1201) | Area (ha)= 86.00
 ID= 1 DT=15.0 min | Total Imp(%)= 36.00 Dir. Conn.(%)= 14.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 30.96 55.04
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 757.19 40.00
 Mannings n = 0.013 0.250
 Max.Eff.Inten.(mm/hr)= 82.76 15.93
 over (min)= 15.00 45.00
 Storage Coeff (min)= 11.43 (ii) 33.73 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03
 PEAK FLOW (cms)= 2.24 1.43 **TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.50 2.820 (iii)
 RUNOFF VOLUME (mm)= 60.70 13.19 19.84
 TOTAL RAINFALL (mm)= 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.21 0.32

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5120) |
 IN= 2--> OUT= 1 |
 DT= 15.0 min |

	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	5.7530	2.0863
	0.2910	0.9015	7.2230	2.4361
	1.6660	1.0219	8.4090	2.7784
	3.1030	1.3984	8.8090	2.7884
	4.3040	1.7049	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (1201)	86.000	2.820	6.00	19.84
OUTFLOW : ID= 1 (5120)	86.000	0.825	7.25	19.83

PEAK FLOW REDUCTION [Qout/Qin](%)= 29.26
 TIME SHIFT OF PEAK FLOW (min)= 75.00
 MAXIMUM STORAGE USED (ha.m.)= 0.9495

CALIB
 NASHYD (1202) | Area (ha)= 161.00 Curve Number (CN)= 58.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.70

Unit Hyd Qpeak (cms)= 3.932
 PEAK FLOW (cms)= 1.324 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 13.539
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.216

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8030) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID= 1 (1202): 161.00 1.324 6.75 13.54
 + ID2= 2 (5120): 86.00 0.825 7.25 19.83
 ID = 3 (8030): 247.00 2.110 7.25 15.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8032)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8022):	4587.71	12.209	11.25	10.19
+ ID2= 2 (8030):	247.00	2.110	7.25	15.73
=====				
ID = 3 (8032):	4834.71	12.955	11.00	10.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6130)
IN= 2--> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (1301.0) -----				
Distance	Elevation	Manning		
0.00	257.18	0.0350		
62.45	254.70	0.0350		
109.29	252.74	0.0350		
150.92	251.55	0.0350		
192.56	248.96	0.0350		
239.39	247.16	0.0350		
291.84	246.00	0.0350		
296.84	245.00	0.0350		
298.84	245.00	0.0350 / 0.0300	Main Channel	
299.24	244.60	0.0300	Main Channel	
301.84	244.60	0.0300	Main Channel	
303.44	244.60	0.0300	Main Channel	
303.84	245.00	0.0300 / 0.0350	Main Channel	
306.84	245.00	0.0350		
309.84	246.00	0.0350		
317.46	247.22	0.0350		
348.68	254.83	0.0350		
359.09	256.70	0.0350		
452.76	258.23	0.0350		
515.22	258.33	0.0350		

----- TRAVEL TIME TABLE -----					
DEPTH (m)	ELEV (m)	VOLUME (cu. m)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.40	245.00	.870E+04	1.2	0.64	124.12
1.04	245.64	.468E+05	9.5	0.96	81.81
1.68	246.28	.109E+06	25.6	1.11	70.86
2.32	246.92	.257E+06	63.5	1.17	67.51
2.96	247.56	.498E+06	142.7	1.36	58.18
3.61	248.21	.800E+06	266.7	1.58	50.01
4.25	248.85	.115E+07	437.4	1.78	44.24
4.89	249.49	.157E+07	659.1	2.01	39.20
5.53	250.13	.203E+07	954.9	2.23	35.36
6.17	250.77	.252E+07	1294.4	2.43	32.42
6.81	251.41	.305E+07	1689.6	2.62	30.07
7.45	252.05	.363E+07	2090.7	2.73	28.93
8.09	252.69	.429E+07	2553.1	2.82	27.98
8.73	253.33	.501E+07	3138.4	2.96	26.60
9.37	253.97	.579E+07	3803.8	3.11	25.36
10.02	254.62	.662E+07	4549.3	3.25	24.25
10.66	255.26	.751E+07	5364.9	3.38	23.32
11.30	255.90	.845E+07	6265.7	3.51	22.49
11.94	256.54	.946E+07	7260.1	3.63	21.72

----- hydrograph -----						-<- pipe / channel->	
AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)		
INFLOW : ID= 2 (8032)	4834.71	12.95	11.00	10.47	1.18	0.99	
OUTFLOW : ID= 1 (6130)	4834.71	12.36	12.25	10.47	1.15	0.99	

CALIB STANDHYD (1301)				Dir. Conn. (%) = 47.00			
1 + 2 = 3		AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
		228.30					
		Total Imp (%) = 65.00					
----- IMPERVIOUS PERVIOUS (i) -----							
Surface Area (ha)	148.39	79.91					
Dep. Storage (mm)	2.00	5.00					
Average Slope (%)	0.50	0.50					
Length (m)	1233.69	40.00					
Mannings n	0.013	0.250					
Max. Eff. Inten. (mm/hr)	82.76	69.84					
over (min)	15.00	30.00					
Storage Coeff. (min)	15.32 (ii)	27.67 (ii)					
Unit Hyd. Tpeak (min)	15.00	30.00					
Unit Hyd. peak (cms)	0.07	0.04					
*****TOTALS* (iii)							
PEAK FLOW (cms)	17.81	6.74	22.103				
TIME TO PEAK (hrs)	6.00	6.25	6.00				

RUNOFF VOLUME (mm)	60.70	29.82	44.33
TOTAL RAINFALL (mm)	62.70	62.70	62.70
RUNOFF COEFFICIENT	0.97	0.48	0.71

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 74.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5130)			
IN= 2--> OUT= 1		DT= 15.0 min	
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	12.4560	7.0142
0.8210	3.9074	14.9550	7.8554
4.8550	4.4209	22.4790	8.9550
7.0620	5.2214	22.8790	8.9650
9.5970	6.0904	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1301)	228.300	22.103	6.00	44.33
OUTFLOW : ID= 1 (5130)	228.300	7.065	6.75	44.33

PEAK FLOW REDUCTION [Qout/Qin] (%) = 31.96	
TIME SHIFT OF PEAK FLOW (min) = 45.00	MAXIMUM STORAGE USED (ha.m.) = 5.2395

CALIB STANDHYD (1302)				Dir. Conn. (%) = 1.50	
1 + 2 = 3		AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
		157.80	1.116	8.75	24.04
		Total Imp (%) = 65.00			
----- IMPERVIOUS PERVIOUS (i) -----					
Surface Area (ha)	148.39	79.91			
Dep. Storage (mm)	2.00	5.00			
Average Slope (%)	0.50	0.50			
Length (m)	1233.69	40.00			
Mannings n	0.013	0.250			
Max. Eff. Inten. (mm/hr)	82.76	69.84			
over (min)	15.00	30.00			
Storage Coeff. (min)	15.32 (ii)	27.67 (ii)			
Unit Hyd. Tpeak (min)	15.00	30.00			
Unit Hyd. peak (cms)	0.07	0.04			
*****TOTALS* (iii)					
PEAK FLOW (cms)	17.81	6.74	22.103		
TIME TO PEAK (hrs)	6.00	6.25	6.00		

ADD HYD (8036)				R.V. (mm)		
1 + 2 = 3		AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
		157.80	1.116	8.75	24.04	
		+ ID2= 2 (5130):	228.30	7.065	6.75	44.33
=====						
		ID = 3 (8036):	386.10	7.936	6.75	36.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8034)				R.V. (mm)		
1 + 2 = 3		AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
		4834.71	12.360	12.25	10.47	
		+ ID2= 2 (8036):	386.10	7.936	6.75	36.03
=====						
		ID = 3 (8034):	5220.81	14.056	12.00	12.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (0188)				Dir. Conn. (%) = 25.00	
1 + 2 = 3		AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
		308.53			
		Total Imp (%) = 41.00			
----- IMPERVIOUS PERVIOUS (i) -----					
Surface Area (ha)	126.50	182.03			
Dep. Storage (mm)	3.00	5.00			
Average Slope (%)	0.10	0.10			
Length (m)	1434.18	40.00			
Mannings n	0.013	0.250			
Max. Eff. Inten. (mm/hr)	56.43	15.52			
over (min)	30.00	75.00			
Storage Coeff. (min)	31.67 (ii)	68.20 (ii)			
Unit Hyd. Tpeak (min)	30.00	75.00			
Unit Hyd. peak (cms)	0.04	0.02			
*****TOTALS* (iii)					
PEAK FLOW (cms)	7.73	3.68			
TIME TO PEAK (hrs)					

TIME TO PEAK (hrs)= 6.25 7.00 6.25
 RUNOFF VOLUME (mm)= 59.70 17.48 28.03
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.95 0.28 0.45

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (8186) | Area (ha)= 351.79
 ID= 1 DT=15.0 min | Total Imp(%)= 42.00 Dir. Conn.(%)= 22.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	147.75	204.04
Dep. Storage (mm)=	3.00	5.00
Average Slope (%)=	0.10	0.10
Length (m)=	1531.43	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	56.43	14.32
over (min)	30.00	75.00
Storage Coeff. (min)=	32.95 (ii)	70.67 (ii)
Unit Hyd. Tpeak (min)=	30.00	75.00
Unit Hyd. peak (cms)=	0.04	0.02
TOTALS		
PEAK FLOW (cms)=	7.60	3.73
TIME TO PEAK (hrs)=	6.25	7.00
RUNOFF VOLUME (mm)=	59.70	15.36
TOTAL RAINFALL (mm)=	62.70	62.70
RUNOFF COEFFICIENT =	0.95	0.24

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 53.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8068) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0186): 351.79 9.213 6.25 25.12
 + ID2= 2 (0188): 308.53 9.324 6.25 28.03
 =====
 ID = 3 (8068): 660.32 18.537 6.25 26.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1821) | Area (ha)= 111.80
 ID= 1 DT=15.0 min | Total Imp(%)= 40.00 Dir. Conn.(%)= 17.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	44.72	67.08
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	863.33	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	82.76	19.18
over (min)	15.00	45.00
Storage Coeff. (min)=	12.37 (ii)	33.07 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03
TOTALS		
PEAK FLOW (cms)=	3.44	2.11
TIME TO PEAK (hrs)=	6.00	6.00
RUNOFF VOLUME (mm)=	60.70	15.27
TOTAL RAINFALL (mm)=	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.24

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 52.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5182) |

IN= 2--> OUT= 1	OUTFLOW	STORAGE	OUTFLOW	STORAGE
DT= 15.0 min	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	5.7800	2.5656
	0.3790	1.0207	7.3720	3.0829
	1.6240	1.1939	8.7710	3.5956
	3.0930	1.6631	9.1710	3.6056
	4.3530	2.1032	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (1821)	111.800	4.304	6.00	23.00
OUTFLOW: ID= 1 (5182)	111.800	1.685	7.00	22.99

PEAK FLOW REDUCTION [Qout/Qin](%)= 39.16
 TIME SHIFT OF PEAK FLOW (min)= 60.00
 MAXIMUM STORAGE USED (ha.m.)= 1.2149

CALIB
 NASHYD (1822) | Area (ha)= 44.40 Curve Number (CN)= 57.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.79

Unit Hyd Qpeak (cms) = 0.966

PEAK FLOW (cms)= 0.325 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 13.159
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.210

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8062)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1822):	44.40	0.325	7.00	13.16
+ ID2= 2 (5182):	111.80	1.685	7.00	22.99
=====				
ID = 3 (8062):	156.20	2.010	7.00	20.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1841) | Area (ha)= 144.40
 ID= 1 DT=15.0 min | Total Imp(%)= 48.00 Dir. Conn.(%)= 19.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	69.31	75.09
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	981.16	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	82.76	44.28
over (min)	15.00	30.00
Storage Coeff. (min)=	13.35 (ii)	28.17 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04
TOTALS		
PEAK FLOW (cms)=	4.82	3.93
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	60.70	18.86
TOTAL RAINFALL (mm)=	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.30

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 56.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5184)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	5.8820	2.3298
	0.2960	1.0006	7.4070	2.7386
	1.6530	1.1560	8.4830	3.1418
	3.0860	1.4687	9.0830	3.1518
	4.3760	1.8752	0.0000	0.0000

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (1841) 144.400 7.247 6.00 26.81
 OUTFLOW: ID= 1 (5184) 144.400 3.423 6.75 26.81

PEAK FLOW REDUCTION [Qout/Qin](%) = 47.24
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 1.5963

CALIB | STANDBY (1842) | Area (ha) = 118.10
 | ID= 1 DT=15.0 min | Total Imp(%) = 42.00 Dir. Conn.(%) = 17.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 49.60 68.50
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 887.32 40.00
 Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr) = 82.76 23.17
 over (min) 15.00 45.00
 Storage Coeff. (min) = 12.57 (ii) 31.77 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.08 0.03

TOTALS

PEAK FLOW (cms) = 3.61 2.65 4.708 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 60.70 17.54 24.96
 TOTAL RAINFALL (mm) = 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.28 0.40

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 56.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8060) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (1842): 118.10 4.708 6.00 24.96
 + ID2= 2 (5184): 144.40 3.423 6.75 26.81

 ID = 3 (8060): 262.50 6.921 6.50 25.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8064) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 + ID1= 1 (8060): 262.50 6.921 6.50 25.98
 + ID2= 2 (8062): 156.20 2.010 7.00 20.20

 ID = 3 (8064): 418.70 7.950 6.50 23.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB | STANDBY (1781) | Area (ha) = 14.00
 | ID= 1 DT=15.0 min | Total Imp(%) = 50.00 Dir. Conn.(%) = 30.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 7.00 7.00
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 305.51 40.00
 Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr) = 82.76 65.21
 over (min) 15.00 30.00
 Storage Coeff. (min) = 6.63 (ii) 19.32 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.10 0.05

TOTALS

PEAK FLOW (cms) = 0.90 0.65 1.321 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 60.70 30.03 39.23
 TOTAL RAINFALL (mm) = 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.48 0.63

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 76.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5178) |
 IN= 2--> OUT= 1 |
 DT= 15.0 min |
 OUTFLOW STORAGE OUTFLOW STORAGE
 (cms) (ha.m.) (cms) (ha.m.)
 0.0000 0.0000 0.9360 0.3780
 0.0470 0.1841 1.1760 0.4277
 0.2710 0.2148 1.3690 0.4763
 0.5050 0.2704 1.7690 0.4863
 0.7010 0.3197 0.0000 0.0000

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (1781) 14.000 1.321 6.00 39.23
 OUTFLOW: ID= 1 (5178) 14.000 0.475 6.50 39.16

PEAK FLOW REDUCTION [Qout/Qin](%) = 35.95
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 0.2667

CALIB | NASHYD (1782) | Area (ha) = 261.40 Curve Number (CN) = 78.0
 | ID= 1 DT=15.0 min | Ia (mm) = 5.00 # of Linear Res.(N) = 1.50
 U.N. Tp(hrs) = 1.15

Unit Hyd Qpeak (cms) = 3.867

PEAK FLOW (cms) = 2.821 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 25.528
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.407

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8046) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (1782): 261.40 2.821 7.50 25.53
 + ID2= 2 (5178): 14.00 0.475 6.50 39.16

 ID = 3 (8046): 275.40 3.149 7.00 26.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB | NASHYD (0180) | Area (ha) = 226.65 Curve Number (CN) = 65.0
 | ID= 1 DT=15.0 min | Ia (mm) = 5.00 # of Linear Res.(N) = 1.50
 U.N. Tp(hrs) = 1.17

Unit Hyd Qpeak (cms) = 3.299

PEAK FLOW (cms) = 1.576 (i)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 16.982
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.271

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8050) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0180): 226.65 1.576 7.75 16.98
 + ID2= 2 (8046): 275.40 3.149 7.00 26.22

 ID = 3 (8050): 502.05 4.694 7.25 22.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB | STANDBY (1741) | Area (ha) = 56.30
 | ID= 1 DT=15.0 min | Total Imp(%) = 35.00 Dir. Conn.(%) = 18.00

Surface Area (ha)= 19.70 36.60
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 612.64 40.00
 Manning's n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 82.76 23.49
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 10.07 (ii) 29.16 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.04

TOTALS
 PEAK FLOW (cms)= 1.96 1.63 2.967 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 60.70 20.06 27.37
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.32 0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 64.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5174)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.1370	1.3070
0.0580	0.5427	1.4280	1.5354
0.3290	0.6196	1.6620	1.7593
0.6130	0.8627	2.0620	1.7693
0.8510	1.0621	0.0000	0.0000

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (1741) 56.300 2.967 6.00 27.37
 OUTFLOW: ID= 1 (5174) 56.300 0.624 7.25 27.33

PEAK FLOW REDUCTION [Qout/Qin](%)= 21.03
 TIME SHIFT OF PEAK FLOW (min)= 75.00
 MAXIMUM STORAGE USED (ha.m.)= 0.8732

CALIB (1742)
 NASHYD (1742) Area (ha)= 82.80 Curve Number (CN)= 71.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.69

Unit Hyd Qpeak (cms)= 2.055
 PEAK FLOW (cms)= 1.057 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 20.256
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.323

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8044)
 1 + 2 = 3

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (1742): 82.80 1.057 6.75 20.26
 + ID2= 2 (5174): 56.30 0.624 7.25 27.33
 ID = 3 (8044): 139.10 1.658 7.00 23.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0172)
 NASHYD (0172) Area (ha)= 202.23 Curve Number (CN)= 78.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.89

Unit Hyd Qpeak (cms)= 3.875
 PEAK FLOW (cms)= 2.677 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 25.429
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.406

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0168)
 NASHYD (0168) Area (ha)= 247.13 Curve Number (CN)= 52.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.75

Unit Hyd Qpeak (cms)= 5.611
 PEAK FLOW (cms)= 1.583 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 11.218
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.179

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0170)
 NASHYD (0170) Area (ha)= 166.91 Curve Number (CN)= 65.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.80

Unit Hyd Qpeak (cms)= 3.553
 PEAK FLOW (cms)= 1.557 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 16.878
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.269

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8040)
 1 + 2 = 3

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0168): 247.13 1.583 7.00 11.22
 + ID2= 2 (0170): 166.91 1.557 7.00 16.88
 ID = 3 (8040): 414.04 3.140 7.00 13.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8042)
 1 + 2 = 3

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0172): 202.23 2.677 7.00 25.43
 + ID2= 2 (8040): 414.04 3.140 7.00 13.50
 ID = 3 (8042): 616.27 5.816 7.00 17.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6174)
 IN= 2--> OUT= 1

Routing time step (min)= 15.00

<----- DATA FOR SECTION (1741.0) ----->

Distance	Elevation	Manning
0.00	296.00	0.0450
13.36	288.94	0.0450
26.71	288.64	0.0450
51.39	287.96	0.0450
73.45	287.32	0.0450
86.81	286.48	0.0450
102.39	285.08	0.0450
110.73	282.76	0.0450
115.73	281.06	0.0450 / 0.0350 Main Channel
115.74	280.35	0.0350 Main Channel
120.73	280.35	0.0350 Main Channel
120.74	281.10	0.0350 / 0.0450 Main Channel
125.74	282.64	0.0450
135.78	286.87	0.0450
149.13	289.58	0.0450
162.49	292.29	0.0450
175.84	293.95	0.0450
195.88	295.13	0.0450
213.68	296.02	0.0450
220.36	296.09	0.0450

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.71	281.06	.136E+05	6.0	1.69	37.89
1.50	281.65	.358E+05	22.5	2.42	26.43

2.28	282.63	.726E+05	53.8	2.85	22.47
3.07	283.42	.124E+06	104.1	3.24	19.79
3.86	284.21	.189E+06	176.7	3.60	17.81
4.64	284.99	.268E+06	274.9	3.94	16.26
5.43	285.78	.369E+06	390.8	4.07	15.73
6.21	286.56	.501E+06	551.6	4.23	15.15
7.00	287.35	.674E+06	750.5	4.28	14.97
7.79	288.14	.920E+06	993.3	4.15	15.44
8.57	288.92	.126E+07	1361.5	4.14	15.47
9.36	289.71	.167E+07	1986.0	4.56	14.04
10.15	290.50	.210E+07	2739.7	5.02	12.76
10.93	291.28	.254E+07	3615.0	5.47	11.70
11.72	292.07	.300E+07	4608.8	5.91	10.83
12.50	292.85	.347E+07	5693.3	6.30	10.16
13.29	293.64	.397E+07	6892.9	6.67	9.60
14.08	294.43	.450E+07	8150.4	6.97	9.19
14.86	295.21	.506E+07	9525.0	7.23	8.86

		<---- hydrograph ---->			<-pipe / channel-->	
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8042)	616.27	5.82	7.00	17.41	0.69	1.69
OUTFLOW: ID= 1 (6174)	616.27	5.32	8.00	17.41	0.63	1.69

ADD HYD (8048)		AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	(mm)
ID1= 1 (6174):	616.27	5.322	8.00	17.41	0.63
+ ID2= 2 (8044):	139.10	1.658	7.00	23.12	0.63
ID = 3 (8048):	755.37	6.828	7.75	18.46	0.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8052)		AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	(mm)
ID1= 1 (8048):	755.37	6.828	7.75	18.46	0.63
+ ID2= 2 (8050):	502.05	4.694	7.25	22.05	0.63
ID = 3 (8052):	1257.42	11.449	7.50	19.90	0.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8058)		AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	(mm)
ID1= 1 (8052):	1257.42	11.449	7.50	19.90	0.63
+ ID2= 2 (8064):	418.70	7.950	6.50	23.82	0.63
ID = 3 (8058):	1676.12	18.020	7.00	20.88	0.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB		Area	(ha)=	162.00
STANDHYD (1762)		Total Imp(%)=	26.00	Dir. Conn.(%)= 11.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	42.12	119.88
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	1039.23	40.00
Mannings n	=	0.013	0.250
Max.Eff.Inten.(mm/hr)=		82.76	18.35
over (min)		15.00	45.00
Storage Coeff. (min)=		13.82 (ii)	34.90 (ii)
Unit Hyd. Tpeak (min)=		15.00	45.00
Unit Hyd. peak (cms)=		0.08	0.03
PEAK FLOW (cms)=		3.09	3.51
TIME TO PEAK (hrs)=		6.00	6.50
RUNOFF VOLUME (mm)=		60.70	16.69
TOTAL RAINFALL (mm)=		62.70	62.70
RUNOFF COEFFICIENT =		0.97	0.27

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB		Area	(ha)=	48.80
STANDHYD (1761)		Total Imp(%)=	50.00	Dir. Conn.(%)= 21.00
ID= 1 DT=15.0 min				
	IMPERVIOUS	PERVIOUS (i)		
Surface Area	(ha)=	24.40	24.40	
Dep. Storage	(mm)=	2.00	5.00	
Average Slope	(%)=	0.50	0.50	
Length	(m)=	570.38	40.00	
Mannings n	=	0.013	0.250	
Max.Eff.Inten.(mm/hr)=		82.76	49.51	
over (min)		15.00	30.00	
Storage Coeff. (min)=		9.64 (ii)	23.81 (ii)	
Unit Hyd. Tpeak (min)=		15.00	30.00	
Unit Hyd. peak (cms)=		0.09	0.04	
PEAK FLOW (cms)=		2.01	1.56	*****
TIME TO PEAK (hrs)=		6.00	6.25	2.986 (iii)
RUNOFF VOLUME (mm)=		60.70	20.70	6.00
TOTAL RAINFALL (mm)=		62.70	62.70	29.10
RUNOFF COEFFICIENT =		0.97	0.33	62.70

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5176)		OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1		(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min					
		0.0000	0.0000	3.2640	1.3175
		0.1650	0.6418	4.0090	1.4908
		0.9450	0.7489	4.7720	1.6604
		1.7610	0.9426	5.1720	1.6704
		2.4420	1.1144	0.0000	0.0000
INFLOW : ID= 2 (1761)		48.800	2.986	6.00	29.10
OUTFLOW: ID= 1 (5176)		48.800	0.959	6.75	29.08
	PEAK FLOW REDUCTION [Qout/Qin](%)=			32.11	
	TIME SHIFT OF PEAK FLOW			(min)= 45.00	
	MAXIMUM STORAGE USED			(ha.m.)= 0.7540	

ADD HYD (8056)		AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	(mm)
ID1= 1 (1762):	162.00	4.523	6.00	21.53	0.63
+ ID2= 2 (5176):	48.80	0.959	6.75	29.08	0.63
ID = 3 (8056):	210.80	5.052	6.50	23.28	0.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8066)		AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	(mm)
ID1= 1 (8056):	210.80	5.052	6.50	23.28	0.63
+ ID2= 2 (8058):	1676.12	18.020	7.00	20.88	0.63
ID = 3 (8066):	1886.92	22.176	6.75	21.15	0.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8070)		AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	(mm)
ID1= 1 (8066):	1886.92	22.176	6.75	21.15	0.63
+ ID2= 2 (8068):	650.32	18.537	6.25	26.48	0.63
ID = 3 (8070):	2547.24	37.913	6.50	22.53	0.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTING TIME STEP (min) = 15.00

DATA FOR SECTION (1901.0) -----

Distance	Elevation	Manning
0.00	261.34	0.0600
19.76	261.64	0.0600
59.28	261.29	0.0600
79.04	261.09	0.0600
103.74	255.99	0.0600
153.13	253.42	0.0600
192.65	252.78	0.0600
212.39	249.80	0.0600
217.29	249.50	0.0600 / 0.0350
217.30	249.05	0.0350
222.29	249.05	0.0350
222.30	249.50	0.0350 / 0.0600
227.30	249.80	0.0600
232.17	252.31	0.0600
276.63	253.53	0.0600
326.03	256.97	0.0600
355.67	257.30	0.0600
400.12	260.08	0.0600
469.28	261.25	0.0600
489.04	262.53	0.0600

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.45	249.50	436E+04	1.0	0.43	75.97
1.07	250.12	205E+05	5.4	0.51	62.96
1.70	250.75	451E+05	14.8	0.64	50.78
2.32	251.37	762E+05	29.2	0.74	43.46
2.94	251.99	114E+06	49.1	0.84	38.61
3.57	252.62	161E+06	70.2	0.85	38.18
4.19	253.24	248E+06	93.4	0.73	44.29
4.81	253.86	398E+06	154.9	0.76	42.79
5.44	254.49	576E+06	246.1	0.83	38.99
6.06	255.11	779E+06	362.7	0.90	35.79
6.68	255.73	101E+07	506.5	0.98	33.15
7.30	256.35	126E+07	687.6	1.06	30.52
7.93	256.98	153E+07	902.8	1.15	28.18
8.55	257.60	183E+07	1114.3	1.18	27.39
9.17	258.22	216E+07	1407.8	1.26	25.55
9.80	258.85	250E+07	1735.4	1.35	24.03
10.42	259.47	286E+07	2097.5	1.42	22.73
11.04	260.09	324E+07	2492.3	1.49	21.63
11.67	260.72	364E+07	2837.8	1.51	21.39

hydrograph <---> <--- pipe / channel --->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8070)	2547.24	37.91	6.50	22.53	2.59	0.78
OUTFLOW: ID= 1 (6190)	2547.24	30.08	7.25	22.53	2.35	0.75

CALIB STANDHYD (0190)

Area (ha) = 287.99
Total Imp(%) = 51.00
Dir. Conn.(%) = 27.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	146.87	141.12
Dep. Storage (mm)	3.00	5.00
Average Slope (%)	0.10	0.10
Length (m)	1385.62	40.00
Mannings n	0.013	0.250
Max.Eff.Inten.(mm/hr) over (min)	56.43 / 30.00	38.92 / 60.00
Storage Coeff. (min)	31.03 (ii)	56.31 (ii)
Unit Hyd. Tpeak (min)	30.00	60.00
Unit Hyd. Tpeak (cms)	0.04	0.02
PEAK FLOW (cms)	7.87	6.28
TIME TO PEAK (hrs)	6.25	6.75
RUNOFF VOLUME (mm)	59.70	27.30
TOTAL RAINFALL (mm)	62.70	62.70
RUNOFF COEFFICIENT	0.95	0.44

TOTALS
11.410 (iii)
6.25

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 71.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8072)

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (0190)	287.99	11.410	6.25	36.05
+ ID= 2 (6190)	2547.24	30.076	7.25	22.53
ID= 3 (8072)	2835.23	38.774	6.75	23.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTING TIME STEP (min) = 15.00

DATA FOR SECTION (1921.0) -----

Distance	Elevation	Manning
0.00	264.40	0.0500
3.64	263.94	0.0500
14.54	261.05	0.0500
29.08	256.78	0.0500
36.36	254.96	0.0500
50.90	252.20	0.0500
65.44	249.78	0.0500
76.35	246.86	0.0500
77.24	245.90	0.0500
82.24	245.45	0.0500 / 0.0350
82.25	244.90	0.0350
87.25	244.90	0.0350
87.26	245.45	0.0350 / 0.0500
94.53	246.89	0.0500
107.87	247.16	0.0500
218.13	248.69	0.0500
265.40	249.22	0.0500
301.75	250.78	0.0500
338.11	253.47	0.0500
359.92	264.00	0.0500

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.55	245.45	345E+04	2.0	0.71	29.20
1.53	246.43	174E+05	14.6	1.05	19.92
2.50	247.40	767E+05	52.8	0.86	24.20
3.48	248.38	224E+06	199.8	1.11	18.71
4.46	249.36	429E+06	441.2	1.29	16.19
5.43	250.33	691E+06	874.3	1.58	13.18
6.41	251.31	987E+06	1468.3	1.86	11.20
7.38	252.28	131E+07	2224.8	2.13	9.79
8.36	253.26	165E+07	3132.4	2.37	8.78
9.34	254.24	201E+07	4256.4	2.65	7.87
10.31	255.21	238E+07	5551.0	2.92	7.15
11.29	256.19	276E+07	6996.8	3.17	6.57
12.27	257.17	314E+07	8585.9	3.42	6.10
13.24	258.14	353E+07	10315.9	3.65	5.71
14.22	259.12	393E+07	12180.7	3.87	5.38
15.19	260.09	434E+07	14178.2	4.09	5.10
16.17	261.07	475E+07	16306.5	4.29	4.85
17.15	262.05	517E+07	18558.6	4.49	4.64
18.12	263.02	559E+07	20939.7	4.68	4.45

hydrograph <---> <--- pipe / channel --->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8072)	2835.23	38.77	6.75	23.90	2.14	0.92
OUTFLOW: ID= 1 (6192)	2835.23	35.49	7.25	23.90	2.06	0.94

CALIB STANDHYD (1921)

Area (ha) = 30.70
Total Imp(%) = 48.00
Dir. Conn.(%) = 24.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	14.74	15.96
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	452.40	40.00
Mannings n	0.013	0.250
Max.Eff.Inten.(mm/hr) over (min)	82.76 / 15.00	22.66 / 30.00
Storage Coeff. (min)	8.39 (ii)	27.76 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. Tpeak (cms)	0.09	0.04
PEAK FLOW (cms)	1.50	0.71
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	60.70	16.95
TOTAL RAINFALL (mm)	62.70	62.70
RUNOFF COEFFICIENT	0.97	0.27

TOTALS
1.934 (iii)
6.00

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 54.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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RESERVOIR (5192)
IN= 2--> OUT= 1
DT= 15.0 min

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	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	2.0560	0.7051
	0.1040	0.3429	2.5820	0.7960
	0.5960	0.4035	3.2900	0.8880
	1.1090	0.5056	3.6900	0.8980
	1.5380	0.5974	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1921)	30.700	1.934	6.00	27.45
OUTFLOW : ID= 1 (5192)	30.700	0.638	6.75	27.42


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PEAK FLOW REDUCTION [Qout/Qin](%)= 33.00
TIME SHIFT OF PEAK FLOW (min)= 45.00
MAXIMUM STORAGE USED (ha.m.)= 0.4164

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CALIB
STANDHYD (1922)
ID= 1 DT=15.0 min

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	Area (ha)	Impr. (%)	Dir. Com. (%)
STANDHYD (1922)	215.90	27.00	14.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	58.29	157.41
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1199.72	40.00
Mannings n	0.013	0.250

	Max. Eff. Inten. (mm/hr)	cover (min)	Storage Coeff. (min)	Unit Hyd. Tpeak (min)	Unit Hyd. Tpeak (cms)
	82.76	15.01	15.00	15.00	0.07
	15.00	45.00	15.06 (ii)	37.91 (ii)	0.03
	15.00	45.00	15.00	15.00	0.07
	0.07	0.03			


```

**TOTALS*
PEAK FLOW (cms)= 5.05
TIME TO PEAK (hrs)= 6.00
RUNOFF VOLUME (mm)= 60.70
TOTAL RAINFALL (mm)= 62.70
RUNOFF COEFFICIENT = 0.97

```

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 54.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8074)
1 + 2 = 3

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	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1922):	215.90	6.517	6.00	20.64
+ ID2= 2 (5192):	30.70	0.638	6.75	27.42
ID = 3 (8074):	246.60	6.588	6.00	21.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8076)
1 + 2 = 3

```

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6192):	2835.23	35.485	7.25	23.90
+ ID2= 2 (8074):	246.60	6.588	6.00	21.48
ID = 3 (8076):	3081.83	38.304	7.25	23.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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CALIB
NASHYD (6196)
ID= 1 DT=15.0 min

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	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res. (N)
NASHYD (6196)	41.47	5.00	70.0	1.50

U.H. Tp(hrs)= 0.24

Unit Hyd Qpeak (cms)= 2.950

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PEAK FLOW (cms)= 1.088 (i)
TIME TO PEAK (hrs)= 6.000
RUNOFF VOLUME (mm)= 18.348
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.293

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(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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CALIB
NASHYD (0194)
ID= 1 DT=15.0 min

```

	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res. (N)
NASHYD (0194)	256.09	5.00	69.0	1.50

U.H. Tp(hrs)= 0.68

Unit Hyd Qpeak (cms)= 6.458

```

PEAK FLOW (cms)= 3.099 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 19.025
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.303

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8078)
1 + 2 = 3

```

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0194):	256.09	3.099	6.75	19.02
+ ID2= 2 (0196):	41.47	1.088	6.00	18.35
ID = 3 (8078):	297.56	3.962	6.50	18.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6198)
ID= 2--> OUT= 1
Routing time step (min)'= 15.00

```

<----- DATA FOR SECTION (1981.0) ----->

Distance	Elevation	Manning
0.00	267.15	0.0500
22.99	265.94	0.0500
51.73	261.39	0.0500
74.72	258.75	0.0500
94.83	257.79	0.0500
114.95	254.36	0.0500
126.44	254.06	0.0500
132.19	253.68	0.0500
135.06	253.35	0.0500
137.94	252.93	0.0500 /0.0300
140.81	252.41	0.0300
143.69	251.89	0.0300
146.56	252.51	0.0300
149.43	253.36	0.0300 /0.0500
158.05	255.89	0.0500
172.42	256.25	0.0500
204.03	259.15	0.0500
229.90	260.36	0.0500
252.89	260.46	0.0500
284.50	260.45	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.35	252.24	.111E+04	0.6	0.97	31.07
0.69	252.58	.442E+04	3.8	1.55	19.42
1.04	252.93	.974E+04	11.1	2.06	14.65
1.51	253.40	.203E+05	31.2	2.77	10.87
1.98	253.87	.357E+05	65.0	3.30	9.14
2.45	254.34	.596E+05	112.9	3.43	8.80
2.92	254.81	.935E+05	186.8	3.62	8.34
3.39	255.28	.131E+06	282.1	3.89	7.75
3.86	255.75	.172E+06	398.2	4.18	7.22
4.33	256.22	.221E+06	527.0	4.31	6.99
4.80	256.69	.282E+06	697.3	4.47	6.75
5.27	257.16	.350E+06	898.5	4.64	6.50
5.74	257.63	.425E+06	1131.4	4.82	6.26
6.21	258.10	.508E+06	1367.4	4.88	6.19
6.68	258.57	.603E+06	1635.6	4.91	6.14
7.15	259.04	.709E+06	1977.2	5.04	5.98
7.62	259.51	.826E+06	2360.5	5.17	5.83
8.09	259.98	.954E+06	2790.7	5.29	5.70
8.56	260.45	.110E+07	3179.6	5.25	5.74

```

<----- hydrograph -----> <-pipe / channel->

```

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8078)	297.56	3.96	6.50	18.93	0.70	1.56
OUTFLOW : ID= 1 (6198)	297.56	3.82	6.75	18.93	0.69	1.55

+ ID2= 2 (8080): 390.60 4.536 7.00 19.61
 ID = 3 (8082): 688.16 8.346 7.00 19.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (1981) Area (ha)= 104.70
 ID= 1 DT=15.0 min Total Imp(%)= 44.00 Dir. Conn.(%)= 20.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	46.07	58.63
Dep. Storage (mm)=	2.00	5.00
Average Slope (ft)=	0.50	0.50
Length (m)=	835.46	40.00
Mannings n	= 0.013	0.250
Max.Eff.Inten.(mm/hr)=	82.76	24.55
over (min)	15.00	45.00
Storage Coeff. (min)=	12.12 (ii)	30.89 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03
TOTALS		
PEAK FLOW (cms)=	3.81	2.44
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	60.70	19.65
TOTAL RAINFALL (mm)=	62.70	62.70
RUNOFF COEFFICIENT	= 0.97	0.30

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING:FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 58.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5198)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	7.9640	2.3173
0.2850	1.2270	9.5520	2.6039
1.7410	1.4052	10.9290	2.8853
3.5350	1.7675	11.3290	2.8953
5.6580	2.0183	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (1981)	104.700	4.829	6.00	27.06
OUTFLOW: ID= 1 (5198)	104.700	1.882	7.00	27.05

PEAK FLOW REDUCTION [Qout/Qin](%)= 38.98
 TIME SHIFT OF PEAK FLOW (min)= 60.00
 MAXIMUM STORAGE USED (ha.m.)= 1.4366

CALIB (1982) Area (ha)= 285.90 Curve Number (CN)= 65.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.81

Unit Hyd Qpeak (cms)= 6.048

PEAK FLOW (cms)= 2.653 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 16.880
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.269

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8080)
 1 + 2 = 3
 ID1= 1 (1982): 285.90 2.653 7.00 16.88
 + ID2= 2 (5198): 104.70 1.882 7.00 27.05
 ID = 3 (8080): 390.60 4.536 7.00 19.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8082)
 1 + 2 = 3
 ID1= 1 (6198): 297.56 3.820 6.75 18.93

ADD HYD (8084)
 1 + 2 = 3
 ID1= 1 (8076): 3081.83 38.304 7.25 23.91
 + ID2= 2 (8082): 688.16 8.346 7.00 19.31
 ID = 3 (8084): 3769.99 46.292 7.25 22.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6200)
 IN= 2--> OUT= 1 Routing time step (min)= 15.00

Distance	Elevation	Manning
0.00	252.47	0.0500
10.38	252.29	0.0500
20.76	252.28	0.0500
41.52	247.66	0.0500
62.28	247.47	0.0500
80.45	247.34	0.0500
90.83	246.85	0.0500
103.81	245.51	0.0500
106.00	244.50	0.0500
108.99	244.05	0.0500 / 0.0300
109.00	243.50	0.0300
114.00	243.50	0.0300
114.01	244.05	0.0300 / 0.0500
118.99	244.50	0.0500
132.36	245.09	0.0500
150.52	245.83	0.0500
199.83	249.24	0.0500
207.62	250.16	0.0500
236.16	253.11	0.0500
256.92	253.83	0.0500

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.28	243.77	.365E+04	0.7	0.84	81.42
0.55	244.05	.730E+04	2.2	0.81	54.59
1.05	244.55	.197E+05	7.2	0.97	45.51
1.54	245.04	.463E+05	17.2	0.99	44.76
2.04	245.54	.895E+05	34.8	1.03	42.81
2.53	246.03	.152E+06	62.8	1.09	40.43
3.03	246.53	.232E+06	104.7	1.20	36.91
3.52	247.02	.328E+06	159.7	1.29	34.19
4.02	247.52	.450E+06	223.4	1.32	33.56
4.51	248.01	.627E+06	326.9	1.38	31.99
5.01	248.51	.820E+06	463.0	1.50	29.52
5.50	249.00	.103E+07	626.0	1.62	27.30
6.00	249.50	.124E+07	820.1	1.75	25.24
6.49	249.99	.147E+07	1044.4	1.89	23.42
6.99	250.49	.170E+07	1292.6	2.01	21.95
7.48	250.98	.195E+07	1565.6	2.13	20.71
7.98	251.48	.220E+07	1864.5	2.25	19.65
8.47	251.97	.246E+07	2189.4	2.36	18.73
8.97	252.47	.274E+07	2427.5	2.35	18.80

hydrograph
 INFLOW : ID= 2 (8084) 3769.99 46.29 7.25 22.91 2.24 1.06
 OUTFLOW: ID= 1 (6200) 3769.99 40.51 8.00 22.91 2.13 1.04

CALIB (2001) Area (ha)= 6.00
 ID= 1 DT=15.0 min Total Imp(%)= 50.00 Dir. Conn.(%)= 23.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	3.00	3.00
Dep. Storage (mm)=	2.00	5.00
Average Slope (ft)=	0.50	0.50
Length (m)=	200.00	40.00
Mannings n	= 0.013	0.250

Max.Eff.Inten.(mm/hr)= 82.76 59.25
 over (min) 15.00 30.00
 Storage Coeff. (min)= 5.14 (ii) 18.33 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.11 0.05

*****TOTALS*
 PEAK FLOW (cms)= 0.31 0.26 0.472 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 60.70 25.12 33.30
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.40 0.53

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 67.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5200)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.4010	0.1620
0.0200	0.0789	0.5040	0.1833
0.1160	0.0921	0.5870	0.2041
0.2170	0.1159	0.9870	0.2141
0.3000	0.1370	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
6.000	0.472	6.00	33.30
6.000	0.155	6.75	33.14

INFLOW : ID= 2 (2001)
 OUTFLOW: ID= 1 (5200)

PEAK FLOW REDUCTION [Qout/Qin](%)= 32.75
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 0.1023

CALIB
 STANDHYD (2002)
 ID= 1 DT=15.0 min

Area (ha)= 109.30
 Total Imp(%)= 22.00 Dir. Conn.(%)= 10.00

IMPERVIOUS (i)	PERVIOUS (i)
24.05	85.25
2.00	5.00
0.50	0.50
853.62	40.00
0.013	0.250

Max. Eff. Inten. (mm/hr)= 82.76 21.92
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 12.28 (ii) 31.91 (iii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03

*****TOTALS*
 PEAK FLOW (cms)= 1.98 3.11 3.596 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.50
 RUNOFF VOLUME (mm)= 60.70 20.42 24.45
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.33 0.39

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 67.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8086)
 1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
109.30	3.596	6.50	24.45
6.00	0.155	6.75	33.14
115.30	3.745	6.50	24.90

ID1= 1 (2002):
 + ID2= 2 (5200):
 ID = 3 (8086):

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8088)
 1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
3769.99	40.511	8.00	22.91
115.30	3.745	6.50	24.90
3885.29	41.457	8.00	22.96

ID1= 1 (6200):
 + ID2= 2 (8086):
 ID = 3 (8088):

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8038)
 1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
5220.81	14.056	12.00	12.36
3885.29	41.457	8.00	22.96
9106.10	52.859	7.75	16.89

ID1= 1 (8034):
 + ID2= 2 (8088):
 ID = 3 (8038):

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (1365)
 IN= 2--> OUT= 1
 Routing time step (min)= 15.00

DATA FOR SECTION (1362.0)

Distance	Elevation	Manning
0.00	252.00	0.0400
2.35	244.50	0.0400
39.94	243.14	0.0400
58.74	242.65	0.0400
91.63	242.03	0.0400
101.03	241.69	0.0400
103.38	241.66	0.0400
105.73	240.76	0.0400
105.93	240.59	0.0400
109.18	240.28	0.0400 / 0.0300
110.43	239.81	0.0300
111.68	239.81	0.0300
113.43	240.09	0.0300 / 0.0400
118.43	240.50	0.0400
138.62	241.68	0.0400
152.72	241.60	0.0400
166.82	242.68	0.0400
185.62	249.09	0.0400
197.36	251.00	0.0400
232.61	252.28	0.0400

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.28	240.09	118E+04	0.1	0.20	142.09
0.91	240.72	114E+05	2.4	0.35	79.92
1.53	241.34	352E+05	9.4	0.45	62.17
2.16	241.97	831E+05	22.8	0.46	60.69
2.79	242.60	172E+06	54.2	0.53	52.74
3.41	243.22	297E+06	111.4	0.63	44.49
4.04	243.85	445E+06	194.1	0.74	38.22
4.67	244.48	612E+06	301.1	0.83	33.88
5.29	245.10	793E+06	443.0	0.94	29.82
5.92	245.73	976E+06	611.3	1.06	26.60
6.55	246.36	1168E+07	802.0	1.17	24.12
7.18	246.99	1358E+07	1014.0	1.27	22.16
7.80	247.61	154E+07	1246.2	1.37	20.56
8.43	248.24	173E+07	1498.0	1.46	19.24
9.06	248.87	192E+07	1768.8	1.55	18.12
9.68	249.49	212E+07	2049.6	1.63	17.24
10.31	250.12	232E+07	2344.7	1.71	16.49
10.94	250.75	252E+07	2658.5	1.78	15.83
11.56	251.37	274E+07	2931.2	1.81	15.56

hydrograph

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
9106.10	52.86	7.75	16.89	2.76	0.53
9106.10	45.64	8.75	16.89	2.61	0.51

INFLOW : ID= 2 (8038)
 OUTFLOW: ID= 1 (1365)

CALIB
 NASHYD (1322)
 ID= 1 DT=15.0 min

Area (ha)= 249.10 Curve Number (CN)= 73.0
 Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.83

Unit Hyd Qpeak (cms)= 5.130
 PEAK FLOW (cms)= 2.946 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 21.659
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.345

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (1321)
 ID= 1 DT=15.0 min

Area (ha)= 41.70
 Total Imp(%)= 55.00 Dir. Conn.(%)= 35.00

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-----
Surface Area (ha)= 22.94 18.77
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 527.26 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 82.76 57.89
over (min) 15.00 30.00
Storage Coeff. (min)= 9.20 (ii) 22.51 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.09 0.04

*TOTALS*
PEAK FLOW (cms)= 2.90 1.45 3.820 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 8.00
RUNOFF VOLUME (mm)= 60.70 26.07 38.19
TOTAL RAINFALL (mm)= 62.70 62.70 62.70
RUNOFF COEFFICIENT = 0.97 0.42 0.61

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
RESERVOIR (S132)
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
OUTFLOW STORAGE OUTFLOW STORAGE
(cms) (ha.m.) (cms) (ha.m.)
0.0000 0.0000 | 0.9630 1.4050
0.1780 1.0280 | 1.4870 1.5525
0.4300 1.0400 | 1.9900 1.7000
0.8150 1.0500 | 2.3900 1.7100
0.8990 1.2275 | 0.0000 0.0000

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (1321) 41.700 3.820 6.00 38.19
OUTFLOW: ID= 1 (S132) 41.700 0.743 7.00 38.16

PEAK FLOW REDUCTION [Qout/Qin](%)= 19.45
TIME SHIFT OF PEAK FLOW (min)= 60.00
MAXIMUM STORAGE USED (ha.m.)= 1.0504

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-----
ADD HYD (8090)
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (1322): 249.10 2.946 7.00 21.66
+ ID2= 2 (S132): 41.70 0.743 7.00 38.16
=====
ID = 3 (8090): 290.80 3.689 7.00 24.02

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
CALIB (0134) Area (ha)= 155.95 Curve Number (CN)= 64.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.64 |
-----
Unit Hyd Qpeak (cms)= 4.167

PEAK FLOW (cms)= 1.675 (i)
TIME TO PEAK (hrs)= 6.750 (min)= 60.00
RUNOFF VOLUME (mm)= 16.270
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.259

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
ADD HYD (8092)
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0134): 155.95 1.675 6.75 16.27
+ ID2= 2 (8090): 290.80 3.689 7.00 24.02
=====
ID = 3 (8092): 446.75 5.337 7.00 21.32

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (1366) |

| IN= 2--> OUT= 1 | Routing time step (min)= 15.00

```

-----
<----- DATA FOR SECTION (1361.0) ----->
Distance Elevation Manning
0.00 262.81 0.0400
37.23 260.54 0.0400
79.12 256.98 0.0400
111.70 254.74 0.0400
172.30 252.76 0.0400
251.32 248.82 0.0400
255.97 248.75 0.0400
260.43 248.76 0.0400
265.43 248.49 0.0400
268.68 248.18 0.0400 /0.0300 Main Channel
269.93 247.71 0.0300 Main Channel
271.18 247.71 0.0300 Main Channel
272.93 247.99 0.0300 Main Channel
277.93 248.40 0.0300 /0.0400 Main Channel
283.90 248.88 0.0400
307.17 249.60 0.0400
321.13 251.59 0.0400
363.01 257.08 0.0400
400.25 257.58 0.0400
460.75 261.91 0.0400

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<----- TRAVEL TIME TABLE ----->
DEPTH ELV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.47 248.18 .881E+04 1.3 0.76 114.66
1.19 248.90 .742E+05 17.3 1.22 71.44
1.92 249.63 .278E+06 78.3 1.48 59.26
2.64 250.35 .589E+06 210.3 1.87 46.69
3.36 251.07 .974E+06 414.3 2.23 39.19
4.08 251.79 .143E+07 696.2 2.55 34.32
4.81 252.52 .197E+07 1063.0 2.83 30.86
5.53 253.24 .259E+07 1505.7 3.06 28.62
6.25 253.96 .331E+07 2051.3 3.26 26.85
6.97 254.68 .413E+07 2720.9 3.46 25.30
7.70 255.41 .504E+07 3584.5 3.73 23.43
8.42 256.13 .601E+07 4580.9 4.00 21.87
9.14 256.85 .704E+07 5707.9 4.25 20.57
9.86 257.57 .818E+07 6636.1 4.26 20.53
10.59 258.30 .945E+07 8115.0 4.50 19.42
11.31 259.02 .108E+08 9759.2 4.74 18.45
12.03 259.74 .122E+08 11569.5 4.96 17.61
12.75 260.46 .137E+08 13550.6 5.18 16.87
13.48 261.19 .153E+08 15636.4 5.37 16.29

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<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8092) 446.75 5.34 7.00 21.32 0.65 0.84
OUTFLOW: ID= 1 (1366) 446.75 3.47 8.75 21.32 0.57 0.80

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CALIB
| STANDHYD (1362) | Area (ha)= 410.10
| ID= 1 DT=15.0 min | Total Imp(%)= 21.00 Dir. Conn.(%)= 11.00

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-----
Surface Area (ha)= 86.12 323.98
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1653.48 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 82.76 19.66
over (min) 15.00 45.00
Storage Coeff. (min)= 18.26 (ii) 38.77 (ii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.06 0.03

*TOTALS*
PEAK FLOW (cms)= 6.91 9.60 12.076 (iii)
TIME TO PEAK (hrs)= 6.00 6.50 8.50
RUNOFF VOLUME (mm)= 60.70 18.89 23.49
TOTAL RAINFALL (mm)= 62.70 62.70 62.70
RUNOFF COEFFICIENT = 0.97 0.30 0.37

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***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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CALIB
| STANDHYD (1361) | Area (ha)= 102.30
| ID= 1 DT=15.0 min | Total Imp(%)= 55.00 Dir. Conn.(%)= 27.00

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	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	56.27	46.03	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	825.83	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	82.76	61.22	
over (min)	15.00	30.00	
Storage Coeff. (min)=	12.04 (ii)	25.06 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.08	0.04	
			TOTALS
PEAK FLOW (cms)=	5.04	3.56	7.290 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	60.70	24.69	34.42
TOTAL RAINFALL (mm)=	62.70	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.39	0.55

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5136)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	2.4000	3.2690
	0.1600	1.6363	2.6450	3.8640
	0.9600	1.7470	2.8900	4.4590
	1.8400	1.9320	3.2900	4.4690
	2.1200	2.6005	0.0000	0.0000
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (1361)	102.300	7.290	6.00	34.42
OUTFLOW: ID= 1 (5136)	102.300	1.876	7.00	34.39
	PEAK FLOW REDUCTION [Qout/qin](%)=	25.73		
	TIME SHIFT OF PEAK FLOW (min)=	60.00		
	MAXIMUM STORAGE USED (ha.m.)=	2.0230		

ADD HYD (8096)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1362):	410.10	12.076	6.50	23.49
+ ID2= 2 (5136):	102.30	1.876	7.00	34.39
=====				
ID = 3 (8096):	512.40	13.599	6.50	25.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8094)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1 = 1 (1366):	446.75	3.469	8.75	21.32
+ ID2= 2 (8096):	512.40	13.599	6.50	25.67
=====				
ID = 3 (8094):	959.15	14.848	6.50	23.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8098)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1 = 1 (1365):	9106.10	45.638	8.75	16.89
+ ID2= 2 (8094):	959.15	14.848	6.50	23.64
=====				
ID = 3 (8098):	10065.25	52.234	8.50	17.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (8142)			
NASHYD (0138) Area (ha)= 396.54 Curve Number (CN)= 63.0			
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50			
----- U.H. Tp(hrs)= 1.32			
Unit Hyd Qpeak	(cms)=	5.133	

PEAK FLOW (cms)=	2.377 (i)
TIME TO PEAK (hrs)=	8.000
RUNOFF VOLUME (mm)=	15.984
TOTAL RAINFALL (mm)=	62.700
RUNOFF COEFFICIENT =	0.255

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0138)			
NASHYD (0138) Area (ha)= 414.79 Curve Number (CN)= 24.0			
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50			
----- U.H. Tp(hrs)= 1.39			

Unit Hyd Qpeak (cms)= 5.087

PEAK FLOW (cms)=	0.559 (i)
TIME TO PEAK (hrs)=	8.250
RUNOFF VOLUME (mm)=	3.838
TOTAL RAINFALL (mm)=	62.700
RUNOFF COEFFICIENT =	0.061

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0140)			
STANDHYD (0140) Area (ha)= 158.58 Dir. Conn.(%)= 21.00			
ID= 1 DT=15.0 min Total Imp(%)= 24.00			

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	38.06	120.52
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1028.20	40.00
Mannings n =	0.013	0.250

Max.Eff.Inten.(mm/hr)=	82.76	5.73
over (min)	15.00	60.00
Storage Coeff. (min)=	13.73 (ii)	47.30 (ii)
Unit Hyd. Tpeak (min)=	15.00	60.00
Unit Hyd. peak (cms)=	0.08	0.02

			TOTALS
PEAK FLOW (cms)=	5.78	1.16	6.125 (iii)
TIME TO PEAK (hrs)=	6.00	6.75	6.00
RUNOFF VOLUME (mm)=	60.70	8.18	19.21
TOTAL RAINFALL (mm)=	62.70	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.13	0.31

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 41.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8102)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0138):	414.79	0.559	8.25	3.84
+ ID2= 2 (0140):	158.58	6.125	6.00	19.21
=====				
ID = 3 (8102):	573.37	6.374	6.00	8.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6142)	
IN= 2--> OUT= 1	
Routing time step (min)= 15.00	

<----- DATA FOR SECTION (1421.0) ----->

Distance	Elevation	Manning	
0.00	295.50	0.0400	
7.25	295.36	0.0400	
14.50	295.22	0.0400	
21.75	295.22	0.0400	
29.00	294.23	0.0400	
52.56	286.45	0.0400	
67.06	281.44	0.0400	
77.94	278.28	0.0400	
82.81	277.20	0.0400 / 0.0300	Main Channel
83.21	276.80	0.0300	Main Channel
88.81	276.80	0.0300	Main Channel
96.41	276.80	0.0300	Main Channel
96.81	277.20	0.0300 / 0.0400	Main Channel
103.31	278.96	0.0400	
108.75	280.16	0.0400	
117.81	282.73	0.0400	

135.94 290.30 0.0400
 159.50 292.14 0.0400
 174.00 293.07 0.0400
 179.44 293.37 0.0400

242.44 249.80 0.0300 Main Channel
 250.04 249.80 0.0300 Main Channel
 250.44 250.20 0.0300 Main Channel
 252.33 250.57 0.0300 / 0.0350 Main Channel
 286.97 253.52 0.0350
 331.50 255.80 0.0350
 380.98 255.55 0.0350
 445.30 258.40 0.0350
 455.19 259.82 0.0350
 489.83 262.22 0.0350

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.40	277.20	238E+05	6.9	1.27	57.28
1.25	278.05	889E+05	50.9	2.50	29.13
2.10	278.90	179E+06	133.4	3.26	22.36
2.95	279.75	293E+06	256.7	3.83	19.03
3.80	280.60	432E+06	426.0	4.31	16.90
4.66	281.46	593E+06	644.3	4.75	15.35
5.51	282.31	776E+06	915.3	5.16	14.12
6.36	283.16	978E+06	1242.2	5.55	13.12
7.21	284.01	120E+07	1626.2	5.94	12.27
8.06	284.86	143E+07	2065.6	6.30	11.57
8.91	285.71	169E+07	2562.4	6.64	10.97
9.76	286.56	196E+07	3118.0	6.97	10.46
10.61	287.41	224E+07	3733.3	7.28	10.02
11.46	288.26	255E+07	4411.9	7.57	9.62
12.31	289.11	287E+07	5155.6	7.86	9.27
13.17	289.97	321E+07	5965.6	8.14	8.96
14.02	290.82	357E+07	6705.0	8.22	8.87
14.87	291.67	398E+07	7494.3	8.24	8.85
15.72	292.52	444E+07	8405.3	8.28	8.80

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.39	250.19	1.69E+05	5.4	1.04	52.00
0.77	250.57	3.73E+05	16.6	1.44	37.46
1.44	251.24	9.11E+05	60.3	2.14	25.19
2.11	251.91	1.69E+06	136.3	2.61	20.64
2.79	252.59	2.70E+06	249.7	2.99	18.05
3.46	253.26	3.96E+06	405.4	3.31	16.28
4.13	253.93	5.49E+06	603.3	3.56	15.15
4.80	254.60	7.37E+06	857.8	3.77	14.31
5.48	255.28	9.63E+06	1174.6	3.95	13.67
6.15	255.95	1.28E+07	1449.5	3.66	14.74
6.82	256.62	1.73E+07	2046.4	3.83	14.07
7.49	257.29	2.27E+07	2665.2	3.81	14.17
8.17	257.97	3.10E+07	3705.2	3.87	13.93
8.84	258.64	4.00E+07	5127.4	4.15	12.99
9.51	259.31	4.92E+07	6858.6	4.51	11.95
10.18	259.98	5.87E+07	8815.5	4.86	11.09
10.86	260.66	6.84E+07	10954.6	5.18	10.41
11.53	261.33	7.85E+07	13244.3	5.49	9.82
12.20	262.00	8.90E+07	15923.2	5.79	9.31

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8102)	573.37	6.37	6.00	8.09	0.37
OUTFLOW : ID= 1 (6142)	573.37	2.57	6.75	8.09	0.15

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8106)	1105.60	6.55	7.00	12.66	0.42
OUTFLOW : ID= 1 (6146)	1105.60	5.67	8.25	12.66	0.39

ADD HYD (8104)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1 = 1 (0142)	396.54	2.377	8.00	15.98
+ ID2= 2 (6142)	573.37	2.571	6.75	8.09
ID = 3 (8104)	969.91	4.695	7.00	11.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0144)	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res. (N)
ID= 1 DT=15.0 min	135.69	5.00	74.0	1.50
U.N. Tp(hrs)		0.71		
Unit Hyd Qpeak (cms)	3.240			
PEAK FLOW (cms)	1.858 (i)			
TIME TO PEAK (hrs)	6.750			
RUNOFF VOLUME (mm)	22.277			
TOTAL RAINFALL (mm)	62.700			
RUNOFF COEFFICIENT	0.355			

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8106)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1 = 1 (0144)	135.69	1.858	6.75	22.28
+ ID2= 2 (8104)	969.91	4.695	7.00	11.32
ID = 3 (8106)	1105.60	6.546	7.00	12.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6146)	Routing time step (min)
IN= 2--> OUT= 1	15.00

----- DATA FOR SECTION (1461.0) -----

Distance	Elevation	Manning
0.00	262.00	0.0350
39.58	257.13	0.0350
123.69	257.51	0.0350
183.07	256.83	0.0350
202.86	255.73	0.0350
212.75	254.73	0.0350
222.65	252.86	0.0350
232.44	250.70	0.0350 / 0.0300
236.44	250.20	0.0300
236.84	249.80	0.0300

CALIB STANDHYD (1462)	Area (ha)	Total Imp(%)	Dir. Conn.(%)
ID= 1 DT=15.0 min	125.00	29.00	12.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	36.25
Dep. Storage (mm)	2.00
Average Slope (%)	0.50
Length (m)	912.87
Mannings n	0.013
Max.Eff.Inten.(mm/hr) over (min)	82.76 / 15.00
Storage Coeff. (min)	12.79 (ii)
Unit Hyd. Tpeak (min)	15.00
Unit Hyd. peak (cms)	0.08
PEAK FLOW (cms)	2.68
TIME TO PEAK (hrs)	6.00
RUNOFF VOLUME (mm)	60.70
TOTAL RAINFALL (mm)	62.70
RUNOFF COEFFICIENT	0.97

TOTALS
 6.421 (lii)
 29.81
 62.70
 0.48

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 73.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1461)	Area (ha)	Total Imp(%)	Dir. Conn.(%)
ID= 1 DT=15.0 min	70.50	36.00	15.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	25.38
Dep. Storage (mm)	2.00
Average Slope (%)	0.50
Length (m)	685.57
Mannings n	0.013
Max.Eff.Inten.(mm/hr) over (min)	82.76 / 15.00
Storage Coeff. (min)	10.77 (ii)
Unit Hyd. Tpeak (min)	15.00
Unit Hyd. peak (cms)	0.09

TOTALS
 24.37 (lii)
 30.00
 0.04

PEAK FLOW (cms)= 2.01 3.17 4.015 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 60.70 26.79 31.87
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.43 0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 73.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5146)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	8.0640	2.2344	
0.2390	0.7085	9.9830	2.6409	
2.2880	0.8391	11.7350	3.0631	
4.3270	1.4661	12.1350	3.0731	
6.1170	1.8197	0.0000	0.0000	

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (1461)	70.500	4.015	6.00	31.87
OUTFLOW: ID= 1 (5146)	70.500	2.478	6.50	31.86

PEAK FLOW REDUCTION [Qout/Qin](%)= 61.72
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 0.9066

ADD HYD (8099)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1462):	125.00	6.421	6.25	29.81
+ ID2= 2 (5146):	70.50	2.478	6.50	31.86
=====				
ID = 3 (8099):	195.50	7.431	6.25	30.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8108)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6146):	1105.60	5.671	8.25	12.66
+ ID2= 2 (8099):	195.50	7.431	6.25	30.55
=====				
ID = 3 (8108):	1301.10	10.051	6.50	15.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8100)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8098):	10065.25	52.234	8.50	17.53
+ ID2= 2 (8108):	1301.10	10.051	6.50	15.35
=====				
ID = 3 (8100):	11366.35	59.112	8.25	17.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6202)
 IN= 2--> OUT= 1
 Routing time step (min)= 15.00

<----- DATA FOR SECTION (2021.0) ----->				
Distance	Elevation	Manning		
0.00	245.48	0.0400		
29.68	242.80	0.0400		
74.20	240.39	0.0400		
155.81	237.12	0.0400		
200.33	234.48	0.0400		
244.85	234.67	0.0400		
259.69	233.96	0.0400		
274.53	233.84	0.0400		
281.68	233.40	0.0400		
285.78	232.40	0.0400 / 0.0300	Main Channel	
286.78	231.40	0.0300	Main Channel	
296.78	231.40	0.0300	Main Channel	

299.78 232.40 0.0300 / 0.0400 Main Channel
 311.62 233.74 0.0400
 333.88 234.34 0.0400
 497.11 235.32 0.0400
 571.31 237.32 0.0400
 675.18 239.10 0.0400
 704.86 242.71 0.0400
 734.54 242.62 0.0400

<----- TRAVEL TIME TABLE ----->						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.50	231.90	221E+05	4.7	0.85	78.67	
1.00	232.40	482E+05	15.3	1.28	52.40	
1.60	233.00	913E+05	38.3	1.68	39.77	
2.20	233.60	154E+06	72.9	1.90	35.24	
2.80	234.20	278E+06	124.2	1.79	37.30	
3.40	234.80	599E+06	214.3	1.44	46.59	
4.01	235.41	125E+07	445.0	1.43	46.90	
4.61	236.01	205E+07	838.4	1.64	40.81	
5.21	236.61	293E+07	1361.6	1.86	35.89	
5.81	237.21	389E+07	2011.2	2.08	32.23	
6.41	237.81	494E+07	2753.5	2.24	29.91	
7.01	238.41	612E+07	3635.0	2.39	28.04	
7.61	239.01	741E+07	4672.5	2.53	26.43	
8.21	239.61	880E+07	5999.3	2.74	24.44	
8.82	240.22	102E+08	7510.0	2.94	22.72	
9.42	240.82	117E+08	9201.9	3.15	21.23	
10.02	241.42	132E+08	11059.7	3.35	19.95	
10.62	242.02	148E+08	13073.4	3.54	18.87	
11.22	242.62	164E+08	15242.1	3.73	17.93	

<----- hydrograph ----> <-pipe / channel-->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW: ID= 2 (8100) ***** 59.11 8.25 17.28 1.96 1.81
 OUTFLOW: ID= 1 (6202) ***** 57.31 9.00 17.28 1.93 1.79

ADD HYD (8112)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6202):	1136.35	57.307	9.00	17.28
+ ID2= 2 (8110):	468.80	24.976	6.00	38.08
=====				
ID = 3 (8112):	11834.85	60.228	8.75	18.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5555)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	*****	19.2100	
2.8500	0.5200	*****	24.7300	
8.1800	1.2600	*****	30.8600	
15.0900	2.2100	*****	37.6100	
23.2700	3.3900	*****	41.2200	
32.5300	4.7800	*****	48.9000	
36.5100	5.3900	*****	66.4700	
42.7600	6.3700	*****	100.1200	
53.8500	8.1300	*****	140.4100	
91.7300	14.3200	*****	140.4200	

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (8112)	*****	60.228	8.75	18.10
OUTFLOW: ID= 1 (5555)	*****	59.458	9.50	18.10

PEAK FLOW REDUCTION [Qout/Qin](%)= 98.72
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 9.0587

ADD HYD (8114)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (5555):	11834.85	59.458	9.50	18.10
+ ID2= 2 (8136):	2288.40	17.785	6.75	17.66
=====				
ID = 3 (8114):	14123.25	70.836	9.00	18.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (2061) | Area (ha)= 25.00

|ID= 1 DT=15.0 min | Total Imp(%)= 93.00 Dir. Conn.(%)= 60.00

Surface Area (ha)= 23.25 1.75
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 408.25 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 82.76 424.60
over (min)= 15.00 15.00
Storage Coeff. (min)= 7.89 (ii) 13.89 (iii)
Unit Hyd. Tpeak (min)= 15.00 15.00
Unit Hyd. peak (cms)= 0.10 0.07
TOTALS
PEAK FLOW (cms)= 3.10 1.53 4.629 (iii)
TIME TO PEAK (hrs)= 6.00 6.00
RUNOFF VOLUME (mm)= 60.70 49.87 56.37
TOTAL RAINFALL (mm)= 62.70 62.70 62.70
RUNOFF COEFFICIENT = 0.97 0.80 0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5206)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
0.0000 0.0000 | 1.8990 1.7095
0.0850 0.5280 | 2.1560 1.9661
0.8990 0.8577 | 2.3740 2.2023
1.3460 1.1501 | 2.7740 2.2123
1.6060 1.4451 | 0.0000 0.0000

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (2061) 25.000 4.629 6.00 56.37
OUTFLOW: ID= 1 (5206) 25.000 0.896 6.50 56.31

PEAK FLOW REDUCTION [Qout/Qin](%)= 19.35
TIME SHIFT OF PEAK FLOW (min)= 30.00
MAXIMUM STORAGE USED (ha.m.)= 0.8595

CALIB
STANDHYD (2062)
ID= 1 DT=15.0 min

Area (ha)= 250.50
Total Imp(%)= 62.00 Dir. Conn.(%)= 39.00

Surface Area (ha)= 155.31 95.19
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1292.28 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 82.76 78.59
over (min)= 15.00 30.00
Storage Coeff. (min)= 15.95 (ii) 27.53 (iii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.07 0.04
TOTALS
PEAK FLOW (cms)= 16.02 9.08 21.850 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 60.70 31.61 42.95
TOTAL RAINFALL (mm)= 62.70 62.70 62.70
RUNOFF COEFFICIENT = 0.97 0.50 0.69

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8144)

1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (2062): 250.50 21.850 6.00 42.95
+ ID2= 2 (5206): 25.00 0.896 6.50 56.31
ID = 3 (8144): 275.50 22.076 6.00 44.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (2041)
ID= 1 DT=15.0 min
Area (ha)= 201.50
Total Imp(%)= 36.00 Dir. Conn.(%)= 20.00

Surface Area (ha)= 72.54 128.96
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1159.02 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 82.76 21.02
over (min)= 15.00 45.00
Storage Coeff. (min)= 14.76 (ii) 34.72 (iii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.07 0.03
TOTALS
PEAK FLOW (cms)= 6.80 4.33 8.582 (iii)
TIME TO PEAK (hrs)= 6.00 6.50 6.00
RUNOFF VOLUME (mm)= 60.70 18.27 26.75
TOTAL RAINFALL (mm)= 62.70 62.70 62.70
RUNOFF COEFFICIENT = 0.97 0.29 0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 61.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5204)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
0.0000 0.0000 | 12.4660 4.9468
0.5050 1.9141 | 15.4770 5.3726
2.8390 2.2056 | 18.2610 6.4806
5.2410 3.6606 | 18.6610 6.4906
7.9920 4.3236 | 0.0000 0.0000

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (2041) 201.500 8.582 6.00 26.75
OUTFLOW: ID= 1 (5204) 201.500 3.314 7.00 26.75

PEAK FLOW REDUCTION [Qout/Qin](%)= 38.61
TIME SHIFT OF PEAK FLOW (min)= 60.00
MAXIMUM STORAGE USED (ha.m.)= 2.5036

CALIB
STANDHYD (2042)
ID= 1 DT=15.0 min

Area (ha)= 122.90
Total Imp(%)= 60.00 Dir. Conn.(%)= 33.00

Surface Area (ha)= 73.74 49.16
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 905.17 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 82.76 57.95
over (min)= 15.00 30.00
Storage Coeff. (min)= 12.72 (ii) 26.03 (iii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04
TOTALS
PEAK FLOW (cms)= 7.26 3.53 9.467 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 60.70 22.76 35.28
TOTAL RAINFALL (mm)= 62.70 62.70 62.70
RUNOFF COEFFICIENT = 0.97 0.36 0.56

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 61.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8140)

1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)

ID1= 1 (2042): 122.90 9.467 6.00 35.28
 + ID2= 2 (5204): 201.50 3.314 7.00 26.75
 =====
 ID = 3 (8140): 324.40 9.766 6.00 29.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6206) |
 IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (2061.0) -----

Distance	Elevation	Manning
0.00	253.07	0.0350
13.98	252.89	0.0350
27.95	251.99	0.0350
34.94	252.18	0.0350
41.93	252.02	0.0350
73.38	250.35	0.0350
94.34	247.09	0.0350
111.81	244.79	0.0350
115.31	244.24	0.0350
117.55	244.20	0.0350 /0.0300 Main Channel
118.80	243.60	0.0300 Main Channel
122.30	243.60	0.0300 Main Channel
122.55	244.60	0.0300 /0.0350 Main Channel
146.76	244.74	0.0350
164.23	247.26	0.0350
213.15	248.92	0.0350
241.10	249.51	0.0350
272.55	250.87	0.0350
300.50	252.26	0.0350
345.93	253.67	0.0350

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.30	249.90	.310E+04	1.4	1.23	35.25
0.60	244.20	.676E+04	4.6	1.81	24.64
1.12	244.72	.221E+05	17.1	2.07	21.60
1.64	245.24	.754E+05	64.1	2.28	19.60
2.17	245.77	.139E+06	149.8	2.88	15.50
2.69	246.29	.214E+06	273.3	3.43	13.04
3.21	246.81	.299E+06	436.1	3.91	11.43
3.73	247.33	.395E+06	632.6	4.30	10.40
4.25	247.85	.511E+06	836.4	4.39	10.18
4.77	248.37	.653E+06	1114.9	4.58	9.76
5.30	248.90	.822E+06	1472.1	4.81	9.30
5.82	249.42	.102E+07	1872.2	4.91	9.10
6.34	249.94	.126E+07	2444.6	5.22	8.56
6.86	250.46	.151E+07	3113.1	5.52	8.09
7.38	250.98	.180E+07	3843.5	5.74	7.79
7.90	251.50	.211E+07	4717.8	6.00	7.45
8.43	252.03	.245E+07	5701.4	6.24	7.16
8.95	252.55	.284E+07	6727.5	6.36	7.03
9.47	253.07	.326E+07	7885.3	6.48	6.90

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8140)	324.40	9.77	6.00	29.98	0.82
OUTFLOW : ID= 1 (6206)	324.40	6.90	6.50	29.98	0.70

ADD HYD (8142) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)

ID1= 1 (6206): 324.40 6.902 6.50 29.98
 + ID2= 2 (8144): 275.50 22.076 6.00 44.37
 =====
 ID = 3 (8142): 599.90 25.768 6.00 36.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8138) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)

ID1= 1 (8114): 14123.25 70.836 9.00 18.03
 + ID2= 2 (8142): 599.90 25.768 6.00 36.50
 =====
 ID = 3 (8138): 14723.15 74.961 8.75 18.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6208) |
 IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (2081.0) -----

Distance	Elevation	Manning
0.00	249.19	0.1000
73.11	244.13	0.1000
121.86	243.39	0.1000
243.71	240.01	0.1000
353.38	238.75	0.1000
450.87	235.40	0.1000
499.61	231.40	0.1000
511.80	230.96	0.1000
523.98	230.12	0.1000
534.50	227.80	0.1000 /0.0500 Main Channel
536.50	226.70	0.0500 Main Channel
546.50	226.70	0.0500 Main Channel
548.50	227.80	0.0500 /0.1000 Main Channel
572.72	230.25	0.1000
682.40	230.50	0.1000
804.25	233.95	0.1000
926.11	235.83	0.1000
1047.96	240.70	0.1000
1145.45	243.26	0.1000
1206.38	247.03	0.1000

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.55	227.25	.111E+05	2.0	0.33	92.44
1.10	227.80	.242E+05	6.5	0.50	61.56
2.23	228.93	.700E+05	26.0	0.68	44.91
3.36	230.06	.150E+06	60.8	0.74	41.04
4.49	231.19	.446E+06	130.8	0.54	56.83
5.62	232.32	.925E+06	287.4	0.57	53.66
6.76	233.46	.152E+07	522.9	0.63	48.32
7.89	234.59	.233E+07	827.2	0.68	44.91
9.02	235.72	.332E+07	1219.5	0.72	42.61
10.15	236.85	.418E+07	1780.6	0.78	39.11
11.28	237.98	.537E+07	2479.2	0.85	36.07
12.41	239.11	.699E+07	3282.9	0.90	33.95
13.54	240.24	.824E+07	4228.6	0.94	32.47
14.67	241.37	.997E+07	5384.6	0.99	30.87
15.81	242.51	.119E+08	6698.4	1.03	29.56
16.94	243.64	.140E+08	8235.2	1.08	28.25
18.07	244.77	.162E+08	10104.2	1.14	26.73
19.20	245.90	.185E+08	12251.5	1.21	25.21
20.33	247.03	.209E+08	14597.0	1.28	23.89

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8138)	*****	74.96	8.75	18.78	3.59
OUTFLOW : ID= 1 (6208)	*****	72.09	9.50	18.78	3.55

CALIB |
 STANDHYD (2082) | Area (ha)= 426.60
 ID= 1 DT=15.0 min | Total Imp(%)= 64.00 Dir. Conn.(%)= 37.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)=	273.02	153.58
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1686.42	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)=	82.76	79.07
HYD OVER (min)=	15.00	45.00
Storage Coeff. (min)=	18.48 (ii)	30.23 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.06	0.03
PEAK FLOW (cms)=	24.03	12.80
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	60.70	29.34
TOTAL RAINFALL (mm)=	62.70	62.70
RUNOFF COEFFICIENT	0.97	0.47

TOTALS
 29.657 (iii)

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 STANDHYD (2081) | Area (ha)= 265.70
 ID= 1 DT=15.0 min | Total Imp(%)= 35.00 Dir. Conn.(%)= 20.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)=	93.00	172.71
Dep. Storage (mm)=	2.00	5.00

Average Slope (%) = 0.50 0.50
 Length (m) = 1330.91 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr) = 82.76 44.12
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 16.03 (ii) 30.87 (iii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.07 0.03

490.46 235.75 0.0800
 568.65 236.24 0.0800
 661.06 241.85 0.0800
 703.71 246.04 0.0800

TOTALS
 PEAK FLOW (cms) = 8.64 7.89 11.972 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RINFALL VOLUME (mm) = 60.70 23.38 30.84
 TOTAL RAINFALL (mm) = 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.37 0.49

***** WARNING FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
 CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5208)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	16.8530	6.3371
0.8360	2.7353	21.1630	7.3975
4.7810	3.1070	25.1140	8.4406
8.3050	4.2484	25.5140	8.4506
12.6070	5.1796	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
265.700	11.972	6.00	30.84
265.700	6.322	7.00	30.84

PEAK FLOW REDUCTION [Qout/Qin](%) = 52.80
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 3.5586

ADD HYD (8148)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (2082):	426.60	29.657	6.00	40.94
+ ID2= 2 (5208):	265.70	6.322	7.00	30.84
ID = 3 (8148):	692.30	30.132	6.00	37.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8146)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6208):	14723.15	72.090	9.50	18.78
+ ID2= 2 (8148):	692.30	30.132	6.00	37.07
ID = 3 (8146):	15415.45	75.635	9.50	19.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6210)
 IN= 2--> OUT= 1

Routing time step (min) = 15.00

----- DATA FOR SECTION (2101.0) -----

Distance	Elevation	Manning
0.00	241.81	0.0800
28.43	241.78	0.0800
85.30	237.04	0.0800
149.27	231.32	0.0800
170.60	230.10	0.0800
255.89	229.37	0.0800
277.22	228.68	0.0800
284.33	228.42	0.0800
291.43	228.16	0.0800
297.76	227.00	0.0800 / 0.0350
298.76	225.00	0.0350
312.76	225.00	0.0350
313.76	227.00	0.0350 / 0.0800
326.97	229.50	0.0800
348.30	230.29	0.0800
376.73	229.76	0.0800

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.57	226.67	141E+05	6.6	0.69	35.86
1.33	226.33	289E+05	20.4	1.04	23.65
2.00	227.00	443E+05	39.3	1.31	18.81
2.93	227.93	730E+05	77.8	1.57	15.64
3.85	228.85	123E+06	131.1	1.57	15.65
4.78	229.78	228E+06	205.6	1.33	18.51
5.70	230.70	503E+06	356.7	1.05	23.49
6.63	231.63	846E+06	608.7	1.06	23.17
7.55	232.55	123E+07	946.4	1.14	21.65
8.48	233.48	165E+07	1367.0	1.22	20.13
9.41	234.41	211E+07	1871.8	1.31	18.80
10.33	235.33	261E+07	2463.2	1.40	17.65
11.26	236.26	317E+07	2999.4	1.40	17.63
12.18	237.18	384E+07	3848.3	1.48	16.63
13.11	238.11	454E+07	4806.0	1.56	15.75
14.03	239.03	528E+07	5876.9	1.64	14.97
14.96	239.96	605E+07	7062.7	1.72	14.29
15.88	240.88	687E+07	8365.0	1.80	13.68
16.81	241.81	771E+07	9506.9	1.82	13.52

----- hydrograph ----- <- pipe / channel ->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8146) ***** 75.64 9.50 19.61 2.87 1.56
 OUTFLOW : ID= 1 (6210) ***** 75.54 9.50 19.61 2.87 1.56

CALIB (0210)

NASHYD (0210) Area (ha) = 218.27 Curve Number (CN) = 80.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp (hrs) = 1.08

Unit Hyd Qpeak (cms) = 3.454

PEAK FLOW (cms) = 2.659 (i)
 TIME TO PEAK (hrs) = 7.500
 RINFALL VOLUME (mm) = 27.219
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.434

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8150)

1 + 2 = 3 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0210): 218.27 2.659 7.50 27.22
 + ID2= 2 (6210): 15415.45 75.537 9.50 19.61
 ID = 3 (8150): 15633.72 77.624 9.50 19.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0218)

NASHYD (0218) Area (ha) = 152.25 Curve Number (CN) = 69.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp (hrs) = 0.79

Unit Hyd Qpeak (cms) = 3.278

PEAK FLOW (cms) = 1.632 (i)
 TIME TO PEAK (hrs) = 7.000
 RINFALL VOLUME (mm) = 19.098
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.305

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0216)

NASHYD (0216) Area (ha) = 145.77 Curve Number (CN) = 75.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp (hrs) = 0.52

Unit Hyd Qpeak (cms) = 4.768

PEAK FLOW (cms) = 2.652 (i)
 TIME TO PEAK (hrs) = 6.500
 RINFALL VOLUME (mm) = 22.762
 TOTAL RAINFALL (mm) = 62.700

RUNOFF COEFFICIENT = 0.363

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANDBYD (2121) | Area (ha)= 56.00
| ID= 1 DT=15.0 min | Total Imp(%)= 93.00 Dir. Conn.(%)= 75.00
-----
| IMPERVIOUS | PERVIOUS (i) |
| Surface Area (ha)= 52.08 | 3.92 |
| Dep. Storage (mm)= 2.00 | 5.00 |
| Average Slope (%)= 0.50 | 0.50 |
| Length (m)= 611.01 | 40.00 |
| Mannings n = 0.013 | 0.250 |
| Max.Eff.Inten.(mm/hr)= 82.76 | 220.17 |
| over (min) 15.00 | 30.00 |
| Storage Coeff. (min)= 10.05 (ii) | 17.85 (ii) |
| Unit Hyd. Tpeak (min)= 15.00 | 30.00 |
| Unit Hyd. peak (cms)= 0.09 | 0.05 |
| PEAK FLOW (cms)= 8.14 | 1.29 | 9.020 (iii)
| TIME TO PEAK (hrs)= 6.00 | 6.25 | 6.00
| RUNOFF VOLUME (mm)= 60.70 | 40.30 | 55.60
| TOTAL RAINFALL (mm)= 62.70 | 62.70 | 62.70
| RUNOFF COEFFICIENT = 0.97 | 0.64 | 0.89

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| RESERVOIR (5212) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
| OUTFLOW | STORAGE | OUTFLOW | STORAGE | |
| (cms) | (ha.m.) | (cms) | (ha.m.) |
| 0.0000 | 0.0000 | 3.7460 | 1.9720 |
| 0.1900 | 1.1850 | 4.7040 | 2.0838 |
| 1.0850 | 1.4413 | 5.4760 | 2.1940 |
| 2.0210 | 1.5949 | 5.8760 | 2.2040 |
| 2.8030 | 1.7848 | 0.0000 | 0.0000 |
| AREA | QPEAK | TPEAK | R.V. |
| (ha) | (cms) | (hrs) | (mm) |
| INFLOW : ID= 2 (2121) | 56.000 | 9.020 | 6.00 | 55.60 |
| OUTFLOW: ID= 1 (5212) | 56.000 | 2.682 | 6.50 | 55.57 |
| PEAK FLOW REDUCTION [Qout/Qin](%)= 29.73 |
| TIME SHIFT OF PEAK FLOW (min)= 30.00 |
| MAXIMUM STORAGE USED (ha.m.)= 1.7669 |

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-----
| CALIB |
| STANDBYD (2122) | Area (ha)= 272.40
| ID= 1 DT=15.0 min | Total Imp(%)= 29.00 Dir. Conn.(%)= 23.00
-----
| IMPERVIOUS | PERVIOUS (i) |
| Surface Area (ha)= 79.00 | 193.40 |
| Dep. Storage (mm)= 2.00 | 5.00 |
| Average Slope (%)= 0.50 | 0.50 |
| Length (m)= 1347.59 | 40.00 |
| Mannings n = 0.013 | 0.250 |
| Max.Eff.Inten.(mm/hr)= 82.76 | 20.88 |
| over (min) 15.00 | 45.00 |
| Storage Coeff. (min)= 16.15 (ii) | 36.17 (ii) |
| Unit Hyd. Tpeak (min)= 15.00 | 45.00 |
| Unit Hyd. peak (cms)= 0.07 | 0.03 |
| PEAK FLOW (cms)= 10.16 | 6.30 | 12.757 (iii)
| TIME TO PEAK (hrs)= 6.00 | 6.50 | 6.00
| RUNOFF VOLUME (mm)= 60.70 | 20.66 | 29.87
| TOTAL RAINFALL (mm)= 62.70 | 62.70 | 62.70
| RUNOFF COEFFICIENT = 0.97 | 0.33 | 0.48

```

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| ADD HYD (8152) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.

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| (ha) (cms) (hrs) (mm) |
| ID1= 1 (2122): 272.40 12.757 6.00 29.87 |
| + ID2= 2 (5212): 56.00 2.682 6.50 55.57 |
| ===== |
| ID = 3 (8152): 328.40 13.064 6.00 34.25 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| ROUTE CHN (6214) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00
-----
|<----- DATA FOR SECTION (2141.0) ----->|
| Distance | Elevation | Manning | |
| 0.00 | 259.29 | 0.0500 |
| 15.33 | 258.91 | 0.0500 |
| 38.33 | 258.57 | 0.0500 |
| 65.16 | 257.93 | 0.0500 |
| 95.82 | 253.44 | 0.0500 |
| 111.15 | 251.21 | 0.0500 |
| 130.32 | 249.67 | 0.0500 |
| 149.48 | 248.51 | 0.0500 |
| 150.66 | 248.50 | 0.0500 / 0.0370 | Main Channel |
| 150.81 | 247.50 | 0.0370 | Main Channel |
| 154.66 | 247.50 | 0.0370 | Main Channel |
| 155.66 | 248.50 | 0.0370 / 0.0500 | Main Channel |
| 160.98 | 248.50 | 0.0500 |
| 187.81 | 249.78 | 0.0500 |
| 199.31 | 252.13 | 0.0500 |
| 245.30 | 254.04 | 0.0500 |
| 264.46 | 253.99 | 0.0500 |
| 298.96 | 255.23 | 0.0500 |
| 329.62 | 257.02 | 0.0500 |
| 379.45 | 258.82 | 0.0500 |

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|<----- TRAVEL TIME TABLE ----->|
| DEPTH | ELEV | VOLUME | FLOW RATE | VELOCITY | TRAV. TIME |
| (m) | (m) | (cu.m.) | (cms) | (m/s) | (min) |
| 0.50 | 248.00 | .648E+04 | 2.1 | 1.00 | 52.30 |
| 1.00 | 248.50 | .139E+05 | 6.4 | 1.44 | 36.35 |
| 1.61 | 249.11 | .570E+05 | 22.6 | 1.24 | 42.01 |
| 2.21 | 249.71 | .143E+06 | 63.6 | 1.39 | 37.61 |
| 2.82 | 250.32 | .263E+06 | 142.9 | 1.70 | 30.69 |
| 3.43 | 250.93 | .403E+06 | 257.2 | 2.00 | 26.11 |
| 4.04 | 251.54 | .562E+06 | 410.4 | 2.29 | 22.82 |
| 4.64 | 252.14 | .735E+06 | 601.3 | 2.56 | 20.37 |
| 5.25 | 252.75 | .933E+06 | 791.0 | 2.66 | 19.67 |
| 5.86 | 253.36 | .117E+07 | 1038.0 | 2.79 | 18.74 |
| 6.46 | 253.96 | .144E+07 | 1344.4 | 2.93 | 17.82 |
| 7.07 | 254.57 | .178E+07 | 1664.1 | 2.93 | 17.83 |
| 7.68 | 255.18 | .217E+07 | 2140.2 | 3.10 | 16.87 |
| 8.28 | 255.78 | .259E+07 | 2722.9 | 3.30 | 15.83 |
| 8.89 | 256.39 | .304E+07 | 3387.1 | 3.50 | 14.94 |
| 9.50 | 257.00 | .351E+07 | 4132.7 | 3.69 | 14.16 |
| 10.11 | 257.61 | .402E+07 | 4913.0 | 3.83 | 13.64 |
| 10.71 | 258.21 | .458E+07 | 5648.3 | 3.87 | 13.50 |
| 11.32 | 258.82 | .521E+07 | 6433.1 | 3.82 | 13.68 |

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|<---- hydrograph ----> |<-- pipe / channel-->| | | | | |
| AREA | QPEAK | TPEAK | R.V. | MAX DEPTH | MAX VEL |
| (ha) | (cms) | (hrs) | (mm) | (m) | (m/s) |
| INFLOW : ID= 2 (8152) | 328.40 | 13.06 | 6.00 | 34.25 | 1.25 | 1.35 |
| OUTFLOW: ID= 1 (6214) | 328.40 | 9.63 | 6.75 | 34.25 | 1.12 | 1.39 |

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-----
| CALIB |
| STANDBYD (0214) | Area (ha)= 316.95
| ID= 1 DT=15.0 min | Total Imp(%)= 25.00 Dir. Conn.(%)= 9.00
-----
| IMPERVIOUS | PERVIOUS (i) |
| Surface Area (ha)= 79.24 | 237.71 |
| Dep. Storage (mm)= 2.00 | 5.00 |
| Average Slope (%)= 0.50 | 0.50 |
| Length (m)= 1453.62 | 40.00 |
| Mannings n = 0.013 | 0.250 |
| Max.Eff.Inten.(mm/hr)= 82.76 | 20.52 |
| over (min) 15.00 | 45.00 |
| Storage Coeff. (min)= 16.90 (ii) | 37.06 (ii) |
| Unit Hyd. Tpeak (min)= 15.00 | 45.00 |
| Unit Hyd. peak (cms)= 0.07 | 0.03 |
| PEAK FLOW (cms)= 4.53 | 7.54 | 9.047 (iii)
| TIME TO PEAK (hrs)= 6.00 | 6.50 | 6.50
| RUNOFF VOLUME (mm)= 60.70 | 18.36 | 22.17
| TOTAL RAINFALL (mm)= 62.70 | 62.70 | 62.70
| RUNOFF COEFFICIENT = 0.97 | 0.29 | 0.35

```

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

CM* = 62.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8154)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0214):	316.95	9.047	6.50	22.17
+ ID2= 2 (6214):	328.40	9.630	6.75	34.25
ID = 3 (8154):	645.35	18.130	6.50	28.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8156)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0216):	145.77	2.652	6.50	22.76
+ ID2= 2 (8154):	645.35	18.130	6.50	28.32
ID = 3 (8156):	791.12	20.782	6.50	27.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6218) |
 | IN= 2--> OUT= 1 |
 Routing time step (min) = 15.00

<----- DATA FOR SECTION (2181.0) ----->				
Distance	Elevation	Manning		
0.00	256.38	0.0450		
7.70	254.85	0.0450		
15.40	253.20	0.0450		
19.25	252.29	0.0450		
23.10	251.17	0.0450		
78.94	236.16	0.0450		
80.87	235.80	0.0450		
82.79	235.45	0.0450		
84.57	234.60	0.0450 / 0.0300	Main Channel	
85.07	233.60	0.0300	Main Channel	
88.57	233.60	0.0300	Main Channel	
93.57	233.60	0.0300	Main Channel	
94.07	234.60	0.0300 / 0.0450	Main Channel	
96.27	235.13	0.0450		
98.19	235.20	0.0450		
100.12	235.32	0.0450		
138.63	239.95	0.0450		
180.98	244.80	0.0450		
188.69	245.00	0.0450		
190.61	256.00	0.0450		

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)
1.00	234.60	.181E+05	6.0	0.66	50.64
2.13	235.73	.518E+05	23.8	0.93	36.27
3.25	236.85	.119E+06	60.0	1.02	33.03
4.38	237.98	.217E+06	120.6	1.12	29.99
5.51	239.11	.346E+06	210.5	1.23	27.39
6.63	240.23	.506E+06	334.1	1.33	25.22
7.76	241.36	.697E+06	495.6	1.43	23.44
8.88	242.48	.920E+06	699.8	1.53	21.91
10.01	243.61	.117E+07	950.8	1.63	20.59
11.14	244.74	.146E+07	1252.5	1.73	19.45
12.26	245.86	.178E+07	1619.6	1.83	18.37
13.39	246.99	.212E+07	2078.5	1.97	17.01
14.52	248.12	.247E+07	2589.0	2.11	15.88
15.64	249.24	.282E+07	3149.7	2.25	14.94
16.77	250.37	.319E+07	3759.8	2.37	14.14
17.89	251.49	.356E+07	4419.5	2.50	13.44
19.02	252.62	.395E+07	5126.4	2.61	12.84
20.15	253.75	.435E+07	5871.6	2.72	12.34
21.27	254.87	.476E+07	6663.2	2.82	11.89

<---- hydrograph ---->					<-pipe / channel->	
INFLW	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
: ID= 2 (8156)	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
OUTFLOW: ID= 1 (6218)	791.12	20.78	6.50	27.30	1.94	0.87
	791.12	15.15	7.00	27.30	1.58	0.78

ADD HYD (8158)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)

ID1= 1 (0218): 152.25 1.632 7.00 19.10
 + ID2= 2 (6218): 791.12 15.148 7.00 27.30
 ID = 3 (8158): 943.37 16.780 7.00 25.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8151)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8150):	15633.72	77.624	9.50	19.71
+ ID2= 2 (8158):	943.37	16.780	7.00	25.97
ID = 3 (8151):	16577.09	90.396	7.25	20.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5504)					
IN= 2--> OUT= 1	DT= 15.0 min	OUTFLOW	STORAGE	OUTFLOW	STORAGE
		(cms)	(ha.m.)	(cms)	(ha.m.)
		0.0000	0.0000	33.0100	0.6700
		2.6400	0.0500	37.4800	0.7800
		4.8300	0.0900	47.5400	1.0100
		7.4100	0.1300	60.2300	1.2800
		10.3300	0.1900	71.9530	1.5258
		13.5300	0.2500	*****	3.0529
		16.9900	0.3200	*****	5.3299
		20.6800	0.3900	*****	8.4148
		24.6000	0.4800	*****	11.7637
		28.7100	0.5700	*****	11.7937

INFLW	AREA	QPEAK	TPEAK	R.V.
: ID= 2 (8151)	(ha)	(cms)	(hrs)	(mm)
OUTFLOW: ID= 1 (5504)	90.396	7.25	20.07	
	90.174	7.00	20.07	

PEAK FLOW REDUCTION [Qout/Qin](%) = 99.76
 TIME SHIFT OF PEAK FLOW (min) = -15.00
 MAXIMUM STORAGE USED (ha.m.) = 1.9004

ROUTE CHN (6220) |
 | IN= 2--> OUT= 1 |
 Routing time step (min) = 15.00

<----- DATA FOR SECTION (2201.0) ----->				
Distance	Elevation	Manning		
0.00	245.59	0.0600		
5.76	245.45	0.0600		
23.03	242.27	0.0600		
57.58	232.35	0.0600		
86.37	226.18	0.0600		
109.41	224.31	0.0600		
115.17	223.86	0.0600		
120.92	222.78	0.0600		
126.68	221.93	0.0600		
128.06	221.50	0.0600 / 0.0300	Main Channel	
129.56	220.00	0.0300	Main Channel	
141.56	220.00	0.0300	Main Channel	
143.06	221.50	0.0300 / 0.0600	Main Channel	
149.72	223.77	0.0600		
155.47	225.09	0.0600		
253.36	229.21	0.0600		
333.98	227.50	0.0600		
454.90	229.25	0.0600		
500.97	233.11	0.0600		
570.07	237.42	0.0600		

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)
0.75	220.75	.262E+05	5.5	0.57	79.44
1.50	221.50	.555E+05	17.5	0.86	52.89
2.44	222.44	.103E+06	42.7	1.14	40.03
3.37	223.37	.171E+06	79.4	1.27	35.98
4.31	224.31	.263E+06	128.1	1.33	34.22
5.25	225.25	.393E+06	191.7	1.34	34.19
6.18	226.18	.594E+06	276.7	1.28	35.77
7.12	227.12	.872E+06	399.9	1.26	36.33
8.06	228.06	.127E+07	542.7	1.17	38.92
8.99	228.99	.199E+07	770.0	1.06	43.01
9.93	229.93	.297E+07	1151.8	1.06	42.95
10.86	230.86	.400E+07	1652.9	1.13	40.33
11.80	231.80	.507E+07	2251.8	1.22	37.53
12.74	232.74	.618E+07	2946.5	1.31	34.97
13.67	233.67	.733E+07	3726.8	1.39	32.78
14.61	234.61	.853E+07	4591.0	1.48	30.96
15.55	235.55	.977E+07	5544.1	1.55	29.37
16.48	236.48	.111E+08	6586.7	1.63	27.99

17.42 237.42 .124E+08 7719.2 1.71 26.77

<----- hydrograph -----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
90.17	7.00	20.07	3.58	1.28	
85.29	9.00	20.07	3.47	1.28	

INFLW : ID= 2 (5504) *****

OUTFLOW: ID= 1 (6220) *****

CALIB |
| STANDHYD (0220) | Area (ha)= 169.10 Curve Number (CN)= 73.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.50

Unit Hyd Qpeak (cms)= 1.923

PEAK FLOW (cms)= 1.264 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 21.832
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.348

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8160) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)

ID1= 1 (0220): 169.10 1.264 8.00 21.83
+ ID2= 2 (6220): 16577.09 85.286 9.00 20.07
=====

ID = 3 (8160): 16746.19 86.510 9.00 20.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB |
| STANDHYD (0226) | Area (ha)= 237.58 Curve Number (CN)= 75.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.05

Unit Hyd Qpeak (cms)= 3.852

PEAK FLOW (cms)= 2.484 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 23.164
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.369

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
| STANDHYD (2221) | Area (ha)= 145.20 Dir. Conn.(%)= 19.00
| ID= 1 DT=15.0 min | Total Imp(%)= 36.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	52.27	92.93
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	983.87	40.00
Mannings n	0.013	0.250
Max.Eff.Inten.(mm/hr)	82.76	53.81
over (min)	15.00	30.00
Storage Coeff. (min)	13.37 (ii)	27.08 (iii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04
PEAK FLOW (cms)	4.84	6.07
TIME TO PEAK (hrs)	6.00	8.675 (iii)
RUNOFF VOLUME (mm)	60.70	27.47
TOTAL RAINFALL (mm)	62.70	62.70
RUNOFF COEFFICIENT	0.97	0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5222) |
| IR= 2--> OUT= 1 |

DT= 15.0 min |

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	6.6430	3.7677
0.4620	2.0014	8.2830	4.2576
2.0160	2.2850	14.1970	4.6909
3.6250	2.7987	14.5970	4.7009
5.0230	3.2496	0.0000	0.0000

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)

INFLW : ID= 2 (2221) 145.200 8.675 6.00 33.78
OUTFLOW: ID= 1 (5222) 145.200 2.918 7.00 33.78

PEAK FLOW REDUCTION [Qout/Qin](%)= 33.63
TIME SHIFT OF PEAK FLOW (min)= 60.00
MAXIMUM STORAGE USED (ha.m.)= 2.5756

CALIB |
| STANDHYD (2222) | Area (ha)= 74.30
| ID= 1 DT=15.0 min | Total Imp(%)= 78.00 Dir. Conn.(%)= 41.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	57.95	16.35
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	703.80	40.00
Mannings n	0.013	0.250

Max.Eff.Inten.(mm/hr)= 82.76 165.14
over (min) 15.00 30.00
Storage Coeff. (min)= 10.94 (ii) 19.69 (iii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.05

*****TOTALS*
PEAK FLOW (cms)= 5.75 3.88 8.373 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 60.70 40.05 48.52
TOTAL RAINFALL (mm)= 62.70 62.70 62.70
RUNOFF COEFFICIENT = 0.97 0.64 0.77

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8164) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)

ID1= 1 (2222): 74.30 8.373 6.00 48.52
+ ID2= 2 (5222): 145.20 2.918 7.00 33.78
=====

ID = 3 (8164): 219.50 8.614 6.00 38.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB |
| STANDHYD (0224) | Area (ha)= 140.45
| ID= 1 DT=15.0 min | Total Imp(%)= 34.00 Dir. Conn.(%)= 26.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	47.75	92.70
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	967.64	40.00
Mannings n	0.013	0.250

Max.Eff.Inten.(mm/hr)= 82.76 50.03
over (min) 15.00 30.00
Storage Coeff. (min)= 13.24 (ii) 27.35 (iii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04

*****TOTALS*
PEAK FLOW (cms)= 6.43 5.60 9.970 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 60.70 28.63 36.97
TOTAL RAINFALL (mm)= 62.70 62.70 62.70
RUNOFF COEFFICIENT = 0.97 0.46 0.59

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8166) |
| 1 + 2 = 3 |
-----
ID1= 1 (0224): 140.45 9.970 6.00 36.97
+ ID2= 2 (8164): 219.50 8.614 6.00 38.77
=====
ID = 3 (8166): 359.95 18.584 6.00 38.07

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6226) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)'= 15.00

```

```

-----
|<----- DATA FOR SECTION (2261.0) ----->
| Distance Elevation Manning |
| 0.00 245.35 0.0600 |
| 8.27 242.72 0.0600 |
| 16.54 240.01 0.0600 |
| 31.02 235.31 0.0600 |
| 41.36 233.14 0.0600 |
| 90.99 227.73 0.0600 |
| 93.06 227.50 0.0600 |
| 95.13 227.27 0.0600 |
| 95.83 227.00 0.0600 |
| 100.83 226.50 0.0600 /0.0350 Main Channel |
| 101.33 225.50 0.0350 Main Channel |
| 101.83 226.50 0.0350 /0.0600 Main Channel |
| 105.83 227.00 0.0600 |
| 107.54 227.39 0.0600 |
| 109.60 227.62 0.0600 |
| 111.67 227.86 0.0600 |
| 146.83 231.76 0.0600 |
| 173.71 234.77 0.0600 |
| 186.12 238.23 0.0600 |
| 202.67 238.35 0.0600 |

```

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-----
|<----- TRAVEL TIME TABLE ----->
| DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME |
| (m) (m) (cu.m.) (cms) (m/s) (min) |
| 0.50 226.00 403E+03 0.0 0.16 330.59 |
| 1.00 226.50 161E+04 0.1 0.26 208.26 |
| 1.70 227.20 173E+05 1.5 0.27 195.72 |
| 2.39 227.89 541E+05 5.9 0.35 152.27 |
| 3.09 228.59 119E+06 16.0 0.43 124.08 |
| 3.79 229.29 212E+06 33.8 0.51 104.65 |
| 4.49 229.99 334E+06 61.2 0.59 90.97 |
| 5.18 230.68 484E+06 99.7 0.66 80.88 |
| 5.88 231.38 663E+06 151.0 0.73 73.13 |
| 6.58 232.08 870E+06 216.5 0.80 66.95 |
| 7.27 232.77 111E+07 297.5 0.87 61.91 |
| 7.97 233.47 137E+07 398.1 0.94 57.29 |
| 8.67 234.17 165E+07 518.9 1.01 53.12 |
| 9.36 234.86 196E+07 658.9 1.08 49.59 |
| 10.06 235.56 228E+07 827.0 1.17 46.02 |
| 10.76 236.26 262E+07 1015.4 1.25 42.96 |
| 11.46 236.96 296E+07 1221.2 1.33 40.41 |
| 12.15 237.65 332E+07 1444.6 1.40 38.26 |
| 12.85 238.35 368E+07 1598.2 1.40 38.42 |

```

```

-----
|<----- hydrograph -----> <-pipe / channel->
| AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL |
| (ha) (cms) (hrs) (mm) (m) (m/s) |
INFLOW : ID= 2 (8166) 359.95 18.58 6.00 38.07 3.19 0.44
OUTFLOW : ID= 1 (6226) 359.95 5.38 7.00 38.06 2.31 0.34

```

```

-----
| ADD HYD (8168) |
| 1 + 2 = 3 |
-----
ID1= 1 (0226): 237.58 2.484 7.50 23.16
+ ID2= 2 (6226): 359.95 5.384 7.00 38.06
=====
ID = 3 (8168): 597.53 7.832 7.00 32.14

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8162) |
| 1 + 2 = 3 |
-----
ID1= 1 (8160): 16746.19 86.510 9.00 20.09
+ ID2= 2 (8168): 597.53 7.832 7.00 32.14
=====
ID = 3 (8162): 17343.72 92.582 8.75 20.50

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6228) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)'= 15.00

```

```

-----
|<----- DATA FOR SECTION (2281.0) ----->
| Distance Elevation Manning |
| 0.00 245.00 0.0600 |
| 18.60 244.74 0.0600 |
| 37.20 242.47 0.0600 |
| 93.01 233.71 0.0600 |
| 125.57 226.95 0.0600 |
| 213.93 221.63 0.0600 |
| 218.58 221.15 0.0600 |
| 223.23 220.45 0.0600 |
| 225.03 220.55 0.0600 /0.0300 Main Channel |
| 225.53 218.05 0.0300 Main Channel |
| 232.53 218.05 0.0300 Main Channel |
| 240.03 218.05 0.0300 Main Channel |
| 241.03 220.55 0.0300 /0.0600 Main Channel |
| 246.48 221.00 0.0600 |
| 251.13 221.27 0.0600 |
| 255.78 221.22 0.0600 |
| 325.54 225.25 0.0600 |
| 367.40 228.91 0.0600 |
| 404.60 231.75 0.0600 |
| 460.41 245.00 0.0600 |

```

```

-----
|<----- TRAVEL TIME TABLE ----->
| DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME |
| (m) (m) (cu.m.) (cms) (m/s) (min) |
| 1.25 219.30 338E+05 16.0 0.86 35.15 |
| 2.50 220.55 696E+05 48.3 1.26 24.02 |
| 3.94 221.99 168E+06 116.0 1.26 24.14 |
| 5.38 223.43 392E+06 247.5 1.15 26.37 |
| 6.81 224.86 743E+06 472.6 1.16 26.20 |
| 8.25 226.30 122E+07 821.6 1.23 24.67 |
| 9.69 227.74 179E+07 1320.1 1.34 22.59 |
| 11.13 229.18 243E+07 1965.7 1.47 20.58 |
| 12.57 230.62 313E+07 2744.2 1.59 19.02 |
| 14.01 232.06 390E+07 3683.4 1.72 17.66 |
| 15.44 233.49 472E+07 4824.1 1.86 16.30 |
| 16.88 234.93 557E+07 6092.4 1.99 15.23 |
| 18.32 236.37 646E+07 7503.3 2.11 14.35 |
| 19.76 237.81 739E+07 9061.2 2.23 13.59 |
| 21.20 239.25 836E+07 10767.6 2.34 12.94 |
| 22.64 240.69 937E+07 12624.8 2.45 12.37 |
| 24.07 242.12 104E+08 14634.9 2.56 11.87 |
| 25.51 243.56 115E+08 16762.8 2.65 11.45 |
| 26.95 245.00 127E+08 18699.9 2.69 11.28 |

```

```

-----
|<----- hydrograph -----> <-pipe / channel->
| AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL |
| (ha) (cms) (hrs) (mm) (m) (m/s) |
INFLOW : ID= 2 (8162) ***** 92.58 8.75 20.50 3.44 1.26
OUTFLOW : ID= 1 (6228) ***** 90.85 9.50 20.50 3.41 1.26

```

```

-----
| CALIB (6228) |
| NASHVD (6228) | Area (ha)= 310.54 Curve Number (CN)= 61.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.12 |

```

Unit Hyd Qpeak (cms)= 4.751

PEAK FLOW (cms)= 1.970 (i)

TIME TO PEAK (hrs)= 7.750

RUNOFF VOLUME (mm)= 14.996

TOTAL RAINFALL (mm)= 62.700

RUNOFF COEFFICIENT = 0.239

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8170) |
| 1 + 2 = 3 |
-----
ID1= 1 (0228): 310.54 1.970 7.75 15.00
+ ID2= 2 (6228): 17343.72 90.854 9.50 20.50
=====
ID = 3 (8170): 17654.26 92.491 9.50 20.40

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| CALIB (2302) |
| STANDHYD (2302) | Area (ha)= 126.70

```

|ID= 1 DT=15.0 min | Total Imp(%)= 23.00 Dir. Conn.(%)= 9.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 29.14 97.56
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 919.06 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 82.76 24.43
over (min) 15.00 45.00
Storage Coeff. (min)= 12.84 (ii) 31.64 (iii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.08 0.03
**TOTALS*
PEAK FLOW (cms)= 2.03 3.97 4.494 (iii)
TIME TO PEAK (hrs)= 6.00 6.50
RUNOFF VOLUME (mm)= 60.70 22.05 25.53
TOTAL RAINFALL (mm)= 62.70 62.70 62.70
RUNOFF COEFFICIENT = 0.97 0.35 0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANBYD (2301) Area (ha)= 78.80
|ID= 1 DT=15.0 min | Total Imp(%)= 50.00 Dir. Conn.(%)= 19.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 39.40 39.40
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 724.80 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 82.76 68.03
over (min) 15.00 30.00
Storage Coeff. (min)= 11.13 (ii) 23.61 (iii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04
**TOTALS*
PEAK FLOW (cms)= 2.81 3.50 5.038 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 60.70 27.33 33.67
TOTAL RAINFALL (mm)= 62.70 62.70 62.70
RUNOFF COEFFICIENT = 0.97 0.44 0.54

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5230)
IN= 2--> OUT= 1
DT= 15.0 min
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
0.0000 0.0000 | 5.6350 1.5914
0.3160 0.7624 | 6.9200 1.8001
1.7040 0.8931 | 7.9010 2.0070
3.5140 1.1218 | 8.3010 2.0170
4.5020 1.3405 | 0.0000 0.0000

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (2301) 78.800 5.038 6.00 33.67
OUTFLOW: ID= 1 (5230) 78.800 2.893 6.50 33.66

PEAK FLOW REDUCTION [Qout/Qin](%)= 57.42
TIME SHIFT OF PEAK FLOW (min)= 30.00
MAXIMUM STORAGE USED (ha.m.)= 1.0508

ADD HYD (8172)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1 = 1 (2302): 126.70 4.494 6.50 25.53
+ ID2 = 2 (5230): 78.80 2.893 6.50 33.66

ID = 3 (8172): 205.50 7.387 6.50 28.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHVD (0232) Area (ha)= 314.80 Curve Number (CN)= 58.0
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.00

Unit Hyd Qpeak (cms)= 5.391
PEAK FLOW (cms)= 1.972 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 13.637
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.217

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8173)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1 = 1 (0232): 314.80 1.972 7.25 13.64
+ ID2 = 2 (8172): 205.50 7.387 6.50 28.65
ID = 3 (8173): 520.30 9.124 6.50 19.57

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8174)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1 = 1 (8170): 17654.26 92.491 9.50 20.40
+ ID2 = 2 (8173): 520.30 9.124 6.50 19.57
ID = 3 (8174): 18174.56 95.170 9.00 20.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6234)
IN= 2--> OUT= 1
Routing time step (min)'= 15.00

Table with 4 columns: Distance, Elevation, Manning, and a fourth unlabeled column. Rows show data for various distances from 0.00 to 1060.79.

TRAVEL TIME TABLE
Table with 6 columns: DEPTH (m), ELEV (m), VOLUME (cu.m.), FLOW RATE (cms), VELOCITY (m/s), TRAV.TIME (min). Rows show data for depths from 0.50 to 9.04.

9.63 226.83 .194E+08 5717.7 1.12 56.68
10.21 227.41 .214E+08 6577.1 1.17 54.18
10.80 228.00 .235E+08 6919.5 1.12 56.61

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8174) ***** 95.17 9.00 20.38 2.21 0.65
OUTFLOW : ID= 1 (6234) ***** 85.53 11.75 20.38 2.12 0.69

CALIB STANDHYD (0234) Area (ha)= 267.16
ID= 1 DT=15.0 min Total Imp(%)= 22.00 Dir. Conn.(%)= 8.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 58.78 208.38
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1334.57 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 82.76 8.97
over (min)= 15.00 45.00
Storage Coeff. (min)= 16.06 (ii) 44.13 (iii)
Unit Hyd. Peak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.07 0.03
PEAK FLOW (cms)= 3.47 2.64 4.519 (iii)
TIME TO PEAK (hrs)= 6.00 6.00
RUNOFF VOLUME (mm)= 60.70 8.65
TOTAL RAINFALL (mm)= 62.70 62.70
RUNOFF COEFFICIENT = 0.97 0.14 0.20

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 39.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8176) |
1 + 2 = 3
ID1= 1 (0234): 267.16 4.519 6.00 12.81
+ ID2= 2 (6234): 18174.56 85.531 11.75 20.38
=====

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0238) Area (ha)= 311.70 Curve Number (CN)= 53.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.56

Unit Hyd Qpeak (cms)= 3.405
PEAK FLOW (cms)= 1.188 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 11.705
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.187

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0236) Area (ha)= 494.49 Curve Number (CN)= 54.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.24

Unit Hyd Qpeak (cms)= 6.830
PEAK FLOW (cms)= 2.329 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 12.057
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.192

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CN (6238) |

IN= 2----> OUT= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (2381.0) -----
Distance Elevation Manning
0.00 241.54 0.0380
602.55 227.00 0.0380
1702.00 224.50 0.0380
1721.25 223.00 0.0380
1725.10 222.60 0.0380
1780.94 222.50 0.0380
1782.87 222.45 0.0380
1784.79 222.40 0.0380 / 0.0300 Main Channel
1786.57 221.75 0.0300 Main Channel
1787.07 220.75 0.0300 Main Channel
1787.57 220.75 0.0300 Main Channel
1791.57 220.75 0.0300 Main Channel
1794.07 221.75 0.0300 Main Channel
1798.27 222.00 0.0300 Main Channel
1800.19 222.35 0.0300 / 0.0380 Main Channel
1802.12 222.47 0.0380
1840.63 223.00 0.0380
1900.00 225.00 0.0380
1950.00 226.00 0.0380
2242.61 240.00 0.0380

----- TRAVEL TIME TABLE -----
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.80 221.55 1.01E+05 5.2 1.14 32.36
1.60 222.35 2.96E+05 18.7 1.40 26.36
2.64 223.39 2.49E+06 151.8 1.35 27.33
3.68 224.43 613E+06 512.2 1.85 19.94
4.71 225.46 1.52E+07 1066.8 1.55 23.80
5.75 226.50 3.99E+07 2824.9 1.75 21.16
6.79 227.54 663E+07 6528.0 2.18 16.93
7.83 228.58 994E+07 12158.6 2.71 13.62
8.87 229.62 1.34E+08 19268.8 3.19 11.58
9.91 230.66 1.70E+08 27785.3 3.63 10.19
10.94 231.69 2.07E+08 37657.1 4.03 9.18
11.98 232.73 2.46E+08 48891.7 4.40 8.40
13.02 233.77 2.87E+08 61448.0 4.75 7.78
14.06 234.81 3.29E+08 75332.7 5.08 7.28
15.10 235.85 3.72E+08 90547.8 5.39 6.85
16.14 236.89 4.17E+08 ***** 5.69 6.49
17.17 237.92 4.64E+08 ***** 5.98 6.18
18.21 238.96 5.12E+08 ***** 6.25 5.91
19.25 240.00 5.61E+08 ***** 6.52 5.67

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (0236) 494.49 2.33 8.00 12.06 0.36 1.14
OUTFLOW : ID= 1 (6238) 494.49 2.26 8.50 12.06 0.34 1.14

ADD HYD (8180) |
1 + 2 = 3
ID1= 1 (0238): 311.70 1.188 8.25 11.70
+ ID2= 2 (6238): 494.49 2.257 8.50 12.06
=====

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8178) |
1 + 2 = 3
ID1= 1 (8176): 18441.72 85.866 11.50 20.27
+ ID2= 2 (8180): 806.19 3.443 8.50 11.92
=====

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6240) |
IN= 2----> OUT= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (2401.0) -----
Distance Elevation Manning
0.00 222.00 0.0450
11.46 221.00 0.0450
208.98 221.00 0.0450
404.04 220.97 0.0450
808.08 220.83 0.0450
905.60 220.17 0.0450
919.53 219.43 0.0450

933.47	219.22	0.0450	
945.26	219.21	0.0450 / 0.0300	Main Channel
946.26	217.81	0.0300	Main Channel
975.26	217.81	0.0300	Main Channel
1003.26	217.81	0.0300	Main Channel
1005.26	219.21	0.0300 / 0.0450	Main Channel
1017.06	219.28	0.0450	
1030.99	219.26	0.0450	
1044.92	219.23	0.0450	
1058.86	219.23	0.0450	
1253.91	219.22	0.0450	
1323.57	221.05	0.0450	
1379.30	222.00	0.0450	

<--- TRAVEL TIME TABLE --->					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.20	218.01	.307E+05	2.2	0.20	227.52
0.40	218.21	.617E+05	7.1	0.31	143.95
0.60	218.41	.928E+05	14.0	0.41	110.33
0.80	218.61	.124E+06	22.6	0.49	91.46
1.00	218.81	.156E+06	32.8	0.57	79.15
1.20	219.01	.188E+06	44.5	0.64	70.38
1.40	219.21	.220E+06	57.5	0.70	63.77
1.63	219.44	.413E+06	82.5	0.54	83.52
1.87	219.68	.632E+06	120.9	0.51	87.03
2.10	219.91	.858E+06	170.0	0.53	84.11
2.33	220.14	.109E+07	228.7	0.56	79.62
2.56	220.37	.134E+07	294.4	0.59	76.03
2.80	220.61	.162E+07	371.1	0.61	72.75
3.03	220.84	.192E+07	456.8	0.64	70.18
3.26	221.07	.253E+07	552.1	0.59	75.74
3.49	221.30	.333E+07	711.8	0.57	78.07
3.73	221.54	.417E+07	907.0	0.58	76.62
3.96	221.77	.502E+07	1134.1	0.61	73.70
4.19	222.00	.587E+07	1390.9	0.64	70.34

<--- hydrograph ---> <-pipe / channel->
 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm) MAX DEPTH (m) MAX VEL (m/s)
 INFLOW : ID= 2 (8178) ***** 88.47 11.50 19.92 1.67 0.53
 OUTFLOW : ID= 1 (6240) ***** 81.73 13.25 19.92 1.62 0.54

CALIB | NASHVD (0240) | Area (ha)= 434.37 | Curve Number (CN)= 46.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 U.N. Tp(hrs)= 3.60

Unit Hyd Opeak (cms) = 2.058

PEAK FLOW (cms) = 0.696 (i)
 TIME TO PEAK (hrs) = 12.000
 RUNOFF VOLUME (mm) = 9.341
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.149

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8182) | AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0240): 434.37 0.696 12.00 9.34
 + ID2= 2 (6240): 19247.91 81.726 13.25 19.92

 ID = 3 (8182): 19682.28 82.394 13.25 19.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB | NASHVD (0242) | Area (ha)= 657.88 | Curve Number (CN)= 52.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 U.N. Tp(hrs)= 5.37

Unit Hyd Opeak (cms) = 2.090

PEAK FLOW (cms) = 0.908 (i)
 TIME TO PEAK (hrs) = 13.000
 RUNOFF VOLUME (mm) = 11.386
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.182

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8184) |

1 + 2 = 3 | AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (0242): 657.88 0.908 13.00 11.39
 + ID2= 2 (8182): 19682.28 82.394 13.25 19.69

 ID = 3 (8184): 20340.16 83.301 13.25 19.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6244) |
 IB= 2 -> OUTF 1 | Routing time step (min)'= 15.00

<--- DATA FOR SECTION (2441.0) --->					
Distance	Elevation	Manning			
0.00	225.00	0.0450			
2.50	220.10	0.0450			
3.00	220.06	0.0450			
3.64	220.07	0.0450			
7.28	220.08	0.0450			
10.91	220.09	0.0450			
14.55	219.81	0.0450			
18.19	219.29	0.0450			
24.13	219.21	0.0450 / 0.0300	Main Channel		
24.63	217.81	0.0300	Main Channel		
69.13	217.81	0.0300	Main Channel		
117.81	217.81	0.0300	Main Channel		
115.13	219.21	0.0300 / 0.0450	Main Channel		
120.06	219.25	0.0450			
123.70	219.32	0.0450			
127.34	219.77	0.0450			
130.98	219.88	0.0450			
134.61	219.99	0.0450			
350.00	220.18	0.0450			
360.18	225.00	0.0450			

<--- TRAVEL TIME TABLE --->					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.35	218.16	.108E+06	9.0	0.29	200.52
0.70	218.51	.216E+06	28.4	0.45	126.89
1.05	218.86	.325E+06	55.7	0.59	97.27
1.40	219.21	.434E+06	89.7	0.71	80.65
1.79	219.60	.574E+06	135.9	0.81	70.36
2.17	219.98	.726E+06	190.8	0.90	63.38
2.56	220.37	.111E+07	264.3	0.82	69.82
2.94	220.75	.157E+07	371.7	0.81	70.36
3.33	221.14	.203E+07	503.4	0.85	67.30
3.72	221.53	.250E+07	656.6	0.90	63.40
4.10	221.91	.296E+07	829.6	0.96	59.54
4.49	222.30	.343E+07	1021.1	1.02	56.00
4.87	222.68	.390E+07	1230.2	1.08	52.83
5.26	223.07	.437E+07	1456.2	1.14	50.01
5.65	223.46	.484E+07	1698.4	1.20	47.51
6.03	223.84	.531E+07	1956.3	1.26	45.28
6.42	224.23	.579E+07	2229.3	1.32	43.28
6.80	224.61	.626E+07	2517.0	1.38	41.48
7.19	225.00	.674E+07	2819.2	1.44	39.85

<--- hydrograph ---> <-pipe / channel->
 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm) MAX DEPTH (m) MAX VEL (m/s)
 INFLOW : ID= 2 (8184) ***** 83.30 13.25 19.42 1.33 0.68
 OUTFLOW : ID= 1 (6244) ***** 79.84 14.00 19.42 1.30 0.67

CALIB | NASHVD (0244) | Area (ha)=1908.71 | Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 U.N. Tp(hrs)= 2.20

Unit Hyd Opeak (cms) = 14.812

PEAK FLOW (cms) = 7.239 (i)
 TIME TO PEAK (hrs) = 9.750
 RUNOFF VOLUME (mm) = 15.079
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.240

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8186) | AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0244): 1908.71 7.239 9.75 15.08
 + ID2= 2 (6244): 20340.16 79.842 14.00 19.42

 ID = 3 (8186): 22248.87 85.102 13.75 19.05

44.00	3.070	106.25	0.439	168.50	0.072	230.75	0.011	293.00	0.002
44.25	3.023	106.50	0.436	168.75	0.072	231.00	0.011	293.25	0.002
44.50	2.978	106.75	0.433	169.00	0.071	231.25	0.011	293.50	0.002
44.75	2.933	107.00	0.430	169.25	0.071	231.50	0.011	293.75	0.002
45.00	2.893	107.25	0.427	169.50	0.070	231.75	0.011	294.00	0.002
45.25	2.848	107.50	0.424	169.75	0.070	232.00	0.011	294.25	0.002
45.50	2.809	107.75	0.421	170.00	0.069	232.25	0.011	294.50	0.002
45.75	2.770	108.00	0.418	170.25	0.069	232.50	0.011	294.75	0.002
46.00	2.733	108.25	0.415	170.50	0.068	232.75	0.011	295.00	0.002
46.25	2.696	108.50	0.412	170.75	0.068	233.00	0.010	295.25	0.002
46.50	2.661	108.75	0.409	171.00	0.067	233.25	0.010	295.50	0.002
46.75	2.627	109.00	0.406	171.25	0.067	233.50	0.010	295.75	0.002
47.00	2.593	109.25	0.403	171.50	0.066	233.75	0.010	296.00	0.002
47.25	2.561	109.50	0.400	171.75	0.066	234.00	0.010	296.25	0.002
47.50	2.529	109.75	0.397	172.00	0.065	234.25	0.010	296.50	0.002
47.75	2.499	110.00	0.395	172.25	0.065	234.50	0.010	296.75	0.001
48.00	2.469	110.25	0.392	172.50	0.064	234.75	0.010	297.00	0.001
48.25	2.440	110.50	0.389	172.75	0.064	235.00	0.010	297.25	0.001
48.50	2.412	110.75	0.386	173.00	0.063	235.25	0.010	297.50	0.001
48.75	2.384	111.00	0.384	173.25	0.063	235.50	0.010	297.75	0.001
49.00	2.358	111.25	0.381	173.50	0.062	235.75	0.010	298.00	0.001
49.25	2.332	111.50	0.378	173.75	0.062	236.00	0.010	298.25	0.001
49.50	2.306	111.75	0.376	174.00	0.061	236.25	0.010	298.50	0.001
49.75	2.282	112.00	0.373	174.25	0.061	236.50	0.009	298.75	0.001
50.00	2.258	112.25	0.370	174.50	0.061	236.75	0.009	299.00	0.001
50.25	2.234	112.50	0.368	174.75	0.060	237.00	0.009	299.25	0.001
50.50	2.211	112.75	0.365	175.00	0.060	237.25	0.009	299.50	0.001
50.75	2.189	113.00	0.362	175.25	0.059	237.50	0.009	299.75	0.001
51.00	2.167	113.25	0.360	175.50	0.059	237.75	0.009	300.00	0.001
51.25	2.145	113.50	0.357	175.75	0.058	238.00	0.009	300.25	0.001
51.50	2.124	113.75	0.355	176.00	0.058	238.25	0.009	300.50	0.001
51.75	2.104	114.00	0.352	176.25	0.058	238.50	0.009	300.75	0.001
52.00	2.084	114.25	0.350	176.50	0.057	238.75	0.009	301.00	0.001
52.25	2.064	114.50	0.347	176.75	0.057	239.00	0.009	301.25	0.001
52.50	2.045	114.75	0.345	177.00	0.056	239.25	0.009	301.50	0.001
52.75	2.026	115.00	0.342	177.25	0.056	239.50	0.009	301.75	0.001
53.00	2.007	115.25	0.340	177.50	0.055	239.75	0.009	302.00	0.001
53.25	1.989	115.50	0.338	177.75	0.055	240.00	0.008	302.25	0.001
53.50	1.973	115.75	0.335	178.00	0.055	240.25	0.008	302.50	0.001
53.75	1.954	116.00	0.333	178.25	0.054	240.50	0.008	302.75	0.001
54.00	1.937	116.25	0.330	178.50	0.054	240.75	0.008	303.00	0.001
54.25	1.920	116.50	0.328	178.75	0.053	241.00	0.008	303.25	0.001
54.50	1.903	116.75	0.326	179.00	0.053	241.25	0.008	303.50	0.001
54.75	1.887	117.00	0.323	179.25	0.053	241.50	0.008	303.75	0.001
55.00	1.871	117.25	0.321	179.50	0.052	241.75	0.008	304.00	0.001
55.25	1.855	117.50	0.319	179.75	0.052	242.00	0.008	304.25	0.001
55.50	1.840	117.75	0.317	180.00	0.051	242.25	0.008	304.50	0.001
55.75	1.825	118.00	0.314	180.25	0.051	242.50	0.008	304.75	0.001
56.00	1.810	118.25	0.312	180.50	0.051	242.75	0.008	305.00	0.001
56.25	1.795	118.50	0.310	180.75	0.050	243.00	0.008	305.25	0.001
56.50	1.781	118.75	0.308	181.00	0.050	243.25	0.008	305.50	0.001
56.75	1.766	119.00	0.305	181.25	0.050	243.50	0.008	305.75	0.001
57.00	1.752	119.25	0.303	181.50	0.049	243.75	0.008	306.00	0.001
57.25	1.738	119.50	0.301	181.75	0.049	244.00	0.008	306.25	0.001
57.50	1.725	119.75	0.299	182.00	0.048	244.25	0.007	306.50	0.001
57.75	1.711	120.00	0.297	182.25	0.048	244.50	0.007	306.75	0.001
58.00	1.698	120.25	0.295	182.50	0.048	244.75	0.007	307.00	0.001
58.25	1.685	120.50	0.293	182.75	0.047	245.00	0.007	307.25	0.001
58.50	1.672	120.75	0.291	183.00	0.047	245.25	0.007	307.50	0.001
58.75	1.659	121.00	0.289	183.25	0.047	245.50	0.007	307.75	0.001
59.00	1.646	121.25	0.286	183.50	0.046	245.75	0.007	308.00	0.001
59.25	1.634	121.50	0.284	183.75	0.046	246.00	0.007	308.25	0.001
59.50	1.621	121.75	0.282	184.00	0.046	246.25	0.007	308.50	0.001
59.75	1.609	122.00	0.280	184.25	0.045	246.50	0.007	308.75	0.001
60.00	1.597	122.25	0.278	184.50	0.045	246.75	0.007	309.00	0.001
60.25	1.585	122.50	0.276	184.75	0.045	247.00	0.007	309.25	0.001
60.50	1.574	122.75	0.274	185.00	0.044	247.25	0.007	309.50	0.001
60.75	1.562	123.00	0.272	185.25	0.044	247.50	0.007	309.75	0.001
61.00	1.550	123.25	0.271	185.50	0.044	247.75	0.007	309.75	0.001
61.25	1.539	123.50	0.269	185.75	0.043	248.00	0.007	309.75	0.001
61.50	1.528	123.75	0.267	186.00	0.043	248.25	0.007	309.75	0.001
61.75	1.517	124.00	0.265	186.25	0.043	248.50	0.007	309.75	0.001
62.00	1.506	124.25	0.263	186.50	0.042	248.75	0.006	309.75	0.001

ADD HYD (8188)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8186):	22248.87	85.102	13.75	19.05
+ ID2= 2 (8190):	1908.71	7.239	9.75	15.08
=====				
ID = 3 (8188):	24157.57	90.556	13.50	18.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB				
NASHVD (4462)	Area	(ha)=1366.10	Curve Number	(CN)= 61.0
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)=	1.50
	U.H. Tp(hrs)=	4.22		
Unit Hyd Qpeak	(cms)=	5.524		

PEAK FLOW (cms)= 3.109 (i)
 TIME TO PEAK (hrs)= 12.000
 RUNOFF VOLUME (mm)= 15.109
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.241

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB			
STANDHYD (4461)			
ID= 1 DT=15.0 min	Area	(ha)= 108.90	Dir. Conn.(%)= 30.00
	Total Imp(%)=	50.00	

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	54.45	54.45
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	852.06	40.00
Mannings n	=	0.013	0.250
Max.Eff.Inten.(mm/hr)	=	82.76	25.92
over	(min)	15.00	45.00
Storage Coeff.	(min)=	12.27 (ii)	30.63 (ii)
Unit Hyd. Tpeak	(min)=	15.00	45.00
Unit Hyd. peak	(cms)=	0.08	0.03
PEAK FLOW	(cms)=	5.92	2.40
TIME TO PEAK	(hrs)=	6.00	6.50
RUNOFF VOLUME	(mm)=	60.70	19.96
TOTAL RAINFALL	(mm)=	62.70	62.70
RUNOFF COEFFICIENT	=	0.97	0.32
			0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 61.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5446)				
IN= 2--> OUT= 1	OUTFLOW	STORAGE	OUTFLOW	STORAGE
DT= 15.0 min	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	1.6700	2.9494
	0.3690	1.4413	1.8200	3.3343
	0.5700	1.6830	1.9700	3.7111
	1.0600	2.1139	2.3700	3.7211
	1.3800	2.4971	0.0000	0.0000
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (4461)	108.900	6.926	6.00	32.18
OUTFLOW : ID= 1 (5446)	108.900	1.087	7.75	32.18
	PEAK FLOW	REDUCTION [Qout/qin](%)=	15.69	
	TIME SHIFT OF PEAK FLOW	(min)=	105.00	
	MAXIMUM STORAGE	USED	(ha.m.)=	2.1490

ADD HYD (8192)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (4462):	1366.10	3.109	12.00	15.11
+ ID2= 2 (5446):	108.90	1.087	7.75	32.18
=====				
ID = 3 (8192):	1475.00	3.691	10.25	16.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8194)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8188):	24157.57	90.556	13.50	18.73
+ ID2= 2 (8192):	1475.00	3.691	10.25	16.37
=====				
ID = 3 (8194):	25632.58	93.944	13.50	18.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB				
NASHVD (0442)	Area	(ha)= 117.26	Curve Number	(CN)= 43.0
ID= 1 DT=15.0 min	Ia	(mm)= 5.00	# of Linear Res.(N)=	1.50
	U.H. Tp(hrs)=	1.17		

Unit Hyd Qpeak (cms) = 1.705

PEAK FLOW (cms) = 0.395 (1)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 8.373
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.134

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0440) Area (ha) = 226.35 Curve Number (CN) = 60.0
NASHYD (0440) Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs) = 1.11

Unit Hyd Qpeak (cms) = 3.481

PEAK FLOW (cms) = 1.396 (1)
TIME TO PEAK (hrs) = 7.750
RUNOFF VOLUME (mm) = 14.536
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.232

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0438) Area (ha) = 130.70 Curve Number (CN) = 51.0
NASHYD (0438) Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs) = 0.60

Unit Hyd Qpeak (cms) = 3.738

PEAK FLOW (cms) = 0.963 (1)
TIME TO PEAK (hrs) = 6.750
RUNOFF VOLUME (mm) = 10.792
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.172

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0436) Area (ha) = 187.51 Curve Number (CN) = 55.0
NASHYD (0436) Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs) = 0.73

Unit Hyd Qpeak (cms) = 4.391

PEAK FLOW (cms) = 1.356 (1)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 12.334
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.197

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0434) Area (ha) = 56.64 Curve Number (CN) = 46.0
NASHYD (0434) Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs) = 0.52

Unit Hyd Qpeak (cms) = 1.867

PEAK FLOW (cms) = 0.394 (1)
TIME TO PEAK (hrs) = 6.500
RUNOFF VOLUME (mm) = 9.103
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.145

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8206) AREA QPEAK TPEAK R.V.
1 + 2 = 3 (ha) (cms) (hrs) (mm)
ID1= 1 (0434): 56.64 0.394 6.50 9.10
+ ID2= 2 (0436): 187.51 1.356 7.00 12.33
ID = 3 (8206): 244.15 1.736 6.75 11.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6438) Routing time step (min) = 15.00
IN= 2--> OUT= 1

DATA FOR SECTION (4381.0)

Table with columns: Distance, Elevation, Manning, and values ranging from 0.00 to 255.55.

TRAVEL TIME TABLE

Table with columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV.TIME and values ranging from 0.50 to 9.74.

hydrograph table with columns: AREA, QPEAK, TPEAK, R.V., MAX DEPTH, MAX VEL and values for INFLOW and OUTFLOW.

ADD HYD (8208) table with columns: AREA, QPEAK, TPEAK, R.V. and values for multiple IDs.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8210) table with columns: AREA, QPEAK, TPEAK, R.V. and values for multiple IDs.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0432) Area (ha) = 114.21 Curve Number (CN) = 53.0
NASHYD (0432) Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs) = 1.21

Unit Hyd Qpeak (cms) = 1.617

PEAK FLOW (cms) = 0.530 (1)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 11.676
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.186

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0430) | Area (ha)= 111.64 Curve Number (CN)= 55.0
| NASHVD (0430) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.52
-----
Unit Hyd Qpeak (cms)= 3.687

PEAK FLOW (cms)= 1.054 (1)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 12.200
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.195

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0428) | Area (ha)= 50.53 Curve Number (CN)= 51.0
| NASHVD (0428) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.50
-----
Unit Hyd Qpeak (cms)= 1.725

PEAK FLOW (cms)= 0.428 (1)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 10.720
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.171

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0426) | Area (ha)= 247.17 Curve Number (CN)= 61.0
| NASHVD (0426) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.98
-----
Unit Hyd Qpeak (cms)= 4.315

PEAK FLOW (cms)= 1.733 (1)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 14.967
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.239

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0424) | Area (ha)= 49.57 Curve Number (CN)= 59.0
| NASHVD (0424) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.37
-----
Unit Hyd Qpeak (cms)= 2.293

PEAK FLOW (cms)= 0.670 (1)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 13.586
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.217

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8198) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
|-----|
| ID1= 1 (0424): 49.57 0.670 6.50 13.59
| + ID2= 2 (0426): 247.17 1.733 7.25 14.97
|-----|
| ID = 3 (8198): 296.74 2.260 6.75 14.74
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

```

-----
| ROUTE CHN (6428) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00
-----
<----- DATA FOR SECTION (4281.0) ----->
Distance Elevation Manning
0.00 246.54 0.0900
8.80 246.29 0.0900
30.78 246.03 0.0900
41.78 243.97 0.0900
63.77 237.16 0.0900

```

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.78	228.28	.205E+04	1.6	0.97	21.60
1.58	229.08	.202E+05	16.6	1.02	20.37
2.39	229.89	.665E+05	64.8	1.22	17.11
3.19	230.69	.133E+06	158.1	1.49	14.05
4.00	231.50	.213E+06	308.6	1.82	11.48
4.80	232.30	.299E+06	504.9	2.11	9.89
5.61	233.11	.394E+06	746.6	2.37	8.79
6.41	233.91	.496E+06	1034.5	2.61	7.99
7.21	234.71	.605E+06	1373.3	2.84	7.35
8.02	235.52	.722E+06	1763.3	3.06	6.82
8.82	236.32	.844E+06	2202.0	3.27	6.39
9.63	237.13	.974E+06	2690.5	3.46	6.03
10.43	237.93	.111E+07	3239.5	3.66	5.71
11.23	238.73	.125E+07	3839.0	3.84	5.43
12.04	239.54	.140E+07	4489.1	4.03	5.18
12.84	240.34	.155E+07	5190.5	4.20	4.97
13.65	241.15	.170E+07	5943.8	4.36	4.78
14.45	241.95	.187E+07	6749.8	4.53	4.61
15.26	242.76	.204E+07	7609.4	4.68	4.46

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.78	228.28	.205E+04	1.6	0.97	21.60
1.58	229.08	.202E+05	16.6	1.02	20.37
2.39	229.89	.665E+05	64.8	1.22	17.11
3.19	230.69	.133E+06	158.1	1.49	14.05
4.00	231.50	.213E+06	308.6	1.82	11.48
4.80	232.30	.299E+06	504.9	2.11	9.89
5.61	233.11	.394E+06	746.6	2.37	8.79
6.41	233.91	.496E+06	1034.5	2.61	7.99
7.21	234.71	.605E+06	1373.3	2.84	7.35
8.02	235.52	.722E+06	1763.3	3.06	6.82
8.82	236.32	.844E+06	2202.0	3.27	6.39
9.63	237.13	.974E+06	2690.5	3.46	6.03
10.43	237.93	.111E+07	3239.5	3.66	5.71
11.23	238.73	.125E+07	3839.0	3.84	5.43
12.04	239.54	.140E+07	4489.1	4.03	5.18
12.84	240.34	.155E+07	5190.5	4.20	4.97
13.65	241.15	.170E+07	5943.8	4.36	4.78
14.45	241.95	.187E+07	6749.8	4.53	4.61
15.26	242.76	.204E+07	7609.4	4.68	4.46

<---- hydrograph ----> <-pipe / channel-->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8198)	296.74	2.26	6.75	14.74	0.82	0.97
OUTFLOW : ID= 1 (6428)	296.74	2.17	7.25	14.74	0.81	0.97

```

-----
| ADD HYD (8200) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
|-----|
| ID1= 1 (0428): 50.53 0.428 6.50 10.72
| + ID2= 2 (6428): 296.74 2.171 7.25 14.74
|-----|
| ID = 3 (8200): 347.27 2.537 7.25 14.15
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

```

-----
| ADD HYD (8202) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
|-----|
| ID1= 1 (0430): 111.64 1.054 6.50 12.20
| + ID2= 2 (8200): 347.27 2.537 7.25 14.15
|-----|
| ID = 3 (8202): 458.91 3.490 7.00 13.68
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6432) | IN= 2--> OUT= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (4321.0) ----->

Distance	Elevation	Manning
0.00	221.00	0.0600
15.29	219.85	0.0600
45.86	219.86	0.0600
76.44	219.96	0.0600
110.84	220.23	0.0600
112.00	220.15	0.0600
114.66	220.09	0.0600
118.48	219.73	0.0600
120.80	219.70	0.0600 / 0.0350
121.30	219.20	0.0350
122.30	219.20	0.0350
122.80	219.70	0.0350 / 0.0600
129.85	220.11	0.0600
175.81	220.24	0.0600
214.03	220.55	0.0600
252.25	220.61	0.0600

290.47 220.70 0.0600
 328.69 221.04 0.0600
 347.80 221.08 0.0600
 378.37 221.04 0.0600

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.08	219.28	.185E+03	0.0	0.23	149.38
0.17	219.37	.399E+03	0.1	0.34	100.62
0.25	219.45	.641E+03	0.1	0.42	80.98
0.33	219.53	.911E+03	0.2	0.49	69.79
0.42	219.62	.121E+04	0.3	0.55	62.35
0.50	219.70	.154E+04	0.5	0.60	56.91
0.61	219.81	.267E+04	0.7	0.56	61.28
0.71	219.91	.901E+04	1.3	0.30	112.96
0.82	220.02	.249E+05	3.4	0.28	122.71
0.92	220.12	.452E+05	6.8	0.31	109.98
1.03	220.23	.747E+05	12.2	0.33	102.05
1.14	220.34	.112E+06	21.3	0.39	87.59
1.24	220.44	.153E+06	33.0	0.44	77.01
1.35	220.55	.196E+06	47.2	0.49	69.22
1.46	220.66	.248E+06	62.3	0.51	66.48
1.56	220.76	.310E+06	83.4	0.55	62.06
1.67	220.87	.376E+06	108.8	0.59	57.56
1.77	220.97	.444E+06	137.7	0.64	53.77
1.88	221.08	.517E+06	163.6	0.65	52.70

----- hydrograph -----> <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8202)	458.91	2.33	9.25	13.68	0.76	0.29
OUTFLOW: ID= 1 (6432)	458.91	2.33	9.25	13.68	0.76	0.29

----- hydrograph -----> <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0432):	114.21	0.530	8.00	11.68
+ ID2= 2 (6432):	458.91	2.333	9.25	13.68
ID = 3 (8204):	573.12	2.816	9.00	13.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- hydrograph -----> <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8204):	573.12	2.816	9.00	13.28
+ ID2= 2 (8210):	601.20	3.750	7.50	12.52
ID = 3 (8212):	1174.32	6.240	8.00	12.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- hydrograph -----> <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8204):	573.12	2.816	9.00	13.28
+ ID2= 2 (8210):	601.20	3.750	7.50	12.52
ID = 3 (8212):	1174.32	6.240	8.00	12.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- DATA FOR SECTION (4421.0) ----->

Distance	Elevation	Manning	
0.00	221.00	0.0350	
26.73	220.80	0.0350	
53.47	220.60	0.0350	
66.83	220.40	0.0350	
80.20	220.20	0.0350	
347.52	220.00	0.0350	
354.21	219.59	0.0350	
360.89	219.50	0.0350	
367.57	219.08	0.0350	
368.76	219.00	0.0350 / 0.0300	Main Channel
369.26	218.50	0.0300	Main Channel
374.26	218.50	0.0300	Main Channel
374.76	219.00	0.0300 / 0.0350	Main Channel
380.94	220.33	0.0350	
387.62	219.62	0.0350	
394.31	219.23	0.0350	
454.45	219.07	0.0350	
514.60	219.09	0.0350	
660.00	219.19	0.0350	
661.63	221.00	0.0350	

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.12	218.62	.100E+04	0.2	0.28	92.89
0.25	218.75	.205E+04	0.6	0.43	60.09
0.37	218.87	.315E+04	1.1	0.55	46.99

0.50	219.00	.430E+04	1.8	0.66	39.67
0.63	219.13	.141E+05	3.4	0.37	69.99
0.77	219.27	.652E+05	13.7	0.33	79.14
0.90	219.40	.124E+06	35.5	0.45	58.02
1.03	219.53	.183E+06	65.7	0.56	46.45
1.17	219.67	.245E+06	103.8	0.66	39.30
1.30	219.80	.307E+06	149.3	0.76	34.30
1.43	219.93	.371E+06	201.6	0.85	30.65
1.57	220.07	.440E+06	256.5	0.91	28.56
1.70	220.20	.541E+06	325.7	0.94	27.70
1.83	220.33	.663E+06	415.2	0.98	26.62
1.97	220.47	.787E+06	520.3	1.03	25.21
2.10	220.60	.913E+06	638.5	1.09	23.82
2.23	220.73	.104E+07	767.2	1.15	22.62
2.37	220.87	.117E+07	908.0	1.21	21.54
2.50	221.00	.131E+07	1060.7	1.27	20.57

----- hydrograph -----> <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8212)	1174.32	6.24	8.00	12.89	0.67	0.36
OUTFLOW: ID= 1 (6442)	1174.32	5.46	9.50	12.89	0.66	0.36

----- hydrograph -----> <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0442):	117.26	0.395	8.00	8.37
+ ID2= 2 (6442):	1174.32	5.460	9.50	12.89
ID = 3 (8214):	1291.58	5.802	9.50	12.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- hydrograph -----> <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8194):	25632.58	93.944	13.50	18.60
+ ID2= 2 (8214):	1291.58	5.802	9.50	12.48
ID = 3 (8196):	26924.15	97.761	13.25	18.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- hydrograph -----> <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0444):	221.65	1.266	7.50	12.82
+ ID2= 2 (8196):	26924.15	97.761	13.25	18.30
ID = 3 (8216):	27145.80	98.177	13.25	18.26

Unit Hyd Qpeak (cms) = 3.656

PEAK FLOW (cms) = 1.266 (1)

TIME TO PEAK (hrs) = 7.500

RUNOFF VOLUME (mm) = 12.815

TOTAL RAINFALL (mm) = 62.700

RUNOFF COEFFICIENT = 0.204

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- hydrograph -----> <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0444):	221.65	1.266	7.50	12.82
+ ID2= 2 (8196):	26924.15	97.761	13.25	18.30
ID = 3 (8216):	27145.80	98.177	13.25	18.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

**EXISTING
EAST HOLLAND RIVER 25-YEAR STORM**

```
V V I SSSS U U A L
V V I SS U U A A L
V V I SS U U A A A L
V V I SS U U A A L
W V I SSSS UUUU A A LLLL
```

```
OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y MM MM O O
O O T T H H Y Y M M O O
OOO T T H H Y Y M M OOO Company Serial
```

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***** DETAILED OUTPUT *****

Input filename: C:\Program Files\Visual Ortymo 2.4r\VO2\voind.dat
Output filename: C:\Users\jscott\AppData\Local\Temp\b43ea7bc-a90f-4b93-818e-3b13e8d86fb7\Scenario.out
Summary filename: C:\Users\jscott\AppData\Local\Temp\b43ea7bc-a90f-4b93-818e-3b13e8d86fb7\Scenario.sum

DATE: 08/22/2012 TIME: 01:27:52

USER:

COMMENTS: _____

** SIMULATION NUMBER: 1 **

READ STORM | Filename: C:\Users\jscott\AppData\Local\Temp\b43ea7bc-a90f-4b93-818e-3b13e8d86fb7\4619ab72
| | Comments: 25-Year 12-Hour SCS II Design Storm

| | Total= 73.10 mm

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.83	3.25	2.92	6.25	13.16	9.25	2.56
0.50	1.83	3.50	2.92	6.50	13.16	9.50	2.56
0.75	1.83	3.75	2.92	6.75	5.85	9.75	2.56
1.00	1.83	4.00	2.92	7.00	5.85	10.00	2.56
1.25	1.83	4.25	4.39	7.25	4.39	10.25	1.46
1.50	1.83	4.50	4.39	7.50	4.39	10.50	1.46
1.75	1.83	4.75	5.85	7.75	4.39	10.75	1.46
2.00	1.83	5.00	5.85	8.00	4.39	11.00	1.46
2.25	2.19	5.25	8.77	8.25	2.56	11.25	1.46
2.50	2.19	5.50	8.77	8.50	2.56	11.50	1.46
2.75	2.19	5.75	35.09	8.75	2.56	11.75	1.46
3.00	2.19	6.00	96.49	9.00	2.56	12.00	1.46

CALIB | STANDBYD (1662) | Area (ha)= 158.10
| ID= 1 DT=15.0 min | Total Imp(%)= 58.00 Dir. Conn.(%)= 30.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	91.70	66.40
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1026.65	40.00
Mannings n =	0.013	0.250
Max.Eff.Inten.(mm/hr)=	96.49	100.04
over (min)	15.00	30.00
Storage Coeff. (min)=	12.90 (ii)	23.60 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04
PEAK FLOW (cms)=	9.84	8.74
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	71.10	38.86
TOTAL RAINFALL (mm)=	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.53

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB | STANDBYD (1661) | Area (ha)= 276.60
| ID= 1 DT=15.0 min | Total Imp(%)= 40.00 Dir. Conn.(%)= 21.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	110.64	165.96
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1357.94	40.00
Mannings n =	0.013	0.250
Max.Eff.Inten.(mm/hr)=	96.49	69.46
over (min)	15.00	30.00
Storage Coeff. (min)=	15.26 (ii)	27.64 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.07	0.04
PEAK FLOW (cms)=	11.26	13.91
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	71.10	34.15
TOTAL RAINFALL (mm)=	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.47

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (S166) | IN= 2--> OUT= 1 | DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0000	6.0870	6.1196
0.3730	3.2753	7.7230	7.0042	
1.3370	3.4678	9.3080	7.8876	
2.9800	4.4433	9.7080	7.8976	
4.4520	5.2716	0.0000	0.0000	

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (1661)	276.600	20.119	6.00	41.91
OUTFLOW: ID= 1 (S166)	276.600	6.179	7.00	41.90

PEAK FLOW REDUCTION [Qout/Qin](%)= 30.71
TIME SHIFT OF PEAK FLOW (min)= 60.00
MAXIMUM STORAGE USED (ha.m.)= 6.1963

ADD HYD (8134) | 1 * 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)

ID= 1 (1662):	158.10	15.535	6.00	48.53
+ ID2= 2 (S166):	276.60	6.179	7.00	41.90
ID = 3 (8134):	434.70	15.830	6.00	44.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB | STANDBYD (1642) | Area (ha)= 132.50
| ID= 1 DT=15.0 min | Total Imp(%)= 20.00 Dir. Conn.(%)= 15.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	26.50	106.00
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	939.86	40.00
Mannings n =	0.013	0.250
Max.Eff.Inten.(mm/hr)=	96.49	47.41
over (min)	15.00	30.00
Storage Coeff. (min)=	12.24 (ii)	26.66 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04
PEAK FLOW (cms)=	4.21	6.14
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	71.10	28.99
TOTAL RAINFALL (mm)=	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.40

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB
 STANDHYD (1641) Area (ha)= 101.70 Dir. Conn.(%)= 52.00
 ID= 1 DT=15.0 min Total Imp(%)= 70.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	71.19	30.51
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	823.41	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	96.49	91.81
over (min)	15.00	30.00
Storage Coeff. (min)	11.30 (ii)	22.37 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04
PEAK FLOW (cms)	11.51	3.77
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	73.10	37.18
TOTAL RAINFALL (mm)	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.51

TOTALS
 13.957 (iii)
 6.00
 54.82
 73.10
 0.75

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5164)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IS 2--> OUT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min	0.0000	0.0000	7.6100	4.1638
	0.3440	1.7056	8.2090	4.5053
	3.1340	1.9730	8.8070	4.8468
	4.1310	3.0607	9.2070	4.8568
	5.1070	3.7142	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (1641)	101.700	13.957	6.00	54.82
OUTFLOW: ID= 1 (5164)	101.700	3.941	6.75	54.80

PEAK FLOW REDUCTION [Qout/Qin](%)= 28.23
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 2.8909

ADD HYD (8132)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1642):	132.50	8.047	6.00	35.31
+ ID2= 2 (5164):	101.70	3.941	6.75	54.80

ID = 3 (8132):	234.20	11.570	6.25	43.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB
 NASHYD (0162) Area (ha)= 190.14 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.79

Unit Hyd Qpeak (cms)= 4.104

PEAK FLOW (cms)= 2.316 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 21.664
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.296

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB
 NASHYD (0160) Area (ha)= 318.82 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.45

Unit Hyd Qpeak (cms)= 3.754

PEAK FLOW (cms)= 2.227 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 20.002
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.274

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB
 NASHYD (0152) Area (ha)= 124.37 Curve Number (CN)= 59.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.89

Unit Hyd Qpeak (cms)= 2.372

PEAK FLOW (cms)= 1.174 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 18.731
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.256

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB
 NASHYD (0148) Area (ha)= 417.89 Curve Number (CN)= 26.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.18

Unit Hyd Qpeak (cms)= 6.067

PEAK FLOW (cms)= 0.972 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 5.816
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.080

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB
 NASHYD (0150) Area (ha)= 105.88 Curve Number (CN)= 17.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.58

Unit Hyd Qpeak (cms)= 3.138

PEAK FLOW (cms)= 0.251 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 3.463
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.047

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8116)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0148):	417.89	0.972	8.00	5.82
+ ID2= 2 (0150):	105.88	0.251	6.75	3.46

ID = 3 (8116):	523.77	1.199	7.50	5.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CNB (6152)
 IIS 2--> OUT= 1 Routing time step (min)= 15.00

<----- DATA FOR SECTION (1521.0) ----->			
Distance	Elevation	Manning	
0.00	290.00	0.0500	
15.34	286.87	0.0500	
46.03	284.84	0.0500	
88.22	283.07	0.0500	
126.57	282.11	0.0500	
153.42	280.34	0.0500	
157.26	280.07	0.0500	
161.09	279.72	0.0500	

167.76	278.94	0.0500	/0.0300	Main Channel
168.01	278.60	0.0300		Main Channel
168.76	278.60	0.0300		Main Channel
169.51	278.60	0.0300		Main Channel
169.76	278.90	0.0300	/0.0500	Main Channel
173.76	279.40	0.0500		
180.27	279.69	0.0500		
184.11	279.93	0.0500		
218.62	281.86	0.0500		
260.82	283.20	0.0500		
360.54	289.25	0.0500		
379.72	289.80	0.0500		

TRAVEL TIME TABLE					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.30	278.90	114E+04	0.4	0.80	45.70
0.87	279.47	932E+04	4.2	0.98	37.22
1.45	280.05	353E+05	17.5	1.09	33.68
2.02	280.62	831E+05	47.8	1.26	29.01
2.59	281.19	155E+06	102.9	1.46	25.06
3.17	281.77	250E+06	189.3	1.66	22.03
3.74	282.34	375E+06	298.5	1.75	20.91
4.32	282.92	547E+06	456.5	1.83	19.98
4.89	283.49	767E+06	705.7	2.02	18.12
5.46	284.06	102E+07	1038.0	2.24	16.34
6.04	284.64	130E+07	1444.1	2.45	14.98
6.61	285.21	161E+07	1940.1	2.66	13.79
7.18	285.78	194E+07	2522.6	2.86	12.80
7.76	286.36	229E+07	3187.4	3.06	11.98
8.33	286.93	267E+07	3942.3	3.25	11.28
8.91	287.51	306E+07	4824.9	3.46	10.58
9.48	288.08	347E+07	5795.7	3.67	9.99
10.05	288.65	390E+07	6855.4	3.86	9.48
10.63	289.23	434E+07	8005.0	4.05	9.04

hydrograph						<-pipe / channel-->	
INFLOW : ID= 2 (8116)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
OUTFLOW : ID= 1 (6152)	523.77	1.18	7.50	5.34	0.42	0.83	
	523.77	1.11	8.50	5.34	0.40	0.83	

ADD HYD (8122)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0152):	124.37	1.174	7.00	18.73
+ ID2= 2 (6152):	523.77	1.107	8.50	5.34
ID = 3 (8122):	648.14	2.191	8.00	7.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB				
NASHVD (0158)	Area (ha)	Ia (mm)	Curve Number	(CN)= 61.0
ID= 1 DT=15.0 min	178.59	5.00	# of Linear Res.(N)=	1.50
U.H. Tp(hrs)=	0.94			

Unit Hyd Qpeak (cms) = 3.230
 PEAK FLOW (cms) = 1.724 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 19.897
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.272

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB				
NASHVD (0156)	Area (ha)	Ia (mm)	Curve Number	(CN)= 45.0
ID= 1 DT=15.0 min	83.49	5.00	# of Linear Res.(N)=	1.50
U.H. Tp(hrs)=	0.62			

Unit Hyd Qpeak (cms) = 2.306
 PEAK FLOW (cms) = 0.666 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 11.996
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.164

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB				
NASHVD (0154)	Area (ha)	Ia (mm)	Curve Number	(CN)= 37.0
ID= 1 DT=15.0 min	200.32	5.00	# of Linear Res.(N)=	1.50

U.H. Tp(hrs)= 0.97

Unit Hyd Qpeak (cms) = 3.537

PEAK FLOW (cms) = 0.852 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 9.165
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.125

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8118)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0154):	200.32	0.852	7.25	9.16
+ ID2= 2 (0156):	83.49	0.666	6.75	12.00
ID = 3 (8118):	283.81	1.496	7.00	10.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6158)		ROUTING time step (min) = 15.00
IN= 2-->	OUT= 1	

DATA FOR SECTION (1581.0)			
Distance	Elevation	Manning	
0.00	290.00	0.0400	
19.20	288.88	0.0400	
48.00	288.26	0.0400	
119.99	283.68	0.0400	
167.99	282.07	0.0400	
196.79	281.32	0.0400	
210.79	280.90	0.0400	
219.79	280.38	0.0400 /0.0300	Main Channel
220.04	279.65	0.0300	Main Channel
220.79	279.65	0.0300	Main Channel
221.54	279.65	0.0300	Main Channel
221.79	280.39	0.0300 /0.0400	Main Channel
225.79	280.59	0.0400	
235.19	280.88	0.0400	
254.39	281.50	0.0400	
273.58	282.56	0.0400	
302.38	283.42	0.0400	
359.98	284.03	0.0400	
436.77	289.37	0.0400	
475.17	290.37	0.0400	

TRAVEL TIME TABLE					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.37	280.01	973E+03	0.5	0.86	31.78
0.73	280.38	210E+04	1.5	1.18	23.16
1.30	280.95	149E+05	8.6	0.95	28.91
1.86	281.51	579E+05	40.0	1.13	24.12
2.43	282.08	133E+06	118.5	1.46	18.75
2.99	282.64	236E+06	254.6	1.77	15.46
3.56	283.21	369E+06	450.3	2.00	13.67
4.13	283.78	542E+06	702.7	2.13	12.86
4.69	284.34	767E+06	1125.2	2.41	11.36
5.26	284.91	101E+07	1697.1	2.75	9.93
5.82	285.47	127E+07	2379.1	3.07	8.91
6.39	286.04	155E+07	3171.4	3.36	8.14
6.95	286.60	184E+07	4075.5	3.64	7.53
7.52	287.17	215E+07	5093.1	3.89	7.03
8.09	287.74	247E+07	6226.5	4.13	6.62
8.65	288.30	281E+07	7461.2	4.36	6.28
9.22	288.87	318E+07	8635.1	4.46	6.13
9.78	289.43	357E+07	10143.6	4.67	5.86
10.35	290.00	398E+07	11608.8	4.79	5.71

hydrograph							<-pipe / channel-->	
INFLOW : ID= 2 (8118)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)		
OUTFLOW : ID= 1 (6158)	283.81	1.46	7.50	10.00	0.71	1.18		
	283.81	1.46	7.50	10.00	0.71	1.16		

ADD HYD (8120)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0158):	178.59	1.724	7.25	19.90
+ ID2= 2 (6158):	283.81	1.458	7.50	10.00
ID = 3 (8120):	462.40	3.182	7.25	13.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8124)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8120):	462.40	3.182	7.25	13.82
+ ID2= 2 (8122):	648.14	2.191	8.00	7.91
=====				
ID = 3 (8124):	1110.54	5.324	7.50	10.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6160) | Routing time step (min) = 15.00
 IN= 2--> OUF= 1 |

----- DATA FOR SECTION (1601.0) -----					
Distance	Elevation	Manning			
0.00	261.02	0.0400			
29.89	254.30	0.0400			
40.75	252.58	0.0400			
62.49	251.30	0.0400			
114.11	250.94	0.0400			
130.41	248.58	0.0400			
141.28	247.50	0.0400			
154.86	246.87	0.0400			
155.20	246.70	0.0400			
160.20	246.30	0.0400 / 0.0300	Main Channel		
160.30	245.30	0.0300	Main Channel		
165.20	245.30	0.0300	Main Channel		
165.30	246.30	0.0300 / 0.0400	Main Channel		
168.45	246.96	0.0400			
195.62	249.20	0.0400			
203.77	250.82	0.0400			
225.50	256.78	0.0400			
244.52	261.14	0.0400			
255.39	261.95	0.0400			
268.97	261.92	0.0400			

----- TRAVEL TIME TABLE -----					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.50	245.80	120E+05	2.9	1.18	68.31
1.00	246.30	242E+05	8.5	1.70	47.47
1.92	247.22	825E+05	31.7	1.86	43.34
2.84	248.14	248E+06	102.5	2.00	40.39
3.76	249.06	505E+06	247.7	2.37	34.03
4.68	249.98	832E+06	486.8	2.83	28.49
5.60	250.90	121E+07	814.9	3.27	24.71
6.52	251.82	180E+07	1157.1	3.11	25.97
7.44	252.74	253E+07	1840.2	3.52	22.92
8.36	253.66	331E+07	2727.1	3.98	20.25
9.29	254.59	414E+07	3782.6	4.43	18.22
10.21	255.51	499E+07	5010.7	4.86	16.61
11.13	256.43	589E+07	6397.4	5.26	15.33
12.05	257.35	681E+07	7932.8	5.64	14.31
12.97	258.27	777E+07	9621.2	5.99	13.46
13.89	259.19	877E+07	11469.4	6.33	12.74
14.81	260.11	990E+07	13478.4	6.66	12.12
15.73	261.03	109E+08	15651.7	6.97	11.58
16.65	261.95	120E+08	17492.5	7.07	11.42

----- hydrograph -----						<-pipe / channel-->	
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL		
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW : ID= 2 (8124) 1110.54	5.32	7.50	10.37	0.72	1.36		
OUTFLOW : ID= 1 (6160) 1110.54	4.68	8.75	10.37	0.66	1.31		

ADD HYD (8126)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0160):	318.82	2.227	8.00	20.00
+ ID2= 2 (6160):	1110.54	4.684	8.75	10.37
=====				
ID = 3 (8126):	1429.36	6.875	8.50	12.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8128)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0162):	190.14	2.316	7.00	21.66
+ ID2= 2 (8126):	1429.36	6.875	8.50	12.52
=====				
ID = 3 (8128):	1619.50	8.752	8.25	13.59

ADD HYD (8130)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8128):	1619.50	8.752	8.25	13.59
+ ID2= 2 (8132):	234.20	11.570	6.25	43.77
=====				
ID = 3 (8130):	1853.70	15.773	6.25	17.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6166) | Routing time step (min) = 15.00
 IN= 2--> OUF= 1 |

----- DATA FOR SECTION (1661.0) -----					
Distance	Elevation	Manning			
0.00	248.36	0.0500			
41.67	246.98	0.0500			
95.82	244.93	0.0500			
149.98	242.42	0.0500			
191.64	241.88	0.0500			
224.97	239.90	0.0500			
229.13	239.35	0.0500			
233.30	237.77	0.0500			
237.47	236.57	0.0500			
240.63	235.60	0.0500 / 0.0300	Main Channel		
241.63	234.10	0.0300	Main Channel		
247.13	234.10	0.0300	Main Channel		
247.53	235.60	0.0300 / 0.0500	Main Channel		
254.13	236.25	0.0500			
266.63	236.77	0.0500			
283.29	237.84	0.0500			
291.63	238.89	0.0500			
329.12	244.66	0.0500			
370.78	249.55	0.0500			
412.44	253.17	0.0500			

----- TRAVEL TIME TABLE -----					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.75	234.85	130E+05	6.0	1.36	36.45
1.50	235.60	276E+05	18.1	1.95	25.41
2.25	236.35	544E+05	40.0	2.18	22.69
3.00	237.10	117E+06	80.1	2.04	24.32
3.75	237.85	213E+06	148.9	2.07	23.89
4.50	238.60	334E+06	255.1	2.27	21.85
5.25	239.35	473E+06	395.8	2.49	19.90
6.00	240.10	631E+06	563.8	2.65	18.66
6.75	240.85	824E+06	773.2	2.79	17.77
7.50	241.60	106E+07	1037.9	2.92	16.97
8.26	242.36	135E+07	1326.7	2.92	16.94
9.01	243.11	173E+07	1758.5	3.01	16.44
9.76	243.86	217E+07	2285.9	3.13	15.81
10.51	244.61	265E+07	2909.5	3.26	15.17
11.26	245.36	318E+07	3610.1	3.37	14.67
12.01	246.11	377E+07	4417.4	3.48	14.21
12.76	246.86	441E+07	5348.0	3.60	13.75
13.51	247.61	512E+07	6394.0	3.71	13.34
14.26	248.36	589E+07	7580.3	3.82	12.95

----- hydrograph -----						<-pipe / channel-->	
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL		
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW : ID= 2 (8130) 1853.70	15.77	6.25	17.41	1.36	1.80		
OUTFLOW : ID= 1 (6166) 1853.70	13.28	7.25	17.41	1.20	1.66		

ADD HYD (8136)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6166):	1853.70	13.285	7.25	17.41
+ ID2= 2 (8134):	434.70	15.830	6.00	44.31
=====				
ID = 3 (8136):	2288.40	24.489	6.50	22.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STAMDHYD (2021) | Area (ha)= 81.20
 | ID= 1 DT=15.0 min | Total Imp(%)= 40.00 Dir. Conn.(%)= 24.00

Surface Area	(ha)=	IMPERVIOUS	PERVIOUS (i)
		36.48	54.72

Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 779.74 40.00
 Mannings n = 0.013 0.250
 Max.Eff.Inten.(mm/hr)= 96.49 65.28
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 10.94 (ii) 23.63 (iii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

 PEAK FLOW (cms)= 4.82 4.66 7.793 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 71.10 33.37 42.42
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.46 0.58

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 73.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR (5202)
 IN= 2--> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	4.0850	2.2999
	0.3100	0.9668	5.0850	2.6924
	1.2590	1.1373	5.9090	3.1012
	2.2140	1.5358	6.3090	3.1112
	3.0620	1.8918	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (2021)	91.200	7.793	6.00	42.42
OUTFLOW: ID= 1 (5202)	91.200	2.867	6.75	42.41

 PEAK FLOW REDUCTION [Qout/Qin](%)= 36.78
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 1.8102

 CALIB
 STANDHYD (2022)
 ID= 1 DT=15.0 min
 Total Imp(%)= 57.00 Dir. Conn.(%)= 34.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	215.06	162.24
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1585.98	40.00
Mannings n	0.013	0.250

	Max.Eff.Inten.(mm/hr)	over (min)	Storage Coeff. (min)	Unit Hyd. Tpeak (min)	Unit Hyd. peak (cms)
	96.49	15.00	15.75 (ii)	15.00	0.07
	84.00	30.00	28.22 (ii)	30.00	0.04

 PEAK FLOW (cms)= 23.85 16.32 34.300 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 71.10 35.49 47.60
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.49 0.65
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 71.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8110)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (2022):	377.30	34.300	6.00	47.60
+ ID2= 2 (5202):	91.20	2.867	6.75	42.41
ID = 3 (8110):	468.50	34.598	6.00	46.59

 NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB
 STANDHYD (0102)
 Area (ha)= 466.00

|ID= 1 DT=15.0 min | Total Imp(%)= 23.00 Dir. Conn.(%)= 9.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	107.18	358.82
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1762.57	40.00
Mannings n	0.013	0.250

	Max.Eff.Inten.(mm/hr)	over (min)	Storage Coeff. (min)	Unit Hyd. Tpeak (min)	Unit Hyd. peak (cms)
	96.49	15.00	17.85 (ii)	15.00	0.06
	45.00	30.00	39.44 (ii)	30.00	0.03

 PEAK FLOW (cms)= 7.57 9.30 11.959 (iii)
 TIME TO PEAK (hrs)= 6.00 6.00 6.50
 RUNOFF VOLUME (mm)= 71.10 16.21 21.15
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.22 0.29

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 49.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB
 NASHYD (0100)
 ID= 1 DT=15.0 min | Area (ha)= 693.84 Curve Number (CN)= 40.0
 | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.95
 Unit Hyd Qpeak (cms)= 6.087
 PEAK FLOW (cms)= 1.955 (i)
 TIME TO PEAK (hrs)= 9.250
 RUNOFF VOLUME (mm)= 10.287
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.141
 (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8000)

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
+ ID1= 1 (0100):	693.84	1.955	9.25	10.29
+ ID2= 2 (0102):	466.00	11.959	6.50	21.15
ID = 3 (8000):	1159.84	13.215	6.50	14.65

 NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6104)
 IN= 2--> OUT= 1
 Routing time step (min)= 15.00

 <----- DATA FOR SECTION (1041.0) ----->

Distance	Elevation	Manning
0.00	330.16	0.0450
50.03	328.36	0.0450
89.34	326.33	0.0450
117.94	324.30	0.0450
160.82	322.75	0.0450
178.34	319.96	0.0450
183.34	319.81	0.0450
184.34	319.38	0.0450 /0.0300 Main Channel
185.09	318.78	0.0300 Main Channel
185.84	318.72	0.0300 Main Channel
186.84	319.32	0.0300 /0.0450 Main Channel
193.84	319.70	0.0450
198.84	320.38	0.0450
200.13	320.81	0.0450
218.00	322.49	0.0450
239.44	323.49	0.0450
250.17	323.96	0.0450
275.18	325.20	0.0450
310.92	326.47	0.0450
353.81	330.00	0.0450

 TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.30	319.02	624E+01	0.1	0.45	73.00
0.60	319.32	.181E+04	0.6	0.67	49.46
1.23	319.95	.128E+05	4.9	0.77	43.38
1.86	320.58	.396E+05	19.1	0.96	34.50

2.48	321.20	.757E+05	43.5	1.15	28.98
3.11	321.83	.125E+06	81.2	1.30	25.58
3.74	322.46	.187E+06	135.9	1.45	22.92
4.37	323.09	.269E+06	195.6	1.45	22.90
5.00	323.72	.387E+06	287.8	1.48	22.44
5.63	324.35	.545E+06	428.4	1.56	21.22
6.25	324.97	.735E+06	630.5	1.71	19.42
6.88	325.60	.952E+06	876.1	1.83	18.12
7.51	326.23	.120E+07	1177.0	1.95	17.03
8.14	326.86	.149E+07	1554.4	2.08	15.93
8.77	327.49	.179E+07	2009.7	2.23	14.88
9.40	328.12	.213E+07	2533.9	2.37	13.99
10.02	328.74	.249E+07	3112.6	2.49	13.31
10.65	329.37	.288E+07	3760.9	2.60	12.75
11.28	330.00	.330E+07	4494.8	2.71	12.23

<---- hydrograph ----> <-pipe / channel-->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8000) 1159.84	13.22	6.50	14.65	1.59	0.87
OUTFLOW: ID= 1 (6104) 1159.84	9.58	6.75	14.65	1.43	0.82

CALIB	Area	(ha)	Curve Number	(CN)=
NASHVD (0104)	527.81	5.00	1.50	1.50
ID= 1 DT=15.0 min	Ia	(mm)	# of Linear Res. (N)	1.50
U.H. Tp(hrs)	1.34			

Unit Hyd Qpeak (cms) = 6.735

PEAK FLOW (cms) = 2.204 (l)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 11.381
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.156

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8002)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0104):	527.81	2.204	8.00	11.38
+ ID2= 2 (6104):	1159.84	9.578	6.75	14.65
ID = 3 (8002):	1687.65	11.468	7.00	13.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)=
NASHVD (0106)	259.52	5.00	1.50	1.50
ID= 1 DT=15.0 min	Ia	(mm)	# of Linear Res. (N)	1.50
U.H. Tp(hrs)	1.23			

Unit Hyd Qpeak (cms) = 3.614

PEAK FLOW (cms) = 1.709 (l)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 16.681
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.228

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8004)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0106):	259.52	1.709	8.00	16.68
+ ID2= 2 (8002):	1687.65	11.468	7.00	13.63
ID = 3 (8004):	1947.17	13.081	7.00	14.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6108)	Routing time step (min)*=
IN= 2--> OUT= 1	15.00

<----- DATA FOR SECTION (1081.0) ----->

Distance	Elevation	Manning
0.00	326.06	0.0400
12.06	325.74	0.0400
30.15	324.28	0.0400
59.27	320.35	0.0400
72.37	317.60	0.0400

87.45	314.52	0.0400
93.48	313.91	0.0400
105.54	313.55	0.0400
106.17	313.40	0.0400 / 0.0300
110.62	313.04	0.0300
110.82	312.97	0.0300
111.57	313.08	0.0300
111.67	313.48	0.0300 / 0.0400
129.66	316.62	0.0400
150.77	318.95	0.0400
180.92	319.61	0.0400
205.04	321.23	0.0400
232.18	322.09	0.0400
268.37	322.31	0.0400
298.52	326.00	0.0400

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.43	313.40	.244E+04	0.7	0.59	58.83
1.09	314.06	.214E+05	10.4	1.01	34.24
1.76	314.73	.600E+05	40.2	1.39	24.89
2.42	315.39	.109E+06	92.1	1.75	19.80
3.08	316.05	.169E+06	166.5	2.05	16.88
3.75	316.72	.237E+06	264.4	2.31	14.97
4.41	317.38	.318E+06	386.3	2.52	13.72
5.07	318.04	.411E+06	539.3	2.72	12.71
5.74	318.71	.517E+06	725.5	2.91	11.88
6.40	319.37	.642E+06	909.2	2.94	11.77
7.06	320.03	.806E+06	1179.3	3.04	11.38
7.72	320.69	.989E+06	1515.9	3.18	10.87
8.39	321.36	.119E+07	1895.5	3.30	10.49
9.05	322.02	.143E+07	2310.2	3.36	10.30
9.71	322.68	.172E+07	2763.2	3.33	10.39
10.38	323.35	.205E+07	3477.6	3.53	9.81
11.04	324.01	.238E+07	4284.3	3.73	9.27
11.70	324.67	.274E+07	5154.0	3.91	8.85
12.37	325.34	.311E+07	6104.0	4.08	8.48

<---- hydrograph ----> <-pipe / channel-->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8004) 1947.17	13.08	7.00	14.04	1.15	1.04
OUTFLOW: ID= 1 (6108) 1947.17	11.95	7.50	14.04	1.13	1.03

CALIB	Area	(ha)	Curve Number	(CN)=
NASHVD (0108)	207.05	5.00	1.50	1.50
ID= 1 DT=15.0 min	Ia	(mm)	# of Linear Res. (N)	1.50
U.H. Tp(hrs)	1.03			

Unit Hyd Qpeak (cms) = 3.432

PEAK FLOW (cms) = 0.554 (l)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 6.084
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.083

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8006)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0108):	207.05	0.554	7.50	6.08
+ ID2= 2 (6108):	1947.17	11.951	7.50	14.04
ID = 3 (8006):	2154.22	12.505	7.50	13.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)=
NASHVD (0110)	323.92	5.00	1.50	1.50
ID= 1 DT=15.0 min	Ia	(mm)	# of Linear Res. (N)	1.50
U.H. Tp(hrs)	1.35			

Unit Hyd Qpeak (cms) = 4.087

PEAK FLOW (cms) = 1.806 (l)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 15.228
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.208

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| ADD HYD (8008) |
| 1 + 2 = 3 |
-----
ID1= 1 (0110): 323.92 1.806 8.00 15.23
+ ID2= 2 (8006): 2154.22 12.505 7.50 13.27
=====
ID = 3 (8008): 2478.14 14.266 7.50 13.53

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| ROUTE CHN (6112) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)'= 15.00

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<----- DATA FOR SECTION (1121.0) ----->
Distance Elevation Manning
0.00 320.59 0.0400
13.48 317.93 0.0400
40.45 311.52 0.0400
64.04 307.68 0.0400
74.15 307.01 0.0400
91.00 307.22 0.0400
107.86 305.21 0.0400
111.23 304.86 0.0400
113.32 304.60 0.0400 /0.0300 Main Channel
117.77 304.24 0.0300 Main Channel
117.97 304.17 0.0300 Main Channel
118.72 304.28 0.0300 Main Channel
118.82 304.68 0.0300 /0.0400 Main Channel
124.71 304.76 0.0400
171.90 306.30 0.0400
205.60 307.24 0.0400
239.31 308.24 0.0400
262.90 310.64 0.0400
289.86 314.34 0.0400
333.68 317.66 0.0400

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<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.43 304.60 372E+04 0.6 0.54 97.65
1.12 305.29 463E+05 12.5 0.85 61.89
1.80 305.97 149E+06 53.0 1.13 46.85
2.49 306.66 311E+06 134.2 1.37 38.62
3.18 307.35 554E+06 261.6 1.50 35.27
3.87 308.04 894E+06 490.7 1.74 30.37
4.55 308.72 129E+07 833.0 2.05 25.75
5.24 309.41 171E+07 1269.8 2.36 22.39
5.93 310.10 215E+07 1789.0 2.64 20.01
6.62 310.79 261E+07 2392.7 2.90 18.21
7.30 311.47 310E+07 3087.8 3.16 16.74
7.99 312.16 361E+07 3873.6 3.40 15.52
8.68 312.85 413E+07 4741.0 3.64 14.52
9.37 313.54 467E+07 5689.3 3.86 13.69
10.05 314.22 523E+07 6718.4 4.07 12.97
10.74 314.91 581E+07 7770.7 4.24 12.46
11.43 315.60 641E+07 8897.5 4.40 12.01
12.12 316.29 704E+07 10113.2 4.55 11.61
12.80 316.97 770E+07 11420.1 4.70 11.24

```

```

<----- hydrograph -----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8008) 2478.14 14.27 7.50 13.53 1.15 0.86
OUTFLOW : ID= 1 (6112) 2478.14 12.19 8.75 13.53 1.10 0.84

```

```

-----
| CALIB |
| NASHYD (0112) | Area (ha)= 414.89 Curve Number (CN)= 30.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.91 |
-----
Unit Hyd Qpeak (cms)= 3.699
PEAK FLOW (cms)= 0.801 (i)
TIME TO PEAK (hrs)= 9.250
RUNOFF VOLUME (mm)= 6.991
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.096
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

```

-----
| ADD HYD (8010) |
| 1 + 2 = 3 |
-----
ID1= 1 (0112): 414.89 0.801 9.25 6.99
+ ID2= 2 (6112): 2478.14 12.188 8.75 13.53
=====

```

ID = 3 (8010): 2893.03 12.987 8.75 12.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6114) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)'= 15.00

```

```

<----- DATA FOR SECTION (1141.0) ----->
Distance Elevation Manning
0.00 312.27 0.0500
14.07 309.70 0.0500
28.14 306.51 0.0500
35.17 305.81 0.0500
42.20 304.95 0.0500
52.75 304.24 0.0500
77.37 303.61 0.0500
98.47 301.33 0.0500
131.76 300.40 0.0500 /0.0300 Main Channel
136.21 300.04 0.0300 Main Channel
136.41 299.97 0.0300 Main Channel
137.16 300.08 0.0300 Main Channel
137.26 300.48 0.0300 /0.0500 Main Channel
140.68 300.53 0.0500
179.36 302.20 0.0500
214.53 303.82 0.0500
242.66 305.27 0.0500
288.38 307.66 0.0500
316.52 311.76 0.0500
348.17 313.48 0.0500

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.43 300.40 366E+04 0.7 0.59 88.31
1.05 301.02 505E+05 12.1 0.75 69.49
1.68 301.65 165E+06 52.7 0.99 52.28
2.30 302.27 324E+06 130.3 1.26 41.40
2.93 302.90 520E+06 247.1 1.48 35.08
3.55 303.52 754E+06 406.8 1.68 30.91
4.18 304.15 104E+07 586.1 1.76 29.55
4.80 304.77 138E+07 856.0 1.93 26.96
5.43 305.40 177E+07 1201.1 2.12 24.56
6.05 306.02 219E+07 1612.2 2.30 22.63
6.68 306.65 264E+07 2090.6 2.47 21.07
7.30 307.27 313E+07 2653.0 2.64 19.65
7.93 307.90 364E+07 3303.9 2.83 18.36
8.55 308.52 417E+07 4053.9 3.03 17.14
9.18 309.15 471E+07 4871.9 3.22 16.12
9.80 309.77 527E+07 5756.2 3.41 15.25
10.43 310.40 584E+07 6699.8 3.58 14.52
11.05 311.02 642E+07 7710.5 3.74 13.89
11.68 311.65 702E+07 8788.5 3.90 13.32

```

```

<----- hydrograph -----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8010) 2893.03 12.99 8.75 12.59 1.07 0.75
OUTFLOW : ID= 1 (6114) 2893.03 11.07 10.00 12.59 1.00 0.73

```

```

-----
| CALIB |
| NASHYD (0114) | Area (ha)= 287.65 Curve Number (CN)= 49.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.12 |
-----
Unit Hyd Qpeak (cms)= 4.389
PEAK FLOW (cms)= 1.670 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 13.829
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.189

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8012) |
| 1 + 2 = 3 |
-----
ID1= 1 (0114): 287.65 1.670 7.75 13.83
+ ID2= 2 (6114): 2893.03 11.071 10.00 12.59
=====
ID = 3 (8012): 3180.68 12.416 9.75 12.70

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |

```

NASHYD (0116) Area (ha)= 206.43 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.80

Unit Hyd Qpeak (cms)= 4.394

PEAK FLOW (cms)= 2.488 (i)
 TIME TO PEAK (hrs)= 7.000
 RINOFF VOLUME (mm)= 21.671
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.296

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8014)
 1 + 2 = 3
 AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (0116): 206.43 2.488 7.00 21.67
 + ID2= 2 (8012): 3180.68 12.416 9.75 12.70
 ID = 3 (8014): 3387.11 13.907 9.50 13.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6118) Routing time step (min)'= 15.00
 IN= 2--> OUT= 1

Distance	Elevation	Manning
0.00	307.18	0.0450
24.89	299.75	0.0450
37.34	296.39	0.0450
71.57	292.75	0.0450
96.46	291.12	0.0450
108.90	289.43	0.0450
112.02	288.86	0.0450
115.13	288.46	0.0450
117.24	288.32	0.0450 / 0.0350 Main Channel
117.49	288.15	0.0350 Main Channel
118.24	288.00	0.0350 Main Channel
118.99	288.20	0.0350 Main Channel
119.24	288.36	0.0350 / 0.0450 Main Channel
121.35	288.40	0.0450
149.35	289.36	0.0450
171.14	290.47	0.0450
208.47	293.14	0.0450
236.48	295.17	0.0450
280.04	299.48	0.0450
308.04	307.00	0.0450

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.32	288.32	.116E+04	0.2	0.62	81.92
1.30	289.30	.631E+05	22.2	1.07	47.43
2.29	290.29	.218E+06	117.5	1.64	30.95
3.27	291.27	.488E+06	314.6	2.15	23.72
4.25	292.25	.756E+06	625.6	2.53	20.13
5.24	293.24	.115E+07	1093.2	2.91	17.50
6.22	294.22	.161E+07	1731.6	3.28	15.50
7.20	295.20	.214E+07	2547.4	3.63	14.01
8.19	296.19	.274E+07	3584.9	4.00	12.72
9.17	297.17	.338E+07	4859.1	4.39	11.60
10.15	298.15	.407E+07	6341.5	4.76	10.70
11.13	299.13	.480E+07	8024.6	5.11	9.97
12.12	300.12	.556E+07	10006.6	5.49	9.27
13.10	301.10	.635E+07	12244.6	5.89	8.64
14.08	302.08	.716E+07	14681.7	6.26	8.13
15.07	303.07	.799E+07	17316.5	6.62	7.69
16.05	304.05	.884E+07	20148.1	6.96	7.31
17.03	305.03	.971E+07	23176.2	7.29	6.98
18.02	306.02	.106E+08	26400.8	7.61	6.69

hydrograph --> <-pipe / channel-->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8014) 3387.11 13.91 9.50 13.25 0.93 0.84
 OUTFLOW: ID= 1 (6118) 3387.11 13.02 10.50 13.25 0.89 0.82

CALIB
 NASHYD (0118) Area (ha)= 340.53 Curve Number (CN)= 45.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.12

Unit Hyd Qpeak (cms)= 5.200

PEAK FLOW (cms)= 1.732 (i)

TIME TO PEAK (hrs)= 7.750
 RINOFF VOLUME (mm)= 12.145
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.166

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8016)
 1 + 2 = 3
 AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (0118): 340.53 1.732 7.75 12.15
 + ID2= 2 (6118): 3387.11 13.020 10.50 13.25
 ID = 3 (8016): 3727.64 14.262 10.50 13.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6122) Routing time step (min)'= 15.00
 IN= 2--> OUT= 1

Distance	Elevation	Manning
0.00	277.00	0.0500
52.18	272.82	0.0500
86.97	269.38	0.0500
134.80	267.35	0.0500
160.89	263.85	0.0500
169.58	262.88	0.0500
178.28	262.45	0.0500
195.67	262.07	0.0500
213.07	260.99	0.0500
226.01	259.80	0.0500 / 0.0300 Main Channel
226.11	259.30	0.0300 Main Channel
230.11	259.30	0.0300 Main Channel
230.21	259.80	0.0300 / 0.0500 Main Channel
235.01	260.20	0.0500
239.15	260.57	0.0500
243.50	260.93	0.0500
300.03	264.43	0.0500
339.16	268.34	0.0500
373.95	272.36	0.0500
430.48	277.46	0.0500

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	259.80	.626E+04	2.5	1.23	41.35
1.41	260.71	.462E+05	21.8	1.44	35.27
2.31	261.61	.149E+06	79.6	1.63	31.19
3.22	262.52	.340E+06	200.1	1.80	28.34
4.12	263.42	.636E+06	446.8	2.15	23.71
5.03	264.33	.995E+06	819.9	2.52	20.23
5.93	265.23	.141E+07	1337.0	2.90	17.56
6.84	266.14	.187E+07	1984.8	3.25	15.67
7.74	267.04	.237E+07	2766.0	3.57	14.26
8.65	267.95	.292E+07	3596.9	3.76	13.53
9.55	268.85	.355E+07	4575.4	3.93	12.95
10.46	269.76	.427E+07	5814.9	4.16	12.23
11.36	270.66	.503E+07	7319.0	4.44	11.46
12.27	271.57	.584E+07	9009.7	4.71	10.81
13.17	272.47	.670E+07	10886.3	4.96	10.26
14.08	273.38	.761E+07	12893.2	5.17	9.84
14.98	274.28	.858E+07	15098.1	5.37	9.48
15.89	275.19	.961E+07	17527.6	5.57	9.14
16.79	276.09	.107E+08	20190.3	5.76	8.83

hydrograph --> <-pipe / channel-->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8016) 3727.64 14.26 10.50 13.15 1.05 1.35
 OUTFLOW: ID= 1 (6122) 3727.64 13.91 11.25 13.15 1.03 1.35

CALIB
 STANDHYD (1221) Area (ha)= 17.70
 ID= 1 DT=15.0 min Total Imp(%)= 36.00 Dir. Conn.(%)= 19.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 6.37 11.33
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 343.51 40.00
 Mannings n = 0.013 0.250
 Max. Rff. Inten.(mm/hr)= 96.49 12.10
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 6.69 (ii) 31.59 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00

Unit Hyd. peak (cms)= 0.10 0.03 *TOTALS*
 PEAK FLOW (cms)= 0.84 0.23 0.931 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 71.10 10.92 22.27
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.15 0.30

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 35.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR (5122) |
 IN= 2--> OUT= 1 |
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.1840	0.4294
0.0600	0.1855	1.4870	0.5014
0.3430	0.2103	1.7310	0.5718
0.6390	0.2878	2.1310	0.5818
0.8860	0.3509	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
17.700	0.931	6.00	22.27
17.700	0.257	6.75	22.23

INFLOW : ID= 2 (1221)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	17.700	0.931	6.00	22.27

OUTFLOW: ID= 1 (5122)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	17.700	0.257	6.75	22.23

PEAK FLOW REDUCTION [Qout/Qin](%)	TIME SHIFT OF PEAK FLOW (min)	MAXIMUM STORAGE USED (ha.m.)
27.65	45.00	0.2033

 CALIB (1222) |
 NASHVD (1222) | Area (ha)= 347.20 Curve Number (CN)= 41.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.52

Unit Hyd Qpeak (cms)= 3.887
 PEAK FLOW (cms)= 1.222 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 10.637
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.146

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8018) |
1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
347.20	1.222	8.25	10.64
17.70	0.257	6.75	22.23

ID= 1 (1222):	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	347.20	1.222	8.25	10.64

+ ID2= 2 (5122):	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	17.70	0.257	6.75	22.23

ID = 3 (8018):	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	364.90	1.332	8.00	11.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8020) |
1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
3727.64	13.910	11.25	13.15
364.90	1.332	8.00	11.20

ID= 1 (6122):	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	3727.64	13.910	11.25	13.15

+ ID2= 2 (8018):	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	364.90	1.332	8.00	11.20

ID = 3 (8020):	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	4092.54	14.980	11.00	12.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB (1261) |
 STANDHYD (1261) | Area (ha)= 50.00
 ID= 1 DT=15.0 min | Total Imp(%)= 36.00 Dir. Conn.(%)= 26.00

IMPERVIOUS (i)	PERVIOUS (i)
18.00	32.00
2.00	5.00
0.50	0.50
577.35	40.00
0.013	0.250

Max.Eff.Inten.(mm/hr)= 96.49 16.03
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 9.14 (ii) 31.38 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.09 0.03 *TOTALS*
 PEAK FLOW (cms)= 3.02 0.87 3.373 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 71.10 15.42 29.90
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.21 0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR (5126) |
 IN= 2--> OUT= 1 |
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.3450	1.2129
0.1690	0.5421	4.2000	1.4164
0.9690	0.5941	4.8890	1.6153
1.8040	0.8130	5.2890	1.6253
2.5020	0.9912	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
50.000	3.373	6.00	29.90
50.000	1.170	6.75	29.88

INFLOW : ID= 2 (1261)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	50.000	3.373	6.00	29.90

OUTFLOW: ID= 1 (5126)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	50.000	1.170	6.75	29.88

PEAK FLOW REDUCTION [Qout/Qin](%)	TIME SHIFT OF PEAK FLOW (min)	MAXIMUM STORAGE USED (ha.m.)
34.68	45.00	0.6561

 CALIB (1262) |
 NASHVD (1262) | Area (ha)= 83.50 Curve Number (CN)= 58.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.56

Unit Hyd Qpeak (cms)= 2.523
 PEAK FLOW (cms)= 1.091 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 17.964
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.246

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8024) |
1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
83.50	1.091	6.50	17.96
50.00	1.170	6.75	29.88

ID= 1 (1262):	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	83.50	1.091	6.50	17.96

+ ID2= 2 (5126):	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	50.00	1.170	6.75	29.88

ID = 3 (8024):	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	133.50	2.253	6.75	22.43

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB (0124) |
 NASHVD (0124) | Area (ha)= 121.43 Curve Number (CN)= 43.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.51

Unit Hyd Qpeak (cms)= 4.049
 PEAK FLOW (cms)= 1.047 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 11.142
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.152

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8026) |
1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
121.43	1.047	6.50	11.14

ID= 1 (0124):	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	121.43	1.047	6.50	11.14

+ ID2= 2 (8024): 133.50 2.253 6.75 22.43
 =====
 ID = 3 (8026): 254.93 3.295 6.50 17.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6128) |
 IN= 2 ---> OUT= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (1281.0) -----

Distance	Elevation	Manning	
0.00	270.58	0.0400	
7.71	270.36	0.0400	
15.43	269.48	0.0400	
23.14	265.88	0.0400	
42.43	261.83	0.0400	
61.71	258.31	0.0400	
75.21	256.62	0.0400	
90.64	255.35	0.0400	
92.49	255.00	0.0400 / 0.0300	Main Channel
93.49	254.00	0.0300	Main Channel
94.49	253.90	0.0300	Main Channel
94.99	253.80	0.0300	Main Channel
95.49	253.70	0.0300	Main Channel
95.99	253.80	0.0300	Main Channel
98.49	255.00	0.0300 / 0.0400	Main Channel
100.28	255.44	0.0400	
134.99	257.45	0.0400	
148.49	259.67	0.0400	
167.77	263.42	0.0400	
190.92	270.00	0.0400	

----- TRAVEL TIME TABLE -----

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.65	254.35	.439E+04	2.0	1.24	36.10
1.30	255.00	.131E+05	9.4	1.92	23.39
2.18	255.88	.433E+05	37.0	2.30	19.54
3.06	256.76	.131E+06	113.3	2.34	19.20
3.95	257.65	.273E+06	273.8	2.70	16.64
4.83	258.53	.453E+06	530.5	3.21	14.01
5.71	259.41	.658E+06	902.9	3.70	12.14
6.59	260.29	.886E+06	1368.4	4.16	10.79
7.48	261.18	.114E+07	1936.3	4.59	9.79
8.36	262.06	.141E+07	2610.9	4.99	9.01
9.24	262.94	.171E+07	3398.7	5.37	8.36
10.12	263.82	.202E+07	4309.6	5.75	7.81
11.01	264.71	.235E+07	5347.5	6.13	7.33
11.89	265.59	.270E+07	6501.0	6.48	6.93
12.77	266.47	.307E+07	7808.6	6.86	6.55
13.65	267.35	.345E+07	9250.5	7.23	6.21
14.54	268.24	.384E+07	10800.3	7.59	5.92
15.42	269.12	.424E+07	12482.5	7.94	5.66
16.30	270.00	.466E+07	14127.4	8.18	5.49

----- hydrograph ----- <- pipe / channel ->

	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8026)	254.93	3.30	6.50	17.05	0.76	1.33
OUTFLOW : ID= 1 (6128)	254.93	2.77	7.25	17.05	0.72	1.29

CALIB
 NASHYD (0128) | Area (ha)= 240.24 Curve Number (CN)= 62.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp (hrs)= 1.00

Unit Hyd Qpeak (cms)= 4.118
 PEAK FLOW (cms)= 2.294 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 20.511
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.281

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8028) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0128): 240.24 2.294 7.25 20.51
 + ID2= 2 (6128): 254.93 2.775 7.25 17.05
 =====
 ID = 3 (8028): 495.17 5.069 7.25 18.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8022) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8020): 4092.84 14.980 11.00 12.97
 + ID2= 2 (8028): 495.17 5.069 7.25 18.73
 =====
 ID = 3 (8022): 4587.71 17.172 10.75 13.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1201) | Area (ha)= 86.00
 ID= 1 DT=15.0 min | Total Imp(%)= 36.00 Dir. Conn.(%)= 14.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 30.96 55.04
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 757.19 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 96.49 21.39
 Storage Coeff (min)= 15.00 45.00
 Storage Coeff (ii)= 10.75 (ii) 30.57 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. Tpeak (cms)= 0.09 0.03
 *****TOTALS*
 PEAK FLOW (cms)= 2.66 2.01 3.494 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 71.10 17.56 25.05
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.24 0.34

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5120) |
 IN= 2 ---> OUT= 1 |
 DT= 15.0 min |
 OUTFLOW STORAGE OUTFLOW STORAGE
 (cms) (ha.m.) (cms) (ha.m.)
 0.0000 0.0000 5.7530 2.0863
 0.2910 0.9015 7.2230 2.4361
 1.6660 1.0219 8.4090 2.7784
 3.1030 1.3984 8.8090 2.7884
 4.3040 1.7049 0.0000 0.0000

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (1201) 86.000 3.494 6.00 25.05
 OUTFLOW : ID= 1 (5120) 86.000 1.620 7.00 25.04

PEAK FLOW REDUCTION [Qout/Qin](%)= 46.35
 TIME SHIFT OF PEAK FLOW (min)= 60.00
 MAXIMUM STORAGE USED (ha.m.)= 1.0260

CALIB
 NASHYD (1202) | Area (ha)= 161.00 Curve Number (CN)= 58.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp (hrs)= 0.70

Unit Hyd Qpeak (cms)= 3.932
 PEAK FLOW (cms)= 1.784 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 18.082
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.247

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8030) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (1202): 161.00 1.784 6.75 18.08
 + ID2= 2 (5120): 86.00 1.620 7.00 25.04
 =====
 ID = 3 (8030): 247.00 3.397 7.00 20.51

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8032)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8022):	4587.71	17.172	10.75	13.60
+ ID2= 2 (8030):	247.00	3.397	7.00	20.51
ID = 3 (8032):	4834.71	18.167	10.50	13.95

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6130) |
IN= 2--> OUT= 1 | Routing time step (min)= 15.00

Distance	Elevation	Manning	Area (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
0.00	257.18	0.0350				
62.45	254.70	0.0350				
109.29	252.74	0.0350				
150.92	251.55	0.0350				
192.56	248.96	0.0350				
239.39	247.16	0.0350				
291.84	246.00	0.0350				
296.84	245.00	0.0350				
298.84	245.00	0.0350 / 0.0300				Main Channel
299.24	244.60	0.0300				Main Channel
301.84	244.60	0.0300				Main Channel
303.44	244.60	0.0300				Main Channel
303.84	245.00	0.0300 / 0.0350				Main Channel
306.84	245.00	0.0350				
309.84	246.00	0.0350				
317.46	247.22	0.0350				
348.68	254.83	0.0350				
359.09	256.70	0.0350				
452.76	258.23	0.0350				
515.22	258.33	0.0350				

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.40	245.00	.870E+04	1.2	0.64	124.12
1.04	245.64	.468E+05	9.5	0.96	81.81
1.68	246.28	.109E+06	25.6	1.11	70.86
2.32	246.92	.257E+06	63.5	1.17	67.51
2.96	247.56	.498E+06	142.7	1.36	58.18
3.61	248.21	.800E+06	266.7	1.58	50.01
4.25	248.85	.115E+07	437.4	1.78	44.24
4.89	249.49	.157E+07	659.1	2.01	39.20
5.53	250.13	.203E+07	954.9	2.23	35.36
6.17	250.77	.252E+07	1294.4	2.43	32.42
6.81	251.41	.305E+07	1689.6	2.62	30.07
7.45	252.05	.363E+07	2090.7	2.73	28.93
8.09	252.69	.429E+07	2553.1	2.82	27.98
8.73	253.33	.501E+07	3138.4	2.96	26.60
9.37	253.97	.579E+07	3803.8	3.11	25.36
10.02	254.62	.662E+07	4549.3	3.25	24.25
10.66	255.26	.751E+07	5364.9	3.38	23.32
11.30	255.90	.845E+07	6265.7	3.51	22.49
11.94	256.54	.946E+07	7260.1	3.63	21.72

INFLOW : ID= 2 (8032)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
4834.71	18.17	10.50	13.95	1.39	1.04	
OUTFLOW : ID= 1 (6130)	4834.71	17.13	11.75	13.95	1.34	1.03

CALIB	STANDHYD (1301)	Area (ha)	Total Imp(%)	Dir. Com.(%)
		228.30	65.00	47.00
Surface Area (ha)	IMPERVIOUS	148.39		
Dep. Storage (mm)	PERVIOUS (i)	79.91		
Average Slope (%)		0.50		
Length (m)		1233.69		
Mannings n		0.013		
Max. Eff. Inten. (mm/hr) over (min)		96.49 / 15.00		
Storage Coeff. (min)		14.41 (ii) / 25.63 (iii)		
Unit Hyd. Tpeak (min)		15.00 / 30.00		
Unit Hyd. peak (cms)		0.07 / 0.04		
PEAK FLOW (cms)		21.31 / 8.93		
TIME TO PEAK (hrs)		6.00 / 6.25		

RUNOFF VOLUME (mm)= 71.10 37.84 53.47
TOTAL RAINFALL (mm)= 73.10 73.10 73.10
RUNOFF COEFFICIENT = 0.97 0.52 0.73

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 74.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5130)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1 DT= 15.0 min	0.0000	0.0000	12.4560	7.0142
	0.8210	3.9074	14.9550	7.8554
	4.8550	4.4209	22.4790	8.9550
	7.0620	5.2214	22.8790	8.9650
	9.5970	6.0904	0.0000	0.0000
INFLOW : ID= 2 (1301)	228.300	27.086	6.00	53.47
OUTFLOW : ID= 1 (5130)	228.300	9.558	6.75	53.47
PEAK FLOW REDUCTION [Qout/Qin](%)	35.29			
TIME SHIFT OF PEAK FLOW (min)	45.00			
MAXIMUM STORAGE USED (ha.m.)	6.1509			

CALIB	NASHYD (1302)	Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res. (N)
		157.80	5.00	76.0	1.50
ID= 1 DT=15.0 min					
U.H. Tp(hrs)		1.82			
Unit Hyd Qpeak (cms)		1.479			
PEAK FLOW (cms)		1.453 (i)			
TIME TO PEAK (hrs)		8.500			
RUNOFF VOLUME (mm)		31.139			
TOTAL RAINFALL (mm)		73.100			
RUNOFF COEFFICIENT		0.426			
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.					

ADD HYD (8036)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (1302):	157.80	1.453	8.50	31.14
+ ID2= 2 (5130):	228.30	9.558	6.75	53.47
ID = 3 (8036):	386.10	10.705	6.75	44.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8034)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (6130):	4834.71	17.128	11.75	13.95
+ ID2= 2 (8036):	386.10	10.705	6.75	44.34
ID = 3 (8034):	5220.81	19.200	11.50	16.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	STANDHYD (6188)	Area (ha)	Total Imp(%)	Dir. Com.(%)
		308.53	41.00	25.00
Surface Area (ha)	IMPERVIOUS	126.50		
Dep. Storage (mm)	PERVIOUS (i)	182.03		
Average Slope (%)		3.00		
Length (m)		0.10		
Mannings n		1434.18		
Max. Eff. Inten. (mm/hr) over (min)		65.79 / 30.00		
Storage Coeff. (min)		29.79 (ii) / 59.03 (iii)		
Unit Hyd. Tpeak (min)		30.00 / 60.00		
Unit Hyd. peak (cms)		0.04 / 0.02		
PEAK FLOW (cms)		21.31 / 8.93		
TIME TO PEAK (hrs)		6.00 / 6.25		

PEAK FLOW (cms)= 9.28 5.52 12.324 (iii)
 TIME TO PEAK (hrs)= 6.25 6.75 6.25
 RUNOFF VOLUME (mm)= 70.10 23.00 34.77
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.96 0.31 0.48

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0186) | Area (ha)= 351.79
 ID= 1 DT=15.0 min | Total Imp(%)= 42.00 Dir. Conn.(%)= 22.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	147.75	204.04	
Dep. Storage (mm)=	3.00	5.00	
Average Slope (%)=	0.10	0.10	
Length (m)=	1531.43	40.00	
Manning's n	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	65.79	19.07	
over (min)	30.00	75.00	
Storage Coeff. (min)=	30.98 (ii)	64.62 (ii)	
Unit Hyd. Tpeak (min)=	30.00	75.00	
Unit Hyd. peak (cms)=	0.04	0.02	
			TOTALS
PEAK FLOW (cms)=	9.14	5.23	11.435 (iii)
TIME TO PEAK (hrs)=	6.25	7.00	6.25
RUNOFF VOLUME (mm)=	70.10	31.27	31.27
TOTAL RAINFALL (mm)=	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.96	0.28	0.43

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 53.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8068) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)

ID1= 1 (0186):	351.79	11.435	6.25	31.27
+ ID2= 2 (0188):	308.53	12.324	6.25	34.77
=====				
ID = 3 (8068):	660.32	23.759	6.25	32.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1821) | Area (ha)= 111.80
 ID= 1 DT=15.0 min | Total Imp(%)= 40.00 Dir. Conn.(%)= 17.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	44.72	67.08	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	863.33	40.00	
Manning's n	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	96.49	25.60	
over (min)	15.00	45.00	
Storage Coeff. (min)=	11.63 (ii)	30.08 (ii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.08	0.03	
			TOTALS
PEAK FLOW (cms)=	4.10	2.95	5.324 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	71.10	20.20	28.86
TOTAL RAINFALL (mm)=	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.28	0.39

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 52.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5182) |
 IIS 2--> OUTF 1 |
 DT= 15.0 min |

	OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha.m.)	(cms)	(ha.m.)	(ha.m.)
0.0000	0.0000	5.7800	2.5656	
0.3790	1.0207	7.3720	3.0829	
1.6240	1.1939	8.7710	3.5956	
3.0930	1.6631	9.1710	3.6056	
4.3530	2.1032	0.0000	0.0000	

	AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)	
INFLOW : ID= 2 (1821)	111.800	5.324	6.00	28.86
OUTFLOW: ID= 1 (5182)	111.800	2.297	7.00	28.85

PEAK FLOW REDUCTION [Qout/Qin](%)= 43.14
 TIME SHIFT OF PEAK FLOW (min)= 60.00
 MAXIMUM STORAGE USED (ha.m.)= 1.4169

CALIB
 NASHYD (1822) | Area (ha)= 44.40 Curve Number (CN)= 57.0
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.79 # of Linear Res. (N)= 1.50

Unit Hyd Qpeak (cms)= 0.966

PEAK FLOW (cms)= 0.437 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 17.596
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.241

- (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8062) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)

ID1= 1 (1822):	44.40	0.437	7.00	17.60
+ ID2= 2 (5182):	111.80	2.297	7.00	28.85
=====				
ID = 3 (8062):	156.20	2.734	7.00	25.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1841) | Area (ha)= 144.40
 ID= 1 DT=15.0 min | Total Imp(%)= 48.00 Dir. Conn.(%)= 19.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	69.31	75.09	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	981.16	40.00	
Manning's n	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	96.49	58.10	
over (min)	15.00	30.00	
Storage Coeff. (min)=	12.56 (ii)	25.85 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.08	0.04	
			TOTALS
PEAK FLOW (cms)=	5.75	5.41	9.137 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	71.10	24.67	33.49
TOTAL RAINFALL (mm)=	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.34	0.46

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 56.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5184) |
 IIS 2--> OUTF 1 |
 DT= 15.0 min |

	OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha.m.)	(cms)	(ha.m.)	(ha.m.)
0.0000	0.0000	5.8820	2.3298	
0.2960	1.0006	7.4070	2.7386	
1.6530	1.1680	8.6830	3.1418	
3.0860	1.4687	9.0830	3.1518	
4.3760	1.8752	0.0000	0.0000	

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1841)	144.400	9.137	6.00	33.49
OUTFLOW: ID= 1 (5184)	144.400	4.419	6.75	33.48

PEAK FLOW REDUCTION [Qout/Qin](%)= 48.37
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 1.9212

CALIB STANDHYD (1842)	Area (ha)	Imp(%)	Dir. Conn.(%)
ID= 1 DT=15.0 min	118.10	42.00	17.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	49.60	68.50
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	887.32	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	96.49	50.05
over (min)	15.00	30.00
Storage Coeff. (min)	11.82 (ii)	25.93 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04
TOTALS		
PEAK FLOW (cms)	4.30	4.24
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	71.10	31.32
TOTAL RAINFALL (mm)	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.32

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 56.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8060)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (1842):	118.10	6.940	6.00	31.32
+ ID2= 2 (5184):	144.40	4.419	6.75	33.48
=====				
ID = 3 (8060):	262.50	9.608	6.25	32.51

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8064)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8060):	262.50	9.608	6.25	32.51
+ ID2= 2 (8062):	156.20	2.734	7.00	25.65
=====				
ID = 3 (8064):	418.70	10.813	6.50	29.95

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1781)	Area (ha)	Imp(%)	Dir. Conn.(%)
ID= 1 DT=15.0 min	14.00	50.00	30.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	7.00	7.00
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	305.51	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	96.49	82.75
over (min)	15.00	30.00
Storage Coeff. (min)	6.24 (ii)	17.77 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.10	0.05
TOTALS		
PEAK FLOW (cms)	1.06	0.86
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	71.10	38.12
TOTAL RAINFALL (mm)	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.52

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 76.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5178)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IR= 2--> OUTF 1	0.0000	0.0000	0.9360	0.3780
DT= 15.0 min	0.0470	0.1841	1.1760	0.4277
	0.2710	0.2148	1.3690	0.4763
	0.5050	0.2704	1.7690	0.4863
	0.7010	0.3197	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1781)	14.000	1.624	6.00	48.01
OUTFLOW: ID= 1 (5178)	14.000	0.659	6.50	47.94

PEAK FLOW REDUCTION [Qout/Qin](%)= 40.60
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 0.3100

CALIB NASHYD (1782)	Area (ha)	Imp(%)	Curve Number (CN)	# of Linear Res. (N)
ID= 1 DT=15.0 min	261.40	5.00	78.0	1.50
			U.H. Tp(hrs)=	1.15

Unit Hyd Qpeak (cms) = 3.867

PEAK FLOW (cms) = 3.663 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 32.913
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.450

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8046)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (1782):	261.40	3.663	7.50	32.91
+ ID2= 2 (5178):	14.00	0.659	6.50	47.94
=====				
ID = 3 (8046):	275.40	4.111	7.00	33.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0180)	Area (ha)	Imp(%)	Curve Number (CN)	# of Linear Res. (N)
ID= 1 DT=15.0 min	226.65	5.00	65.0	1.50
			U.H. Tp(hrs)=	1.17

Unit Hyd Qpeak (cms) = 3.299

PEAK FLOW (cms) = 2.096 (i)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 22.455
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.307

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8050)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0180):	226.65	2.096	7.75	22.45
+ ID2= 2 (8046):	275.40	4.111	7.00	33.68
=====				
ID = 3 (8050):	502.05	6.156	7.25	28.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1741)	Area (ha)	Imp(%)	Dir. Conn.(%)
ID= 1 DT=15.0 min	56.30	35.00	18.00

```

-----
Surface Area (ha)= 19.70 36.60
Dep. Storage (mm)= 2.00 5.00
Average Slope (ft)= 0.50 0.50
Length (ft)= 612.64 40.00
Manning's n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 96.49 50.36
over (min) 15.00 30.00
Storage Coeff. (min)= 9.47 (ii) 23.54 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.09 0.04

*TOTALS*
PEAK FLOW (cms)= 2.33 2.40 3.833 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 71.10 26.21 34.29
TOTAL RAINFALL (mm)= 73.10 73.10 73.10
RUNOFF COEFFICIENT = 0.97 0.36 0.47

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
TO THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
RESERVOIR (5174)
IN= 2--> OUT= 1
DT= 15.0 min
-----
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
-----
0.0000 0.0000 | 1.1370 1.3070
0.0580 0.5427 | 1.4280 1.5354
0.3290 0.6196 | 1.6620 1.7593
0.6130 0.8627 | 2.0620 1.7693
0.8510 1.0621 | 0.0000 0.0000
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (1741) 56.300 3.833 6.00 34.29
OUTFLOW: ID= 1 (5174) 56.300 0.866 7.25 34.25
-----
PEAK FLOW REDUCTION [Qout/Qin](%)= 22.58
TIME SHIFT OF PEAK FLOW (min)= 75.00
MAXIMUM STORAGE USED (ha.m.)= 1.0796

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-----
CALIB (1742) | Area (ha)= 82.80 Curve Number (CN)= 71.0
NASHVD (1742) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.69
-----
Unit Hyd Qpeak (cms)= 2.055
PEAK FLOW (cms)= 1.397 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 26.508
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.363

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
ADD HYD (8044) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
-----
ID1= 1 (1742): 82.80 1.397 6.75 26.51
+ ID2= 2 (5174): 56.30 0.866 7.25 34.25
-----
ID = 3 (8044): 139.10 2.246 7.00 29.64

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
CALIB (0172) | Area (ha)= 202.23 Curve Number (CN)= 78.0
NASHVD (0172) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.89
-----
Unit Hyd Qpeak (cms)= 3.875
PEAK FLOW (cms)= 3.482 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 32.785
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.448

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB (0168) | Area (ha)= 247.13 Curve Number (CN)= 52.0
NASHVD (0168) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.75
-----
Unit Hyd Qpeak (cms)= 5.611
PEAK FLOW (cms)= 2.144 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 15.089
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.206

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB (0170) | Area (ha)= 166.91 Curve Number (CN)= 65.0
NASHVD (0170) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.80
-----
Unit Hyd Qpeak (cms)= 3.553
PEAK FLOW (cms)= 2.075 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 22.317
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.305

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
ADD HYD (8040) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
-----
ID1= 1 (0168): 247.13 2.144 7.00 15.09
+ ID2= 2 (0170): 166.91 2.075 7.00 22.32
-----
ID = 3 (8040): 414.04 4.218 7.00 18.00

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ADD HYD (8042) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
-----
ID1= 1 (0172): 202.23 3.482 7.00 32.79
+ ID2= 2 (8040): 414.04 4.218 7.00 18.00
-----
ID = 3 (8042): 616.27 7.700 7.00 22.85

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ROUTE CHN (6174) |
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
-----

```

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-----
<----- DATA FOR SECTION (1741.0) ----->
Distance Elevation Manning
0.00 296.00 0.0450
13.36 288.94 0.0450
26.71 288.64 0.0450
51.19 287.96 0.0450
73.45 287.32 0.0450
86.81 286.48 0.0450
102.39 285.08 0.0450
110.73 282.76 0.0450
115.73 281.06 0.0450 / 0.0350 Main Channel
115.74 280.35 0.0350 Main Channel
120.73 280.35 0.0350 Main Channel
120.74 281.10 0.0350 / 0.0450 Main Channel
125.74 282.64 0.0450
135.76 286.87 0.0450
149.13 289.58 0.0450
162.49 292.29 0.0450
175.84 293.95 0.0450
195.88 295.13 0.0450
213.68 296.02 0.0450
220.36 296.09 0.0450

```

```

-----
<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.71 281.06 .136E+05 6.0 1.69 37.89

```

1.50	281.85	.358E+05	22.5	2.42	26.43
2.28	282.63	.726E+05	53.8	2.85	22.47
3.07	283.42	.124E+06	104.1	3.24	19.79
3.86	284.21	.189E+06	176.7	3.60	17.81
4.64	284.99	.268E+06	274.9	3.94	16.26
5.43	285.78	.369E+06	390.8	4.07	15.73
6.21	286.56	.501E+06	551.6	4.23	15.15
7.00	287.35	.674E+06	750.5	4.28	14.97
7.79	288.14	.920E+06	993.3	4.15	15.44
8.57	288.92	.126E+07	1361.5	4.14	15.47
9.36	289.71	.167E+07	1986.0	4.56	14.04
10.15	290.50	.210E+07	2739.7	5.02	12.76
10.93	291.28	.254E+07	3615.0	5.47	11.70
11.72	292.07	.300E+07	4608.8	5.91	10.83
12.50	292.85	.347E+07	5693.3	6.30	10.16
13.29	293.64	.397E+07	6892.9	6.67	9.60
14.08	294.43	.450E+07	8150.4	6.97	9.19
14.86	295.21	.506E+07	9525.0	7.23	8.86

<---- hydrograph ----> <-pipe / channel-->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8042)	616.27	7.70	7.00	22.85	0.79	1.74
OUTFLOW: ID= 1 (6174)	616.27	7.05	7.75	22.85	0.76	1.72

ADD HYD (8048)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (6174):	616.27	7.054	7.75	22.85
+ ID2= 2 (8044):	139.10	2.246	7.00	29.64
=====				
ID = 3 (8048):	755.37	9.077	7.75	24.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8052)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8048):	755.37	9.077	7.75	24.10
+ ID2= 2 (8050):	502.05	6.156	7.25	28.61
=====				
ID = 3 (8052):	1257.42	15.164	7.50	25.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8058)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8052):	1257.42	15.164	7.50	25.90
+ ID2= 2 (8064):	418.70	10.813	6.50	29.95
=====				
ID = 3 (8058):	1676.12	23.384	6.75	26.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area (ha)	Imp(%)	Dir. Conn.(%)
STANBYD (1762)	162.00		
ID= 1 DT=15.0 min	162.00	26.00	11.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	42.12	119.88
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1039.23	40.00
Mannings n	0.013	0.250
Max.Eff.Inten.(mm/hr)=	96.49	24.46
over (min)	15.00	45.00
Storage Coeff. (min)=	13.00 (ii)	31.79 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03
		TOTALS
PEAK FLOW (cms)=	3.69	4.89
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	71.10	22.04
TOTAL RAINFALL (mm)=	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.30

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

CM* = 59.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Imp(%)	Dir. Conn.(%)
STANBYD (1761)	48.80		
ID= 1 DT=15.0 min	48.80	50.00	21.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	24.40	24.40
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	570.38	40.00
Mannings n	0.013	0.250
Max.Eff.Inten.(mm/hr)=	96.49	64.60
over (min)	15.00	30.00
Storage Coeff. (min)=	9.07 (ii)	21.81 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.05
		TOTALS
PEAK FLOW (cms)=	2.38	2.13
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	71.10	26.93
TOTAL RAINFALL (mm)=	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.37

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CM* = 59.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5176)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
ID= 2 --> OUT= 1				
DT= 15.0 min				
	0.0000	0.0000	3.2640	1.3175
	0.1650	0.6418	4.0090	1.4908
	0.9450	0.7489	4.7720	1.6604
	1.7610	0.9426	5.1720	1.6704
	2.4420	1.1144	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1761)	48.800	3.733	6.00	36.20
OUTFLOW: ID= 1 (5176)	48.800	1.394	6.75	36.18

PEAK FLOW REDUCTION [Qout/Qin](%)= 37.36
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 0.8625

ADD HYD (8056)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (1762):	162.00	5.848	6.50	27.43
+ ID2= 2 (5176):	48.80	1.394	6.75	36.18
=====				
ID = 3 (8056):	210.80	7.163	6.50	29.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8066)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8056):	210.80	7.163	6.50	29.46
+ ID2= 2 (8058):	1676.12	23.384	6.75	26.91
=====				
ID = 3 (8066):	1886.92	29.803	6.50	27.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8070)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8066):	1886.92	29.803	6.50	27.20
+ ID2= 2 (8068):	660.32	23.759	6.25	32.91
=====				
ID = 3 (8070):	2547.24	51.027	6.50	28.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6190) | Routing time step (min)'= 15.00
 IN= 2--> OUT= 1 |

DATA FOR SECTION (1901.0) ----->				
Distance	Elevation	Manning		
0.00	261.34	0.0600		
19.76	261.64	0.0600		
59.28	261.29	0.0600		
79.04	261.09	0.0600		
103.74	255.99	0.0600		
153.13	253.42	0.0600		
192.65	252.78	0.0600		
212.29	249.80	0.0600		
217.29	249.50	0.0600 /0.0350	Main Channel	
217.30	249.05	0.0350	Main Channel	
222.29	249.05	0.0350	Main Channel	
222.30	249.50	0.0350 /0.0600	Main Channel	
227.30	249.80	0.0600		
232.17	252.31	0.0600		
276.63	253.53	0.0600		
326.03	256.97	0.0600		
355.67	257.30	0.0600		
400.12	260.08	0.0600		
469.28	261.25	0.0600		
489.04	262.53	0.0600		

TRAVEL TIME TABLE ----->						
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)	
0.45	249.50	435E+04	1.0	0.43	75.97	
1.07	250.12	205E+05	5.4	0.51	62.96	
1.70	250.75	451E+05	14.8	0.64	50.78	
2.32	251.37	762E+05	29.2	0.74	43.46	
2.94	251.99	114E+06	49.1	0.84	38.61	
3.57	252.62	161E+06	70.2	0.85	38.18	
4.19	253.24	248E+06	93.4	0.73	44.29	
4.81	253.86	398E+06	154.9	0.76	42.79	
5.44	254.49	576E+06	245.1	0.83	38.99	
6.06	255.11	779E+06	362.7	0.90	35.79	
6.68	255.73	101E+07	506.5	0.98	33.15	
7.30	256.35	126E+07	687.6	1.06	30.52	
7.93	256.98	153E+07	902.8	1.15	28.18	
8.55	257.60	183E+07	1114.3	1.18	27.39	
9.17	258.22	216E+07	1407.8	1.26	25.55	
9.80	258.85	250E+07	1735.4	1.35	24.03	
10.42	259.47	286E+07	2097.5	1.42	22.73	
11.04	260.09	324E+07	2492.3	1.49	21.63	
11.67	260.72	364E+07	2837.8	1.51	21.39	

hydrograph ----->						
AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (8070) 2547.24	51.03	6.50	28.68	3.00	0.84	
OUTFLOW : ID= 1 (6190) 2547.24	40.27	7.00	28.68	2.67	0.79	

CALIB | STANDBYD (6190) | Area (ha)= 287.99
 | ID= 1 DT=15.0 min | Total Imp(%)= 51.00 Dir. Conn.(%)= 27.00

IMPERVIOUS PVIOUS (i)		
Surface Area (ha)=	146.87	141.12
Dep. Storage (mm)=	3.00	5.00
Average Slope (%)=	0.10	0.10
Length (m)=	1385.62	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	65.79	50.15
over (min)=	30.00	60.00
Storage Coeff. (min)=	29.18 (ii)	52.03 (ii)
Unit Hyd. Tpeak (min)=	30.00	60.00
Unit Hyd. peak (cms)=	0.04	0.02
PEAK FLOW (cms)=	9.45	8.44
TIME TO PEAK (hrs)=	6.25	6.75
RUNOFF VOLUME (mm)=	70.10	34.90
TOTAL RAINFALL (mm)=	73.10	73.10
RUNOFF COEFFICIENT =	0.96	0.48

(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
 CN* = 71.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8072) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID= 1 (0190): 287.99 14.281 6.25 44.40
 + ID2= 2 (6190): 2547.24 40.272 7.00 28.68
 ID = 3 (8072): 2835.23 52.447 6.75 30.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6192) | Routing time step (min)'= 15.00
 IN= 2--> OUT= 1 |

DATA FOR SECTION (1921.0) ----->				
Distance	Elevation	Manning		
0.00	264.40	0.0500		
3.64	263.94	0.0500		
14.54	261.05	0.0500		
29.08	256.78	0.0500		
36.36	254.96	0.0500		
50.90	252.20	0.0500		
65.44	249.78	0.0500		
76.35	246.86	0.0500		
77.24	245.90	0.0500		
82.24	245.45	0.0500 /0.0350	Main Channel	
82.25	244.90	0.0350	Main Channel	
87.25	244.90	0.0350	Main Channel	
87.26	245.45	0.0350 /0.0500	Main Channel	
94.83	246.89	0.0500		
170.87	247.16	0.0500		
218.13	248.69	0.0500		
265.40	249.22	0.0500		
301.75	250.78	0.0500		
338.11	253.47	0.0500		
359.92	264.00	0.0500		

TRAVEL TIME TABLE ----->						
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)	
0.55	245.45	345E+04	2.0	0.71	29.20	
1.53	246.43	174E+05	14.6	1.05	19.92	
2.50	247.40	767E+05	52.8	0.86	24.20	
3.48	248.38	224E+06	199.8	1.11	18.71	
4.46	249.36	439E+06	441.2	1.29	16.19	
5.43	250.33	691E+06	874.3	1.58	13.18	
6.41	251.31	987E+06	1468.3	1.86	11.20	
7.38	252.28	131E+07	2224.8	2.13	9.79	
8.36	253.26	165E+07	3132.4	2.37	8.78	
9.34	254.24	201E+07	4256.4	2.65	7.87	
10.31	255.21	238E+07	5551.0	2.92	7.15	
11.29	256.19	276E+07	6996.8	3.17	6.57	
12.27	257.17	314E+07	8585.9	3.42	6.10	
13.24	258.14	353E+07	10315.9	3.65	5.71	
14.22	259.12	393E+07	12180.7	3.87	5.38	
15.19	260.09	434E+07	14178.2	4.09	5.10	
16.17	261.07	475E+07	16306.5	4.29	4.85	
17.15	262.05	517E+07	18558.6	4.49	4.64	
18.12	263.02	559E+07	20939.7	4.68	4.45	

hydrograph ----->						
AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (8072) 2835.23	52.45	6.75	30.28	2.49	0.86	
OUTFLOW : ID= 1 (6192) 2835.23	46.40	7.25	30.28	2.34	0.89	

CALIB | STANDBYD (1921) | Area (ha)= 30.70
 | ID= 1 DT=15.0 min | Total Imp(%)= 48.00 Dir. Conn.(%)= 24.00

IMPERVIOUS PVIOUS (i)		
Surface Area (ha)=	14.74	15.96
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	452.40	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	96.49	49.07
over (min)=	15.00	30.00
Storage Coeff. (min)=	7.89 (ii)	22.11 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.10	0.04
PEAK FLOW (cms)=	1.78	1.05
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	71.10	22.30
TOTAL RAINFALL (mm)=	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.31

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 54.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5192)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	2.0560	0.7051
	0.1040	0.3429	2.5820	0.7960
	0.5960	0.4035	3.2900	0.8880
	1.1090	0.5056	3.4900	0.8980
	1.5380	0.5974	0.0000	0.0000
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (1921)	30.700	2.430	6.00	34.01
OUTFLOW: ID= 1 (5192)	30.700	0.942	6.50	33.98
	PEAK FLOW REDUCTION [Qout/Qin](%)=	38.79		
	TIME SHIFT OF PEAK FLOW	(min)= 30.00		
	MAXIMUM STORAGE USED	(ha.m.)= 0.4760		

CALIB (1922)			
STANDHYD (1922)			
ID= 1 DT=15.0 min			
	Area	(ha)	Total Imp(%)=
	IMPERVIOUS		Dir. Conn.(%)=
	PERVIOUS (i)		
	58.29	157.61	27.00
Surface Area (ha)=	58.29	157.61	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	1193.72	40.00	
Manning's n	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	96.49	20.15	
over (min)	15.00	45.00	
Storage Coeff. (min)=	14.17 (ii)	34.47 (ii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.07	0.03	
	TOTALS		
PEAK FLOW (cms)=	6.04	5.11	8.142 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	71.10	18.78	26.10
TOTAL RAINFALL (mm)=	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.26	0.36

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 54.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8074)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1922):	215.90	8.142	6.00	26.10
+ ID2= 2 (5192):	30.70	0.942	6.50	33.98
ID = 3 (8074):	246.60	8.230	6.00	27.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8076)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6192):	2835.23	46.404	7.25	30.28
+ ID2= 2 (8074):	246.60	8.230	6.00	27.08
ID = 3 (8076):	3081.83	50.054	7.25	30.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB				
NASHYD (0196)				
ID= 1 DT=15.0 min				
	Area	(ha)	Curve Number	(CN)=
	Ia	(mm)	# of Linear Res. (N)=	1.50
	U.H. Tp(hrs)=	0.24		

Unit Hyd Qpeak (cms)= 2.950

PEAK FLOW (cms)= 1.446 (i)
TIME TO PEAK (hrs)= 6.000
RUNOFF VOLUME (mm)= 24.056
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.329

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB				
NASHYD (0194)				
ID= 1 DT=15.0 min				
	Area	(ha)	Curve Number	(CN)=
	Ia	(mm)	# of Linear Res. (N)=	1.50
	U.H. Tp(hrs)=	0.68		

Unit Hyd Qpeak (cms)= 6.458

PEAK FLOW (cms)= 4.108 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 24.989
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.342

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8078)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0194):	256.09	4.108	6.75	24.99
+ ID2= 2 (0196):	41.47	1.446	6.00	24.06
ID = 3 (8078):	297.56	5.253	6.50	24.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6198)				
IN= 2--> OUT= 1				
Routing time step (min)= 15.00				

<----- DATA FOR SECTION (1981.0) ----->				
Distance	Elevation	Manning		
0.00	267.15	0.0500		
22.99	265.94	0.0500		
51.73	261.39	0.0500		
74.72	258.75	0.0500		
94.83	257.79	0.0500		
114.95	254.36	0.0500		
126.44	254.06	0.0500		
132.19	253.68	0.0500		
135.06	253.35	0.0500		
137.94	252.93	0.0500 / 0.0300	Main Channel	
140.81	252.41	0.0300	Main Channel	
143.69	251.89	0.0300	Main Channel	
146.56	252.51	0.0300	Main Channel	
149.43	253.36	0.0300 / 0.0500	Main Channel	
158.05	255.89	0.0500		
172.42	256.25	0.0500		
204.03	259.15	0.0500		
229.90	260.36	0.0500		
252.89	260.46	0.0500		
284.50	260.45	0.0500		

<----- TRAVEL TIME TABLE ----->						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.35	252.24	111E+04	0.6	0.97	31.07	
0.69	252.58	442E+04	3.8	1.55	19.42	
1.04	252.93	974E+04	11.1	2.06	14.65	
1.51	253.40	203E+05	31.2	2.77	10.87	
1.98	253.87	357E+05	65.0	3.30	9.14	
2.45	254.34	596E+05	112.9	3.43	8.80	
2.92	254.81	935E+05	186.8	3.62	8.34	
3.39	255.28	131E+06	282.1	3.89	7.75	
3.86	255.75	172E+06	398.2	4.18	7.22	
4.33	256.22	221E+06	527.0	4.31	6.99	
4.80	256.69	282E+06	697.3	4.47	6.75	
5.27	257.16	350E+06	898.5	4.64	6.50	
5.74	257.63	425E+06	1131.4	4.82	6.26	
6.21	258.10	508E+06	1367.4	4.88	6.19	
6.68	258.57	603E+06	1635.6	4.91	6.14	
7.15	259.04	709E+06	1977.2	5.04	5.98	
7.62	259.51	826E+06	2360.5	5.17	5.83	
8.09	259.98	954E+06	2790.7	5.29	5.70	
8.56	260.45	110E+07	3179.6	5.25	5.74	

<----- hydrograph -----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)

INFLOW : ID= 2 (8078) 297.56 5.25 6.50 24.86 0.76 1.63
 OUTFLOW: ID= 1 (6198) 297.56 5.05 6.75 24.86 0.75 1.62

CALIB
 STANDHYD (1981) | Area (ha)= 104.70 | Dir. Conn.(%)= 20.00
 ID= 1 DT=15.0 min | Total Imp(%)= 44.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	46.07	58.63
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	835.46	40.00
Mannings n	0.013	0.250
Max.Eff.Inten.(mm/hr)	96.49	52.80
over (min)	15.00	30.00
Storage Coeff. (min)	11.40 (ii)	25.21 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04
PEAK FLOW (cms)	4.54	3.89
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	71.10	24.42
TOTAL RAINFALL (mm)	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.33

TOTALS
 6.972 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 58.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5198)
 ID= 2 --> OUTF= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	7.9640	2.3173
0.2850	1.2270	9.4520	2.6039
1.7410	1.4052	10.9290	2.8853
3.5350	1.7675	11.3290	2.8953
5.6580	2.0183	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
104.700	6.972	6.00	33.76
104.700	2.903	6.75	33.75

PEAK FLOW REDUCTION [Qout/Qin](%)= 41.63
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 1.6531

CALIB
 NASHYD (1982) | Area (ha)= 285.90 | Curve Number (CN)= 65.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.81

Unit Hyd Opeak (cms) = 6.048

PEAK FLOW (cms) = 3.537 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 22.320
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.305

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8080)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (1982)	285.90	3.537	7.00	22.32
+ ID2= 2 (5198)	104.70	2.903	6.75	33.75
ID = 3 (8080)	390.60	6.394	6.75	25.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8082)
 1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
104.70	6.972	6.00	33.76
104.70	2.903	6.75	33.75

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6198)	297.56	5.049	6.75	24.86
+ ID2= 2 (8080)	390.60	6.394	6.75	25.38
ID = 3 (8082)	688.16	11.443	6.75	25.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8084)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8076)	3081.83	50.054	7.25	30.02
+ ID2= 2 (8082)	688.16	11.443	6.75	25.16
ID = 3 (8084)	3769.99	61.030	7.00	29.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN= (6200)
 IN= 2 --> OUF= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (2001.0) ----->

Distance	Elevation	Manning
0.00	252.47	0.0500
10.38	252.29	0.0500
20.76	252.28	0.0500
41.52	247.86	0.0500
62.28	247.47	0.0500
80.45	247.34	0.0500
90.83	246.85	0.0500
103.81	245.51	0.0500
106.00	244.50	0.0500
108.99	244.05	0.0500 / 0.0300
109.00	243.50	0.0300
114.00	243.50	0.0300
114.01	244.05	0.0300 / 0.0500
118.99	244.50	0.0500
132.36	245.09	0.0500
150.52	245.83	0.0500
199.83	249.24	0.0500
207.62	250.16	0.0500
236.16	253.11	0.0500
256.92	253.83	0.0500

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.28	243.77	.365E+04	0.7	0.54	81.42
0.55	244.05	.730E+04	2.2	0.81	54.59
1.05	244.55	.197E+05	7.2	0.97	45.51
1.54	245.04	.463E+05	17.2	0.99	44.76
2.04	245.54	.895E+05	34.8	1.03	42.81
2.53	246.03	.152E+06	62.8	1.09	40.43
3.03	246.53	.232E+06	104.7	1.20	36.91
3.52	247.02	.328E+06	159.7	1.29	34.19
4.02	247.52	.450E+06	223.4	1.32	33.56
4.51	248.01	.607E+06	326.9	1.38	31.99
5.01	248.51	.820E+06	463.0	1.50	29.52
5.50	249.00	.103E+07	626.0	1.62	27.30
6.00	249.50	.148E+07	820.1	1.75	25.24
6.49	249.99	.147E+07	1044.4	1.89	23.42
6.99	250.49	.170E+07	1292.6	2.01	21.95
7.48	250.98	.195E+07	1565.6	2.13	20.71
7.98	251.48	.220E+07	1864.5	2.25	19.65
8.47	251.97	.246E+07	2189.4	2.36	18.73
8.97	252.47	.274E+07	2427.5	2.35	18.80

hydrograph ----> <- pipe / channel ->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8084) 3769.99	61.03	7.00	29.13	2.50	1.09
OUTFLOW: ID= 1 (6200) 3769.99	53.16	8.00	29.13	2.36	1.07

CALIB
 STANDHYD (2001) | Area (ha)= 6.00 | Dir. Conn.(%)= 23.00
 ID= 1 DT=15.0 min | Total Imp(%)= 50.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	3.00	3.00
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	200.00	40.00
Mannings n	0.013	0.250
Max.Eff.Inten.(mm/hr)	96.49	76.31
over (min)	15.00	30.00
Storage Coeff. (min)	4.84 (ii)	16.75 (ii)

Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.11 0.05

*****TOTALS*
 PEAK FLOW (cms)= 0.36 0.35 0.584 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 71.10 32.30 41.22
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.44 0.56

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 67.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5200)		OUTFLOW		STORAGE	
IN= 2--> OUT= 1	DT= 15.0 min	(cms)	(ha.m.)	(cms)	(ha.m.)
		0.0000	0.0000	0.4010	0.1620
		0.0200	0.0789	0.5040	0.1833
		0.1160	0.0921	0.5870	0.2041
		0.2170	0.1159	0.9870	0.2141
		0.3000	0.1370	0.0000	0.0000

INFLOW : ID= 2 (2001)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
6.000	6.000	0.584	6.00	41.22
OUTFLOW: ID= 1 (5200)	6.000	0.221	6.50	41.06

PEAK FLOW REDUCTION [Qout/Qin](%)= 37.88
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 0.1185

CALIB STANDHYD (2002)		Area (ha)= 109.30	Total Imp(%)= 22.00	Dir. Com.(%)= 10.00
IMPERVIOUS PERVIOUS (i)				
Surface Area (ha)	24.05	85.25		
Dep. Storage (mm)	2.00	5.00		
Average Slope (%)	0.50	0.50		
Length (m)	853.62	40.00		
Manning's n	0.013	0.250		
Max. Eff. Inten. (mm/hr)	96.49	47.05		
over (min)	15.00	30.00		
Storage Coeff. (min)	11.55 (ii)	26.01 (ii)		
Unit Hyd. Tpeak (min)	15.00	30.00		
Unit Hyd. peak (cms)	0.08	0.04		

PEAK FLOW (cms)	2.36	4.96	*****TOTALS* 5.891 (iii)
TIME TO PEAK (hrs)	6.00	6.25	6.25
RUNOFF VOLUME (mm)	71.10	26.69	31.13
TOTAL RAINFALL (mm)	73.10	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.37	0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 67.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8086)	1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (2002):	109.30	5.891	6.25	31.13	
+ ID2= 2 (5200):	6.00	0.221	6.50	41.06	
ID= 3 (8086):	115.30	6.060	6.25	31.65	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8088)	1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6200):	3769.99	53.161	8.00	29.13	
+ ID2= 2 (8086):	115.30	6.060	6.25	31.65	

ID = 3 (8088): 3885.29 54.224 7.75 29.21
 NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8038)	1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8034):	5220.81	19.200	11.50	16.20	
+ ID2= 2 (8088):	3885.29	54.224	7.75	29.21	
ID= 3 (8038):	9106.10	70.690	8.00	21.75	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (1365) Routing time step (min)= 15.00

DATA FOR SECTION (1362.0)					
Distance	Elevation	Manning			
0.00	252.00	0.0400			
2.35	244.60	0.0400			
39.94	243.14	0.0400			
58.74	242.65	0.0400			
91.63	242.03	0.0400			
101.03	241.69	0.0400			
103.38	241.66	0.0400			
105.73	240.76	0.0400			
105.93	240.59	0.0400			
109.18	240.28	0.0400 / 0.0300	Main Channel		
110.43	239.81	0.0300	Main Channel		
111.68	239.81	0.0300	Main Channel		
113.43	240.09	0.0300 / 0.0400	Main Channel		
118.43	240.50	0.0400			
118.62	241.68	0.0400			
152.72	241.60	0.0400			
166.82	242.68	0.0400			
185.62	249.09	0.0400			
197.36	251.00	0.0400			
232.61	252.28	0.0400			

TRAVEL TIME TABLE						
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)	
0.28	240.09	.118E+04	0.1	0.20	142.09	
0.91	240.72	.114E+05	2.4	0.35	79.92	
1.53	241.34	.352E+05	9.4	0.45	62.17	
2.16	241.97	.831E+05	22.8	0.46	60.69	
2.79	242.60	.172E+06	54.2	0.53	52.74	
3.41	243.22	.297E+06	111.4	0.63	44.49	
4.04	243.85	.445E+06	194.1	0.74	38.22	
4.67	244.48	.612E+06	301.1	0.83	33.88	
5.29	245.10	.793E+06	443.0	0.94	29.82	
5.92	245.73	.976E+06	613.3	1.06	26.60	
6.55	246.36	.116E+07	802.0	1.17	24.12	
7.18	246.99	.135E+07	1014.0	1.27	22.16	
7.80	247.61	.154E+07	1246.2	1.37	20.56	
8.43	248.24	.173E+07	1498.0	1.46	19.24	
9.06	248.87	.192E+07	1768.8	1.55	18.12	
9.68	249.49	.212E+07	2049.6	1.63	17.24	
10.31	250.12	.232E+07	2344.7	1.71	16.49	
10.94	250.75	.252E+07	2658.5	1.78	15.83	
11.56	251.37	.274E+07	2931.2	1.81	15.56	

<---- hydrograph ----> <-pipe / channel->						
INFLOW : ID= 2 (8038)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
9106.10	70.69	8.00	21.75	2.97	0.56	
OUTFLOW: ID= 1 (1365)	9106.10	61.22	8.75	21.75	2.86	0.54

CALIB NASHVD (1322)	Area (ha)= 249.10	Curve Number (CN)= 73.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50
	U.H. Tp (hrs)= 0.83	

Unit Hyd Qpeak (cms)= 5.130
 PEAK FLOW (cms)= 3.873 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 28.234
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.386

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

STANDHYD (1321) | Area (ha)= 41.70
 ID= 1 DT=15.0 min | Total Imp(%)= 55.00 Dir. Conn.(%)= 35.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	22.94	18.77
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	527.26	40.00
Mannings n =	0.013	0.250
Max.Eff.Inten.(mm/hr)=	96.49	74.37
over (min)=	15.00	30.00
Storage Coeff (min)=	8.65 (ii)	20.69 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.05
	**TOTALS*	
PEAK FLOW (cms)=	3.44	1.94
TIME TO PEAK (hrs)=	6.00	6.25
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	71.10	33.44
TOTAL RAINFALL (mm)=	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.46

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5132)
 IN= 2--> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	0.9630	1.4050
	0.1780	1.0280	1.4870	1.5525
	0.4300	1.0400	1.9900	1.7000
	0.8150	1.0500	2.3900	1.7100
	0.8990	1.2275	0.0000	0.0000
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1321)	41.700	4.687	6.00	46.62
OUTFLOW: ID= 1 (5132)	41.700	0.874	7.00	46.59
	PEAK FLOW REDUCTION [Qout/ Qin](%)=	18.65		
	TIME SHIFT OF PEAK FLOW (min)=	60.00		
	MAXIMUM STORAGE USED (ha.m.)=	1.1770		

ADD HYD (8090)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (1322):	249.10	3.873	7.00	28.23
+ ID2= 2 (5132):	41.70	0.874	7.00	46.59
=====				
ID = 3 (8090):	290.80	4.747	7.00	30.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0134)
 ID= 1 DT=15.0 min

	Area (ha)=	155.95	Curve Number (CN)=	64.0
	Ia (mm)=	5.00	# of Linear Res.(N)=	1.50
	U.H. Tp(hrs)=	0.64		
Unit Hyd Qpeak (cms)=	4.167			
PEAK FLOW (cms)=	2.237 (i)			
TIME TO PEAK (hrs)=	6.750			
RUNOFF VOLUME (mm)=	21.547			
TOTAL RAINFALL (mm)=	73.100			
RUNOFF COEFFICIENT =	0.295			

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8092)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (0134):	155.95	2.237	6.75	21.55
+ ID2= 2 (8090):	290.80	4.747	7.00	30.87
=====				
ID = 3 (8092):	446.75	6.943	7.00	27.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (1366)
 IN= 2--> OUT= 1

Routing time step (min)'= 15.00

----- DATA FOR SECTION (1361.0) ----->

Distance	Elevation	Manning
0.00	262.81	0.0400
37.23	260.54	0.0400
79.12	256.98	0.0400
111.70	254.74	0.0400
172.20	252.76	0.0400
251.32	248.82	0.0400
255.97	248.75	0.0400
260.43	248.76	0.0400
265.43	248.49	0.0400
268.68	248.18	0.0400 / 0.0300
269.93	247.71	0.0300
271.18	247.71	0.0300
272.93	247.99	0.0300
277.93	248.40	0.0300 / 0.0400
283.90	248.88	0.0400
307.17	249.60	0.0400
321.13	251.59	0.0400
363.01	257.08	0.0400
400.25	257.58	0.0400
460.75	261.91	0.0400

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.47	248.18	.881E+04	1.3	0.76	114.66
1.19	248.90	.742E+05	17.3	1.22	71.44
1.92	249.63	.278E+06	78.3	1.48	59.26
2.64	250.35	.589E+06	210.3	1.87	46.69
3.36	251.07	.974E+06	414.3	2.23	39.19
4.08	251.79	.143E+07	696.2	2.55	34.32
4.81	252.52	.197E+07	1063.0	2.83	30.86
5.53	253.24	.259E+07	1505.7	3.06	28.62
6.25	253.96	1.199E+07	2051.3	3.26	26.85
6.97	254.68	.413E+07	2720.9	3.46	25.30
7.70	255.41	.504E+07	3584.5	3.73	23.43
8.42	256.13	.601E+07	4580.9	4.00	21.87
9.14	256.85	.704E+07	5707.9	4.25	20.57
9.86	257.57	.818E+07	6636.1	4.26	20.53
10.59	258.30	.945E+07	8115.0	4.50	19.42
11.31	259.02	.108E+08	9759.2	4.74	18.45
12.03	259.74	.132E+08	11569.5	4.96	17.61
12.75	260.46	.137E+08	13550.6	5.18	16.87
13.48	261.19	.153E+08	15636.4	5.37	16.29

----- hydrograph ----->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8092)	446.75	6.94	7.00	27.61	0.73	0.88
OUTFLOW: ID= 1 (1366)	446.75	4.76	8.50	27.61	0.63	0.83

CALIB
 STANDHYD (1362)
 ID= 1 DT=15.0 min

Area (ha)= 410.10
 Total Imp(%)= 21.00 Dir. Conn.(%)= 11.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	86.12	323.98
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1653.48	40.00
Mannings n =	0.013	0.250
Max.Eff.Inten.(mm/hr)=	96.49	26.08
over (min)=	15.00	45.00
Storage Coeff (min)=	17.17 (ii)	35.49 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.07	0.03

**TOTALS*

PEAK FLOW (cms)=	8.29	13.31
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	71.10	24.80
TOTAL RAINFALL (mm)=	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.34

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (1361)

Area (ha)= 102.30

|ID= 1 DT=15.0 min | Total Imp(%)= 55.00 Dir. Conn.(%)= 27.00

Surface Area (ha)= 56.27 46.03
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 825.83 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 96.49 78.92
over (min) 15.00 30.00
Storage Coeff. (min)= 11.32 (ii) 23.08 (iii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04
PEAK FLOW (cms)= 6.01 4.80
TIME TO PEAK (hrs)= 6.00 6.25
RUNOFF VOLUME (mm)= 71.10 31.77
TOTAL RAINFALL (mm)= 73.10 73.10
RUNOFF COEFFICIENT = 0.97 0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 65.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5136)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
0.0000 0.0000 | 2.4000 3.2690
0.1600 1.6363 | 2.6450 3.8640
0.9600 1.7470 | 2.8900 4.4590
1.8400 1.9320 | 3.2900 4.4590
2.1200 2.6005 | 0.0000 0.0000

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (1361) 102.300 9.073 6.00 42.39
OUTFLOW: ID= 1 (5136) 102.300 2.051 7.00 42.36

PEAK FLOW REDUCTION [Qout/Qin](%)= 22.61
TIME SHIFT OF PEAK FLOW (min)= 60.00
MAXIMUM STORAGE USED (ha.m.)= 2.4388

ADD HYD (8096)
1 + 2 = 3

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (1362): 410.10 16.114 6.50 29.90
+ ID2= 2 (5136): 102.30 2.051 7.00 42.36
ID = 3 (8096): 512.40 18.075 6.50 32.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8094)
1 + 2 = 3

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (1366): 446.75 4.761 8.50 27.61
+ ID2= 2 (8096): 512.40 18.075 6.50 32.39
ID = 3 (8094): 959.15 19.838 6.50 30.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8098)
1 + 2 = 3

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (1365): 9106.10 61.215 8.75 21.75
+ ID2= 2 (8094): 959.15 19.838 6.50 30.16
ID = 3 (8098): 10065.25 70.815 8.50 22.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHVD (0142) Area (ha)= 396.54 Curve Number (CN)= 63.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.32

Unit Hyd Qpeak (cms)= 5.133

PEAK FLOW (cms)= 3.169 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 21.200
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.290

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHVD (0138) Area (ha)= 414.79 Curve Number (CN)= 24.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.39

Unit Hyd Qpeak (cms)= 5.087

PEAK FLOW (cms)= 0.772 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 5.283
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.072

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (0140) Area (ha)= 158.58
ID= 1 DT=15.0 min Total Imp(%)= 24.00 Dir. Conn.(%)= 21.00

Surface Area (ha)= 38.06 120.52
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1028.20 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 96.49 10.19
over (min) 15.00 45.00
Storage Coeff. (min)= 12.92 (ii) 39.58 (iii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.08 0.03

PEAK FLOW (cms)= 6.91 1.85 7.642 (iii)
TIME TO PEAK (hrs)= 6.00 6.50 6.00
RUNOFF VOLUME (mm)= 71.10 11.11 23.70
TOTAL RAINFALL (mm)= 73.10 73.10 73.10
RUNOFF COEFFICIENT = 0.97 0.15 0.32

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 41.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8102)
1 + 2 = 3

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0138): 414.79 0.772 8.00 5.28
+ ID2= 2 (0140): 158.58 7.642 6.00 23.70
ID = 3 (8102): 573.37 7.991 6.00 10.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6142)
IN= 2--> OUT= 1

Routing time step (min)'= 15.00

Distance Elevation Manning
0.00 295.50 0.0400
7.25 295.36 0.0400
14.50 295.22 0.0400
21.75 295.22 0.0400
29.00 294.23 0.0400
52.56 286.45 0.0400
67.96 281.44 0.0400
77.94 278.28 0.0400
82.81 277.20 0.0400 /0.0300 Main Channel
83.21 276.80 0.0300 Main Channel
88.81 276.80 0.0300 Main Channel
96.41 276.80 0.0300 Main Channel
96.81 277.20 0.0300 /0.0400 Main Channel
103.31 278.96 0.0400

108.75	280.16	0.0400
117.81	282.73	0.0400
135.94	290.30	0.0400
159.50	292.14	0.0400
174.00	293.07	0.0400
179.44	293.37	0.0400

236.44	250.20	0.0300	Main Channel
236.84	249.80	0.0300	Main Channel
242.44	249.80	0.0300	Main Channel
250.04	249.80	0.0300	Main Channel
250.44	250.20	0.0300	Main Channel
252.33	250.57	0.0300 / 0.0350	Main Channel
286.97	253.52	0.0350	
331.50	255.80	0.0350	
380.98	255.55	0.0350	
445.30	258.40	0.0350	
455.19	259.82	0.0350	
489.83	262.22	0.0350	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.40	277.20	.238E+05	6.9	1.27	57.28
1.25	278.05	.889E+05	50.9	2.50	29.13
2.10	278.90	.179E+06	133.4	3.26	22.36
2.95	279.75	.293E+06	256.7	3.83	19.03
3.80	280.60	.432E+06	426.0	4.31	16.90
4.66	281.46	.593E+06	644.3	4.75	15.35
5.51	282.31	.776E+06	915.3	5.16	14.12
6.36	283.16	.978E+06	1242.2	5.55	13.12
7.21	284.01	.120E+07	1626.2	5.94	12.27
8.06	284.86	.143E+07	2065.6	6.30	11.57
8.91	285.71	.169E+07	2562.4	6.64	10.97
9.76	286.56	.196E+07	3118.0	6.97	10.46
10.61	287.41	.224E+07	3733.3	7.28	10.02
11.46	288.26	.255E+07	4411.9	7.57	9.62
12.31	289.11	.287E+07	5155.6	7.86	9.27
13.17	289.97	.321E+07	5966.6	8.14	8.96
14.02	290.82	.357E+07	6705.0	8.22	8.87
14.87	291.67	.398E+07	7494.3	8.24	8.85
15.72	292.52	.444E+07	8405.3	8.28	8.80

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.39	250.19	.169E+05	5.4	1.04	52.00
0.77	250.57	.373E+05	16.6	1.44	37.46
1.44	251.24	.913E+05	60.3	2.14	25.19
2.11	251.91	.169E+06	136.3	2.61	20.64
2.79	252.59	.270E+06	249.7	2.99	18.05
3.46	253.26	.396E+06	405.4	3.31	16.28
4.13	253.93	.549E+06	603.3	3.56	15.15
4.80	254.60	.737E+06	857.8	3.77	14.31
5.48	255.28	.963E+06	1174.6	3.95	13.67
6.15	255.95	.128E+07	1449.5	3.66	14.74
6.82	256.62	.173E+07	2046.4	3.83	14.07
7.49	257.29	.227E+07	2665.2	3.81	14.17
8.17	257.97	.310E+07	3705.2	3.87	13.93
8.84	258.64	.400E+07	5127.4	4.15	12.99
9.51	259.31	.492E+07	6858.6	4.51	11.95
10.18	259.98	.587E+07	8815.5	4.86	11.09
10.86	260.66	.684E+07	10954.6	5.18	10.41
11.53	261.33	.785E+07	13324.3	5.49	9.82
12.20	262.00	.890E+07	15923.2	5.79	9.31

----- hydrograph ----- <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (8102)	573.37	7.99	6.00	10.38	0.42	1.29
OUTFLOW : ID= 1 (6142)	573.37	3.38	6.75	10.38	0.20	1.27

----- hydrograph ----- <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (8106)	1105.60	8.63	7.00	16.54	0.50	1.13
OUTFLOW : ID= 1 (6146)	1105.60	7.53	8.00	16.54	0.46	1.10

----- ADD HYD (8104) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0142):	396.54	3.169	8.00	21.20
+ ID2= 2 (6142):	573.37	3.379	6.75	10.38
ID = 3 (8104):	969.91	6.204	7.00	14.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- CALIB -----

NASHYD (0144)	Area (ha)	Curve Number (CN)	U.H. Tp (hrs)
ID= 1 DT=15.0 min	135.69	74.0	0.71

Unit Hyd Qpeak (cms) = 3.240
 PEAK FLOW (cms) = 2.441 (i)
 TIME TO PEAK (hrs) = 6.750
 RINFLOW VOLUME (mm) = 28.940
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.396

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- ADD HYD (8106) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0144):	135.69	2.441	6.75	28.98
+ ID2= 2 (8104):	969.91	6.204	7.00	14.80
ID = 3 (8106):	1105.60	8.629	7.00	16.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ROUTE CHN (6146) -----

Routing time step (min) = 15.00

----- DATA FOR SECTION (1461.0) -----

Distance	Elevation	Manning
0.00	252.00	0.0350
39.58	257.13	0.0350
123.69	257.51	0.0350
183.07	256.83	0.0350
202.86	255.73	0.0350
212.75	254.73	0.0350
222.65	252.86	0.0350
232.44	250.70	0.0350 / 0.0300

----- CALIB -----

STANDHYD (1462)	Area (ha)	Dir. Conn. (%)
ID= 1 DT=15.0 min	125.00	12.00

Surface Area (ha) = 36.25
 Dep. Storage (mm) = 2.00
 Average Slope (ft) = 0.50
 Length (m) = 912.87
 Mannings n = 0.013
 Max. Eff. Inten. (mm/hr) = 96.49
 over (min) = 15.00
 Storage Coeff. (min) = 12.03 (ii)
 Unit Hyd. Tpeak (min) = 15.00
 Unit Hyd. peak (cms) = 0.08
 PEAK FLOW (cms) = 3.19
 TIME TO PEAK (hrs) = 6.00
 RINFLOW VOLUME (mm) = 71.10
 TOTAL RAINFALL (mm) = 73.10
 RUNOFF COEFFICIENT = 0.97
 IMPERVIOUS PERVIOUS (i)
 88.75 5.00
 0.50 0.50
 40.00 0.250
 63.03 24.89 (ii)
 30.00 0.04
 TOTALS
 8.419 (iii)
 6.25 37.51
 73.10 73.10
 0.45 0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 73.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB -----

STANDHYD (1461)	Area (ha)	Dir. Conn. (%)
ID= 1 DT=15.0 min	70.50	15.00

Surface Area (ha) = 25.38
 Dep. Storage (mm) = 2.00
 Average Slope (ft) = 0.50
 Length (m) = 685.57
 Mannings n = 0.013
 Max. Eff. Inten. (mm/hr) = 96.49
 over (min) = 15.00
 Storage Coeff. (min) = 10.13 (ii)
 Unit Hyd. Tpeak (min) = 15.00
 IMPERVIOUS PERVIOUS (i)
 45.12 5.00
 0.50 0.50
 40.00 0.250
 70.42 30.00
 22.44 (ii) 30.00

Unit Hyd. peak (cms)= 0.09 0.04 *TOTALS*
 PEAK FLOW (cms)= 2.38 4.25 5.118 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 71.10 34.32 39.84
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.47 0.54

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 73.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5146)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	8.0640	2.2344
	0.2390	0.7085	9.9830	2.6409
	2.2880	0.8391	11.7350	3.0631
	4.3270	1.4661	12.1350	3.0731
	6.1170	1.8197	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (1461)	70.500	5.118	6.00	39.84
OUTFLOW: ID= 1 (5146)	70.500	2.974	6.50	39.83

PEAK FLOW REDUCTION [Qout/Qin](%)	TIME SHIFT OF PEAK FLOW (min)	MAXIMUM STORAGE USED (ha.m.)
58.10	30.00	1.0679

ADD HYD (8099)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1462):	125.00	8.419	6.25	37.51
+ ID2= 2 (5146):	70.50	2.974	6.50	39.83
=====				
ID = 3 (8099):	195.50	10.960	6.25	38.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8108)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6146):	1105.60	7.529	8.00	16.54
+ ID2= 2 (8099):	195.50	10.960	6.25	38.35
=====				
ID = 3 (8108):	1301.10	13.629	6.25	19.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8100)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8098):	10065.25	70.815	8.50	22.55
+ ID2= 2 (8108):	1301.10	13.629	6.25	19.82
=====				
ID = 3 (8100):	11366.35	79.640	8.25	22.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6202)
 IN= 2--> OUT= 1
 Routing time step (min)= 15.00

<----- DATA FOR SECTION (2021.0) ----->			
Distance	Elevation	Manning	
0.00	245.48	0.0400	
29.68	242.80	0.0400	
74.20	240.39	0.0400	
155.81	237.12	0.0400	
200.33	234.48	0.0400	
244.85	234.67	0.0400	
259.69	233.96	0.0400	
274.53	233.84	0.0400	
281.68	233.40	0.0400	
285.78	232.40	0.0400 / 0.0300	Main Channel

286.78	231.40	0.0300	Main Channel
296.78	231.40	0.0300	Main Channel
299.78	232.40	0.0300 / 0.0400	Main Channel
311.62	233.74	0.0400	
333.88	234.34	0.0400	
497.11	235.32	0.0400	
571.31	237.32	0.0400	
675.18	239.10	0.0400	
704.86	242.71	0.0400	
734.54	242.62	0.0400	

<----- TRAVEL TIME TABLE ----->						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.50	231.90	.221E+05	4.7	0.85	78.67	
1.00	232.40	.482E+05	15.3	1.28	52.40	
1.50	233.00	.913E+05	38.3	1.68	39.77	
2.20	233.60	.154E+06	72.9	1.90	35.24	
2.80	234.20	.278E+06	124.2	1.79	37.30	
3.40	234.80	.599E+06	214.3	1.44	46.59	
4.01	235.41	.125E+07	445.0	1.43	46.90	
4.61	236.01	.205E+07	838.4	1.64	40.81	
5.21	236.61	.293E+07	1361.6	1.86	35.89	
5.81	237.21	.389E+07	2011.2	2.08	32.23	
6.41	237.81	.494E+07	2753.5	2.24	29.91	
7.01	238.41	.612E+07	3635.0	2.39	28.04	
7.61	239.01	.741E+07	4672.5	2.53	26.43	
8.21	239.61	.880E+07	5999.3	2.74	24.48	
8.82	240.22	.102E+08	7510.0	2.94	22.72	
9.42	240.82	.117E+08	9201.9	3.15	21.23	
10.02	241.42	.132E+08	11059.7	3.35	19.95	
10.62	242.02	.148E+08	13071.4	3.54	18.87	
11.22	242.62	.164E+08	15242.1	3.73	17.93	

<---- hydrograph ---->							<-pipe / channel->		
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL			
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)			
INFLOW : ID= 2 (8100) *****	79.64	8.25	22.24	2.28	2.28	1.88			
OUTFLOW: ID= 1 (6202) *****	77.03	9.00	22.24	2.25	2.25	1.89			

ADD HYD (8112)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6202):	11366.35	77.028	9.00	22.24
+ ID2= 2 (8110):	468.50	34.598	6.00	46.59
=====				
ID = 3 (8112):	11834.85	80.595	8.75	23.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5555)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	*****	19.2100
	2.8500	0.5200	*****	24.7900
	8.1800	1.2600	*****	30.8600
	15.0900	2.2100	*****	37.6100
	23.2700	3.3900	*****	41.2200
	32.5300	4.7800	*****	48.9000
	36.5100	5.3900	*****	66.4700
	42.7600	6.3700	*****	100.1200
	53.8500	8.1300	*****	140.4100
	91.7300	14.3200	*****	140.4200

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8112) *****	80.595	8.75	23.20	
OUTFLOW: ID= 1 (5555) *****	79.480	9.25	23.20	

PEAK FLOW REDUCTION [Qout/Qin](%)	TIME SHIFT OF PEAK FLOW (min)	MAXIMUM STORAGE USED (ha.m.)
98.62	30.00	12.3389

ADD HYD (8114)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (5555):	11834.85	79.480	9.25	23.20
+ ID2= 2 (8136):	2286.40	24.489	6.50	22.52
=====				
ID = 3 (8114):	14123.25	94.228	9.00	23.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (2061) Area (ha)= 25.00
ID= 1 DT=15.0 min Total Imp(%)= 93.00 Dir. Conn.(%)= 60.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	23.25	1.75	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (°)=	0.50	0.50	
Length (m)=	408.25	40.00	
Manning's n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	96.49	506.03	
over (min)=	15.00	15.00	
Storage Coeff. (min)=	7.42 (ii)	13.01 (ii)	
Unit Hyd. Tpeak (min)=	15.00	15.00	
Unit Hyd. peak (cms)=	0.10	0.08	
TOTALS			
PEAK FLOW (cms)=	3.66	1.88	5.539 (iii)
TIME TO PEAK (hrs)=	6.00	6.00	6.00
RUNOFF VOLUME (mm)=	71.10	59.93	66.63
TOTAL RAINFALL (mm)=	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.82	0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5206) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.8990	1.7295
0.0850	0.5280	2.1560	1.9661
0.8990	0.8577	2.3740	2.2023
1.3460	1.1601	2.7740	2.2123
1.6060	1.4451	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (2061)	25.000	5.539	6.00	66.63
OUTFLOW: ID= 1 (5206)	25.000	1.094	6.50	66.57

PEAK FLOW REDUCTION [Qout/Qin](%)= 19.75
TIME SHIFT OF PEAK FLOW (min)= 30.00
MAXIMUM STORAGE USED (ha.m.)= 0.9902

CALIB
STANDHYD (2062) Area (ha)= 250.50
ID= 1 DT=15.0 min Total Imp(%)= 62.00 Dir. Conn.(%)= 39.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	155.31	95.19	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (°)=	0.50	0.50	
Length (m)=	1292.28	40.00	
Manning's n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	96.49	99.14	
over (min)=	15.00	30.00	
Storage Coeff. (min)=	14.81 (ii)	25.55 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.07	0.04	
TOTALS			
PEAK FLOW (cms)=	19.17	11.95	26.964 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	71.10	39.91	52.07
TOTAL RAINFALL (mm)=	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.55	0.71

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8144) |
| 1 + 2 = 3 |

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (2062):	250.50	26.964	6.00	52.07
+ ID2= 2 (5206):	25.00	1.094	6.50	66.57
=====				
ID = 3 (8144):	275.50	27.462	6.00	53.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (2041) Area (ha)= 201.50
ID= 1 DT=15.0 min Total Imp(%)= 36.00 Dir. Conn.(%)= 20.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	72.54	128.96	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (°)=	0.50	0.50	
Length (m)=	1159.02	40.00	
Manning's n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	96.49	45.46	
over (min)=	15.00	30.00	
Storage Coeff. (min)=	13.88 (ii)	28.54 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.07	0.04	
TOTALS			
PEAK FLOW (cms)=	8.13	6.89	12.393 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	71.10	23.99	33.41
TOTAL RAINFALL (mm)=	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.33	0.46

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5204) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	12.4660	4.9468
0.5050	1.9141	15.4770	5.3728
2.8390	2.2056	18.2610	6.4806
5.2410	3.6606	18.6610	6.4906
7.9920	4.3236	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (2041)	201.500	12.393	6.00	33.41
OUTFLOW: ID= 1 (5204)	201.500	4.334	7.00	33.41

PEAK FLOW REDUCTION [Qout/Qin](%)= 34.97
TIME SHIFT OF PEAK FLOW (min)= 60.00
MAXIMUM STORAGE USED (ha.m.)= 3.1377

CALIB
STANDHYD (2042) Area (ha)= 122.90
ID= 1 DT=15.0 min Total Imp(%)= 60.00 Dir. Conn.(%)= 33.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	73.74	49.16	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (°)=	0.50	0.50	
Length (m)=	905.17	40.00	
Manning's n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	96.49	75.13	
over (min)=	15.00	30.00	
Storage Coeff. (min)=	11.96 (ii)	23.96 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.08	0.04	
TOTALS			
PEAK FLOW (cms)=	8.65	4.78	11.688 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	71.10	29.43	43.18
TOTAL RAINFALL (mm)=	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.40	0.59

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8140)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (2042):	122.90	11.688	6.00	43.18
+ ID2= 2 (5204):	201.50	4.334	7.00	33.41
ID = 3 (8140):	324.40	12.093	6.00	37.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6206) | Routing time step (min)'= 15.00

Distance	Elevation	Manning
0.00	253.07	0.0350
13.98	252.89	0.0350
27.95	251.99	0.0350
34.94	252.18	0.0350
41.93	252.02	0.0350
73.38	250.35	0.0350
94.34	247.09	0.0350
111.81	244.79	0.0350
115.31	244.24	0.0350
117.55	244.20	0.0350 / 0.0300
118.80	243.60	0.0300
122.30	243.60	0.0300
122.55	244.60	0.0300 / 0.0350
146.76	244.74	0.0350
164.23	247.26	0.0350
213.15	248.92	0.0350
241.10	249.51	0.0350
272.55	250.87	0.0350
300.50	252.26	0.0350
345.93	253.67	0.0350

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.30	243.90	.310E+04	1.4	1.23	36.25
0.60	241.20	.676E+04	4.6	1.81	24.64
1.12	244.72	.221E+05	17.1	2.07	21.60
1.64	245.24	.754E+05	64.1	2.28	19.60
2.17	245.77	.139E+06	149.8	2.88	15.50
2.69	246.29	.214E+06	273.3	3.43	13.04
3.21	246.81	.299E+06	436.1	3.91	11.43
3.73	247.33	.395E+06	632.6	4.30	10.40
4.25	247.85	.511E+06	836.4	4.39	10.18
4.77	248.37	.653E+06	1114.9	4.58	9.76
5.30	248.90	.822E+06	1472.1	4.81	9.30
5.82	249.42	.102E+07	1872.2	4.91	9.10
6.34	249.94	.126E+07	2444.6	5.22	8.56
6.86	250.46	.151E+07	3113.1	5.52	8.09
7.38	250.98	.180E+07	3843.5	5.74	7.79
7.90	251.50	.211E+07	4717.8	6.00	7.45
8.43	252.03	.245E+07	5703.4	6.24	7.16
8.95	252.55	.284E+07	6727.5	6.36	7.03
9.47	253.07	.326E+07	7885.3	6.48	6.90

hydrograph <-> pipe / channel

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8140)	324.40	12.09	6.00	37.11	0.91
OUTFLOW: ID= 1 (6206)	324.40	9.60	6.50	37.11	0.81

ADD HYD (8142)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6206):	324.40	9.600	6.50	37.11
+ ID2= 2 (8144):	275.50	27.462	6.00	53.39
ID = 3 (8142):	599.90	32.262	6.00	44.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8138)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8114):	14123.25	94.228	9.00	23.09
+ ID2= 2 (8142):	599.90	32.262	6.00	44.59
ID = 3 (8138):	14723.15	99.850	8.50	23.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6208) | Routing time step (min)'= 15.00

Distance	Elevation	Manning
0.00	249.19	0.1000
73.11	244.13	0.1000
121.86	243.39	0.1000
243.71	240.01	0.1000
353.38	238.75	0.1000
450.87	235.40	0.1000
499.61	231.40	0.1000
511.80	230.96	0.1000
523.98	230.12	0.1000
534.50	227.80	0.1000 / 0.0500
536.50	226.70	0.0500
546.50	226.70	0.0500
548.50	227.80	0.0500 / 0.1000
572.72	230.25	0.1000
662.40	230.50	0.1000
804.25	233.95	0.1000
926.11	235.83	0.1000
1047.96	240.70	0.1000
1145.45	243.26	0.1000
1206.38	247.03	0.1000

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.55	227.25	.111E+05	2.0	0.33	92.44
1.10	227.80	.242E+05	6.5	0.50	61.56
2.23	228.93	.700E+05	26.0	0.68	44.91
3.36	230.06	.150E+06	60.8	0.74	41.04
4.49	231.19	.446E+06	130.8	0.54	56.83
5.62	232.32	.925E+06	287.4	0.57	53.66
6.76	233.46	.152E+07	522.9	0.63	48.32
7.89	234.59	.223E+07	827.2	0.68	44.91
9.02	235.72	.312E+07	1219.5	0.72	42.61
10.15	236.85	.418E+07	1780.6	0.78	39.11
11.28	237.98	.537E+07	2479.2	0.85	36.07
12.41	239.11	.669E+07	3282.9	0.90	33.95
13.54	240.24	.824E+07	4228.6	0.94	32.47
14.67	241.37	.997E+07	5384.6	0.99	30.87
15.81	242.51	.119E+08	6698.4	1.03	29.56
16.94	243.64	.140E+08	8235.2	1.08	28.25
18.07	244.77	.162E+08	10104.2	1.14	26.73
19.20	245.90	.185E+08	12251.5	1.21	25.21
20.33	247.03	.209E+08	14597.0	1.28	23.89

hydrograph <-> pipe / channel

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8138)	99.85	8.50	23.97	3.99	0.61
OUTFLOW: ID= 1 (6208)	94.31	9.75	23.97	3.90	0.63

CALIB STANDHYD (2082) | Area (ha)= 426.60 | Total Imp(%)= 64.00 | Dir. Com.(%)= 37.00

Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
273.02	0.91	153.58
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1686.42	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	96.49	100.43
over (min)	15.00	30.00
Storage Coeff. (min)	17.38 (ii)	28.06 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.07	0.04
PEAK FLOW (cms)	28.85	18.58
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	71.10	37.25
TOTAL RAINFALL (mm)	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.51
TOTAL		
		40.830 (iii)
		6.00
		49.78
		73.10
		0.68

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2081) | Area (ha)= 265.70 | Total Imp(%)= 35.00 | Dir. Com.(%)= 20.00

Surface Area	(ha)=	IMPERVIOUS	PERVIOUS (i)
Dep. Storage	(mm)=	93.00	172.71
Average Slope	(%)=	2.00	5.00
Length	(m)=	0.50	0.50
Mannings n	=	1330.91	40.00
		0.013	0.250
Max. Eff. Inten. (mm/hr)=		96.49	57.25
over (min)		15.00	30.00
Storage Coeff. (min)=		15.08 (ii)	28.45 (ii)
Unit Hyd. Tpeak (min)=		15.00	30.00
Unit Hyd. peak (cms)=		0.07	0.04
		TOTALS	
PEAK FLOW (cms)=		10.35	11.70
TIME TO PEAK (hrs)=		6.00	6.00
RUNOFF VOLUME (mm)=		71.10	30.26
TOTAL RAINFALL (mm)=		73.10	73.10
RUNOFF COEFFICIENT =		0.97	0.53

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| RESERVOIR (5208) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min   |
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|
```

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	16.8530	6.3371
0.8360	2.7353	21.1630	7.3975
4.7810	3.1070	25.1140	8.4406
8.9050	4.2484	25.5140	8.4506
12.6070	5.1796	0.0000	0.0000

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AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
265.700	17.708	6.00	38.43
265.700	8.764	6.75	38.43

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PEAK FLOW REDUCTION [Qout/Qin] (%)	TIME SHIFT OF PEAK FLOW (min)	MAXIMUM STORAGE USED (ha.m.)
49.49	45.00	4.2273

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-----
| ADD HYD (8148) |
| 1 + 2 = 3     |
-----
|
```

ID#	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (2082):	426.60	40.830	6.00	49.78
+ ID2= 2 (5208):	265.70	8.764	6.75	38.43
=====				
ID = 3 (8148):	692.30	41.488	6.00	45.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| ADD HYD (8146) |
| 1 + 2 = 3     |
-----
|
```

ID#	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6208):	14723.15	94.307	9.75	23.97
+ ID2= 2 (8148):	692.30	41.488	6.00	45.42
=====				
ID = 3 (8146):	15415.45	98.384	9.75	24.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| ROUTE CHN (6210) |
| IN= 2--> OUT= 1 |
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|
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Routing time step (min) = 15.00

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|<----- DATA FOR SECTION (2101.0) -----|
|
```

Distance	Elevation	Manning
0.00	241.81	0.0800
28.43	241.78	0.0800
85.30	237.04	0.0800
149.27	231.32	0.0800
170.60	230.10	0.0800
255.89	229.37	0.0800
277.22	228.68	0.0800
284.33	228.42	0.0800
291.43	228.16	0.0800
297.96	227.00	0.0800 / 0.0350
298.76	225.00	0.0350
312.76	225.00	0.0350
313.76	227.00	0.0350 / 0.0800

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326.97	229.50	0.0800
348.30	230.29	0.0800
376.73	229.76	0.0800
490.46	235.75	0.0800
568.65	236.24	0.0800
661.06	241.85	0.0800
703.71	246.04	0.0800

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|<----- TRAVEL TIME TABLE -----|
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DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.67	225.67	141E+05	6.6	0.69	35.86
1.33	226.33	289E+05	20.4	1.04	23.65
2.00	227.00	443E+05	39.3	1.31	18.81
2.93	227.93	730E+05	77.8	1.57	15.64
3.85	228.85	1123E+06	131.1	1.57	15.65
4.78	229.78	228E+06	205.6	1.33	18.51
5.70	230.70	503E+06	356.7	1.05	23.49
6.63	231.63	846E+06	608.7	1.06	23.17
7.55	232.55	123E+07	946.4	1.14	21.65
8.48	233.48	165E+07	1367.0	1.22	20.13
9.41	234.41	211E+07	1871.8	1.31	18.80
10.33	235.33	261E+07	2463.2	1.40	17.65
11.26	236.26	317E+07	2999.4	1.40	17.63
12.18	237.18	384E+07	3848.3	1.48	16.63
13.11	238.11	454E+07	4806.0	1.56	15.75
14.03	239.03	528E+07	5876.9	1.64	14.97
14.96	239.96	605E+07	7062.7	1.72	14.29
15.88	240.88	687E+07	8365.0	1.80	13.68
16.81	241.81	771E+07	9506.9	1.82	13.52

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|<----- hydrograph -----> <-pipe / channel->
|
```

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
98.38	98.38	9.75	24.93	3.28	1.57

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INFLOW : ID= 2 (8146) *****

OUTFLOW : ID= 1 (6210) *****

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-----
| CALIB (0210) |
| NASHVD (0210) |
| ID= 1 DT=15.0 min |
| U.H. Tp (hrs)= 1.08 |
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|
```

Unit Hyd Qpeak (cms) = 3.454

PEAK FLOW (cms) = 3.436 (i)

TIME TO PEAK (hrs) = 7.250

RUNOFF VOLUME (mm) = 34.919

TOTAL RAINFALL (mm) = 73.100

RUNOFF COEFFICIENT = 0.478

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| ADD HYD (8150) |
| 1 + 2 = 3     |
-----
|
```

ID#	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0210):	218.27	3.436	7.25	34.92
+ ID2= 2 (6210):	15415.45	98.271	10.00	24.93
=====				
ID = 3 (8150):	15633.72	100.715	10.00	25.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| CALIB (0218) |
| NASHVD (0218) |
| ID= 1 DT=15.0 min |
| U.H. Tp (hrs)= 0.79 |
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|
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Unit Hyd Qpeak (cms) = 3.278

PEAK FLOW (cms) = 2.161 (i)

TIME TO PEAK (hrs) = 7.000

RUNOFF VOLUME (mm) = 25.085

TOTAL RAINFALL (mm) = 73.100

RUNOFF COEFFICIENT = 0.343

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| CALIB (0216) |
| NASHVD (0216) |
| ID= 1 DT=15.0 min |
| U.H. Tp (hrs)= 0.52 |
-----
|
```

Unit Hyd Qpeak (cms) = 4.768

PEAK FLOW (cms) = 3.476 (i)

TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 29.848
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.404

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (2121) | Area (ha)= 56.00
 ID= 1 DT=15.0 min | Total Imp(%)= 93.00 Dir. Conn.(%)= 75.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	52.08	3.92
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	611.01	40.00
Mannings n	0.013	0.250
Max.Eff.Inten.(mm/hr)	96.49	269.87
over (min)	15.00	30.00
Storage Coeff. (min)	9.45 (ii)	16.64 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.05
PEAK FLOW (cms)	9.66	1.63
TIME TO PEAK (hrs)	6.00	10.787 (iii)
RUNOFF VOLUME (mm)	71.10	49.60
TOTAL RAINFALL (mm)	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.68

TOTALS

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5212)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.7460	1.9720
0.1900	1.1850	4.7040	2.0838
1.0850	1.4413	5.4760	2.1940
2.0210	1.5949	5.8760	2.2040
2.8030	1.7848	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
INFLOW : ID= 2 (2121)	56.000	10.787	6.00	65.72
OUTFLOW : ID= 1 (5212)	56.000	3.632	6.50	65.70

PEAK FLOW REDUCTION [Qout/Qin](%)= 33.67
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 1.9869

CALIB
 STANDHYD (2122) | Area (ha)= 272.40
 ID= 1 DT=15.0 min | Total Imp(%)= 29.00 Dir. Conn.(%)= 23.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	79.00	193.40
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1347.59	40.00
Mannings n	0.013	0.250
Max.Eff.Inten.(mm/hr)	96.49	44.85
over (min)	15.00	30.00
Storage Coeff. (min)	15.19 (ii)	29.93 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.07	0.04
PEAK FLOW (cms)	12.17	9.93
TIME TO PEAK (hrs)	6.00	18.332 (iii)
RUNOFF VOLUME (mm)	71.10	27.00
TOTAL RAINFALL (mm)	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.37

TOTALS

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (2122)	272.40	18.332	6.00	37.15
+ ID2= 2 (5212)	56.00	3.632	6.50	65.70
ID = 3 (8152)	328.40	19.430	6.25	42.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6214)
 IN= 2--> OUT= 1 Routing time step (min)= 15.00

Distance	Elevation	Manning
0.00	259.29	0.0500
15.33	258.91	0.0500
38.33	258.57	0.0500
65.16	257.93	0.0500
95.82	253.44	0.0500
111.35	251.21	0.0500
130.32	249.67	0.0500
149.48	248.51	0.0500
150.66	248.50	0.0500 / 0.0370
150.81	247.50	0.0370
154.66	247.50	0.0370
155.66	248.50	0.0370 / 0.0500
160.98	248.50	0.0500
187.81	249.78	0.0500
199.31	252.13	0.0500
245.30	254.04	0.0500
264.46	253.99	0.0500
298.96	255.23	0.0500
329.62	257.02	0.0500
379.45	258.82	0.0500

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.50	248.00	6.648E+04	2.1	1.00	52.30
1.00	248.50	1.139E+05	6.4	1.44	36.35
1.61	249.11	1.570E+05	22.6	1.24	42.01
2.21	249.71	1.438E+06	63.6	1.39	37.61
2.82	250.32	2.638E+06	143.9	1.70	30.69
3.43	250.93	4.038E+06	257.2	2.00	26.11
4.04	251.54	5.628E+06	410.4	2.29	22.82
4.64	252.14	7.358E+06	601.3	2.56	20.37
5.25	252.75	9.338E+06	791.0	2.66	19.67
5.86	253.36	1.178E+07	1038.0	2.79	18.74
6.46	253.96	1.448E+07	1344.4	2.93	17.82
7.07	254.57	1.738E+07	1664.1	2.93	17.83
7.68	255.18	2.178E+07	2140.2	3.10	16.87
8.28	255.78	2.598E+07	2722.9	3.30	15.83
8.89	256.39	3.048E+07	3387.1	3.50	14.94
9.50	257.00	3.518E+07	4132.7	3.69	14.16
10.11	257.61	4.028E+07	4913.0	3.83	13.64
10.71	258.21	4.588E+07	5648.3	3.87	13.50
11.32	258.82	5.218E+07	6343.1	3.82	13.68

hydrograph <----> <--pipe / channel-->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8152)	328.40	19.43	6.25	42.01	1.49	1.28
OUTFLOW : ID= 1 (6214)	328.40	12.82	6.75	42.01	1.24	1.35

CALIB
 STANDHYD (0214) | Area (ha)= 316.95
 ID= 1 DT=15.0 min | Total Imp(%)= 25.00 Dir. Conn.(%)= 9.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	78.24	237.71
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1453.62	40.00
Mannings n	0.013	0.250
Max.Eff.Inten.(mm/hr)	96.49	44.40
over (min)	15.00	45.00
Storage Coeff. (min)	15.90 (ii)	30.70 (ii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.07	0.03
PEAK FLOW (cms)	5.43	10.96
TIME TO PEAK (hrs)	6.00	12.670 (iii)
RUNOFF VOLUME (mm)	71.10	24.12
TOTAL RAINFALL (mm)	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.33

TOTALS

***** WARNING:FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%

YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 62.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8154)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0214):	316.95	12.670	6.50	28.35
+ ID2= 2 (6214):	328.40	12.815	6.75	42.01
=====				
ID = 3 (8154):	645.35	25.308	6.50	35.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8156)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0216):	145.77	3.476	6.50	29.55
+ ID2= 2 (8154):	645.35	25.308	6.50	35.30
=====				
ID = 3 (8156):	791.12	28.784	6.50	34.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6218)
IN= 2--> OUT= 1
Routing time step (min)= 15.00

<----- DATA FOR SECTION (2181.0) ----->

Distance	Elevation	Manning
0.00	256.38	0.0450
7.70	254.85	0.0450
15.40	253.20	0.0450
19.25	252.29	0.0450
23.10	251.17	0.0450
78.94	236.16	0.0450
80.87	235.80	0.0450
82.79	235.45	0.0450
84.57	234.60	0.0450 / 0.0300
85.07	233.60	0.0300
88.57	233.60	0.0300
93.57	233.60	0.0300
94.07	234.50	0.0300 / 0.0450
96.27	235.13	0.0450
98.19	235.20	0.0450
100.12	235.32	0.0450
138.63	239.95	0.0450
180.98	244.80	0.0450
188.69	245.00	0.0450
190.61	256.00	0.0450

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
1.00	234.60	181E+05	6.0	0.66	50.64
2.13	235.73	518E+05	23.8	0.93	36.27
3.25	236.85	119E+06	60.0	1.02	33.03
4.38	237.98	217E+06	120.6	1.12	29.99
5.51	239.11	346E+06	210.5	1.23	27.39
6.63	240.23	506E+06	334.1	1.33	25.22
7.76	241.36	697E+06	495.6	1.43	23.44
8.88	242.48	920E+06	699.8	1.53	21.91
10.01	243.61	117E+07	950.8	1.63	20.59
11.14	244.74	146E+07	1252.5	1.73	19.45
12.26	245.86	178E+07	1619.6	1.83	18.37
13.39	246.99	212E+07	2078.5	1.97	17.01
14.52	248.12	247E+07	2589.0	2.11	15.88
15.64	249.24	282E+07	3149.7	2.25	14.94
16.77	250.37	319E+07	3759.8	2.37	14.14
17.89	251.49	356E+07	4419.5	2.50	13.44
19.02	252.62	395E+07	5126.4	2.61	12.84
20.15	253.75	435E+07	5871.6	2.72	12.34
21.27	254.87	476E+07	6663.2	2.82	11.89

<----- hydrograph -----> <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8156)	791.12	28.78	6.50	34.24	2.28
OUTFLOW : ID= 1 (6218)	791.12	21.19	6.75	34.24	1.96

ADD HYD (8158)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0218):	352.25	2.161	7.00	25.08
+ ID2= 2 (6218):	791.12	21.195	6.75	34.24
=====				
ID = 3 (8158):	943.37	23.336	6.75	32.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8151)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8150):	15633.72	100.715	10.00	25.07
+ ID2= 2 (8158):	943.37	23.336	6.75	32.76
=====				
ID = 3 (8151):	16577.09	114.696	7.00	25.51

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5504)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1				
DT= 15.0 min				
0.0000	0.0000	33.0100	0.6700	
2.6400	0.0500	37.4800	0.7800	
4.8300	0.0900	47.8400	1.0100	
7.4100	0.1300	60.2300	1.2800	
10.3300	0.1900	71.9530	1.5258	
13.5300	0.2500	*****	3.0529	
16.9900	0.3200	*****	5.3299	
20.6800	0.3900	*****	8.4148	
24.6000	0.4800	*****	11.7637	
28.7100	0.5700	*****	11.7737	

INFLOW : ID= 2 (8151)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
OUTFLOW : ID= 1 (5504)	*****	114.696	7.00	25.51
	*****	114.638	7.00	25.51

PEAK FLOW REDUCTION [Qout/Qin] (%)= 99.95
 TIME SHIFT OF PEAK FLOW (min)= 0.00
 MAXIMUM STORAGE USED (ha.m.)= 2.4340

ROUTE CHN (6220)
 IN= 2--> OUT= 1
 Routing time step (min)= 15.00

<----- DATA FOR SECTION (2201.0) ----->

Distance	Elevation	Manning
0.00	245.59	0.0600
5.76	245.45	0.0600
23.03	242.27	0.0600
57.58	232.35	0.0600
86.37	226.18	0.0600
109.41	224.31	0.0600
115.17	223.86	0.0600
120.92	222.78	0.0600
126.68	221.93	0.0600
128.06	221.50	0.0600 / 0.0300
129.56	220.00	0.0300
141.56	220.00	0.0300
143.06	221.50	0.0300 / 0.0600
149.72	223.77	0.0600
155.47	225.09	0.0600
253.36	229.21	0.0600
333.98	227.50	0.0600
454.90	229.25	0.0600
500.97	233.11	0.0600
570.07	237.42	0.0600

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.75	220.75	262E+05	5.5	0.57	79.44
1.50	221.50	555E+05	17.5	0.86	52.89
2.44	222.44	103E+06	42.7	1.14	40.03
3.37	223.37	171E+06	79.4	1.27	35.98
4.31	224.31	263E+06	128.1	1.33	34.22
5.25	225.25	393E+06	191.7	1.34	34.19
6.18	226.18	594E+06	276.7	1.28	35.77
7.12	227.12	872E+06	399.9	1.26	36.33
8.06	228.06	127E+07	542.7	1.17	38.92
8.99	228.99	199E+07	770.0	1.06	43.01
9.93	229.93	297E+07	1151.8	1.06	42.95
10.86	230.86	400E+07	1653.9	1.13	40.33
11.80	231.80	507E+07	2251.8	1.22	37.53
12.74	232.74	618E+07	2946.5	1.31	34.97
13.67	233.67	733E+07	3726.8	1.39	32.79

14.61	234.61	.853E+07	4591.0	1.48	30.96
15.55	235.55	.977E+07	5544.1	1.55	29.37
16.48	236.48	.111E+08	6586.7	1.63	27.99
17.42	237.42	.124E+08	7719.2	1.71	26.77

<---- hydrograph ----> <-pipe / channel-->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLW : ID= 2 (5504)	*****	114.64	7.00	25.51	4.05	1.32
OUTFLOW: ID= 1 (6220)	*****	108.56	9.25	25.51	3.94	1.31

CALIB	NASHYD (8220)	Area (ha)	Curve Number (CN)
-----	-----	169.10	73.0
ID= 1 DT=15.0 min	Ia (mm)	5.00	# of Linear Res. (N)= 1.50
-----	-----	U.H. Tp(hrs)= 1.50	-----

Unit Hyd Qpeak (cms) = 1.923

PEAK FLOW (cms) = 1.657 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 28.460
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.389

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8160)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0220):	169.10	1.657	8.00	28.46
+ ID2= 2 (6220):	16577.09	108.563	9.25	25.51
-----	-----	-----	-----	-----
ID = 3 (8160):	16746.19	110.133	9.25	25.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	NASHYD (8226)	Area (ha)	Curve Number (CN)
-----	-----	237.58	75.0
ID= 1 DT=15.0 min	Ia (mm)	5.00	# of Linear Res. (N)= 1.50
-----	-----	U.H. Tp(hrs)= 1.05	-----

Unit Hyd Qpeak (cms) = 3.852

PEAK FLOW (cms) = 3.250 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 30.071
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.411

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (2221)	Area (ha)	Dir. Conn. (%)
-----	-----	145.20	19.00
ID= 1 DT=15.0 min	Total Imp(%)	36.00	Dir. Conn. (%) = 19.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	52.27	92.93
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	983.87	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (min)	96.49 / 15.00	68.91 / 30.00
Storage Coeff. (min)	12.58 (ii)	24.99 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04
-----	-----	-----
PEAK FLOW (cms)	5.78	8.14
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	71.10	35.15
TOTAL RAINFALL (mm)	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.48

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5222)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min	0.0000	0.0000	5.6430	3.7577
	0.4620	2.0014	8.2830	4.2576
	2.0160	2.2850	14.1970	4.6909
	3.6250	2.7987	14.5970	4.7009
	5.0230	3.2496	0.0000	0.0000

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
INFLW : ID= 2 (2221)	145.200	10.996	6.00
OUTFLOW: ID= 1 (5222)	145.200	4.189	7.00

PEAK FLOW REDUCTION [Qout/Qin](%) = 38.10
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 3.0140

CALIB	STANDHYD (2222)	Area (ha)	Dir. Conn. (%)
-----	-----	74.30	41.00
ID= 1 DT=15.0 min	Total Imp(%)	78.00	Dir. Conn. (%) = 41.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	57.95	16.35
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	703.80	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) over (min)	96.49 / 15.00	202.58 / 30.00
Storage Coeff. (min)	10.29 (ii)	18.35 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.05
-----	-----	-----
PEAK FLOW (cms)	6.83	4.91
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	71.10	49.36
TOTAL RAINFALL (mm)	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.68

TOTALS

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8164)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (2222):	74.30	10.202	6.00	58.27
+ ID2= 2 (5222):	145.20	4.189	7.00	41.97
-----	-----	-----	-----	-----
ID = 3 (8164):	219.50	10.509	6.00	47.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	STANDHYD (0224)	Area (ha)	Dir. Conn. (%)
-----	-----	140.45	26.00
ID= 1 DT=15.0 min	Total Imp(%)	34.00	Dir. Conn. (%) = 26.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	47.75	92.70
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	967.64	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) over (min)	96.49 / 15.00	63.88 / 30.00
Storage Coeff. (min)	12.45 (ii)	25.25 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04
-----	-----	-----
PEAK FLOW (cms)	7.49	7.49
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	71.10	36.56
TOTAL RAINFALL (mm)	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.50

TOTALS

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8166)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0224):	140.45	12.486	6.00	45.54
+ ID2= 2 (8164):	219.50	10.509	6.00	47.49
ID = 3 (8166):	359.95	22.994	6.00	46.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CEN (6226) |
 IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (2261.0) -----

Distance	Elevation	Manning
0.00	245.35	0.0600
8.27	242.72	0.0600
16.54	240.01	0.0600
31.02	235.31	0.0600
41.36	233.14	0.0600
90.99	227.73	0.0600
93.06	227.50	0.0600
95.13	227.27	0.0600
95.83	227.00	0.0600
100.83	226.50	0.0600 / 0.0350
101.33	225.50	0.0350
101.83	226.50	0.0350 / 0.0600
105.83	227.00	0.0600
107.54	227.39	0.0600
109.60	227.62	0.0600
111.67	227.86	0.0600
146.83	231.76	0.0600
173.71	234.77	0.0600
186.12	238.23	0.0600
202.67	238.35	0.0600

----- TRAVEL TIME TABLE -----

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.50	226.00	.403E+03	0.0	0.16	330.59
1.00	226.50	.163E+04	0.1	0.26	208.26
1.70	227.20	.173E+05	1.5	0.27	195.72
2.39	227.89	.541E+05	5.9	0.35	152.27
3.09	228.59	.119E+06	16.0	0.43	124.08
3.79	229.29	.212E+06	33.8	0.51	104.65
4.49	229.99	.334E+06	61.2	0.59	90.97
5.18	230.68	.484E+06	99.7	0.66	80.88
5.88	231.38	.663E+06	151.0	0.73	73.13
6.58	232.08	.870E+06	216.5	0.80	66.95
7.27	232.77	.111E+07	297.5	0.87	61.91
7.97	233.47	.137E+07	398.1	0.94	57.29
8.67	234.17	.165E+07	518.9	1.01	53.12
9.36	234.86	.196E+07	658.9	1.08	49.59
10.06	235.56	.228E+07	827.0	1.17	46.02
10.76	236.26	.262E+07	1015.4	1.25	42.96
11.46	236.96	.296E+07	1221.2	1.33	40.41
12.15	237.65	.332E+07	1444.6	1.40	38.26
12.85	238.35	.368E+07	1598.2	1.40	38.42

----- hydrograph ----- <-pipe / channel-->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW: ID= 2 (8166)	359.95	22.99	6.00	46.73	3.37
OUTFLOW: ID= 1 (6226)	359.95	7.25	6.75	46.73	2.49

ADD HYD (8168)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0226):	237.58	3.250	7.25	30.07
+ ID2= 2 (6226):	359.95	7.253	6.75	46.73
ID = 3 (8168):	597.53	10.343	6.75	40.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8162)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8160):	16746.19	110.133	9.25	25.54

+ ID2= 2 (8168): 597.53 10.343 6.75 40.10
 ID = 3 (8162): 17343.72 118.043 8.50 26.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CEN (6228) |
 IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (2281.0) -----

Distance	Elevation	Manning
0.00	245.00	0.0600
18.60	244.74	0.0600
37.20	242.47	0.0600
93.01	233.73	0.0600
125.57	226.95	0.0600
213.93	221.63	0.0600
218.58	221.15	0.0600
223.23	220.45	0.0600
225.03	220.55	0.0600 / 0.0300
225.53	218.05	0.0300
240.03	218.05	0.0300
241.03	220.55	0.0300 / 0.0600
246.48	221.00	0.0600
251.13	221.27	0.0600
255.78	221.22	0.0600
325.54	225.25	0.0600
367.40	228.91	0.0600
404.60	231.75	0.0600
460.41	245.00	0.0600

----- TRAVEL TIME TABLE -----

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
1.25	219.30	.338E+05	16.0	0.86	35.15
2.50	220.55	.696E+05	48.3	1.26	24.02
3.94	221.99	.168E+06	116.0	1.26	24.14
5.38	223.43	.392E+06	247.5	1.15	26.37
6.81	224.86	.743E+06	472.6	1.16	26.20
8.25	226.30	.122E+07	821.6	1.23	24.67
9.69	227.74	.179E+07	1320.1	1.34	22.59
11.13	229.18	.243E+07	1955.7	1.47	20.58
12.57	230.62	.313E+07	2744.2	1.59	19.02
14.01	232.06	.390E+07	3683.4	1.72	17.66
15.44	233.49	.472E+07	4824.1	1.86	16.30
16.88	234.93	.557E+07	6092.4	1.99	15.23
18.32	236.37	.646E+07	7503.3	2.11	14.35
19.76	237.81	.739E+07	9061.2	2.23	13.59
21.20	239.25	.836E+07	10767.6	2.34	12.94
22.64	240.69	.937E+07	12624.8	2.45	12.37
24.07	242.12	.104E+08	14634.9	2.56	11.87
25.51	243.56	.115E+08	16762.8	2.65	11.45
26.95	245.00	.127E+08	18699.9	2.69	11.28

----- hydrograph ----- <-pipe / channel-->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW: ID= 2 (8162)	116.13	8.50	26.04	3.96	1.25
OUTFLOW: ID= 1 (6228)	116.13	9.75	26.04	3.93	1.26

CALIB |
 NASHYD (0228) | Area (ha)= 310.54 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 1.12

Unit Hyd Qpeak (cms)= 4.751
 PEAK FLOW (cms)= 2.635 (l)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 19.946
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.273

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8170)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0228):	310.54	2.635	7.50	19.95
+ ID2= 2 (6228):	17343.72	116.125	9.75	26.04
ID = 3 (8170):	17654.26	118.234	9.50	25.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| STANDHYD (2302) | Area (ha)= 126.70
| ID= 1 DT=15.0 min | Total Imp(%)= 23.00 Dir. Conn.(%)= 9.00
-----

```

```

-----
| IMPERVIOUS | PERVIOUS (i) |
| Surface Area (ha)= 29.14 | 97.56 |
| Dep. Storage (mm)= 2.00 | 5.00 |
| Average Slope (%)= 0.50 | 0.50 |
| Length (m)= 919.06 | 40.00 |
| Mannings n = 0.013 | 0.250 |
-----

```

```

-----
| Max.Eff.Inten.(mm/hr)= 96.49 | 51.97 | |
| over (min) | 15.00 | 30.00 |
| Storage Coeff. (min)= 12.07 (ii) | 25.97 (ii) |
| Unit Hyd. Tpeak (min)= 15.00 | 30.00 |
| Unit Hyd. peak (cms)= 0.08 | 0.04 |
-----

```

```

-----
| PEAK FLOW (cms)= 2.42 | 6.28 | *TOTALS* |
| TIME TO PEAK (hrs)= 6.00 | 6.25 | 7.277 (iii) |
| RUNOFF VOLUME (mm)= 71.10 | 26.67 | 32.49 |
| TOTAL RAINFALL (mm)= 73.10 | 73.10 | 73.10 |
| RUNOFF COEFFICIENT = 0.97 | 0.39 | 0.44 |
-----

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

```

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
-----

```

```

-----
| CALIB |
| STANDHYD (2301) | Area (ha)= 78.80
| ID= 1 DT=15.0 min | Total Imp(%)= 50.00 Dir. Conn.(%)= 19.00
-----

```

```

-----
| IMPERVIOUS | PERVIOUS (i) |
| Surface Area (ha)= 39.40 | 39.40 |
| Dep. Storage (mm)= 2.00 | 5.00 |
| Average Slope (%)= 0.50 | 0.50 |
| Length (m)= 724.80 | 40.00 |
| Mannings n = 0.013 | 0.250 |
-----

```

```

-----
| Max.Eff.Inten.(mm/hr)= 96.49 | 86.99 | |
| over (min) | 15.00 | 30.00 |
| Storage Coeff. (min)= 10.47 (ii) | 21.78 (ii) |
| Unit Hyd. Tpeak (min)= 15.00 | 30.00 |
| Unit Hyd. peak (cms)= 0.09 | 0.05 |
-----

```

```

-----
| PEAK FLOW (cms)= 3.34 | 4.66 | *TOTALS* |
| TIME TO PEAK (hrs)= 6.00 | 6.25 | 6.353 (iii) |
| RUNOFF VOLUME (mm)= 71.10 | 34.90 | 41.78 |
| TOTAL RAINFALL (mm)= 73.10 | 73.10 | 73.10 |
| RUNOFF COEFFICIENT = 0.97 | 0.48 | 0.57 |
-----

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

```

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
-----

```

```

-----
| RESERVOIR (5230) |
| ID= 2--> OUT= 1 |
| DT= 15.0 min |
-----
| OUTFLOW | STORAGE | OUTFLOW | STORAGE |
| (cms) | (ha.m.) | (cms) | (ha.m.) |
| 0.0000 | 0.0000 | 5.6350 | 1.5914 |
| 0.3160 | 0.7624 | 6.9200 | 1.8001 |
| 1.7040 | 0.8931 | 7.9010 | 2.0070 |
| 3.5140 | 1.1218 | 8.3010 | 2.0170 |
| 4.5020 | 1.3405 | 0.0000 | 0.0000 |
-----

```

```

-----
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) |
| INFLOW : ID= 2 (2301) | 78.800 | 6.353 | 6.00 | 41.78 |
| OUTFLOW: ID= 1 (5230) | 78.800 | 3.939 | 6.50 | 41.77 |
-----

```

```

| PEAK FLOW REDUCTION [Qout/Qin](%)= 62.00 |
| TIME SHIFT OF PEAK FLOW (min)= 30.00 |
| MAXIMUM STORAGE USED (ha.m.)= 1.2213 |
-----

```

```

-----
| ADD HYD (8172) |
| 1 + 2 = 3 |
| AREA | QPEAK | TPEAK | R.V. |
-----

```

```

-----
| (ha) | (cms) | (hrs) | (mm) | |
| ID1= 1 (2302): | 126.70 | 7.277 | 6.25 | 32.49 |
| + ID2= 2 (5230): | 78.80 | 3.939 | 6.50 | 41.77 |
| ***** |
| ID = 3 (8172): | 205.50 | 10.709 | 6.25 | 36.05 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHVD (0232) | Area (ha)= 314.80 Curve Number (CN)= 58.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.00 |
-----

```

Unit Hyd Qpeak (cms)= 5.391

```

-----
| PEAK FLOW (cms)= 2.653 (i) |
| TIME TO PEAK (hrs)= 7.250 |
| RUNOFF VOLUME (mm)= 18.212 |
| TOTAL RAINFALL (mm)= 73.100 |
| RUNOFF COEFFICIENT = 0.249 |
-----

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8173) |
| 1 + 2 = 3 |
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) |
| ID1= 1 (0232): | 314.80 | 2.653 | 7.25 | 18.21 |
| + ID2= 2 (8172): | 205.50 | 10.709 | 6.25 | 36.05 |
| ***** |
| ID = 3 (8173): | 520.30 | 12.711 | 6.25 | 25.26 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8174) |
| 1 + 2 + 3 |
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) |
| ID1 = 1 (8170): | 17654.26 | 118.234 | 9.50 | 25.93 |
| + ID2= 2 (8173): | 520.30 | 12.711 | 6.25 | 25.26 |
| ***** |
| ID = 3 (8174): | 18174.56 | 121.348 | 9.50 | 25.91 |
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CN# (5234) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
-----

```

```

-----
| DATA FOR SECTION (2341.0) |----->
| Distance | Elevation | Manning |
| 0.00 | 228.00 | 0.0550 |
| 42.86 | 223.21 | 0.0550 |
| 85.72 | 219.56 | 0.0550 |
| 107.15 | 219.42 | 0.0550 |
| 128.58 | 219.39 | 0.0550 |
| 150.00 | 219.30 | 0.0550 |
| 171.43 | 219.26 | 0.0550 |
| 192.86 | 219.24 | 0.0550 |
| 214.30 | 219.30 | 0.0550 |
| 235.73 | 219.24 | 0.0550 |
| 257.16 | 219.20 | 0.0350 / 0.0350 Main Channel |
| 278.59 | 217.20 | 0.0350 Main Channel |
| 300.02 | 219.20 | 0.0350 / 0.0550 Main Channel |
| 321.45 | 219.27 | 0.0550 |
| 342.88 | 219.26 | 0.0550 |
| 364.31 | 219.29 | 0.0550 |
| 385.74 | 222.91 | 0.0550 |
| 407.17 | 227.43 | 0.0550 |
| 428.60 | 227.80 | 0.0550 |
| 450.03 | 228.00 | 0.0550 |
-----

```

```

-----
| TRAVEL TIME TABLE |----->
| DEPTH | ELEV | VOLUME | FLOW RATE | VELOCITY | TRAV.TIME |
| (m) | (m) | (cu.m.) | (cms) | (m/s) | (min) |
| 0.50 | 217.70 | .887E+05 | 7.2 | 0.31 | 206.54 |
| 1.00 | 218.20 | 1.79E+06 | 22.7 | 0.48 | 131.74 |
| 1.50 | 218.70 | 2.72E+06 | 44.5 | 0.62 | 101.75 |
| 2.00 | 219.20 | 3.66E+06 | 71.8 | 0.75 | 84.97 |
| 2.59 | 219.79 | 4.93E+06 | 136.1 | 0.52 | 121.59 |
| 3.17 | 220.37 | 6.19E+07 | 258.6 | 0.52 | 121.80 |
| 3.76 | 220.96 | 7.29E+07 | 434.7 | 0.56 | 112.82 |
| 4.35 | 221.55 | 8.41E+07 | 665.9 | 0.61 | 103.88 |
| 4.93 | 222.13 | 9.51E+07 | 955.0 | 0.66 | 96.22 |
| 5.52 | 222.72 | 1.03E+08 | 1305.1 | 0.71 | 89.80 |
| 6.11 | 223.31 | 1.11E+08 | 1744.9 | 0.77 | 82.91 |
| 6.69 | 223.89 | 1.19E+08 | 2261.4 | 0.83 | 76.46 |
| 7.28 | 224.48 | 1.21E+08 | 2838.0 | 0.89 | 71.12 |
-----

```

7.87	225.07	.138E+08	3472.9	0.95	66.63
8.45	225.65	.157E+08	4165.0	1.01	62.82
9.04	226.24	.176E+08	4913.5	1.07	59.54
9.63	226.83	.194E+08	5717.7	1.12	56.68
10.21	227.41	.214E+08	6577.1	1.17	54.18
10.80	228.00	.235E+08	6919.5	1.12	56.61

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8174) ***** 121.35 9.50 25.91 2.45 0.56
OUTFLOW : ID= 1 (6234) ***** 106.85 12.00 25.91 2.32 0.61

```

```

-----
| CALIB | Area (ha)= 267.16
| STANHYD (0234) | Total Imp(%)= 22.00 Dir. Conn.(%)= 8.00
| ID= 1 DT=15.0 min |
-----

```

```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 58.78 208.38
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1334.57 40.00
Mannings n = 0.013 0.250
Max. Eff. Inten. (mm/hr)= 96.49 12.21
over (min) 15.00 45.00
Storage Coeff. (min)= 15.10 (ii) 39.91 (ii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.07 0.03
**TOTALS**
PEAK FLOW (cms)= 4.16 3.81 5.688 (iii)
TIME TO PEAK (hrs)= 6.00 6.50 6.00
RUNOFF VOLUME (mm)= 71.10 11.69 16.44
TOTAL RAINFALL (mm)= 73.10 73.10 73.10
RUNOFF COEFFICIENT = 0.97 0.16 0.22

```

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 39.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8176) | AREA QPEAK TPEAK R.V.
| 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
|-----|
| ID1= 1 (0234): 267.16 5.688 6.00 16.44
| + ID2= 2 (6234): 18174.56 106.845 12.00 25.91
|-----|
| ID = 3 (8176): 18441.72 107.257 12.00 25.77
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB | Area (ha)= 311.70 Curve Number (CN)= 53.0
| NASHYD (0238) | Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.56
-----
Unit Hyd Qpeak (cms)= 3.405
PEAK FLOW (cms)= 1.603 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 15.726
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.215

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB | Area (ha)= 494.49 Curve Number (CN)= 54.0
| NASHYD (0236) | Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.24
-----

```

Unit Hyd Qpeak (cms)= 6.830

```

PEAK FLOW (cms)= 3.138 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 16.181
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.221

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ROUTE CHN (6238) | Routing time step (min)'= 15.00
| IN= 2--> OUT= 1 |
-----

```

```

<----- DATA FOR SECTION (2381.0) ----->
Distance Elevation Manning
0.00 241.54 0.0380
602.55 227.00 0.0380
1702.00 224.50 0.0380
1721.25 223.00 0.0380
1725.10 222.60 0.0380
1780.94 222.50 0.0380
1782.87 222.45 0.0380
1784.79 222.40 0.0380 /0.0300 Main Channel
1786.57 221.75 0.0300 Main Channel
1787.07 220.75 0.0300 Main Channel
1787.57 220.75 0.0300 Main Channel
1791.57 220.75 0.0300 Main Channel
1794.07 221.75 0.0300 Main Channel
1798.27 222.00 0.0300 Main Channel
1800.19 222.35 0.0300 /0.0380 Main Channel
1802.12 222.47 0.0380
1840.63 223.00 0.0380
1900.00 225.00 0.0380
1950.00 226.00 0.0380
2242.61 240.00 0.0380

```

```

-----
| TRAVEL TIME TABLE |
| DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME |
| (m) (m) (cu.m.) (cms) (m/s) (min) |
|-----|
| 0.80 221.55 103E+05 5.2 1.14 32.36 |
| 1.60 222.35 296E+05 18.7 1.40 26.36 |
| 2.64 223.39 249E+06 151.8 1.35 27.33 |
| 3.68 224.43 613E+06 512.2 1.85 19.94 |
| 4.71 225.46 152E+07 1066.8 1.55 23.80 |
| 5.75 226.50 359E+07 2824.9 1.75 21.16 |
| 6.79 227.54 663E+07 6528.0 2.18 16.93 |
| 7.83 228.58 994E+07 12158.6 2.71 13.62 |
| 8.87 229.62 134E+08 19268.8 3.19 11.58 |
| 9.91 230.66 170E+08 27785.3 3.63 10.19 |
| 10.94 231.69 207E+08 37667.1 4.03 9.18 |
| 11.98 232.73 246E+08 48891.7 4.40 8.40 |
| 13.02 233.77 287E+08 61448.0 4.75 7.78 |
| 14.06 234.81 329E+08 75332.7 5.08 7.28 |
| 15.10 235.85 372E+08 90547.8 5.39 6.85 |
| 16.14 236.89 417E+08 ***** 5.69 6.49 |
| 17.17 237.92 464E+08 ***** 5.98 6.18 |
| 18.21 238.96 512E+08 ***** 6.25 5.91 |
| 19.25 240.00 561E+08 ***** 6.52 5.67 |
-----

```

```

-----
| ADD HYD (8176) | AREA QPEAK TPEAK R.V.
| 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
|-----|
| ID1= 1 (0236): 494.49 3.14 8.00 16.18
| + ID2= 2 (6238): 18174.56 106.845 12.00 25.91
|-----|
| ID = 3 (8176): 18441.72 107.257 12.00 25.77
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8180) | AREA QPEAK TPEAK R.V.
| 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
|-----|
| ID1= 1 (0238): 311.70 1.603 8.25 15.73
| + ID2= 2 (6238): 494.49 3.041 8.50 16.18
|-----|
| ID = 3 (8180): 806.19 4.640 8.50 16.01
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8178) | AREA QPEAK TPEAK R.V.
| 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
|-----|
| ID1= 1 (8176): 18441.72 107.257 12.00 25.77
| + ID2= 2 (8180): 806.19 4.640 8.50 16.01
|-----|
| ID = 3 (8178): 19247.91 110.592 11.75 25.37
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6240) | Routing time step (min)'= 15.00
| IN= 2--> OUT= 1 |
-----

```

```

<----- DATA FOR SECTION (2401.0) ----->
Distance Elevation Manning
0.00 222.00 0.0450
11.46 221.00 0.0450
208.98 221.00 0.0450
404.04 220.97 0.0450

```

808.08	220.83	0.0450	
905.60	220.17	0.0450	
919.53	219.43	0.0450	
933.47	219.22	0.0450	
945.26	219.21	0.0450 / 0.0300	Main Channel
946.26	217.81	0.0300	Main Channel
975.26	217.81	0.0300	Main Channel
1003.26	217.81	0.0300	Main Channel
1005.26	219.21	0.0300 / 0.0450	Main Channel
1017.06	219.28	0.0450	
1030.99	219.26	0.0450	
1044.92	219.23	0.0450	
1058.86	219.23	0.0450	
1253.91	219.22	0.0450	
1323.57	221.05	0.0450	
1379.30	222.00	0.0450	

TRAVEL TIME TABLE					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.20	218.01	307E+05	2.2	0.20	227.52
0.40	218.21	617E+05	7.1	0.31	143.95
0.60	218.41	928E+05	14.0	0.41	110.33
0.80	218.61	124E+06	22.6	0.49	91.46
1.00	218.81	156E+06	32.8	0.57	79.15
1.20	219.01	188E+06	44.5	0.64	70.38
1.40	219.21	220E+06	57.5	0.70	63.77
1.63	219.44	413E+06	82.5	0.84	53.52
1.87	219.68	632E+06	120.9	0.91	47.03
2.10	219.91	858E+06	170.0	0.98	41.11
2.33	220.14	109E+07	228.7	1.06	36.22
2.56	220.37	134E+07	294.4	1.13	32.33
2.80	220.61	162E+07	371.1	1.20	29.25
3.03	220.84	192E+07	456.8	1.27	26.71
3.26	221.07	253E+07	552.1	1.34	24.54
3.49	221.30	333E+07	711.8	1.41	22.62
3.73	221.54	417E+07	907.0	1.48	21.00
3.96	221.77	502E+07	1134.1	1.55	19.60
4.19	222.00	587E+07	1390.9	1.62	18.40

hydrograph						<-pipe / channel->	
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL		
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW : ID= 2 (8178) *****	110.59	11.75	25.37	1.80	0.52		
OUTFLOW: ID= 1 (6240) *****	103.56	13.25	25.37	1.76	0.52		

CALIB (0240) | Area (ha)= 434.37 | Curve Number (CN)= 46.0
 NASHVD (0240) | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 3.60

Unit Hyd Qpeak (cms)= 2.058
 PEAK FLOW (cms)= 0.942 (1)
 TIME TO PEAK (hrs)= 12.000
 RUNOFF VOLUME (mm)= 12.642
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.173

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8182) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0240): 434.37 0.942 12.00 12.64
 + ID2= 2 (6240): 19247.91 103.562 13.25 25.37
 ID= 3 (8182): 19682.28 104.464 13.25 25.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0242) | Area (ha)= 657.88 | Curve Number (CN)= 52.0
 NASHVD (0242) | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 5.37

Unit Hyd Qpeak (cms)= 2.090
 PEAK FLOW (cms)= 1.221 (1)
 TIME TO PEAK (hrs)= 12.750
 RUNOFF VOLUME (mm)= 15.315
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.210

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8184) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0242): 657.88 1.221 12.75 15.32
 + ID2= 2 (8182): 19682.28 104.464 13.25 25.08
 ID= 3 (8184): 20340.16 105.684 13.25 24.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6244) | Routing time step (min)= 15.00
 IN= 2--> OUT= 1 |

DATA FOR SECTION (2441.0)			
Distance	Elevation	Manning	
0.00	225.00	0.0450	
2.50	220.10	0.0450	
3.00	220.06	0.0450	
3.64	220.07	0.0450	
7.28	220.08	0.0450	
10.91	220.09	0.0450	
14.55	219.81	0.0450	
18.19	219.29	0.0450	
24.13	219.21	0.0450 / 0.0300	Main Channel
24.63	217.81	0.0300	Main Channel
69.13	217.81	0.0300	Main Channel
114.13	217.81	0.0300	Main Channel
145.13	219.21	0.0300 / 0.0450	Main Channel
120.06	219.25	0.0450	
123.70	219.32	0.0450	
127.34	219.77	0.0450	
130.98	219.88	0.0450	
134.61	219.99	0.0450	
350.00	220.18	0.0450	
360.18	225.00	0.0450	

TRAVEL TIME TABLE					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.35	218.16	108E+06	9.0	0.29	200.52
0.70	218.51	216E+06	28.4	0.45	126.89
1.05	218.86	325E+06	55.7	0.59	97.27
1.40	219.21	434E+06	89.7	0.71	80.65
1.79	219.60	574E+06	135.9	0.81	70.36
2.17	219.98	726E+06	190.8	0.90	63.38
2.56	220.37	111E+07	264.3	0.82	69.82
2.94	220.75	157E+07	371.7	0.81	70.36
3.33	221.14	203E+07	503.4	0.85	67.30
3.72	221.53	250E+07	656.6	0.90	63.40
4.10	221.91	296E+07	829.6	0.96	59.54
4.49	222.30	343E+07	1021.1	1.02	56.00
4.87	222.68	390E+07	1230.2	1.08	52.83
5.26	223.07	437E+07	1456.2	1.14	50.01
5.65	223.46	484E+07	1698.4	1.20	47.51
6.03	223.84	531E+07	1956.3	1.26	45.28
6.42	224.23	579E+07	2229.3	1.32	43.28
6.80	224.61	626E+07	2519.0	1.38	41.48
7.19	225.00	674E+07	2819.2	1.44	39.85

hydrograph						<-pipe / channel->	
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL		
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW : ID= 2 (8184) *****	105.68	13.25	24.77	1.53	0.74		
OUTFLOW: ID= 1 (6244) *****	101.74	14.25	24.77	1.50	0.73		

CALIB (0244) | Area (ha)=1908.71 | Curve Number (CN)= 61.0
 NASHVD (0244) | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 2.20

Unit Hyd Qpeak (cms)= 14.812
 PEAK FLOW (cms)= 9.643 (1)
 TIME TO PEAK (hrs)= 9.750
 RUNOFF VOLUME (mm)= 20.057
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.274

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8186) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0244): 1908.71 9.643 9.75 20.06
 + ID2= 2 (6244): 20340.16 101.742 14.25 24.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHVD (0442) Area (ha)= 117.26 Curve Number (CN)= 43.0 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 U.H. Tp(hrs)= 1.17

Unit Hyd Qpeak (cms)= 1.705

PEAK FLOW (cms)= 0.538 (i) TIME TO PEAK (hrs)= 8.000 RUNOFF VOLIME (mm)= 11.364 TOTAL RAINFALL (mm)= 73.100 RUNOFF COEFFICIENT = 0.155

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHVD (0440) Area (ha)= 226.35 Curve Number (CN)= 60.0 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 U.H. Tp(hrs)= 1.11

Unit Hyd Qpeak (cms)= 3.481

PEAK FLOW (cms)= 1.870 (i) TIME TO PEAK (hrs)= 7.500 RUNOFF VOLIME (mm)= 19.362 TOTAL RAINFALL (mm)= 73.100 RUNOFF COEFFICIENT = 0.265

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHVD (0438) Area (ha)= 130.70 Curve Number (CN)= 51.0 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 U.H. Tp(hrs)= 0.60

Unit Hyd Qpeak (cms)= 3.738

PEAK FLOW (cms)= 1.306 (i) TIME TO PEAK (hrs)= 6.750 RUNOFF VOLIME (mm)= 14.532 TOTAL RAINFALL (mm)= 73.100 RUNOFF COEFFICIENT = 0.199

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHVD (0436) Area (ha)= 187.51 Curve Number (CN)= 55.0 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 U.H. Tp(hrs)= 0.73

Unit Hyd Qpeak (cms)= 4.391

PEAK FLOW (cms)= 1.830 (i) TIME TO PEAK (hrs)= 7.000 RUNOFF VOLIME (mm)= 16.534 TOTAL RAINFALL (mm)= 73.100 RUNOFF COEFFICIENT = 0.226

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHVD (0434) Area (ha)= 56.64 Curve Number (CN)= 46.0 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50 U.H. Tp(hrs)= 0.52

Unit Hyd Qpeak (cms)= 1.867

PEAK FLOW (cms)= 0.537 (i) TIME TO PEAK (hrs)= 6.500 RUNOFF VOLIME (mm)= 12.320 TOTAL RAINFALL (mm)= 73.100 RUNOFF COEFFICIENT = 0.169

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8206) 1 + 2 = 3 AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm) ID1= 1 (0434): 56.64 0.537 6.50 12.32

+ ID2= 2 (0436): 187.51 1.830 7.00 16.53 ID = 3 (8206): 244.15 2.351 6.75 15.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CUN (6438) IN= 2--> OUT= 1 Routing time step (min)'= 15.00

Table with columns: Distance, Elevation, Manning. Rows show data for section 4381.0.

TRAVEL TIME TABLE with columns: DEPTH (m), ELEV (m), VOLUME (cu.m.), FLOW RATE (cms), VELOCITY (m/s), TRAV.TIME (min). Rows show data for various depths and elevations.

Summary table with columns: AREA (ha), QPEAK (cms), TPEAK (hrs), R.V. (mm), MAX DEPTH (m), MAX VEL (m/s). Rows for INFLOW and OUTFLOW.

ADD HYD (8208) 1 + 2 = 3 AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm) ID1= 1 (0438): 130.70 1.306 6.75 14.53 + ID2= 2 (6438): 244.15 2.063 7.75 15.56 ID = 3 (8208): 374.85 3.183 7.25 15.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8210) 1 + 2 = 3 AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm) ID1= 1 (0440): 226.35 1.870 7.50 19.36 + ID2= 2 (8208): 374.85 3.183 7.25 15.20 ID = 3 (8210): 601.20 5.052 7.50 16.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHVD (0432) Area (ha)= 114.21 Curve Number (CN)= 53.0 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50

U.H. Tp(hrs)= 1.21

Unit Hyd Qpeak (cms)= 1.617

PEAK FLOW (cms)= 0.714 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 15.687
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.215

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0430) Area (ha)= 111.64 Curve Number (CN)= 55.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.52

Unit Hyd Qpeak (cms)= 3.687

PEAK FLOW (cms)= 1.425 (i)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 16.354
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.224

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0428) Area (ha)= 50.53 Curve Number (CN)= 51.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.50

Unit Hyd Qpeak (cms)= 1.725

PEAK FLOW (cms)= 0.582 (i)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 14.436
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.197

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0426) Area (ha)= 247.17 Curve Number (CN)= 61.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.98

Unit Hyd Qpeak (cms)= 4.315

PEAK FLOW (cms)= 2.322 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 19.908
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.272

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0424) Area (ha)= 49.57 Curve Number (CN)= 59.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.37

Unit Hyd Qpeak (cms)= 2.293

PEAK FLOW (cms)= 0.900 (i)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 18.121
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.248

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8198) 1 + 2 = 3 AREA QPEAK TPEAK R.V.
ID= 1 (0424): 49.57 0.900 6.50 18.12
+ ID= 2 (0426): 247.17 2.322 7.25 19.91
ID = 3 (8198): 296.74 3.036 6.75 19.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6428) Routing time step (min)'= 15.00
IN= 2--> OUT= 1

DATA FOR SECTION (4281.0)

Table with columns: Distance, Elevation, Manning. Lists data points for the routing section.

TRAVEL TIME TABLE

Table with columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV. TIME. Shows travel time data for various depths and elevations.

Summary table with columns: AREA, QPEAK, TPEAK, R.V., MAX DEPTH, MAX VEL. Shows inflow and outflow characteristics.

ADD HYD (8200) 1 + 2 = 3 AREA QPEAK TPEAK R.V.
ID= 1 (0428): 50.53 0.582 6.50 14.44
+ ID= 2 (6428): 296.74 2.916 7.25 19.61
ID = 3 (8200): 347.27 3.410 7.25 18.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8202) 1 + 2 = 3 AREA QPEAK TPEAK R.V.
ID= 1 (0430): 111.64 1.425 6.50 16.35
+ ID= 2 (8200): 347.27 3.410 7.25 18.86
ID = 3 (8202): 458.91 4.703 7.00 18.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6432) Routing time step (min)'= 15.00
IN= 2--> OUT= 1

DATA FOR SECTION (4321.0)

Table with columns: Distance, Elevation, Manning. Lists data points for the routing section.

114.66	220.09	0.0600	
118.48	219.73	0.0600	
120.80	219.70	0.0600 / 0.0350	Main Channel
121.30	219.20	0.0350	Main Channel
122.30	219.20	0.0350	Main Channel
122.80	219.70	0.0350 / 0.0600	Main Channel
129.95	220.11	0.0600	
175.81	220.24	0.0600	
214.03	220.55	0.0600	
252.25	220.61	0.0600	
290.47	220.70	0.0600	
328.69	221.04	0.0600	
347.80	221.08	0.0600	
378.37	221.04	0.0600	

514.60	219.09	0.0350
660.00	219.19	0.0350
661.63	221.00	0.0350

TRAVEL TIME TABLE						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	(min)
0.12	218.62	1.00E+04	0.2	0.28	92.89	
0.25	218.75	2.05E+04	0.6	0.43	60.09	
0.37	218.87	3.15E+04	1.1	0.55	46.99	
0.50	219.00	4.30E+04	1.8	0.66	39.67	
0.63	219.13	5.41E+04	2.4	0.77	34.99	
0.77	219.27	6.53E+04	3.1	0.88	31.14	
0.90	219.40	7.64E+04	3.8	0.99	28.02	
1.03	219.53	8.73E+04	4.5	1.10	25.45	
1.17	219.67	9.80E+04	5.2	1.21	23.30	
1.30	219.80	1.07E+05	6.0	1.32	21.60	
1.43	219.93	1.17E+05	6.8	1.43	20.25	
1.57	220.07	1.27E+05	7.6	1.54	19.15	
1.70	220.20	1.37E+05	8.4	1.65	18.25	
1.83	220.33	1.47E+05	9.2	1.76	17.50	
1.97	220.47	1.57E+05	10.0	1.87	16.85	
2.10	220.60	1.67E+05	10.8	1.98	16.25	
2.23	220.73	1.77E+05	11.6	2.09	15.70	
2.37	220.87	1.87E+05	12.4	2.20	15.20	
2.50	221.00	1.97E+05	13.2	2.31	14.75	

TRAVEL TIME TABLE						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	(min)
0.08	219.28	1.85E+03	0.0	0.23	149.38	
0.17	219.37	3.99E+03	0.1	0.34	100.62	
0.25	219.45	6.41E+03	0.1	0.42	80.98	
0.33	219.53	9.11E+03	0.2	0.49	69.79	
0.42	219.62	1.21E+04	0.3	0.55	62.35	
0.50	219.70	1.54E+04	0.5	0.60	56.91	
0.61	219.81	2.07E+04	0.7	0.56	61.28	
0.71	219.91	2.71E+04	1.0	0.30	112.96	
0.82	220.02	3.49E+04	1.4	0.28	122.71	
0.92	220.12	4.42E+04	1.9	0.31	109.98	
1.03	220.23	5.51E+04	2.5	0.33	102.05	
1.14	220.34	6.76E+04	3.2	0.39	87.59	
1.24	220.44	8.18E+04	4.0	0.44	77.01	
1.35	220.55	9.77E+04	4.9	0.49	69.22	
1.46	220.66	1.15E+05	5.9	0.51	66.48	
1.56	220.76	1.34E+05	7.0	0.55	62.06	
1.67	220.87	1.54E+05	8.2	0.59	57.56	
1.77	220.97	1.76E+05	9.5	0.64	53.77	
1.88	221.08	2.00E+05	1.1	0.65	52.70	

<--- hydrograph ---> <--- pipe / channel --->						
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	(m/s)
INFLOW: ID= 2 (8202)	458.91	4.70	7.00	18.25	0.86	0.29
OUTFLOW: ID= 1 (6432)	458.91	3.07	9.25	18.25	0.80	0.28

ADD HYD (8204)						
1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0432)	:	114.21	0.714	8.00	15.69	
+ ID2= 2 (6432)	:	458.91	3.071	9.25	18.25	
ID = 3 (8204)	:	573.12	3.717	9.00	17.74	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8212)						
1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8204)	:	573.12	3.717	9.00	17.74	
+ ID2= 2 (8210)	:	601.20	5.052	7.50	16.77	
ID = 3 (8212)	:	1174.32	8.390	8.00	17.24	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6442)
IN= 2--> OUT= 1 | Routing time step (min)= 15.00

DATA FOR SECTION (4421.0)			
Distance	Elevation	Manning	
0.00	221.00	0.0350	
46.73	220.80	0.0350	
53.47	220.60	0.0350	
66.83	220.40	0.0350	
80.20	220.20	0.0350	
347.52	220.00	0.0350	
354.21	219.59	0.0350	
360.89	219.50	0.0350	
367.57	219.08	0.0350	
368.76	219.00	0.0350 / 0.0300	Main Channel
369.26	218.50	0.0300	Main Channel
374.26	218.50	0.0300	Main Channel
374.76	219.00	0.0300 / 0.0350	Main Channel
380.94	220.33	0.0350	
387.62	219.62	0.0350	
394.31	219.23	0.0350	
454.45	219.07	0.0350	

<--- hydrograph ---> <--- pipe / channel --->						
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	(m/s)
INFLOW: ID= 2 (8212)	1174.32	8.39	8.00	17.24	0.70	0.35
OUTFLOW: ID= 1 (6442)	1174.32	7.28	9.50	17.24	0.68	0.35

ADD HYD (8214)						
1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0442)	:	117.26	0.538	8.00	11.36	
+ ID2= 2 (6442)	:	1174.32	7.285	9.50	17.24	
ID = 3 (8214)	:	1291.58	7.748	9.50	16.71	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8196)						
1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8194)	:	25632.58	119.935	13.50	23.88	
+ ID2= 2 (8214)	:	1291.58	7.748	9.50	16.71	
ID = 3 (8196)	:	26924.15	124.675	13.50	23.54	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB						
NASHVD	(0444)	Area	(ha)	= 221.65	Curve Number	(CN)= 56.0
ID= 1	DT=15.0 min	Ia	(mm)	= 5.00	# of Linear Res.	(N)= 1.50
U.H. Tp	(hrs)	= 1.03				

Unit Hyd Qpeak (cms) = 3.656
PEAK FLOW (cms) = 1.705 (1)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 17.158
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.235

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8216)						
1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0444)	:	221.65	1.705	7.50	17.16	
+ ID2= 2 (8196)	:	26924.15	124.675	13.50	23.54	
ID = 3 (8216)	:	27145.80	125.177	13.50	23.48	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

EXISTING
EAST HOLLAND RIVER 50-YEAR STORM

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V V I SSSS U U A L
V V I SS U U A L
V V I SS U U AAAA L
V V I SS U U A A L
VV I SSSS UUUU A A LLLL

OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y MM MM O O
O O T T H H Y Y M M O O Company
OOO T T H H Y Y M M OOO Serial
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***** DETAILED OUTPUT *****

Input filename: C:\Program Files\Visual Octhymo 2.4r\VO2\voindat
Output filename: C:\Users\jscott\AppData\Local\Temp\ed93b362-511c-49a6-add4-43ba8075dbf0\Scenario.out
Summary filename: C:\Users\jscott\AppData\Local\Temp\ed93b362-511c-49a6-add4-43ba8075dbf0\Scenario.sum

DATE: 08/22/2012 TIME: 01:28:33

USER:

COMMENTS: _____

** SIMULATION NUMBER: 1 **

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File name: C:\Users\jscott\AppData\Local\Temp\ed93b362-511c-49a6-add4-43ba8075dbf0\Sb314efd
Comments: 50-Year 12-Hour SCS II Design Storm

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	2.02	3.25	3.23	6.25	14.54	9.25	2.83
0.50	2.02	3.50	3.23	6.50	14.54	9.50	2.83
0.75	2.02	3.75	3.23	6.75	6.46	9.75	2.83
1.00	2.02	4.00	3.23	7.00	6.46	10.00	2.83
1.25	2.02	4.25	4.85	7.25	4.85	10.25	1.62
1.50	2.02	4.50	4.85	7.50	4.85	10.50	1.62
1.75	2.02	4.75	6.46	7.75	4.85	10.75	1.62
2.00	2.02	5.00	6.46	8.00	4.85	11.00	1.62
2.25	2.42	5.25	9.70	8.25	2.83	11.25	1.62
2.50	2.42	5.50	9.70	8.50	2.83	11.50	1.62
2.75	2.42	5.75	38.78	8.75	2.83	11.75	1.62
3.00	2.42	6.00	106.66	9.00	2.83	12.00	1.62

CALIB
STANDHYD (1662) Area (ha)= 158.10
ID= 1 DT=15.0 min Total Imp(%)= 58.00 Dir. Conn.(%)= 30.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 91.70 66.40
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1026.65 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 106.66 116.05
over (min) 15.00 30.00
Storage Coeff. (min)= 12.40 (ii) 22.48 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04
TOTALS
PEAK FLOW (cms)= 11.04 10.39 17.879 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 78.80 45.11 55.22
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.56 0.68

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.0 Is = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (1661) Area (ha)= 276.60
ID= 1 DT=15.0 min Total Imp(%)= 40.00 Dir. Conn.(%)= 21.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 110.64 165.96
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1357.94 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 106.66 81.32
over (min) 15.00 30.00
Storage Coeff. (min)= 14.66 (ii) 26.28 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.07 0.04
TOTALS
PEAK FLOW (cms)= 12.66 16.77 23.436 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 78.80 39.97 48.12
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.49 0.60

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.0 Is = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5166)
ID= 2 --> OUT= 1
DT= 15.0 min

	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	0.0000	6.0870	6.1196
0.3750	3.2753	7.7230	7.0042	
1.3370	3.4678	9.3080	7.8876	
2.9800	4.4433	9.7080	7.8976	
4.4520	5.2716	0.0000	0.0000	

AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID= 2 (1661) 276.600 23.436 6.00 48.12
OUTFLOW: ID= 1 (5166) 276.600 7.630 7.00 48.11
PEAK FLOW REDUCTION [Qout/Qin]= 32.56
TIME SHIFT OF PEAK FLOW (min)= 60.00
MAXIMUM STORAGE USED (ha.m.)= 6.9542

ADD HYD (8134)
1 + 2 + 3
ID= 1 (1662): 158.10 17.879 6.00 55.22
+ ID2= 2 (5166): 276.60 7.630 7.00 48.11

ID= 3 (8134): 434.70 18.852 6.25 50.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (1642) Area (ha)= 132.50
ID= 1 DT=15.0 min Total Imp(%)= 20.00 Dir. Conn.(%)= 15.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 26.50 106.00
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 939.86 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 106.66 56.08
over (min) 15.00 30.00
Storage Coeff. (min)= 11.76 (ii) 25.24 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04
TOTALS
PEAK FLOW (cms)= 4.72 7.49 9.449 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 78.80 34.25 40.93

TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.42 0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (1641) | Area (ha)= 101.70
 ID= 1 DT=15.0 min | Total Imp(%)= 70.00 Dir. Conn.(%)= 52.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	71.19	30.51
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	823.41	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)=	106.66	106.84
cover (min)	15.00	30.00
Storage Coeff. (min)=	10.86 (ii)	21.28 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.05
TOTALS		
PEAK FLOW (cms)=	12.89	4.50
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	78.80	43.28
TOTAL RAINFALL (mm)=	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.54
		0.76

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5164) |
 ID= 2 -> OUT= 1 |
 DT= 15.0 min |

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	7.6100	4.1638
	0.3440	1.7056	8.2090	4.5053
	3.1340	1.9730	8.8070	4.8468
	4.1310	3.0607	9.2070	4.8568
	5.1070	3.7142	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (1641)	101.700	15.841	6.00	61.75
OUTFLOW: ID= 1 (5164)	101.700	4.257	6.75	61.74

PEAK FLOW REDUCTION [Qout/qin](%)= 26.87
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 3.1814

ADD HYD (8132) |
 1 + 2 = 3 |

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1642):	132.50	9.449	6.00	40.93
+ ID2= 2 (5164):	101.70	4.257	6.75	61.74
ID = 3 (8132):	234.20	13.291	6.25	49.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0162) | Area (ha)= 190.14 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.79

Unit Hyd Qpeak (cms)= 4.104
 PEAK FLOW (cms)= 2.781 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 25.895
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.320

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0160) | Area (ha)= 318.82 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.45

Unit Hyd Qpeak (cms)= 3.754
 PEAK FLOW (cms)= 2.679 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 23.980
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.297

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0152) | Area (ha)= 124.37 Curve Number (CN)= 59.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.89

Unit Hyd Qpeak (cms)= 2.372
 PEAK FLOW (cms)= 1.418 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 22.498
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.278

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0148) | Area (ha)= 417.89 Curve Number (CN)= 26.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.18

Unit Hyd Qpeak (cms)= 6.067
 PEAK FLOW (cms)= 1.194 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 7.136
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.088

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0150) | Area (ha)= 105.88 Curve Number (CN)= 17.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.58

Unit Hyd Qpeak (cms)= 3.138
 PEAK FLOW (cms)= 0.310 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 4.266
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.053

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8116) |
 1 + 2 = 3 |

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0148):	417.89	1.194	8.00	7.14
+ ID2= 2 (0150):	105.88	0.310	6.75	4.27
ID = 3 (8116):	523.77	1.451	7.50	6.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6152) |
 IN= 2 -> OUT= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (1521.0) ----->

Distance	Elevation	Manning
0.00	290.00	0.0500
15.34	286.87	0.0500
46.03	284.84	0.0500
88.22	283.07	0.0500
126.57	282.11	0.0500

153.42	280.34	0.0500	
157.26	280.07	0.0500	
161.09	279.72	0.0500	
167.76	278.94	0.0500 / 0.0300	Main Channel
168.01	278.60	0.0300	Main Channel
168.76	278.60	0.0300	Main Channel
169.51	278.60	0.0300	Main Channel
169.76	278.90	0.0300 / 0.0500	Main Channel
173.76	279.40	0.0500	
180.27	279.69	0.0500	
184.11	279.93	0.0500	
218.62	281.86	0.0500	
260.82	283.20	0.0500	
360.54	289.25	0.0500	
379.72	289.80	0.0500	

CALIB (0154) | Area (ha)= 200.32 | Curve Number (CN)= 37.0
 NASHYD (0154) | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.97

Unit Hyd Qpeak (cms)= 3.537

PEAK FLOW (cms)= 1.044 (1)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 11.183
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.138

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-----><----- TRAVEL TIME TABLE -----><-----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.30	278.90	.114E+04	0.4	0.80	45.70
0.87	279.47	.932E+04	4.2	0.98	37.22
1.45	280.05	.353E+05	17.5	1.09	33.68
2.02	280.62	.831E+05	47.8	1.26	29.01
2.59	281.19	.155E+06	102.9	1.46	25.06
3.17	281.77	.250E+06	189.3	1.66	22.03
3.74	282.34	.375E+06	298.5	1.75	20.91
4.32	282.92	.547E+06	456.5	1.83	19.98
4.89	283.49	.767E+06	705.7	2.02	18.12
5.46	284.06	.102E+07	1038.0	2.24	16.34
6.04	284.64	.130E+07	1444.1	2.45	14.98
6.61	285.21	.161E+07	1940.1	2.66	13.79
7.18	285.78	.194E+07	2522.6	2.86	12.80
7.76	286.36	.229E+07	3187.4	3.06	11.98
8.33	286.93	.267E+07	3942.3	3.25	11.28
8.91	287.51	.306E+07	4824.9	3.46	10.58
9.48	288.08	.347E+07	5795.7	3.67	9.99
10.05	288.65	.390E+07	6855.4	3.86	9.48
10.63	289.23	.434E+07	8005.0	4.05	9.04

-----><----- hydrograph -----><-----><----- pipe / channel -----><-----

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8116)	523.77	1.363	8.50	6.56	0.45
OUTFLOW: ID= 1 (6152)	523.77	1.36	8.50	6.56	0.44

-----><----- ADD HYD (8122) -----><-----

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (0152)	124.37	1.418	7.00	22.50
+ ID= 2 (6152)	523.77	1.363	8.50	6.56
ID= 3 (8122)	648.14	2.671	8.00	9.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----><----- CALIB (0158) -----><-----

CALIB (0158) Area (ha)= 178.59 Curve Number (CN)= 61.0
NASHYD (0158) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.94

Unit Hyd Qpeak (cms)= 3.230

PEAK FLOW (cms)= 2.076 (1)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 23.854
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.295

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-----><----- CALIB (0156) -----><-----

CALIB (0156) Area (ha)= 83.49 Curve Number (CN)= 45.0
NASHYD (0156) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min U.H. Tp(hrs)= 0.62

Unit Hyd Qpeak (cms)= 2.306

PEAK FLOW (cms)= 0.812 (1)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 14.566
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.180

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-----><----- ADD HYD (8118) -----><-----

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (0154)	200.32	1.044	7.25	11.18
+ ID= 2 (0156)	83.49	0.812	6.75	14.57
ID= 3 (8118)	283.81	1.829	7.00	12.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----><----- ROUTE CHN (6158) -----><-----

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 2 -> QPE= 1				
Routing time step (min)= 15.00				

-----><----- DATA FOR SECTION (1581.0) -----><-----

Distance (m)	Elevation (m)	Manning
0.00	290.00	0.0400
19.20	288.88	0.0400
48.00	288.26	0.0400
119.99	283.66	0.0400
167.99	282.07	0.0400
196.79	281.32	0.0400
210.79	280.90	0.0400
219.79	280.38	0.0400 / 0.0300
220.04	279.65	0.0300
220.79	279.65	0.0300
221.54	279.65	0.0300
221.79	280.39	0.0300 / 0.0400
225.79	280.59	0.0400
235.19	280.88	0.0400
254.39	281.50	0.0400
273.58	282.56	0.0400
302.38	283.42	0.0400
359.98	284.03	0.0400
416.77	289.37	0.0400
475.17	290.37	0.0400

-----><----- TRAVEL TIME TABLE -----><-----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.37	280.01	.973E+03	0.5	0.86	31.78
0.73	280.38	.210E+04	1.5	1.18	23.18
1.30	280.95	.149E+05	8.6	0.95	28.91
1.86	281.51	.579E+05	40.0	1.13	24.12
2.43	282.08	.133E+06	118.5	1.46	18.75
2.99	282.64	.236E+06	254.6	1.77	15.46
3.56	283.21	.369E+06	450.3	2.00	13.67
4.13	283.78	.542E+06	702.7	2.13	12.86
4.69	284.34	.767E+06	1125.2	2.41	11.36
5.26	284.91	.101E+07	1697.1	2.75	9.93
5.82	285.47	.127E+07	2379.1	3.07	8.91
6.39	286.04	.155E+07	3171.4	3.36	8.14
6.95	286.60	.184E+07	4075.5	3.64	7.53
7.52	287.17	.215E+07	5093.1	3.89	7.03
8.09	287.74	.247E+07	6226.5	4.13	6.62
8.65	288.30	.281E+07	7461.2	4.36	6.28
9.22	288.87	.318E+07	8635.1	4.46	6.13
9.78	289.43	.357E+07	10143.6	4.67	5.86
10.35	290.00	.398E+07	11608.8	4.79	5.71

-----><----- hydrograph -----><-----><----- pipe / channel -----><-----

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8118)	283.81	1.83	7.00	12.18	0.76
OUTFLOW: ID= 1 (6158)	283.81	1.77	7.50	12.18	0.75

-----><----- ADD HYD (8120) -----><-----

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (0158)	178.59	2.076	7.25	23.85
+ ID= 2 (6158)	283.81	1.766	7.50	12.18

ID = 3 (8120): 462.40 3.829 7.25 16.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8124)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8120):	462.40	3.829	7.25	16.69
+ ID2= 2 (8122):	648.14	2.671	8.00	9.61
ID = 3 (8124):	1110.54	6.455	7.50	12.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6160)
 IN= 2--> OUT= 1 Routing time step (min)= 15.00

DATA FOR SECTION (1601.0) -----				
Distance	Elevation	Manning		
0.00	261.02	0.0400		
29.89	254.30	0.0400		
40.75	252.58	0.0400		
62.49	251.30	0.0400		
114.11	250.94	0.0400		
130.41	248.58	0.0400		
141.28	247.50	0.0400		
154.86	246.87	0.0400		
155.20	246.70	0.0400		
160.20	246.30	0.0400 / 0.0300	Main Channel	
160.30	245.30	0.0300	Main Channel	
165.20	245.30	0.0300	Main Channel	
165.30	246.30	0.0300 / 0.0400	Main Channel	
168.45	246.96	0.0400		
195.52	249.20	0.0400		
203.77	250.82	0.0400		
225.50	256.78	0.0400		
244.52	261.14	0.0400		
255.39	261.95	0.0400		
268.97	261.92	0.0400		

TRAVEL TIME TABLE -----					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.50	245.80	.120E+05	2.9	1.18	68.31
1.00	246.30	.242E+05	8.5	1.70	47.47
1.92	247.22	.825E+05	31.7	1.86	43.34
2.84	248.14	.248E+06	102.5	2.00	40.39
3.76	249.06	.506E+06	247.7	2.37	34.03
4.68	249.98	.832E+06	486.8	2.83	28.49
5.60	250.90	.121E+07	814.9	3.27	24.71
6.52	251.82	.180E+07	1157.1	3.11	25.97
7.44	252.74	.253E+07	1840.2	3.52	22.92
8.36	253.66	.331E+07	2727.1	3.98	20.25
9.29	254.59	.414E+07	3782.6	4.43	18.22
10.21	255.51	.499E+07	5010.7	4.86	16.61
11.13	256.43	.589E+07	6397.4	5.26	15.33
12.05	257.35	.681E+07	7932.8	5.64	14.31
12.97	258.27	.777E+07	9621.2	5.99	13.46
13.89	259.19	.877E+07	11469.4	6.33	12.74
14.81	260.11	.980E+07	13478.4	6.66	12.12
15.73	261.03	.109E+08	15651.7	6.97	11.58
16.65	261.95	.120E+08	17492.5	7.07	11.42

hydrograph		pipe / channel	
AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8124) 1110.54	6.46	7.50	12.56
OUTFLOW : ID= 1 (6160) 1110.54	5.81	8.50	12.56

ADD HYD (8126)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0160):	318.82	2.679	8.00	23.98
+ ID2= 2 (6160):	1110.54	5.809	8.50	12.56
ID = 3 (8126):	1429.36	8.460	8.50	15.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8128)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0162):	190.14	2.781	7.00	25.90

+ ID2= 2 (8126): 1429.36 8.460 8.50 15.11

ID = 3 (8128): 1619.50 10.731 8.25 16.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8130)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8128):	1619.50	10.731	8.25	16.37
+ ID2= 2 (8132):	234.20	13.271	6.25	49.97
ID = 3 (8130):	1853.70	18.397	6.25	20.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6166)
 IN= 2--> OUT= 1 Routing time step (min)= 15.00

DATA FOR SECTION (1661.0) -----				
Distance	Elevation	Manning		
0.00	248.36	0.0500		
41.67	246.98	0.0500		
95.82	244.93	0.0500		
149.98	242.42	0.0500		
191.64	241.08	0.0500		
224.97	239.30	0.0500		
229.13	239.35	0.0500		
233.30	237.77	0.0500		
237.47	236.57	0.0500		
240.63	235.60	0.0500 / 0.0300	Main Channel	
241.63	234.10	0.0300	Main Channel	
247.13	234.10	0.0300	Main Channel	
247.53	235.60	0.0300 / 0.0500	Main Channel	
254.13	236.25	0.0500		
266.63	236.77	0.0500		
283.29	237.84	0.0500		
291.63	238.89	0.0500		
329.12	244.66	0.0500		
370.78	249.55	0.0500		
412.44	253.17	0.0500		

TRAVEL TIME TABLE -----					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.75	234.85	.130E+05	6.0	1.36	36.45
1.50	235.60	.276E+05	18.1	1.95	25.41
2.25	236.35	.544E+05	40.0	2.18	22.69
3.00	237.10	.117E+06	80.1	2.04	24.32
3.75	237.85	.213E+06	148.9	2.07	23.89
4.50	238.60	.334E+06	255.1	2.27	21.85
5.25	239.35	.473E+06	395.8	2.49	19.90
6.00	240.10	.631E+06	563.8	2.65	18.66
6.75	240.85	.824E+06	773.2	2.79	17.77
7.50	241.60	.106E+07	1037.9	2.92	16.97
8.26	242.36	.135E+07	1326.7	2.92	16.94
9.01	243.11	.173E+07	1758.5	3.01	16.44
9.76	243.86	.217E+07	2285.9	3.13	15.81
10.51	244.61	.265E+07	2909.5	3.26	15.17
11.26	245.36	.318E+07	3610.1	3.37	14.67
12.01	246.11	.377E+07	4417.4	3.48	14.21
12.76	246.86	.441E+07	5348.0	3.60	13.75
13.51	247.61	.512E+07	6394.0	3.71	13.34
14.26	248.36	.589E+07	7580.3	3.82	12.95

hydrograph		pipe / channel	
AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8130) 1853.70	18.40	6.25	20.62
OUTFLOW : ID= 1 (6166) 1853.70	15.72	7.00	20.62

ADD HYD (8136)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6166):	1853.70	15.721	7.00	20.62
+ ID2= 2 (8134):	434.70	18.852	6.25	50.70
ID = 3 (8136):	2288.40	30.030	6.50	26.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALB
 STANDHYD (2021) Area (ha)= 91.20
 ID= 1 DT=15.0 min Total Imp(%)= 40.00 Dir. Conn.(%)= 24.00

```

-----
Surface Area (ha)= 36.48 54.72
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 779.74 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 106.66 76.55
over (min) 15.00 30.00
Storage Coeff. (min)= 10.51 (ii) 22.41 (iii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.09 0.04

*TOTALS*
PEAK FLOW (cms)= 5.39 5.62 9.016 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 78.80 39.11 48.64
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.48 0.60

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
RESERVOIR (5202)
IN= 2--> OUT= 1
DT= 15.0 min
-----
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
0.0000 0.0000 | 4.0850 2.2999
0.3100 0.9668 | 5.0850 2.6924
1.2590 1.1373 | 5.9090 3.1012
2.2140 1.5358 | 6.3090 3.1112
3.0620 1.8918 | 0.0000 0.0000
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (2021) 91.200 9.016 6.00 48.64
OUTFLOW: ID= 1 (5202) 91.200 3.409 6.75 48.63

PEAK FLOW REDUCTION [Qout/Qin](%)= 37.81
TIME SHIFT OF PEAK FLOW (min)= 45.00
MAXIMUM STORAGE USED (ha.m.)= 2.0375

```

```

-----
CALIB
STANDHYD (2022)
ID= 1 DT=15.0 min
Total Imp(%)= 57.00 Dir. Conn.(%)= 34.00
-----
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 215.06 162.24
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1585.98 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 106.66 98.05
over (min) 15.00 30.00
Storage Coeff. (min)= 16.09 (ii) 26.87 (iii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.07 0.04

*TOTALS*
PEAK FLOW (cms)= 26.85 19.59 39.504 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 78.80 41.43 54.14
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.51 0.67

```

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 71.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
ADD HYD (8110)
1 + 2 = 3
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID= 1 (2022): 377.30 39.504 6.00 54.14
+ ID2= 2 (5202): 91.20 3.409 6.75 48.63
-----
ID = 3 (8110): 468.50 40.315 6.00 53.06

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
CALIB
STANDHYD (0102)
ID= 1 DT=15.0 min
Area (ha)= 466.00
Total Imp(%)= 23.00 Dir. Conn.(%)= 9.00
-----
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 107.18 358.82
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1762.57 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 106.66 20.93
over (min) 15.00 45.00
Storage Coeff. (min)= 17.15 (ii) 37.14 (iii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.07 0.03

*TOTALS*
PEAK FLOW (cms)= 8.53 11.62 14.506 (iii)
TIME TO PEAK (hrs)= 6.00 6.50 6.50
RUNOFF VOLUME (mm)= 78.80 19.53 24.86
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.24 0.31

```

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 49.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB
NASHYD (0100)
ID= 1 DT=15.0 min
Area (ha)= 693.84 Curve Number (CN)= 40.0
Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.95
-----
Unit Hyd Qpeak (cms)= 6.087

PEAK FLOW (cms)= 2.384 (i)
TIME TO PEAK (hrs)= 9.250
RUNOFF VOLUME (mm)= 12.530
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.155

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
ADD HYD (8000)
1 + 2 = 3
-----
ID= 1 (0100): 693.84 2.384 9.25 12.53
+ ID2= 2 (0100): 466.00 14.506 6.50 24.86
-----
ID = 3 (8000): 1159.84 16.049 6.50 17.48

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ROUTE CHN (6104)
IN= 2--> OUT= 1
Routing time step (min)= 15.00
-----
<----- DATA FOR SECTION (1041.0) ----->
Distance Elevation Manning
0.00 330.16 0.0450
50.03 328.36 0.0450
89.94 326.33 0.0450
117.94 324.30 0.0450
160.82 322.75 0.0450
178.34 319.96 0.0450
183.34 319.81 0.0450
184.34 319.38 0.0450 /0.0300 Main Channel
185.09 318.78 0.0300 Main Channel
185.84 318.72 0.0300 Main Channel
186.84 319.32 0.0300 /0.0450 Main Channel
193.84 319.70 0.0450
198.84 320.38 0.0450
200.13 320.81 0.0450
218.00 322.49 0.0450
239.44 323.49 0.0450
250.17 323.96 0.0450
275.18 325.20 0.0450
310.92 326.47 0.0450
353.81 330.00 0.0450

```

```

-----
TRAVEL TIME TABLE
----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.30 319.02 .624E+03 0.1 0.45 73.00

```

0.60	319.32	.181E+04	0.6	0.67	49.46
1.23	319.95	.128E+05	4.9	0.77	43.38
1.86	320.58	.396E+05	19.1	0.96	34.50
2.48	321.20	.757E+05	43.5	1.15	28.98
3.11	321.83	.135E+06	81.2	1.30	25.58
3.74	322.46	.187E+06	135.9	1.45	22.92
4.37	323.09	.269E+06	195.6	1.45	22.90
5.00	323.72	.387E+06	287.8	1.48	22.44
5.63	324.35	.545E+06	426.4	1.56	21.22
6.25	324.97	.735E+06	630.5	1.71	19.42
6.88	325.60	.952E+06	876.1	1.83	18.12
7.51	326.23	.120E+07	1177.0	1.95	17.03
8.14	326.86	.149E+07	1554.4	2.08	15.93
8.77	327.49	.179E+07	2009.7	2.23	14.88
9.40	328.12	.213E+07	2533.9	2.37	13.99
10.02	328.74	.249E+07	3112.6	2.49	13.31
10.65	329.37	.288E+07	3760.9	2.60	12.75
11.28	330.00	.330E+07	4494.8	2.71	12.23

30.15	324.28	0.0400			
39.27	320.35	0.0400			
72.37	317.60	0.0400			
87.45	314.52	0.0400			
93.48	313.91	0.0400			
105.54	313.55	0.0400			
106.17	313.40	0.0400 / 0.0300	Main Channel		
110.62	313.04	0.0300	Main Channel		
110.82	312.97	0.0300	Main Channel		
111.57	313.08	0.0300	Main Channel		
111.67	313.48	0.0300 / 0.0400	Main Channel		
129.66	316.62	0.0400			
150.77	318.95	0.0400			
180.92	319.61	0.0400			
205.04	321.23	0.0400			
232.18	322.09	0.0400			
268.37	322.33	0.0400			
298.52	326.00	0.0400			

```

<---- hydrograph ----> <-pipe / channel->
AREA      QPEAK  TPEAK  R.V.  MAX DEPTH  MAX VEL
(ha)      (cms)  (hrs)  (mm)  (m)         (m/s)
INFLOW : ID= 2 (8000) 1159.84 16.05  6.50 17.48  1.72  0.91
OUTFLOW : ID= 1 (6104) 1159.84 11.71  6.75 17.48  1.53  0.85

```

```

----- TRAVEL TIME TABLE -----
DEPTH  ELEV  VOLUME  FLOW RATE  VELOCITY  TRAV.TIME
(m)    (m)    (cu.m.) (cms)      (m/s)     (min)
0.43  313.40  .244E+04  0.7  0.59  58.83
1.09  314.06  .214E+05  10.4  1.01  34.24
1.76  314.73  .600E+05  40.2  1.39  24.89
2.42  315.39  .109E+06  92.1  1.75  19.80
3.08  316.05  .169E+06  166.5  2.05  16.88
3.75  316.72  .237E+06  264.4  2.31  14.97
4.41  317.38  .318E+06  386.3  2.52  13.72
5.07  318.04  .411E+06  539.3  2.72  12.71
5.74  318.71  .517E+06  725.5  2.91  11.88
6.40  319.37  .642E+06  909.2  2.94  11.77
7.06  320.03  .806E+06  1179.3  3.04  11.38
7.72  320.69  .989E+06  1515.9  3.18  10.87
8.39  321.36  .119E+07  1895.5  3.30  10.49
9.05  322.02  .143E+07  2310.2  3.36  10.30
9.71  322.68  .172E+07  2763.2  3.33  10.39
10.38  323.35  .205E+07  3477.6  3.53  9.81
11.04  324.01  .238E+07  4284.3  3.73  9.27
11.70  324.67  .274E+07  5154.0  3.91  8.85
12.37  325.34  .311E+07  6104.0  4.08  8.48

```

```

-----
| CALIB |
| NASHYD (0104) | Area (ha)= 527.81 Curve Number (CN)= 43.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.34

```

```

Unit Hyd Qpeak (cms)= 6.735
PEAK FLOW (cms)= 2.687 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 13.837
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.171

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8002) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
| ID= 1 (0104): 527.81 2.687 8.00 13.84
| + ID2= 2 (6104): 1159.84 11.706 6.75 17.48
|-----|
| ID = 3 (8002): 1687.65 14.008 6.75 16.34

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0106) | Area (ha)= 259.52 Curve Number (CN)= 55.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.23

```

```

Unit Hyd Qpeak (cms)= 3.614
PEAK FLOW (cms)= 2.065 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 20.106
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.249

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8004) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
| ID= 1 (0106): 259.52 2.065 8.00 20.11
| + ID2= 2 (8002): 1687.65 14.008 6.75 16.34
|-----|
| ID = 3 (8004): 1947.17 15.855 6.75 16.84

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6108) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

```

```

----- DATA FOR SECTION (1081.0) -----
Distance Elevation Manning
0.00 326.06 0.0400
12.06 325.74 0.0400

```

```

<---- hydrograph ----> <-pipe / channel->
AREA      QPEAK  TPEAK  R.V.  MAX DEPTH  MAX VEL
(ha)      (cms)  (hrs)  (mm)  (m)         (m/s)
INFLOW : ID= 2 (8004) 1947.17 15.86  6.75 16.84  1.21  1.06
OUTFLOW : ID= 1 (6108) 1947.17 14.33  7.50 16.84  1.18  1.05

```

```

-----
| CALIB |
| NASHYD (0108) | Area (ha)= 207.05 Curve Number (CN)= 27.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.03

```

```

Unit Hyd Qpeak (cms)= 3.432
PEAK FLOW (cms)= 0.681 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 7.461
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.092

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8006) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
| ID= 1 (0108): 207.05 0.681 7.50 7.46
| + ID2= 2 (6108): 1947.17 14.326 7.50 16.84
|-----|
| ID = 3 (8006): 2154.22 15.008 7.50 15.94

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0110) | Area (ha)= 323.92 Curve Number (CN)= 52.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.35

```

```

Unit Hyd Qpeak (cms)= 4.087
PEAK FLOW (cms)= 2.188 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 18.398
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.228

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ID1= 1 (0112): 414.89 0.982 9.25 8.56
 + ID2= 2 (6112): 2478.14 14.688 8.75 16.26
 =====
 ID = 3 (8010): 2893.03 15.668 8.75 15.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8008) |
| 1 + 2 = 3 |
-----
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0110): 323.92 2.188 8.00 18.40
+ ID2= 2 (8006): 2154.22 15.008 7.50 15.94
=====
ID = 3 (8008): 2478.14 17.144 7.50 16.26
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6112) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)'= 15.00
  
```

----- DATA FOR SECTION (1121.0) -----					
Distance	Elevation	Manning			
0.00	320.59	0.0400			
13.48	317.93	0.0400			
40.45	311.52	0.0400			
64.04	307.68	0.0400			
74.15	307.01	0.0400			
91.00	307.22	0.0400			
107.86	305.21	0.0400			
111.23	304.86	0.0400			
113.32	304.60	0.0400 / 0.0300	Main Channel		
117.77	304.24	0.0300	Main Channel		
119.97	304.17	0.0300	Main Channel		
118.72	304.28	0.0300	Main Channel		
118.82	304.68	0.0300 / 0.0400	Main Channel		
124.71	304.76	0.0400			
171.90	306.30	0.0400			
205.60	307.24	0.0400			
239.31	308.24	0.0400			
262.90	310.64	0.0400			
289.86	314.34	0.0400			
333.68	317.66	0.0400			

----- TRAVEL TIME TABLE -----					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.43	304.60	.372E+04	0.6	0.54	97.65
1.12	305.29	.463E+05	15.5	0.85	61.89
1.80	305.97	.149E+06	53.0	1.13	46.85
2.49	306.66	.311E+06	134.2	1.37	38.62
3.18	307.35	.554E+06	261.6	1.50	35.27
3.87	308.04	.894E+06	496.7	1.74	30.37
4.55	308.72	.129E+07	833.0	2.05	25.75
5.24	309.41	.171E+07	1269.8	2.36	22.39
5.93	310.10	.215E+07	1789.0	2.64	20.01
6.62	310.79	.261E+07	2392.7	2.90	18.21
7.30	311.47	.310E+07	3087.8	3.16	16.74
7.99	312.16	.361E+07	3873.6	3.40	15.52
8.68	312.85	.413E+07	4741.0	3.64	14.52
9.37	313.54	.467E+07	5689.3	3.86	13.69
10.05	314.22	.523E+07	6718.4	4.07	12.97
10.74	314.91	.581E+07	7770.7	4.24	12.46
11.43	315.60	.641E+07	8897.5	4.40	12.01
12.12	316.29	.704E+07	10113.2	4.55	11.61
12.80	316.97	.770E+07	11420.1	4.70	11.24

```

----- hydrograph ----> <-pipe / channel->
AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8008) 2478.14 17.14 7.50 16.26 1.20 0.88
OUTFLOW: ID= 1 (6112) 2478.14 14.69 8.75 16.26 1.15 0.87
  
```

```

-----
| CALIB |
| NASHYD (0112) |
| ID= 1 DT=15.0 min |
| U.H. Tp(hrs)= 1.91 |
-----
Unit Hyd Qpeak (cms)= 3.699
PEAK FLOW (cms)= 0.982 (i)
TIME TO PEAK (hrs)= 9.250
RUNOFF VOLUME (mm)= 8.562
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.106
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8010) |
| 1 + 2 = 3 |
-----
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
  
```

```

-----
| ROUTE CHN (6114) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)'= 15.00
  
```

----- DATA FOR SECTION (1141.0) -----					
Distance	Elevation	Manning			
0.00	312.27	0.0500			
14.07	309.70	0.0500			
28.14	306.51	0.0500			
35.17	305.81	0.0500			
42.20	304.95	0.0500			
52.75	304.24	0.0500			
77.37	303.61	0.0500			
98.47	301.33	0.0500			
131.76	300.40	0.0500 / 0.0300	Main Channel		
136.21	300.04	0.0300	Main Channel		
136.41	299.97	0.0300	Main Channel		
137.16	300.08	0.0300	Main Channel		
137.26	300.48	0.0300 / 0.0500	Main Channel		
140.68	300.53	0.0500			
179.36	302.20	0.0500			
214.53	303.82	0.0500			
242.66	305.27	0.0500			
289.38	307.66	0.0500			
316.52	311.76	0.0500			
348.17	313.48	0.0500			

----- TRAVEL TIME TABLE -----					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.43	300.40	.368E+04	0.7	0.59	88.31
1.05	301.02	.505E+05	12.1	0.75	69.49
1.68	301.65	.165E+06	52.7	0.99	52.28
2.30	302.27	.324E+06	130.3	1.26	41.40
2.93	302.90	.520E+06	247.1	1.48	35.08
3.55	303.52	.754E+06	406.8	1.68	30.91
4.18	304.15	.104E+07	586.1	1.76	29.55
4.80	304.77	.138E+07	856.0	1.93	26.96
5.43	305.40	.177E+07	1201.1	2.12	24.56
6.05	306.02	.219E+07	1612.2	2.30	22.63
6.68	306.65	.264E+07	2090.6	2.47	21.07
7.30	307.27	.313E+07	2653.0	2.64	19.65
7.93	307.90	.364E+07	3303.9	2.83	18.36
8.55	308.52	.417E+07	4053.9	3.03	17.14
9.18	309.15	.471E+07	4871.9	3.22	16.12
9.80	309.77	.527E+07	5756.2	3.41	15.25
10.43	310.40	.584E+07	6699.8	3.58	14.52
11.05	311.02	.642E+07	7710.5	3.74	13.89
11.68	311.65	.702E+07	8788.5	3.90	13.32

```

----- hydrograph ----> <-pipe / channel->
AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8010) 2893.03 15.67 8.75 15.16 1.11 0.76
OUTFLOW: ID= 1 (6114) 2893.03 13.89 10.00 15.16 1.08 0.76
  
```

```

-----
| CALIB |
| NASHYD (0114) |
| ID= 1 DT=15.0 min |
| U.H. Tp(hrs)= 1.12 |
-----
  
```

Unit Hyd Qpeak (cms)= 4.389

```

PEAK FLOW (cms)= 2.028 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 16.745
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.207
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8012) |
| 1 + 2 = 3 |
-----
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0114): 287.65 2.028 7.75 16.74
+ ID2= 2 (6114): 2893.03 13.887 10.00 15.16
=====
ID = 3 (8012): 3180.68 15.462 9.75 15.30
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | CALIB |
 | NASHVD (0116) | Area (ha)= 206.43 Curve Number (CN)= 64.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 | U.H. Tp(hrs)= 0.80

Unit Hyd Qpeak (cms)= 4.394

PEAK FLOW (cms)= 2.988 (1)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 25.903
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.321

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | ADD HYD (8014) |
 | 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 | (ha) (cms) (hrs) (mm)
 | ID1= 1 (0116): 206.43 2.988 7.00 25.90
 + ID2= 2 (8012): 3180.68 15.462 9.75 15.30
 =====
 | ID = 3 (8014): 3387.11 17.202 9.75 15.95

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | ROUTE CHN (6118) |
 | IN= 2--> OUT= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (1181.0) ----->

Distance	Elevation	Manning
0.00	307.18	0.0450
24.89	299.75	0.0450
37.34	296.39	0.0450
71.57	292.75	0.0450
96.46	291.12	0.0450
108.90	289.43	0.0450
112.02	288.86	0.0450
115.13	288.46	0.0450
117.24	288.32	0.0450 /0.0350 Main Channel
117.49	288.15	0.0350 Main Channel
118.24	288.00	0.0350 Main Channel
118.99	288.20	0.0350 Main Channel
119.24	288.36	0.0350 /0.0450 Main Channel
121.35	288.40	0.0450
149.35	289.36	0.0450
171.14	290.47	0.0450
208.47	293.14	0.0450
236.48	295.17	0.0450
280.04	299.48	0.0450
308.04	307.00	0.0450

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.32	288.32	.118E+04	0.2	0.62	81.92
1.30	289.30	.631E+05	22.2	1.07	47.43
2.29	290.29	.218E+06	117.5	1.64	30.95
3.27	291.27	.448E+06	314.6	2.15	23.72
4.25	292.25	.756E+06	625.6	2.53	20.13
5.24	293.24	.115E+07	1093.2	2.91	17.50
6.22	294.22	.161E+07	1731.6	3.28	15.50
7.20	295.20	.214E+07	2547.4	3.63	14.01
8.19	296.19	.274E+07	3584.9	4.00	12.72
9.17	297.17	.338E+07	4859.1	4.39	11.60
10.15	298.15	.407E+07	6341.5	4.76	10.70
11.13	299.13	.490E+07	8024.6	5.11	9.97
12.12	300.12	.556E+07	10006.6	5.49	9.27
13.10	301.10	.635E+07	12244.6	5.89	8.64
14.08	302.08	.716E+07	14681.7	6.26	8.13
15.07	303.07	.799E+07	17316.5	6.62	7.69
16.05	304.05	.884E+07	20148.1	6.96	7.31
17.03	305.03	.971E+07	23176.2	7.29	6.98
18.02	306.02	.106E+08	26400.8	7.61	6.69

<---- hydrograph ----> <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8014) 3387.11	17.20	9.75	15.95	1.08	0.92
OUTFLOW: ID= 1 (6118) 3387.11	16.41	10.50	15.95	1.04	0.90

 | CALIB |
 | NASHVD (0118) | Area (ha)= 340.53 Curve Number (CN)= 45.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 | U.H. Tp(hrs)= 1.12

Unit Hyd Qpeak (cms)= 5.200

PEAK FLOW (cms)= 2.109 (1)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 14.747
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.183

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | ADD HYD (8016) |
 | 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 | (ha) (cms) (hrs) (mm)
 | ID1= 1 (0118): 340.53 2.109 7.75 14.75
 + ID2= 2 (6118): 3387.11 16.410 10.50 15.95
 =====
 | ID = 3 (8016): 3727.64 17.957 10.25 15.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | ROUTE CHN (6122) |
 | IN= 2--> OUT= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (1221.0) ----->

Distance	Elevation	Manning
0.00	277.00	0.0500
52.18	272.82	0.0500
86.97	269.38	0.0500
134.80	267.35	0.0500
160.89	263.85	0.0500
169.58	262.88	0.0500
178.28	262.45	0.0500
195.67	262.07	0.0500
213.07	260.99	0.0500
226.01	259.80	0.0500 /0.0300 Main Channel
226.11	259.30	0.0300 Main Channel
230.11	259.30	0.0300 Main Channel
230.20	259.80	0.0300 /0.0500 Main Channel
235.01	260.20	0.0500
239.15	260.57	0.0500
243.50	260.93	0.0500
300.03	264.43	0.0500
339.16	268.34	0.0500
373.95	272.36	0.0500
430.48	277.46	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.50	259.80	.626E+04	2.5	1.23	41.35
1.41	260.71	.462E+05	21.8	1.44	35.27
2.31	261.61	.149E+06	79.6	1.63	31.19
3.22	262.52	.340E+06	200.1	1.80	28.34
4.12	263.42	.636E+06	446.8	2.15	23.71
5.03	264.33	.995E+06	819.9	2.52	20.23
5.93	265.23	.141E+07	1337.0	2.90	17.56
6.84	266.14	.187E+07	1984.8	3.25	15.67
7.74	267.04	.237E+07	2766.0	3.57	14.26
8.65	267.95	.292E+07	3596.9	3.76	13.53
9.55	268.85	.355E+07	4575.4	3.93	12.95
10.46	269.76	.427E+07	5814.9	4.16	12.23
11.36	270.66	.503E+07	7319.0	4.44	11.46
12.27	271.57	.584E+07	9009.7	4.71	10.81
13.17	272.47	.670E+07	10865.3	4.96	10.26
14.08	273.38	.761E+07	12893.2	5.17	9.84
14.98	274.28	.858E+07	15098.1	5.37	9.48
15.89	275.19	.961E+07	17527.6	5.57	9.14
16.79	276.09	.107E+08	20190.3	5.76	8.83

<---- hydrograph ----> <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8016) 3727.64	17.96	10.25	15.84	1.22	1.39
OUTFLOW: ID= 1 (6122) 3727.64	17.49	11.00	15.84	1.20	1.39

 | STANDHYD (1221) | Area (ha)= 17.70
 | ID= 1 DT=15.0 min | Total Imp(%)= 36.00 Dir. Conn.(%)= 19.00

 | CALIB |
 | Surface Area (ha)= 6.37 IMPERVIOUS 11.33
 | Dep. Storage (mm)= 2.00 PERVIOUS (i) 5.00
 | Average Slope (%)= 0.50 0.50
 | Length (m)= 345.51 40.00
 | Mannings n = 0.013 0.250
 Max.Eff.Inten.(mm/hr)= 106.66 14.75

over (min) 15.00 30.00
 Storage Coeff. (min)= 6.43 (ii) 29.43 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.10 0.04

TOTALS
 PEAK FLOW (cms)= 0.93 0.32 1.124 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 78.80 13.14 25.61
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.16 0.32

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 15.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | RESERVOIR (5122) |
 | IN= 2--> OUT= 1 |
 | DT= 15.0 min |

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.1840	0.4294
0.0600	0.1855	1.4870	0.5014
0.3430	0.2103	1.7310	0.5718
0.6390	0.2878	2.1310	0.5818
0.8860	0.3509	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
17.700	1.124	6.00	25.61
17.700	0.360	6.50	25.57

INFLOW : ID= 2 (1221) 17.700 1.124 6.00 25.61
 OUTFLOW: ID= 1 (5122) 17.700 0.360 6.50 25.57

PEAK FLOW REDUCTION [Qout/Qin](%)= 32.03
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 0.2161

 | CALIB (1222) |
 | NASHYD (1222) | Area (ha)= 347.20 Curve Number (CN)= 41.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 | U.H. Tp(hrs)= 1.52 |

Unit Hyd Qpeak (cms) = 3.887

PEAK FLOW (cms) = 1.491 (i)
 TIME TO PEAK (hrs) = 8.250
 RUNOFF VOLUME (mm) = 12.949
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.160

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | ADD HYD (8018) |
 | 1 + 2 = 3 |

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
347.20	1.491	8.25	12.95
17.70	0.360	6.50	25.57
364.90	1.608	8.00	13.56

ID1= 1 (1222): 347.20 1.491 8.25 12.95
 + ID2= 2 (5122): 17.70 0.360 6.50 25.57
 ID = 3 (8018): 364.90 1.608 8.00 13.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | ADD HYD (8020) |
 | 1 + 2 = 3 |

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
3727.64	17.487	11.00	15.84
364.90	1.608	8.00	13.56
4092.54	18.784	10.75	15.64

ID1= 1 (6122): 3727.64 17.487 11.00 15.84
 + ID2= 2 (8018): 364.90 1.608 8.00 13.56
 ID = 3 (8020): 4092.54 18.784 10.75 15.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | CALIB (1261) |
 | STANDHYD (1261) | Area (ha)= 50.00
 | ID= 1 DT=15.0 min | Total Imp(%)= 36.00 Dir. Conn.(%)= 26.00

IMPERVIOUS (%)	PERVIOUS (i)
18.00	32.00
2.00	5.00
0.50	0.50

Surface Area (ha)= 18.00 32.00
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50

Length (m)= 577.35 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten.(mm/hr)= 106.66 19.45
 over (min) 15.00 30.00
 Storage Coeff. (min)= 8.78 (ii) 29.37 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.04

TOTALS
 PEAK FLOW (cms)= 3.37 1.18 4.092 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 78.80 18.60 34.25
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.23 0.42

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | RESERVOIR (5126) |
 | IN= 2--> OUT= 1 |
 | DT= 15.0 min |

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.3450	1.2129
0.1690	0.5421	4.2000	1.4164
0.9690	0.5941	4.8890	1.6153
1.8040	0.8130	5.2890	1.6253
2.5020	0.9912	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
50.000	4.092	6.00	34.25
50.000	1.573	6.50	34.23

INFLOW : ID= 2 (1261) 50.000 4.092 6.00 34.25
 OUTFLOW: ID= 1 (5126) 50.000 1.573 6.50 34.23

PEAK FLOW REDUCTION [Qout/Qin](%)= 38.45
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 0.7544

 | CALIB (1262) |
 | NASHYD (1262) | Area (ha)= 83.50 Curve Number (CN)= 58.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 | U.H. Tp(hrs)= 0.56 |

Unit Hyd Qpeak (cms) = 2.523

PEAK FLOW (cms) = 1.319 (i)
 TIME TO PEAK (hrs) = 6.500
 RUNOFF VOLUME (mm) = 21.597
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.267

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | ADD HYD (8024) |
 | 1 + 2 = 3 |

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
83.50	1.319	6.50	21.60
50.00	1.573	6.50	34.23
133.50	2.892	6.50	26.33

ID1= 1 (1262): 83.50 1.319 6.50 21.60
 + ID2= 2 (5126): 50.00 1.573 6.50 34.23
 ID = 3 (8024): 133.50 2.892 6.50 26.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 | CALIB (0124) |
 | NASHYD (0124) | Area (ha)= 121.43 Curve Number (CN)= 43.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 | U.H. Tp(hrs)= 0.51 |

Unit Hyd Qpeak (cms) = 4.049

PEAK FLOW (cms) = 1.279 (i)
 TIME TO PEAK (hrs) = 6.500
 RUNOFF VOLUME (mm) = 13.547
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.168

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | ADD HYD (8026) |

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0124):	121.43	1.279	6.50	13.55
+ ID2= 2 (8024):	133.50	2.892	6.50	26.33
ID = 3 (8026):	254.93	4.171	6.50	20.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6128)
ID= 2 -> OUT= 1
Routing time step (min)'= 15.00

----- DATA FOR SECTION (1281.0) -----

Distance	Elevation	Manning	
0.00	270.58	0.0400	
7.71	270.36	0.0400	
15.43	269.48	0.0400	
23.14	265.88	0.0400	
42.43	261.83	0.0400	
61.71	258.31	0.0400	
75.21	256.62	0.0400	
90.64	255.35	0.0400	
92.49	255.00	0.0400 / 0.0300	Main Channel
93.49	254.00	0.0300	Main Channel
94.49	253.90	0.0300	Main Channel
94.99	253.80	0.0300	Main Channel
95.49	253.70	0.0300	Main Channel
95.99	253.80	0.0300	Main Channel
98.49	255.00	0.0300 / 0.0400	Main Channel
100.28	255.44	0.0400	
134.99	257.45	0.0400	
148.49	259.67	0.0400	
167.77	263.42	0.0400	
190.92	270.00	0.0400	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.65	254.35	4.39E+04	2.0	1.24	36.10
1.30	255.00	1.31E+05	9.4	1.92	23.39
2.18	255.88	4.33E+05	37.0	2.30	19.54
3.06	256.76	1.31E+06	113.3	2.34	19.20
3.95	257.65	2.73E+06	273.8	2.70	16.64
4.83	258.53	4.53E+06	538.5	3.21	14.01
5.71	259.41	6.98E+06	902.9	3.70	12.14
6.59	260.29	8.86E+06	1368.4	4.16	10.79
7.48	261.18	1.14E+07	1936.3	4.59	9.79
8.36	262.06	1.41E+07	2610.9	4.99	9.01
9.24	262.94	1.71E+07	3398.7	5.37	8.36
10.12	263.82	2.02E+07	4309.6	5.75	7.81
11.01	264.71	2.35E+07	5347.5	6.13	7.33
11.89	265.59	2.70E+07	6501.0	6.48	6.93
12.77	266.47	3.07E+07	7808.6	6.86	6.55
13.65	267.35	3.45E+07	9250.5	7.23	6.21
14.54	268.24	3.84E+07	10800.3	7.59	5.92
15.42	269.12	4.24E+07	12482.5	7.94	5.66
16.30	270.00	4.66E+07	14127.4	8.18	5.49

----- hydrograph ----- <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8026)	254.93	4.17	6.50	20.24	0.84
OUTFLOW : ID= 1 (6128)	254.93	3.44	7.25	20.24	0.78

CALIB NASHVD (0128) Area (ha)= 240.24 Curve Number (CN)= 62.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.00

Unit Hyd Opeak (cms)= 4.118
PEAK FLOW (cms)= 2.761 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 24.567
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.304

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0128):	240.24	2.761	7.25	24.57
+ ID2= 2 (6128):	254.93	3.444	7.25	20.24
ID = 3 (8028):	495.17	6.205	7.25	22.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8020):	4092.54	18.784	10.75	15.64
+ ID2= 2 (8028):	495.17	6.205	7.25	22.34
ID = 3 (8022):	4587.71	21.488	10.50	16.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDBYD (1201) Area (ha)= 86.00 Dir. Conn.(%)= 14.00
ID= 1 DT=15.0 min Total Imp(%)= 36.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 30.96 55.04
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 757.19 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 106.66 25.83
over (min) 15.00 30.00
Storage Coeff. (min)= 10.33 (ii) 28.71 (iii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.09 0.04
PEAK FLOW (cms)= 2.98 2.72 4.657 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 78.80 21.08 29.16
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.26 0.36

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 48.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5120)	DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
		0.0000	0.0000	5.7530	2.0863
		0.2910	0.9015	7.2230	2.4361
		1.6660	1.0219	8.4090	2.7784
		3.1030	1.3984	8.8090	2.7884
		4.3040	1.7049	0.0000	0.0000

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (1201) 86.000 4.657 6.00 29.16
OUTFLOW : ID= 1 (5120) 86.000 2.056 6.75 29.15

PEAK FLOW REDUCTION [Qout/Qin](%)= 44.15
TIME SHIFT OF PEAK FLOW (min)= 45.00
MAXIMUM STORAGE USED (ha.m.)= 1.1304

CALIB NASHVD (1202) Area (ha)= 161.00 Curve Number (CN)= 58.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.70

Unit Hyd Opeak (cms)= 3.932
PEAK FLOW (cms)= 2.157 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 21.738
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.269

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1202):	161.00	2.157	6.75	21.74

+ ID2= 2 (5120): 86.00 2.056 6.75 29.15
 ID = 3 (8030): 247.00 4.213 6.75 24.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8032)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8022):	4587.71	21.488	10.50	16.36
+ ID2= 2 (8030):	247.00	4.213	6.75	24.32
ID = 3 (8032):	4834.71	22.672	10.50	16.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6130) Routing time step (min)= 15.00
 IN= 2--> OUT= 1

Distance	Elevation	Manning	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
0.00	257.18	0.0350				
62.45	254.70	0.0350				
109.29	252.74	0.0350				
150.92	251.55	0.0350				
192.56	248.96	0.0350				
239.19	247.16	0.0350				
291.84	246.00	0.0350				
296.84	245.00	0.0350				
298.84	245.00	0.0350 / 0.0300				Main Channel
299.24	244.60	0.0300				Main Channel
301.84	244.60	0.0300				Main Channel
303.44	244.60	0.0300				Main Channel
303.84	245.00	0.0300 / 0.0350				Main Channel
306.84	245.00	0.0350				
309.84	246.00	0.0350				
317.46	247.22	0.0350				
348.68	254.83	0.0350				
359.09	256.70	0.0350				
452.76	258.23	0.0350				
515.22	258.33	0.0350				

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.40	245.00	8708E+04	1.2	0.64	124.12
1.04	245.64	4688E+05	9.5	0.96	81.81
1.68	246.28	1098E+06	25.6	1.11	70.86
2.32	246.92	2578E+06	63.5	1.17	67.51
2.96	247.56	4988E+06	142.7	1.36	58.18
3.61	248.21	8008E+06	266.7	1.58	50.01
4.25	248.85	1168E+07	437.4	1.78	44.24
4.89	249.49	1578E+07	669.1	2.01	39.20
5.53	250.13	2038E+07	954.9	2.23	35.36
6.17	250.77	2528E+07	1294.4	2.43	32.42
6.81	251.41	3058E+07	1689.6	2.62	30.07
7.45	252.05	3638E+07	2090.7	2.73	28.93
8.09	252.69	4298E+07	2553.1	2.82	27.98
8.73	253.33	5018E+07	3138.4	2.96	26.60
9.37	253.97	5798E+07	3803.8	3.11	25.36
10.02	254.62	6628E+07	4549.3	3.25	24.28
10.66	255.26	7518E+07	5364.9	3.38	23.32
11.30	255.90	8458E+07	6265.7	3.51	22.49
11.94	256.54	9468E+07	7260.1	3.63	21.72

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8032) 4834.71	22.67	10.50	16.77	1.57	1.08
OUTFLOW: ID= 1 (6130) 4834.71	21.30	11.50	16.77	1.50	1.07

STANDHYD (1301)	Area (ha)	Total Imp(%)	Dir. Conn.(%)
ID= 1 DT=15.0 min	228.30	65.00	47.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)= 148.39	79.91
Dep. Storage (mm)= 2.00	5.00
Average Slope (%)= 0.50	0.50
Length (m)= 1233.69	40.00
Mannings n = 0.013	0.250

Max. Eff. Inten. (mm/hr)	over (min)
106.66	103.05
15.00	30.00
Storage Coeff. (min)= 13.84 (ii)	24.41 (ii)
Unit Hyd. Tpeak (min)= 15.00	30.00
Unit Hyd. peak (cms)= 0.07	0.04

*****TOTALS*
 PEAK FLOW (cms)= 23.94 10.67 30.904 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 78.80 44.02 60.37
 TOTAL RAINFALL (mm)= 80.60 80.60 80.60
 RUNOFF COEFFICIENT = 0.98 0.54 0.75

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 74.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5130)	IN= 2--> OUT= 1	DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
			0.0000	0.0000	12.4560	7.0142
			0.8210	3.9074	14.9550	7.8554
			4.8550	4.4209	22.4790	8.9550
			7.0620	5.2214	22.8790	8.9650
			9.5970	6.0904	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1301) 228.300	30.904	6.00	60.37
OUTFLOW: ID= 1 (5130) 228.300	11.302	6.75	60.36

PEAK FLOW REDUCTION [Qout/Qin](%)= 36.57
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 6.7529

CALIB (1302) Area (ha)= 157.80 Curve Number (CN)= 76.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.82

Unit Hyd Qpeak (cms)= 1.479

PEAK FLOW (cms)= 1.716 (i)
 TIME TO PEAK (hrs)= 8.500
 RUNOFF VOLUME (mm)= 36.674
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.454

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8036)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1302):	157.80	1.716	8.50	36.67
+ ID2= 2 (5130):	228.30	11.302	6.75	60.36
ID = 3 (8036):	386.10	12.666	6.75	50.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8034)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (6130):	4834.71	21.305	11.50	16.77
+ ID2= 2 (8036):	386.10	12.666	6.75	50.68
ID = 3 (8034):	5220.81	23.758	11.25	19.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

STANDHYD (0188)	Area (ha)	Total Imp(%)	Dir. Conn.(%)
ID= 1 DT=15.0 min	308.53	41.00	25.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)= 126.50	182.03
Dep. Storage (mm)= 3.00	5.00
Average Slope (%)= 0.10	0.10
Length (m)= 1434.18	40.00
Mannings n = 0.013	0.250

Max. Eff. Inten. (mm/hr)	over (min)
72.72	32.44
30.00	60.00
Storage Coeff. (min)= 28.62 (ii)	55.62 (ii)

Unit Hyd. Tpeak (min)= 30.00 60.00
 Unit Hyd. peak (cms)= 0.04 0.02

TOTALS
 PEAK FLOW (cms)= 10.45 6.83 14.249 (iii)
 TIME TO PEAK (hrs)= 6.25 6.75 6.25
 RUNOFF VOLUME (mm)= 77.80 27.39 39.99
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.96 0.34 0.49

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0186) Area (ha)= 351.79
 ID= 1 DT=15.0 min Total Imp(%)= 42.00 Dir. Conn.(%)= 22.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	147.75	204.04
Dep. Storage (mm)=	3.00	5.00
Average Slope (%)=	0.10	0.10
Length (m)=	1531.43	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 72.72 30.11
 over (min) 30.00 60.00
 Storage Coeff. (min)= 29.77 (ii) 57.79 (ii)
 Unit Hyd. Tpeak (min)= 30.00 60.00
 Unit Hyd. peak (cms)= 0.04 0.02

TOTALS
 PEAK FLOW (cms)= 10.30 6.98 14.152 (iii)
 TIME TO PEAK (hrs)= 6.25 6.75 6.25
 RUNOFF VOLUME (mm)= 77.80 24.29 36.07
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.96 0.30 0.45

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 53.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8068)
 1 + 2 = 3

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0186):	351.79	14.152	6.25	36.07
+ ID2= 2 (0188):	308.53	14.249	6.25	39.99
=====				
ID = 3 (8068):	660.32	28.401	6.25	37.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1821) Area (ha)= 111.80
 ID= 1 DT=15.0 min Total Imp(%)= 40.00 Dir. Conn.(%)= 17.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	44.72	67.08
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	863.33	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 106.66 50.21
 over (min) 15.00 30.00
 Storage Coeff. (min)= 11.17 (ii) 25.26 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

TOTALS
 PEAK FLOW (cms)= 4.59 4.22 7.214 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 78.80 24.15 33.44
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.30 0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 52.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5182)
 IN= 2--> OUT= 1
 DT= 15.0 min

	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	5.7800	2.5656	
0.3790	1.0207	7.3720	3.0629	
1.6240	1.1939	8.7710	3.5956	
3.0930	1.6631	9.1710	3.6056	
4.3530	2.1032	0.0000	0.0000	

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW: ID= 2 (1821) 111.800 7.214 6.00 33.44
 OUTFLOW: ID= 1 (5182) 111.800 2.963 6.75 33.44

PEAK FLOW REDUCTION [Qout/Qin](%)= 41.08
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 1.6292

CALIB
 NASHYD (1822) Area (ha)= 44.40 Curve Number (CN)= 57.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.79

Unit Hyd Qpeak (cms)= 0.966

PEAK FLOW (cms)= 0.529 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 21.172
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.262

- (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8062)
 1 + 2 = 3

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1822):	44.40	0.529	7.00	21.17
+ ID2= 2 (5182):	111.80	2.963	6.75	33.44
=====				
ID = 3 (8062):	156.20	3.486	6.75	29.95

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1841) Area (ha)= 144.40
 ID= 1 DT=15.0 min Total Imp(%)= 48.00 Dir. Conn.(%)= 19.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	69.31	75.09
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	981.16	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 106.66 69.06
 over (min) 15.00 30.00
 Storage Coeff. (min)= 12.06 (ii) 24.47 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

TOTALS
 PEAK FLOW (cms)= 6.45 6.63 10.629 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 78.80 29.27 38.68
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.36 0.48

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 56.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5184)
 IN= 2--> OUT= 1
 DT= 15.0 min

	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	5.8820	2.3298	
0.2960	1.0006	7.4070	2.7386	

1.6530	1.1680	8.6830	3.1418
3.0860	1.4687	9.0830	3.1518
4.3760	1.8752	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1841)	144.400	10.629	6.00	38.68
OUTFLOW: ID= 1 (5184)	144.400	5.190	6.75	38.67

PEAK FLOW REDUCTION [Qout/Qin](%)= 48.83
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 2.1635

CALIB
 STANDHYD (1842)
 ID= 1 DT=15.0 min

Area (ha)	= 118.10
Total Imp(%)	= 42.00
Dir. Conn.(%)	= 17.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	49.60	68.50
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	887.32	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	= 106.66	59.67
over (min)	15.00	30.00
Storage Coeff. (min)	11.36 (ii)	24.51 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04

TOTALS
 PEAK FLOW (cms)= 4.82 5.21 8.090 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 78.80 27.57 36.28
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.34 0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 56.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8060)
 1 + 2 = 3

	AREA	QPEAK	TPEAK	R.V.
ID1= 1 (1842):	118.10	8.090	6.00	36.28
+ ID2= 2 (5184):	144.40	5.190	6.75	38.67
ID = 3 (8060):	262.50	11.354	6.25	37.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8064)
 1 + 2 = 3

	AREA	QPEAK	TPEAK	R.V.
ID1= 1 (8060):	262.50	11.354	6.25	37.59
+ ID2= 2 (8062):	156.20	3.486	6.75	29.95
ID = 3 (8064):	418.70	13.866	6.25	34.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1781)
 ID= 1 DT=15.0 min

Area (ha)	= 14.00
Total Imp(%)	= 50.00
Dir. Conn.(%)	= 30.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	7.00	7.00
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	305.51	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	= 106.66	96.17
over (min)	15.00	30.00
Storage Coeff. (min)	5.99 (ii)	16.86 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.10	0.05

TOTALS
 PEAK FLOW (cms)= 1.18 1.02 1.856 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00

RUNOFF VOLUME (mm)	= 78.80	44.34	54.68
TOTAL RAINFALL (mm)	= 80.80	80.80	80.80
RUNOFF COEFFICIENT	= 0.98	0.55	0.68

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 76.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5178)
 IN= 2--> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	0.9360	0.3780
	0.0470	0.1841	1.1760	0.4277
	0.2710	0.2148	1.3690	0.4763
	0.5050	0.2704	1.7690	0.4863
	0.7010	0.3197	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
INFLOW : ID= 2 (1781)	14.000	1.856	6.00	54.68
OUTFLOW: ID= 1 (5178)	14.000	0.779	6.50	54.60

PEAK FLOW REDUCTION [Qout/Qin](%)= 41.99
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 0.3396

CALIB
 NASHYD (1782)
 ID= 1 DT=15.0 min

Area (ha)	= 261.40
Curve Number (CN)	= 78.0
Ia (mm)	= 5.00
# of Linear Res. (N)	= 1.50
U.H. Tp (hrs)	= 1.15

Unit Hyd Qpeak (cms)= 3.867

PEAK FLOW (cms)	= 4.319 (i)
TIME TO PEAK (hrs)	= 7.500
RUNOFF VOLUME (mm)	= 38.648
TOTAL RAINFALL (mm)	= 80.800
RUNOFF COEFFICIENT	= 0.478

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8046)
 1 + 2 = 3

	AREA	QPEAK	TPEAK	R.V.
ID1= 1 (1782):	261.40	4.319	7.50	38.65
+ ID2= 2 (5278):	14.00	0.779	6.50	54.60
ID = 3 (8046):	275.40	4.845	7.00	39.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0180)
 ID= 1 DT=15.0 min

Area (ha)	= 226.65
Curve Number (CN)	= 65.0
Ia (mm)	= 5.00
# of Linear Res. (N)	= 1.50
U.H. Tp (hrs)	= 1.17

Unit Hyd Qpeak (cms)= 3.299

PEAK FLOW (cms)	= 2.512 (i)
TIME TO PEAK (hrs)	= 7.750
RUNOFF VOLUME (mm)	= 26.812
TOTAL RAINFALL (mm)	= 80.800
RUNOFF COEFFICIENT	= 0.332

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8050)
 1 + 2 = 3

	AREA	QPEAK	TPEAK	R.V.
ID1= 1 (0180):	226.65	2.512	7.75	26.81
+ ID2= 2 (8046):	275.40	4.845	7.00	39.46
ID = 3 (8050):	502.05	7.297	7.25	33.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (1741) | Area (ha)= 56.30
ID= 1 DT=15.0 min | Total Imp(%)= 35.00 Dir. Conn.(%)= 18.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	19.70	36.60
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	612.64	40.00
Manning's n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	106.66	59.79	
cover (min)	15.00	30.00	
Storage Coeff. (min)=	9.09 (ii)	22.24 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.09	0.04	
TOTALS			
PEAK FLOW (cms)=	2.60	2.93	4.458 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	78.80	31.06	39.65
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.38	0.49

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5174)
IN= 2 -> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.1370	1.3070
0.0580	0.5427	1.4280	1.5354
0.3290	0.6196	1.6620	1.7593
0.6130	0.8627	2.0620	1.7593
0.8510	1.0621	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1741)	56.300	4.458	6.00	39.65
OUTFLOW : ID= 1 (5174)	56.300	1.050	7.00	39.61

PEAK FLOW REDUCTION [Qout/Qin](%)= 23.55
TIME SHIFT OF PEAK FLOW (min)= 60.00
MAXIMUM STORAGE USED (ha.m.)= 1.2370

CALIB
NASHYD (1742) | Area (ha)= 82.80 Curve Number (CN)= 71.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.69

Unit Hyd Qpeak (cms)= 2.055
PEAK FLOW (cms)= 1.665 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 31.433
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.389

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8044)
1 + 2 = 3
ID1= 1 (1742): 82.80 1.665 6.75 31.43
+ ID2= 2 (5174): 56.30 1.050 7.00 39.61
ID = 3 (8044): 139.10 2.696 7.00 34.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHYD (0172) | Area (ha)= 202.23 Curve Number (CN)= 78.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.89

Unit Hyd Qpeak (cms)= 3.875
PEAK FLOW (cms)= 4.110 (i)
TIME TO PEAK (hrs)= 7.000

RUNOFF VOLUME (mm)= 38.497
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.476

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0168) | Area (ha)= 247.13 Curve Number (CN)= 52.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.75

Unit Hyd Qpeak (cms)= 5.611

PEAK FLOW (cms)= 2.601 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 18.230
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.226

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0170) | Area (ha)= 166.91 Curve Number (CN)= 65.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.80

Unit Hyd Qpeak (cms)= 3.553

PEAK FLOW (cms)= 2.489 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 26.647
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.330

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8040)
1 + 2 = 3
ID1= 1 (0168): 247.13 2.601 7.00 18.23
+ ID2= 2 (0170): 166.91 2.489 7.00 26.65
ID = 3 (8040): 414.04 5.090 7.00 21.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8042)
1 + 2 = 3
ID1= 1 (0172): 202.23 4.110 7.00 38.50
+ ID2= 2 (8040): 414.04 5.090 7.00 21.62
ID = 3 (8042): 616.27 9.200 7.00 27.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (5174)
IN= 2 -> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (1741.0) -----
Distance Elevation Manning
0.00 296.00 0.0450
13.36 288.94 0.0450
26.71 288.64 0.0450
40.07 287.96 0.0450
53.42 287.32 0.0450
66.78 286.48 0.0450
80.13 285.08 0.0450
93.49 282.76 0.0450
106.84 281.06 0.0450 / 0.0350 Main Channel
120.19 280.35 0.0350 Main Channel
133.54 280.35 0.0350 Main Channel
146.89 281.10 0.0350 / 0.0450 Main Channel
160.24 282.64 0.0450
173.59 286.87 0.0450
186.94 289.58 0.0450
200.29 292.29 0.0450
213.64 293.95 0.0450
226.99 295.13 0.0450
240.34 296.02 0.0450
253.69 296.09 0.0450

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.71	281.06	.136E+05	6.0	1.69	37.89
1.50	281.85	.358E+05	22.5	2.42	26.43
2.28	282.63	.746E+05	53.9	2.65	22.47
3.07	283.42	.124E+06	104.1	3.24	19.79
3.86	284.21	.189E+06	176.7	3.60	17.81
4.64	284.99	.268E+06	274.9	3.94	16.26
5.43	285.78	.369E+06	390.8	4.07	15.73
6.21	286.56	.501E+06	551.6	4.23	15.15
7.00	287.35	.674E+06	750.5	4.28	14.97
7.79	288.14	.920E+06	993.3	4.15	15.44
8.57	288.92	.124E+07	1361.5	4.14	15.47
9.36	289.71	.167E+07	1986.0	4.56	14.04
10.15	290.50	.210E+07	2739.7	5.02	12.76
10.93	291.28	.254E+07	3615.0	5.47	11.70
11.72	292.07	.300E+07	4608.8	5.91	10.83
12.50	292.85	.347E+07	5693.3	6.30	10.16
13.29	293.64	.397E+07	6892.9	6.67	9.60
14.08	294.43	.450E+07	8150.4	6.97	9.19
14.86	295.21	.506E+07	9525.0	7.23	8.86

		<---- hydrograph ---->			<-pipe / channel->		
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (8042)	616.27	9.20	7.00	27.16	0.86	1.80	
OUTFLOW : ID= 1 (6174)	616.27	8.48	7.75	27.16	0.83	1.77	

ADD HYD (8048)						
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (6174):	616.27	8.476	7.75	27.16		
+ ID2= 2 (8044):	139.10	2.696	7.00	34.74		
ID = 3 (8048):	755.37	10.888	7.50	28.56		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8052)						
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (8048):	755.37	10.888	7.50	28.56		
+ ID2= 2 (8050):	502.05	7.297	7.25	33.75		
ID = 3 (8052):	1257.42	18.135	7.50	30.63		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8058)						
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (8052):	1257.42	18.135	7.50	30.63		
+ ID2= 2 (8064):	418.70	13.866	6.25	34.74		
ID = 3 (8058):	1676.12	28.104	6.75	31.66		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB			
STANDHYD (1762)	Area	(ha)=	162.00
ID= 1 DT=15.0 min	Total Imp(%)=		26.00

		IMPERVIOUS		PERVIOUS (i)	
Surface Area	(ha)=	42.12		119.88	
Dep. Storage	(mm)=	2.00		5.00	
Average Slope	(%)=	0.50		0.50	
Length	(m)=	1039.23		40.00	
Mannings n	=	0.013		0.250	
Max.Eff.Inten.(mm/hr)		106.66		47.86	
over	(min)	15.00		30.00	
Storage Coeff.	(min)=	12.49 (ii)		26.85 (ii)	
Unit Hyd. Tpeak	(min)=	15.00		30.00	
Unit Hyd. peak	(cms)=	0.08		0.04	
PEAK FLOW	(cms)=	4.14		6.97	8.715 (iii)
TIME TO PEAK	(hrs)=	6.00		6.25	6.25
RUNOFF VOLUME	(mm)=	78.80		26.29	32.07
TOTAL RAINFALL	(mm)=	80.80		80.80	80.80
RUNOFF COEFFICIENT	=	0.98		0.33	0.40

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING:FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%

YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB			
STANDHYD (1761)	Area	(ha)=	48.80
ID= 1 DT=15.0 min	Total Imp(%)=		50.00

		IMPERVIOUS		PERVIOUS (i)	
Surface Area	(ha)=	24.40		24.40	
Dep. Storage	(mm)=	2.00		5.00	
Average Slope	(%)=	0.50		0.50	
Length	(m)=	570.38		40.00	
Mannings n	=	0.013		0.250	
Max.Eff.Inten.(mm/hr)		106.66		76.51	
over	(min)	15.00		30.00	
Storage Coeff.	(min)=	8.71 (ii)		20.62 (ii)	
Unit Hyd. Tpeak	(min)=	15.00		30.00	
Unit Hyd. peak	(cms)=	0.09		0.05	
PEAK FLOW	(cms)=	2.66		2.59	4.318 (iii)
TIME TO PEAK	(hrs)=	6.00		6.25	6.00
RUNOFF VOLUME	(mm)=	78.80		31.83	41.69
TOTAL RAINFALL	(mm)=	80.80		80.80	80.80
RUNOFF COEFFICIENT	=	0.98		0.39	0.52

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5176)					
IN= 2--> OUT= 1	DT= 15.0 min	OUTFLOW	STORAGE	OUTFLOW	STORAGE
		(cms)	(ha.m.)	(cms)	(ha.m.)
		0.0000	0.0000	3.2640	1.3175
		0.1650	0.6418	4.0090	1.4908
		0.9450	0.7489	4.7720	1.6604
		1.7610	0.9426	5.1720	1.6704
		2.4420	1.1144	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (1761)	48.800	4.318	6.00	41.69
OUTFLOW : ID= 1 (5176)	48.800	1.734	6.75	41.67

PEAK FLOW REDUCTION [Qout/Qin]= 40.15
TIME SHIFT OF PEAK FLOW (min)= 45.00
MAXIMUM STORAGE USED (ha.m.)= 0.9502

ADD HYD (8056)						
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (1762):	162.00	8.715	6.25	32.07		
+ ID2= 2 (5176):	48.80	1.734	6.75	41.67		
ID = 3 (8056):	210.80	9.902	6.25	34.29		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8066)						
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (8056):	210.80	9.902	6.25	34.29		
+ ID2= 2 (8058):	1676.12	28.104	6.75	31.66		
ID = 3 (8066):	1886.92	35.435	6.50	31.95		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8070)						
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.		
	(ha)	(cms)	(hrs)	(mm)		
ID1= 1 (8066):	1886.92	35.435	6.50	31.95		

+ ID# 2 (8068): 660.32 28.401 6.25 37.90
 ID = 3 (8070): 2547.24 63.398 6.25 33.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTER CHN (6190) |
 IN# 2--> OUT# 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (1901.0) -----

Distance	Elevation	Manning
0.00	261.34	0.0600
19.76	261.64	0.0600
59.28	261.29	0.0600
79.04	261.09	0.0600
103.74	255.99	0.0600
153.13	253.42	0.0600
192.65	252.78	0.0600
212.29	249.80	0.0600
217.29	249.50	0.0600 / 0.0350
217.30	249.05	0.0350
222.29	249.05	0.0350
222.30	249.50	0.0350 / 0.0600
227.30	249.80	0.0600
232.17	252.31	0.0600
276.63	253.53	0.0600
326.03	256.97	0.0600
355.67	257.30	0.0600
400.12	260.08	0.0600
469.28	261.25	0.0600
489.04	262.53	0.0600

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.45	249.50	.436E+04	1.0	0.43	75.97
1.07	250.12	.203E+05	5.4	0.51	62.96
1.70	250.75	.451E+05	14.8	0.64	50.78
2.32	251.37	.762E+05	29.2	0.74	43.46
2.94	251.99	1.14E+06	49.1	0.84	38.61
3.57	252.62	1.61E+06	70.2	0.85	38.18
4.19	253.24	2.24E+06	93.4	0.73	44.29
4.81	253.86	3.98E+06	154.9	0.76	42.79
5.44	254.49	5.76E+06	246.1	0.83	38.99
6.06	255.11	7.79E+06	362.7	0.90	35.79
6.68	255.73	1.01E+07	506.5	0.98	33.15
7.30	256.35	1.26E+07	687.6	1.06	30.52
7.93	256.98	1.53E+07	902.8	1.15	28.18
8.55	257.60	1.83E+07	1114.3	1.18	27.39
9.17	258.22	2.16E+07	1407.8	1.26	25.55
9.80	258.85	2.50E+07	1735.4	1.35	24.03
10.42	259.47	2.86E+07	2097.5	1.42	22.73
11.04	260.09	3.24E+07	2492.3	1.49	21.63
11.67	260.72	3.64E+07	2837.8	1.51	21.39

----- hydrograph ----- <-pipe / channel-->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID# 2 (8070)	2547.24	63.40	6.25	33.49	3.36	0.84
OUTFLOW : ID# 1 (6190)	2547.24	48.78	7.00	33.49	2.92	0.83

CALIB | STANDBYD (0190) |
 ID# 1 DT=15.0 min | Area (ha)= 287.99
 Total Imp(%)= 51.00 Dir. Conn.(%)= 27.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	146.87	141.12
Dep. Storage (mm)	3.00	5.00
Average Slope (%)	0.10	0.10
Length (m)	1385.62	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	72.72	58.88
over (min)	30.00	60.00
Storage Coeff. (min)	28.03 (ii)	49.46 (ii)
Unit Hyd. Tpeak (min)	30.00	60.00
Unit Hyd. peak (cms)	0.04	0.02
PEAK FLOW (cms)	10.64	10.17
TIME TO PEAK (hrs)	6.25	6.25
RUNOFF VOLUME (mm)	77.80	40.78
TOTAL RAINFALL (mm)	80.80	80.80
RUNOFF COEFFICIENT	0.96	0.50

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES;
 CN* = 71.0 Ia = Dep. Storage (above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8072) |
 1 * 2 = 3 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID# 1 (0190): 287.99 16.519 6.25 50.77
 + ID# 2 (6190): 2547.24 48.783 7.00 33.49
 ID = 3 (8072): 2835.23 63.679 6.75 35.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTER CHN (6192) |
 IN# 2--> OUT# 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (1921.0) -----

Distance	Elevation	Manning
0.00	264.40	0.0500
3.64	263.94	0.0500
14.54	261.05	0.0500
29.08	256.78	0.0500
36.36	254.96	0.0500
50.90	252.20	0.0500
65.44	249.78	0.0500
76.35	246.86	0.0500
77.24	245.90	0.0500
82.24	245.45	0.0500 / 0.0350
82.25	244.90	0.0350
87.25	244.90	0.0350
87.26	245.45	0.0350 / 0.0500
94.53	246.89	0.0500
170.87	247.16	0.0500
218.13	248.69	0.0500
265.40	249.22	0.0500
301.75	250.78	0.0500
338.11	253.47	0.0500
359.92	264.00	0.0500

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.55	245.45	.345E+04	2.0	0.71	29.20
1.53	246.43	.174E+05	14.6	1.05	19.92
2.50	247.40	.767E+05	52.8	0.86	24.20
3.48	248.38	1.224E+06	199.8	1.11	18.71
4.46	249.36	1.699E+06	441.2	1.29	16.19
5.43	250.33	2.191E+06	874.3	1.58	13.18
6.41	251.31	2.697E+06	1468.3	1.86	11.20
7.38	252.28	3.211E+07	2224.8	2.13	9.79
8.36	253.26	3.738E+07	3132.4	2.37	8.78
9.34	254.24	4.271E+07	4256.4	2.65	7.87
10.31	255.21	4.813E+07	5551.0	2.92	7.15
11.29	256.19	5.364E+07	6996.8	3.17	6.57
12.27	257.17	5.924E+07	8585.9	3.42	6.10
13.24	258.14	6.493E+07	10315.9	3.65	5.71
14.22	259.12	7.071E+07	12180.7	3.87	5.38
15.19	260.09	7.658E+07	14178.2	4.09	5.10
16.17	261.07	8.254E+07	16306.5	4.29	4.85
17.15	262.05	8.859E+07	18558.6	4.49	4.64
18.12	263.02	9.473E+07	20939.7	4.68	4.45

----- hydrograph ----- <-pipe / channel-->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID# 2 (8072)	2835.23	63.68	6.75	35.25	2.57	0.88
OUTFLOW : ID# 1 (6192)	2835.23	55.67	7.25	35.25	2.52	0.87

CALIB | STANDBYD (1921) |
 ID# 1 DT=15.0 min | Area (ha)= 30.70
 Total Imp(%)= 48.00 Dir. Conn.(%)= 24.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	14.74	15.96
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	452.40	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	106.66	58.58
over (min)	15.00	30.00
Storage Coeff. (min)	7.58 (ii)	20.83 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.10	0.05
PEAK FLOW (cms)	1.98	1.29
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	78.80	26.56
TOTAL RAINFALL (mm)	80.80	80.80

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES;
 CN* = 71.0 Ia = Dep. Storage (above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RUNOFF COEFFICIENT = 0.98 0.33 0.48

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR Pervious Losses: CN* = 54.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5192)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	2.0560	0.7051
	0.1040	0.3429	2.5820	0.7960
	0.5960	0.4035	3.2900	0.8880
	1.1090	0.5056	3.6900	0.8980
	1.5380	0.5974	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (1921)	30.700	2.791	6.00	39.10
OUTFLOW: ID= 1 (5192)	30.700	1.201	6.50	39.07

PEAK FLOW REDUCTION [Qout/Qin](%)	TIME SHIFT OF PEAK FLOW (min)	MAXIMUM STORAGE USED (ha.m.)
43.03	30.00	0.5259

CALIB			
STANDARD (1922)			
ID= 1 DT=15.0 min			
	Area	(ha)	Total Imp(%)
	(ha)	(cms)	Dir. Conn.(%)
	215.90		14.00

	IMPERVIOUS	Pervious (i)
Surface Area (ha)	58.29	157.61
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1199.72	40.00
Mannings n	0.013	0.250

	Max. Eff. Inten. (mm/hr)	over (min)	Storage Coeff. (min)	Unit Hyd. Tpeak (min)	Unit Hyd. peak (cms)
	106.66	24.32	13.61 (ii)	15.00	0.08

	PEAK FLOW (cms)	TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
	6.79	6.34	78.80	80.80	0.98

	PEAK FLOW (cms)	TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
	6.79	6.34	78.80	80.80	0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR Pervious Losses: CN* = 54.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8074)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1922):	215.90	9.423	6.00	30.40
+ ID2= 2 (5192):	30.70	1.201	6.50	39.07
ID = 3 (8074):	246.60	9.524	6.00	31.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8076)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6192):	2835.23	55.672	7.25	35.25
+ ID2= 2 (8074):	246.60	9.524	6.00	31.48
ID = 3 (8076):	3081.83	59.938	7.25	34.95

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB

NASHYD (0196)	Area (ha)	Curve Number (CN)
41.47	5.00	70.0

Unit Hyd Qpeak (cms)= 2.950

PEAK FLOW (cms)	1.731 (i)
TIME TO PEAK (hrs)	6.000
RUNOFF VOLUME (mm)	28.561
TOTAL RAINFALL (mm)	80.800
RUNOFF COEFFICIENT	0.353

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB			
NASHYD (0194)			
ID= 1 DT=15.0 min			
	Area	(ha)	Curve Number (CN)
	(ha)	(cms)	# of Linear Res.(N)
	256.09	5.00	69.0

Unit Hyd Qpeak (cms)= 6.458

PEAK FLOW (cms)	4.910 (i)
TIME TO PEAK (hrs)	6.750
RUNOFF VOLUME (mm)	29.704
TOTAL RAINFALL (mm)	80.800
RUNOFF COEFFICIENT	0.368

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8078)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0194):	256.09	4.910	6.75	29.70
+ ID2= 2 (0194):	41.47	1.731	6.00	28.56
ID = 3 (8078):	297.56	6.279	6.50	29.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6198)	
IN= 2--> OUT= 1	
Routing time step (min)= 15.00	

<----- DATA FOR SECTION (1981.0) ----->				
Distance	Elevation	Manning		
0.00	267.15	0.0500		
22.99	265.94	0.0500		
51.73	261.39	0.0500		
74.72	258.75	0.0500		
94.83	257.79	0.0500		
114.95	254.36	0.0500		
126.44	254.06	0.0500		
132.19	253.68	0.0500		
135.06	253.35	0.0500		
137.94	252.93	0.0500 / 0.0300	Main Channel	
140.81	252.41	0.0300	Main Channel	
143.69	251.89	0.0300	Main Channel	
146.56	252.51	0.0300	Main Channel	
149.43	253.36	0.0300 / 0.0500	Main Channel	
158.05	255.89	0.0500		
172.42	256.25	0.0500		
204.03	259.15	0.0500		
229.90	260.36	0.0500		
252.89	260.46	0.0500		
284.50	260.45	0.0500		

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.35	252.24	.111E+04	0.6	0.97	31.07
0.69	252.58	.442E+04	3.8	1.55	19.42
1.04	252.93	.974E+04	11.1	2.06	14.65
1.51	253.40	.203E+05	31.2	2.77	10.87
1.98	253.87	.357E+05	65.0	3.30	9.14
2.45	254.34	.596E+05	112.9	3.43	8.80
2.92	254.81	.935E+05	186.8	3.62	8.34
3.39	255.28	.131E+06	282.1	3.89	7.75
3.86	255.75	.172E+06	398.2	4.18	7.22
4.33	256.22	.221E+06	527.0	4.31	6.99
4.80	256.69	.282E+06	697.3	4.47	6.75
5.27	257.16	.350E+06	898.5	4.64	6.50
5.74	257.63	.425E+06	1131.4	4.82	6.26
6.21	258.10	.508E+06	1367.4	4.88	6.19
6.68	258.57	.603E+06	1635.6	4.91	6.14
7.15	259.04	.709E+06	1977.2	5.04	5.98
7.62	259.51	.828E+06	2360.5	5.17	5.83
8.09	259.98	.954E+06	2790.7	5.29	5.70
8.56	260.45	.110E+07	3179.6	5.25	5.74

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8078) 297.56 6.28 6.50 29.54 0.81 1.69
OUTFLOW : ID= 1 (6198) 297.56 6.04 6.75 29.54 0.80 1.68

```

```

CALIB
STANDHYD (1981) | Area (ha)= 104.70
ID= 1 DT=15.0 min | Total Imp(%)= 44.00 Dir. Conn.(%)= 20.00

```

```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 46.07 58.63
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 835.46 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten.(mm/hr)= 106.66 62.82
over (min) 15.00 30.00
Storage Coeff. (min)= 10.95 (ii) 23.84 (iii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04

*TOTALS*
PEAK FLOW (cms)= 5.09 4.77 8.091 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 78.80 29.00 38.96
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.36 0.48

```

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 **** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 58.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

RESERVOIR (5198) |
ID= 2--> OUT= 1 |
DT= 15.0 min |
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
0.0000 0.0000 | 7.9640 2.3173
0.2850 1.2270 | 9.5520 2.6039
1.7410 1.4052 | 10.9290 2.8953
3.5350 1.7675 | 11.3290 2.8953
5.6580 2.0183 | 0.0000 0.0000

```

```

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (1981) 104.700 8.091 6.00 38.96
OUTFLOW : ID= 1 (5198) 104.700 3.668 6.75 38.95

```

PEAK FLOW REDUCTION [Qout/Qin](%)= 45.33
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 1.8131

```

CALIB
NASHYD (1982) | Area (ha)= 285.90 Curve Number (CN)= 65.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.N. Tp(hrs)= 0.81

```

Unit Hyd Qpeak (cms)= 6.048

```

PEAK FLOW (cms)= 4.244 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 26.651
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.330

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8080) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (1982): 285.90 4.244 7.00 26.65
+ ID2= 2 (5198): 104.70 3.668 6.75 38.95
=====
ID = 3 (8080): 390.60 7.863 6.75 29.95

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8082) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (6198): 297.56 6.038 6.75 29.54
+ ID2= 2 (8080): 390.60 7.863 6.75 29.95
=====
ID = 3 (8082): 688.16 13.901 6.75 29.77

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8084) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8076): 3081.83 59.938 7.25 34.95
+ ID2= 2 (8082): 688.16 13.901 6.75 29.77
=====
ID = 3 (8084): 3769.99 73.128 7.00 34.00

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6200) |
ID= 2--> OUT= 1 | Routing time step (min)= 15.00

```

<----- DATA FOR SECTION (2001.0) ----->

Distance	Elevation	Manning
0.00	252.47	0.0500
10.38	252.29	0.0500
20.76	252.28	0.0500
41.52	247.66	0.0500
62.28	247.47	0.0500
80.45	247.34	0.0500
90.83	246.85	0.0500
103.81	245.51	0.0500
106.00	244.50	0.0500
108.99	244.05	0.0500 / 0.0300
109.00	243.50	0.0300
114.00	243.50	0.0300
114.01	244.05	0.0300 / 0.0500
118.99	244.50	0.0500
132.36	245.09	0.0500
150.52	245.83	0.0500
199.83	249.24	0.0500
207.62	250.16	0.0500
236.16	253.11	0.0500
256.92	253.83	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELRV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.28	243.77	.365E+04	0.7	0.54	81.42
0.55	244.05	.730E+04	2.2	0.81	54.59
1.05	244.55	.197E+05	7.2	0.97	45.51
1.54	245.04	.463E+05	17.2	0.99	44.76
2.04	245.54	.895E+05	34.8	1.03	42.81
2.53	246.03	.152E+06	62.8	1.09	40.43
3.03	246.53	.232E+06	104.7	1.20	36.91
3.52	247.02	.328E+06	159.7	1.29	34.19
4.02	247.52	.450E+06	223.4	1.32	33.56
4.51	248.01	.627E+06	326.9	1.38	31.99
5.01	248.51	.820E+06	463.0	1.50	29.52
5.50	249.00	.103E+07	626.0	1.62	27.30
6.00	249.50	.124E+07	820.1	1.75	25.24
6.49	249.99	.147E+07	1044.4	1.89	23.42
6.99	250.49	.170E+07	1292.6	2.01	21.95
7.48	250.98	.195E+07	1565.6	2.13	20.71
7.98	251.48	.220E+07	1864.5	2.25	19.65
8.47	251.97	.246E+07	2189.4	2.36	18.73
8.97	252.47	.274E+07	2427.5	2.35	18.80

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8084) 3769.99 73.13 7.00 34.00 2.65 1.12
OUTFLOW : ID= 1 (6200) 3769.99 63.66 7.75 34.00 2.54 1.09

```

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CALIB
STANDHYD (2001) | Area (ha)= 6.00
ID= 1 DT=15.0 min | Total Imp(%)= 50.00 Dir. Conn.(%)= 23.00

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```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 3.00 3.00
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 200.00 40.00
Mannings n = 0.013 0.250

```

Max.Eff.Inten.(mm/hr)= 106.66 89.58
 over (min) 15.00 30.00
 Storage Coeff. (min)= 4.65 (ii) 15.82 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.11 0.05

TOTALS
 PEAK FLOW (cms)= 0.40 0.42 0.671 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 78.80 37.58 47.29
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.47 0.59

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 67.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5200)
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.4010	0.1620
0.0200	0.0789	0.5040	0.1833
0.1160	0.0921	0.5870	0.2041
0.2170	0.1159	0.9870	0.2141
0.3000	0.1370	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
6.000	0.671	6.00	47.29
6.000	0.278	6.50	47.12

INFLOW : ID= 2 (2001)
 OUTFLOW: ID= 1 (5200)

PEAK FLOW REDUCTION [Qout/Qin](%)= 41.43
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 0.1321

CALIB
 STANDHYD (2002)
 ID= 1 DT=15.0 min

Area (ha)= 109.30
 Total Imp(%)= 22.00 Dir. Conn.(%)= 10.00

IMPERVIOUS (i)	PERVIOUS (i)
24.05	85.25
2.00	5.00
0.50	0.50
853.62	40.00
0.013	0.250

Max.Eff.Inten.(mm/hr)= 106.66 55.85
 over (min) 15.00 30.00
 Storage Coeff. (min)= 11.10 (ii) 24.60 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

TOTALS
 PEAK FLOW (cms)= 2.65 6.07 7.079 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.25
 RUNOFF VOLUME (mm)= 78.80 31.62 36.34
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.39 0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 67.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8086)
 1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (2002): 109.30	7.079	6.25	36.34
+ ID2= 2 (5200): 6.00	0.278	6.50	47.12
ID = 3 (8086): 115.30	7.310	6.25	36.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8088)
 1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
-----------	-------------	-------------	-----------

ID1= 1 (8200): 3769.99 63.657 7.75 34.00
 + ID2= 2 (8086): 115.30 7.310 6.25 36.90
 ID = 3 (8088): 3885.29 64.979 7.75 34.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8038)
 1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8034): 5220.81	23.758	11.25	19.27
+ ID2= 2 (8088): 3885.29	64.979	7.75	34.09
ID = 3 (8038): 9106.10	84.675	7.75	25.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (1365)
 DT= 2--> OUT= 1
 Routing time step (min)= 15.00

DATA FOR SECTION (1362.0)

Distance	Elevation	Manning
0.00	252.00	0.0400
2.35	244.60	0.0400
39.94	243.14	0.0400
58.74	242.65	0.0400
91.63	242.03	0.0400
101.03	241.69	0.0400
103.38	241.66	0.0400
105.73	240.76	0.0400
105.93	240.59	0.0400
109.18	240.28	0.0400 / 0.0300
110.43	239.81	0.0300
111.68	239.81	0.0300
113.43	240.09	0.0300 / 0.0400
118.43	240.50	0.0400
138.62	241.68	0.0400
152.72	241.60	0.0400
166.82	242.68	0.0400
185.62	249.09	0.0400
197.36	251.00	0.0400
232.61	252.28	0.0400

TRAVEL TIME TABLE

DEPTH (m)	RELV (m)	VOLUME (cu.m)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.28	240.09	.118E+04	0.1	0.20	142.09
0.91	240.72	.114E+05	2.4	0.35	79.92
1.53	241.34	.352E+05	9.4	0.45	62.17
2.16	241.97	.831E+05	22.8	0.46	60.69
2.79	242.60	.172E+06	54.2	0.53	52.74
3.41	243.22	.297E+06	111.4	0.63	44.49
4.04	243.85	.445E+06	194.1	0.74	38.22
4.67	244.48	.612E+06	301.1	0.83	33.88
5.29	245.10	.793E+06	443.0	0.94	29.82
5.92	245.73	.976E+06	611.2	1.06	26.60
6.55	246.36	.118E+07	802.0	1.17	24.12
7.18	246.99	.135E+07	1014.0	1.27	22.16
7.80	247.61	.154E+07	1246.2	1.37	20.56
8.43	248.24	.173E+07	1498.0	1.46	19.24
9.06	248.87	.192E+07	1768.8	1.55	18.12
9.68	249.49	.212E+07	2049.6	1.63	17.24
10.31	250.12	.232E+07	2344.7	1.71	16.49
10.94	250.75	.252E+07	2659.5	1.78	15.83
11.56	251.37	.274E+07	2931.2	1.81	15.56

hydrograph

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8038) 9106.10	84.67	7.75	25.60	3.12	0.58
OUTFLOW: ID= 1 (1365) 9106.10	73.96	8.50	25.60	3.00	0.56

CALIB
 NASHYD (1322)
 ID= 1 DT=15.0 min

Area (ha)= 249.10 Curve Number (CN)= 73.0
 Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.N. Tp(hrs)= 0.83

Unit Hyd Qpeak (cms)= 5.130
 PEAK FLOW (cms)= 4.604 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 33.393
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.413

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB
 STANDHYD (1321) | Area (ha)= 41.70
 ID= 1 DT=15.0 min | Total Imp(%)= 55.00 Dir. Conn.(%)= 35.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	22.94	18.77
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	527.26	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	106.66	87.15
over (min)	15.00	30.00
Storage Coeff. (min)=	8.31 (ii)	19.61 (iii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.05
		TOTALS
PEAK FLOW (cms)=	3.04	2.33
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	78.80	39.17
TOTAL RAINFALL (mm)=	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.48

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR (5132)
 IN= 2 -> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.9630	1.4050
0.1780	1.0280	1.4870	1.5525
0.4300	1.0400	1.9900	1.7000
0.8150	1.0500	2.3900	1.7100
0.8990	1.2275	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1321)	41.700	5.354	6.00	53.04
OUTFLOW: ID= 1 (5132)	41.700	0.933	7.00	53.01

PEAK FLOW REDUCTION [Qout/Qin](%)= 17.43
 TIME SHIFT OF PEAK FLOW (min)= 60.00
 MAXIMUM STORAGE USED (ha.m.)= 1.3226

 ADD HYD (8090)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (1322):	249.10	4.604	7.00	33.39
+ ID= 2 (5132):	41.70	0.933	7.00	53.01
ID = 3 (8090):	290.80	5.537	7.00	36.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB
 NASHYD (0134) | Area (ha)= 155.95
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.64

Unit Hyd Qpeak (cms)= 4.167

PEAK FLOW (cms)=	2.687 (i)
TIME TO PEAK (hrs)=	6.750
RUNOFF VOLUME (mm)=	25.755
TOTAL RAINFALL (mm)=	80.800
RUNOFF COEFFICIENT =	0.319

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8092)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (0134):	155.95	2.687	6.75	25.75
+ ID= 2 (8090):	290.80	5.537	7.00	36.20
ID = 3 (8092):	446.75	8.172	7.00	32.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (1366)
 IN= 2 -> OUT= 1

Routing time step (min)= 15.00

----- DATA FOR SECTION (1361.0) -----

Distance	Elevation	Manning
0.00	262.81	0.0400
37.23	260.54	0.0400
79.12	256.98	0.0400
111.70	254.74	0.0400
172.20	252.76	0.0400
251.32	248.82	0.0400
255.97	248.75	0.0400
260.43	248.76	0.0400
265.43	248.49	0.0400
268.68	248.18	0.0400 / 0.0300
269.93	247.71	0.0300
271.18	247.71	0.0300
272.93	247.99	0.0300
277.93	248.40	0.0300 / 0.0400
283.90	248.88	0.0400
307.17	249.60	0.0400
321.13	251.59	0.0400
363.01	257.08	0.0400
400.25	257.58	0.0400
460.75	261.91	0.0400

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.47	248.18	.881E+04	1.3	0.76	114.66
1.19	248.90	.742E+05	17.3	1.22	71.44
1.92	249.63	.278E+06	78.3	1.48	59.26
2.64	250.35	.589E+06	210.3	1.87	46.69
3.36	251.07	.974E+06	414.3	2.23	39.19
4.08	251.79	.143E+07	696.2	2.55	34.32
4.81	252.52	.197E+07	1063.0	2.83	30.86
5.53	253.24	.259E+07	1505.7	3.06	28.62
6.25	253.96	.331E+07	2051.3	3.26	26.85
6.97	254.68	.413E+07	2720.9	3.46	25.30
7.70	255.41	.504E+07	3584.5	3.73	23.43
8.42	256.13	.601E+07	4580.9	4.00	21.87
9.14	256.85	.704E+07	5707.9	4.25	20.57
9.86	257.57	.818E+07	6936.1	4.26	20.53
10.59	258.30	.945E+07	8115.0	4.50	19.42
11.31	259.02	.108E+08	9759.2	4.74	18.45
12.03	259.74	.122E+08	11569.5	4.96	17.61
12.75	260.46	.137E+08	13550.6	5.18	16.87
13.48	261.19	.153E+08	15636.4	5.37	16.29

----- hydrograph ----- <-pipe / channel-->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8092)	446.75	8.17	7.00	32.56	0.78	0.91
OUTFLOW: ID= 1 (1366)	446.75	5.81	8.75	32.56	0.67	0.85

 CALIB
 STANDHYD (1362) | Area (ha)= 410.10
 ID= 1 DT=15.0 min | Total Imp(%)= 21.00 Dir. Conn.(%)= 11.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	86.12	323.98
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1653.48	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	106.66	50.66
over (min)	15.00	45.00
Storage Coeff. (min)=	16.50 (ii)	30.54 (iii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.07	0.03

PEAK FLOW (cms)=	9.34	17.11
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	78.80	29.49
TOTAL RAINFALL (mm)=	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.36

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (1361) Area (ha)= 102.30
 ID= 1 DT=15.0 min Total Imp(%)= 55.00 Dir. Conn.(%)= 27.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	56.27	46.03
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	825.83	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	106.66	92.70
over (min)=	15.00	30.00
Storage Coeff. (min)=	10.88 (ii)	21.91 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.05
	TOTALS	
PEAK FLOW (cms)=	6.73	5.78
TIME TO PEAK (hrs)=	6.00	6.00 (iii)
RUNOFF VOLUME (mm)=	78.80	48.49
TOTAL RAINFALL (mm)=	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.60

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5136)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.4000	3.2690
0.1600	1.6363	2.6450	3.8640
0.9600	1.7470	2.8900	4.4590
1.8400	1.9320	3.2900	4.4690
2.1200	2.6005	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (1361)	102.300	10.460	6.00
OUTFLOW: ID= 1 (5136)	102.300	2.210	7.00

PEAK FLOW REDUCTION [Qout/Qin](%)= 21.13
 TIME SHIFT OF PEAK FLOW (min)= 60.00
 MAXIMUM STORAGE USED (ha.m.)= 2.8226

ADD HYD (8096)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1362):	410.10	20.157	6.50	34.91
+ ID2= 2 (5136):	102.30	2.210	7.00	48.46
ID = 3 (8096):	512.40	22.254	6.50	37.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8094)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1365):	446.75	5.815	8.75	32.56
+ ID2= 2 (8096):	512.40	22.254	6.50	37.62
ID = 3 (8094):	959.15	24.467	6.50	35.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8098)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1365):	9106.10	73.956	8.50	25.60
+ ID2= 2 (8094):	959.15	24.467	6.50	35.26
ID = 3 (8098):	10065.25	85.129	8.50	26.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB

NASHYD (0142) Area (ha)= 396.54 Curve Number (CN)= 63.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.32

Unit Hyd Qpeak (cms)= 5.133
 PEAK FLOW (cms)= 3.803 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 25.366
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.314

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0138) Area (ha)= 414.79 Curve Number (CN)= 24.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.39

Unit Hyd Qpeak (cms)= 5.087
 PEAK FLOW (cms)= 0.949 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 6.487
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.080

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0140) Area (ha)= 158.58
 ID= 1 DT=15.0 min Total Imp(%)= 24.00 Dir. Conn.(%)= 21.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	38.06	120.52
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1028.20	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	106.66	12.46
over (min)=	15.00	45.00
Storage Coeff. (min)=	12.41 (ii)	37.02 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03
	TOTALS	
PEAK FLOW (cms)=	7.75	2.34
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	78.80	13.50
TOTAL RAINFALL (mm)=	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.17

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 41.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8102)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0138):	414.79	0.949	8.00	6.49
+ ID2= 2 (0140):	158.58	8.690	6.00	27.22
ID = 3 (8102):	573.37	9.123	6.00	12.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6142)
 IN= 2--> OUT= 1

Routing time step (min)= 15.00

----- DATA FOR SECTION (1421.0) -----

Distance	Elevation	Manning
0.00	295.50	0.0400
7.25	295.36	0.0400
14.50	295.22	0.0400
21.75	295.22	0.0400
29.00	294.23	0.0400
36.25	286.45	0.0400
43.50	281.44	0.0400
50.75	278.28	0.0400
58.00	277.20	0.0400 / 0.0300
65.25	276.80	0.0300
72.50	276.80	0.0300

Main Channel
 Main Channel
 Main Channel

96.41	276.80	0.0300	Main Channel
96.81	277.20	0.0300 / 0.0400	Main Channel
103.31	278.96	0.0400	
108.75	280.16	0.0400	
117.82	282.73	0.0400	
135.94	290.30	0.0400	
159.50	292.14	0.0400	
174.00	293.07	0.0400	
179.44	293.37	0.0400	

212.75	254.73	0.0350	
222.65	252.86	0.0350	
232.44	250.70	0.0350 / 0.0300	Main Channel
236.44	250.20	0.0300	Main Channel
236.84	249.80	0.0300	Main Channel
242.44	249.80	0.0300	Main Channel
250.04	249.80	0.0300	Main Channel
250.44	250.20	0.0300	Main Channel
252.33	250.37	0.0300 / 0.0350	Main Channel
286.97	253.52	0.0350	
331.50	255.80	0.0350	
380.98	255.55	0.0350	
445.30	258.40	0.0350	
455.19	259.82	0.0350	
489.83	262.22	0.0350	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.40	277.20	.238E+05	6.9	1.27	57.28
1.25	278.05	.889E+05	50.9	2.50	29.13
2.10	278.90	.179E+06	133.4	3.26	22.36
2.95	279.75	.293E+06	256.7	3.83	19.03
3.80	280.60	.432E+06	426.0	4.31	16.90
4.66	281.46	.593E+06	644.3	4.75	15.35
5.51	282.31	.776E+06	915.3	5.16	14.12
6.36	283.16	.978E+06	1242.2	5.55	13.12
7.21	284.01	.120E+07	1626.2	5.94	12.27
8.06	284.86	.143E+07	2065.6	6.30	11.57
8.91	285.71	.169E+07	2562.4	6.64	10.97
9.76	286.56	.196E+07	3118.0	6.97	10.46
10.61	287.41	.224E+07	3733.3	7.28	10.02
11.46	288.26	.255E+07	4411.9	7.57	9.62
12.31	289.11	.287E+07	5155.6	7.86	9.27
13.17	289.97	.321E+07	5966.6	8.14	8.96
14.02	290.82	.357E+07	6705.0	8.22	8.87
14.87	291.67	.398E+07	7494.3	8.24	8.85
15.72	292.52	.444E+07	8405.3	8.28	8.80

----- hydrograph ----- <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8102)	573.37	9.12	6.00	12.22	0.44
OUTFLOW : ID= 1 (6142)	573.37	3.92	6.75	12.22	0.23

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.39	250.19	.169E+05	5.4	1.04	52.00
0.77	250.57	.373E+05	16.6	1.44	37.46
1.44	251.24	.911E+05	60.3	2.14	25.19
2.11	251.91	.169E+06	136.3	2.61	20.64
2.79	252.59	.270E+06	249.7	2.99	18.05
3.46	253.26	.396E+06	405.4	3.31	16.28
4.13	253.93	.549E+06	603.3	3.56	15.15
4.80	254.60	.737E+06	857.8	3.77	14.31
5.48	255.28	.963E+06	1174.6	3.95	13.67
6.15	255.95	.128E+07	1449.5	3.66	14.74
6.82	256.62	.173E+07	2046.4	3.83	14.07
7.49	257.29	.227E+07	2665.2	3.81	14.17
8.17	257.97	.310E+07	3705.2	3.87	13.93
8.84	258.64	.400E+07	5127.4	4.15	12.99
9.51	259.31	.492E+07	6898.6	4.51	11.95
10.18	259.98	.597E+07	8815.5	4.86	11.09
10.86	260.66	.684E+07	10954.6	5.18	10.41
11.53	261.33	.785E+07	13324.3	5.49	9.82
12.20	262.00	.890E+07	15923.2	5.79	9.31

----- hydrograph ----- <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8106)	1105.60	9.01	7.00	19.64	0.55
OUTFLOW : ID= 1 (6146)	1105.60	3.01	8.00	19.64	0.51

ADD HYD (8104)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0142):	396.54	3.803	8.00	25.37
+ ID2= 2 (6142):	573.37	3.915	6.75	12.22
ID = 3 (8104):	969.91	7.344	7.00	17.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0144)	135.69	74.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50
	U.H. Tp(hrs)= 0.71	

Unit Hyd Qpeak (cms) = 3.240

PEAK FLOW (cms) = 2.899 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 34.229
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.424

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8106)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0144):	135.69	2.899	6.75	34.23
+ ID2= 2 (8104):	969.91	7.344	7.00	17.59
ID = 3 (8106):	1105.60	10.220	7.00	19.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6146)	Routing time step (min) = 15.00
IN= 2--> OUT= 1	

----- DATA FOR SECTION (1461.0) -----

Distance	Elevation	Manning
0.00	262.00	0.0350
39.58	257.13	0.0350
123.69	257.51	0.0350
183.07	256.83	0.0350
202.86	255.73	0.0350

CALIB	STANDHYD (1462)	Area (ha)	Dir. Conn. (%)
ID= 1 DT=15.0 min	Total Imp (%) = 29.00	125.00	12.00

Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
36.25		88.75
Dep. Storage (mm) = 2.00		5.00
Average Slope (%) = 0.50		0.50
Length (m) = 912.87		40.00
Mannings n = 0.013		0.250
Max. Eff. Inten. (mm/hr) = 106.66		73.97
over (min) = 15.00		30.00
Storage Coeff. (min) = 11.55 (ii)		23.62 (ii)
Unit Hyd. Tpeak (min) = 15.00		30.00
Unit Hyd. peak (cms) = 0.08		0.04
PEAK FLOW (cms) = 3.58		8.58
TIME TO PEAK (hrs) = 6.00		6.25
RUNOFF VOLUME (mm) = 78.80		38.63
TOTAL RAINFALL (mm) = 80.80		80.80
RUNOFF COEFFICIENT = 0.98		0.48

TOTAL
 10.003 (iii)
 43.45
 80.80
 0.54

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 73.0 Ia = Dep. Storage (above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (1461)	Area (ha)	Dir. Conn. (%)
ID= 1 DT=15.0 min	Total Imp (%) = 36.00	70.50	15.00

Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
25.38		45.12
Dep. Storage (mm) = 2.00		5.00
Average Slope (%) = 0.50		0.50
Length (m) = 685.57		40.00
Mannings n = 0.013		0.250
Max. Eff. Inten. (mm/hr) = 106.66		82.42

over (min) 15.00 30.00
 Storage Coeff. (min)= 9.73 (ii) 21.29 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.05

PEAK FLOW (cms)= 2.67 5.11 6.018 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.25
 RUNOFF VOLUME (mm)= 78.80 40.16 45.96
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.50 0.57

*****TOTALS*

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
- CN* = 73.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5146)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	8.0640	2.2344
	0.2390	0.7085	9.9830	2.6409
	2.2880	0.8391	11.7350	3.0631
	4.3270	1.4661	12.1350	3.0731
	6.1170	1.8197	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (1461)	70.500	6.018	6.25	45.96
OUTFLOW: ID= 1 (5146)	70.500	3.540	6.50	45.95

PEAK FLOW REDUCTION [Qout/Qin](%)= 58.82				
TIME SHIFT OF PEAK FLOW (min)= 15.00				
	MAXIMUM	STORAGE	USED	(ha.m.)= 1.2403

ADD HYD (8099)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1462):	125.00	10.003	6.25	43.45
+ ID2= 2 (5146):	70.50	3.540	6.50	45.95
*****	*****	*****	*****	*****
ID = 3 (8099):	195.50	13.057	6.25	44.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8108)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6146):	1105.60	9.006	8.00	19.64
+ ID2= 2 (8099):	195.50	13.057	6.25	44.35
*****	*****	*****	*****	*****
ID = 3 (8108):	1301.10	16.253	6.25	23.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8100)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8098):	10065.25	85.129	8.50	26.52
+ ID2= 2 (8108):	1301.10	16.253	6.25	23.35
*****	*****	*****	*****	*****
ID = 3 (8100):	11366.35	95.844	8.25	26.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6202) | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2021.0) ----->			
Distance	Elevation	Manning	
0.00	245.48	0.0400	
29.68	242.80	0.0400	
74.20	240.39	0.0400	
155.81	237.12	0.0400	
200.33	234.48	0.0400	
244.85	234.67	0.0400	
259.69	233.96	0.0400	

274.53	233.84	0.0400	
281.68	233.40	0.0400	
285.78	232.40	0.0400 / 0.0300	Main Channel
286.78	231.40	0.0300	Main Channel
296.78	231.40	0.0300	Main Channel
299.78	232.40	0.0300 / 0.0400	Main Channel
311.62	233.74	0.0400	
333.88	234.34	0.0400	
497.11	235.32	0.0400	
571.31	237.32	0.0400	
675.18	239.10	0.0400	
704.86	242.71	0.0400	
734.54	242.62	0.0400	

<----- TRAVEL TIME TABLE ----->						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.50	231.90	.221E+05	4.7	0.85	78.67	
1.00	232.40	.482E+05	15.3	1.28	52.40	
1.60	233.00	.913E+05	38.3	1.68	39.77	
2.20	233.60	1.54E+06	72.9	1.90	35.24	
2.80	234.20	.278E+06	124.2	1.79	37.30	
3.40	234.80	.599E+06	214.3	1.44	46.59	
4.01	235.41	1.25E+07	445.0	1.43	46.90	
4.61	236.01	.205E+07	838.4	1.64	40.81	
5.21	236.61	.293E+07	1361.6	1.86	35.89	
5.81	237.21	.389E+07	2011.2	2.08	32.23	
6.41	237.81	.494E+07	2753.5	2.24	29.91	
7.01	238.41	.612E+07	3635.0	2.39	28.04	
7.61	239.01	.741E+07	4672.5	2.53	26.43	
8.21	239.61	.880E+07	5999.3	2.74	24.44	
8.82	240.22	1.02E+08	7510.0	2.94	22.72	
9.42	240.82	1.17E+08	9201.9	3.15	21.23	
10.02	241.42	1.32E+08	11059.7	3.35	19.95	
10.62	242.02	1.48E+08	13073.4	3.54	18.87	
11.22	242.62	1.64E+08	15242.1	3.73	17.93	

<---- hydrograph ---->						
<-pipe / channel->						
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8100) *****	95.84	8.25	26.15	2.47	1.85	
OUTFLOW: ID= 1 (6202) *****	91.98	9.00	26.15	2.42	1.86	

ADD HYD (8112)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6202):	11366.35	91.977	9.00	26.15
+ ID2= 2 (8110):	468.50	40.315	6.00	53.06
*****	*****	*****	*****	*****
ID = 3 (8112):	11834.85	96.040	9.00	27.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5555)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	*****	19.2100
	2.8500	0.5200	*****	24.7300
	8.1800	1.2600	*****	30.8600
	15.0900	2.2100	*****	37.6100
	23.2700	3.3900	*****	41.2200
	32.5300	4.7800	*****	48.9000
	36.5100	5.3900	*****	66.4700
	42.7600	6.3700	*****	100.1200
	53.8500	8.1300	*****	140.4100
	91.7300	14.3200	*****	140.4200

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8112) *****	96.040	9.00	27.22	
OUTFLOW: ID= 1 (5555) *****	94.779	9.50	27.22	

PEAK FLOW REDUCTION [Qout/Qin](%)= 98.69				
TIME SHIFT OF PEAK FLOW (min)= 30.00				
	MAXIMUM	STORAGE	USED	(ha.m.)= 14.8583

ADD HYD (8114)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (5555):	11834.85	94.779	9.50	27.22
+ ID2= 2 (8136):	2288.40	30.030	6.50	26.33
*****	*****	*****	*****	*****
ID = 3 (8114):	14123.25	112.204	9.00	27.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

* ID2= 2 (5206): 25.00 1.244 6.50 74.19
 ID = 3 (8144): 275.50 31.597 6.00 60.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (2061) Area (ha)= 25.00
 ID= 1 DT=15.0 min Total Imp(%)= 93.00 Dir. Conn.(%)= 60.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	23.25	1.75	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	408.25	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	106.66	566.17	
over (min)	15.00	15.00	
Storage Coeff. (min)=	7.13 (ii)	12.48 (ii)	
Unit Hyd. Tpeak (min)=	15.00	15.00	
Unit Hyd. peak (cms)=	0.10	0.08	
TOTALS			
PEAK FLOW (cms)=	4.08	2.14	6.218 (iii)
TIME TO PEAK (hrs)=	6.00	6.00	6.00
RUNOFF VOLUME (mm)=	78.80	67.43	74.25
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.83	0.92

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5206)
 IN= 2--> OUT= 1
 DT= 15.0 min

		OUTFLOW	STORAGE	OUTFLOW	STORAGE
		(cms)	(ha.m.)	(cms)	(ha.m.)
		0.0000	0.0000	1.8990	1.7295
		0.0850	0.5280	2.1560	1.9661
		0.8990	0.8577	2.3740	2.2023
		1.3460	1.1601	2.7740	2.2123
		1.6060	1.4451	0.0000	0.0000

		AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (2061)	25.000	6.218	6.00	74.25	
OUTFLOW: ID= 1 (5206)	25.000	1.244	6.50	74.19	

PEAK FLOW REDUCTION [Qout/Qin](%)= 20.01
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 1.0930

CALIB
 STANDHYD (2062) Area (ha)= 250.50
 ID= 1 DT=15.0 min Total Imp(%)= 62.00 Dir. Conn.(%)= 39.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	155.31	95.19	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	1292.28	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	106.66	114.81	
over (min)	15.00	30.00	
Storage Coeff. (min)=	14.23 (ii)	24.35 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.07	0.04	
TOTALS			
PEAK FLOW (cms)=	21.55	14.21	30.895 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	78.80	46.27	58.96
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.57	0.73

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8144)
 1 + 2 = 3
 ID1= 1 (2062): 250.50 30.895 6.00 58.96

CALIB
 STANDHYD (2041) Area (ha)= 201.50
 ID= 1 DT=15.0 min Total Imp(%)= 36.00 Dir. Conn.(%)= 20.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	72.54	128.96	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	1159.02	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	106.66	54.18	
over (min)	15.00	30.00	
Storage Coeff. (min)=	13.33 (ii)	27.00 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.08	0.04	
TOTALS			
PEAK FLOW (cms)=	9.13	8.48	14.420 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	78.80	28.53	38.59
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.35	0.48

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5204)
 IN= 2--> OUT= 1
 DT= 15.0 min

		OUTFLOW	STORAGE	OUTFLOW	STORAGE
		(cms)	(ha.m.)	(cms)	(ha.m.)
		0.0000	0.0000	12.4660	4.9468
		0.5050	1.9141	15.4770	5.3726
		2.8390	2.2056	18.2610	6.4806
		5.2410	3.6606	18.6610	6.4906
		7.9920	4.3236	0.0000	0.0000

		AREA	QPEAK	TPEAK	R.V.
		(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (2041)	201.500	14.420	6.00	38.59	
OUTFLOW: ID= 1 (5204)	201.500	5.123	7.00	38.58	

PEAK FLOW REDUCTION [Qout/Qin](%)= 35.53
 TIME SHIFT OF PEAK FLOW (min)= 60.00
 MAXIMUM STORAGE USED (ha.m.)= 3.6272

CALIB
 STANDHYD (2042) Area (ha)= 122.90
 ID= 1 DT=15.0 min Total Imp(%)= 60.00 Dir. Conn.(%)= 33.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	73.74	49.16	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	905.17	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	106.66	88.58	
over (min)	15.00	30.00	
Storage Coeff. (min)=	11.49 (ii)	22.72 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.08	0.04	
TOTALS			
PEAK FLOW (cms)=	9.70	5.79	13.409 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	78.80	34.65	49.22
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.43	0.61

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8140)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (2042):	122.90	13.409	6.00	49.22
+ ID2= 2 (5204):	201.50	5.123	7.00	38.58

ID = 3 (8140):	324.40	13.879	6.00	42.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6206)				
IN# 2--> OUT# 1	Routing time step (min)'= 15.00			

----- DATA FOR SECTION (2061.0) -----				
Distance	Elevation	Manning		
0.00	253.07	0.0350		
13.98	252.89	0.0350		
27.95	251.99	0.0350		
34.94	252.18	0.0350		
41.93	252.02	0.0350		
73.38	250.35	0.0350		
94.34	247.09	0.0350		
111.81	244.79	0.0350		
115.31	244.24	0.0350		
117.55	244.20	0.0350 / 0.0300	Main Channel	
118.80	243.60	0.0300	Main Channel	
122.30	243.60	0.0300	Main Channel	
122.55	244.60	0.0300 / 0.0350	Main Channel	
146.76	244.74	0.0350		
164.23	247.26	0.0350		
213.15	248.92	0.0350		
241.10	249.51	0.0350		
272.55	250.87	0.0350		
300.50	252.26	0.0350		
345.93	253.67	0.0350		

----- TRAVEL TIME TABLE -----					
DEPTH	ELIV	VOLUME	FLOW RATE	VELOCITY	TRAV TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.30	243.90	.310E+04	1.4	1.23	36.25
0.60	244.20	.676E+04	4.6	1.81	24.64
1.12	244.72	.221E+05	17.1	2.07	21.60
1.64	245.24	.754E+05	64.1	2.28	19.60
2.17	245.77	.139E+06	149.8	2.88	15.50
2.69	246.29	.214E+06	273.3	3.43	13.04
3.21	246.81	.299E+06	436.1	3.91	11.43
3.73	247.33	.395E+06	632.6	4.30	10.40
4.25	247.85	.511E+06	836.4	4.39	10.18
4.77	248.37	.653E+06	1114.9	4.58	9.76
5.30	248.90	.822E+06	1472.1	4.81	9.30
5.82	249.42	.102E+07	1872.2	4.91	9.10
6.34	249.94	.126E+07	2444.6	5.22	8.56
6.86	250.46	.151E+07	3113.1	5.52	8.09
7.38	250.98	.180E+07	3843.5	5.74	7.79
7.90	251.50	.211E+07	4717.8	6.00	7.45
8.43	252.03	.245E+07	5701.4	6.24	7.16
8.95	252.55	.284E+07	6727.5	6.36	7.03
9.47	253.07	.326E+07	7885.3	6.48	6.90

----- hydrograph -----						
<-pipe / channel->						
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8140)	324.40	13.88	6.00	42.61	0.99	2.00
OUTFLOW : ID= 1 (6206)	324.40	11.19	6.50	42.61	0.87	1.94

ADD HYD (8142)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6206):	324.40	11.189	6.50	42.61
+ ID2= 2 (8144):	275.50	31.597	6.00	60.34

ID = 3 (8142):	599.90	37.223	6.00	50.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8138)				
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8114):	14123.25	112.204	9.00	27.08
+ ID2= 2 (8143):	599.90	37.223	6.00	50.75

ID = 3 (8138):	14723.15	119.210	8.50	28.04

ROUTE CHN (6208)		
IN# 2--> OUT# 1	Routing time step (min)'= 15.00	

----- DATA FOR SECTION (2081.0) -----			
Distance	Elevation	Manning	
0.00	249.19	0.1000	
73.11	244.13	0.1000	
121.86	243.39	0.1000	
243.71	240.01	0.1000	
353.38	238.75	0.1000	
450.87	235.40	0.1000	
499.61	231.40	0.1000	
511.80	230.96	0.1000	
523.98	230.12	0.1000	
534.50	227.80	0.1000 / 0.0500	Main Channel
536.50	226.70	0.0500	Main Channel
546.50	226.70	0.0500	Main Channel
548.50	227.80	0.0500 / 0.1000	Main Channel
572.72	230.25	0.1000	
682.40	230.50	0.1000	
804.25	233.95	0.1000	
926.11	235.83	0.1000	
1047.96	240.70	0.1000	
1145.45	243.26	0.1000	
1206.38	247.03	0.1000	

----- TRAVEL TIME TABLE -----					
DEPTH	ELIV	VOLUME	FLOW RATE	VELOCITY	TRAV TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.55	227.25	.111E+05	2.0	0.33	92.44
1.10	227.80	.242E+05	6.5	0.50	61.56
2.23	228.93	.700E+05	26.0	0.68	44.91
3.36	230.06	.150E+06	60.8	0.74	41.04
4.49	231.19	.446E+06	130.8	0.54	56.83
5.62	232.32	.925E+06	287.4	0.57	53.66
6.76	233.46	.152E+07	522.9	0.63	48.32
7.89	234.59	.223E+07	827.2	0.68	44.91
9.02	235.72	.312E+07	1219.5	0.72	42.61
10.15	236.85	.418E+07	1780.6	0.78	39.11
11.28	237.98	.537E+07	2479.2	0.85	36.07
12.41	239.11	.669E+07	3282.9	0.90	33.95
13.54	240.24	.824E+07	4228.6	0.94	32.47
14.67	241.37	.997E+07	5384.6	0.99	30.87
15.81	242.51	.119E+08	6698.4	1.03	29.56
16.94	243.64	.140E+08	8235.2	1.08	28.25
18.07	244.77	.162E+08	10104.2	1.14	26.73
19.20	245.90	.185E+08	12251.5	1.21	25.21
20.33	247.03	.209E+08	14597.0	1.28	23.89

----- hydrograph -----						
<-pipe / channel->						
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8138)	*****	119.21	8.50	28.04	4.31	0.56
OUTFLOW : ID= 1 (6208)	*****	110.93	10.00	28.04	4.18	0.58

CALIB			
STANDHYD (2082)	Area	(ha)= 426.60	
ID= 1 DT=15.0 min	Total Imp(%)=	64.00	Dir. Conn.(%)= 37.00

IMPERVIOUS			PERVIOUS (i)		
Surface Area	(ha)=	273.02	153.58		
Dep. Storage	(mm)=	2.00	5.00		
Average Slope	(%)=	0.50	0.50		
Length	(m)=	1686.42	40.00		
Mannings n	=	0.013	0.250		
Max. Ref. Inten. (mm/hr)=		106.66	116.83		
over (min)		15.00	30.00		
Storage Coeff. (min)=	16.70 (ii)	26.75 (ii)			
Unit Hyd. Peak (min)=	15.00	30.00			
Unit Hyd. peak (cms)=	0.07	0.04			
PEAK FLOW (cms)=	32.49	22.21	46.93 (iii)		
TIME TO PEAK (hrs)=	6.00	6.25	6.00		
RUNOFF VOLUME (mm)=	78.80	43.35	56.47		
TOTAL RAINFALL (mm)=	80.80	80.80	80.80		
RUNOFF COEFFICIENT =	0.98	0.54	0.70		

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

STANDHYD (2081) Area (ha)= 265.70
 ID= 1 DT=15.0 min Total Imp(%)= 35.00 Dir. Conn.(%)= 20.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 93.00 172.71
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1330.91 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten.(mm/hr)= 106.66 67.52
 Cover (min)= 15.00 30.00
 Storage Coeff. (min)= 14.49 (ii) 27.00 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.07 0.04

PEAK FLOW (cms)= 11.64 14.22 **TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.25 20.662 (iii)
 RUNOFF VOLUME (mm)= 78.80 35.65 44.28
 TOTAL RAINFALL (mm)= 80.60 80.60 80.80
 RUNOFF COEFFICIENT = 0.98 0.44 0.55

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5208)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	15.8530	6.3371
0.8360	2.7353	21.1630	7.3975
4.7810	3.1070	25.1140	8.4406
8.9050	4.2484	25.5140	8.4506
12.6070	5.1796	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
265.700	20.662	6.00	44.28
265.700	10.813	6.75	44.28

INFLOW : ID= 2 (2081) 265.700 20.662 6.00 44.28
 OUTFLOW: ID= 1 (5208) 265.700 10.813 6.75 44.28

PEAK FLOW REDUCTION [Qout/Qin](%)= 52.33
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 4.7832

ADD HYD (8148)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (2082):	426.60	46.933	6.00	56.47
+ ID2= 2 (5208):	265.70	10.813	6.75	44.28
=====				
ID = 3 (8148):	692.30	47.701	6.00	51.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8146)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (5208):	14723.15	110.934	10.00	28.04
+ ID2= 2 (8148):	692.30	47.701	6.00	51.79
=====				
ID = 3 (8146):	15415.45	115.482	10.00	29.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6210)
 IN= 2--> OUT= 1

Routing time step (min)= 15.00

<----- DATA FOR SECTION (2101.0) ----->

Distance	Elevation	Manning
0.00	241.81	0.0800
28.43	241.78	0.0800
85.30	237.04	0.0800
149.27	231.32	0.0800
170.60	230.10	0.0800
255.89	229.37	0.0800
277.22	228.68	0.0800
284.33	228.42	0.0800
291.43	228.16	0.0800

297.76	227.00	0.0800 / 0.0350	Main Channel
298.76	225.00	0.0350	Main Channel
312.76	225.00	0.0350	Main Channel
313.76	227.00	0.0350 / 0.0800	Main Channel
326.97	229.50	0.0800	
348.30	230.29	0.0800	
376.73	229.76	0.0800	
490.46	235.75	0.0800	
568.55	236.24	0.0800	
661.06	241.85	0.0800	
703.71	246.04	0.0800	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.67	225.67	.141E+05	6.6	0.69	35.86
1.33	226.33	.289E+05	20.4	1.04	23.65
2.00	227.00	.443E+05	39.3	1.31	18.81
2.93	227.93	.730E+05	77.8	1.57	15.64
3.85	228.85	.123E+06	131.1	1.57	15.65
4.78	229.78	.228E+06	205.6	1.33	18.51
5.70	230.70	.503E+06	356.7	1.05	23.49
6.63	231.63	.846E+06	608.7	1.06	23.17
7.55	232.55	.123E+07	946.4	1.14	21.65
8.48	233.48	.165E+07	1367.0	1.22	20.13
9.41	234.41	.211E+07	1871.8	1.31	18.80
10.33	235.33	.261E+07	2463.2	1.40	17.65
11.26	236.26	.317E+07	2999.4	1.40	17.63
12.18	237.18	.384E+07	3848.3	1.48	16.63
13.11	238.11	.454E+07	4806.0	1.56	15.75
14.03	239.03	.528E+07	5876.9	1.64	14.97
14.96	239.96	.608E+07	7063.7	1.72	14.29
15.88	240.88	.687E+07	8365.0	1.80	13.68
16.81	241.81	.771E+07	9506.9	1.82	13.52

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
218.27	115.39	10.00	29.11	3.56	1.57

INFLOW : ID= 2 (8146) *****
 OUTFLOW: ID= 1 (6210) *****

CALIB (0210)

Area (ha)= 218.27 Curve Number (CN)= 80.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.08

Unit Hyd Qpeak (cms)= 3.454

PEAK FLOW (cms)= 4.041 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 40.871
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.506

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8150)

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0210):	218.27	4.041	7.25	40.87
+ ID2= 2 (6210):	15415.45	115.393	10.00	29.11
=====				
ID = 3 (8150):	15633.72	118.274	9.75	29.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0218)

Area (ha)= 152.25 Curve Number (CN)= 69.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.79

Unit Hyd Qpeak (cms)= 3.278

PEAK FLOW (cms)= 2.582 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 29.818
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.369

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0216)

Area (ha)= 145.77 Curve Number (CN)= 75.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.52

Unit Hyd Qpeak (cms) = 4.768

PEAK FLOW (cms) = 4.123 (i)
TIME TO PEAK (hrs) = 6.500
RUNOFF VOLUME (mm) = 34.852
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.431

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (2121) | Area (ha) = 56.00
| ID= 1 DT=15.0 min | Total Imp(%) = 93.00 Dir. Conn.(%) = 75.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 52.08 3.92
Dep. Storage (mm) = 2.00 5.00
Average Slope (%) = 0.50 0.50
Length (m) = 611.01 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr) = 106.66 307.06
over (min) 15.00 30.00
Storage Coeff. (min) = 9.08 (ii) 15.91 (iii)
Unit Hyd. Tpeak (min) = 15.00 30.00
Unit Hyd. peak (cms) = 0.09 0.05

TOTALS
PEAK FLOW (cms) = 10.80 1.89 12.113 (iii)
TIME TO PEAK (hrs) = 6.00 6.25 6.00
RUNOFF VOLUME (mm) = 78.80 56.62 73.25
TOTAL RAINFALL (mm) = 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.70 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5212)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.7460	1.9729
0.1900	1.1850	4.7040	2.0838
1.0850	1.4413	5.4760	2.1940
2.0210	1.5949	5.8760	2.2040
2.8030	1.7848	0.0000	0.0000

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (2121) 56.000 12.113 6.00 73.25
OUTFLOW: ID= 1 (5212) 56.000 4.586 6.25 73.25

PEAK FLOW REDUCTION [Qout/Qin](%) = 37.86
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha.m.) = 2.1034

CALIB
STANDHYD (2122) | Area (ha) = 272.40
| ID= 1 DT=15.0 min | Total Imp(%) = 29.00 Dir. Conn.(%) = 23.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 79.00 193.40
Dep. Storage (mm) = 2.00 5.00
Average Slope (%) = 0.50 0.50
Length (m) = 1347.59 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr) = 106.66 53.23
over (min) 15.00 30.00
Storage Coeff. (min) = 14.59 (ii) 28.36 (iii)
Unit Hyd. Tpeak (min) = 15.00 30.00
Unit Hyd. peak (cms) = 0.07 0.04

TOTALS
PEAK FLOW (cms) = 13.68 12.18 21.309 (iii)
TIME TO PEAK (hrs) = 6.00 6.25 6.00
RUNOFF VOLUME (mm) = 78.80 32.00 42.76
TOTAL RAINFALL (mm) = 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.40 0.53

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8152)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (2122): 272.40 21.309 6.00 42.76
+ ID2= 2 (5212): 56.00 4.586 6.25 73.25
ID = 3 (8152): 328.40 23.287 6.25 47.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6214)
IN= 2--> OUT= 1 | Routing time step (min) = 15.00

DATA FOR SECTION (2141.0)

Distance	Elevation	Manning
0.00	259.29	0.0500
15.33	258.91	0.0500
38.33	258.57	0.0500
65.16	257.93	0.0500
95.82	253.44	0.0500
111.15	251.21	0.0500
130.32	249.67	0.0500
149.48	248.51	0.0500
150.66	248.50	0.0500 / 0.0370
150.81	247.50	0.0370
154.66	247.50	0.0370
155.66	248.50	0.0370 / 0.0500
160.98	248.50	0.0500
187.81	249.78	0.0500
199.31	252.13	0.0500
245.30	254.04	0.0500
264.46	253.99	0.0500
298.96	255.23	0.0500
329.62	257.02	0.0500
379.45	258.82	0.0500

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.50	248.00	.648E+04	2.1	1.00	52.30
1.00	248.50	.139E+05	6.4	1.44	36.35
1.61	249.11	.570E+05	22.6	1.24	42.01
2.21	249.71	.143E+06	63.6	1.39	37.61
2.82	250.32	.263E+06	142.9	1.70	30.69
3.43	250.93	.403E+06	257.2	2.00	26.11
4.04	251.54	.562E+06	410.4	2.29	22.82
4.64	252.14	.735E+06	601.3	2.56	20.37
5.25	252.75	.933E+06	791.0	2.66	19.67
5.86	253.36	.117E+07	1038.0	2.79	18.74
6.46	253.96	.144E+07	1344.4	2.93	17.82
7.07	254.57	.178E+07	1664.1	2.93	17.83
7.68	255.18	.217E+07	2140.2	3.10	16.87
8.28	255.78	.259E+07	2722.9	3.20	15.83
8.89	256.39	.304E+07	3387.1	3.50	14.94
9.50	257.00	.351E+07	4132.7	3.69	14.16
10.11	257.61	.402E+07	4913.0	3.83	13.64
10.71	258.21	.458E+07	5648.3	3.87	13.50
11.32	258.82	.521E+07	6343.1	3.82	13.68

hydrograph
INFLOW : ID= 2 (8152) 328.40 23.29 6.25 47.96 1.62 1.24
OUTFLOW: ID= 1 (6214) 328.40 14.92 6.75 47.96 1.32 1.33

CALIB
STANDHYD (0214) | Area (ha) = 316.95
| ID= 1 DT=15.0 min | Total Imp(%) = 25.00 Dir. Conn.(%) = 9.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 79.24 237.71
Dep. Storage (mm) = 2.00 5.00
Average Slope (%) = 0.50 0.50
Length (m) = 1453.62 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr) = 106.66 52.91
over (min) 15.00 30.00
Storage Coeff. (min) = 15.27 (ii) 29.07 (iii)
Unit Hyd. Tpeak (min) = 15.00 30.00
Unit Hyd. peak (cms) = 0.07 0.04

TOTALS
PEAK FLOW (cms) = 6.11 14.66 17.672 (iii)
TIME TO PEAK (hrs) = 6.00 6.25 6.25

RUNOFF VOLUME (mm)= 78.80 28.68 33.19
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.35 0.41

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 62.0 Is = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8154)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0214):	316.95	17.672	6.25	33.19
+ ID2= 2 (6214):	328.40	14.922	6.75	47.96
ID = 3 (8154):	645.35	30.007	6.25	40.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8156)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0216):	145.77	4.123	6.50	34.85
+ ID2= 2 (8154):	645.35	30.007	6.25	40.71
ID = 3 (8156):	791.12	33.881	6.25	39.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6218) |
 | I#= 2--> OUTF= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2181.0) ----->				
Distance	Elevation	Manning		
0.00	256.38	0.0450		
7.70	254.85	0.0450		
15.40	253.20	0.0450		
19.25	252.29	0.0450		
23.10	251.17	0.0450		
78.94	236.16	0.0450		
80.87	235.80	0.0450		
82.79	235.45	0.0450		
84.57	234.60	0.0450 /0.0300	Main Channel	
85.07	233.60	0.0300	Main Channel	
88.57	233.60	0.0300	Main Channel	
93.57	233.60	0.0300	Main Channel	
94.07	234.60	0.0300 /0.0450	Main Channel	
96.27	235.13	0.0450		
98.19	235.20	0.0450		
100.12	235.32	0.0450		
138.63	239.95	0.0450		
180.98	244.80	0.0450		
188.69	245.00	0.0450		
190.61	256.00	0.0450		

<----- TRAVEL TIME TABLE ----->					
DEPTH	RELV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
1.00	234.60	.181E+05	6.0	0.66	50.64
2.13	235.73	.518E+05	23.8	0.93	36.27
3.25	236.85	.119E+06	60.0	1.02	33.03
4.38	237.98	.217E+06	120.6	1.12	29.99
5.51	239.11	.346E+06	210.5	1.23	27.39
6.63	240.23	.506E+06	334.1	1.33	25.22
7.76	241.36	.697E+06	495.6	1.43	23.44
8.88	242.48	.920E+06	699.8	1.53	21.91
10.01	243.61	.117E+07	950.8	1.63	20.59
11.14	244.74	.146E+07	1252.5	1.73	19.45
12.26	245.86	.178E+07	1619.6	1.83	18.37
13.39	246.99	.212E+07	2078.5	1.97	17.01
14.52	248.12	.247E+07	2589.0	2.11	15.88
15.64	249.24	.282E+07	3149.7	2.25	14.94
16.77	250.37	.319E+07	3759.8	2.37	14.14
17.89	251.49	.356E+07	4419.5	2.50	13.44
19.02	252.62	.395E+07	5126.4	2.61	12.84
20.15	253.75	.435E+07	5871.6	2.72	12.34
21.27	254.87	.476E+07	6663.2	2.82	11.89

<----- hydrograph ----->						<-pipe / channel->	
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL		
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW : ID= 2 (8156)	791.12	33.88	6.25	39.63	2.44	0.95	
OUTFLOW : ID= 1 (6218)	791.12	24.67	6.75	39.63	2.16	0.93	

ADD HYD (8158)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0218):	152.25	2.582	7.00	29.82
+ ID2= 2 (6218):	791.12	24.673	6.75	39.63
ID = 3 (8158):	943.37	27.234	6.75	38.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8151)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8150):	15633.72	118.274	9.75	29.27
+ ID2= 2 (8158):	943.37	27.234	6.75	38.04
ID = 3 (8151):	16577.09	132.759	7.00	29.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5504)				
I#= 2--> OUTF= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	33.0100	0.6700
	2.6400	0.0500	37.4800	0.7800
	4.8300	0.0900	47.5400	1.0100
	7.4100	0.1300	60.2300	1.2800
	10.3300	0.1900	71.9300	1.5258
	13.5300	0.2500	*****	3.0529
	16.9900	0.3200	*****	5.3299
	20.6800	0.3900	*****	8.4148
	24.6000	0.4800	*****	11.7637
	28.7100	0.5700	*****	11.7737

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8151)	*****	132.759	7.00	29.77
OUTFLOW : ID= 1 (5504)	*****	132.578	7.00	29.77

PEAK FLOW REDUCTION [Qout/Qin](%)= 99.86
 TIME SHIFT OF PEAK FLOW (min)= 0.00
 MAXIMUM STORAGE USED (ha.m.)= 2.8201

ROUTE CHN (6220) |
 | I#= 2--> OUTF= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2201.0) ----->				
Distance	Elevation	Manning		
0.00	245.59	0.0600		
5.76	245.45	0.0600		
23.03	242.27	0.0600		
57.58	232.35	0.0600		
86.37	226.18	0.0600		
109.41	224.31	0.0600		
115.17	223.86	0.0600		
120.92	222.78	0.0600		
126.68	221.93	0.0600		
128.06	221.50	0.0600 /0.0300	Main Channel	
129.56	220.00	0.0300	Main Channel	
141.56	220.00	0.0300	Main Channel	
143.06	221.50	0.0300 /0.0600	Main Channel	
149.72	223.77	0.0600		
155.47	225.09	0.0600		
253.36	229.21	0.0600		
333.98	227.50	0.0600		
454.90	229.25	0.0600		
500.97	233.11	0.0600		
570.07	237.42	0.0600		

<----- TRAVEL TIME TABLE ----->					
DEPTH	RELV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.75	220.75	.262E+05	5.5	0.57	79.44
1.50	221.50	.505E+05	17.5	0.86	52.89
2.44	222.44	.103E+06	42.7	1.14	40.03
3.37	223.37	.171E+06	79.4	1.27	35.98
4.31	224.31	.263E+06	128.1	1.33	34.22
5.25	225.25	.393E+06	191.7	1.34	34.19
6.18	226.18	.594E+06	276.7	1.28	35.77
7.12	227.12	.872E+06	399.9	1.26	36.33
8.06	228.06	.127E+07	542.7	1.17	38.92
8.99	228.99	.199E+07	770.0	1.06	43.01

9.93	229.93	.297E+07	1151.8	1.06	42.95
10.86	230.86	.400E+07	1652.9	1.13	40.33
11.80	231.80	.507E+07	2251.8	1.22	37.53
12.74	232.74	.618E+07	2946.5	1.31	34.97
13.67	233.67	.733E+07	3726.8	1.39	32.79
14.61	234.61	.853E+07	4591.0	1.48	30.96
15.55	235.55	.977E+07	5544.1	1.55	29.37
16.48	236.48	.111E+08	6586.7	1.63	27.99
17.42	237.42	.124E+08	7719.2	1.71	26.77

<---- hydrograph ----> <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
132.58	7.00	29.77	4.38	1.33	
127.03	9.75	29.77	4.29	1.33	

INFLOW : ID= 2 (5504) *****

OUTFLOW : ID= 1 (6220) *****

CALIB	Area	QPEAK	TPEAK	R.V.
(ha)	(ha)	(cms)	(hrs)	(mm)
169.10	1.967	8.000	33.66	
16577.09	127.031	9.75	29.77	
16746.19	128.836	9.50	29.81	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	QPEAK	TPEAK	R.V.
(ha)	(ha)	(cms)	(hrs)	(mm)
169.10	1.967	8.000	33.66	
16577.09	127.031	9.75	29.77	
16746.19	128.836	9.50	29.81	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	QPEAK	TPEAK	R.V.
(ha)	(ha)	(cms)	(hrs)	(mm)
237.58	3.852	7.250	48.25	
127.031	9.75	29.77		
128.836	9.50	29.81		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	QPEAK	TPEAK	R.V.
(ha)	(ha)	(cms)	(hrs)	(mm)
145.20	36.00	19.00		
52.27	92.93			
2.00	5.00			
0.50	0.50			
983.87	40.00			
0.013	0.250			
106.66	80.54			
15.00	30.00			
12.08	23.75			
15.00	30.00			
0.08	0.04			
6.48	9.78			
6.00	6.25			
78.80	41.09			
80.80	80.80			
0.98	0.51			

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	QPEAK	TPEAK	R.V.
(ha)	(ha)	(cms)	(hrs)	(mm)
140.45	34.00	26.00		
47.75	92.70			
2.00	5.00			
0.50	0.50			
967.64	40.00			
0.013	0.250			
106.66	74.51			
15.00	30.00			
11.96	24.00			
15.00	30.00			
0.08	0.04			
8.61	8.99			
6.00	6.25			
78.80	42.68			
80.80	80.80			
0.98	0.53			

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5222)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min	0.0000	0.0000	6.6430	3.7677
	0.4620	2.0014	8.2830	4.2576
	2.0160	2.2850	14.1970	4.6909
	3.6250	2.7987	14.5970	4.7009
	5.0230	3.2496	0.0000	0.0000

AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
145.200	12.813	6.00	48.25
145.200	5.253	6.75	48.24

INFLOW : ID= 2 (2221)

OUTFLOW : ID= 1 (5222)

PEAK FLOW REDUCTION [Qout/Qin](%)= 41.00

TIME SHIFT OF PEAK FLOW (min)= 45.00

MAXIMUM STORAGE USED (ha.m.)= 3.3405

CALIB	Area	QPEAK	TPEAK	R.V.
(ha)	(ha)	(cms)	(hrs)	(mm)
74.30	78.00	41.00		

STANDHYD (2222)

ID= 1 DT=15.0 min

IMPERVIOUS	PERVIOUS (i)
57.95	16.35
2.00	5.00
0.50	0.50
703.80	40.00
0.013	0.250
106.66	230.59
15.00	30.00
9.88	17.54
15.00	30.00
0.09	0.05
7.65	5.70
6.00	6.25
78.80	56.38
80.80	80.80
0.98	0.70

***** TOTALS*
PEAK FLOW (cms)= 11.588 (iii)
TIME TO PEAK (hrs)= 6.00
RUNOFF VOLUME (mm)= 65.57
TOTAL RAINFALL (mm)= 80.80
RUNOFF COEFFICIENT = 0.81

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	QPEAK	TPEAK	R.V.
(ha)	(ha)	(cms)	(hrs)	(mm)
74.30	11.588	6.00	65.57	
145.20	5.253	6.75	48.24	
219.50	11.946	6.00	54.11	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	QPEAK	TPEAK	R.V.
(ha)	(ha)	(cms)	(hrs)	(mm)
140.45	34.00	26.00		

STANDHYD (0224)

ID= 1 DT=15.0 min

IMPERVIOUS	PERVIOUS (i)
47.75	92.70
2.00	5.00
0.50	0.50
967.64	40.00
0.013	0.250
106.66	74.51
15.00	30.00
11.96	24.00
15.00	30.00
0.08	0.04
8.61	8.99
6.00	6.25
78.80	42.68
80.80	80.80
0.98	0.53

***** TOTALS*
PEAK FLOW (cms)= 14.439 (liii)
TIME TO PEAK (hrs)= 6.00
RUNOFF VOLUME (mm)= 52.07
TOTAL RAINFALL (mm)= 80.80
RUNOFF COEFFICIENT = 0.64

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
CN = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8166)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0224):	140.45	14.439	6.00	52.07
+ ID2= 2 (8164):	219.50	11.946	6.00	54.11
=====				
ID = 3 (8166):	359.95	26.385	6.00	53.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6226) | Routing time step (min)'= 15.00
IN= 2--> OUT= 1 |

<----- DATA FOR SECTION (2261.0) ----->					
Distance	Elevation	Manning			
0.00	245.35	0.0600			
8.27	242.72	0.0600			
16.54	240.01	0.0600			
31.02	235.31	0.0600			
41.36	233.14	0.0600			
90.59	227.73	0.0600			
93.06	227.50	0.0600			
95.13	227.27	0.0600			
95.83	227.00	0.0600			
100.83	226.50	0.0600 / 0.0350	Main Channel		
101.33	225.50	0.0350	Main Channel		
101.83	226.50	0.0350 / 0.0600	Main Channel		
105.83	227.00	0.0600			
107.54	227.39	0.0600			
109.60	227.62	0.0600			
111.67	227.86	0.0600			
146.83	231.76	0.0600			
173.71	234.77	0.0600			
186.12	238.23	0.0600			
202.67	238.35	0.0600			

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.50	226.00	403E+03	0.0	0.16	330.59
1.00	226.50	1.61E+04	0.1	0.26	208.26
1.70	227.20	1.73E+05	1.5	0.27	195.72
2.39	227.89	5.41E+05	5.9	0.35	152.27
3.09	228.59	1.19E+06	16.0	0.43	124.08
3.79	229.29	2.12E+06	33.8	0.51	104.65
4.49	229.99	3.34E+06	61.2	0.59	90.97
5.18	230.68	4.84E+06	99.7	0.66	80.88
5.88	231.38	6.63E+06	151.0	0.73	73.13
6.58	232.08	8.70E+06	216.5	0.80	66.95
7.27	232.77	1.11E+07	297.5	0.87	61.91
7.97	233.47	1.37E+07	396.1	0.94	57.29
8.67	234.17	1.65E+07	518.9	1.01	53.12
9.36	234.86	1.96E+07	658.9	1.08	49.59
10.06	235.56	2.28E+07	827.0	1.17	46.02
10.76	236.26	2.62E+07	1015.4	1.25	42.96
11.46	236.96	2.96E+07	1221.2	1.33	40.41
12.15	237.65	3.32E+07	1444.6	1.40	38.26
12.85	238.35	3.68E+07	1598.2	1.40	38.42

<----- hydrograph ----->					
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8166)	359.95	26.39	6.00	53.31	3.50
OUTFLOW : ID= 1 (6226)	359.95	8.73	6.75	53.31	2.59

ADD HYD (8168)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0226):	237.58	3.852	7.25	35.47
+ ID2= 2 (6226):	359.95	8.734	6.75	53.31
=====				
ID = 3 (8168):	597.53	12.407	6.75	46.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8162)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8160):	16746.19	120.836	9.50	29.81
+ ID2= 2 (8168):	597.53	12.407	6.75	46.22
=====				
ID = 3 (8162):	17343.72	137.689	8.50	30.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6228) | Routing time step (min)'= 15.00
IN= 2--> OUT= 1 |

<----- DATA FOR SECTION (2281.0) ----->					
Distance	Elevation	Manning			
0.00	245.00	0.0600			
18.60	244.74	0.0600			
37.20	242.47	0.0600			
93.01	233.71	0.0600			
125.57	226.95	0.0600			
213.93	221.63	0.0600			
218.58	221.15	0.0600			
223.23	220.45	0.0600			
225.03	220.55	0.0600 / 0.0300	Main Channel		
225.53	218.05	0.0300	Main Channel		
232.53	218.05	0.0300	Main Channel		
240.03	218.05	0.0300	Main Channel		
241.03	220.55	0.0300 / 0.0600	Main Channel		
246.48	221.00	0.0600			
251.13	221.27	0.0600			
255.78	221.22	0.0600			
325.54	225.25	0.0600			
367.40	228.91	0.0600			
404.60	231.75	0.0600			
460.41	245.00	0.0600			

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
1.25	219.30	338E+05	16.0	0.86	35.15
2.50	220.55	6.96E+05	48.3	1.26	24.02
3.94	221.99	1.68E+06	116.0	1.26	24.14
5.38	223.43	3.92E+06	247.5	1.15	26.37
6.81	224.86	7.43E+06	473.6	1.16	26.20
8.25	226.30	1.22E+07	821.6	1.23	24.67
9.69	227.74	1.79E+07	1320.1	1.34	22.59
11.13	229.18	2.43E+07	1965.7	1.47	20.58
12.57	230.62	3.13E+07	2744.2	1.59	19.02
14.01	232.06	3.90E+07	3683.4	1.72	17.66
15.44	233.49	4.72E+07	4824.1	1.86	16.30
16.88	234.93	5.57E+07	6092.4	1.99	15.23
18.32	236.37	6.46E+07	7503.3	2.11	14.35
19.76	237.81	7.39E+07	9061.2	2.23	13.59
21.20	239.25	8.36E+07	10767.6	2.34	12.94
22.64	240.69	9.37E+07	12624.8	2.45	12.37
24.07	242.12	1.04E+08	14634.9	2.56	11.87
25.51	243.56	1.15E+08	16762.8	2.65	11.45
26.95	245.00	1.27E+08	18699.9	2.69	11.28

<----- hydrograph ----->					
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8162)	137.69	8.50	30.37	4.18	1.24
OUTFLOW : ID= 1 (6228)	135.70	9.50	30.37	4.16	1.24

CALIB (0228) | Area (ha)= 310.54 Curve Number (CN)= 61.0
 NASHYD (0228) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.12

Unit Hyd Qpeak (cms)= 4.751
 PEAK FLOW (cms)= 3.173 (1)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 23.913
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.296

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8170)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0228):	310.54	3.173	7.50	23.91
+ ID2= 2 (6228):	17343.72	135.701	9.50	30.37
=====				

ID = 3 (8170): 17654.26 138.376 9.25 30.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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| CALIB |
| STANDBY (2302) | Area (ha)= 126.70
| ID= 1 DT=15.0 min | Total Imp(%)= 23.00 Dir. Conn.(%)= 9.00
-----
| IMPERVIOUS | PERVIOUS (i) |
| Surface Area (ha)= 29.14 | 97.56 |
| Dep. Storage (mm)= 2.00 | 5.00 |
| Average Slope (%)= 0.50 | 0.50 |
| Length (m)= 919.06 | 40.00 |
| Mannings n = 0.013 | 0.250 |
| Max.Eff.Inten.(mm/hr)= 106.66 | 61.47 |
| over (min) 15.00 | 30.00 |
| Storage Coeff. (min)= 11.60 (ii) | 24.60 (ii) |
| Unit Hyd. Tpeak (min)= 15.00 | 30.00 |
| Unit Hyd. peak (cms)= 0.08 | 0.04 |
|
| PEAK FLOW (cms)= 2.72 | 7.66 |
| TIME TO PEAK (hrs)= 6.00 | 6.25 |
| RUNOFF VOLUME (mm)= 78.80 | 33.86 |
| TOTAL RAINFALL (mm)= 80.80 | 80.80 |
| RUNOFF COEFFICIENT = 0.98 | 0.42 |
|
| *****
| TOTALS* |
| 8.736 (iii) |
|
| *****
| WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
| WARNING:FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
| YOU SHOULD CONSIDER SPLITTING THE AREA.
|
| (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
| CN = 69.0 Ia = Dep. Storage (Above)
| (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
| THAN THE STORAGE COEFFICIENT.
| (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
|
-----

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-----
| CALIB |
| STANDBY (2301) | Area (ha)= 78.80
| ID= 1 DT=15.0 min | Total Imp(%)= 50.00 Dir. Conn.(%)= 19.00
-----
| IMPERVIOUS | PERVIOUS (i) |
| Surface Area (ha)= 39.40 | 39.40 |
| Dep. Storage (mm)= 2.00 | 5.00 |
| Average Slope (%)= 0.50 | 0.50 |
| Length (m)= 724.80 | 40.00 |
| Mannings n = 0.013 | 0.250 |
| Max.Eff.Inten.(mm/hr)= 106.66 | 101.64 |
| over (min) 15.00 | 30.00 |
| Storage Coeff. (min)= 10.06 (ii) | 20.69 (ii) |
| Unit Hyd. Tpeak (min)= 15.00 | 30.00 |
| Unit Hyd. peak (cms)= 0.09 | 0.05 |
|
| PEAK FLOW (cms)= 3.74 | 5.58 |
| TIME TO PEAK (hrs)= 6.00 | 6.25 |
| RUNOFF VOLUME (mm)= 78.80 | 40.76 |
| TOTAL RAINFALL (mm)= 80.80 | 80.80 |
| RUNOFF COEFFICIENT = 0.98 | 0.50 |
|
| *****
| TOTALS* |
| 7.381 (iii) |
|
| *****
| WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
| WARNING:FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
| YOU SHOULD CONSIDER SPLITTING THE AREA.
|
| (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
| CN = 69.0 Ia = Dep. Storage (Above)
| (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
| THAN THE STORAGE COEFFICIENT.
| (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
|
-----

```

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-----
| RESERVOIR (5230) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
| OUTFLOW | STORAGE | OUTFLOW | STORAGE |
| (cms) | (ha.m.) | (cms) | (ha.m.) |
| 0.0000 | 0.0000 | 5.6350 | 1.5914 |
| 0.3160 | 0.7624 | 6.9200 | 1.8001 |
| 1.7040 | 0.8931 | 7.9010 | 2.0070 |
| 3.5140 | 1.1218 | 8.3010 | 2.0170 |
| 4.5020 | 1.3405 | 0.0000 | 0.0000 |
|
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) |
| INFLOW : ID= 2 (2301) | 78.800 | 7.381 | 6.00 | 47.99 |
| OUTFLOW : ID= 1 (5230) | 78.800 | 4.586 | 6.50 | 47.98 |
|
| PEAK FLOW REDUCTION [Qout/Qin](%)= 62.13
| TIME SHIFT OF PEAK FLOW (min)= 30.00
| MAXIMUM STORAGE USED (ha.m.)= 1.3679
|
-----

```

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-----
| ADD HYD (8172) |
| 1 + 2 = 3 |
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) |
| ID= 1 (2302): | 126.70 | 8.736 | 6.25 | 37.91 |
| + ID2= 2 (5230): | 78.80 | 4.586 | 6.50 | 47.98 |
| *****
| ID = 3 (8172): | 205.50 | 12.783 | 6.25 | 41.77 |
|
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHBY (0232) | Area (ha)= 314.80 Curve Number (CN)= 58.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.00 |
-----
| Unit Hyd Qpeak (cms)= 5.391 |
| PEAK FLOW (cms)= 3.203 (i) |
| TIME TO PEAK (hrs)= 7.250 |
| RUNOFF VOLUME (mm)= 21.895 |
| TOTAL RAINFALL (mm)= 80.800 |
| RUNOFF COEFFICIENT = 0.271 |
|
| (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
|
-----

```

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-----
| ADD HYD (8173) |
| 1 + 2 = 3 |
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) |
| ID= 1 (0232): | 314.80 | 3.203 | 7.25 | 21.89 |
| + ID2= 2 (8172): | 205.50 | 12.783 | 6.25 | 41.77 |
| *****
| ID = 3 (8173): | 520.30 | 15.216 | 6.25 | 29.75 |
|
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8174) |
| 1 + 2 = 3 |
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) |
| ID= 1 (8170): | 17654.26 | 138.376 | 9.25 | 30.26 |
| + ID2= 2 (8173): | 520.30 | 15.216 | 6.25 | 29.75 |
| *****
| ID = 3 (8174): | 18174.56 | 142.190 | 9.25 | 30.25 |
|
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6234) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00
-----

```

<----- DATA FOR SECTION (2341.0) ----->

Distance	Elevation	Manning
0.00	228.00	0.0550
42.86	223.21	0.0550
85.72	219.56	0.0550
107.15	219.42	0.0550
128.58	219.39	0.0550
214.30	219.30	0.0550
225.02	219.26	0.0550
235.73	219.24	0.0550
255.16	219.20	0.0550 / 0.0350
257.16	217.20	0.0350
303.16	217.20	0.0350
305.16	219.20	0.0350 / 0.0550
310.74	219.28	0.0550
321.45	219.27	0.0550
332.17	219.26	0.0550
396.46	219.29	0.0550
782.20	222.91	0.0550
878.64	227.43	0.0550
964.36	227.80	0.0550
1060.79	228.00	0.0550

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.50	217.70	.887E+05	7.2	0.31	206.54
1.00	218.20	.179E+06	22.7	0.48	131.74
1.50	218.70	.272E+06	44.5	0.62	101.75
2.00	219.20	.368E+06	71.8	0.75	84.97
2.50	219.70	.493E+06	136.1	0.92	71.59
3.17	220.37	.189E+07	258.6	0.52	121.80
3.76	220.96	.294E+07	434.7	0.56	112.82
4.35	221.55	.415E+07	665.9	0.61	103.88

4.93	222.13	.551E+07	955.0	0.66	96.22
5.52	222.72	.703E+07	1305.1	0.71	89.80
6.11	223.31	.868E+07	1744.9	0.77	82.91
6.69	223.89	.104E+08	2261.4	0.83	76.46
7.28	224.48	.121E+08	2838.0	0.89	71.12
7.87	225.07	.139E+08	3472.9	0.95	66.63
8.45	225.65	.157E+08	4165.0	1.01	62.82
9.04	226.24	.176E+08	4913.5	1.07	59.54
9.63	226.83	.194E+08	5717.7	1.12	56.68
10.21	227.41	.214E+08	6577.1	1.17	54.18
10.80	228.00	.235E+08	6919.5	1.12	56.61

```

<---- hydrograph ----> <-pipe / channel-->
AREA   QPEAK  TPEAK  R.V.  MAX DEPTH  MAX VEL
(ha)   (cms)  (hrs)  (mm)  (m)        (m/s)
INFLOW : ID= 2 (8174) ***** 142.19  9.25 30.25  2.62  0.52
OUTFLOW: ID= 1 (6234) ***** 122.82  12.25 30.25  2.47  0.56

```

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-----
| CALIB |
| STANHYD (0234) | Area (ha)= 267.16
| ID= 1 DT=15.0 min | Total Imp(%)= 22.00 Dir. Conn.(%)= 8.00
-----
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 58.78 208.38
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1334.57 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 106.66 14.89
over (min)= 18.00 45.00
Storage Coeff. (min)= 14.51 (ii) 37.42 (ii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.07 0.03
*TOTALS*
PEAK FLOW (cms)= 4.68 4.80 6.621 (iii)
TIME TO PEAK (hrs)= 6.00 6.50 6.00
RUNOFF VOLUME (mm)= 78.80 14.18 19.35
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.18 0.24

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CN* = 39.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8176) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
| ID= 1 (0234): 267.16 6.621 6.00 19.35
+ ID= 2 (6234): 18174.56 122.817 12.25 30.25
=====
| ID = 3 (8176): 18441.72 123.200 12.25 30.09

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0238) | Area (ha)= 311.70 Curve Number (CN)= 53.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.56

```

```

Unit Hyd Qpeak (cms)= 3.405
PEAK FLOW (cms)= 1.940 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 18.985
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.235

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0236) | Area (ha)= 494.49 Curve Number (CN)= 54.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.24

```

```

Unit Hyd Qpeak (cms)= 6.830
PEAK FLOW (cms)= 3.795 (i)
TIME TO PEAK (hrs)= 8.000

```

```

RUNOFF VOLUME (mm)= 19.519
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.242

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ROUTE CHN (6238) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

```

```

----- DATA FOR SECTION (2381.0) -----
Distance Elevation Manning
0.00 241.54 0.0380
602.55 227.00 0.0380
1702.00 224.50 0.0380
1721.25 223.00 0.0380
1725.10 222.60 0.0380
1780.94 222.50 0.0380
1782.87 222.45 0.0380
1784.79 222.40 0.0380 / 0.0300 Main Channel
1786.57 221.75 0.0300 Main Channel
1787.07 220.75 0.0300 Main Channel
1787.57 220.75 0.0300 Main Channel
1791.57 220.75 0.0300 Main Channel
1794.07 221.75 0.0300 Main Channel
1798.27 222.00 0.0300 Main Channel
1800.19 222.35 0.0300 / 0.0380 Main Channel
1802.12 222.47 0.0380
1840.63 223.00 0.0380
1900.00 225.00 0.0380
1950.00 226.00 0.0380
2242.61 240.00 0.0380

```

```

----- TRAVEL TIME TABLE -----
DEPTH ELLEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.80 221.55 .101E+05 5.2 1.14 32.36
1.60 222.25 .296E+05 15.7 3.40 26.36
2.64 223.39 .249E+06 151.8 1.35 27.33
3.68 224.43 .613E+06 512.2 1.85 19.94
4.71 225.46 .152E+07 1066.8 1.55 23.80
5.75 226.50 .359E+07 2824.9 1.75 21.16
6.79 227.54 .663E+07 6528.0 2.18 16.93
7.83 228.58 .994E+07 12158.6 2.71 13.62
8.87 229.62 .134E+08 19268.8 3.19 11.58
9.91 230.66 .170E+08 27785.3 3.63 10.19
10.94 231.69 .207E+08 37667.1 4.03 9.18
11.98 232.73 .246E+08 48891.7 4.40 8.40
13.02 233.77 .287E+08 61448.0 4.75 7.78
14.06 234.81 .329E+08 75332.7 5.08 7.28
15.10 235.85 .372E+08 90547.8 5.39 6.85
16.14 236.89 .417E+08 ***** 5.69 6.49
17.17 237.92 .464E+08 ***** 5.98 6.18
18.21 238.96 .512E+08 ***** 6.25 5.91
19.25 240.00 .561E+08 ***** 6.52 5.67

```

```

----- hydrograph ----> <-pipe / channel-->
AREA   QPEAK  TPEAK  R.V.  MAX DEPTH  MAX VEL
(ha)   (cms)  (hrs)  (mm)  (m)        (m/s)
INFLOW : ID= 2 (0236) 494.49  3.80 8.00 19.52  0.58  1.14
OUTFLOW: ID= 1 (6238) 494.49  3.68 8.50 19.52  0.56  1.14

```

```

-----
| ADD HYD (8180) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
| ID= 1 (0238): 311.70 1.940 8.25 18.99
+ ID= 2 (6238): 494.49 3.676 8.50 19.52
=====
| ID = 3 (8180): 806.19 5.612 8.50 19.31

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8178) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
| ID= 1 (8176): 18441.72 123.200 12.25 30.09
+ ID= 2 (8180): 806.19 5.612 8.50 19.31
=====
| ID = 3 (8178): 19247.91 127.205 11.75 29.64

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6240) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

```

----- DATA FOR SECTION (2401.0) -----

Table with 3 columns: Distance, Elevation, Manning. Rows show data points from 0.00 to 1379.30.

----- TRAVEL TIME TABLE -----

Table with 6 columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV.TIME. Rows show hydrograph data from 0.20 to 4.19.

Summary table with 6 columns: AREA, QPEAK, TPEAK, R.V., MAX DEPTH, MAX VEL. Includes INFLOW and OUTFLOW data.

CALIB table for NASHYD (0240) with parameters: Area, Curve Number, U.H. Tp.

Unit Hyd Qpeak (cms) = 2.058
PEAK FLOW (cms) = 1.142 (i)
TIME TO PEAK (hrs) = 12.000
RUNOFF VOLUME (mm) = 15.341
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.190

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8182) table with 5 columns: AREA, QPEAK, TPEAK, R.V. Includes ID1, ID2, ID3 data.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB table for NASHYD (0242) with parameters: Area, Curve Number, U.H. Tp.

Unit Hyd Qpeak (cms) = 2.090
PEAK FLOW (cms) = 1.476 (i)
TIME TO PEAK (hrs) = 12.750
RUNOFF VOLUME (mm) = 18.503

TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.229

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8184) table with 5 columns: AREA, QPEAK, TPEAK, R.V. Includes ID1, ID2, ID3 data.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6244) table with 2 columns: ROUTE CHN, Routing time step.

----- DATA FOR SECTION (2441.0) -----

Table with 3 columns: Distance, Elevation, Manning. Rows show data points from 0.00 to 360.18.

----- TRAVEL TIME TABLE -----

Table with 6 columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV.TIME. Rows show hydrograph data from 0.35 to 7.19.

Summary table with 6 columns: AREA, QPEAK, TPEAK, R.V., MAX DEPTH, MAX VEL. Includes INFLOW and OUTFLOW data.

CALIB table for NASHYD (0244) with parameters: Area, Curve Number, U.H. Tp.

Unit Hyd Qpeak (cms) = 14.812

PEAK FLOW (cms) = 11.577 (i)
TIME TO PEAK (hrs) = 9.500
RUNOFF VOLUME (mm) = 24.046
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.298

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns for time (e.g., 41.75, 42.00, 42.25), elevation (e.g., 5.548, 5.447, 5.350), and flow characteristics (e.g., 106.75, 107.00, 107.25). It represents a hydrograph or flow profile data.

ADD HYD (8188)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
ID1= 1 (8188): 2246.87 125.680 13.75 28.55
+ ID2= 2 (8190): 1908.71 11.577 9.50 24.05
ID = 3 (8188): 2415.7 134.080 13.50 28.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHYD (4462) Area (ha)=1366.10 Curve Number (CN)= 61.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 4.22

Unit Hyd Qpeak (cms)= 5.524

PEAK FLOW (cms)= 4.956 (i)
TIME TO PEAK (hrs)= 12.000
RUNOFF VOLUME (mm)= 24.093
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.298

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (4461) Area (ha)= 108.90
ID= 1 DT=15.0 min Total Imp(%)= 50.00 Dir. Conn.(%)= 30.00

Surface Area (ha)= IMPERVIOUS PERVIOUS (i)
54.45 54.45
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 852.06 40.00
Mannings n = 0.013 0.250
Max. Ref. Inten. (mm/hr)= 106.66 65.78
Storage Coeff. (min)= 11.09 (ii) 23.73 (ii)

Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04
PEAK FLOW (cms)= 7.91 4.65 10.849 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 78.00 30.87 45.25
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.38 0.56

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 61.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5446)

IN= 2-> OUT= 1
DT= 15.0 min
OUTFLOW STORAGE OUTFLOW STORAGE
(cms) (ha.m.) (cms) (ha.m.)
0.0000 0.0000 | 1.6700 2.9494
0.3690 1.4413 | 1.8200 3.3343
0.5700 1.6830 | 1.9700 3.7111
1.0600 2.1339 | 2.3700 3.7211
1.3800 2.4971 | 0.0000 0.0000

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID= 2 (4461): 108.90 10.849 6.00 45.25
OUTFLOW: ID= 1 (5446): 108.90 1.675 7.25 45.24

PEAK FLOW REDUCTION [Qout/Qin](%)= 15.44
TIME SHIFT OF PEAK FLOW (min)= 75.00
MAXIMUM STORAGE USED (ha.m.)= 2.9627

ADD HYD (8192)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
ID1= 1 (4462): 1366.10 4.956 12.00 24.09
+ ID2= 2 (5446): 108.90 1.675 7.25 45.24
ID = 3 (8192): 1475.00 5.931 10.50 25.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8194) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8188): 24157.57 134.080 13.50 28.19
+ ID2= 2 (8192): 1475.00 5.931 10.50 25.65
=====
ID = 3 (8194): 25632.58 139.389 13.50 28.05

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB (0442) |
| NASHYD (0442) | Area (ha)= 117.26 Curve Number (CN)= 43.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.17
-----

```

Unit Hyd Qpeak (cms)= 1.705

```

PEAK FLOW (cms)= 0.655 (1)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 13.817
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.171

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0440) |
| NASHYD (0440) | Area (ha)= 226.35 Curve Number (CN)= 60.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.11
-----

```

Unit Hyd Qpeak (cms)= 3.481

```

PEAK FLOW (cms)= 2.254 (1)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 23.234
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.288

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0438) |
| NASHYD (0438) | Area (ha)= 130.70 Curve Number (CN)= 51.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.60
-----

```

Unit Hyd Qpeak (cms)= 3.738

```

PEAK FLOW (cms)= 1.586 (1)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 17.571
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.217

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0436) |
| NASHYD (0436) | Area (ha)= 187.51 Curve Number (CN)= 55.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.73
-----

```

Unit Hyd Qpeak (cms)= 4.391

```

PEAK FLOW (cms)= 2.215 (1)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 19.928
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.247

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0434) |
| NASHYD (0434) | Area (ha)= 56.64 Curve Number (CN)= 46.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.52
-----

```

Unit Hyd Qpeak (cms)= 1.867

```

PEAK FLOW (cms)= 0.654 (1)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 14.950
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.185

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8206) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0434): 56.64 0.654 6.50 14.95
+ ID2= 2 (0436): 187.51 2.215 7.00 19.93
=====
ID = 3 (8206): 244.15 2.853 6.75 18.77

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6438) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00
-----

```

<----- DATA FOR SECTION (4381.0) ----->

Distance	Elevation	Manning
0.00	256.00	0.0600
9.23	255.50	0.0600
27.69	255.00	0.0600
50.77	254.90	0.0600
64.62	254.85	0.0600
78.47	252.26	0.0600
96.93	249.44	0.0600
129.24	245.94	0.0600
145.40	245.76	0.0600
146.20	245.80	0.0600 / 0.0350
146.70	245.30	0.0350
147.70	245.30	0.0350
148.20	245.80	0.0350 / 0.0600
150.01	245.78	0.0600
152.32	245.79	0.0600
163.86	246.72	0.0600
175.40	249.07	0.0600
186.94	253.15	0.0600
196.17	255.38	0.0600
228.48	255.55	0.0600

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	245.80	.221E+04	0.9	1.02	40.44
1.01	246.31	.352E+05	14.0	0.99	41.77
1.53	246.83	.841E+05	47.3	1.39	29.61
2.04	247.34	.144E+06	102.0	1.76	23.49
2.55	247.85	.213E+06	177.3	2.07	19.99
3.07	248.37	.291E+06	274.4	2.34	17.66
3.58	248.88	.378E+06	394.6	2.58	15.97
4.09	249.39	.474E+06	541.4	2.83	14.60
4.61	249.91	.578E+06	719.9	3.08	13.38
5.12	250.42	.688E+06	923.8	3.33	12.40
5.63	250.93	.803E+06	1153.3	3.56	11.61
6.14	251.44	.925E+06	1408.6	3.77	10.95
6.66	251.96	.105E+07	1690.6	3.98	10.38
7.17	252.47	.119E+07	2002.3	4.18	9.88
7.68	252.98	.133E+07	2345.4	4.38	9.43
8.20	253.50	.147E+07	2710.1	4.56	9.05
8.71	254.01	.162E+07	3101.0	4.73	8.72
9.22	254.52	.178E+07	3521.8	4.90	8.43
9.74	255.04	.195E+07	3967.6	4.99	8.39

```

-----
| INFLW : ID= 2 (8206) 244.15 2.85 6.75 18.77 0.58 1.02
| OUTFLOW: ID= 1 (6438) 244.15 2.50 7.75 18.77 0.56 1.02
-----

```

```

-----
| ADD HYD (8208) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0438): 130.70 1.586 6.75 17.57
+ ID2= 2 (6438): 244.15 2.498 7.75 18.77
=====
ID = 3 (8208): 374.85 3.861 7.25 18.35

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8210) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0440): 226.35 2.254 7.50 23.23
+ ID2= 2 (8208): 374.85 3.861 7.25 18.35

```

ID = 3 (8210): 601.20 6.108 7.50 20.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| CALIB |
| NASHYD (0432) | Area (ha)= 114.21 Curve Number (CN)= 53.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.21

Unit Hyd Qpeak (cms)= 1.617

PEAK FLOW (cms)= 0.865 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 18.938
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.234

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHYD (0430) | Area (ha)= 111.64 Curve Number (CN)= 55.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.52

Unit Hyd Qpeak (cms)= 3.687

PEAK FLOW (cms)= 1.726 (i)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 19.711
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.244

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHYD (0428) | Area (ha)= 50.53 Curve Number (CN)= 51.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.50

Unit Hyd Qpeak (cms)= 1.725

PEAK FLOW (cms)= 0.707 (i)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 17.454
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.216

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHYD (0426) | Area (ha)= 247.17 Curve Number (CN)= 61.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.98

Unit Hyd Qpeak (cms)= 4.315

PEAK FLOW (cms)= 2.796 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 23.867
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.295

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| NASHYD (0424) | Area (ha)= 49.57 Curve Number (CN)= 59.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.37

Unit Hyd Qpeak (cms)= 2.293

PEAK FLOW (cms)= 1.085 (i)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 21.765
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.269

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| ADD HYD (8198) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.

----- (ha) (cms) (hrs) (mm)
ID1= 1 (0424): 49.57 1.085 6.50 21.77
+ ID2= 2 (0426): 247.17 2.796 7.25 23.87
=====

ID = 3 (8198): 296.74 3.662 6.75 23.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ROUTE CHN (6428) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00

<----- DATA FOR SECTION (4281.0) ----->

Distance	Elevation	Manning
0.00	246.54	0.0900
8.80	246.29	0.0900
30.78	246.03	0.0900
41.78	243.97	0.0900
63.77	237.16	0.0900
85.76	232.53	0.0900
96.75	230.16	0.0900
109.94	228.40	0.0900
112.34	228.28	0.0900 / 0.0450
112.84	228.00	0.0450
113.34	227.50	0.0450
114.34	227.50	0.0450
114.84	228.00	0.0450
116.54	228.29	0.0450 / 0.0900
118.74	228.40	0.0900
120.94	228.52	0.0900
149.52	229.51	0.0900
169.31	230.69	0.0900
186.00	234.27	0.0900
217.69	243.56	0.0900

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.78	228.28	.205E+04	1.6	0.97	21.60
1.58	229.08	.202E+05	16.6	1.02	20.37
2.39	229.89	.665E+05	64.8	1.22	17.11
3.19	230.69	.133E+06	258.1	1.49	14.05
4.00	231.50	.213E+06	308.6	1.82	11.48
4.80	232.30	.299E+06	504.9	2.11	9.89
5.61	233.11	.394E+06	746.6	2.37	8.79
6.41	233.91	.498E+06	1034.5	2.61	7.99
7.21	234.71	.605E+06	1373.3	2.84	7.35
8.02	235.52	.722E+06	1763.3	3.06	6.82
8.82	236.32	.844E+06	2202.0	3.27	6.39
9.63	237.13	.974E+06	2690.5	3.46	6.03
10.43	237.93	.111E+07	3239.5	3.66	5.71
11.23	238.73	.125E+07	3839.0	3.84	5.43
12.04	239.54	.140E+07	4489.1	4.03	5.18
12.84	240.34	.155E+07	5190.5	4.20	4.97
13.65	241.15	.170E+07	5943.8	4.36	4.78
14.45	241.95	.187E+07	6749.8	4.53	4.61
15.26	242.76	.204E+07	7609.4	4.68	4.46

<---- hydrograph ----> <-pipe / channel-->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8198) 296.74 3.66 6.75 23.52 0.89 0.97
OUTFLOW: ID= 1 (6428) 296.74 3.52 7.25 23.52 0.88 0.97

| ADD HYD (8200) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0428): 50.53 0.707 6.50 17.45
+ ID2= 2 (6428): 296.74 3.517 7.25 23.52
=====

ID = 3 (8200): 347.27 4.114 7.25 22.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ADD HYD (8202) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0430): 111.64 1.726 6.50 19.71
+ ID2= 2 (8200): 347.27 4.114 7.25 22.63
=====

ID = 3 (8202): 458.91 5.684 7.00 21.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ROUTE CHN (6432) |

IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (4321.0) -----
Distance Elevation Manning
0.00 221.00 0.0600
15.29 219.85 0.0600
45.86 219.86 0.0600
76.44 219.96 0.0600
110.84 220.23 0.0600
112.00 220.15 0.0600
114.66 220.09 0.0600
118.48 219.73 0.0600
120.80 219.70 0.0600 / 0.0350 Main Channel
121.30 219.20 0.0350 Main Channel
122.30 219.20 0.0350 Main Channel
122.80 219.70 0.0350 / 0.0600 Main Channel
129.95 220.11 0.0600
175.81 220.24 0.0600
214.03 220.55 0.0600
252.25 220.61 0.0600
290.47 220.70 0.0600
328.69 221.04 0.0600
347.80 221.08 0.0600
378.37 221.04 0.0600

TRAVEL TIME TABLE
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.08 219.28 .185E+03 0.0 0.23 149.38
0.17 219.37 .399E+03 0.1 0.34 100.62
0.25 219.45 .641E+03 0.1 0.42 80.98
0.33 219.53 .911E+03 0.2 0.49 69.79
0.42 219.62 .121E+04 0.3 0.55 62.35
0.50 219.70 .154E+04 0.5 0.60 56.91
0.61 219.81 .207E+04 0.7 0.66 61.28
0.71 219.91 .301E+04 1.3 0.30 112.96
0.82 220.02 .249E+05 3.4 0.28 122.71
0.92 220.12 .452E+05 6.8 0.31 109.98
1.03 220.23 .747E+05 12.2 0.33 102.05
1.14 220.34 .112E+06 21.3 0.39 87.59
1.24 220.44 .153E+06 33.0 0.44 77.01
1.35 220.55 .196E+06 47.2 0.49 69.22
1.46 220.66 .248E+06 62.3 0.51 66.48
1.56 220.76 .310E+06 83.4 0.55 62.06
1.67 220.87 .376E+06 108.8 0.59 57.56
1.77 220.97 .444E+06 137.7 0.64 53.77
1.88 221.08 .517E+06 163.6 0.65 52.70

hydrograph
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLW : ID= 2 (8202) 458.91 5.68 7.00 21.92 0.89 0.30
OUTFLOW: ID= 1 (6432) 458.91 3.70 9.00 21.92 0.83 0.28

ADD HYD (8204)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0432): 114.21 0.865 8.00 18.94
+ ID2= 2 (6432): 458.91 3.698 9.00 21.92
ID = 3 (8204): 573.12 4.494 8.75 21.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8212)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8204): 573.12 4.494 8.75 21.33
+ ID2= 2 (8210): 601.20 6.108 7.50 20.19
ID = 3 (8212): 1174.32 10.140 8.00 20.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6442)
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (4421.0) -----
Distance Elevation Manning
0.00 221.00 0.0350
26.73 220.80 0.0350
53.47 220.60 0.0350
66.83 220.40 0.0350
80.20 220.20 0.0350
347.52 220.00 0.0350
354.21 219.59 0.0350

360.89 219.50 0.0350
367.57 219.08 0.0350
368.76 219.00 0.0350 / 0.0300 Main Channel
369.26 218.50 0.0300 Main Channel
374.86 218.50 0.0300 Main Channel
374.76 219.00 0.0300 / 0.0350 Main Channel
380.94 220.33 0.0350
387.62 219.62 0.0350
394.31 219.23 0.0350
454.45 219.07 0.0350
514.60 219.09 0.0350
660.00 219.19 0.0350
661.63 221.00 0.0350

TRAVEL TIME TABLE
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.12 218.62 .100E+04 0.2 0.28 92.89
0.25 218.75 .205E+04 0.6 0.43 60.09
0.37 218.87 .315E+04 1.1 0.55 46.99
0.50 219.00 .430E+04 1.8 0.66 39.67
0.63 219.13 .141E+05 3.4 0.37 69.99
0.77 219.27 .652E+05 13.7 0.33 79.14
0.90 219.40 .124E+06 35.5 0.45 58.02
1.03 219.53 .183E+06 65.7 0.56 46.45
1.17 219.67 .245E+06 103.8 0.66 39.30
1.30 219.80 .307E+06 149.3 0.76 34.30
1.43 219.93 .371E+06 203.6 0.85 30.65
1.57 220.07 .440E+06 256.5 0.91 28.56
1.70 220.20 .541E+06 325.7 0.94 27.70
1.83 220.33 .663E+06 415.2 0.98 26.62
1.97 220.47 .787E+06 520.3 1.03 25.21
2.10 220.60 .913E+06 638.5 1.09 23.82
2.23 220.73 .104E+07 767.2 1.15 22.62
2.37 220.87 .117E+07 908.0 1.21 21.54
2.50 221.00 .131E+07 1060.7 1.27 20.57

hydrograph
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLW : ID= 2 (8212) 1174.32 10.14 8.00 20.75 0.72 0.34
OUTFLOW: ID= 1 (6442) 1174.32 8.76 9.50 20.75 0.70 0.35

ADD HYD (8214)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0442): 117.26 0.655 7.75 13.82
+ ID2= 2 (6442): 1174.32 8.763 9.50 20.75
ID = 3 (8214): 1291.58 9.326 9.50 20.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8196)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8194): 25632.58 139.389 13.50 28.05
+ ID2= 2 (8214): 1291.58 9.326 9.50 20.12
ID = 3 (8196): 26924.15 145.033 13.50 27.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0444)
Area (ha)= 221.65 Curve Number (CN)= 56.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.03

Unit Hyd Qpeak (cms)= 3.656
PEAK FLOW (cms)= 2.061 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 20.663
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.256

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8216)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0444): 221.65 2.061 7.50 20.66
+ ID2= 2 (8196): 26924.15 145.033 13.50 27.67

 ID = 3 (8216): 27145.80 145.632 13.50 27.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

EXISTING
 EAST HOLLAND RIVER 100-YEAR STORM

V V I SSSS U U A L
 V V I SS U U AAA L
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OOO TTTT TTTT H H Y Y M M OOO TM
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 O O T T H H Y Y M M O O Company
 OOO T T H H Y Y M M OOO Serial

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***** DETAILED OUTPUT *****

Input filename: C:\Program Files\Visual Gchymo 2.4r\VO2\voind.dat
 Output filename: C:\Users\jscott\AppData\Local\Temp\16ff4504-54bc-421d-b768-1cbd43d1bfaf\Scenario.out
 Summary filename: C:\Users\jscott\AppData\Local\Temp\16ff4504-54bc-421d-b768-1cbd43d1bfaf\Scenario.sum

DATE: 08/22/2012 TIME: 01:29:37

USER:

COMMENTS:

 ** SIMULATION NUMBER: 1 **

 READ STORM | Filename: C:\Users\jscott\AppData\Local\Temp\16ff4504-54bc-421d-b768-1cbd43d1bfaf\5e0022d8
 Ptotal= 88.50 mm | Comments: 100-Year 12-Hour SCS II Design Storm

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	2.21	3.25	3.54	6.25	15.93	9.25	3.10
0.50	2.21	3.50	3.54	6.50	15.93	9.50	3.10
0.75	2.21	3.75	3.54	6.75	7.08	9.75	3.10
1.00	2.21	4.00	3.54	7.00	7.08	10.00	3.10
1.25	2.21	4.25	5.31	7.25	5.31	10.25	1.77
1.50	2.21	4.50	5.31	7.50	5.31	10.50	1.77
1.75	2.21	4.75	7.08	7.75	5.31	10.75	1.77
2.00	2.21	5.00	7.08	8.00	5.31	11.00	1.77
2.25	2.65	5.25	10.62	8.25	3.10	11.25	1.77
2.50	2.65	5.50	10.62	8.50	3.10	11.50	1.77
2.75	2.65	5.75	42.48	8.75	3.10	11.75	1.77
3.00	2.65	6.00	116.82	9.00	3.10	12.00	1.77

 CALIB |
 STANDBYD (1662) | Area (ha)= 158.10
 ID= 1 DT=15.0 min | Total Imp(%)= 58.00 Dir. Conn.(%)= 30.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 91.70 66.40
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1026.65 40.00
 Mannings n = 0.013 0.250
 Max.Eff.Inten.(mm/hr)= 116.82 132.40
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 11.95 (ii) 21.51 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.05

 PEAK FLOW (cms)= 12.25 12.12 20.294 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 86.50 51.53 62.02
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.58 0.70

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 73.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB |
 STANDBYD (1661) | Area (ha)= 276.60
 ID= 1 DT=15.0 min | Total Imp(%)= 40.00 Dir. Conn.(%)= 21.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 110.64 165.96
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1357.94 40.00
 Mannings n = 0.013 0.250
 Max.Eff.Inten.(mm/hr)= 116.82 93.53
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 14.14 (ii) 25.13 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.07 0.04

 PEAK FLOW (cms)= 14.07 19.78 26.893 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 86.50 45.97 54.48
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.52 0.62

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 73.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR (5166) |
 IN= 2--> OUT= 1
 DT= 15.0 min |
 OUTFLOW STORAGE | OUTFLOW STORAGE
 (cms) (ha.m.) | (cms) (ha.m.)
 0.0000 0.0000 | 6.0870 6.1196
 0.3730 3.2753 | 7.7230 7.0042
 1.3370 3.4678 | 9.3080 7.8876
 2.9800 4.4433 | 9.7080 7.8976
 4.4520 5.2716 | 0.0000 0.0000

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (1661) 276.600 26.893 6.00 54.48
 OUTFLOW: ID= 1 (5166) 276.600 9.020 7.00 54.48

PEAK FLOW REDUCTION [Qout/Qin](%)= 33.54
 TIME SHIFT OF PEAK FLOW (min)= 60.00
 MAXIMUM STORAGE USED (ha.m.)= 7.7516

 ADD HYD (8134) |
 1 + 2 = 3 |
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID= 1 (1662): 158.10 20.294 6.00 62.02
 + ID= 2 (5166): 276.60 9.020 7.00 54.48
 ID = 3 (8134): 434.70 22.116 6.25 57.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB |
 STANDBYD (1642) | Area (ha)= 132.50
 ID= 1 DT=15.0 min | Total Imp(%)= 20.00 Dir. Conn.(%)= 15.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 26.50 106.00
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 939.86 40.00
 Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 116.82 65.10
 over (min) 15.00 30.00
 Storage Coeff. (min)= 11.34 (ii) 24.04 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

TOTALS
 PEAK FLOW (cms)= 5.23 8.93 10.964 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.25
 RUNOFF VOLUME (mm)= 86.50 39.72 46.74
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.45 0.53

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING:FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (1641) | Area (ha)= 101.70
 ID= 1 DT=15.0 min | Total Imp(%)= 70.00 Dir. Conn.(%)= 52.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	71.19	30.51
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.80	0.50
Length (m)=	823.41	40.00
Mannings n =	0.013	0.250

Max.Eff.Inten.(mm/hr)= 116.82 122.23
 over (min) 15.00 30.00
 Storage Coeff. (min)= 10.47 (ii) 20.34 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.05

TOTALS
 PEAK FLOW (cms)= 14.29 5.26 17.763 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.25
 RUNOFF VOLUME (mm)= 86.50 49.55 68.76
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.56 0.78

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5164)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	7.6100	4.1638
0.3440	1.7056	8.2090	4.5053
3.1340	1.9730	8.8070	4.8468
4.1110	3.0607	9.2070	4.8568
5.1070	3.7142	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1641)	101.700	17.763	6.00	68.76
OUTFLOW: ID= 1 (5164)	101.700	4.829	6.75	68.75

PEAK FLOW REDUCTION [Qout/Qin](%)= 27.19
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 3.5747

ADD HYD (8132)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (1642):	132.50	10.964	6.25	46.74
+ ID= 2 (5164):	101.70	4.829	6.75	68.75

ID = 3 (8132):	234.20	15.233	6.25	56.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0162) | Area (ha)= 190.14 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.79

Unit Hyd Qpeak (cms)= 4.104

PEAK FLOW (cms)= 3.274 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 30.355
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.343

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0160) | Area (ha)= 318.82 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.45

Unit Hyd Qpeak (cms)= 3.754

PEAK FLOW (cms)= 3.158 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 28.188
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.319

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0152) | Area (ha)= 124.37 Curve Number (CN)= 59.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.89

Unit Hyd Qpeak (cms)= 2.372

PEAK FLOW (cms)= 1.677 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 26.493
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.299

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0148) | Area (ha)= 417.89 Curve Number (CN)= 26.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.18

Unit Hyd Qpeak (cms)= 6.067

PEAK FLOW (cms)= 1.437 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 8.577
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.097

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0150) | Area (ha)= 105.88 Curve Number (CN)= 17.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.58

Unit Hyd Qpeak (cms)= 3.138

PEAK FLOW (cms)= 0.375 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 5.146
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.058

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8116)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (0148):	417.89	1.437	8.00	8.58
+ ID= 2 (0150):	105.88	0.375	6.75	5.15

ID = 3 (8116):	523.77	1.749	7.50	7.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6152)

IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (1521.0)
Distance Elevation Manning
0.00 290.00 0.0500
15.34 286.87 0.0500
46.03 284.84 0.0500
88.22 283.07 0.0500
126.57 282.11 0.0500
153.42 280.34 0.0500
157.26 280.07 0.0500
161.09 279.72 0.0500
167.76 278.94 0.0500 / 0.0300 Main Channel
168.01 278.60 0.0300 Main Channel
168.76 278.60 0.0300 Main Channel
169.51 278.60 0.0300 Main Channel
169.76 278.90 0.0300 / 0.0500 Main Channel
173.76 279.40 0.0500
180.27 279.69 0.0500
184.11 279.93 0.0500
218.62 281.86 0.0500
260.82 283.20 0.0500
360.54 289.25 0.0500
379.72 289.80 0.0500

TRAVEL TIME TABLE
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.30 278.90 .114E+04 0.4 0.80 45.70
0.87 279.47 .932E+04 4.2 0.98 37.22
1.45 280.05 .353E+05 17.5 1.09 33.68
2.02 280.62 .811E+05 47.8 1.26 29.01
2.59 281.19 .155E+06 102.9 1.46 25.06
3.17 281.77 .250E+06 189.3 1.66 22.03
3.74 282.34 .375E+06 298.5 1.75 20.91
4.32 282.92 .547E+06 456.5 1.83 19.98
4.89 283.49 .767E+06 705.7 2.02 18.12
5.46 284.06 .102E+07 1038.0 2.24 16.34
6.04 284.64 .130E+07 1444.1 2.45 14.98
6.61 285.21 .161E+07 1940.1 2.66 13.79
7.18 285.78 .194E+07 2522.6 2.86 12.80
7.76 286.36 .229E+07 3187.4 3.06 11.98
8.33 286.93 .267E+07 3942.3 3.25 11.28
8.91 287.51 .306E+07 4824.9 3.46 10.58
9.48 288.08 .347E+07 5795.7 3.67 9.99
10.05 288.65 .390E+07 6855.4 3.86 9.48
10.63 289.23 .434E+07 8005.0 4.05 9.04

hydrograph
AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8116) 523.77 1.75 7.50 7.88 0.50 0.86
OUTFLOW : ID= 1 (6152) 523.77 1.64 8.50 7.88 0.49 0.85

ADD HYD (8122)
1 + 2 = 3
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0152): 124.27 1.677 7.00 26.49
+ ID2= 2 (6152): 523.77 1.644 8.50 7.88
ID = 3 (8122): 648.14 3.190 8.00 11.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0158)
Area (ha)= 178.59 Curve Number (CN)= 61.0
NASHVD (0154) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.94

Unit Hyd Opeak (cms)= 3.230
PEAK FLOW (cms)= 2.450 (I)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 28.040
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.317

(I) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0156)
Area (ha)= 83.49 Curve Number (CN)= 45.0
NASHVD (0154) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.62

Unit Hyd Opeak (cms)= 2.306
PEAK FLOW (cms)= 0.970 (I)

TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 17.331
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.196

(I) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0154)
Area (ha)= 200.32 Curve Number (CN)= 37.0
NASHVD (0154) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.97

Unit Hyd Opeak (cms)= 3.537
PEAK FLOW (cms)= 1.251 (I)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 13.368
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.151

(I) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8118)
1 + 2 = 3
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0154): 200.32 1.251 7.25 13.37
+ ID2= 2 (0155): 83.49 0.970 6.75 17.33
ID = 3 (8118): 283.81 2.188 7.00 14.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6158)
Routing time step (min)'= 15.00

DATA FOR SECTION (1581.0)
Distance Elevation Manning
0.00 290.00 0.0400
19.20 288.88 0.0400
48.00 288.26 0.0400
119.99 283.68 0.0400
167.99 282.07 0.0400
196.79 281.32 0.0400
210.79 280.90 0.0400
219.79 280.38 0.0400 / 0.0300 Main Channel
220.04 279.65 0.0300 Main Channel
220.79 279.65 0.0300 Main Channel
221.54 279.65 0.0300 Main Channel
221.79 280.39 0.0300 / 0.0400 Main Channel
225.79 280.59 0.0400
235.19 280.88 0.0400
254.39 281.50 0.0400
273.58 282.56 0.0400
302.38 283.42 0.0400
359.98 284.03 0.0400
436.77 289.37 0.0400
475.17 290.37 0.0400

TRAVEL TIME TABLE
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.37 280.01 .973E+03 0.5 0.86 31.78
0.73 280.38 .210E+04 1.5 1.18 23.16
1.30 280.95 .149E+05 8.6 0.95 28.91
1.86 281.51 .579E+05 40.0 1.13 24.12
2.43 282.08 .133E+06 118.5 1.46 18.75
2.99 282.64 .236E+06 254.6 1.77 15.46
3.56 283.21 .369E+06 450.3 2.00 13.67
4.13 283.78 .542E+06 702.7 2.13 12.86
4.69 284.34 .767E+06 1125.2 2.41 11.36
5.26 284.91 .101E+07 1697.1 2.75 9.93
5.82 285.47 .127E+07 2379.1 3.07 8.91
6.39 286.04 .155E+07 3171.4 3.36 8.14
6.95 286.60 .184E+07 4075.5 3.64 7.53
7.52 287.17 .215E+07 5093.1 3.89 7.03
8.09 287.74 .247E+07 6226.5 4.13 6.62
8.65 288.30 .281E+07 7461.2 4.36 6.28
9.22 288.87 .318E+07 8635.1 4.46 6.13
9.78 289.43 .357E+07 10143.6 4.67 5.86
10.35 290.00 .398E+07 11608.8 4.79 5.71

hydrograph
AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8118) 283.81 2.19 7.00 14.53 0.78 1.15
OUTFLOW : ID= 1 (6158) 283.81 2.10 7.50 14.53 0.78 1.16

ADD HYD (8120)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0158):	178.59	2.450	7.25	28.04	
+ ID2= 2 (6158):	283.81	2.102	7.50	14.53	
ID = 3 (8120):	462.40	4.528	7.50	19.75	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8124)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (8120):	462.40	4.528	7.50	19.75	
+ ID2= 2 (8122):	648.14	3.190	8.00	11.45	
ID = 3 (8124):	1110.54	7.675	7.50	14.91	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6160)
IN= 2--> OUT= 1

Routing time step (min)'= 15.00

----- DATA FOR SECTION (1601.0) -----

Distance	Elevation	Manning	
0.00	261.02	0.0400	
29.89	254.30	0.0400	
40.75	252.58	0.0400	
62.49	251.30	0.0400	
114.11	250.94	0.0400	
130.41	248.58	0.0400	
141.28	247.50	0.0400	
154.86	246.87	0.0400	
155.20	246.70	0.0400	
160.20	246.30	0.0400 /0.0300	Main Channel
160.30	245.30	0.0300	Main Channel
165.20	245.30	0.0300	Main Channel
165.30	246.30	0.0300 /0.0400	Main Channel
168.45	246.96	0.0400	
195.62	249.20	0.0400	
203.77	250.82	0.0400	
225.50	256.78	0.0400	
244.52	261.14	0.0400	
255.39	261.95	0.0400	
268.97	261.92	0.0400	

----- TRAVEL TIME TABLE -----

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.50	246.80	.120E+05	2.9	1.18	68.31
1.00	246.30	.242E+05	8.5	1.70	47.47
1.92	247.22	.825E+05	31.7	1.86	43.34
2.84	248.14	.248E+06	102.5	2.00	40.39
3.76	249.06	.508E+06	247.7	2.37	34.03
4.68	249.98	.832E+06	486.8	2.83	28.49
5.60	250.90	.121E+07	814.9	3.27	24.71
6.52	251.82	.180E+07	1157.1	3.11	25.97
7.44	252.74	.253E+07	1840.2	3.52	22.92
8.36	253.66	.331E+07	2727.1	3.98	20.25
9.29	254.59	.414E+07	3782.6	4.43	18.22
10.21	255.51	.499E+07	5010.7	4.86	16.61
11.13	256.43	.589E+07	6397.4	5.26	15.33
12.05	257.35	.681E+07	7932.8	5.64	14.31
12.97	258.27	.777E+07	9621.2	5.99	13.46
13.89	259.19	.877E+07	11469.4	6.33	12.74
14.81	260.11	.980E+07	13478.4	6.66	12.12
15.73	261.03	.109E+08	15651.7	6.97	11.58
16.65	261.95	.120E+08	17492.5	7.07	11.42

----- hydrograph -----

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8124) 1110.54	7.68	7.50	14.91	0.93	1.60
OUTFLOW : ID= 1 (6160) 1110.54	7.05	8.50	14.91	0.87	1.52

ADD HYD (8126)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0160):	318.82	3.158	8.00	28.19	
+ ID2= 2 (6160):	1110.54	7.045	8.50	14.91	
ID = 3 (8126):	1429.36	10.167	8.50	17.87	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8128)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0162):	190.14	3.274	7.00	30.35	
+ ID2= 2 (8126):	1429.36	10.167	8.50	17.87	
ID = 3 (8128):	1619.50	12.890	8.25	19.34	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8130)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (8128):	1619.50	12.890	8.25	19.34	
+ ID2= 2 (8132):	234.20	15.233	6.25	56.30	
ID = 3 (8130):	1853.70	21.360	6.25	24.01	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6166)
IN= 2--> OUT= 1

Routing time step (min)'= 15.00

----- DATA FOR SECTION (1661.0) -----

Distance	Elevation	Manning	
0.00	248.36	0.0500	
41.67	246.98	0.0500	
95.82	244.93	0.0500	
149.98	242.42	0.0500	
191.64	241.88	0.0500	
224.97	239.90	0.0500	
229.13	239.35	0.0500	
233.30	237.77	0.0500	
237.47	236.57	0.0500	
240.63	235.60	0.0500 /0.0300	Main Channel
241.63	234.10	0.0300	Main Channel
247.13	234.10	0.0300	Main Channel
247.53	235.60	0.0300 /0.0500	Main Channel
254.13	236.25	0.0500	
266.63	236.77	0.0500	
283.29	237.84	0.0500	
291.63	238.89	0.0500	
329.12	244.66	0.0500	
370.78	249.55	0.0500	
412.44	253.17	0.0500	

----- TRAVEL TIME TABLE -----

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.75	234.85	.130E+05	6.0	1.36	36.45
1.50	235.60	.276E+05	18.1	1.95	25.41
2.25	236.35	.544E+05	40.0	2.18	22.69
3.00	237.10	.117E+06	80.1	2.04	24.32
3.75	237.85	.213E+06	148.9	2.07	23.89
4.50	238.60	.334E+06	255.1	2.27	21.85
5.25	239.35	.473E+06	395.8	2.49	19.90
6.00	240.10	.631E+06	563.8	2.65	18.66
6.75	240.85	.824E+06	773.2	2.79	17.77
7.50	241.60	.105E+07	1037.9	2.92	16.97
8.26	242.36	.135E+07	1326.7	2.92	16.94
9.01	243.11	.173E+07	1758.5	3.01	16.44
9.76	243.86	.217E+07	2285.9	3.13	15.81
10.51	244.61	.265E+07	2909.5	3.26	15.17
11.26	245.36	.318E+07	3610.1	3.37	14.67
12.01	246.11	.377E+07	4417.4	3.48	14.21
12.76	246.86	.441E+07	5348.0	3.60	13.75
13.51	247.61	.512E+07	6394.0	3.71	13.34
14.26	248.36	.589E+07	7580.3	3.82	12.95

----- hydrograph -----

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8130) 1853.70	21.36	6.25	24.01	1.61	1.98
OUTFLOW : ID= 1 (6166) 1853.70	18.80	7.00	24.01	1.52	1.95

ADD HYD (8136)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (6166):	1853.70	18.797	7.00	24.01	
+ ID2= 2 (8134):	434.70	22.116	6.25	57.22	

ID = 3 (8136): 2288.40 35.381 6.50 30.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| CALIB |
| STANDBY (2021) | Area (ha)= 91.20 | Dir. Conn.(%)= 24.00
| ID= 1 DT=15.0 min | Total Imp(%)= 40.00
-----
| IMPERVIOUS | PERVIOUS (i) |
| Surface Area (ha)= 36.48 | 54.72 |
| Dep. Storage (mm)= 2.00 | 5.00 |
| Average Slope (%)= 0.50 | 0.50 |
| Length (m)= 779.74 | 40.00 |
| Mannings n = 0.013 | 0.250 |
| Max.Eff.Inten.(mm/hr)= 116.82 | 88.16 |
| over (min) = 15.00 | 30.00 |
| Storage Coeff. (min)= 10.13 (ii) | 21.39 (ii) |
| Unit Hyd. Tpeak (min)= 15.00 | 30.00 |
| Unit Hyd. peak (cms)= 0.09 | 0.05 |
| *TOTALS* |
| PEAK FLOW (cms)= 5.97 | 6.62 | 10.284 (iii) |
| TIME TO PEAK (hrs)= 6.00 | 6.25 | 6.00 |
| RUNOFF VOLUME (mm)= 86.50 | 45.04 | 54.99 |
| TOTAL RAINFALL (mm)= 88.50 | 88.50 | 88.50 |
| RUNOFF COEFFICIENT = 0.98 | 0.51 | 0.62 |

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| RESERVOIR (5202) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
| OUTFLOW | STORAGE | OUTFLOW | STORAGE |
| (cms) | (ha.m.) | (cms) | (ha.m.) |
| 0.0000 | 0.0000 | 4.0850 | 2.2999 |
| 0.3100 | 0.9668 | 5.0850 | 2.6924 |
| 1.2590 | 1.1373 | 5.9090 | 3.1012 |
| 2.2140 | 1.5358 | 6.3090 | 3.1112 |
| 3.0620 | 1.8918 | 0.0000 | 0.0000 |
-----
| AREA | QPEAK | TPEAK | R.V. | |
| (ha) | (cms) | (hrs) | (mm) |
| INFLOW : ID= 2 (2021) | 91.200 | 10.284 | 6.00 | 54.99 |
| OUTFLOW : ID= 1 (5202) | 91.200 | 3.970 | 6.75 | 54.98 |
-----
| PEAK FLOW REDUCTION [Qout/Qin](%)= 38.60 |
| TIME SHIFT OF PEAK FLOW (min)= 45.00 |
| MAXIMUM STORAGE USED (ha.m.)= 2.2690 |

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-----
| CALIB |
| STANDBY (2022) | Area (ha)= 377.30 | Dir. Conn.(%)= 34.00
| ID= 1 DT=15.0 min | Total Imp(%)= 57.00
-----
| IMPERVIOUS | PERVIOUS (i) |
| Surface Area (ha)= 215.06 | 162.24 |
| Dep. Storage (mm)= 2.00 | 5.00 |
| Average Slope (%)= 0.50 | 0.50 |
| Length (m)= 1585.98 | 40.00 |
| Mannings n = 0.013 | 0.250 |
| Max.Eff.Inten.(mm/hr)= 116.82 | 112.48 |
| over (min) = 15.00 | 30.00 |
| Storage Coeff. (min)= 15.52 (ii) | 25.72 (ii) |
| Unit Hyd. Tpeak (min)= 15.00 | 30.00 |
| Unit Hyd. peak (cms)= 0.07 | 0.04 |
| *TOTALS* |
| PEAK FLOW (cms)= 29.88 | 23.04 | 44.884 (iii) |
| TIME TO PEAK (hrs)= 6.00 | 6.25 | 6.00 |
| RUNOFF VOLUME (mm)= 86.50 | 47.54 | 60.79 |
| TOTAL RAINFALL (mm)= 88.50 | 88.50 | 88.50 |
| RUNOFF COEFFICIENT = 0.98 | 0.54 | 0.69 |

```

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 71.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| ADD HYD (8110) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.

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-----
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (2022): 377.30 44.884 6.00 60.79 |
| + ID2= 2 (5202): 91.20 3.970 6.75 54.98 |
| ***** |
| ID = 3 (8110): 468.50 46.262 6.00 59.66 |

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| STANDBY (0102) | Area (ha)= 466.00 | Dir. Conn.(%)= 9.00
| ID= 1 DT=15.0 min | Total Imp(%)= 23.00
-----
| IMPERVIOUS | PERVIOUS (i) |
| Surface Area (ha)= 107.18 | 358.82 |
| Dep. Storage (mm)= 2.00 | 5.00 |
| Average Slope (%)= 0.50 | 0.50 |
| Length (m)= 1762.57 | 40.00 |
| Mannings n = 0.013 | 0.250 |
| Max.Eff.Inten.(mm/hr)= 116.82 | 24.85 |
| over (min) = 15.00 | 45.00 |
| Storage Coeff. (min)= 16.53 (ii) | 35.20 (ii) |
| Unit Hyd. Tpeak (min)= 15.00 | 45.00 |
| Unit Hyd. peak (cms)= 0.07 | 0.03 |
| *TOTALS* |
| PEAK FLOW (cms)= 9.50 | 14.17 | 17.272 (iii) |
| TIME TO PEAK (hrs)= 6.00 | 6.50 | 6.50 |
| RUNOFF VOLUME (mm)= 86.50 | 23.06 | 28.77 |
| TOTAL RAINFALL (mm)= 88.50 | 88.50 | 88.50 |
| RUNOFF COEFFICIENT = 0.98 | 0.26 | 0.33 |

```

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 49.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| CALIB |
| NASHVD (0100) | Area (ha)= 693.84 | Curve Number (CN)= 40.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.95 |
-----
| Unit Hyd Qpeak (cms)= 6.087 | R.V. |
| PEAK FLOW (cms)= 2.848 (i) |
| TIME TO PEAK (hrs)= 9.000 |
| RUNOFF VOLUME (mm)= 14.953 |
| TOTAL RAINFALL (mm)= 88.500 |
| RUNOFF COEFFICIENT = 0.169 |

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| ADD HYD (8000) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
-----
| ID1= 1 (0100): 693.84 2.848 9.00 14.95 |
| + ID2= 2 (0102): 466.00 17.272 6.50 28.77 |
| ***** |
| ID = 3 (8000): 1159.84 19.127 6.50 20.50 |

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| ROUTE CHN (6104) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00
-----
| <----- DATA FOR SECTION (1041.0) -----> |
| Distance | Elevation | Manning |
| 0.00 | 330.16 | 0.0450 |
| 50.03 | 328.36 | 0.0450 |
| 89.34 | 326.33 | 0.0450 |
| 117.94 | 324.30 | 0.0450 |
| 160.82 | 322.75 | 0.0450 |
| 178.34 | 319.96 | 0.0450 |
| 183.34 | 319.81 | 0.0450 |
| 184.34 | 319.38 | 0.0450 / 0.0300 Main Channel |
| 185.09 | 318.78 | 0.0300 Main Channel |
| 185.84 | 318.72 | 0.0300 Main Channel |
| 186.84 | 319.32 | 0.0300 / 0.0450 Main Channel |
| 193.84 | 319.70 | 0.0450 |
| 198.84 | 320.38 | 0.0450 |
| 200.13 | 320.81 | 0.0450 |
| 218.00 | 322.49 | 0.0450 |
| 239.44 | 323.49 | 0.0450 |

```

250.17 323.96 0.0450
 275.18 325.20 0.0450
 310.92 326.47 0.0450
 353.81 330.00 0.0450

TRAVEL TIME TABLE					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.30	319.02	.624E+03	0.1	0.45	73.00
0.60	319.32	1.81E+04	0.6	0.67	49.46
1.23	319.95	1.28E+05	4.9	0.77	43.38
1.86	320.58	3.96E+05	19.1	0.96	34.50
2.48	321.20	7.57E+05	43.5	1.15	28.98
3.11	321.83	1.25E+06	81.2	1.30	25.58
3.74	322.46	1.87E+06	135.9	1.45	22.92
4.37	323.09	2.69E+06	195.6	1.45	22.90
5.00	323.72	3.87E+06	287.8	1.48	22.44
5.63	324.35	5.45E+06	428.4	1.56	21.22
6.25	324.97	7.35E+06	630.5	1.71	19.42
6.88	325.60	9.52E+06	876.1	1.83	18.12
7.51	326.23	1.20E+07	1177.0	1.95	17.03
8.14	326.86	1.49E+07	1554.4	2.08	15.93
8.77	327.49	1.79E+07	2009.7	2.23	14.88
9.40	328.12	2.13E+07	2533.9	2.37	13.99
10.02	328.74	2.49E+07	3112.6	2.49	13.31
10.65	329.37	2.88E+07	3760.9	2.60	12.75
11.28	330.00	3.30E+07	4494.8	2.71	12.23

hydrograph						pipe / channel					
AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLW : ID= 2 (8000)	1159.84	14.10	6.75	20.50	1.63	0.88					
OUTFLOW: ID= 1 (6104)	1159.84	14.10	6.75	20.50	1.63	0.88					

CALIB (0104) Area (ha)= 527.81 Curve Number (CN)= 43.0
 NASHVD (0104) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.34
 Unit Hyd Qpeak (cms)= 6.735
 PEAK FLOW (cms)= 3.207 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 16.483
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.186
 (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8002) 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0104): 527.81 3.207 8.00 16.48
 + ID2= 2 (6104): 1159.84 14.095 6.75 20.50
 =====
 ID = 3 (8002): 1687.65 16.853 6.75 19.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0106) Area (ha)= 259.52 Curve Number (CN)= 55.0
 NASHVD (0106) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.23
 Unit Hyd Qpeak (cms)= 3.614
 PEAK FLOW (cms)= 2.445 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 23.753
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.268
 (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8004) 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0106): 259.52 2.445 8.00 23.75
 + ID2= 2 (8002): 1687.65 16.853 6.75 19.25
 =====
 ID = 3 (8004): 1947.17 19.050 6.75 19.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6108) Routing time step (min)= 15.00
 IN= 2--> OUT= 1

DATA FOR SECTION (1081.0)			
Distance	Elevation	Manning	
0.00	326.06	0.0400	
110.62	325.74	0.0400	
30.15	324.28	0.0400	
59.27	320.35	0.0400	
72.37	317.60	0.0400	
87.45	314.52	0.0400	
93.48	313.91	0.0400	
105.54	313.55	0.0400	
106.17	313.40	0.0400 / 0.0300	Main Channel
110.62	313.04	0.0300	Main Channel
110.82	312.97	0.0300	Main Channel
111.57	313.08	0.0300	Main Channel
111.67	313.48	0.0300 / 0.0400	Main Channel
129.66	316.62	0.0400	
150.77	318.95	0.0400	
180.92	319.61	0.0400	
205.04	321.23	0.0400	
232.18	322.09	0.0400	
268.37	322.31	0.0400	
298.52	326.00	0.0400	

TRAVEL TIME TABLE					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.43	318.40	2.44E+04	0.7	0.59	58.83
1.09	314.06	2.14E+05	10.4	1.01	34.24
1.76	314.73	6.00E+05	40.2	1.39	24.89
2.42	315.39	1.09E+06	92.1	1.75	19.80
3.08	316.05	1.69E+06	166.5	2.05	16.88
3.75	316.72	2.37E+06	264.4	2.31	14.97
4.41	317.38	3.18E+06	386.3	2.52	13.72
5.07	318.04	4.11E+06	539.3	2.72	12.71
5.74	318.71	5.17E+06	725.5	2.91	11.88
6.40	319.37	6.42E+06	909.2	2.94	11.77
7.06	320.03	8.06E+06	1179.3	3.04	11.38
7.72	320.69	9.99E+06	1515.9	3.18	10.87
8.39	321.36	1.19E+07	1895.5	3.30	10.49
9.05	322.02	1.43E+07	2310.2	3.36	10.30
9.71	322.68	1.72E+07	2763.2	3.33	10.39
10.38	323.35	2.05E+07	3477.6	3.53	9.81
11.04	324.01	2.38E+07	4284.3	3.73	9.27
11.70	324.67	2.74E+07	5154.0	3.91	8.85
12.37	325.34	3.11E+07	6104.0	4.08	8.48

hydrograph						pipe / channel					
AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLW : ID= 2 (8004)	1947.17	19.05	6.75	19.85	1.29	1.10					
OUTFLOW: ID= 1 (6108)	1947.17	16.97	7.50	19.85	1.24	1.08					

CALIB (0108) Area (ha)= 207.05 Curve Number (CN)= 27.0
 NASHVD (0108) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.03

Unit Hyd Qpeak (cms)= 3.432
 PEAK FLOW (cms)= 0.820 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 8.964
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.101
 (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8006) 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0108): 207.05 0.820 7.50 8.96
 + ID2= 2 (6108): 1947.17 16.975 7.50 19.85
 =====
 ID = 3 (8006): 2154.22 17.795 7.50 18.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0110) Area (ha)= 323.92 Curve Number (CN)= 52.0
 NASHVD (0110) Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.35

Unit Hyd Qpeak (cms)= 4.087

PEAK FLOW (cms)= 2.597 (1)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 21.785
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.246

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8008) |
1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 1 (0110): 323.92 2.597 8.00 21.79
+ ID2= 2 (8006): 2154.22 17.795 7.50 18.80
ID = 3 (8008): 2478.14 20.335 7.50 19.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6112) |
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (1121.0) -----
Distance Elevation Manning
0.00 320.59 0.0400
13.48 317.93 0.0400
40.45 311.52 0.0400
64.04 307.68 0.0400
74.15 307.01 0.0400
91.00 307.22 0.0400
107.86 305.21 0.0400
111.23 304.86 0.0400
113.32 304.60 0.0400 / 0.0300 Main Channel
117.77 304.24 0.0300 Main Channel
117.97 304.17 0.0300 Main Channel
118.72 304.28 0.0300 Main Channel
118.82 304.68 0.0300 / 0.0400 Main Channel
124.71 304.76 0.0400
171.90 306.30 0.0400
205.60 307.24 0.0400
239.31 308.24 0.0400
262.90 310.64 0.0400
289.86 314.34 0.0400
333.68 317.66 0.0400

TRAVEL TIME TABLE -----
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.43 304.60 3.72E+04 0.6 0.54 97.65
1.12 305.29 4.63E+05 12.5 0.85 61.89
1.80 305.97 1.49E+06 53.0 1.13 46.85
2.49 306.66 3.11E+06 134.2 1.37 38.62
3.18 307.35 5.54E+06 261.6 1.50 35.27
3.87 308.04 8.94E+06 490.7 1.74 30.37
4.55 308.72 1.29E+07 833.0 2.05 25.75
5.24 309.41 1.71E+07 1269.8 2.36 22.39
5.93 310.10 2.15E+07 1789.0 2.64 20.01
6.62 310.79 2.61E+07 2392.7 2.90 18.21
7.30 311.47 3.10E+07 3087.8 3.16 16.74
7.99 312.16 3.61E+07 3873.6 3.40 15.52
8.68 312.85 4.13E+07 4741.0 3.64 14.52
9.37 313.54 4.67E+07 5689.3 3.86 13.69
10.05 314.22 5.23E+07 6718.4 4.07 12.97
10.74 314.91 5.81E+07 7770.7 4.24 12.46
11.43 315.60 6.41E+07 8897.5 4.40 12.01
12.12 316.29 7.04E+07 10113.2 4.55 11.61
12.80 316.97 7.70E+07 11420.1 4.70 11.24

hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8008) 2478.14 20.33 7.50 19.19 1.25 0.90
OUTFLOW: ID= 1 (6112) 2478.14 17.28 8.75 19.19 1.20 0.88

CALIB |
NASHYD (0112) | Area (ha)= 414.89 Curve Number (CN)= 30.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.91

Unit Hyd Qpeak (cms)= 3.699
PEAK FLOW (cms)= 1.180 (1)
TIME TO PEAK (hrs)= 9.000
RUNOFF VOLUME (mm)= 10.271
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.116

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8010) |
1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 1 (0112): 414.89 1.180 9.00 10.27
+ ID2= 2 (6112): 2478.14 17.284 8.75 19.19
ID = 3 (8010): 2893.03 18.461 8.75 17.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6114) |
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (1141.0) -----
Distance Elevation Manning
0.00 312.27 0.0500
14.07 309.70 0.0500
28.14 306.51 0.0500
35.17 305.81 0.0500
42.20 304.95 0.0500
52.75 304.24 0.0500
77.37 303.61 0.0500
98.47 301.33 0.0500
131.76 300.40 0.0500 / 0.0300 Main Channel
136.21 300.04 0.0300 Main Channel
136.41 299.97 0.0300 Main Channel
137.16 300.08 0.0300 Main Channel
137.26 300.48 0.0300 / 0.0500 Main Channel
140.68 300.53 0.0500
179.36 302.20 0.0500
214.53 303.82 0.0500
242.66 305.27 0.0500
288.38 307.66 0.0500
316.52 311.76 0.0500
348.17 313.48 0.0500

TRAVEL TIME TABLE -----
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.43 300.40 3.66E+04 0.7 0.59 88.31
1.05 301.02 5.05E+05 12.1 0.75 69.49
1.68 301.65 1.65E+06 52.7 0.99 52.28
2.30 302.27 3.24E+06 130.3 1.26 41.40
2.93 302.90 5.20E+06 247.1 1.48 35.08
3.55 303.52 7.54E+06 406.8 1.68 30.91
4.18 304.15 1.04E+07 586.1 1.76 29.55
4.80 304.77 1.38E+07 856.0 1.93 26.96
5.43 305.40 1.77E+07 1201.1 2.12 24.56
6.05 306.02 2.19E+07 1612.2 2.30 22.63
6.68 306.65 2.64E+07 2090.6 2.47 21.07
7.30 307.27 3.13E+07 2653.0 2.64 19.65
7.93 307.90 3.64E+07 3303.9 2.83 18.36
8.55 308.52 4.17E+07 4053.9 3.03 17.14
9.18 309.15 4.71E+07 4871.9 3.22 16.12
9.80 309.77 5.27E+07 5756.2 3.41 15.25
10.43 310.40 5.84E+07 6699.8 3.58 14.52
11.05 311.02 6.42E+07 7710.5 3.74 13.89
11.68 311.65 7.02E+07 8788.5 3.90 13.32

hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8010) 2893.03 18.46 8.75 17.91 1.15 0.78
OUTFLOW: ID= 1 (6114) 2893.03 16.51 10.00 17.91 1.12 0.77

CALIB |
NASHYD (0114) | Area (ha)= 287.65 Curve Number (CN)= 49.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.12

Unit Hyd Qpeak (cms)= 4.389

PEAK FLOW (cms)= 2.412 (1)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 19.870
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.225

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8012) |

```

| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----| (ha) (cms) (hrs) (mm)
ID1= 1 (0114): 287.65 2.412 7.75 19.87
+ ID2= 2 (6114): 2893.03 16.510 10.00 17.91
+-----+
ID = 3 (8012): 3180.68 18.373 10.00 18.09

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| CALIB |
|-----|
| NASHYD (0116) | Area (ha)= 206.43 Curve Number (CN)= 64.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.80

```

Unit Hyd Qpeak (cms)= 4.394

```

PEAK FLOW (cms)= 3.517 (1)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 30.364
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.343

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

| ADD HYD (8014) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----| (ha) (cms) (hrs) (mm)
ID1= 1 (0116): 206.43 3.517 7.00 30.36
+ ID2= 2 (8012): 3180.68 18.373 10.00 18.09
+-----+
ID = 3 (8014): 3387.11 20.376 9.75 18.84

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ROUTE CHN (6118) |
| IIS 2---> OUF 1 | Routing time step (min)'= 15.00

```

<----- DATA FOR SECTION (1181.0) ----->

Distance	Elevation	Manning	
0.00	307.18	0.0450	
24.89	299.75	0.0450	
37.34	296.39	0.0450	
71.57	292.75	0.0450	
96.46	291.12	0.0450	
108.90	289.43	0.0450	
112.02	288.86	0.0450	
115.13	288.46	0.0450	
117.24	288.32	0.0450 /0.0350	Main Channel
117.49	288.15	0.0350	Main Channel
118.24	288.00	0.0350	Main Channel
118.99	288.20	0.0350	Main Channel
119.24	288.36	0.0350 /0.0450	Main Channel
121.35	288.40	0.0450	
149.35	289.36	0.0450	
171.14	290.47	0.0450	
208.47	293.14	0.0450	
236.48	295.17	0.0450	
280.04	299.48	0.0450	
308.04	307.00	0.0450	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.32	288.32	1.16E+04	0.2	0.62	81.92
1.30	289.30	6.31E+05	22.2	1.07	47.43
2.29	290.29	2.18E+06	117.5	1.64	30.95
3.27	291.27	4.48E+06	314.6	2.15	23.72
4.25	292.25	7.56E+06	625.6	2.53	20.13
5.24	293.24	1.15E+07	1093.2	2.91	17.50
6.22	294.22	1.61E+07	1731.6	3.28	15.50
7.20	295.20	2.14E+07	2547.4	3.63	14.01
8.19	296.19	2.74E+07	3584.9	4.00	12.72
9.17	297.17	3.38E+07	4859.1	4.39	11.60
10.15	298.15	4.07E+07	6341.5	4.76	10.70
11.13	299.13	4.80E+07	8024.6	5.11	9.97
12.12	300.12	5.56E+07	10006.6	5.49	9.27
13.10	301.10	6.35E+07	12244.6	5.89	8.64
14.08	302.08	7.16E+07	14681.7	6.26	8.13
15.07	303.07	7.99E+07	17316.5	6.62	7.69
16.05	304.05	8.84E+07	20148.1	6.96	7.31
17.03	305.03	9.71E+07	23176.2	7.29	6.98
18.02	306.02	1.06E+08	26400.8	7.61	6.69

<----- hydrograph ----> <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8014)	3387.11	20.38	9.75	18.84	1.22	1.01
OUTFLOW: ID= 1 (6118)	3387.11	19.83	10.25	18.84	1.19	0.99

```

| CALIB |
|-----|
| NASHYD (0118) | Area (ha)= 340.53 Curve Number (CN)= 45.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.12

```

Unit Hyd Qpeak (cms)= 5.200

```

PEAK FLOW (cms)= 2.515 (1)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 17.546
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.198

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

| ADD HYD (8016) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----| (ha) (cms) (hrs) (mm)
ID1= 1 (0118): 340.53 2.515 7.75 17.55
+ ID2= 2 (6118): 3387.11 19.833 10.25 18.84
+-----+
ID = 3 (8016): 3727.64 21.700 10.25 18.72

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

| ROUTE CHN (6122) |
| IIS 2---> OUF 1 | Routing time step (min)'= 15.00

```

<----- DATA FOR SECTION (1221.0) ----->

Distance	Elevation	Manning	
0.00	277.00	0.0500	
52.18	272.82	0.0500	
86.97	269.38	0.0500	
134.80	267.35	0.0500	
160.89	263.85	0.0500	
169.58	262.88	0.0500	
178.28	262.45	0.0500	
185.67	262.07	0.0500	
213.07	260.99	0.0500	
226.01	259.80	0.0500 /0.0300	Main Channel
226.11	259.30	0.0300	Main Channel
230.11	259.30	0.0300	Main Channel
230.21	259.80	0.0300 /0.0500	Main Channel
235.01	260.20	0.0500	
239.15	260.57	0.0500	
243.50	260.93	0.0500	
300.03	264.43	0.0500	
339.16	268.34	0.0500	
373.95	272.36	0.0500	
430.48	277.46	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.50	259.80	6.26E+04	2.5	1.23	41.35
1.41	260.71	4.62E+05	21.8	1.44	35.27
2.31	261.61	1.49E+06	79.6	1.63	31.19
3.22	262.52	3.40E+06	200.1	1.80	28.34
4.12	263.42	6.36E+06	446.8	2.15	23.71
5.03	264.33	9.95E+06	819.9	2.52	20.23
5.93	265.23	1.41E+07	1337.0	2.90	17.56
6.84	266.14	1.87E+07	1984.8	3.25	15.67
7.74	267.04	2.37E+07	2766.0	3.57	14.26
8.65	267.95	2.92E+07	3596.9	3.76	13.53
9.55	268.85	3.55E+07	4575.4	3.93	12.95
10.46	269.76	4.27E+07	5814.9	4.16	12.23
11.36	270.66	5.03E+07	7319.0	4.44	11.46
12.27	271.57	5.84E+07	9009.7	4.71	10.81
13.17	272.47	6.70E+07	10886.3	4.96	10.26
14.08	273.38	7.61E+07	12893.2	5.17	9.84
14.98	274.28	8.58E+07	15098.1	5.37	9.48
15.89	275.19	9.61E+07	17527.6	5.57	9.14
16.79	276.09	1.07E+08	20190.3	5.76	8.83

<----- hydrograph ----> <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8016)	3727.64	21.70	10.25	18.72	1.40	1.44
OUTFLOW: ID= 1 (6122)	3727.64	21.18	10.75	18.72	1.37	1.43

```

| CALIB |
|-----|
| STANDHYD (1221) | Area (ha)= 17.70
| ID= 1 DT=15.0 min | Total Imp(%)= 36.00 Dir. Conn.(%)= 19.00

```



```

-----
Surface Area (ha)= 6.37 11.33
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 343.51 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 116.82 17.63
over (min) 15.00 30.00
Storage Coeff. (min)= 6.20 (ii) 27.62 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.10 0.04

*TOTALS*
PEAK FLOW (cms)= 1.03 0.39 1.266 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 86.50 15.63 29.10
TOTAL RAINFALL (mm)= 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.18 0.33

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 35.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
RESERVOIR (5122)
IN= 2--> OUT= 1
DT= 15.0 min
-----
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) (cms) (ha.m.)
0.0000 0.0000 | 1.1840 0.4294
0.0600 0.1855 | 1.4870 0.5014
0.3430 0.2103 | 1.7310 0.5718
0.6390 0.2878 | 2.1310 0.5818
0.8860 0.3509 | 0.0000 0.0000
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (1221) 17.700 1.266 6.00 29.10
OUTFLOW: ID= 1 (5122) 17.700 0.424 6.50 29.05
-----
PEAK FLOW REDUCTION [Qout/Qin](%)= 33.47
TIME SHIFT OF PEAK FLOW (min)= 30.00
MAXIMUM STORAGE USED (ha.m.)= 0.2328

```

```

-----
CALIB NASHVD (1222) Area (ha)= 347.20 Curve Number (CN)= 41.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.52
-----
Unit Hyd Qpeak (cms)= 3.887
PEAK FLOW (cms)= 1.782 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 15.443
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.175

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
ADD HYD (8018)
1 + 2 = 3
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (1222): 347.20 1.782 8.25 15.44
+ ID2= 2 (5122): 17.70 0.424 6.50 29.05
-----
ID = 3 (8018): 364.90 1.940 7.25 16.10

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ADD HYD (8020)
1 + 2 = 3
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (6122): 3727.64 21.181 10.75 18.72
+ ID2= 2 (8018): 364.90 1.940 7.25 16.10
-----
ID = 3 (8020): 4092.54 22.752 10.75 18.49

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
CALIB STANDHYD (1261) Area (ha)= 50.00
ID= 1 DT=15.0 min Total Imp(%)= 36.00 Dir. Com.(%)= 26.00
-----

```

```

-----
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 18.00 32.00
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 577.35 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 116.82 23.11
over (min) 15.00 30.00
Storage Coeff. (min)= 8.46 (ii) 27.68 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.09 0.04

```

```

*TOTALS*
PEAK FLOW (cms)= 3.73 1.45 4.617 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 86.50 21.99 38.77
TOTAL RAINFALL (mm)= 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.25 0.44

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
RESERVOIR (5126)
IN= 2--> OUT= 1
DT= 15.0 min
-----
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) (cms) (ha.m.)
0.0000 0.0000 | 3.3450 1.2129
0.1690 0.5421 | 4.2000 1.4164
0.9690 0.5941 | 4.8890 1.6153
1.8040 0.8130 | 5.2890 1.6253
2.5020 0.9912 | 0.0000 0.0000
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (1261) 50.000 4.617 6.00 38.77
OUTFLOW: ID= 1 (5126) 50.000 1.836 6.50 38.75

```

```

PEAK FLOW REDUCTION [Qout/Qin](%)= 39.77
TIME SHIFT OF PEAK FLOW (min)= 30.00
MAXIMUM STORAGE USED (ha.m.)= 0.8246

```

```

-----
CALIB NASHVD (1262) Area (ha)= 83.50 Curve Number (CN)= 58.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.56
-----

```

```

Unit Hyd Qpeak (cms)= 2.523
PEAK FLOW (cms)= 1.563 (i)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 25.453
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.288

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
ADD HYD (8024)
1 + 2 = 3
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (1262): 83.50 1.563 6.50 25.45
+ ID2= 2 (5126): 50.00 1.836 6.50 38.75
-----
ID = 3 (8024): 133.50 3.398 6.50 30.43

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
CALIB NASHVD (0124) Area (ha)= 121.43 Curve Number (CN)= 43.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.51
-----

```

```

Unit Hyd Qpeak (cms)= 4.049
PEAK FLOW (cms)= 1.530 (i)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 16.138

```

TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.182

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8026)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0124):	121.43	1.530	6.50	16.14
+ ID2= 2 (8024):	133.50	3.398	6.50	30.43
ID = 3 (8026):	254.93	4.928	6.50	23.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6128)
 IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (1281.0) ----->

Distance	Elevation	Manning
0.00	270.58	0.0400
7.71	270.36	0.0400
15.43	269.48	0.0400
23.14	265.88	0.0400
42.43	261.83	0.0400
61.71	258.31	0.0400
75.21	256.62	0.0400
90.64	255.35	0.0400
92.49	255.00	0.0400 / 0.0300
93.49	254.00	0.0300
94.49	253.90	0.0300
94.99	253.80	0.0300
95.49	253.70	0.0300
95.99	253.80	0.0300
98.49	255.00	0.0300 / 0.0400
100.28	255.44	0.0400
134.99	257.45	0.0400
148.49	259.67	0.0400
167.77	263.42	0.0400
190.92	270.00	0.0400

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.65	254.35	439E+04	2.0	1.24	36.10
1.30	255.00	131E+05	9.4	1.92	23.39
2.18	255.88	433E+05	37.0	2.30	19.54
3.06	256.76	131E+06	113.3	2.34	19.20
3.95	257.65	273E+06	273.8	2.70	15.64
4.83	258.53	453E+06	538.5	3.21	14.01
5.71	259.41	658E+06	902.9	3.70	12.14
6.59	260.29	886E+06	1368.4	4.16	10.79
7.48	261.18	114E+07	1936.3	4.59	9.79
8.36	262.06	141E+07	2610.9	4.99	9.01
9.24	262.94	171E+07	3398.7	5.37	8.36
10.12	263.82	202E+07	4309.6	5.75	7.81
11.01	264.71	235E+07	5347.5	6.13	7.33
11.89	265.59	270E+07	6501.0	6.48	6.93
12.77	266.47	307E+07	7808.6	6.86	6.55
13.65	267.35	345E+07	9250.5	7.23	6.21
14.54	268.24	384E+07	10800.3	7.59	5.92
15.42	269.12	424E+07	12482.5	7.94	5.66
16.30	270.00	466E+07	14127.4	8.18	5.49

hydrograph -----> <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8026)	254.93	4.93	6.50	23.62	0.91
OUTFLOW: ID= 1 (6128)	254.93	4.11	7.25	23.62	0.83

CALIB (6128) | Area (ha)= 240.24 Curve Number (CN)= 62.0
 NASHVD | Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.00

Unit Hyd Qpeak (cms)= 4.118

PEAK FLOW (cms)= 3.255 (i)
 TIME TO PEAK (hrs)= 7.250
 RINOFF VOLUME (mm)= 28.852
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.326

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8028)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0128):	240.24	3.255	7.25	28.85
+ ID2= 2 (6128):	254.93	4.108	7.25	23.62
ID = 3 (8028):	495.17	7.363	7.25	26.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8022)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8020):	4092.54	22.752	10.75	18.49
+ ID2= 2 (8028):	495.17	7.363	7.25	26.16
ID = 3 (8022):	4587.71	26.030	10.50	19.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (1201) | Area (ha)= 86.00
 STASHVD | Total Imp(%)= 36.00 Dir. Conn.(%)= 14.00
 ID= 1 DT=15.0 min

Surface Area (ha)	IMPERVIOUS (i)	PERVIOUS (ii)
30.96	55.04	
Dep. Storage (mm)= 2.00	5.00	
Average Slope (ft)= 0.50	0.50	
Length (m)= 757.19	40.00	
Mannings n = 0.013	0.250	
Max. Eff. Inten. (mm/hr)= 116.82	49.93	
over (min)= 15.00	30.00	
Storage Coeff. (min)= 9.96 (ii)	24.08 (ii)	
Unit Hyd. Tpeak (min)= 15.00	30.00	
Unit Hyd. peak (cms)= 0.09	0.04	
PEAK FLOW (cms)= 3.30	3.53	5.496 (iii)
TIME TO PEAK (hrs)= 6.00	6.25	6.00
RUNOFF VOLUME (mm)= 86.50	24.82	33.46
TOTAL RAINFALL (mm)= 88.50	88.50	88.50
RUNOFF COEFFICIENT = 0.98	0.28	0.38

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 48.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5120)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1 DT= 15.0 min	0.0000	0.0000	5.7530	2.0863
	0.2910	0.9015	7.2230	2.4361
	1.6660	1.0219	8.4090	2.7784
	3.1030	1.3984	8.8090	2.7884
	4.3040	1.7049	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1201)	86.000	5.496	6.00
OUTFLOW: ID= 1 (5120)	86.000	2.429	6.75

PEAK FLOW REDUCTION [Qout/Qin](%)= 44.20
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 1.2331

CALIB (1202) | Area (ha)= 161.00 Curve Number (CN)= 58.0
 NASHVD | Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.70

Unit Hyd Qpeak (cms)= 3.932

PEAK FLOW (cms)= 2.554 (i)
 TIME TO PEAK (hrs)= 6.750
 RINOFF VOLUME (mm)= 26.619
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.289

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8030)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1202):	161.00	2.554	6.75	25.62
+ ID2= 2 (5120):	86.00	2.429	6.75	33.45
=====				
ID = 3 (8030):	247.00	4.983	6.75	28.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8032)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8022):	4587.71	26.030	10.50	19.31
+ ID2= 2 (8030):	247.00	4.983	6.75	28.35
=====				
ID = 3 (8032):	4834.71	27.494	10.25	19.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (5130)		Routing time step (min)'= 15.00	
IN= 2-->	OUT= 1		

<----- DATA FOR SECTION (1301.0) ----->

Distance	Elevation	Manning	
0.00	257.18	0.0350	
62.45	254.70	0.0350	
109.29	252.74	0.0350	
150.92	251.55	0.0350	
192.56	248.96	0.0350	
239.39	247.16	0.0350	
291.84	246.00	0.0350	
296.84	245.00	0.0350	
298.84	245.00	0.0350 /0.0300	Main Channel
299.24	244.60	0.0300	Main Channel
301.84	244.60	0.0300	Main Channel
303.44	244.60	0.0300	Main Channel
303.84	245.00	0.0300 /0.0350	Main Channel
306.84	245.00	0.0350	
309.84	246.00	0.0350	
317.46	247.22	0.0350	
348.68	254.83	0.0350	
359.09	256.70	0.0350	
452.76	258.23	0.0350	
515.22	258.33	0.0350	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.40	245.00	.870E+04	1.2	0.64	124.12
1.04	245.64	.468E+05	9.5	0.96	81.81
1.68	246.28	.109E+06	25.6	1.11	70.86
2.32	246.92	.257E+06	63.5	1.17	67.51
2.96	247.56	.498E+06	142.7	1.36	58.18
3.61	248.21	.800E+06	266.7	1.58	50.01
4.25	248.85	.116E+07	437.4	1.78	44.24
4.89	249.49	.157E+07	669.1	2.01	39.20
5.53	250.13	.203E+07	954.9	2.23	35.36
6.17	250.77	.252E+07	1294.4	2.43	32.42
6.81	251.41	.305E+07	1689.6	2.62	30.07
7.45	252.05	.363E+07	2090.7	2.73	28.93
8.09	252.69	.429E+07	2553.1	2.82	27.98
8.73	253.33	.501E+07	3138.4	2.96	26.60
9.37	253.97	.579E+07	3803.8	3.11	25.36
10.02	254.62	.662E+07	4549.3	3.25	24.25
10.66	255.26	.751E+07	5364.9	3.38	23.32
11.30	255.90	.845E+07	6265.7	3.51	22.49
11.94	256.54	.946E+07	7260.1	3.63	21.72

<----- hydrograph -----> <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8032)	4834.71	27.49	10.25	19.78	1.71	1.12
OUTFLOW : ID= 1 (6130)	4834.71	25.85	11.25	19.78	1.68	1.11

CALIB STANDHYD (1301)			
ID= 1 DT=15.0 min	Area (ha)	Imp(%)= 65.00	Dir. Conn.(%)= 47.00

	IMPERVIOUS (mm)	PERVIOUS (i)
Surface Area	148.39	79.91
Dep. Storage	2.00	5.00

Average Slope (%)	=	0.50	0.50
length (m)	=	1233.69	40.00
Mannings n	=	0.013	0.250

Max.Eff.Inten.(mm/hr)	=	116.62	117.77	
over (min)	=	15.00	30.00	
Storage Coeff. (min)	=	13.35 (ii)	23.37 (iii)	
Unit Hyd. Tpeak (min)	=	15.00	30.00	
Unit Hyd. peak (cms)	=	0.08	0.04	
=====				
PEAK FLOW (cms)	=	26.60	12.48	*TOTALS*
TIME TO PEAK (hrs)	=	6.00	6.25	34.817 (iii)
RUNOFF VOLUME (mm)	=	86.50	50.36	6.00
TOTAL RAINFALL (mm)	=	88.50	88.50	67.35
RUNOFF COEFFICIENT	=	0.98	0.57	88.50
				0.76

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5130)					
IN= 2-->	OUT= 1				
DT= 15.0 min		OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
		0.0000	0.0000	12.4560	7.0142
		0.8210	3.9074	14.9550	7.8554
		4.8550	4.4209	22.4790	8.9550
		7.0620	5.2214	22.8790	8.9650
		9.5970	6.0904	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1301)	228.300	34.817	6.00	67.35
OUTFLOW : ID= 1 (5130)	228.300	12.963	6.50	67.34

PEAK FLOW REDUCTION [Qout/Qin](%)= 37.23
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 7.3018

CALIB NASHVD (1302)			
ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)= 5.00	# of Linear Res.(N)= 1.50

U.H. Tp(hrs)= 1.82

Unit Hyd Qpeak (cms) = 1.479

PEAK FLOW (cms) = 1.989 (i)

TIME TO PEAK (hrs) = 8.500

RUNOFF VOLUME (mm) = 42.411

TOTAL RAINFALL (mm) = 88.500

RUNOFF COEFFICIENT = 0.479

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8036)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1302):	157.80	1.989	8.50	42.41
+ ID2= 2 (5130):	228.30	12.963	6.50	67.34
=====				
ID = 3 (8036):	386.10	14.478	6.75	57.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8034)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6130):	4834.71	25.846	11.25	19.78
+ ID2= 2 (8036):	386.10	14.478	6.75	57.15
=====				
ID = 3 (8034):	5220.81	28.659	11.25	22.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (0188)			
ID= 1 DT=15.0 min	Area (ha)	Imp(%)= 41.00	Dir. Conn.(%)= 25.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area	148.39	79.91
Dep. Storage	2.00	5.00

Surface Area (ha)= 126.50 182.03
 Dep. Storage (mm)= 3.00 5.00
 Average Slope (%)= 0.10 0.10
 Length (m)= 1434.18 40.00
 Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 79.65 38.11
 over (min)= 30.00 60.00
 Storage Coeff. (min)= 27.59 (ii) 53.10 (ii)
 Unit Hyd. Tpeak (min)= 30.00 60.00
 Unit Hyd. peak (cms)= 0.04 0.02

PEAK FLOW (cms)= 11.65 8.24 *TOTALS*
 TIME TO PEAK (hrs)= 6.25 6.75 16.261 (iii)
 RUNOFF VOLUME (mm)= 88.50 32.01 45.38
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.97 0.36 0.51

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0186) | Area (ha)= 351.79
 ID= 1 DT=15.0 min | Total Imp(%)= 42.00 Dir. Conn.(%)= 22.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	147.75	204.04
Dep. Storage (mm)	3.00	5.00
Average Slope (%)	0.10	0.10
Length (m)	1531.43	40.00
Mannings n	0.013	0.250
Max.Eff.Inten.(mm/hr)	79.65	35.50
over (min)	30.00	60.00
Storage Coeff. (min)	28.70 (ii)	54.94 (ii)
Unit Hyd. Tpeak (min)	30.00	60.00
Unit Hyd. peak (cms)	0.04	0.02
PEAK FLOW (cms)	11.47	8.47
TIME TO PEAK (hrs)	6.25	6.75
RUNOFF VOLUME (mm)	88.50	45.03
TOTAL RAINFALL (mm)	88.50	88.50
RUNOFF COEFFICIENT	0.97	0.32

TOTALS
 16.182 (iii)

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 53.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8068) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)

ID1= 1 (0186): 351.79 16.182 6.25 41.03
 + ID2= 2 (0186): 308.53 16.261 6.25 45.38

 ID = 3 (8068): 660.32 32.443 6.25 43.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1821) | Area (ha)= 111.80
 ID= 1 DT=15.0 min | Total Imp(%)= 40.00 Dir. Conn.(%)= 17.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	44.72	67.08
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	863.33	40.00
Mannings n	0.013	0.250
Max.Eff.Inten.(mm/hr)	116.82	59.01
over (min)	15.00	30.00
Storage Coeff. (min)	10.77 (ii)	23.98 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.04
PEAK FLOW (cms)	5.09	5.10
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	86.50	38.22
TOTAL RAINFALL (mm)	88.50	88.50
RUNOFF COEFFICIENT	0.98	0.32

TOTALS
 8.281 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING:FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 52.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5182)
 IN= 2--> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	5.7800	2.5656
	0.3790	1.0207	7.3720	3.0829
	1.6240	1.1939	8.7710	3.5956
	3.0930	1.6631	9.1710	3.6056
	4.3530	2.1032	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1821)	111.800	8.281	6.00	38.22
OUTFLOW: ID= 1 (5182)	111.800	3.544	6.75	38.21

PEAK FLOW REDUCTION [Qout/Qin](%)= 42.80
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 1.8363

CALIB
 NASHYD (1822) | Area (ha)= 44.40 Curve Number (CN)= 57.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.79

Unit Hyd Qpeak (cms)= 0.966

PEAK FLOW (cms)= 0.626 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 24.973
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.282

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8062) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)

ID1= 1 (1822): 44.40 0.626 7.00 24.97
 + ID2= 2 (5182): 111.80 3.544 6.75 38.21

 ID = 3 (8062): 156.20 4.164 6.75 34.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1841) | Area (ha)= 144.40
 ID= 1 DT=15.0 min | Total Imp(%)= 48.00 Dir. Conn.(%)= 19.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	69.31	75.09
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	981.16	40.00
Mannings n	0.013	0.250

Max.Eff.Inten.(mm/hr)= 116.82 80.58
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 11.63 (ii) 23.30 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

PEAK FLOW (cms)= 7.16 7.94 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.25 12.195 (iii)
 RUNOFF VOLUME (mm)= 86.50 34.09 44.04
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.39 0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING:FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 56.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
RESERVOIR (5184)
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
OUTFLOW STORAGE OUTFLOW STORAGE
(cms) (ha.m.) (cms) (ha.m.)
0.0000 0.0000 5.8820 2.3298
0.2860 1.0006 7.4070 2.7386
1.6530 1.1680 8.6830 3.1418
3.0860 1.4687 9.0830 3.1518
4.3760 1.8752 0.0000 0.0000

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (1841) 144.400 12.195 6.00 44.04
OUTFLOW : ID= 1 (5184) 144.400 6.055 6.50 44.04

PEAK FLOW REDUCTION [Qout/Qin](%)= 49.65
TIME SHIFT OF PEAK FLOW (min)= 30.00
MAXIMUM STORAGE USED (ha.m.)= 2.4267

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-----
CALIB
STANDHYD (1842)
| ID= 1 DT=15.0 min |
-----
Area (ha)= 118.10
Total Imp(%)= 42.00 Dir. Conn.(%)= 17.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 49.60 68.50
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 887.32 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten.(mm/hr)= 116.82 69.80
over (min)= 15.00 30.00
Storage Coeff. (min)= 10.95 (ii) 23.30 (iii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04

**TOTALS*
PEAK FLOW (cms)= 5.35 6.26 9.300 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 86.50 32.19 41.42
TOTAL RAINFALL (mm)= 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.36 0.47

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***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 56.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
ADD HYD (8060)
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (1842): 118.10 9.300 6.00 41.42
+ ID2= 2 (5184): 144.40 6.055 6.50 44.04
=====
ID = 3 (8060): 262.50 13.289 6.25 42.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
ADD HYD (8064)
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8060): 262.50 13.289 6.25 42.86
+ ID2= 2 (8062): 156.20 4.164 6.75 34.45
=====
ID = 3 (8064): 418.70 16.470 6.25 39.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
CALIB
STANDHYD (1781)
| ID= 1 DT=15.0 min |
-----
Area (ha)= 14.00
Total Imp(%)= 50.00 Dir. Conn.(%)= 30.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 7.00 7.00
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 305.51 40.00
Mannings n = 0.013 0.250

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Max. Eff. Inten.(mm/hr)= 116.82 109.87
over (min)= 15.00 30.00
Storage Coeff. (min)= 5.78 (ii) 16.08 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.10 0.05

**TOTALS*
PEAK FLOW (cms)= 1.30 1.19 2.094 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 86.50 50.72 61.45
TOTAL RAINFALL (mm)= 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.57 0.69

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***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 76.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
RESERVOIR (5178)
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
OUTFLOW STORAGE OUTFLOW STORAGE
(cms) (ha.m.) (cms) (ha.m.)
0.0000 0.0000 0.9360 0.3780
0.0470 0.1841 1.1760 0.4277
0.2710 0.2148 1.3690 0.4763
0.5050 0.2704 1.7690 0.4863
0.7010 0.3197 0.0000 0.0000

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (1781) 14.000 2.094 6.00 61.45
OUTFLOW : ID= 1 (5178) 14.000 0.915 6.50 61.38

PEAK FLOW REDUCTION [Qout/Qin](%)= 43.68
TIME SHIFT OF PEAK FLOW (min)= 30.00
MAXIMUM STORAGE USED (ha.m.)= 0.3750

```

```

-----
CALIB
NASHYD (1782)
| ID= 1 DT=15.0 min |
-----
Area (ha)= 261.40 Curve Number (CN)= 78.0
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U.H. Tp(hrs)= 1.15

```

Unit Hyd Qpeak (cms)= 3.867

```

PEAK FLOW (cms)= 4.998 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 44.571
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.504

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
ADD HYD (8046)
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (1782): 261.40 4.998 7.50 44.57
+ ID2= 2 (5178): 14.00 0.915 6.50 61.38
=====
ID = 3 (8046): 275.40 5.610 7.00 45.43

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
CALIB
NASHYD (0180)
| ID= 1 DT=15.0 min |
-----
Area (ha)= 226.65 Curve Number (CN)= 65.0
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U.H. Tp(hrs)= 1.17

```

Unit Hyd Qpeak (cms)= 3.299

```

PEAK FLOW (cms)= 2.950 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 31.398
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.355

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
ADD HYD (8050)
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)

```

ID1= 1 (0180): 226.65 2.950 7.75 31.40
 + ID2= 2 (8046): 275.40 5.610 7.00 45.43
 =====
 ID = 3 (8050): 502.05 8.487 7.25 39.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANHYD (1741) | Area (ha)= 56.30 | Curve Number (CN)= 18.00
 ID= 1 DT=15.0 min | Total Imp(%)= 35.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	19.70	36.60
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	612.64	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	116.82	69.65
over (min)	15.00	30.00
Storage Coeff. (min)	8.77 (ii)	21.13 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.05
PEAK FLOW (cms)	2.88	3.50
TIME TO PEAK (hrs)	6.00	6.00
RUNOFF VOLUME (mm)	86.50	36.13
TOTAL RAINFALL (mm)	88.50	88.50
RUNOFF COEFFICIENT	0.98	0.41

TOTALS
 5.113 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 64.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5174)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.1370	1.3070
0.0580	0.5427	1.4280	1.5354
0.3290	0.6196	1.6620	1.7593
0.6130	0.8627	2.0620	1.7693
0.8510	1.0621	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1741)	56.300	5.113	6.00	45.20
OUTFLOW: ID= 1 (5174)	56.300	1.236	7.00	45.16

PEAK FLOW REDUCTION [Qout/Qin]= 24.16
 TIME SHIFT OF PEAK FLOW (min)= 60.00
 MAXIMUM STORAGE USED (ha.m.)= 1.3867

CALIB
 NASHYD (1742) | Area (ha)= 82.80 | Curve Number (CN)= 71.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.69

Unit Hyd Qpeak (cms)= 2.055
 PEAK FLOW (cms)= 1.947 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 36.575
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.413

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8044)
 1 + 2 = 3
 ID1= 1 (1742): 82.80 1.947 6.75 36.57
 + ID2= 2 (5174): 56.30 1.236 7.00 45.16
 =====
 ID = 3 (8044): 139.10 3.157 7.00 40.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0172) | Area (ha)= 202.23 | Curve Number (CN)= 78.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.89

Unit Hyd Qpeak (cms)= 3.875
 PEAK FLOW (cms)= 4.760 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 44.397
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.502

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0168) | Area (ha)= 247.13 | Curve Number (CN)= 52.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.75

Unit Hyd Qpeak (cms)= 5.611
 PEAK FLOW (cms)= 3.090 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 21.587
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.244

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0170) | Area (ha)= 166.91 | Curve Number (CN)= 65.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.80

Unit Hyd Qpeak (cms)= 3.553
 PEAK FLOW (cms)= 2.928 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 31.205
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.353

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8040)
 1 + 2 = 3
 ID1= 1 (0168): 247.13 3.090 7.00 21.59
 + ID2= 2 (0170): 166.91 2.928 7.00 31.21
 =====
 ID = 3 (8040): 414.04 6.018 7.00 25.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8042)
 1 + 2 = 3
 ID1= 1 (0172): 202.23 4.760 7.00 44.40
 + ID2= 2 (8040): 414.04 6.018 7.00 25.46
 =====
 ID = 3 (8042): 616.27 10.778 7.00 31.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6174)
 IN= 2--> OUT= 1
 Routing time step (min)= 15.00

<----- DATA FOR SECTION (1741.0) ----->

Distance	Elevation	Manning
0.00	296.00	0.0450
13.36	288.94	0.0450
26.71	288.64	0.0450
51.19	287.96	0.0450
73.45	287.32	0.0450
86.81	286.48	0.0450
102.39	285.08	0.0450
110.73	282.76	0.0450
115.73	281.06	0.0450 / 0.0350
115.74	280.35	0.0350
120.73	280.35	0.0350
120.74	281.10	0.0350 / 0.0450
125.74	282.64	0.0450

135.78	286.87	0.0450
149.13	289.58	0.0450
162.49	292.29	0.0450
175.84	293.95	0.0450
195.88	295.13	0.0450
213.68	296.02	0.0450
220.36	296.09	0.0450

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.71	281.06	1.36E+05	6.0	1.69	37.89
1.50	281.85	358E+05	22.5	2.42	26.43
2.28	282.63	726E+05	53.8	2.85	22.47
3.07	283.42	124E+06	104.1	3.24	19.79
3.86	284.21	189E+06	176.7	3.60	17.81
4.64	284.99	268E+06	274.9	3.94	16.26
5.43	285.78	369E+06	390.8	4.07	15.73
6.21	286.56	501E+06	551.6	4.23	15.15
7.00	287.35	674E+06	750.5	4.28	14.97
7.79	288.14	920E+06	993.3	4.15	15.44
8.57	288.92	126E+07	1361.5	4.14	15.47
9.36	289.71	167E+07	1986.0	4.56	14.04
10.15	290.50	210E+07	2739.7	5.02	12.76
10.93	291.28	254E+07	3615.0	5.47	11.70
11.72	292.07	300E+07	4608.8	5.91	10.83
12.50	292.85	347E+07	5693.3	6.30	10.16
13.29	293.64	397E+07	6892.9	6.67	9.60
14.08	294.43	450E+07	8150.4	6.97	9.19
14.86	295.21	506E+07	9525.0	7.23	8.86

----- hydrograph -----

INFLOW	ID= 2 (8042)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW	ID= 1 (6174)	616.27	9.98	7.75	31.68	0.90	1.82

ADD HYD (8048)

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6174):	616.27	9.979	7.75	31.68
+ ID2= 2 (8044):	139.10	3.157	7.00	40.05
ID= 3 (8048):	755.37	12.829	7.50	33.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8052)

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8048):	755.37	12.829	7.50	33.22
+ ID2= 2 (8050):	502.05	8.487	7.25	39.09
ID= 3 (8052):	1257.42	21.254	7.50	35.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8058)

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8052):	1257.42	21.254	7.50	35.56
+ ID2= 2 (8064):	418.70	16.470	6.25	39.72
ID= 3 (8058):	1676.12	32.961	6.75	36.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (1762)

STANDHYD (1762)	Area (ha)= 162.00
ID= 1 DT=15.0 min	Total Imp(%)= 26.00 Dir. Conn.(%)= 11.00

Surface Area (ha)	IMPERVIOUS (i)	PERVIOUS (i)
42.12	119.88	
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1039.23	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	116.82	56.13
over (min)	15.00	30.00
Storage Coeff. (min)	12.04 (ii)	25.52 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04

PEAK FLOW (cms)	TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
4.59	8.41	86.50	88.50	0.98
6.00	6.25	86.50	88.50	0.98
8.41	6.25	86.50	88.50	0.98
6.00	6.25	86.50	88.50	0.98

*****TOTALS*****

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)= 48.80
ID= 1 DT=15.0 min	Total Imp(%)= 50.00 Dir. Conn.(%)= 21.00

Surface Area (ha)	IMPERVIOUS (i)	PERVIOUS (i)
24.40	24.40	
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	570.38	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	116.82	88.96
over (min)	15.00	30.00
Storage Coeff. (min)	8.40 (ii)	19.61 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.05

*****TOTALS*****

PEAK FLOW (cms)	TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
2.94	3.08	4.929	4.929	0.54
6.00	6.25	86.50	88.50	0.98
8.41	6.25	86.50	88.50	0.98
6.00	6.25	86.50	88.50	0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5176)

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1762):	162.00	10.290	6.25	36.91
+ ID2= 2 (5176):	48.80	2.117	6.50	47.34
ID= 3 (8056):	210.80	11.917	6.25	39.32

INFLOW	ID= 2 (1761)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
OUTFLOW	ID= 1 (5176)	48.800	4.929	6.00	47.36

PEAK FLOW REDUCTION [Qout/Qin](%)= 42.96
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 1.0495

ADD HYD (8056)

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1762):	162.00	10.290	6.25	36.91
+ ID2= 2 (5176):	48.80	2.117	6.50	47.34
ID= 3 (8056):	210.80	11.917	6.25	39.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8066)

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8056):	210.80	11.917	6.25	39.32
+ ID2= 2 (8058):	1676.12	32.961	6.75	36.60
ID= 3 (8066):	1886.92	41.721	6.50	36.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8070)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8056):	1886.92	41.721	6.50	36.91
+ ID2= 2 (8058):	660.32	32.443	6.25	43.07
ID = 3 (8070):	2547.24	74.126	6.25	38.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6190)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

Distance	Elevation	Manning	
0.00	261.34	0.0600	
19.76	261.64	0.0600	
59.28	261.29	0.0600	
79.04	261.09	0.0600	
103.74	255.99	0.0600	
153.13	253.42	0.0600	
192.65	252.78	0.0600	
212.29	249.80	0.0600	
217.29	249.50	0.0600 / 0.0350	Main Channel
217.30	249.05	0.0350	Main Channel
222.29	249.05	0.0350	Main Channel
222.30	249.50	0.0350 / 0.0600	Main Channel
227.30	249.80	0.0600	
232.17	252.31	0.0600	
276.63	253.53	0.0600	
326.03	256.97	0.0600	
355.67	257.30	0.0600	
400.12	260.08	0.0600	
469.28	261.25	0.0600	
489.04	262.53	0.0600	

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.45	249.50	438E+04	1.0	0.43	75.97
1.07	250.12	205E+05	5.4	0.51	62.96
1.70	250.75	451E+05	14.8	0.64	50.78
2.32	251.37	762E+05	29.2	0.74	43.46
2.94	251.99	114E+06	49.1	0.84	38.61
3.57	252.62	161E+06	70.2	0.85	38.18
4.19	253.24	248E+06	93.4	0.73	44.29
4.81	253.86	398E+06	154.9	0.76	42.79
5.44	254.49	576E+06	246.1	0.83	38.99
6.06	255.11	779E+06	362.7	0.90	35.79
6.68	255.73	101E+07	506.5	0.98	33.15
7.30	256.35	126E+07	687.6	1.06	30.52
7.93	256.98	153E+07	902.8	1.15	28.18
8.55	257.60	183E+07	1114.3	1.18	27.39
9.17	258.22	216E+07	1407.8	1.26	25.55
9.80	258.85	250E+07	1735.4	1.35	24.03
10.42	259.47	286E+07	2097.5	1.42	22.73
11.04	260.09	324E+07	2492.3	1.49	21.63
11.67	260.72	364E+07	2837.8	1.51	21.39

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8070)	2547.24	74.13	6.25	38.50	3.67
OUTFLOW : ID= 1 (6190)	2547.24	56.73	7.00	38.50	3.15

CALIB STANDBYD (0190)
ID= 1 DT=15.0 min

Area (ha)	IMPERVIOUS (%)	PERVIOUS (%)
146.87	141.12	
3.00	5.00	
0.10	0.10	
1385.62	40.00	
0.013	0.250	
79.65	67.90	
30.00	60.00	
27.03 (ii)	47.27 (ii)	
30.00	60.00	
0.04	0.02	
11.85	11.99	18.842 (iii)
6.25	6.75	6.25
85.50	46.84	57.28

TOTAL RAINFALL (mm)= 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.97 0.53 0.65

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 71.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8072)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0190):	287.99	18.842	6.25	57.28
+ ID2= 2 (6190):	2547.24	56.733	7.00	38.50
ID = 3 (8072):	2835.23	74.278	6.75	40.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6192)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

Distance	Elevation	Manning	
0.00	264.40	0.0500	
3.64	263.94	0.0500	
14.54	261.05	0.0500	
29.08	256.78	0.0500	
36.36	254.96	0.0500	
50.90	252.20	0.0500	
65.44	249.78	0.0500	
76.35	246.86	0.0500	
77.24	245.90	0.0500	
82.24	245.45	0.0500 / 0.0350	Main Channel
82.25	244.90	0.0350	Main Channel
87.25	244.90	0.0350	Main Channel
87.26	245.45	0.0350 / 0.0500	Main Channel
94.53	246.89	0.0500	
170.87	247.16	0.0500	
218.13	248.69	0.0500	
265.40	249.22	0.0500	
301.75	250.78	0.0500	
338.11	253.47	0.0500	
359.92	264.00	0.0500	

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.55	245.45	345E+04	2.0	0.71	29.20
1.53	246.43	174E+05	14.6	1.05	19.92
2.50	247.40	767E+05	52.8	0.86	24.20
3.48	248.38	224E+06	139.8	1.11	18.71
4.46	249.36	429E+06	441.2	1.29	16.19
5.43	250.33	691E+06	874.3	1.58	13.18
6.41	251.31	987E+06	1468.3	1.86	11.20
7.38	252.28	131E+07	2224.8	2.13	9.79
8.36	253.26	165E+07	3132.4	2.37	8.78
9.34	254.24	201E+07	4256.4	2.65	7.87
10.31	255.21	238E+07	5551.0	2.92	7.15
11.29	256.19	276E+07	6996.8	3.17	6.57
12.27	257.17	314E+07	8585.9	3.42	6.10
13.24	258.14	353E+07	10315.9	3.65	5.71
14.22	259.12	393E+07	12180.7	3.87	5.38
15.19	260.09	434E+07	14178.2	4.09	5.10
16.17	261.07	475E+07	16306.5	4.29	4.85
17.15	262.05	517E+07	18558.6	4.49	4.64
18.12	263.02	559E+07	20939.7	4.68	4.45

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8072)	2835.23	74.28	6.75	40.41	2.65
OUTFLOW : ID= 1 (6192)	2835.23	65.48	7.25	40.41	2.59

CALIB STANDBYD (1921)
ID= 1 DT=15.0 min

Area (ha)	IMPERVIOUS (%)	PERVIOUS (%)
14.74	15.96	
2.00	5.00	
0.50	0.50	
452.40	40.00	
0.013	0.250	
116.82	68.61	

over (min) 15.00 30.00
 Storage Coeff. (min)= 7.31 (ii) 19.75 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.10 0.05

 PEAK FLOW (cms)= 2.18 1.55 3.166 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 86.50 31.05 44.36
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.35 0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 54.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5192)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.0560	0.7051
0.1040	0.3429	2.5820	0.7960
0.5960	0.4035	3.2900	0.8880
1.1090	0.5056	3.6900	0.8980
1.5380	0.5974	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1921)	30.700	3.166	6.00 44.36
OUTFLOW: ID= 1 (5192)	30.700	1.419	6.50 44.33

PEAK FLOW REDUCTION [Qout/qin](%)= 44.83
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 0.5748

CALIB
 STANDHYD (1922)
 ID= 1 DT=15.0 min

Area (ha)	Imp(%)	Dir. Conn.(%)
215.90	27.00	14.00

Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
58.29	157.61	
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1199.72	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	over (min)	Storage Coeff. (min)	Unit Hyd. Tpeak (min)	Unit Hyd. peak (cms)
116.82	15.00	13.12 (ii)	15.00	0.08
			27.60 (ii)	0.04

 ***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 54.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8074)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1922):	215.90	13.025	6.00	34.90
+ ID2= 2 (5192):	30.70	1.419	6.50	44.33
ID= 3 (8074):	246.60	13.423	6.25	36.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8076)
 1 + 2 = 3

ID1	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6192):	2835.23	65.479	7.25	40.41

+ ID2= 2 (8074): 246.60 13.423 6.25 36.07
 ID= 3 (8076): 3081.83 69.676 7.25 40.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0196)
 ID= 1 DT=15.0 min

Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res. (N)
41.47	5.00	70.0	1.50

U.H. Tp(hrs)= 0.24
 Unit Hyd Qpeak (cms)= 2.950
 PEAK FLOW (cms)= 2.030 (i)
 TIME TO PEAK (hrs)= 6.000
 RUNOFF VOLUME (mm)= 33.271
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.376

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0194)
 ID= 1 DT=15.0 min

Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res. (N)
256.09	5.00	69.0	1.50

U.H. Tp(hrs)= 0.68
 Unit Hyd Qpeak (cms)= 6.458
 PEAK FLOW (cms)= 5.753 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 34.641
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.391

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8078)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0194):	256.09	5.753	6.75	34.64
+ ID2= 2 (0196):	41.47	2.030	6.00	33.27
ID= 3 (8078):	297.56	7.357	6.50	34.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6198)
 IN= 2--> OUT= 1
 Routing time step (min)= 15.00

----- DATA FOR SECTION (1981.0) -----

Distance	Elevation	Manning
0.00	267.15	0.0500
22.99	265.94	0.0500
51.73	261.39	0.0500
74.72	258.75	0.0500
94.83	257.79	0.0500
114.95	254.36	0.0500
126.44	254.06	0.0500
132.19	253.68	0.0500
135.06	253.35	0.0500
137.94	252.93	0.0500 / 0.0300
140.81	252.41	0.0300
143.69	251.89	0.0300
146.56	252.51	0.0300
149.43	253.36	0.0300 / 0.0500
158.05	255.89	0.0500
172.42	256.25	0.0500
204.03	259.15	0.0500
229.90	260.36	0.0500
252.89	260.46	0.0500
284.50	260.45	0.0500

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.35	252.24	.111E+04	0.6	0.97	31.07
0.69	252.58	.442E+04	3.8	1.55	19.42
1.04	252.93	.974E+04	11.1	2.06	14.65
1.51	253.40	.203E+05	31.2	2.77	10.87
1.98	253.87	.357E+05	65.0	3.30	9.14
2.45	254.34	.596E+05	112.9	3.43	8.80
2.92	254.81	.935E+05	186.8	3.62	8.34
3.39	255.28	.131E+06	282.1	3.89	7.75
3.86	255.75	.172E+06	398.2	4.18	7.22
4.33	256.22	.221E+06	527.0	4.31	6.99
4.80	256.69	.282E+06	697.3	4.47	6.75

5.27	257.16	.350E+06	898.5	4.64	6.50
5.74	257.63	.425E+06	1131.4	4.82	6.26
6.21	258.10	.508E+06	1367.4	4.88	6.19
6.68	258.57	.603E+06	1635.6	4.91	6.14
7.15	259.04	.709E+06	1977.2	5.04	5.98
7.62	259.51	.826E+06	2365.5	5.17	5.83
8.09	259.98	.954E+06	2790.7	5.29	5.70
8.56	260.45	.110E+07	3179.6	5.25	5.74

<---- hydrograph ---->						<-pipe / channel-->	
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL		
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLW : ID= 2 (8078)	297.56	7.36	6.50	34.45	0.86	1.76	
OUTFLOW: ID= 1 (6198)	297.56	7.13	6.75	34.45	0.85	1.75	

CALIB	STANDHYD (1981)	Area (ha)= 104.70		
ID= 1 DT=15.0 min	Total Imp(%)= 44.00	Dir. Conn.(%)= 20.00		

IMPERVIOUS			PERVIOUS (i)		
Surface Area (ha)=	46.07	58.63			
Dep. Storage (mm)=	2.00	5.00			
Average Slope (%)=	0.50	0.50			
Length (m)=	835.46	40.00			
Mannings n	0.013	0.250			
Max.Eff.Inten.(mm/hr)=	116.82	73.35			
over (min)	15.00	30.00			
Storage Coeff. (min)=	10.86 (ii)	22.67 (ii)			
Unit Hyd. Tpeak (min)=	15.00	30.00			
Unit Hyd. peak (cms)=	0.09	0.04			
			**TOTALS*		
PEAK FLOW (cms)=	5.64	5.71	9.264 (iii)		
TIME TO PEAK (hrs)=	6.00	6.25	6.00		
RUNOFF VOLUME (mm)=	86.50	33.80	44.34		
TOTAL RAINFALL (mm)=	88.50	88.50	88.50		
RUNOFF COEFFICIENT	0.98	0.38	0.50		

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CM* = 58.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5198)			
IN= 2--> OUT= 1			
DT= 15.0 min			
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	7.9640	2.3173
0.2850	1.2270	9.5520	2.6039
1.7410	1.4052	10.9290	2.8853
3.5350	1.7675	11.3290	2.8953
5.6580	2.0183	0.0000	0.0000

AREA	QPEAK	TPEAK	R.V.	
(ha)	(cms)	(hrs)	(mm)	
INFLW : ID= 2 (1981)	104.700	9.264	6.00	44.34
OUTFLOW: ID= 1 (5198)	104.700	4.999	6.50	44.33

PEAK FLOW REDUCTION [Qout/Qin](%)= 53.96
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 1.9461

CALIB	NASHYD (1982)	Area (ha)= 285.90	Curve Number (CN)= 65.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50	
U.H. Tp (hrs)=	0.81		

Unit Hyd Qpeak (cms)=	6.048
PEAK FLOW (cms)=	4.991 (i)
TIME TO PEAK (hrs)=	7.000
RUNOFF VOLUME (mm)=	31.210
TOTAL RAINFALL (mm)=	88.500
RUNOFF COEFFICIENT	0.353

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8080)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)

ID= 1 (1982):	285.90	4.991	7.00	31.21
+ ID2= 2 (5198):	104.70	4.999	6.50	44.33
ID = 3 (8080):	390.60	9.740	6.50	34.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8082)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6198):	297.56	7.128	6.75	34.45
+ ID2= 2 (8080):	390.60	9.740	6.50	34.73
ID = 3 (8082):	688.16	16.662	6.75	34.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8084)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8076):	3081.83	69.676	7.25	40.06
+ ID2= 2 (8082):	688.16	16.662	6.75	34.61
ID = 3 (8084):	3769.99	84.884	7.00	39.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6200)				
IN= 2--> OUT= 1	Routing time step (min)= 15.00			

DATA FOR SECTION (2001.0)----->			
Distance	Elevation	Manning	
0.00	252.47	0.0500	
10.38	252.29	0.0500	
20.76	252.28	0.0500	
41.52	247.66	0.0500	
62.28	247.47	0.0500	
80.45	247.34	0.0500	
90.83	246.85	0.0500	
103.81	245.51	0.0500	
106.00	244.50	0.0500	
108.99	244.05	0.0500 / 0.0300	Main Channel
109.00	243.50	0.0300	Main Channel
114.00	243.50	0.0300	Main Channel
114.01	244.05	0.0300 / 0.0500	Main Channel
118.99	244.50	0.0500	
132.36	245.09	0.0500	
150.52	245.83	0.0500	
199.83	249.24	0.0500	
207.62	250.16	0.0500	
236.16	253.11	0.0500	
256.92	253.83	0.0500	

TRAVEL TIME TABLE----->					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.28	243.77	.365E+04	0.7	0.54	81.42
0.55	244.05	.730E+04	2.2	0.81	54.59
1.05	244.55	.197E+05	7.2	0.97	45.51
1.54	245.04	.463E+05	17.2	0.99	44.76
2.04	245.54	.895E+05	34.8	1.03	42.81
2.53	246.03	.152E+06	62.8	1.09	40.43
3.03	246.53	.232E+06	104.7	1.20	36.91
3.52	247.02	.328E+06	159.7	1.29	34.19
4.02	247.52	.450E+06	223.4	1.32	33.56
4.51	248.01	.627E+06	326.9	1.38	31.99
5.01	248.51	.820E+06	463.0	1.50	29.52
5.50	249.00	.103E+07	626.0	1.62	27.30
6.00	249.50	.124E+07	820.1	1.75	25.24
6.49	249.99	.147E+07	1044.4	1.89	23.42
6.99	250.49	.170E+07	1292.6	2.01	21.95
7.48	250.98	.195E+07	1565.6	2.13	20.71
7.98	251.48	.220E+07	1864.5	2.25	19.65
8.47	251.97	.246E+07	2189.4	2.36	18.73
8.97	252.47	.274E+07	2427.5	2.35	18.80

<---- hydrograph ---->						<-pipe / channel-->	
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL		
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLW : ID= 2 (8084)	3769.99	84.88	7.00	39.07	2.79	1.15	
OUTFLOW: ID= 1 (6200)	3769.99	74.35	7.75	39.07	2.67	1.12	

CALIB	STANDHYD (2001)	Area (ha)= 6.00		
-------	-----------------	-----------------	--	--

|ID= 1 DT=15.0 min | Total Imp(%)= 50.00 Dir. Conn.(%)= 23.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 3.00 3.00
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 200.00 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 116.82 103.29
over (min) 15.00 30.00
Storage Coeff. (min)= 4.48 (ii) 15.04 (iii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.11 0.05
**TOTALS*
PEAK FLOW (cms)= 0.44 0.49 0.761 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 86.50 43.65 53.51
TOTAL RAINFALL (mm)= 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.49 0.60

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 67.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5200)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
0.0000 0.0000 | 0.4010 0.1620
0.0200 0.0789 | 0.5040 0.1833
0.1160 0.0921 | 0.5870 0.2041
0.2170 0.1159 | 0.9870 0.2141
0.3000 0.1370 | 0.0000 0.0000
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (2001) 6.000 0.761 6.00 53.51
OUTFLOW: ID= 1 (5200) 6.000 0.327 6.50 53.35

PEAK FLOW REDUCTION [Qout/Qin](%)= 42.96
TIME SHIFT OF PEAK FLOW (min)= 30.00
MAXIMUM STORAGE USED (ha.m.)= 0.1438

CALIB
STANDHYD (2002)
ID= 1 DT=15.0 min

Area (ha)= 109.30
Total Imp(%)= 22.00 Dir. Conn.(%)= 10.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 21.05 85.25
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 853.62 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 116.82 65.05
over (min) 15.00 30.00
Storage Coeff. (min)= 10.70 (ii) 23.41 (iii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.09 0.04
**TOTALS*
PEAK FLOW (cms)= 2.93 7.26 8.343 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.25
RUNOFF VOLUME (mm)= 86.50 36.78 41.75
TOTAL RAINFALL (mm)= 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.42 0.47

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 67.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8086)
1 + 2 = 3

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (2002): 109.30 8.343 6.25 41.75
+ ID2= 2 (5200): 6.00 0.327 6.50 53.35
ID = 3 (8086): 115.30 8.617 6.25 42.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8088)
1 + 2 = 3

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (6200): 3769.99 74.346 7.75 39.07
+ ID2= 2 (8086): 115.30 8.617 6.25 42.36
ID = 3 (8088): 3885.29 75.823 7.75 39.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8038)
1 + 2 = 3

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8034): 5220.81 28.659 11.25 22.54
+ ID2= 2 (8088): 3885.29 75.823 7.75 39.16
ID = 3 (8038): 9106.10 98.987 7.75 29.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (1365)
IN= 2--> OUT= 1

Routing time step (min)'= 15.00

DATA FOR SECTION (1362.0) ----->

Distance Elevation Manning
0.00 252.00 0.0400
2.35 244.60 0.0400
39.94 243.14 0.0400
58.74 242.65 0.0400
91.63 242.03 0.0400
101.03 241.69 0.0400
103.38 241.66 0.0400
105.73 240.76 0.0400
105.93 240.59 0.0400
109.18 240.28 0.0400 / 0.0300 Main Channel
110.43 239.81 0.0300 Main Channel
111.68 239.81 0.0300 Main Channel
113.43 240.09 0.0300 / 0.0400 Main Channel
118.43 240.50 0.0400
138.62 241.68 0.0400
152.72 241.60 0.0400
166.82 242.68 0.0400
185.62 249.09 0.0400
197.36 251.00 0.0400
232.61 252.28 0.0400

TRAVEL TIME TABLE ----->

DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.28 240.09 .118E+04 0.1 0.20 142.09
0.91 240.72 .114E+05 2.4 0.35 79.92
1.53 241.34 .352E+05 9.4 0.45 62.17
2.16 241.97 .831E+05 22.8 0.46 60.69
2.79 242.60 .172E+06 54.2 0.53 52.74
3.41 243.22 .297E+06 111.4 0.63 44.49
4.04 243.85 .445E+06 194.1 0.74 38.22
4.67 244.48 .612E+06 301.1 0.83 33.88
5.29 245.10 .793E+06 443.0 0.94 29.82
5.92 245.73 .976E+06 611.3 1.06 26.60
6.55 246.36 .116E+07 802.0 1.17 24.12
7.18 246.99 .135E+07 1014.0 1.27 22.16
7.80 247.61 .154E+07 1245.2 1.37 20.56
8.43 248.24 .173E+07 1498.0 1.46 19.24
9.06 248.87 .192E+07 1768.8 1.55 18.12
9.68 249.49 .212E+07 2049.6 1.63 17.24
10.31 250.12 .232E+07 2344.7 1.71 16.49
10.94 250.75 .252E+07 2658.5 1.78 15.83
11.56 251.37 .274E+07 2931.2 1.81 15.56

hydrograph ----> <-pipe / channel->

AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8038) 9106.10 98.99 7.75 29.63 3.28 0.61
OUTFLOW: ID= 1 (1365) 9106.10 86.73 8.50 29.63 3.15 0.59

CALIB (1322)
NASHYD (1322)
ID= 1 DT=15.0 min

Area (ha)= 249.10 Curve Number (CN)= 73.0
Ia (ha) = 5.00 # of Linear Res. (N)= 1.50
U.H. Tp(hrs)= 0.83

Unit Hyd Qpeak (cms)= 5.130

PEAK FLOW (cms)= 5.368 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 38.763
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.438

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB | STANHYD (1321) | Area (ha)= 41.70
 | ID= 1 DT=15.0 min | Total Imp(%)= 55.00 Dir. Conn.(%)= 35.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 22.94 18.77
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 527.26 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.82 100.32
 over (min) 15.00 30.00
 Storage Coeff. (min)= 8.01 (ii) 18.70 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.10 0.05

TOTALS

PEAK FLOW (cms)= 4.24 2.74 6.039 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 86.50 45.08 59.58
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.51 0.67

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5132) |
 | IN= 2--> OUT= 1 |
 | DT= 15.0 min |

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.9630	1.4050
0.1780	1.0280	1.4870	1.5525
0.4300	1.0400	1.9900	1.7000
0.8150	1.0500	2.3900	1.7100
0.8990	1.2275	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
41.700	6.039	6.00	59.58
41.700	1.173	7.00	59.54

PEAK FLOW REDUCTION [Qout/Qin](%)= 19.42
 TIME SHIFT OF PEAK FLOW (min)= 60.00
 MAXIMUM STORAGE USED (ha.m.)= 1.4712

ADD HYD (8090) |
 | 1 + 2 = 3 |

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
249.10	5.368	7.00	38.76
41.70	1.173	7.00	59.54
290.80	6.540	7.00	41.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB | NASHYD (5134) | Area (ha)= 155.95 Curve Number (CN)= 64.0
 | ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 | U.H. Tp (hrs)= 0.64

Unit Hyd Qpeak (cms)= 4.167

PEAK FLOW (cms)= 3.164 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 30.190
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.341

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8092) |
 | 1 + 2 = 3 |

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
155.95	3.164	6.75	30.19
290.80	6.540	7.00	41.74
446.75	9.638	7.00	37.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (1366) |
 | IN= 2--> OUT= 1 | Routing time step (min)= 15.00

DATA FOR SECTION (1361.0) ----->

Distance	Elevation	Manning
0.00	262.81	0.0400
37.23	260.54	0.0400
79.12	256.98	0.0400
111.70	254.74	0.0400
172.20	252.76	0.0400
251.32	248.82	0.0400
255.97	248.75	0.0400
260.43	248.76	0.0400
265.43	248.49	0.0400
268.68	248.18	0.0400 / 0.0300
269.93	247.71	0.0300
271.18	247.71	0.0300
272.93	247.99	0.0300
277.93	248.40	0.0300 / 0.0400
283.90	248.88	0.0400
307.17	249.60	0.0400
321.13	251.59	0.0400
363.01	257.08	0.0400
400.25	257.58	0.0400
460.75	261.91	0.0400

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.47	248.18	881E+04	1.3	0.76	114.66
1.19	248.90	.742E+05	17.3	1.22	71.44
1.92	249.63	.278E+06	78.3	1.48	59.26
2.64	250.35	.589E+06	210.3	1.87	46.69
3.36	251.07	.974E+06	414.3	2.23	39.19
4.08	251.79	.143E+07	696.2	2.55	34.32
4.81	252.52	.197E+07	1063.0	2.83	30.86
5.53	253.24	.259E+07	1505.7	3.06	28.62
6.25	253.96	.331E+07	2051.3	3.26	26.88
6.97	254.68	.413E+07	2720.9	3.46	25.30
7.70	255.41	.504E+07	3584.5	3.73	23.43
8.42	256.13	.601E+07	4580.9	4.00	21.87
9.14	256.85	.704E+07	5707.9	4.25	20.57
9.86	257.57	.818E+07	6636.1	4.26	20.53
10.59	258.30	.945E+07	8115.0	4.50	19.42
11.31	259.02	.108E+08	9759.2	4.74	18.45
12.03	259.74	.122E+08	11569.5	4.96	17.61
12.75	260.46	.137E+08	13550.6	5.18	16.87
13.48	261.19	.153E+08	15636.4	5.37	16.29

hydrograph -----> <-- pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
446.75	9.64	7.00	37.71	0.85	0.95
446.75	6.81	8.50	37.71	0.72	0.88

CALIB | STANHYD (1362) | Area (ha)= 410.10
 | ID= 1 DT=15.0 min | Total Imp(%)= 21.00 Dir. Conn.(%)= 11.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 86.12 323.98
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1651.48 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.82 59.19
 over (min) 15.00 30.00
 Storage Coeff. (min)= 15.91 (ii) 29.10 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.07 0.04

TOTALS

PEAK FLOW (cms)= 10.39 22.40 27.662 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.25
 RUNOFF VOLUME (mm)= 86.50 31.29 40.12
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.39 0.45

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%

YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB		STANDHYD (1361)		Area (ha)= 102.30	Total Imp(%)= 55.00	Dir. Conn.(%)= 27.00

		IMPERVIOUS		PERVIOUS (i)		
Surface Area	(ha)	56.27	46.03			
Dep. Storage	(mm)	2.00	5.00			
Average Slope	(%)	0.50	0.50			
Length	(m)	825.83	40.00			
Mannings n	=	0.013	0.250			
Max.Eff.Inten.(mm/hr)	=	116.82	106.96			
over	(min)	15.00	30.00			
Storage Coeff.	(min)	10.49 (ii)	20.90 (ii)			
Unit Hyd. Tpeak	(min)	15.00	30.00			
Unit Hyd. peak	(cms)	0.09	0.05			

		TOTALS		11.897 (iii)		
PEAK FLOW	(cms)	7.46	6.83			
TIME TO PEAK	(hrs)	6.00	6.25			
RUNOFF VOLUME	(mm)	86.50	42.99			
TOTAL RAINFALL	(mm)	88.50	88.50			
RUNOFF COEFFICIENT	=	0.98	0.49			

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5136)		OUTFLOW		STORAGE		OUTFLOW		STORAGE	
IN= 2--> OUT= 1		(cms)		(ha.m.)		(cms)		(ha.m.)	
DT= 15.0 min									
		0.0000	0.0000	2.4000	3.2690				
		0.1600	1.6363	2.6450	3.8640				
		0.9600	1.7470	2.8900	4.4590				
		1.8400	1.9320	3.2900	4.4690				
		2.1200	2.6005	0.0000	0.0000				
		AREA		QPEAK		TPEAK		R.V.	
		(ha)		(cms)		(hrs)		(mm)	
INFLOW: ID= 2 (1361)		102.300	11.897	6.00	54.74				
OUTFLOW: ID= 1 (5136)		102.300	2.388	7.00	54.71				
		PEAK FLOW REDUCTION [Qout/Qin](%)		= 20.07					
		TIME SHIFT OF PEAK FLOW		(min)= 60.00					
		MAXIMUM STORAGE USED		(ha.m.)= 3.2500					

ADD HYD (8096)		AREA		QPEAK		TPEAK		R.V.	
1 + 2 = 3		(ha)		(cms)		(hrs)		(mm)	
		ID1= 1 (1362):	410.10	27.662	6.25	40.12			
		+ ID2= 2 (5136):	102.30	2.388	7.00	54.71			
		=====							
		ID = 3 (8096):	512.40	29.704	6.25	43.04			

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8094)		AREA		QPEAK		TPEAK		R.V.	
1 + 2 = 3		(ha)		(cms)		(hrs)		(mm)	
		ID1= 1 (1366):	446.75	6.806	8.50	37.71			
		+ ID2= 2 (8096):	512.40	29.704	6.25	43.04			
		=====							
		ID = 3 (8094):	959.15	31.394	6.25	40.56			

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8098)		AREA		QPEAK		TPEAK		R.V.	
1 + 2 = 3		(ha)		(cms)		(hrs)		(mm)	
		ID1= 1 (1365):	9106.10	86.736	8.50	29.63			

+ ID2= 2 (8094): 959.15 31.394 6.25 40.56
 =====
 ID = 3 (8098): 10065.25 99.549 8.25 30.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB		STANDHYD (0142)		Area (ha)= 396.54	Curve Number (CN)= 63.0
ID= 1 DT=15.0 min		Ia (mm)= 5.00		# of Linear Res.(N)= 1.50	
		U.H. Tp(hrs)= 1.32			

Unit Hyd Qpeak		(cms)= 5.133			
PEAK FLOW		(cms)= 4.473 (i)			
TIME TO PEAK		(hrs)= 8.000			
RUNOFF VOLUME		(mm)= 29.763			
TOTAL RAINFALL		(mm)= 88.500			
RUNOFF COEFFICIENT		= 0.336			

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB		STANDHYD (0138)		Area (ha)= 414.79	Curve Number (CN)= 24.0
ID= 1 DT=15.0 min		Ia (mm)= 5.00		# of Linear Res.(N)= 1.50	
		U.H. Tp(hrs)= 1.39			

Unit Hyd Qpeak		(cms)= 5.087			
PEAK FLOW		(cms)= 1.144 (i)			
TIME TO PEAK		(hrs)= 8.000			
RUNOFF VOLUME		(mm)= 7.804			
TOTAL RAINFALL		(mm)= 88.500			
RUNOFF COEFFICIENT		= 0.088			

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB		STANDHYD (0140)		Area (ha)= 158.58	Dir. Conn.(%)= 21.00
ID= 1 DT=15.0 min		Total Imp(%)= 24.00			

		IMPERVIOUS		PERVIOUS (i)	
Surface Area	(ha)	38.06	120.52		
Dep. Storage	(mm)	2.00	5.00		
Average Slope	(%)	0.50	0.50		
Length	(m)	1028.20	40.00		
Mannings n	=	0.013	0.250		
Max.Eff.Inten.(mm/hr)	=	116.82	14.92		
over	(min)	15.00	45.00		
Storage Coeff.	(min)	11.96 (ii)	34.86 (ii)		
Unit Hyd. Tpeak	(min)	15.00	45.00		
Unit Hyd. peak	(cms)	0.08	0.03		

		TOTALS		9.772 (iii)	
PEAK FLOW	(cms)	8.60	2.89		
TIME TO PEAK	(hrs)	6.00	6.50		
RUNOFF VOLUME	(mm)	86.50	16.09		
TOTAL RAINFALL	(mm)	88.50	88.50		
RUNOFF COEFFICIENT	=	0.98	0.18		

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 41.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8102)		AREA		QPEAK		TPEAK		R.V.	
1 + 2 = 3		(ha)		(cms)		(hrs)		(mm)	
		ID1= 1 (0138):	414.79	1.144	8.00	7.80			
		+ ID2= 2 (0140):	158.58	9.772	6.00	30.88			
		=====							
		ID = 3 (8102):	573.37	10.297	6.00	14.19			

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6142)		Routing time step (min)'= 15.00		
IN= 2--> OUT= 1				

<----- DATA FOR SECTION (1421.0) ----->				
Distance	Elevation	Manning		
0.00	295.50	0.0400		
7.25	295.36	0.0400		

14.50	295.22	0.0400			
21.75	295.22	0.0400			
29.00	294.23	0.0400			
52.56	286.45	0.0400			
67.06	281.44	0.0400			
77.94	278.28	0.0400			
82.81	277.20	0.0400 / 0.0300	Main Channel		
83.21	276.80	0.0300	Main Channel		
88.81	276.80	0.0300	Main Channel		
96.41	276.80	0.0300	Main Channel		
96.81	277.20	0.0300 / 0.0400	Main Channel		
103.31	278.96	0.0400			
108.75	280.16	0.0400			
117.81	282.73	0.0400			
135.94	290.30	0.0400			
159.50	292.14	0.0400			
174.00	293.07	0.0400			
179.44	293.37	0.0400			

| IN= 2--> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (1461.0) -----

Distance	Elevation	Manning
0.00	252.00	0.0350
39.58	257.13	0.0350
123.69	257.51	0.0350
183.07	256.83	0.0350
202.86	255.73	0.0350
212.75	254.73	0.0350
222.65	252.86	0.0350
232.44	250.70	0.0350 / 0.0300
236.44	250.20	0.0300
236.84	249.80	0.0300
242.44	249.80	0.0300
250.04	249.80	0.0300
250.44	250.20	0.0300
252.33	250.57	0.0300 / 0.0350
286.97	253.52	0.0350
331.50	255.80	0.0350
380.98	255.55	0.0350
445.30	258.40	0.0350
455.19	259.82	0.0350
489.83	262.22	0.0350

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.40	277.20	.238E+05	6.9	1.27	57.28
1.25	278.05	.889E+05	50.9	2.50	29.13
2.10	278.90	.179E+06	133.4	3.26	22.36
2.95	279.75	.293E+06	256.7	3.83	19.03
3.80	280.60	.432E+06	426.0	4.31	16.90
4.66	281.46	.593E+06	644.3	4.75	15.35
5.51	282.31	.776E+06	915.3	5.16	14.12
6.36	283.16	.978E+06	1242.2	5.55	13.12
7.21	284.01	1.20E+07	1626.2	5.94	12.27
8.06	284.86	1.43E+07	2065.6	6.30	11.57
8.91	285.71	1.69E+07	2562.4	6.64	10.97
9.76	286.56	1.96E+07	3118.0	6.97	10.46
10.61	287.41	2.24E+07	3733.3	7.28	10.02
11.46	288.26	2.55E+07	4411.9	7.57	9.62
12.31	289.11	2.87E+07	5155.6	7.86	9.27
13.17	289.97	3.21E+07	5966.6	8.14	8.96
14.02	290.82	3.57E+07	6705.0	8.22	8.87
14.87	291.67	3.98E+07	7494.3	8.24	8.85
15.72	292.52	4.44E+07	8405.3	8.28	8.80

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.39	250.19	.169E+05	5.4	1.04	52.00
0.77	250.57	.373E+05	16.6	1.44	37.46
1.44	251.24	.911E+05	60.3	2.14	25.19
2.11	251.91	1.69E+06	136.3	2.61	20.64
2.79	252.59	2.70E+06	249.7	2.99	18.05
3.46	253.26	3.96E+06	405.4	3.31	16.28
4.13	253.93	5.49E+06	603.3	3.56	15.15
4.80	254.60	7.37E+06	857.8	3.77	14.31
5.48	255.28	9.63E+06	1174.6	3.95	13.67
6.15	255.95	1.28E+07	1449.5	3.66	14.74
6.82	256.62	1.73E+07	2046.4	3.83	14.07
7.49	257.29	2.27E+07	2665.2	3.81	14.17
8.17	257.97	3.10E+07	3705.2	3.87	13.93
8.84	258.64	4.00E+07	5127.4	4.15	12.99
9.51	259.31	4.92E+07	6858.6	4.51	11.95
10.18	259.98	5.87E+07	8815.5	4.86	11.09
10.86	260.66	6.84E+07	10954.6	5.18	10.41
11.53	261.33	7.85E+07	13224.3	5.49	9.82
12.20	262.00	8.90E+07	15923.2	5.79	9.31

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8102)	573.37	10.30	6.00	14.19	0.47
OUTFLOW : ID= 1 (6142)	573.37	4.48	6.75	14.19	0.26

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8106)	1105.60	11.90	7.00	22.90	0.61
OUTFLOW : ID= 1 (6146)	1105.60	10.59	8.00	22.90	0.56

----- ADD HYD (8104) -----

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0142)	396.54	4.473	8.00	29.76
+ ID2= 2 (6142)	573.37	4.484	6.75	14.19
ID= 3 (8104)	969.91	8.555	7.00	20.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- CALIB (0144) -----

Area (ha)	Curve Number (CN)
Area (ha)= 135.69	Curve Number (CN)= 74.0
IA (mm)= 5.00	# of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.71	

Unit Hyd Qpeak (cms)= 3.240

PEAK FLOW (cms)	TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
3.378 (i)	6.750	39.685	88.500	0.448

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- ADD HYD (8106) -----

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0144)	135.69	3.378	6.75	39.69
+ ID2= 2 (8104)	969.91	8.555	7.00	20.55
ID= 3 (8106)	1105.60	11.902	7.00	22.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6146)

----- CALIB (1462) -----

Area (ha)	Total Imp(%)	Dir. Conn.(%)
Area (ha)= 125.00	Total Imp(%)= 29.00	Dir. Conn.(%)= 12.00

----- IMPERVIOUS PERVIOUS (i) -----

Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
36.25	88.75	
Dep. Storage (mm)= 2.00	5.00	
Average Slope (ft)= 0.50	0.50	
Length (m)= 912.87	40.00	
Mannings n = 0.013	0.250	
Max. Eff. Inten. (mm/hr)= 116.82	85.26	
over (min)= 15.00	30.00	
Storage Coeff. (min)= 11.14 (ii)	22.54 (ii)	
Unit Hyd. Tpeak (min)= 15.00	30.00	
Unit Hyd. peak (cms)= 0.08	0.04	

TOTALS

PEAK FLOW (cms)	TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
3.97	10.14	11.663 (iii)	88.50	0.56
6.00	6.25	49.56	88.50	0.56
86.50	44.52	49.56	88.50	0.56
88.50	88.50	88.50	88.50	0.56
0.98	0.50	0.56	88.50	0.56

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CN* = 73.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB (1461) -----

Area (ha)	Total Imp(%)	Dir. Conn.(%)
Area (ha)= 70.50	Total Imp(%)= 36.00	Dir. Conn.(%)= 15.00

```

-----
Surface Area (ha)= 25.38 45.12
Dep. Storage (mm)= 2.00 5.00
Average Slope (ft)= 0.50 0.50
Length (m)= 685.57 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.82 94.76
over (min) 15.00 30.00
Storage Coeff. (min)= 9.38 (ii) 20.31 (iii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.09 0.05

*TOTALS*
PEAK FLOW (cms)= 2.95 6.01 6.982 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.25
RUNOFF VOLUME (mm)= 86.50 46.18 52.23
TOTAL RAINFALL (mm)= 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.52 0.59

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.0 Ia = Dep. Storage (ABOVE)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
RESERVOIR (5146)
IN= 2--> OUT= 1
DT= 15.0 min
-----
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
-----
0.0000 0.0000 | 8.0640 2.2344
0.2390 0.7085 | 9.9830 2.6409
2.2880 0.8391 | 11.7350 3.0631
4.3270 1.4661 | 12.1350 3.0731
6.1170 1.8197 | 0.0000 0.0000
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (1461) 70.500 6.982 6.25 52.23
OUTFLOW : ID= 1 (5146) 70.500 3.993 6.50 52.22

```

```

PEAK FLOW REDUCTION [Qout/Qin] (%) = 57.19
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha.m.) = 1.3832

```

```

-----
ADD HYD (8099)
1 + 2 = 3
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (1462): 125.00 11.663 6.25 49.56
+ ID2= 2 (5146): 70.50 3.993 6.50 52.22
=====
ID = 3 (8099): 195.50 15.069 6.25 50.52

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ADD HYD (8108)
1 + 2 = 3
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (6146): 1105.60 10.591 8.00 22.90
+ ID2= 2 (8099): 195.50 15.069 6.25 50.52
=====
ID = 3 (8108): 1301.10 18.842 6.25 27.05

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ADD HYD (8100)
1 + 2 = 3
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8098): 1006.25 99.549 8.25 30.67
+ ID2= 2 (8108): 1301.10 18.842 6.25 27.05
=====
ID = 3 (8100): 11366.35 112.219 8.25 30.26

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ROUTE CHN (6202)
IN= 2--> OUT= 1
Routing time step (min) = 15.00

```

<----- DATA FOR SECTION (2021.0) ----->

```

Distance Elevation Manning
0.00 245.48 0.0400
29.68 242.80 0.0400
74.20 240.39 0.0400
155.81 237.12 0.0400
200.33 234.48 0.0400
244.85 234.67 0.0400
259.69 233.96 0.0400
274.53 233.84 0.0400
281.68 233.40 0.0400
285.78 232.40 0.0400 / 0.0300 Main Channel
286.78 231.40 0.0300 Main Channel
296.78 231.40 0.0300 Main Channel
299.78 232.40 0.0300 / 0.0400 Main Channel
311.62 233.74 0.0400
333.88 234.34 0.0400
497.11 235.32 0.0400
571.31 237.32 0.0400
675.18 239.10 0.0400
704.86 242.71 0.0400
734.54 242.62 0.0400

```

<----- TRAVEL TIME TABLE ----->

```

DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.50 231.90 221E+05 4.7 0.85 78.67
1.00 232.40 482E+05 15.3 1.28 52.40
1.60 233.00 913E+05 38.3 1.68 39.77
2.20 233.60 154E+06 72.9 1.90 35.24
2.80 234.20 278E+06 124.2 1.79 37.30
3.40 234.80 599E+06 214.3 1.44 46.59
4.01 235.41 125E+07 445.0 1.43 46.90
4.61 236.01 205E+07 838.4 1.64 40.81
5.21 236.61 293E+07 1361.6 1.86 35.89
5.81 237.21 389E+07 2011.2 2.08 32.23
6.41 237.81 494E+07 2753.5 2.24 29.91
7.01 238.41 612E+07 3635.0 2.39 28.04
7.61 239.01 741E+07 4572.5 2.53 26.43
8.21 239.61 880E+07 5599.3 2.74 24.44
8.82 240.22 102E+08 7510.0 2.94 22.72
9.42 240.82 117E+08 9201.9 3.15 21.23
10.02 241.42 132E+08 11059.7 3.35 19.95
10.62 242.02 148E+08 13073.4 3.54 18.87
11.22 242.62 164E+08 15242.1 3.73 17.93

```

<---- hydrograph ----> <-pipe / channel-->

```

AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8100) ***** 112.22 8.25 30.26 2.66 1.82
OUTFLOW : ID= 1 (6202) ***** 107.11 9.00 30.26 2.60 1.83

```

```

-----
ADD HYD (8112)
1 + 2 = 3
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (6202): 11366.35 107.112 9.00 30.26
+ ID2= 2 (8110): 468.50 46.262 6.00 59.66
=====
ID = 3 (8112): 11834.85 111.629 9.00 31.42

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
RESERVOIR (5555)
IN= 2--> OUT= 1
DT= 15.0 min
-----
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
-----
0.0000 0.0000 | ***** 19.2100
2.8500 0.5200 | ***** 24.7300
8.1800 1.2600 | ***** 30.8600
15.0900 2.2100 | ***** 37.6100
23.2700 3.3900 | ***** 41.2200
32.5300 4.7800 | ***** 48.9000
36.5100 5.3900 | ***** 66.4700
42.7600 6.3700 | ***** 100.1200
53.8500 8.1300 | ***** 140.4100
91.7300 14.3200 | ***** 140.4200

```

```

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (8112) ***** 111.629 9.00 31.42
OUTFLOW : ID= 1 (5555) ***** 110.160 9.50 31.42

```

```

PEAK FLOW REDUCTION [Qout/Qin] (%) = 98.68
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha.m.) = 17.4893

```

ADD HYD (8114)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (5555):	11834.85	110.160	9.50	31.42
+ ID2= 2 (8136):	2288.40	35.381	6.50	30.32
ID = 3 (8114):	14123.25	130.998	9.00	31.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	Total Imp(%)	Dir. Conn.(%)
STANDHYD (2061)	Area	(ha) = 25.00	Total Imp(%) = 93.00	Dir. Conn.(%) = 60.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)	23.25	1.75	
Dep. Storage (mm)	2.00	5.00	
Average Slope (%)	0.50	0.50	
Length (m)	408.25	40.00	
Mannings n	0.013	0.250	
Max.Eff.Inten.(mm/hr)	116.82	626.18	
over (min)	15.00	15.00	
Storage Coeff. (min)	6.87 (ii)	12.01 (ii)	
Unit Hyd. Tpeak (min)	15.00	15.00	
Unit Hyd. peak (cms)	0.10	0.08	
PEAK FLOW (cms)	4.50	2.40	*TOTALS*
TIME TO PEAK (hrs)	6.00	6.00	6.901 (iii)
RUNOFF VOLUME (mm)	86.50	74.95	81.88
TOTAL RAINFALL (mm)	88.50	88.50	88.50
RUNOFF COEFFICIENT	0.98	0.85	0.93

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CN* = 75.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5206)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IS 2--> OUT 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min	0.0000	0.0000	1.8990	1.7295
	0.0850	0.5280	2.1560	1.9661
	0.8990	0.8577	2.3740	2.2023
	1.3460	1.1601	2.7740	2.2123
	1.6060	1.4451	0.0000	0.0000
INFLOW : ID= 2 (2061)	25.000	6.901	6.00	81.88
OUTFLOW : ID= 1 (5206)	25.000	1.377	6.50	81.82

PEAK FLOW REDUCTION [Qout/Qin](%) = 19.95
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 1.1973

CALIB	Area	(ha)	Total Imp(%)	Dir. Conn.(%)
STANDHYD (2062)	Area	(ha) = 250.50	Total Imp(%) = 62.00	Dir. Conn.(%) = 39.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)	155.31	95.19	
Dep. Storage (mm)	2.00	5.00	
Average Slope (%)	0.50	0.50	
Length (m)	1292.28	40.00	
Mannings n	0.013	0.250	
Max.Eff.Inten.(mm/hr)	116.82	130.77	
over (min)	15.00	30.00	
Storage Coeff. (min)	13.72 (ii)	23.33 (ii)	
Unit Hyd. Tpeak (min)	15.00	30.00	
Unit Hyd. peak (cms)	0.08	0.04	
PEAK FLOW (cms)	23.96	16.56	*TOTALS*
TIME TO PEAK (hrs)	6.00	6.25	34.933 (iii)
RUNOFF VOLUME (mm)	86.50	52.78	65.93
TOTAL RAINFALL (mm)	88.50	88.50	88.50
RUNOFF COEFFICIENT	0.98	0.60	0.74

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CN* = 75.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8144)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (2052):	250.50	34.933	6.00	65.93
+ ID2= 2 (5206):	25.00	1.377	6.50	81.82
ID = 3 (8144):	275.50	35.836	6.00	67.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	Total Imp(%)	Dir. Conn.(%)
STANDHYD (2041)	Area	(ha) = 201.50	Total Imp(%) = 36.00	Dir. Conn.(%) = 20.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)	72.54	128.96	
Dep. Storage (mm)	2.00	5.00	
Average Slope (%)	0.50	0.50	
Length (m)	1159.02	40.00	
Mannings n	0.013	0.250	
Max.Eff.Inten.(mm/hr)	116.82	63.34	
over (min)	15.00	30.00	
Storage Coeff. (min)	12.86 (ii)	25.70 (ii)	
Unit Hyd. Tpeak (min)	15.00	30.00	
Unit Hyd. peak (cms)	0.08	0.04	
PEAK FLOW (cms)	10.14	10.19	*TOTALS*
TIME TO PEAK (hrs)	6.00	6.25	16.548 (iii)
RUNOFF VOLUME (mm)	86.50	33.30	43.94
TOTAL RAINFALL (mm)	88.50	88.50	88.50
RUNOFF COEFFICIENT	0.98	0.38	0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5204)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IS 2--> OUT 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min	0.0000	0.0000	12.4660	4.9468
	0.5050	1.9141	15.4770	5.3726
	2.8390	2.2056	18.2610	6.4806
	5.2410	3.6606	18.6610	6.4906
	7.9920	4.3236	0.0000	0.0000
INFLOW : ID= 2 (2041)	201.500	16.548	6.00	43.94
OUTFLOW : ID= 1 (5204)	201.500	6.626	6.75	43.94

PEAK FLOW REDUCTION [Qout/Qin](%) = 40.04
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 4.0032

CALIB	Area	(ha)	Total Imp(%)	Dir. Conn.(%)
STANDHYD (2042)	Area	(ha) = 122.90	Total Imp(%) = 60.00	Dir. Conn.(%) = 33.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)	73.74	49.16	
Dep. Storage (mm)	2.00	5.00	
Average Slope (%)	0.50	0.50	
Length (m)	905.17	40.00	
Mannings n	0.013	0.250	
Max.Eff.Inten.(mm/hr)	116.82	102.58	
over (min)	15.00	30.00	
Storage Coeff. (min)	11.08 (ii)	21.67 (ii)	
Unit Hyd. Tpeak (min)	15.00	30.00	
Unit Hyd. peak (cms)	0.08	0.05	
PEAK FLOW (cms)	10.76	6.87	*TOTALS*
TIME TO PEAK (hrs)	6.00	6.25	15.187 (iii)
RUNOFF VOLUME (mm)	86.50	40.08	55.40
TOTAL RAINFALL (mm)	88.50	88.50	88.50
RUNOFF COEFFICIENT	0.98	0.45	0.63

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
CN = 61.0 Is = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8140)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (2042):	322.90	15.187	6.00	55.40
+ ID2= 2 (5204):	201.50	6.626	6.75	43.94

ID = 3 (8140):	324.40	16.435	6.00	48.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6206) | Routing time step (min)'= 15.00
IN= 2--> OUT= 1 |

Distance	Elevation	Manning
0.00	253.07	0.0350
13.98	252.89	0.0350
27.95	251.99	0.0350
34.94	252.18	0.0350
41.93	252.02	0.0350
73.38	250.35	0.0350
94.34	247.09	0.0350
111.81	244.79	0.0350
115.31	244.24	0.0350
117.55	244.20	0.0350 / 0.0300
118.80	243.60	0.0300
122.30	243.60	0.0300
122.55	244.60	0.0300 / 0.0350
146.76	244.74	0.0350
164.23	247.26	0.0350
213.15	248.92	0.0350
241.10	249.51	0.0350
272.55	250.87	0.0350
300.50	252.26	0.0350
345.93	253.67	0.0350

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.30	243.90	.310E+04	1.4	1.23	36.25
0.60	244.20	.676E+04	4.6	1.81	24.64
1.12	244.72	.221E+05	17.1	2.07	21.60
1.64	245.24	.754E+05	64.1	2.28	19.60
2.17	245.77	.139E+06	149.8	2.88	15.50
2.69	246.29	.214E+06	273.3	3.43	13.04
3.21	246.81	.299E+06	436.1	3.91	11.43
3.73	247.33	.395E+06	632.6	4.30	10.40
4.25	247.85	.511E+06	836.4	4.39	10.18
4.77	248.37	.653E+06	1114.9	4.58	9.76
5.30	248.90	.822E+06	1472.1	4.81	9.30
5.82	249.42	.102E+07	1872.2	4.91	9.10
6.34	249.94	.126E+07	2444.6	5.22	8.56
6.86	250.46	.151E+07	3113.1	5.52	8.09
7.38	250.98	.180E+07	3843.5	5.74	7.79
7.90	251.50	.211E+07	4717.8	6.00	7.45
8.43	252.03	.245E+07	5701.4	6.24	7.16
8.95	252.55	.284E+07	6727.5	6.36	7.03
9.47	253.07	.326E+07	7885.3	6.48	6.90

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW: ID= 2 (8140)	324.40	16.44	6.00	48.28	1.09
OUTFLOW: ID= 1 (6206)	324.40	13.04	6.50	48.28	0.95

ADD HYD (8142)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6206):	324.40	13.037	6.50	48.28
+ ID2= 2 (8144):	275.50	35.836	6.00	67.37

ID = 3 (8142):	599.90	42.451	6.00	57.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8138)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8114):	14123.25	130.998	9.00	31.24
+ ID2= 2 (8142):	599.90	42.451	6.00	57.05

ID = 3 (8138):	14723.15	138.773	8.75	32.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6208) | Routing time step (min)'= 15.00
IN= 2--> OUT= 1 |

Distance	Elevation	Manning
0.00	249.19	0.1000
73.11	244.13	0.1000
121.86	243.39	0.1000
243.71	240.01	0.1000
353.38	238.75	0.1000
450.87	235.40	0.1000
499.61	231.40	0.1000
511.80	230.96	0.1000
523.98	230.12	0.1000
534.50	227.80	0.1000 / 0.0500
536.50	226.70	0.0500
546.50	226.70	0.0500
548.50	227.80	0.0500 / 0.1000
572.72	230.25	0.1000
682.40	230.50	0.1000
804.25	233.95	0.1000
926.11	235.83	0.1000
1047.96	240.70	0.1000
1145.45	243.26	0.1000
1206.38	247.03	0.1000

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.55	227.25	.111E+05	2.0	0.33	92.44
1.10	227.80	.242E+05	6.5	0.50	61.56
2.23	228.93	.700E+05	26.0	0.68	44.91
3.36	230.06	.150E+06	60.8	0.74	41.04
4.49	231.19	.446E+06	130.8	0.54	56.83
5.62	232.32	.925E+06	287.4	0.57	53.66
6.76	233.46	.152E+07	522.9	0.63	48.32
7.89	234.59	.233E+07	827.2	0.68	44.91
9.02	235.72	.312E+07	1219.5	0.72	42.61
10.15	236.85	.418E+07	1780.6	0.78	39.11
11.28	237.98	.537E+07	2479.2	0.85	36.07
12.41	239.11	.669E+07	3282.9	0.90	33.95
13.54	240.24	.824E+07	4228.6	0.94	32.47
14.67	241.37	.997E+07	5384.6	0.99	30.87
15.81	242.51	.119E+08	6698.4	1.03	29.56
16.94	243.64	.140E+08	8235.2	1.08	28.25
18.07	244.77	.162E+08	10104.2	1.14	26.73
19.20	245.90	.185E+08	12251.5	1.21	25.21
20.33	247.03	.209E+08	14597.0	1.28	23.89

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW: ID= 2 (8138)	138.77	8.75	32.29	4.55	0.54
OUTFLOW: ID= 1 (6208)	128.52	10.00	32.29	4.44	0.54

CALIB | STARCHYD (2082) | Area (ha)= 426.60
| ID= 1 DT=15.0 min | Total Imp(%)= 64.00 Dir. Conn.(%)= 37.00

Surface Area	IMPERVIOUS	PERVIOUS (I)
(ha)	(%)	(mm)
273.02	153.58	
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1686.42	40.00
Mannings n	0.113	0.250

Max. Eff. Inten. (mm/hr)=	116.82	133.62
over (min)	15.00	30.00
Storage Coeff. (min)=	16.10 (ii)	25.63 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.07	0.04

PEAK FLOW (cms)=	36.17	26.02
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	86.50	49.51
TOTAL RAINFALL (mm)=	86.50	86.50
RUNOFF COEFFICIENT =	0.98	0.56

TOTALS*		53.229 (iii)
		6.00
		63.26
		86.50
		0.71

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANHYD (2081) | Area (ha)= 265.70
| ID= 1 DT=15.0 min | Total Imp(%)= 35.00 Dir. Conn.(%)= 20.00
-----
| IMPERVIOUS | PERVIOUS (i)
| Surface Area (ha)= 93.00 | 172.71
| Dep. Storage (mm)= 2.00 | 5.00
| Average Slope (%)= 0.50 | 0.50
| Length (m)= 1330.91 | 40.00
| Mannings n = 0.013 | 0.250
-----
Max.Eff.Inten.(mm/hr)= 116.82 | 78.16
over (min)= 15.00 | 30.00
Storage Coeff. (min)= 13.97 (ii) | 25.77 (ii)
Unit Hyd. Tpeak (min)= 15.00 | 30.00
Unit Hyd. peak (cms)= 0.07 | 0.04
-----
*TOTALS*
PEAK FLOW (cms)= 12.94 | 16.91 | 23.754 (iii)
TIME TO PEAK (hrs)= 6.00 | 6.25 | 6.00
RUNOFF VOLUME (mm)= 86.50 | 41.24 | 50.29
TOTAL RAINFALL (mm)= 88.50 | 88.50 | 88.50
RUNOFF COEFFICIENT = 0.98 | 0.47 | 0.57
  
```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| RESERVOIR (5208) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
| OUTFLOW | STORAGE | OUTFLOW | STORAGE
| (cms) | (ha.m.) | (cms) | (ha.m.)
-----
| 0.0000 | 0.0000 | 16.8530 | 6.3371
| 0.8360 | 2.7353 | 21.1630 | 7.3975
| 4.7810 | 3.1070 | 25.1140 | 8.4406
| 8.9050 | 4.2484 | 25.5140 | 8.4506
| 12.6070 | 5.1796 | 0.0000 | 0.0000
-----
| AREA | QPEAK | TPEAK | R.V.
| (ha) | (cms) | (hrs) | (mm)
-----
| INFLOW : ID= 2 (2081) | 265.700 | 23.754 | 6.00 | 50.29
| OUTFLOW : ID= 1 (5208) | 265.700 | 12.754 | 6.75 | 50.29
-----
| PEAK FLOW REDUCTION [Qout/Qin](%)= 53.69
| TIME SHIFT OF PEAK FLOW (min)= 45.00
| MAXIMUM STORAGE USED (ha.m.)= 5.3059
  
```

```

-----
| ADD HYD (8148) |
| 1 + 2 = 3 |
-----
| ID1= 1 (2082): | AREA QPEAK TPEAK R.V.
| | (ha) (cms) (hrs) (mm)
-----
| + ID2= 2 (5208): | 265.70 12.754 6.75 50.29
| |
| =====
| ID = 3 (8148): | 692.30 55.182 6.00 58.28
| |
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8146) |
| 1 + 2 = 3 |
-----
| ID1= 1 (6208): | AREA QPEAK TPEAK R.V.
| | (ha) (cms) (hrs) (mm)
-----
| + ID2= 2 (8148): | 692.30 55.182 6.00 58.28
| |
| =====
| ID = 3 (8146): | 15415.45 133.593 10.00 33.46
| |
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6210) |
| ID= 2--> OUT= 1 |
| Routing time step (min)'= 15.00
-----
| <----- DATA FOR SECTION (2101.0) ----->
| Distance Elevation Manning
  
```

	0.00	241.81	0.0800
	28.43	241.78	0.0800
	85.30	237.04	0.0800
	149.27	231.32	0.0800
	170.60	230.10	0.0800
	255.89	229.37	0.0800
	277.22	228.68	0.0800
	284.33	228.42	0.0800
	291.43	228.16	0.0800
	297.76	227.00	0.0800 / 0.0350
	298.76	225.00	0.0350
	312.76	225.00	0.0350
	313.76	227.00	0.0350 / 0.0800
	326.97	229.50	0.0800
	348.30	230.29	0.0800
	376.73	229.76	0.0800
	490.46	235.75	0.0800
	568.65	236.24	0.0800
	661.06	241.85	0.0800
	703.71	246.04	0.0800

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
-----
0.67 225.67 141E+05 6.6 0.69 35.86
1.33 226.33 289E+05 20.4 1.04 23.65
2.00 227.00 443E+05 39.3 1.31 18.81
2.93 227.93 730E+05 77.8 1.57 15.64
3.85 228.85 123E+06 131.1 1.57 15.65
4.78 229.78 228E+06 205.6 1.33 18.51
5.70 230.70 503E+06 356.7 1.05 23.49
6.63 231.63 848E+06 608.7 1.06 23.17
7.55 232.55 123E+07 946.4 1.14 21.65
8.48 233.48 165E+07 1367.0 1.22 20.13
9.41 234.41 211E+07 1871.8 1.31 18.80
10.33 235.33 261E+07 2463.2 1.40 17.65
11.26 236.26 317E+07 2999.4 1.40 17.63
12.18 237.18 384E+07 3848.3 1.48 16.63
13.11 238.11 454E+07 4806.0 1.56 15.75
14.03 239.03 528E+07 5876.9 1.64 14.97
14.96 239.96 605E+07 7062.7 1.72 14.29
15.88 240.88 687E+07 8365.0 1.80 13.68
16.81 241.81 771E+07 9506.9 1.82 13.52
  
```

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
-----
INFLOW : ID= 2 (8146) ***** 133.59 10.00 33.46 3.88 1.56
OUTFLOW : ID= 1 (6210) ***** 132.71 10.25 33.46 3.87 1.57
  
```

```

-----
| CALIB |
| NASHYD (0210) | Area (ha)= 218.27 Curve Number (CN)= 80.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.08
-----
| Unit Hyd Qpeak (cms)= 3.454
  
```

```

PEAK FLOW (cms)= 4.665 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 46.998
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.531
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8150) |
| 1 + 2 = 3 |
-----
| ID1= 1 (0210): | AREA QPEAK TPEAK R.V.
| | (ha) (cms) (hrs) (mm)
-----
| + ID2= 2 (6210): | 15415.45 132.705 10.25 33.46
| |
| =====
| ID = 3 (8150): | 15633.72 135.954 10.00 33.65
| |
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0218) | Area (ha)= 152.25 Curve Number (CN)= 69.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.79
-----
| Unit Hyd Qpeak (cms)= 3.278
  
```

```

PEAK FLOW (cms)= 3.023 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 34.774
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.393
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (0216)
ID= 1 DT=15.0 min
Area (ha)= 145.77
Curve Number (CN)= 75.0
Ia (mm)= 5.00
of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.52

Unit Hyd Qpeak (cms)= 4.768
PEAK FLOW (cms)= 4.797 (i)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 40.355
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.456

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (2121)
ID= 1 DT=15.0 min
Area (ha)= 56.00
Total Imp(%)= 93.00
Dir. Conn.(%)= 75.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 52.08 3.92
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 611.01 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten.(mm/hr)= 116.82 344.46
over (min)= 15.00 30.00
Storage Coeff. (min)= 8.96 (ii) 15.28 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.09 0.05
TOTALS
PEAK FLOW (cms)= 11.94 2.15 13.450 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 86.50 63.72 80.81
TOTAL RAINFALL (mm)= 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.72 0.91

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5212)
IN= 2--> OUT= 1
DT= 15.0 min
OUTFLOW STORAGE OUTFLOW STORAGE
(cms) (ha.m.) (cms) (ha.m.)
0.0000 0.0000 3.7460 1.9720
0.1900 1.1850 4.7040 2.0838
1.0850 1.4413 5.4760 2.1940
2.0210 1.5949 5.8760 2.2040
2.8030 1.7848 0.0000 0.0000

AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (2121) 56.000 13.450 6.00 80.81
OUTFLOW: ID= 1 (5212) 56.000 6.106 6.25 80.78

PEAK FLOW REDUCTION [Qout/qin](%)= 45.40
TIME SHIFT OF PEAK FLOW (min)= 15.00
MAXIMUM STORAGE USED (ha.m.)= 2.2225

CALIB
STANDHYD (2122)
ID= 1 DT=15.0 min
Area (ha)= 272.40
Total Imp(%)= 29.00
Dir. Conn.(%)= 23.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 79.00 193.40
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1347.59 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten.(mm/hr)= 116.82 61.99
over (min)= 15.00 30.00
Storage Coeff. (min)= 14.07 (ii) 27.03 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.07 0.04
TOTALS
PEAK FLOW (cms)= 15.21 14.59 24.423 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00

RUNOFF VOLUME (mm)= 86.50 37.21 48.55
TOTAL RAINFALL (mm)= 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.42 0.55

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8152)
1 * 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID= 1 (2122): 272.40 24.423 6.00 48.55
+ ID= 2 (5212): 56.00 6.106 6.25 80.78
ID = 3 (8152): 328.40 27.753 6.25 54.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6214)
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (2141.0)
Distance Elevation Manning
0.00 259.29 0.0500
15.33 258.91 0.0500
38.33 258.57 0.0500
65.16 257.93 0.0500
95.82 253.44 0.0500
111.15 251.21 0.0500
130.32 249.87 0.0500
149.48 248.51 0.0500
150.66 248.50 0.0500 /0.0370 Main Channel
150.81 247.50 0.0370 Main Channel
154.66 247.50 0.0370 Main Channel
155.66 248.50 0.0370 /0.0500 Main Channel
160.98 248.50 0.0500
187.81 249.78 0.0500
199.81 252.13 0.0500
245.30 254.04 0.0500
264.46 253.99 0.0500
298.96 255.23 0.0500
329.62 257.02 0.0500
379.45 258.82 0.0500

TRAVEL TIME TABLE
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.50 248.00 .648E+04 2.1 1.00 52.30
1.00 248.50 .139E+05 6.4 1.44 36.35
1.61 249.11 .570E+05 22.6 1.24 42.01
2.21 249.71 .143E+06 63.6 1.39 37.61
2.82 250.32 .263E+06 142.9 1.70 30.69
3.43 250.93 .403E+06 257.2 2.00 26.11
4.04 251.54 .562E+06 410.4 2.29 22.82
4.64 252.14 .735E+06 601.3 2.56 20.37
5.25 252.75 .933E+06 791.0 2.66 19.67
5.86 253.36 .117E+07 1038.0 2.79 18.74
6.46 253.96 .144E+07 1344.4 2.93 17.82
7.07 254.57 .178E+07 1664.1 2.93 17.83
7.68 255.18 .217E+07 2140.2 3.10 16.87
8.28 255.78 .259E+07 2722.9 3.30 15.83
8.89 256.39 .304E+07 3387.1 3.50 14.94
9.50 257.00 .351E+07 4132.7 3.69 14.16
10.11 257.61 .402E+07 4913.0 3.83 13.64
10.71 258.21 .458E+07 5648.3 3.87 13.50
11.32 258.82 .521E+07 6343.1 3.82 13.68

hydrograph
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8152) 328.40 27.75 6.25 54.04 1.68 1.26
OUTFLOW: ID= 1 (6214) 328.40 17.05 6.50 54.04 1.40 1.30

CALIB
STANDHYD (0214)
ID= 1 DT=15.0 min
Area (ha)= 316.95
Total Imp(%)= 25.00
Dir. Conn.(%)= 9.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 79.24 237.71
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1453.62 40.00
Mannings n = 0.013 0.250

Max. Ref. Inten. (mm/hr) = 116.82 61.86
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 14.73 (ii) 27.69 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.07 0.04

TOTALS
 PEAK FLOW (cms) = 6.80 17.64 20.900 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.25
 RUNOFF VOLUME (mm) = 86.50 33.48 38.25
 TOTAL RAINFALL (mm) = 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.38 0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 62.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8154) |
 1 + 2 = 3 |
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0214): 316.95 20.900 6.25 38.25
 + ID2= 2 (6214): 328.40 17.047 6.50 54.04
 ID = 3 (8154): 645.35 35.268 6.25 46.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8156) |
 1 + 2 = 3 |
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0216): 145.77 4.797 6.50 40.36
 + ID2= 2 (8154): 645.35 35.268 6.25 46.29
 ID = 3 (8156): 791.12 39.789 6.25 45.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6218) |
 IN= 2--> OUT= 1 |
 Routing time step (min) = 15.00

<----- DATA FOR SECTION (2181.0) ----->
 Distance Elevation Manning
 0.00 256.38 0.0450
 7.70 254.85 0.0450
 15.40 253.20 0.0450
 19.25 252.29 0.0450
 23.10 251.17 0.0450
 28.94 236.16 0.0450
 80.87 235.80 0.0450
 82.79 235.45 0.0450
 84.57 234.60 0.0450 / 0.0300 Main Channel
 85.07 233.60 0.0300 Main Channel
 88.57 233.60 0.0300 Main Channel
 93.57 233.60 0.0300 Main Channel
 94.07 234.60 0.0300 / 0.0450 Main Channel
 96.27 235.13 0.0450
 98.19 235.20 0.0450
 100.12 235.32 0.0450
 138.63 239.95 0.0450
 180.98 244.80 0.0450
 188.69 245.00 0.0450
 190.61 256.00 0.0450

TRAVEL TIME TABLE
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
 (m) (m) (cu.m.) (cms) (m/s) (min)
 1.00 234.60 181E+05 6.0 0.66 50.64
 2.13 235.73 518E+05 23.8 0.93 36.27
 3.25 236.85 119E+06 60.0 1.02 33.03
 4.38 237.98 217E+06 120.6 1.12 29.99
 5.51 239.11 346E+06 210.5 1.23 27.39
 6.63 240.23 506E+06 334.1 1.33 25.22
 7.76 241.36 697E+06 495.6 1.43 23.44
 8.88 242.48 920E+06 699.8 1.53 21.91
 10.01 243.61 117E+07 950.8 1.63 20.59
 11.14 244.74 146E+07 1252.5 1.73 19.45
 12.26 245.86 178E+07 1619.6 1.83 18.37
 13.39 246.99 212E+07 2078.5 1.97 17.01
 14.52 248.12 247E+07 2589.0 2.11 15.88
 15.64 249.24 282E+07 3149.7 2.25 14.94
 16.77 250.37 319E+07 3759.8 2.37 14.14

17.89 251.49 356E+07 4419.5 2.50 13.44
 19.02 252.62 395E+07 5126.4 2.61 12.84
 20.15 253.75 435E+07 5871.6 2.72 12.34
 21.27 254.87 476E+07 6663.2 2.82 11.89

<---- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8156) 791.12 39.79 6.25 45.19 2.62 0.96
 OUTFLOW : ID= 1 (6218) 791.12 28.70 6.75 45.19 2.28 0.94

ADD HYD (8158) |
 1 + 2 = 3 |
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0218): 352.25 3.023 7.00 34.77
 + ID2= 2 (6218): 791.12 28.701 6.75 45.19
 ID = 3 (8158): 943.37 31.704 6.75 43.51

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8151) |
 1 + 2 = 3 |
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8150): 15633.72 135.954 10.00 33.65
 + ID2= 2 (8158): 943.37 31.704 6.75 43.51
 ID = 3 (8151): 16577.09 152.407 7.00 34.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5504) |
 IN= 2--> OUT= 1 |
 DT= 15.0 min
 OUTFLOW STORAGE OUTFLOW STORAGE
 (cms) (ha.m.) (cms) (ha.m.)
 0.0000 0.0000 33.0100 0.6700
 2.6400 0.0500 37.4800 0.7800
 4.8300 0.0900 47.5400 1.0100
 7.4100 0.1300 60.2300 1.2800
 10.3300 0.1900 71.9530 1.5258
 13.5300 0.2500 ***** 3.0529
 16.9900 0.3200 ***** 5.3299
 20.6800 0.3900 ***** 8.4148
 24.6000 0.4800 ***** 11.7637
 28.7100 0.5700 ***** 11.7737

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (8151) ***** 152.407 7.00 34.21
 OUTFLOW : ID= 1 (5504) ***** 152.405 7.25 34.21

PEAK FLOW REDUCTION [Qout/ Qin] (\$) = 100.00
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha.m.) = 3.3458

ROUTE CHN (6220) |
 IN= 2--> OUT= 1 |
 Routing time step (min) = 15.00

<----- DATA FOR SECTION (2201.0) ----->
 Distance Elevation Manning
 0.00 245.59 0.0600
 5.76 245.45 0.0600
 23.03 242.27 0.0600
 57.58 232.35 0.0600
 86.37 226.18 0.0600
 109.41 224.31 0.0600
 115.17 223.86 0.0600
 120.92 222.78 0.0600
 126.68 221.93 0.0600
 128.06 221.50 0.0600 / 0.0300 Main Channel
 129.56 220.00 0.0300 Main Channel
 141.56 220.00 0.0300 Main Channel
 143.06 221.50 0.0300 / 0.0600 Main Channel
 149.72 223.77 0.0600
 155.47 225.09 0.0600
 253.36 229.21 0.0600
 333.98 227.50 0.0600
 454.90 229.25 0.0600
 500.97 233.11 0.0600
 570.07 237.42 0.0600

TRAVEL TIME TABLE
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
 (m) (m) (cu.m.) (cms) (m/s) (min)

0.75	220.75	.262E+05	5.5	0.57	79.44
1.50	221.50	.555E+05	17.5	0.86	52.89
2.44	222.44	.103E+06	42.7	1.14	40.03
3.37	223.37	.171E+06	79.4	1.27	35.98
4.31	224.31	.263E+06	128.1	1.33	34.22
5.25	225.25	.393E+06	191.7	1.34	34.19
6.18	226.18	.594E+06	276.7	1.28	35.77
7.12	227.12	.872E+06	399.9	1.26	36.33
8.06	228.06	1.27E+07	542.7	1.17	38.92
8.99	228.99	1.99E+07	770.0	1.06	43.01
9.93	229.93	2.97E+07	1151.8	1.06	42.95
10.86	230.86	4.00E+07	1652.9	1.13	40.33
11.80	231.80	5.07E+07	2251.8	1.22	37.53
12.74	232.74	6.18E+07	2946.5	1.31	34.97
13.67	233.67	7.33E+07	3726.8	1.39	32.79
14.61	234.61	8.53E+07	4591.0	1.48	30.96
15.55	235.55	9.77E+07	5544.1	1.55	29.37
16.48	236.48	1.11E+08	6586.7	1.63	27.99
17.42	237.42	1.24E+08	7719.2	1.71	26.77

<---- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (5504) ***** 152.40 7.25 34.21 4.67 1.33
 OUTFLOW : ID= 1 (6220) ***** 145.34 9.75 34.21 4.57 1.33

CALIB (0220) | Area (ha)= 169.10 Curve Number (CN)= 73.0
 NASHYD (0220) | Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.50

Unit Hyd Qpeak (cms)= 1.923
 PEAK FLOW (cms)= 2.291 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 39.073
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.442

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8160) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0220): 169.10 2.291 8.00 39.07
 + ID2= 2 (6220): 16577.09 145.339 9.75 34.21
 ID = 3 (8160): 16746.19 147.406 9.75 34.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0225) | Area (ha)= 237.58 Curve Number (CN)= 75.0
 NASHYD (0225) | Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.05

Unit Hyd Qpeak (cms)= 3.852
 PEAK FLOW (cms)= 4.479 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 41.069
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.464

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (2221) | Area (ha)= 145.20
 ID= 1 DT=15.0 min | Total Imp(%)= 36.00 Dir. Conn.(%)= 19.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area	(ha)= 52.27	92.93	
Dep. Storage	(mm)= 2.00	5.00	
Average Slope	(%)= 0.50	0.50	
Length	(m)= 983.87	40.00	
Mannings n	= 0.013	0.250	
Max. Eff. Inten. (mm/hr)	= 116.82	92.49	
over (min)	15.00	30.00	
Storage Coeff. (min)	= 11.65 (ii)	22.69 (iii)	
Unit Hyd. Tpeak (min)	= 15.00	30.00	
Unit Hyd. peak (cms)	= 0.08	0.04	
TOTALS			
PEAK FLOW (cms)	= 7.19	11.51	14.703 (iii1)
TIME TO PEAK (hrs)	= 6.00	6.25	6.00

RUNOFF VOLUME (mm)= 86.50 47.20 54.67
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.53 0.62

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5222) |
 IN= 2 -> OUT= 1 |
 DT= 15.0 min |

OUTFLOW		STORAGE		OUTFLOW		STORAGE	
(cms)	(ha.m.)	(cms)	(ha.m.)	(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	0.0000	6.6430	5.6430	3.7677		
0.4620	2.0014	8.2830	4.2576				
2.0160	2.2850	14.1970	4.6909				
3.6250	2.7987	14.5970	4.7009				
5.0230	3.2496	0.0000	0.0000				

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (2221) 145.200 14.703 6.00 54.67
 OUTFLOW : ID= 1 (5222) 145.200 6.391 6.75 54.66

PEAK FLOW REDUCTION [Qout/Qin](%)= 43.47
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 3.6903

CALIB STANHYD (2222) | Area (ha)= 74.30
 ID= 1 DT=15.0 min | Total Imp(%)= 78.00 Dir. Conn.(%)= 41.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area	(ha)= 57.95	16.35	
Dep. Storage	(mm)= 2.00	5.00	
Average Slope	(%)= 0.50	0.50	
Length	(m)= 703.80	40.00	
Mannings n	= 0.013	0.250	

Max. Eff. Inten. (mm/hr)= 116.82 258.74
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 9.93 (ii) 16.84 (iii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.05

PEAK FLOW (cms)	= 8.47	6.50	12.995 (iii)
TIME TO PEAK (hrs)	= 6.00	6.25	6.00
RUNOFF VOLUME (mm)	= 86.50	63.50	72.93
TOTAL RAINFALL (mm)	= 88.50	88.50	88.50
RUNOFF COEFFICIENT	= 0.98	0.72	0.82

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8164) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (2222): 74.30 12.995 6.00 72.93
 + ID2= 2 (5222): 145.20 6.391 6.75 54.66
 ID = 3 (8164): 219.50 13.408 6.00 60.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANHYD (0224) | Area (ha)= 140.45
 ID= 1 DT=15.0 min | Total Imp(%)= 34.00 Dir. Conn.(%)= 26.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area	(ha)= 47.75	92.70	
Dep. Storage	(mm)= 2.00	5.00	
Average Slope	(%)= 0.50	0.50	
Length	(m)= 967.64	40.00	
Mannings n	= 0.013	0.250	
Max. Eff. Inten. (mm/hr)	= 116.82	85.39	

over (min) 15.00 30.00
 Storage Coeff (min)= 11.54 (ii) 22.93 (iii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

 PEAK FLOW (cms)= 9.55 10.55 16.459 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 86.50 48.96 58.72
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.55 0.66

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8166)	1	2	3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0224):	140.45	16.459	6.00	58.72			
+ ID2= 2 (8164):	219.50	13.408	6.00	60.84			
ID = 3 (8166):	359.95	29.867	6.00	60.02			

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN# (6226) |
 IN= 2----> OUT= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (2261.0) -----

Distance	Elevation	Manning
0.00	245.35	0.0600
8.27	242.72	0.0600
16.54	240.01	0.0600
31.02	235.31	0.0600
41.36	233.14	0.0600
90.99	227.73	0.0600
93.06	227.50	0.0600
95.13	227.27	0.0600
95.83	227.00	0.0600
100.83	226.50	0.0600 /0.0350 Main Channel
101.33	225.50	0.0350 Main Channel
101.83	226.50	0.0350 /0.0600 Main Channel
105.83	227.00	0.0600
107.54	227.39	0.0600
109.60	227.62	0.0600
111.67	227.86	0.0600
146.83	231.76	0.0600
173.71	234.77	0.0600
186.12	238.23	0.0600
202.67	238.35	0.0600

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	226.00	.403E+03	0.0	0.16	330.59
1.00	226.50	.161E+04	0.1	0.26	208.26
1.70	227.20	.193E+05	1.5	0.27	195.72
2.39	227.89	.541E+05	5.9	0.35	152.27
3.09	228.59	.119E+06	16.0	0.43	124.08
3.79	229.29	.212E+06	33.8	0.51	104.65
4.49	229.99	.334E+06	61.2	0.59	90.97
5.18	230.68	.484E+06	99.7	0.66	80.88
5.88	231.38	.663E+06	151.0	0.73	73.13
6.58	232.08	.870E+06	216.5	0.80	66.95
7.27	232.77	.111E+07	297.5	0.87	61.91
7.97	233.47	.137E+07	398.1	0.94	57.29
8.67	234.17	.165E+07	518.9	1.01	53.12
9.36	234.86	.196E+07	658.9	1.08	49.59
10.06	235.56	.228E+07	827.0	1.17	46.02
10.76	236.26	.262E+07	1015.4	1.25	42.96
11.46	236.96	.296E+07	1223.2	1.33	40.41
12.15	237.65	.332E+07	1444.6	1.40	38.26
12.85	238.35	.368E+07	1598.2	1.40	38.42

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8166)	359.95	29.87	6.00	60.02	3.64
OUTFLOW: ID= 1 (6226)	359.95	10.29	6.75	60.01	2.70

ADD HYD (8168)	1	2	3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
----------------	---	---	---	-----------	-------------	-------------	-----------

ID#	Area (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0226):	237.58	4.479	7.25	41.07
+ ID2= 2 (6226):	359.95	10.292	6.75	60.01
ID = 3 (8168):	597.53	14.573	6.75	52.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8162)	1	2	3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8160):	16746.19	147.406	9.75	34.26			
+ ID2= 2 (8168):	597.53	14.573	6.75	52.48			
ID = 3 (8162):	17343.72	157.961	8.75	34.89			

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN# (6228) |
 IN= 2----> OUT= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (2281.0) -----

Distance	Elevation	Manning
0.00	245.00	0.0600
18.60	244.74	0.0600
37.20	242.47	0.0600
93.01	233.71	0.0600
125.57	226.95	0.0600
213.93	221.63	0.0600
218.58	221.15	0.0600
223.23	220.45	0.0600
225.03	220.55	0.0600 /0.0300 Main Channel
225.53	218.05	0.0300 Main Channel
232.55	218.05	0.0300 Main Channel
240.03	218.05	0.0300 Main Channel
241.03	220.55	0.0300 /0.0600 Main Channel
246.48	221.00	0.0600
251.13	221.27	0.0600
255.78	221.22	0.0600
325.54	225.25	0.0600
367.40	228.91	0.0600
404.60	231.75	0.0600
460.41	245.00	0.0600

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
1.25	219.30	.338E+05	16.0	0.86	35.15
2.50	220.55	.696E+05	48.3	1.26	24.02
3.94	221.99	.168E+06	116.0	1.26	24.14
5.38	223.43	.392E+06	247.5	1.15	26.37
6.81	224.86	.743E+06	472.6	1.16	26.20
8.25	226.30	.122E+07	821.6	1.23	24.67
9.69	227.74	.179E+07	1320.1	1.34	22.59
11.13	229.18	.243E+07	1965.7	1.47	20.58
12.57	230.62	.313E+07	2744.2	1.59	19.02
14.01	232.06	.390E+07	3683.4	1.72	17.66
15.44	233.49	.472E+07	4824.1	1.86	16.30
16.88	234.93	.557E+07	6092.4	1.99	15.23
18.32	236.37	.646E+07	7503.3	2.11	14.35
19.76	237.81	.739E+07	9061.2	2.23	13.59
21.20	239.25	.836E+07	10767.6	2.34	12.94
22.64	240.69	.937E+07	12624.8	2.45	12.37
24.07	242.12	.104E+08	14634.9	2.56	11.87
25.51	243.56	.115E+08	16762.8	2.65	11.45
26.95	245.00	.127E+08	18699.9	2.69	11.28

----- hydrograph ----- <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8162)	157.96	8.75	34.89	4.40	1.22
OUTFLOW: ID= 1 (6228)	155.60	9.75	34.89	4.36	1.22

CALIB	NASHVD (0228)	Area (ha)	Curve Number (CN)= 61.0
		310.54	
		5.00	# of Linear Res. (N)= 1.50
		1.12	U.N. Tp (hrs)= 1.12

Unit Hyd Qpeak (cms)= 4.751

PEAK FLOW (cms)= 3.743 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 28.109
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.318

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8170) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0228): 310.54 3.743 7.50 28.11
+ ID2= 2 (6228): 17343.72 155.597 9.75 34.89
=====
ID = 3 (8170): 17654.26 158.534 9.75 34.77

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| STANDHYD (2302) |
| ID= 1 DT=15.0 min |
-----
Area (ha)= 126.70
Total Imp(%)= 23.00 Dir. Conn.(%)= 9.00

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-----
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 29.14 97.56
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 919.06 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 116.82 71.35
over (min) 15.00 30.00
Storage Coeff. (min)= 11.19 (ii) 23.43 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04
*TOTALS*
PEAK FLOW (cms)= 3.01 9.12 10.282 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.25
RUNOFF VOLUME (mm)= 86.50 39.27 43.52
TOTAL RAINFALL (mm)= 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.44 0.49

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANDHYD (2301) |
| ID= 1 DT=15.0 min |
-----
Area (ha)= 78.80
Total Imp(%)= 50.00 Dir. Conn.(%)= 19.00

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-----
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 39.40 39.40
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 724.80 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 116.82 116.70
over (min) 15.00 30.00
Storage Coeff. (min)= 9.70 (ii) 19.76 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.09 0.05
*TOTALS*
PEAK FLOW (cms)= 4.14 6.55 8.449 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 86.50 46.81 54.35
TOTAL RAINFALL (mm)= 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.53 0.61

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| RESERVOIR (5230) |
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
0.0000 0.0000 | 5.6350 1.5914
0.3160 0.7624 | 6.9200 1.8001
1.7040 0.8931 | 7.9010 2.0070
3.5140 1.1218 | 8.3010 2.0170
4.5020 1.3405 | 0.0000 0.0000

```

```

-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (2301) 78.800 8.449 6.00 54.35
OUTFLOW: ID= 1 (5230) 78.800 5.220 6.50 54.34
-----
PEAK FLOW REDUCTION [Qout/qin](%)= 61.78
TIME SHIFT OF PEAK FLOW (min)= 30.00
MAXIMUM STORAGE USED (ha.m.)= 1.5108

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-----
| ADD HYD (8172) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (2302): 126.70 10.282 6.25 43.52
+ ID2= 2 (5230): 78.80 5.220 6.50 54.34
=====
ID = 3 (8172): 205.50 14.869 6.25 47.67

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0232) |
| ID= 1 DT=15.0 min |
-----
Area (ha)= 314.80 Curve Number (CN)= 58.0
Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 1.00

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```

Unit Hyd Qpeak (cms)= 5.391
PEAK FLOW (cms)= 3.790 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 25.804
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.292

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8173) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0232): 314.80 3.790 7.25 25.80
+ ID2= 2 (8172): 205.50 14.869 6.25 47.67
=====
ID = 3 (8173): 520.30 17.765 6.25 34.44

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8174) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8170): 17654.26 158.534 9.75 34.77
+ ID2= 2 (8173): 520.30 17.765 6.25 34.44
=====
ID = 3 (8174): 18174.56 162.676 9.50 34.76

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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| ROUTE CN# (6234) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

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-----
<----- DATA FOR SECTION (2341.0) ----->
Distance Elevation Manning
0.00 228.00 0.0550
42.86 223.21 0.0550
85.72 219.56 0.0550
107.15 219.42 0.0550
128.58 219.39 0.0550
149.99 219.30 0.0550
171.40 219.26 0.0550
192.81 219.24 0.0550
214.22 219.20 0.0550 /0.0350 Main Channel
235.63 217.20 0.0350 Main Channel
257.04 217.20 0.0350 Main Channel
278.45 217.20 0.0350 Main Channel
300.00 219.20 0.0350 /0.0550 Main Channel
321.41 219.27 0.0550
342.82 219.26 0.0550
364.23 219.29 0.0550
385.64 222.91 0.0550
407.05 227.43 0.0550
428.46 227.80 0.0550
449.87 228.00 0.0550
471.28 228.00 0.0550

```

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.50	217.70	.887E+05	7.2	0.31	206.54
1.00	218.20	.179E+06	22.7	0.48	131.74
1.50	218.70	.272E+06	44.5	0.62	101.75
2.00	219.20	.366E+06	71.8	0.75	84.97
2.59	219.79	.993E+06	136.1	0.52	121.59
3.17	220.37	.189E+07	258.6	0.52	121.80
3.76	220.96	.294E+07	434.7	0.56	112.82
4.35	221.55	.415E+07	665.9	0.61	103.88
4.93	222.13	.551E+07	955.0	0.66	96.22
5.52	222.72	.703E+07	1305.1	0.71	89.80
6.11	223.31	.868E+07	1744.9	0.77	82.91
6.69	223.89	.104E+08	2261.4	0.83	76.46
7.28	224.48	.121E+08	2838.0	0.89	71.12
7.87	225.07	.139E+08	3472.9	0.95	66.63
8.45	225.65	.157E+08	4165.0	1.01	62.82
9.04	226.24	.176E+08	4913.5	1.07	59.54
9.63	226.83	.194E+08	5717.7	1.12	56.68
10.21	227.41	.214E+08	6577.1	1.17	54.18
10.80	228.00	.235E+08	6919.5	1.12	56.61

		<---- hydrograph ---->		<-pipe / channel-->	
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH
	(ha)	(cms)	(hrs)	(mm)	(m)
INFLOW : ID= 2 (8174)	*****	162.68	9.50	34.76	2.71
OUTFLOW : ID= 1 (6234)	*****	139.73	12.25	34.76	2.60

CALIB
STANDHYD (0234)
ID= 1 DT=15.0 min

Area (ha) = 267.16
Total Imp(%) = 22.00
Dir. Conn.(%) = 8.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	58.78	208.38
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1334.57	40.00
Mannings n	0.013	0.250

Max. Ref. Inten. (mm/hr)	116.82	17.79
over (min)	15.00	45.00
Storage Coeff. (min)	13.99 (ii)	35.33 (ii)
Unit Hyd. Peak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.07	0.03

TOTALS

PEAK FLOW (cms)	5.20	5.91	7.610 (iii)
TIME TO PEAK (hrs)	6.00	6.50	6.00
RUNOFF VOLUME (mm)	86.50	16.86	22.43
TOTAL RAINFALL (mm)	88.50	88.50	88.50
RUNOFF COEFFICIENT	0.98	0.19	0.25

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 39.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8176)
1 + 2 = 3

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0234):	267.16	7.610	6.00	22.43
+ ID2= 2 (6234):	18174.56	139.730	12.25	34.76
ID = 3 (8176):	18441.72	140.272	12.00	34.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHYD (0238)
ID= 1 DT=15.0 min

Area (ha) = 311.70
Curve Number (CN) = 53.0
Ia (mm) = 5.00
of Linear Res. (N) = 1.50
U.H. Tp (hrs) = 1.56

Unit Hyd Qpeak (cms) = 3.405

PEAK FLOW (cms)	2.301 (i)
TIME TO PEAK (hrs)	8.250
RUNOFF VOLUME (mm)	22.464
TOTAL RAINFALL (mm)	88.500
RUNOFF COEFFICIENT	0.254

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0236)
ID= 1 DT=15.0 min

Area (ha) = 494.49
Curve Number (CN) = 54.0
Ia (mm) = 5.00
of Linear Res. (N) = 1.50
U.H. Tp (hrs) = 1.24

Unit Hyd Qpeak (cms) = 6.830

PEAK FLOW (cms)	4.497 (i)
TIME TO PEAK (hrs)	8.000
RUNOFF VOLUME (mm)	23.078
TOTAL RAINFALL (mm)	88.500
RUNOFF COEFFICIENT	0.261

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6238)
IN= 2--> OUT= 1

Routing time step (min) = 15.00

----- DATA FOR SECTION (2381.0) -----

Distance	Elevation	Manning
0.00	241.54	0.0380
602.55	227.00	0.0380
1702.00	224.50	0.0380
1721.25	223.00	0.0380
1725.10	222.60	0.0380
1780.94	222.50	0.0380
1782.87	222.45	0.0380
1784.79	222.40	0.0380 / 0.0300
1786.67	221.75	0.0300
1787.07	220.75	0.0300
1787.57	220.75	0.0300
1791.57	220.75	0.0300
1794.07	221.75	0.0300
1798.27	222.00	0.0300
1800.19	222.35	0.0300 / 0.0380
1802.12	222.47	0.0380
1840.63	223.00	0.0380
1900.00	225.00	0.0380
1950.00	226.00	0.0380
2242.61	240.00	0.0380

----- TRAVEL TIME TABLE -----

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.80	221.55	.101E+05	5.2	1.14	32.36
1.60	222.35	.296E+05	18.7	1.40	26.36
2.64	223.39	.249E+06	151.8	1.35	27.33
3.68	224.43	.613E+06	512.2	1.85	19.94
4.71	225.46	.152E+07	1066.8	1.55	23.80
5.75	226.50	.399E+07	2824.9	1.75	21.16
6.79	227.54	.663E+07	6528.0	2.18	16.93
7.83	228.58	.994E+07	12158.6	2.71	13.62
8.87	229.62	.134E+08	19268.8	3.19	11.58
9.91	230.66	.170E+08	27785.3	3.63	10.19
10.94	231.69	.207E+08	37667.1	4.03	9.18
11.98	232.73	.246E+08	48891.7	4.40	8.40
13.02	233.77	.287E+08	61448.0	4.75	7.78
14.06	234.81	.329E+08	75332.7	5.08	7.28
15.10	235.85	.372E+08	90547.8	5.39	6.85
16.14	236.89	.417E+08	*****	5.69	6.49
17.17	237.92	.464E+08	*****	5.98	6.18
18.21	238.96	.512E+08	*****	6.25	5.91
19.25	240.00	.561E+08	*****	6.52	5.67

ADD HYD (8180)
1 + 2 = 3

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0236)	494.49	4.50	8.00	23.08
OUTFLOW : ID= 1 (6238)	494.49	4.36	8.50	23.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8178)
1 + 2 = 3

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8176):	18441.72	140.272	12.00	34.58
+ ID2= 2 (8180):	806.19	6.649	8.50	22.84

ID = 3 (8178): 19247.91 144.835 12.00 34.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6240)	ROUTING TIME STEP (min) = 15.00	
----- DATA FOR SECTION (2401.0) -----		
Distance	Elevation	Manning
0.00	222.00	0.0450
11.46	221.00	0.0450
208.98	221.00	0.0450
404.04	220.97	0.0450
808.08	220.83	0.0450
905.60	220.17	0.0450
919.53	219.43	0.0450
933.47	219.22	0.0450
945.26	219.21	0.0450 / 0.0300
946.26	217.81	0.0300
975.26	217.81	0.0300
1003.26	217.81	0.0300
1005.26	219.21	0.0300 / 0.0450
1017.06	219.28	0.0450
1030.99	219.26	0.0450
1044.92	219.23	0.0450
1058.86	219.23	0.0450
1253.91	219.22	0.0450
1323.57	221.05	0.0450
1379.30	222.00	0.0450

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.20	218.01	.307E+05	2.2	0.20	227.52
0.40	218.21	.617E+05	7.1	0.31	143.95
0.50	218.41	.928E+05	14.0	0.41	110.33
0.80	218.61	.124E+06	22.6	0.49	91.46
1.00	218.81	.156E+06	32.8	0.57	79.15
1.20	219.01	.188E+06	44.5	0.64	70.38
1.40	219.21	.220E+06	57.5	0.70	63.77
1.63	219.44	.413E+06	82.5	0.54	83.52
1.87	219.68	.632E+06	120.9	0.51	87.03
2.10	219.91	.858E+06	170.0	0.53	84.11
2.33	220.14	.109E+07	228.7	0.56	79.62
2.56	220.37	.134E+07	294.4	0.59	76.03
2.80	220.61	.162E+07	371.1	0.61	72.75
3.03	220.84	.192E+07	456.8	0.64	70.18
3.26	221.07	.251E+07	552.1	0.59	75.74
3.49	221.30	.333E+07	711.8	0.57	78.07
3.73	221.54	.417E+07	907.0	0.58	76.62
3.96	221.77	.502E+07	1134.1	0.61	73.70
4.19	222.00	.587E+07	1390.9	0.64	70.34

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8178) *****	144.84	12.00	34.09	1.98	0.52
OUTFLOW : ID= 1 (6240) *****	135.69	13.25	34.09	1.93	0.52

CALIB	Area	Curve Number
NASHYD (0240)	(ha) = 434.37	(CN) = 46.0
ID= 1 DT=15.0 min	Ia (mm) = 5.00	# of Linear Res.(N) = 1.50
U.H. Tp(hrs)=	3.60	

Unit Hyd Qpeak (cms) = 2.058

PEAK FLOW (cms) = 1.357 (i)
TIME TO PEAK (hrs) = 12.000
RUNOFF VOLUME (mm) = 18.240
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.206

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8182)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1 = 1 (0240):	434.37	1.357	12.00	18.24
+ ID2 = 2 (6240):	19247.91	135.694	13.25	34.09

ID = 3 (8182):	19682.28	136.993	13.25	33.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	Curve Number
NASHYD (0242)	(ha) = 657.88	(CN) = 52.0
ID= 1 DT=15.0 min	Ia (mm) = 5.00	# of Linear Res.(N) = 1.50
U.H. Tp(hrs)=	5.37	

Unit Hyd Qpeak (cms) = 2.090

PEAK FLOW (cms) = 1.747 (i)
TIME TO PEAK (hrs) = 12.750
RUNOFF VOLUME (mm) = 21.910
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.248

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8184)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1 = 1 (0242):	657.88	1.747	12.75	21.91
+ ID2 = 2 (8182):	19682.28	136.993	13.25	33.74

ID = 3 (8184):	20340.16	138.737	13.25	33.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6244)	ROUTING TIME STEP (min) = 15.00
------------------	---------------------------------

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.00	225.00	0.0450			
2.50	220.10	0.0450			
3.00	220.06	0.0450			
3.64	220.07	0.0450			
7.28	220.08	0.0450			
10.91	220.09	0.0450			
14.55	219.81	0.0450			
18.19	219.29	0.0450			
24.13	219.21	0.0450 / 0.0300			
24.63	217.81	0.0300			
69.13	217.81	0.0300			
117.43	217.81	0.0300			
115.13	219.21	0.0300 / 0.0450			
120.06	219.25	0.0450			
123.70	219.32	0.0450			
127.34	219.77	0.0450			
130.98	219.88	0.0450			
134.61	219.99	0.0450			
350.00	220.18	0.0450			
360.18	225.00	0.0450			

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.35	218.16	.108E+06	9.0	0.29	200.52
0.70	218.51	.216E+06	28.4	0.45	126.89
1.05	218.86	.325E+06	55.7	0.59	97.27
1.40	219.21	.434E+06	89.7	0.71	80.65
1.79	219.60	.574E+06	135.9	0.81	70.36
2.17	219.98	.725E+06	190.8	0.90	63.38
2.56	220.37	.111E+07	264.3	0.82	69.82
2.94	220.75	.157E+07	371.7	0.81	70.36
3.33	221.14	.203E+07	503.4	0.85	67.30
3.72	221.53	.250E+07	656.6	0.90	63.40
4.10	221.91	.296E+07	829.6	0.96	59.54
4.49	222.30	.343E+07	1021.1	1.02	56.00
4.87	222.68	.390E+07	1230.2	1.08	52.83
5.26	223.07	.437E+07	1456.2	1.14	50.01
5.65	223.46	.484E+07	1698.4	1.20	47.51
6.03	223.84	.531E+07	1956.3	1.26	45.28
6.42	224.23	.579E+07	2229.3	1.32	43.28
6.80	224.61	.626E+07	2517.0	1.38	41.48
7.19	225.00	.674E+07	2819.2	1.44	39.85

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8184) *****	138.74	13.25	33.36	1.81	0.82
OUTFLOW : ID= 1 (6244) *****	134.63	14.25	33.36	1.77	0.81

CALIB	Area	Curve Number
NASHYD (0244)	(ha) = 1908.71	(CN) = 61.0
ID= 1 DT=15.0 min	Ia (mm) = 5.00	# of Linear Res.(N) = 1.50
U.H. Tp(hrs)=	2.20	

Unit Hyd Qpeak (cms) = 14.812

PEAK FLOW (cms) = 13.625 (1)
 TIME TO PEAK (hrs) = 9.500
 RUNOFF VOLUME (mm) = 28.265
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.319

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8186) |
| 1 2 3 |
|-----|
| AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
|-----|-----|-----|-----|
| ID1= 1 (0244): 1908.71 | 13.625 | 9.50 | 28.27 |
| + ID2= 2 (6244): 20340.16 | 134.635 | 14.25 | 33.36 |
| ===== | ===== | ===== | ===== |
| ID = 3 (8186): 22248.87 | 143.859 | 14.00 | 32.92 |
-----

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| READ HYD (8190) | AREA (ha) | ***** |
| DT=15.0 min | TPEAK (hrs) | 12.00 |
|-----|-----|-----|
| VOLUME (mm) | 32.67 |
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Filename: S:\2010 Projects\DE-EE (E10)\WR\W10-487 East Gwillimbury SWM Master Plan\Caics\Hydrologic Models\Exis Comments:

HOWEVER THE TABLE BELOW AND HYDROGRAPH FILES DO.

TIME	ACTUAL PEAK FLOW	PEAK (184.91)	+ BASE (0.00)	= 184.91 (cms)
hrs	cms	cms	cms	cms
0.00	0.000	66.00	2.413	132.00
0.25	0.000	66.25	2.396	132.25
0.50	0.000	66.50	2.379	132.50
0.75	0.000	66.75	2.363	132.75
1.00	0.066	67.00	2.346	133.00
1.25	0.075	67.25	2.330	133.25
1.50	0.360	67.50	2.313	133.50
1.75	0.470	67.75	2.297	133.75
2.00	0.560	68.00	2.281	134.00
2.25	0.670	68.25	2.265	134.25
2.50	0.769	68.50	2.250	134.50
2.75	0.866	68.75	2.234	134.75
3.00	0.966	69.00	2.218	135.00
3.25	1.066	69.25	2.203	135.25
3.50	1.151	69.50	2.188	135.50
3.75	1.576	69.75	2.173	135.75
4.00	1.826	70.00	2.158	136.00
4.25	2.282	70.25	2.143	136.25
4.50	2.803	70.50	2.128	136.50
4.75	3.600	70.75	2.113	136.75
5.00	4.525	71.00	2.099	137.00
5.25	6.018	71.25	2.084	137.25
5.50	7.811	71.50	2.070	137.50
5.75	14.446	71.75	2.055	137.75
6.00	37.810	72.00	2.041	138.00
6.25	56.299	72.25	2.027	138.25
6.50	74.311	72.50	2.013	138.50
6.75	85.240	72.75	1.999	138.75
7.00	94.756	73.00	1.985	139.00
7.25	103.047	73.25	1.972	139.25
7.50	111.399	73.50	1.958	139.50
7.75	119.900	73.75	1.945	139.75
8.00	128.605	74.00	1.931	140.00
8.25	136.903	74.25	1.918	140.25
8.50	143.804	74.50	1.905	140.50
8.75	149.056	74.75	1.892	140.75
9.00	154.312	75.00	1.879	141.00
9.25	159.495	75.25	1.866	141.25
9.50	164.480	75.50	1.853	141.50
9.75	169.146	75.75	1.840	141.75
10.00	173.362	76.00	1.828	142.00
10.25	176.789	76.25	1.815	142.25
10.50	179.616	76.50	1.802	142.50
10.75	181.896	76.75	1.790	142.75
11.00	183.693	77.00	1.778	143.00
11.25	184.204	77.25	1.765	143.25
11.50	184.573	77.50	1.753	143.50
11.75	184.804	77.75	1.741	143.75
12.00	184.909	78.00	1.729	144.00
12.25	184.558	78.25	1.717	144.25
12.50	183.963	78.50	1.706	144.50
12.75	183.100	78.75	1.694	144.75
13.00	182.083	79.00	1.682	145.00
13.25	180.918	79.25	1.670	145.25
13.50	179.616	79.50	1.659	145.50
13.75	178.189	79.75	1.648	145.75
14.00	176.647	80.00	1.636	146.00
14.25	175.002	80.25	1.625	146.25
14.50	173.265	80.50	1.614	146.50
14.75	171.444	80.75	1.603	146.75
15.00	169.546	81.00	1.592	147.00
15.25	167.581	81.25	1.581	147.25

15.50	165.495	81.50	1.570	147.50
15.75	163.234	81.75	1.559	147.75
16.00	160.828	82.00	1.548	148.00
16.25	158.280	82.25	1.538	148.25
16.50	155.599	82.50	1.527	148.50
16.75	152.796	82.75	1.517	148.75
17.00	149.747	83.00	1.506	149.00
17.25	145.823	83.25	1.496	149.25
17.50	141.953	83.50	1.485	149.50
17.75	138.154	83.75	1.475	149.75
18.00	134.431	84.00	1.465	150.00
18.25	130.790	84.25	1.455	150.25
18.50	127.236	84.50	1.446	150.50
18.75	123.771	84.75	1.435	150.75
19.00	120.394	85.00	1.425	151.00
19.25	117.086	85.25	1.415	151.25
19.50	113.835	85.50	1.405	151.50
19.75	110.649	85.75	1.396	151.75
20.00	107.542	86.00	1.386	152.00
20.25	104.527	86.25	1.377	152.25
20.50	101.607	86.50	1.367	152.50
20.75	98.786	86.75	1.358	152.75
21.00	96.063	87.00	1.348	153.00
21.25	93.433	87.25	1.339	153.25
21.50	90.892	87.50	1.330	153.50
21.75	88.437	87.75	1.320	153.75
22.00	85.337	88.00	1.311	154.00
22.25	82.030	88.25	1.302	154.25
22.50	79.122	88.50	1.293	154.50
22.75	76.521	88.75	1.284	154.75
23.00	74.162	89.00	1.275	155.00
23.25	72.000	89.25	1.266	155.25
23.50	69.980	89.50	1.258	155.50
23.75	68.092	89.75	1.249	155.75
24.00	66.307	90.00	1.240	156.00
24.25	64.527	90.25	1.232	156.25
24.50	62.596	90.50	1.223	156.50
24.75	60.496	90.75	1.215	156.75
25.00	58.210	91.00	1.206	157.00
25.25	56.096	91.25	1.198	157.25
25.50	53.892	91.50	1.190	157.50
25.75	51.725	91.75	1.181	157.75
26.00	49.594	92.00	1.172	158.00
26.25	47.567	92.25	1.165	158.25
26.50	45.594	92.50	1.157	158.50
26.75	43.699	92.75	1.149	158.75
27.00	41.883	93.00	1.141	159.00
27.25	40.150	93.25	1.133	159.25
27.50	38.497	93.50	1.125	159.50
27.75	36.925	93.75	1.117	159.75
28.00	35.422	94.00	1.110	160.00
28.25	33.992	94.25	1.102	160.25
28.50	32.629	94.50	1.094	160.50
28.75	31.330	94.75	1.087	160.75
29.00	30.092	95.00	1.079	161.00
29.25	28.911	95.25	1.072	161.25
29.50	27.785	95.50	1.064	161.50
29.75	26.712	95.75	1.057	161.75
30.00	25.689	96.00	1.049	162.00
30.25	24.714	96.25	1.042	162.25
30.50	23.783	96.50	1.035	162.50
30.75	22.896	96.75	1.028	162.75
31.00	22.050	97.00	1.020	163.00
31.25	21.243	97.25	1.013	163.25
31.50	20.473	97.50	1.006	163.50
31.75	19.738	97.75	0.999	163.75
32.00	19.037	98.00	0.992	164.00
32.25	18.368	98.25	0.985	164.25
32.50	17.729	98.50	0.978	164.50
32.75	17.119	98.75	0.971	164.75
33.00	16.537	99.00	0.965	165.00
33.25	15.980	99.25	0.958	165.25
33.50	15.449	99.50	0.951	165.50
33.75	14.942	99.75	0.945	165.75
34.00	14.457	100.00	0.938	166.00
34.25	13.995	100.25	0.931	166.25
34.50	13.552	100.50	0.925	166.50
34.75	13.130	100.75	0.918	166.75
35.00	12.727	101.00	0.912	167.00
35.25	12.341	101.25	0.906	167.25
35.50	11.972	101.50	0.899	167.50
35.75	11.620	101.75	0.893	167.75
36.00	11.284	102.00	0.887	168.00
36.25	10.962	102.25	0.881	168.25
36.50	10.655	102.50	0.874	168.50
36.75	10.361	102.75	0.868	168.75
37.00	10.080	103.00	0.862	169.00
37.25	9.812	103.25	0.856	169.25
37.50	9.555	103.50	0.850	169.50
37.75	9.309	103.75	0.844	169.75
38.00	9.074	104.00	0.838	170.00
38.25	8.849	104.25	0.832	170.25
38.50	8.634	104.50	0.826	170.50
38.75	8.428	104.75	0.821	170.75
39.00	8.230	105.00	0.815	171.00


```

-----
| ADD HYD (8192) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (4462): 1366.10 5.826 12.00 28.32
+ ID2= 2 (5446): 108.90 1.828 7.25 51.07
=====
ID = 3 (8192): 1475.00 7.003 10.50 30.00

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8194) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8188): 24157.57 153.790 13.50 32.55
+ ID2= 2 (8192): 1475.00 7.003 10.50 30.00
=====
ID = 3 (8194): 25632.58 160.032 13.50 32.41

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB (0442) |
| NASHYD (0442) | Area (ha)= 117.26 Curve Number (CN)= 43.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.17 |
-----

```

Unit Hyd Qpeak (cms) = 1.705

```

PEAK FLOW (cms) = 0.783 (1)
TIME TO PEAK (hrs) = 7.750
RUNOFF VOLUME (mm) = 16.459
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.186

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0440) |
| NASHYD (0440) | Area (ha)= 226.35 Curve Number (CN)= 60.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.11 |
-----

```

Unit Hyd Qpeak (cms) = 3.481

```

PEAK FLOW (cms) = 2.661 (1)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 27.335
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.309

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0438) |
| NASHYD (0438) | Area (ha)= 130.70 Curve Number (CN)= 51.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.60 |
-----

```

Unit Hyd Qpeak (cms) = 3.738

```

PEAK FLOW (cms) = 1.886 (1)
TIME TO PEAK (hrs) = 6.750
RUNOFF VOLUME (mm) = 20.821
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.235

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0436) |
| NASHYD (0436) | Area (ha)= 187.51 Curve Number (CN)= 55.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.73 |
-----

```

Unit Hyd Qpeak (cms) = 4.391

```

PEAK FLOW (cms) = 2.626 (1)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 23.543
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.266

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (0434) |
| NASHYD (0434) | Area (ha)= 56.64 Curve Number (CN)= 46.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.52 |
-----

```

Unit Hyd Qpeak (cms) = 1.867

```

PEAK FLOW (cms) = 0.782 (1)
TIME TO PEAK (hrs) = 6.500
RUNOFF VOLUME (mm) = 17.775
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.201

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8206) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0434): 56.64 0.782 6.50 17.78
+ ID2= 2 (0436): 187.51 2.626 7.00 23.54
=====
ID = 3 (8206): 244.15 3.390 6.75 22.21

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6438) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00
-----

```

<----- DATA FOR SECTION (4381.0) ----->

Distance	Elevation	Manning
0.00	256.00	0.0600
9.23	255.50	0.0600
27.69	255.00	0.0600
50.77	254.90	0.0600
64.62	254.85	0.0600
78.47	252.26	0.0600
96.93	249.44	0.0600
129.24	245.94	0.0600
145.40	245.76	0.0600
146.00	245.80	0.0600 / 0.0350
146.70	245.30	0.0350
147.70	245.30	0.0350
148.20	245.80	0.0350 / 0.0600
150.01	245.78	0.0600
152.32	245.79	0.0600
163.86	246.72	0.0600
175.40	249.07	0.0600
186.94	253.15	0.0600
196.17	255.38	0.0600
228.48	255.55	0.0600

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.50	245.80	223E+04	0.9	1.02	40.44
1.01	246.31	352E+05	14.0	0.99	41.77
1.53	246.83	841E+05	47.3	1.39	29.61
2.04	247.34	144E+06	102.0	1.76	23.49
2.55	247.85	213E+06	177.3	2.07	19.99
3.07	248.37	291E+06	274.4	2.34	17.66
3.58	248.88	378E+06	394.6	2.58	15.97
4.09	249.39	474E+06	541.4	2.83	14.60
4.61	249.91	578E+06	719.9	3.08	13.38
5.12	250.42	688E+06	923.8	3.33	12.40
5.63	250.93	803E+06	1153.3	3.56	11.61
6.14	251.44	925E+06	1408.6	3.77	10.95
6.66	251.96	105E+07	1690.6	3.98	10.38
7.17	252.47	119E+07	2002.3	4.18	9.88
7.68	252.98	133E+07	2345.4	4.38	9.43
8.20	253.50	147E+07	2710.1	4.56	9.05
8.71	254.01	162E+07	3101.0	4.73	8.72
9.22	254.52	178E+07	3521.8	4.90	8.43
9.74	255.04	195E+07	3467.6	4.39	9.39

<----- hydrograph -----> <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8206)	244.15	3.39	6.75	22.21	0.60	1.01
OUTFLOW: ID= 1 (6438)	244.15	2.96	7.75	22.21	0.58	1.02

```

-----
| ADD HYD (8208) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0438): 130.70 1.886 6.75 20.82

```

+ ID2= 2 (6438): 244.15 2.963 7.75 22.21
 ID = 3 (8208): 374.85 4.587 7.25 21.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8210) |
| 1 + 2 = 3 |
|-----|
| ID# 1 (0440): | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
| 226.35 | 2.661 | 7.50 | 27.34 |
+ ID2= 2 (8208): | 374.85 | 4.587 | 7.25 | 21.72 |
|-----|
| ID = 3 (8210): | 601.20 | 7.236 | 7.50 | 23.84 |
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0432) | Area (ha) = 114.21 | Curve Number (CN) = 53.0 |
| ID= 1 DT=15.0 min | Ia (mm) = 5.00 | # of Linear Res.(N) = 1.50 |
|-----|
| U.H. Tp(hrs) = 1.21 |
  
```

Unit Hyd Qpeak (cms) = 1.617

PEAK FLOW (cms) = 1.026 (1)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 22.408
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.253

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0430) | Area (ha) = 111.64 | Curve Number (CN) = 55.0 |
| ID= 1 DT=15.0 min | Ia (mm) = 5.00 | # of Linear Res.(N) = 1.50 |
|-----|
| U.H. Tp(hrs) = 0.52 |
  
```

Unit Hyd Qpeak (cms) = 3.687

PEAK FLOW (cms) = 2.049 (1)
 TIME TO PEAK (hrs) = 6.500
 RUNOFF VOLUME (mm) = 23.287
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.263

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0428) | Area (ha) = 50.53 | Curve Number (CN) = 51.0 |
| ID= 1 DT=15.0 min | Ia (mm) = 5.00 | # of Linear Res.(N) = 1.50 |
|-----|
| U.H. Tp(hrs) = 0.50 |
  
```

Unit Hyd Qpeak (cms) = 1.725

PEAK FLOW (cms) = 0.841 (1)
 TIME TO PEAK (hrs) = 6.500
 RUNOFF VOLUME (mm) = 20.682
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.234

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0426) | Area (ha) = 247.17 | Curve Number (CN) = 61.0 |
| ID= 1 DT=15.0 min | Ia (mm) = 5.00 | # of Linear Res.(N) = 1.50 |
|-----|
| U.H. Tp(hrs) = 0.98 |
  
```

Unit Hyd Qpeak (cms) = 4.315

PEAK FLOW (cms) = 3.300 (1)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 28.056
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.317

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0424) | Area (ha) = 49.57 | Curve Number (CN) = 59.0 |
| ID= 1 DT=15.0 min | Ia (mm) = 5.00 | # of Linear Res.(N) = 1.50 |
|-----|
| U.H. Tp(hrs) = 0.37 |
  
```

Unit Hyd Qpeak (cms) = 2.293

PEAK FLOW (cms) = 1.282 (1)
 TIME TO PEAK (hrs) = 6.500
 RUNOFF VOLUME (mm) = 25.630
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.290

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8198) |
| 1 + 2 = 3 |
|-----|
| ID# 1 (0424): | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
| 49.57 | 1.282 | 6.50 | 25.63 |
+ ID2= 2 (0426): | 247.17 | 3.300 | 7.25 | 28.06 |
|-----|
| ID = 3 (8198): | 296.74 | 4.328 | 6.75 | 27.65 |
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6428) |
| IN= 2--> OUT= 1 |
|-----|
| Routing time step (min) = 15.00 |
  
```

DATA FOR SECTION (4281.0) -----			
Distance	Elevation	Manning	
0.00	246.54	0.0900	
8.80	246.29	0.0900	
30.78	246.03	0.0900	
41.78	243.97	0.0900	
63.77	237.16	0.0900	
85.76	232.53	0.0900	
96.75	230.16	0.0900	
109.94	228.40	0.0900	
112.14	228.28	0.0900 / 0.0450	Main Channel
112.84	228.00	0.0450	Main Channel
113.34	227.50	0.0450	Main Channel
114.34	227.50	0.0450	Main Channel
114.84	228.00	0.0450	Main Channel
116.54	228.29	0.0450 / 0.0900	Main Channel
118.74	228.40	0.0900	
120.94	228.52	0.0900	
149.52	229.51	0.0900	
169.31	230.69	0.0900	
186.00	234.27	0.0900	
217.69	243.56	0.0900	

TRAVEL TIME TABLE -----					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.78	228.28	2.05E+04	1.6	0.97	21.60
1.58	229.08	2.02E+05	16.6	1.02	20.37
2.39	229.89	.665E+05	64.8	1.22	17.11
3.19	230.69	.133E+06	158.1	1.49	14.05
4.00	231.50	2.13E+06	308.6	1.82	11.48
4.80	232.30	2.99E+06	504.9	2.11	9.89
5.61	233.11	.394E+06	746.6	2.37	8.79
6.41	233.91	4.96E+06	1034.5	2.61	7.99
7.21	234.71	6.05E+06	1373.3	2.84	7.35
8.02	235.52	.722E+06	1763.3	3.06	6.82
8.82	236.32	.844E+06	2202.0	3.27	6.39
9.63	237.13	974E+06	2690.5	3.46	6.03
10.43	237.93	1.11E+07	3239.5	3.66	5.71
11.23	238.73	1.25E+07	3839.0	3.84	5.43
12.04	239.54	1.40E+07	4489.1	4.03	5.18
12.84	240.34	1.55E+07	5190.5	4.20	4.97
13.65	241.15	1.70E+07	5943.8	4.36	4.78
14.45	241.95	1.87E+07	6749.8	4.53	4.61
15.26	242.76	2.04E+07	7609.4	4.68	4.46

```

-----
| hydrograph ----- |
| AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL |
| (ha) (cms) (hrs) (mm) (m) (m/s) |
INFLOW : ID= 2 (8198) 296.74 4.33 6.75 27.65 0.92 0.98
OUTFLOW : ID= 1 (6428) 296.74 4.16 7.25 27.65 0.92 0.98
  
```

```

-----
| ADD HYD (8200) |
| 1 + 2 = 3 |
|-----|
| ID# 1 (0428): | AREA (ha) | QPEAK (cms) | TPEAK (hrs) | R.V. (mm) |
| 50.53 | 0.841 | 6.50 | 20.68 |
+ ID2= 2 (6428): | 296.74 | 4.156 | 7.25 | 27.65 |
|-----|
| ID = 3 (8200): | 347.27 | 4.864 | 7.25 | 26.64 |
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8202) |
| 1 + 2 = 3 |
-----
| ID1= 1 (0430): 111.64 2.049 6.50 23.39
+ ID2= 2 (8200): 347.27 4.864 7.25 26.64
-----
| ID = 3 (8202): 458.91 6.730 7.00 25.82
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6432) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)'= 15.00
-----

```

```

-----
|<----- DATA FOR SECTION (4321.0) ----->
| Distance Elevation Manning
| 0.00 221.00 0.0600
| 15.29 219.85 0.0600
| 45.86 219.86 0.0600
| 76.44 219.96 0.0600
| 110.84 220.23 0.0600
| 112.00 220.15 0.0600
| 114.66 220.09 0.0600
| 118.48 219.73 0.0600
| 120.80 219.70 0.0600 / 0.0350 Main Channel
| 121.30 219.20 0.0350 Main Channel
| 122.30 219.20 0.0350 Main Channel
| 122.80 219.70 0.0350 / 0.0600 Main Channel
| 129.95 220.11 0.0600
| 175.81 220.24 0.0600
| 214.03 220.55 0.0600
| 252.25 220.61 0.0600
| 290.47 220.70 0.0600
| 328.69 221.04 0.0600
| 347.80 221.08 0.0600
| 378.37 221.04 0.0600
-----

```

```

-----
|<----- TRAVEL TIME TABLE ----->
| DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
| (m) (m) (cu.m.) (cms) (m/s) (min)
| 0.08 219.28 .185E+03 0.0 0.23 149.38
| 0.17 219.37 .399E+03 0.1 0.34 100.62
| 0.25 219.45 .641E+03 0.1 0.42 80.98
| 0.33 219.53 .911E+03 0.2 0.49 69.79
| 0.42 219.62 .121E+04 0.3 0.55 62.35
| 0.50 219.70 .154E+04 0.5 0.60 56.91
| 0.61 219.81 .267E+04 0.7 0.56 61.28
| 0.71 219.91 .301E+04 1.3 0.30 112.96
| 0.82 220.02 .249E+05 3.4 0.28 122.71
| 0.92 220.12 .452E+05 6.8 0.31 109.98
| 1.03 220.23 .747E+05 12.2 0.33 102.05
| 1.14 220.34 .112E+06 21.3 0.39 87.59
| 1.24 220.44 .153E+06 33.0 0.44 77.01
| 1.35 220.55 .196E+06 47.2 0.49 69.22
| 1.46 220.66 .248E+06 62.3 0.51 66.48
| 1.56 220.76 .310E+06 83.4 0.55 62.06
| 1.67 220.87 .376E+06 108.8 0.59 57.56
| 1.77 220.97 .444E+06 137.7 0.64 53.77
| 1.88 221.08 .517E+06 163.6 0.65 52.70
-----

```

```

-----
|<----- hydrograph -----> <-pipe / channel->
| AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
| (ha) (cms) (hrs) (mm) (m) (m/s)
| INFLOW : ID= 2 (8202) 458.91 6.73 7.00 25.82 0.92 0.31
| OUTFLOW: ID= 1 (6432) 458.91 4.41 9.00 25.82 0.85 0.29
-----

```

```

-----
| ADD HYD (8204) |
| 1 + 2 = 3 |
-----
| ID1= 1 (0432): 114.21 1.026 7.75 22.41
+ ID2= 2 (6432): 458.91 4.414 9.00 25.82
-----
| ID = 3 (8204): 573.12 5.371 8.75 25.14
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8212) |
| 1 + 2 = 3 |
-----
| ID1= 1 (8204): 573.12 5.371 8.75 25.14
+ ID2= 2 (8210): 601.20 7.236 7.50 23.84
-----
| ID = 3 (8212): 1174.32 12.072 8.00 24.47
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6442) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)'= 15.00
-----

```

```

-----
|<----- DATA FOR SECTION (4421.0) ----->
| Distance Elevation Manning
| 0.00 221.00 0.0350
| 26.73 220.80 0.0350
| 53.47 220.60 0.0350
| 66.83 220.40 0.0350
| 80.20 220.20 0.0350
| 347.52 220.00 0.0350
| 354.21 219.59 0.0350
| 360.89 219.50 0.0350
| 367.57 219.08 0.0350
| 368.76 219.00 0.0350 / 0.0300 Main Channel
| 369.26 218.50 0.0300 Main Channel
| 374.26 218.50 0.0300 Main Channel
| 374.76 219.00 0.0300 / 0.0350 Main Channel
| 380.94 220.33 0.0350
| 387.62 219.62 0.0350
| 394.31 219.23 0.0350
| 454.45 219.07 0.0350
| 514.60 219.09 0.0350
| 600.00 219.19 0.0350
| 661.63 221.00 0.0350
-----

```

```

-----
|<----- TRAVEL TIME TABLE ----->
| DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
| (m) (m) (cu.m.) (cms) (m/s) (min)
| 0.12 218.62 .100E+04 0.2 0.28 92.89
| 0.25 218.75 .205E+04 0.6 0.43 60.09
| 0.37 218.87 .315E+04 1.1 0.55 46.99
| 0.50 219.00 .430E+04 1.8 0.66 39.67
| 0.63 219.13 .541E+04 3.4 0.87 29.99
| 0.77 219.27 .652E+04 5.0 1.13 22.54
| 0.90 219.40 .763E+04 6.8 1.37 17.54
| 1.03 219.53 .873E+04 8.6 1.61 13.54
| 1.17 219.67 .984E+04 10.3 1.85 10.30
| 1.30 219.80 .109E+05 12.0 2.09 8.10
| 1.43 219.93 .119E+05 13.7 2.33 6.60
| 1.57 220.07 .129E+05 15.4 2.57 5.40
| 1.70 220.20 .139E+05 17.1 2.81 4.50
| 1.83 220.33 .149E+05 18.8 3.05 3.80
| 1.97 220.47 .159E+05 20.5 3.29 3.20
| 2.10 220.60 .169E+05 22.2 3.53 2.70
| 2.23 220.73 .179E+05 23.9 3.77 2.30
| 2.37 220.87 .189E+05 25.6 4.01 2.00
| 2.50 221.00 .199E+05 27.3 4.25 1.70
-----

```

```

-----
|<----- hydrograph -----> <-pipe / channel->
| AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
| (ha) (cms) (hrs) (mm) (m) (m/s)
| INFLOW : ID= 2 (8212) 1174.32 12.07 8.00 24.47 0.75 0.34
| OUTFLOW: ID= 1 (6442) 1174.32 10.36 9.50 24.47 0.72 0.34
-----

```

```

-----
| ADD HYD (8214) |
| 1 + 2 = 3 |
-----
| ID1= 1 (0442): 117.26 0.783 7.75 16.46
+ ID2= 2 (6442): 1174.32 10.360 9.50 24.47
-----
| ID = 3 (8214): 1291.58 11.029 9.50 23.75
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8196) |
| 1 + 2 = 3 |
-----
| ID1= 1 (8194): 25632.58 160.032 13.50 32.41
+ ID2= 2 (8214): 1291.58 11.029 9.50 23.75
-----
| ID = 3 (8196): 26924.15 166.925 13.25 31.99
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0444) | Area (ha)= 221.65 Curve Number (CN)= 56.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
| U.H. Tp(hrs)= 1.03
-----

```

Unit Hyd Qpeak (cms)= 3.656

PEAK FLOW (cms)= 2.441 (1)

TIME TO PEAK (hrs)= 7.500

RUNOFF VOLUME (mm)= 24.392
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.276

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8216)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0444):	221.65	2.441	7.50	24.39
+ ID2= 2 (8196):	26924.15	166.925	13.25	31.99
ID = 3 (8216):	27145.80	167.692	13.25	31.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

**EXISTING
 EAST HOLLAND RIVER REGIONAL STORM (RF1000)**

```
V V I SSSSS U U A L
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
VV I SSSSS UUUU A A LLLL

OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y M M O O O
O O T T H H Y Y M M O O
OOO T T H H Y Y M M OOO Company Serial
```

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***** DETAILED OUTPUT *****

Input filename: C:\Program Files\Visual Orthymo 2.4r\VO2\voim.dat
 Output filename: C:\Users\jscott\AppData\Local\Temp\b8b87b6c-95e8-4f32-be02-029672ef5e41\Scenario.out
 Summary filename: C:\Users\jscott\AppData\Local\Temp\b8b87b6c-95e8-4f32-be02-029672ef5e41\Scenario.sum

DATE: 08/22/2012 TIME: 02:18:26

USER:

COMMENTS: _____

 ** SIMULATION NUMBER: 1 **

READ STORM	Filename: C:\Users\jscott\AppData\Local\Temp\b8b87b6c-95e8-4f32-be02-029672ef5e41\7f992cf3
Ptotal=212.00 mm	Comments: 12-Hour Hurricane Hazel

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	6.00	3.25	13.00	6.25	23.00	9.25	53.00
0.50	6.00	3.50	13.00	6.50	23.00	9.50	53.00
0.75	6.00	3.75	13.00	6.75	23.00	9.75	53.00
1.00	6.00	4.00	13.00	7.00	23.00	10.00	53.00
1.25	4.00	4.25	17.00	7.25	13.00	10.25	38.00
1.50	4.00	4.50	17.00	7.50	13.00	10.50	38.00
1.75	4.00	4.75	17.00	7.75	13.00	10.75	38.00
2.00	4.00	5.00	17.00	8.00	13.00	11.00	38.00
2.25	6.00	5.25	13.00	8.25	13.00	11.25	13.00
2.50	6.00	5.50	13.00	8.50	13.00	11.50	13.00
2.75	6.00	5.75	13.00	8.75	13.00	11.75	13.00
3.00	6.00	6.00	13.00	9.00	13.00	12.00	13.00

CALIB	Area (ha)= 91.20
STANDHYD (2021)	Total Imp(%)= 40.00 Dir. Com.(%)= 24.00
ID= 1 DT=15.0 min	

	IMPERVIOUS	PERVIOUS (1)
Surface Area (ha)=	36.48	54.72
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	779.74	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)=	53.00	65.39
over (min)	15.00	30.00
Storage Coeff. (min)=	13.90 (ii)	26.58 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.07	0.04
PEAK FLOW (cms)=	3.19	8.76
TIME TO PEAK (hrs)=	10.00	10.25
RUNOFF VOLUME (mm)=	210.00	182.67
TOTAL RAINFALL (mm)=	212.00	212.00
RUNOFF COEFFICIENT =	0.99	0.86

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 87.4 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
STANDHYD (2022) | Area (ha)= 377.30
|ID= 1 DT=15.0 min | Total Imp(%)= 57.00 Dir. Conn.(%)= 34.00
-----
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 215.06 162.24
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1585.98 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 53.00 79.83
over (min) 15.00 45.00
Storage Coeff. (min)= 21.29 (ii) 32.99 (ii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.06 0.03
-----
*TOTALS*
PEAK FLOW (cms)= 18.04 29.96 45.821 (iii)
TIME TO PEAK (hrs)= 10.00 10.50 10.00
RUNOFF VOLUME (mm)= 210.00 187.33 195.04
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.88 0.92
  
```

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 87.4 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8110) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
-----
+ ID2= 1 (2022): 91.20 11.930 10.00 189.23
+ ID2= 2 (2022): 377.30 45.821 10.00 195.04
=====
ID = 3 (8110): 468.50 57.751 10.00 193.91
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB
STANDHYD (0102) | Area (ha)= 466.00
|ID= 1 DT=15.0 min | Total Imp(%)= 23.00 Dir. Conn.(%)= 9.00
-----
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 107.18 358.82
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1762.57 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 53.00 53.09
over (min) 30.00 45.00
Storage Coeff. (min)= 22.68 (ii) 36.46 (ii)
Unit Hyd. Tpeak (min)= 30.00 45.00
Unit Hyd. peak (cms)= 0.04 0.03
-----
*TOTALS*
PEAK FLOW (cms)= 5.66 42.62 47.639 (iii)
TIME TO PEAK (hrs)= 10.00 10.50 10.50
RUNOFF VOLUME (mm)= 210.00 142.06 148.17
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.67 0.70
  
```

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.1 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHYD (0100) | Area (ha)= 693.84 Curve Number (CN)= 59.7
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
|-----| U.H. Tp(hrs)= 1.95
-----
Unit Hyd Qpeak (cms)= 6.087
PEAK FLOW (cms)= 23.121 (i)
TIME TO PEAK (hrs)= 12.000
RUNOFF VOLUME (mm)= 112.790
  
```

TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.532

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8000) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
-----
ID= 1 (0100): 693.84 23.121 12.00 112.79
+ ID2= 2 (0101): 466.00 47.639 10.50 148.17
=====
ID = 3 (8000): 1159.84 65.982 11.00 127.01
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6104) |
|IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
  
```

```

-----
DATA FOR SECTION (1041.0) -----
Distance Elevation Manning
0.00 330.16 0.0450
50.03 328.36 0.0450
89.34 326.33 0.0450
117.94 324.30 0.0450
160.82 322.75 0.0450
178.34 319.96 0.0450
183.34 319.81 0.0450
184.34 319.38 0.0450 / 0.0300 Main Channel
185.09 318.78 0.0300 Main Channel
185.84 318.72 0.0300 Main Channel
186.84 319.32 0.0300 / 0.0450 Main Channel
193.84 319.70 0.0450
198.84 320.38 0.0450
200.13 320.81 0.0450
218.00 322.49 0.0450
239.44 323.49 0.0450
250.17 323.96 0.0450
275.18 325.20 0.0450
310.92 326.47 0.0450
353.81 330.00 0.0450
  
```

```

-----
TRAVEL TIME TABLE -----
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.30 319.02 624E+03 0.1 0.45 73.00
0.60 319.32 181E+04 0.6 0.67 49.46
1.23 319.95 128E+05 4.9 0.77 43.38
1.86 320.58 396E+05 19.1 0.96 34.50
2.48 321.20 757E+05 43.5 1.15 28.98
3.11 321.83 125E+06 81.2 1.30 25.58
3.74 322.46 187E+06 135.9 1.45 22.92
4.37 323.09 269E+06 195.6 1.45 22.90
5.00 323.72 387E+06 287.8 1.48 22.44
5.63 324.35 545E+06 428.4 1.56 21.22
6.25 324.97 735E+06 630.5 1.71 19.42
6.88 325.60 952E+06 876.1 1.83 18.12
7.51 326.23 120E+07 1177.0 1.95 17.03
8.14 326.86 149E+07 1554.4 2.08 15.93
8.77 327.49 179E+07 2009.7 2.23 14.88
9.40 328.12 213E+07 2533.9 2.37 13.99
10.02 328.74 249E+07 3112.6 2.49 13.31
10.65 329.37 288E+07 3760.9 2.60 12.75
11.28 330.00 330E+07 4494.8 2.71 12.23
  
```

```

-----
hydrograph ----> <- pipe / channel ->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8000) 1159.84 65.98 11.00 127.01 2.86 1.23
OUTFLOW : ID= 1 (6104) 1159.84 63.65 11.25 127.01 2.81 1.22
  
```

```

CALIB
NASHYD (0104) | Area (ha)= 527.81 Curve Number (CN)= 62.6
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
|-----| U.H. Tp(hrs)= 1.34
  
```

Unit Hyd Qpeak (cms)= 6.735

```

PEAK FLOW (cms)= 23.631 (i)
TIME TO PEAK (hrs)= 11.750
RUNOFF VOLUME (mm)= 118.649
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.560
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.


```

-----
| ADD HYD (8002) |
| 1 + 2 = 3 |
-----
| ID# 1 (0104): 527.81 23.631 11.75 118.65
+ ID# 2 (6104): 1159.84 63.646 11.25 127.01
=====
| ID = 3 (8002): 1687.65 86.922 11.25 124.39
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0106) | Area (ha)= 259.52 Curve Number (CN)= 74.2
| ID# 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.23
-----

```

Unit Hyd Qpeak (cms)= 3.614

```

PEAK FLOW (cms)= 14.485 (1)
TIME TO PEAK (hrs)= 11.500
RUNOFF VOLUME (mm)= 144.001
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.679

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8004) |
| 1 + 2 = 3 |
-----
| ID# 1 (0106): 259.52 14.485 11.50 144.00
+ ID# 2 (8002): 1687.65 86.922 11.25 124.39
=====
| ID = 3 (8004): 1947.17 101.361 11.25 127.01
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6108) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
-----

```

```

<----- DATA FOR SECTION (1081.0) ----->
Distance Elevation Manning
0.00 326.06 0.0400
12.06 325.74 0.0400
30.15 324.28 0.0400
59.27 320.35 0.0400
72.37 317.60 0.0400
87.45 314.52 0.0400
93.48 313.91 0.0400
105.54 313.55 0.0400
106.17 313.40 0.0400 /0.0300 Main Channel
110.62 313.04 0.0300 Main Channel
110.82 312.97 0.0300 Main Channel
111.57 313.08 0.0300 Main Channel
111.67 313.48 0.0300 /0.0400 Main Channel
129.66 316.62 0.0400
150.77 318.95 0.0400
180.92 319.61 0.0400
205.04 321.23 0.0400
232.18 322.09 0.0400
268.37 322.31 0.0400
298.52 326.00 0.0400

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.43 313.40 .244E+04 0.7 0.59 58.83
1.09 314.06 .214E+05 10.4 1.01 34.24
1.76 314.73 .600E+05 40.2 1.39 24.89
2.42 315.39 .109E+06 92.1 1.75 19.80
3.08 316.05 .169E+06 166.5 2.05 16.88
3.75 316.72 .237E+06 264.4 2.31 14.97
4.41 317.38 .318E+06 386.3 2.52 13.72
5.07 318.04 .411E+06 539.3 2.72 12.71
5.74 318.71 .517E+06 725.5 2.91 11.88
6.40 319.37 .642E+06 909.2 2.94 11.77
7.06 320.03 .806E+06 1179.3 3.04 11.38
7.72 320.69 .989E+06 1515.9 3.18 10.87
8.39 321.36 .119E+07 1895.5 3.30 10.49
9.05 322.02 .143E+07 2310.2 3.36 10.30
9.71 322.68 .172E+07 2763.2 3.33 10.39
10.38 323.35 .205E+07 3477.6 3.53 9.81
11.04 324.01 .238E+07 4284.3 3.73 9.27
11.70 324.67 .274E+07 5154.0 3.91 8.85
12.37 325.34 .311E+07 6104.0 4.08 8.48

```

```

<----- hydrograph -----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)

```

```

INFLOW : ID= 2 (8004) 1947.17 101.36 11.25 127.01 2.50 1.78
OUTFLOW : ID= 1 (6108) 1947.17 99.83 11.50 127.01 2.49 1.78

```

```

-----
| CALIB |
| NASHYD (0108) | Area (ha)= 207.05 Curve Number (CN)= 45.4
| ID# 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.03
-----

```

Unit Hyd Qpeak (cms)= 3.432

```

PEAK FLOW (cms)= 7.761 (1)
TIME TO PEAK (hrs)= 11.500
RUNOFF VOLUME (mm)= 82.796
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.391

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8006) |
| 1 + 2 = 3 |
-----
| ID# 1 (0108): 207.05 7.761 11.50 82.80
+ ID# 2 (6108): 1947.17 99.827 11.50 127.01
=====
| ID = 3 (8006): 2154.22 107.588 11.50 122.76
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| NASHYD (0110) | Area (ha)= 323.92 Curve Number (CN)= 70.5
| ID# 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.35
-----

```

Unit Hyd Qpeak (cms)= 4.087

```

PEAK FLOW (cms)= 16.234 (1)
TIME TO PEAK (hrs)= 11.750
RUNOFF VOLUME (mm)= 135.883
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.641

```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8008) |
| 1 + 2 = 3 |
-----
| ID# 1 (0110): 323.92 16.234 11.75 135.88
+ ID# 2 (8006): 2154.22 107.588 11.50 122.76
=====
| ID = 3 (8008): 2478.14 123.796 11.50 124.47
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6112) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
-----

```

```

<----- DATA FOR SECTION (1121.0) ----->
Distance Elevation Manning
0.00 320.59 0.0400
13.48 317.93 0.0400
40.45 311.52 0.0400
64.04 307.68 0.0400
74.15 307.01 0.0400
91.00 307.22 0.0400
107.86 305.21 0.0400
111.23 304.86 0.0400
113.32 304.60 0.0400 /0.0300 Main Channel
117.77 304.24 0.0300 Main Channel
117.97 304.17 0.0300 Main Channel
118.72 304.28 0.0300 Main Channel
118.82 304.68 0.0300 /0.0400 Main Channel
124.71 304.76 0.0400
171.90 306.30 0.0400
205.60 307.24 0.0400
239.31 308.24 0.0400
262.90 310.64 0.0400
289.86 314.34 0.0400
333.68 317.66 0.0400

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)

```

0.43	304.60	.372E+04	0.6	0.54	97.65
1.12	305.29	.463E+05	12.5	0.85	61.89
1.80	305.97	.149E+06	53.0	1.13	46.85
2.49	306.66	.311E+06	134.2	1.37	38.62
3.18	307.35	.554E+06	261.6	1.50	35.27
3.87	308.04	.894E+06	490.7	1.74	30.37
4.55	308.72	.129E+07	833.0	2.05	25.75
5.24	309.41	.171E+07	1269.8	2.36	22.39
5.93	310.10	.215E+07	1789.0	2.64	20.01
6.62	310.79	.261E+07	2392.7	2.90	18.21
7.30	311.47	.310E+07	3087.8	3.16	16.74
7.99	312.16	.361E+07	3873.6	3.40	15.52
8.68	312.85	.413E+07	4741.0	3.64	14.52
9.37	313.54	.467E+07	5689.3	3.86	13.69
10.05	314.22	.523E+07	6718.4	4.07	12.97
10.74	314.91	.581E+07	7770.7	4.24	12.46
11.43	315.60	.641E+07	8897.5	4.40	12.01
12.12	316.29	.704E+07	10113.2	4.55	11.61
12.80	316.97	.770E+07	11420.1	4.70	11.24

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8008) 2478.14 122.80 11:50 124.47 2.40 1.33
OUTFLOW: ID= 1 (6112) 2478.14 116.99 12:00 124.47 2.35 1.31

```

```

CALIB (0112) | Area (ha)= 414.89 Curve Number (CN)= 49.9
NASHYD (0114) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.91

```

Unit Hyd Qpeak (cms)= 3.699

PEAK FLOW (cms)= 11.571 (i)
TIME TO PEAK (hrs)= 12.000
RUNOFF VOLUME (mm)= 92.383
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.436

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8010) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0112): 414.89 11.571 12.00 92.38
+ ID2= 2 (6112): 2478.14 116.991 12.00 124.47
=====
ID = 3 (8010): 2893.03 128.562 12.00 119.87

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6114) |
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

```

```

<----- DATA FOR SECTION (1141.0) ----->
Distance Elevation Manning
0.00 312.27 0.0500
14.07 309.70 0.0500
28.14 306.51 0.0500
35.17 305.81 0.0500
42.20 304.95 0.0500
52.75 304.24 0.0500
77.37 303.61 0.0500
98.47 301.33 0.0500
131.76 300.40 0.0500 /0.0300 Main Channel
136.21 300.04 0.0300 Main Channel
136.41 299.97 0.0300 Main Channel
137.16 300.08 0.0300 Main Channel
137.96 300.48 0.0300 /0.0500 Main Channel
140.68 300.53 0.0500
179.36 302.20 0.0500
214.53 303.82 0.0500
242.66 305.27 0.0500
288.38 307.66 0.0500
316.52 311.76 0.0500
348.17 313.48 0.0500

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m) (cms) (m/s) (min)
0.43 300.40 .366E+04 0.7 0.59 88.31
1.05 301.02 .505E+05 12.1 0.75 69.49
1.68 301.65 .165E+06 52.7 0.99 52.28
2.30 302.27 .394E+06 130.3 1.26 41.40
2.93 302.90 .520E+06 247.1 1.48 35.08
3.55 303.52 .754E+06 406.8 1.68 30.91
4.18 304.15 .104E+07 586.1 1.76 29.55

```

4.80	304.77	.138E+07	856.0	1.93	26.96
5.43	305.40	.177E+07	1201.1	2.12	24.56
6.05	306.02	.219E+07	1612.2	2.30	22.63
6.68	306.65	.264E+07	2090.6	2.47	21.07
7.30	307.27	.313E+07	2653.0	2.64	19.65
7.93	307.90	.364E+07	3303.9	2.83	18.36
8.55	308.52	.417E+07	4053.9	3.03	17.14
9.18	309.15	.471E+07	4871.9	3.22	16.12
9.80	309.77	.527E+07	5756.2	3.41	15.25
10.43	310.40	.584E+07	6699.8	3.58	14.52
11.05	311.02	.642E+07	7710.5	3.74	13.89
11.68	311.65	.702E+07	8788.5	3.90	13.32

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8010) 2893.03 122.86 12:00 119.87 2.29 1.25
OUTFLOW: ID= 1 (6114) 2893.03 122.65 12:50 119.87 2.24 1.22

```

```

CALIB (0114) | Area (ha)= 287.65 Curve Number (CN)= 69.3
NASHYD (0114) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.12

```

Unit Hyd Qpeak (cms)= 4.389

PEAK FLOW (cms)= 15.808 (i)
TIME TO PEAK (hrs)= 11.250
RUNOFF VOLUME (mm)= 132.945
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.627

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8012) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0114): 287.65 15.808 11.25 132.95
+ ID2= 2 (6114): 2893.03 122.654 12.50 119.87
=====
ID = 3 (8012): 3180.68 136.686 12.50 121.05

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB (0116) | Area (ha)= 206.43 Curve Number (CN)= 61.3
NASHYD (0116) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.80

```

Unit Hyd Qpeak (cms)= 4.394

PEAK FLOW (cms)= 15.661 (i)
TIME TO PEAK (hrs)= 11.000
RUNOFF VOLUME (mm)= 159.152
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.751

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8014) |
1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0116): 206.43 15.661 11.00 159.15
+ ID2= 2 (8012): 3180.68 136.686 12.50 121.05
=====
ID = 3 (8014): 3387.11 146.961 12.25 123.37

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6118) |
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

```

```

<----- DATA FOR SECTION (1181.0) ----->
Distance Elevation Manning
0.00 307.18 0.0450
24.89 299.75 0.0450
37.34 296.39 0.0450
71.57 292.75 0.0450
96.46 291.12 0.0450
108.90 289.43 0.0450
112.02 288.86 0.0450
115.13 288.46 0.0450

```

117.24	288.32	0.0450	/0.0350	Main Channel
117.49	288.15	0.0350		Main Channel
118.24	288.00	0.0350		Main Channel
118.99	288.20	0.0350		Main Channel
119.24	288.36	0.0350	/0.0450	Main Channel
121.35	288.40	0.0450		
149.35	289.36	0.0450		
171.14	290.47	0.0450		
208.47	293.14	0.0450		
236.48	295.17	0.0450		
280.04	299.48	0.0450		
308.04	307.00	0.0450		

243.50	260.93	0.0500
300.03	264.43	0.0500
339.16	268.34	0.0500
373.95	272.36	0.0500
430.48	277.46	0.0500

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.32	288.32	118E+04	0.2	0.62	81.92
1.30	289.30	631E+05	22.2	1.07	47.43
2.29	290.29	218E+06	117.5	1.64	30.95
3.27	291.27	448E+06	314.6	2.15	23.72
4.25	292.25	756E+06	625.6	2.53	20.13
5.24	293.24	115E+07	1093.2	2.91	17.50
6.22	294.22	161E+07	1731.6	3.28	15.50
7.20	295.20	215E+07	2547.4	3.63	14.01
8.19	296.19	274E+07	3584.9	4.00	12.72
9.17	297.17	338E+07	4859.1	4.39	11.60
10.15	298.15	407E+07	6341.5	4.76	10.70
11.13	299.13	480E+07	8024.6	5.11	9.97
12.12	300.12	556E+07	10006.6	5.49	9.27
13.10	301.10	635E+07	12244.6	5.89	8.64
14.08	302.08	716E+07	14681.7	6.26	8.13
15.07	303.07	799E+07	17316.5	6.62	7.69
16.05	304.05	884E+07	20148.1	6.96	7.31
17.03	305.03	971E+07	23176.2	7.29	6.98
18.02	306.02	106E+08	26400.8	7.61	6.69

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.50	259.80	626E+04	2.5	1.23	41.35
1.41	260.71	462E+05	21.8	1.44	35.27
2.31	261.61	149E+06	79.6	1.63	31.19
3.22	262.52	340E+06	200.1	1.80	28.34
4.12	263.42	636E+06	446.8	2.15	23.71
5.03	264.33	995E+06	819.9	2.52	20.23
5.93	265.23	141E+07	1337.0	2.90	17.56
6.84	266.14	187E+07	1984.8	3.25	15.67
7.74	267.04	237E+07	2766.0	3.57	14.26
8.65	267.95	292E+07	3596.9	3.76	13.53
9.55	268.85	355E+07	4575.4	3.93	12.95
10.46	269.76	427E+07	5814.9	4.16	12.23
11.36	270.66	503E+07	7319.0	4.44	11.46
12.27	271.57	584E+07	9009.7	4.71	10.81
13.17	272.47	670E+07	10863.3	4.96	10.26
14.08	273.38	761E+07	12893.2	5.17	9.84
14.98	274.28	858E+07	15098.1	5.37	9.48
15.89	275.19	961E+07	17527.6	5.57	9.14
16.79	276.09	107E+08	20190.3	5.76	8.83

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (8016)	3727.64	160.10	12.50	123.31	2.92	1.74
OUTFLOW : ID= 1 (6122)	3727.64	156.58	13.00	123.31	2.89	1.73

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (8014)	3387.11	148.96	12.25	123.37	2.44	1.71
OUTFLOW : ID= 1 (6118)	3387.11	145.42	12.75	123.37	2.42	1.70

STANDHYD (1221)	Area (ha)= 17.70
ID= 1 DT=15.0 min	Total Imp(%)= 36.00
	Div. Conn.(%)= 19.00

IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)= 6.37	11.33	
Dep. Storage (mm)= 2.00	5.00	
Average Slope (%)= 0.50	0.50	
Length (m)= 343.51	40.00	
Mannings n = 0.013	0.250	
Max.Eff.Inten.(mm/hr)= 53.00	48.94	
over (min)= 15.00	30.00	
Storage Coeff. (min)= 8.50 (ii)	22.74 (ii)	
Unit Hyd. Tpeak (min)= 15.00	30.00	
Unit Hyd. peak (cms)= 0.09	0.04	
	TOTALS	
PEAK FLOW (cms)= 0.49	1.36	1.838 (iii)
TIME TO PEAK (hrs)= 10.00	10.25	10.00
RUNOFF VOLUME (mm)= 210.00	116.69	134.42
TOTAL RAINFALL (mm)= 212.00	212.00	212.00
RUNOFF COEFFICIENT = 0.99	0.55	0.63

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 55.2 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0118)	Area (ha)= 340.53	Curve Number (CN)= 64.6
NASHVD (0118)	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min	U.H. Tp(hrs)= 1.12	
Unit Hyd Qpeak (cms)= 5.200		
PEAK FLOW (cms)= 17.459 (i)		
TIME TO PEAK (hrs)= 11.500		
RUNOFF VOLUME (mm)= 122.703		
TOTAL RAINFALL (mm)= 212.000		
RUNOFF COEFFICIENT = 0.579		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8016)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0118):	340.53	17.459	11.50	122.70
+ ID2= 2 (6118):	3387.11	145.423	12.75	123.37
ID= 3 (8016):	3727.64	160.104	12.50	123.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6122)	Routing time step (min)'= 15.00
ID= 2--> OUF= 1	

Distance	Elevation	Manning
0.00	277.00	0.0500
52.18	272.82	0.0500
86.97	269.38	0.0500
134.80	267.35	0.0500
160.89	263.85	0.0500
169.58	262.88	0.0500
178.28	262.45	0.0500
195.67	262.07	0.0500
213.07	260.99	0.0500
226.01	259.80	0.0500 /0.0300 Main Channel
226.11	259.30	0.0300 Main Channel
230.11	259.30	0.0300 Main Channel
230.21	259.80	0.0300 /0.0500 Main Channel
235.01	260.20	0.0500
239.15	260.57	0.0500

CALIB (1222)	Area (ha)= 347.20	Curve Number (CN)= 60.6
NASHVD (1222)	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
ID= 1 DT=15.0 min	U.H. Tp(hrs)= 1.52	
Unit Hyd Qpeak (cms)= 3.887		

PEAK FLOW (cms)= 13.912 (i)	
TIME TO PEAK (hrs)= 12.000	
RUNOFF VOLUME (mm)= 114.514	
TOTAL RAINFALL (mm)= 212.000	
RUNOFF COEFFICIENT 0.540	

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8018)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1221):	17.70	1.838	10.00	134.42

+ ID2= 2 (1222): 347.20 13.912 12.00 114.51
 ID = 3 (8018): 364.90 14.643 11.25 115.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8020)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (6122):	3727.64	156.583	13.00	123.31
+ ID2= 2 (8018):	364.90	14.643	11.25	115.48
ID = 3 (8020):	4092.54	169.052	13.00	122.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1261)	Area (ha)	Imp(%)	Dir. Conn.(%)
ID= 1 DT=15.0 min	50.00	36.00	26.00
IMPERVIOUS PERVIOUS (i)			
Surface Area (ha)	18.00	32.00	
Dep. Storage (mm)	2.00	5.00	
Average Slope (%)	0.50	0.50	
Length (m)	577.35	40.00	
Manning's n	0.013	0.250	
Max. Eff. Inten. (mm/hr)	53.00	51.13	
cover (min)	15.00	30.00	
Storage Coeff. (min)	11.61 (ii)	25.60 (ii)	
Unit Hyd. Tpeak (min)	15.00	30.00	
Unit Hyd. peak (cms)	0.08	0.04	
**TOTALS*			
PEAK FLOW (cms)	1.91	3.97	5.825 (iii)
TIME TO PEAK (hrs)	10.00	10.25	10.00
RUNOFF VOLUME (mm)	210.00	138.87	157.36
TOTAL RAINFALL (mm)	212.00	212.00	212.00
RUNOFF COEFFICIENT	0.99	0.66	0.74

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 68.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (1262)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	83.50	75.7
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50		
U.H. Tp(hrs) = 0.56		
Unit Hyd Qpeak (cms)	2.523	
PEAK FLOW (cms)	6.883 (i)	
TIME TO PEAK (hrs)	11.000	
RUNOFF VOLUME (mm)	144.982	
TOTAL RAINFALL (mm)	212.000	
RUNOFF COEFFICIENT	0.684	

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8024)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (1261):	50.00	5.825	10.00	157.36
+ ID2= 2 (1262):	83.50	6.883	11.00	144.98
ID = 3 (8024):	133.50	11.741	11.00	149.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0124)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	121.43	63.4
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50		
U.H. Tp(hrs) = 0.51		
Unit Hyd Qpeak (cms)	4.049	
PEAK FLOW (cms)	8.855 (i)	
TIME TO PEAK (hrs)	11.000	
RUNOFF VOLUME (mm)	117.847	
TOTAL RAINFALL (mm)	212.000	

RUNOFF COEFFICIENT = 0.556

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8026)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0124):	121.43	8.855	11.00	117.85
+ ID2= 2 (8024):	133.50	11.741	11.00	149.62
ID = 3 (8026):	254.93	20.597	11.00	134.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6128)	Routing time step (min)
IN= 2--> OUP= 1	15.00

Distance	Elevation	Manning
0.00	270.58	0.0400
7.71	270.36	0.0400
15.43	269.48	0.0400
23.14	265.88	0.0400
30.85	261.83	0.0400
38.56	258.31	0.0400
46.27	256.62	0.0400
53.98	255.35	0.0400
61.69	255.00	0.0400 / 0.0300
69.40	254.00	0.0300
77.11	253.90	0.0300
84.82	253.80	0.0300
92.53	253.70	0.0300
100.24	253.80	0.0300
107.95	255.00	0.0300 / 0.0400
115.66	255.44	0.0400
123.37	257.45	0.0400
131.08	259.67	0.0400
138.79	263.42	0.0400
146.50	270.00	0.0400

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.65	254.35	.439E+04	2.0	1.24	36.10
1.30	255.00	.131E+05	9.4	1.92	23.39
2.18	255.88	.433E+05	37.0	2.30	19.54
3.06	256.76	.131E+06	113.3	2.34	19.20
3.95	257.65	.273E+06	273.8	2.70	16.64
4.83	258.53	.453E+06	538.5	3.21	14.01
5.71	259.41	.658E+06	902.9	3.70	12.14
6.59	260.29	.886E+06	1368.4	4.16	10.79
7.48	261.18	1.148E+07	1936.3	4.59	9.79
8.36	262.06	1.411E+07	2610.9	4.99	9.01
9.24	262.94	1.71E+07	3398.7	5.37	8.36
10.12	263.82	2.02E+07	4309.6	5.75	7.81
11.01	264.71	2.35E+07	5347.5	6.13	7.33
11.89	265.59	2.70E+07	6501.0	6.48	6.93
12.77	266.47	3.07E+07	7808.6	6.86	6.55
13.65	267.35	3.45E+07	9250.5	7.23	6.21
14.54	268.24	3.84E+07	10808.3	7.59	5.92
15.42	269.12	4.24E+07	12482.5	7.94	5.66
16.30	270.00	4.66E+07	14127.4	8.18	5.49

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8026)	254.93	20.60	11.00	134.48	1.66
OUTFLOW : ID= 1 (6128)	254.93	19.87	11.00	134.48	1.64

CALIB NASHYD (0128)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	240.24	78.8
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50		
U.H. Tp(hrs) = 1.00		

Unit Hyd Qpeak (cms)	4.118
PEAK FLOW (cms)	15.900 (i)
TIME TO PEAK (hrs)	11.250
RUNOFF VOLUME (mm)	154.028
TOTAL RAINFALL (mm)	212.000
RUNOFF COEFFICIENT	0.727

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8028)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0128):	240.24	15.900	11.25	154.03
+ ID2= 2 (6128):	254.93	19.873	11.00	134.48
ID = 3 (8028):	495.17	35.722	11.00	143.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8022)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8020):	4092.54	169.052	13.00	122.62
+ ID2= 2 (8028):	495.17	35.722	11.00	143.97
ID = 3 (8022):	4587.71	190.592	12.75	124.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDBYD (1201)	Area	(ha)	= 86.00	Dir. Conn.(%)	= 14.00
ID= 1 DT=15.0 min	Total Imp(%)	= 36.00			

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	30.96	55.04
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	757.19	40.00
Mannings n	= 0.013	0.250
Max.Eff.Inten.(mm/hr)	53.00	61.34
over (min)	15.00	30.00
Storage Coeff. (min)	13.66 (ii)	26.67 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04
PEAK FLOW (cms)	1.76	8.17
TIME TO PEAK (hrs)	10.00	10.25
RUNOFF VOLUME (mm)	210.00	145.75
TOTAL RAINFALL (mm)	212.00	212.00
RUNOFF COEFFICIENT	= 0.99	0.69
		0.73

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (1202)	Area	(ha)	= 161.00	Curve Number (CN)= 75.6
ID= 1 DT=15.0 min	Ia	(mm)	= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)	= 0.70		

Unit Hyd Qpeak (cms) = 3.932

PEAK FLOW (cms)	= 12.223 (i)
TIME TO PEAK (hrs)	= 11.000
RUNOFF VOLUME (mm)	= 145.706
TOTAL RAINFALL (mm)	= 212.000
RUNOFF COEFFICIENT	= 0.687

- (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8030)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1201):	86.00	9.802	10.00	154.74
+ ID2= 2 (1202):	161.00	12.223	11.00	145.71
ID = 3 (8030):	247.00	20.664	11.00	148.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8032)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8022):	4587.71	190.592	12.75	124.92

+ ID2= 2 (8030):	247.00	20.664	11.00	148.85
ID = 3 (8032):	4834.71	200.104	12.50	126.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6130)	ROUTING TIME STEP (min)	= 15.00
IN= 2---> OUT= 1		

DATA FOR SECTION (1301.0) ----->				
Distance	Elevation	Manning		
0.00	257.18	0.0350		
62.45	254.70	0.0350		
109.29	252.74	0.0350		
150.92	251.55	0.0350		
192.56	248.96	0.0350		
239.39	247.16	0.0350		
291.84	246.00	0.0350		
296.84	245.00	0.0350		
298.84	245.00	0.0350 /0.0300	Main Channel	
299.24	244.60	0.0300	Main Channel	
301.84	244.60	0.0300	Main Channel	
303.44	244.60	0.0300	Main Channel	
303.84	245.00	0.0300 /0.0350	Main Channel	
306.84	245.00	0.0350		
309.84	246.00	0.0350		
317.46	247.22	0.0350		
348.68	254.83	0.0350		
359.09	256.70	0.0350		
452.76	258.23	0.0350		
515.22	258.33	0.0350		

TRAVEL TIME TABLE ----->						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.40	245.00	.870E+04	1.2	0.64	124.12	
1.04	245.64	4.68E+05	9.5	0.96	81.81	
1.68	246.28	1.09E+06	25.6	1.11	70.86	
2.32	246.92	2.57E+06	63.5	1.17	67.51	
2.96	247.56	4.98E+06	142.7	1.36	58.18	
3.61	248.21	8.00E+06	265.7	1.58	50.01	
4.25	248.85	1.16E+07	437.4	1.78	44.24	
4.89	249.49	1.57E+07	669.1	2.01	39.20	
5.53	250.13	2.03E+07	954.9	2.23	35.36	
6.17	250.77	2.52E+07	1294.4	2.43	32.42	
6.81	251.41	3.05E+07	1689.6	2.62	30.07	
7.45	252.05	3.63E+07	2090.7	2.73	28.93	
8.09	252.69	4.20E+07	2553.1	2.82	27.98	
8.73	253.33	4.81E+07	3138.4	2.96	26.60	
9.37	253.97	5.49E+07	3803.8	3.11	25.36	
10.02	254.62	6.22E+07	4549.3	3.25	24.25	
10.66	255.26	7.01E+07	5364.9	3.38	23.32	
11.30	255.90	7.84E+07	6265.7	3.51	22.49	
11.94	256.54	8.74E+07	7260.1	3.63	21.72	

<---- hydrograph ----> <- pipe / channel ->						
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW: ID= 2 (8032)	4834.71	200.10	12.50	126.14	3.26	1.45
OUTFLOW: ID= 1 (6130)	4834.71	189.83	13.25	126.14	3.21	1.43

CALIB STANDBYD (1301)	Area	(ha)	= 228.30	Dir. Conn.(%)	= 47.00
ID= 1 DT=15.0 min	Total Imp(%)	= 65.00			

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	148.39	79.91
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1233.69	40.00
Mannings n	= 0.013	0.250
Max.Eff.Inten.(mm/hr)	53.00	78.80
over (min)	15.00	45.00
Storage Coeff. (min)	18.31 (ii)	30.08 (ii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.06	0.03
PEAK FLOW (cms)	15.35	14.84
TIME TO PEAK (hrs)	10.00	10.25
RUNOFF VOLUME (mm)	210.00	187.56
TOTAL RAINFALL (mm)	212.00	212.00
RUNOFF COEFFICIENT	= 0.99	0.88
		0.93

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CM* = 87.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHVD (1302) | Area (ha)= 157.80 Curve Number (CN)= 68.6
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 1.82

```

Unit Hyd Qpeak (cms) = 1.479

```

PEAK FLOW (cms)= 8.249 (i)
TIME TO PEAK (hrs)= 12.000
RUNOFF VOLUME (mm)= 178.508
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.842

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8036) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----| (ha) (cms) (hrs) (mm)
ID1= 1 (1301): 228.30 29.249 10.00 198.10
+ ID2= 2 (1302): 157.80 8.249 12.00 178.51
=====
ID = 3 (8036): 386.10 35.168 10.00 190.10

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8034) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----| (ha) (cms) (hrs) (mm)
ID1= 1 (6130): 4834.71 189.834 13.25 126.14
+ ID2= 2 (8036): 386.10 35.168 10.00 190.10
=====
ID = 3 (8034): 5220.81 198.568 13.00 130.87

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| STANDHYD (0188) | Area (ha)= 308.53
| ID= 1 DT=15.0 min | Total Imp(%)= 41.00 Dir. Conn.(%)= 25.00
|-----|

```

```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 126.50 182.03
Dep. Storage (mm)= 3.00 5.00
Average Slope (%)= 0.10 0.10
Length (m)= 1434.18 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 53.00 61.65
over (min) 30.00 60.00
Storage Coeff. (min)= 32.48 (ii) 53.52 (ii)
Unit Hyd. Tpeak (min)= 30.00 60.00
Unit Hyd. peak (cms)= 0.04 0.02

```

TOTALS

```

PEAK FLOW (cms)= 9.72 23.10 31.864 (iii)
TIME TO PEAK (hrs)= 10.25 10.75 10.75
RUNOFF VOLUME (mm)= 209.00 162.09 173.82
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.76 0.82

```

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 77.2 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANDHYD (0186) | Area (ha)= 351.79
| ID= 1 DT=15.0 min | Total Imp(%)= 42.00 Dir. Conn.(%)= 22.00
|-----|

```

```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 147.75 204.04
Dep. Storage (mm)= 3.00 5.00
Average Slope (%)= 0.10 0.10
Length (m)= 1531.43 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 53.00 62.85
over (min) 30.00 60.00
Storage Coeff. (min)= 33.78 (ii) 54.66 (ii)
Unit Hyd. Tpeak (min)= 30.00 60.00
Unit Hyd. peak (cms)= 0.03 0.02

```

TOTALS

```

PEAK FLOW (cms)= 9.68 26.21 34.971 (iii)
TIME TO PEAK (hrs)= 10.25 11.00 10.75
RUNOFF VOLUME (mm)= 209.00 152.98 165.31
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.72 0.78

```

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 71.5 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8068) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----| (ha) (cms) (hrs) (mm)
ID1= 1 (0186): 351.79 34.971 10.75 165.31
+ ID2= 2 (0188): 308.53 31.864 10.75 173.82
=====
ID = 3 (8068): 660.32 66.835 10.75 169.29

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| STANDHYD (1821) | Area (ha)= 111.80
| ID= 1 DT=15.0 min | Total Imp(%)= 40.00 Dir. Conn.(%)= 17.00
|-----|

```

```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 44.72 67.08
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 863.33 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 53.00 65.38
over (min) 15.00 30.00
Storage Coeff. (min)= 14.78 (ii) 27.46 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.07 0.04

```

TOTALS

```

PEAK FLOW (cms)= 2.76 10.59 13.197 (iii)
TIME TO PEAK (hrs)= 10.00 10.25 10.00
RUNOFF VOLUME (mm)= 210.00 153.83 163.38
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.73 0.77

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 71.3 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHVD (1822) | Area (ha)= 44.40 Curve Number (CN)= 75.2
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----|
| U.H. Tp(hrs)= 0.79

```

Unit Hyd Qpeak (cms) = 0.966

```

PEAK FLOW (cms)= 3.182 (i)
TIME TO PEAK (hrs)= 11.000
RUNOFF VOLUME (mm)= 145.213
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.685

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8062) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
|-----| (ha) (cms) (hrs) (mm)
ID1= 1 (1821): 111.80 13.197 10.00 163.38
+ ID2= 2 (1822): 44.40 3.182 11.00 145.21
=====
ID = 3 (8062): 156.20 15.644 10.00 158.21

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| STANDHYD (1841) | Area (ha)= 144.40
|-----|

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```

|ID= 1 DT=15.0 min | Total Imp(%)= 48.00 Dir. Conn.(%)= 19.00
-----
Surface Area (ha)= 69.31 IMPERVIOUS 75.09 PERVIOUS (i)
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 981.16 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 53.00 76.69
over (min) 15.00 30.00
Storage Coeff. (min)= 15.96 (ii) 27.85 (iii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.07 0.04

*TOTALS*
PEAK FLOW (cms)= 3.97 13.93 17.727 (iii)
TIME TO PEAK (hrs)= 10.00 10.25 10.00
RUNOFF VOLUME (mm)= 210.00 165.30 173.79
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.78 0.82

```

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 74.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANDHYD (1842) | Area (ha)= 118.10
|ID= 1 DT=15.0 min | Total Imp(%)= 42.00 Dir. Conn.(%)= 17.00
-----
Surface Area (ha)= 49.60 IMPERVIOUS 68.50 PERVIOUS (i)
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 887.32 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 53.00 69.73
over (min) 15.00 30.00
Storage Coeff. (min)= 15.02 (ii) 27.38 (iii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.07 0.04

*TOTALS*
PEAK FLOW (cms)= 2.91 11.57 14.353 (iii)
TIME TO PEAK (hrs)= 10.00 10.25 10.00
RUNOFF VOLUME (mm)= 210.00 162.04 170.19
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.76 0.80

```

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 74.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8060) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
-----
ID1= 1 (1841): 144.40 17.727 10.00 173.79
+ ID2= 2 (1842): 118.10 14.353 10.00 170.19
=====
ID = 3 (8060): 262.50 32.080 10.00 172.17

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8064) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
-----
ID1= 1 (8060): 262.50 32.080 10.00 172.17
+ ID2= 2 (8062): 156.20 15.644 10.00 158.21
=====
ID = 3 (8064): 418.70 47.724 10.00 166.96

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| STANDHYD (1781) | Area (ha)= 14.00
|ID= 1 DT=15.0 min | Total Imp(%)= 50.00 Dir. Conn.(%)= 30.00
-----

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-----
| CALIB |
| STANDHYD (1782) | Area (ha)= 261.40 Curve Number (CN)= 89.8
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.15
-----
Unit Hyd Qpeak (cms)= 3.867

PEAK FLOW (cms)= 17.878 (i)
TIME TO PEAK (hrs)= 11.250
RUNOFF VOLUME (mm)= 180.180
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.850

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 89.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANDHYD (0180) | Area (ha)= 226.65 Curve Number (CN)= 81.6
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.17
-----

```

Unit Hyd Qpeak (cms)= 3.299

```

PEAK FLOW (cms)= 14.175 (i)
TIME TO PEAK (hrs)= 11.250
RUNOFF VOLUME (mm)= 160.833
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.759

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8046) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
-----
ID1= 1 (1781): 14.00 1.939 10.00 194.20
+ ID2= 2 (1782): 261.40 17.878 11.25 180.18
=====
ID = 3 (8046): 275.40 19.234 11.00 180.89

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| STANDHYD (0180) | Area (ha)= 226.65 Curve Number (CN)= 81.6
|ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.17
-----

```

Unit Hyd Qpeak (cms)= 3.299

```

PEAK FLOW (cms)= 14.175 (i)
TIME TO PEAK (hrs)= 11.250
RUNOFF VOLUME (mm)= 160.833
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.759

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8050) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
-----
ID1= 1 (0180): 226.65 14.175 11.25 160.83
+ ID2= 2 (8046): 275.40 19.234 11.00 180.89
=====
ID = 3 (8050): 502.05 33.242 11.00 171.84

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB |
| STANDHYD (1741) | Area (ha)= 56.30
|ID= 1 DT=15.0 min | Total Imp(%)= 35.00 Dir. Conn.(%)= 18.00
-----
IMPERVIOUS PERVIOUS (i)

```

Surface Area (ha)= 19.70 36.60
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 612.64 40.00
 Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 53.00 63.15
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 12.03 (ii) 24.69 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

PEAK FLOW (cms)= 1.48 5.69
 TIME TO PEAK (hrs)= 10.00 10.00
 RUNOFF VOLUME (mm)= 210.00 170.16
 TOTAL RAINFALL (mm)= 212.00 212.00
 RUNOFF COEFFICIENT = 0.99 0.84

TOTALS
 7.176 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 81.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (1742) | Area (ha)= 82.80 Curve Number (CN)= 85.6
 NASHVD (1742) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.69

Unit Hyd Qpeak (cms)= 2.055

PEAK FLOW (cms)= 6.977 (i)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 168.537
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.795

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8044) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (1741): 56.30 7.176 10.00 177.33
 + ID2= 2 (1742): 82.80 6.977 11.00 168.54
 ID = 3 (8044): 139.10 12.962 10.25 172.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0172) | Area (ha)= 202.23 Curve Number (CN)= 90.3
 NASHVD (0172) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.89

Unit Hyd Qpeak (cms)= 3.875

PEAK FLOW (cms)= 15.793 (i)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 180.678
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.852

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0168) | Area (ha)= 247.13 Curve Number (CN)= 70.5
 NASHVD (0168) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.75

Unit Hyd Qpeak (cms)= 5.611

PEAK FLOW (cms)= 17.048 (i)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 134.645
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.635

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

NASHVD (0170) | Area (ha)= 166.91 Curve Number (CN)= 82.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.80

Unit Hyd Qpeak (cms)= 3.553

PEAK FLOW (cms)= 12.751 (i)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 160.768
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.758

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8040) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0168): 247.13 17.048 11.00 134.65
 + ID2= 2 (0170): 166.91 12.751 11.00 160.77
 ID = 3 (8040): 414.04 29.799 11.00 145.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8042) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0172): 202.23 15.793 11.00 180.68
 + ID2= 2 (8040): 414.04 29.799 11.00 145.18
 ID = 3 (8042): 616.27 45.592 11.00 156.83

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6174) | Routing time step (min)'= 15.00
 IN= 2---> OUT= 1 |

DATA FOR SECTION (1741.0) ----->

Distance	Elevation	Manning
0.00	296.00	0.0450
13.36	288.94	0.0450
26.71	288.64	0.0450
51.19	287.96	0.0450
73.45	287.32	0.0450
86.81	286.48	0.0450
102.39	285.08	0.0450
110.73	282.76	0.0450
115.73	281.06	0.0450 / 0.0350 Main Channel
115.74	280.35	0.0350 Main Channel
120.73	280.35	0.0350 Main Channel
120.74	281.10	0.0350 / 0.0450 Main Channel
125.74	282.64	0.0450
135.78	286.87	0.0450
149.13	289.58	0.0450
162.49	292.29	0.0450
175.84	293.95	0.0450
195.88	295.13	0.0450
213.68	296.02	0.0450
220.36	296.09	0.0450

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.71	281.06	1.36E+05	6.0	1.69	37.89
1.50	281.85	358E+05	22.5	2.42	26.43
2.28	282.63	726E+05	53.8	2.85	22.47
3.07	283.42	124E+06	104.1	3.24	19.79
3.86	284.21	189E+06	176.7	3.60	17.81
4.64	284.99	268E+06	274.9	3.94	16.26
5.43	285.78	369E+06	390.8	4.07	15.73
6.21	286.56	501E+06	551.6	4.23	15.15
7.00	287.35	674E+06	750.5	4.28	14.97
7.79	288.14	920E+06	993.3	4.15	15.44
8.57	288.92	126E+07	1361.5	4.14	15.47
9.36	289.71	167E+07	1986.0	4.56	14.04
10.15	290.50	210E+07	2739.7	5.02	12.76
10.93	291.28	254E+07	3615.0	5.47	11.70
11.72	292.07	300E+07	4608.8	5.91	10.83
12.50	292.85	347E+07	5693.3	6.30	10.16
13.29	293.64	397E+07	6892.9	6.67	9.60
14.08	294.43	450E+07	8150.4	6.97	9.19
14.86	295.21	506E+07	9525.0	7.23	8.86

hydrograph <---> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8042) 616.27 45.59 11.00 156.83 2.08 2.72

OUTFLOW: ID= 1 (6174) 616.27 44.20 11.50 156.83 2.04 2.70

ADD HYD (8048)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (6174): 616.27 44.205 11.50 156.83
+ ID2= 2 (8044): 139.10 12.962 10.25 172.10
ID = 3 (8048): 755.37 55.662 11.25 159.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8052)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8048): 755.37 55.662 11.25 159.64
+ ID2= 2 (8050): 502.05 33.242 11.00 171.84
ID = 3 (8052): 1257.42 88.805 11.25 164.51

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8058)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8052): 1257.42 88.805 11.25 164.51
+ ID2= 2 (8064): 418.70 47.724 10.00 166.96
ID = 3 (8058): 1676.12 130.407 11.00 165.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (1762)
Area (ha)= 162.00
Total Imp(%)= 26.00 Dir. Com.(%)= 11.00
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 42.12 119.88
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1039.23 40.00
Mannings n = 0.013 0.250
Max. Eff. Inten. (mm/hr)= 53.00 58.29
cover (min)= 15.00 30.00
Storage Coeff. (min)= 16.52 (ii) 29.79 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.07 0.04
TOTALS
PEAK FLOW (cms)= 2.57 16.68 18.921 (iii)
TIME TO PEAK (hrs)= 10.00 10.25 10.00
RUNOFF VOLUME (mm)= 210.00 160.08 165.57
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.76 0.78

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 77.3 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (1761)
Area (ha)= 48.80
Total Imp(%)= 50.00 Dir. Com.(%)= 21.00
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 24.40 24.40
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 570.38 40.00
Mannings n = 0.013 0.250
Max. Eff. Inten. (mm/hr)= 53.00 78.99
cover (min)= 15.00 30.00
Storage Coeff. (min)= 11.52 (ii) 23.28 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04
TOTALS

PEAK FLOW (cms)= 1.50 4.82 6.318 (iii)
TIME TO PEAK (hrs)= 10.00 10.00 10.00
RUNOFF VOLUME (mm)= 210.00 170.33 178.66
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.80 0.84

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 77.3 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8056)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (1761): 48.80 6.318 10.00 178.66
+ ID2= 2 (1762): 162.00 18.921 10.00 165.57
ID = 3 (8056): 210.80 25.239 10.00 168.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8066)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8056): 210.80 25.239 10.00 168.60
+ ID2= 2 (8058): 1676.12 130.407 11.00 165.12
ID = 3 (8066): 1886.92 152.158 11.00 165.51

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8070)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8066): 1886.92 152.158 11.00 165.51
+ ID2= 2 (8068): 660.32 66.835 10.75 169.29
ID = 3 (8070): 2547.24 218.510 11.00 166.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6190)
IN= 2--> OUT= 1 Routing time step (min)= 15.00

----- DATA FOR SECTION (1901.0) -----

Distance Elevation Manning
0.00 261.34 0.0600
19.76 261.64 0.0600
59.28 261.29 0.0600
79.04 261.09 0.0600
103.74 255.99 0.0600
153.13 253.42 0.0600
192.65 252.78 0.0600
212.29 249.80 0.0600
217.29 249.50 0.0600 / 0.0350 Main Channel
217.30 249.05 0.0350 Main Channel
222.29 249.05 0.0350 Main Channel
222.30 249.50 0.0350 / 0.0600 Main Channel
227.30 249.80 0.0600
232.17 252.31 0.0600
276.63 253.53 0.0600
326.03 256.97 0.0600
355.67 257.30 0.0600
400.12 260.08 0.0600
469.28 261.25 0.0600
489.04 262.53 0.0600

----- TRAVEL TIME TABLE -----

DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.45 249.50 436E+04 1.0 0.43 75.97
1.07 250.12 205E+05 5.4 0.51 62.96
1.70 250.75 451E+05 14.8 0.64 50.78
2.32 251.37 762E+05 29.2 0.74 43.46
2.94 251.99 114E+06 49.1 0.84 38.61
3.57 252.62 161E+06 70.2 0.85 38.18
4.19 253.24 248E+06 93.4 0.73 44.29
4.81 253.86 398E+06 154.9 0.76 42.78
5.44 254.49 576E+06 246.1 0.83 38.99
6.06 255.11 779E+06 362.7 0.90 35.79
6.68 255.73 101E+07 506.5 0.98 33.15

7.30	256.35	.126E+07	687.6	1.06	30.52
7.83	256.98	.153E+07	902.8	1.15	28.18
8.55	257.60	.183E+07	1114.3	1.18	27.39
9.17	258.22	.216E+07	1407.8	1.26	25.55
9.80	258.85	.250E+07	1735.4	1.35	24.03
10.42	259.47	.286E+07	2097.5	1.42	22.73
11.04	260.09	.324E+07	2492.3	1.49	21.63
11.67	260.72	.364E+07	2837.8	1.51	21.39

```

<---- hydrograph ----> <-pipe / channel-->
AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8070) 2547.24 218.51 11.00 166.49 5.25 0.81
OUTFLOW : ID= 1 (6190) 2547.24 197.72 11.25 166.49 5.10 0.79

```

```

CALIB STANDBY (6190) Area (ha)= 287.99
ID= 1 DT=15.0 min Total Imp(%)= 51.00 Dir. Conn.(%)= 27.00

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```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 146.87 141.12
Dep. Storage (mm)= 3.00 5.00
Average Slope (%)= 0.10 0.10
Length (m)= 1385.62 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 53.00 76.81
over (min)= 30.00 60.00
Storage Coeff. (min)= 31.81 (ii) 51.08 (ii)
Unit Hyd. Tpeak (min)= 30.00 60.00
Unit Hyd. peak (cms)= 0.04 0.02

**TOTALS*
PEAK FLOW (cms)= 9.84 22.69 31.529 (iii)
TIME TO PEAK (hrs)= 10.25 10.75
RUNOFF VOLUME (mm)= 209.00 183.61 190.46
TOTAL RAINFALL (mm)= 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.87 0.90

```

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 85.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8072)
1 + 2 = 3
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0190): 287.99 31.529 10.75 190.46
+ ID2= 2 (6190): 2547.24 197.721 11.25 166.49
=====
ID = 3 (8072): 2835.23 227.194 11.00 168.92

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6192)
IN= 2--> OUT= 1
Routing time step (min)= 15.00

```

```

<----- DATA FOR SECTION (1921.0) ----->
Distance Elevation Manning
0.00 264.40 0.0500
3.64 263.94 0.0500
14.54 261.05 0.0500
29.08 256.78 0.0500
35.36 254.96 0.0500
50.90 252.20 0.0500
65.44 249.78 0.0500
76.35 246.86 0.0500
77.24 245.90 0.0500
82.24 245.45 0.0500 / 0.0350 Main Channel
82.25 244.90 0.0350 Main Channel
87.25 244.90 0.0350 Main Channel
87.26 245.45 0.0350 / 0.0500 Main Channel
94.53 246.89 0.0500
170.87 247.16 0.0500
218.13 248.69 0.0500
265.40 249.22 0.0500
301.75 250.78 0.0500
338.11 253.47 0.0500
359.92 264.00 0.0500

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu. m) (cms) (m/s) (min)
0.55 245.45 .345E+04 2.0 0.71 29.20
1.53 246.43 .174E+05 14.6 1.05 19.92
2.50 247.40 .767E+05 52.8 0.86 24.20

```

3.48	248.38	.224E+06	199.8	1.11	18.71
4.46	249.36	.429E+06	441.2	1.29	16.19
5.43	250.33	.691E+06	874.3	1.58	13.18
6.41	251.31	.987E+06	1468.3	1.86	11.20
7.38	252.28	.131E+07	2224.8	2.13	9.79
8.36	253.26	.165E+07	3132.4	2.37	8.78
9.34	254.24	.201E+07	4256.4	2.65	7.87
10.31	255.21	.238E+07	5551.0	2.92	7.15
11.29	256.19	.275E+07	6996.8	3.17	6.57
12.27	257.17	.314E+07	8585.9	3.42	6.10
13.24	258.14	.353E+07	10315.9	3.65	5.71
14.22	259.12	.393E+07	12180.7	3.87	5.38
15.19	260.09	.434E+07	14178.2	4.09	5.10
16.17	261.07	.475E+07	16306.5	4.29	4.85
17.15	262.05	.517E+07	18558.6	4.49	4.64
18.12	263.02	.559E+07	20939.7	4.68	4.45

```

<---- hydrograph ----> <-pipe / channel-->
AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8072) 2835.23 227.19 11.00 168.92 3.59 1.13
OUTFLOW : ID= 1 (6192) 2835.23 222.52 11.25 168.92 3.57 1.13

```

```

CALIB STANDBY (1921) Area (ha)= 30.70
ID= 1 DT=15.0 min Total Imp(%)= 48.00 Dir. Conn.(%)= 24.00

```

```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 14.74 15.96
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 452.40 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 53.00 70.35
over (min)= 15.00 30.00
Storage Coeff. (min)= 10.03 (ii) 22.34 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.09 0.04

**TOTALS*
PEAK FLOW (cms)= 1.08 2.81 3.892 (iii)
TIME TO PEAK (hrs)= 10.00 10.00 10.00
RUNOFF VOLUME (mm)= 210.00 158.68 171.00
TOTAL RAINFALL (mm)= 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.75 0.81

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB STANDBY (1822) Area (ha)= 215.90
ID= 1 DT=15.0 min Total Imp(%)= 27.00 Dir. Conn.(%)= 14.00

```

```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 58.29 157.61
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1199.72 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 53.00 54.72
over (min)= 15.00 45.00
Storage Coeff. (min)= 18.00 (ii) 31.62 (ii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.06 0.03

**TOTALS*
PEAK FLOW (cms)= 4.33 19.88 23.352 (iii)
TIME TO PEAK (hrs)= 10.00 10.50 10.25
RUNOFF VOLUME (mm)= 210.00 149.29 157.79
TOTAL RAINFALL (mm)= 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.70 0.74

```

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8074)

```

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1921):	30.70	3.892	10.00	171.00
+ ID2= 2 (1922):	215.90	23.352	10.25	157.79
=====				
ID = 3 (8074):	246.60	26.990	10.25	159.43

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6192):	2835.23	222.517	11.25	168.92
+ ID2= 2 (8074):	246.60	26.990	10.25	159.43
=====				
ID = 3 (8076):	3081.83	244.105	11.25	168.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHVD (0196)	41.47	84.5
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 0.24	

Unit Hyd Qpeak (cms) = 2.950

PEAK FLOW (cms) = 4.379 (i)
 TIME TO PEAK (hrs) = 10.000
 RUNOFF VOLUME (mm) = 155.096
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.732

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHVD (0194)	256.09	84.5
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 0.68	

Unit Hyd Qpeak (cms) = 6.458

PEAK FLOW (cms) = 21.501 (i)
 TIME TO PEAK (hrs) = 11.000
 RUNOFF VOLUME (mm) = 165.665
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.781

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0194):	256.09	21.501	11.00	165.66
+ ID2= 2 (0196):	41.47	4.379	10.00	155.10
=====				
ID = 3 (8078):	297.56	25.505	11.00	164.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6198)	Routing time step (min) = 15.00
ID= 2 -> ODT= 1	

Distance	Elevation	Manning
0.00	267.15	0.0500
22.99	265.94	0.0500
51.73	261.39	0.0500
74.72	258.75	0.0500
94.83	257.79	0.0500
114.95	254.36	0.0500
126.44	254.06	0.0500
132.19	253.68	0.0500
135.06	253.35	0.0500
137.94	252.93	0.0500 / 0.0300
140.81	252.41	0.0300
143.69	251.89	0.0300
146.56	252.51	0.0300
149.43	253.36	0.0300 / 0.0500
158.05	255.89	0.0500
172.42	256.25	0.0500
204.03	259.15	0.0500
229.90	260.36	0.0500
252.89	260.46	0.0500

284.50 260.45 0.0500

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.35	252.24	.111E+04	0.6	0.87	31.07
0.69	252.58	.442E+04	3.8	1.55	19.42
1.04	252.93	.974E+04	11.1	2.06	14.65
1.51	253.40	.203E+05	31.2	2.77	10.87
1.98	253.87	.357E+05	65.0	3.30	9.14
2.45	254.34	.596E+05	112.9	3.43	8.80
2.92	254.81	.935E+05	186.8	3.62	8.34
3.39	255.28	1.131E+06	282.1	3.89	7.75
3.86	255.75	1.72E+06	398.2	4.18	7.22
4.33	256.22	2.21E+06	527.0	4.31	6.99
4.80	256.69	2.82E+06	697.3	4.47	6.75
5.27	257.16	3.50E+06	898.5	4.64	6.50
5.74	257.63	4.25E+06	1131.4	4.82	6.26
6.21	258.10	5.08E+06	1367.4	4.88	6.19
6.68	258.57	6.03E+06	1635.6	4.91	6.14
7.15	259.04	7.09E+06	1977.2	5.04	5.98
7.62	259.51	8.26E+06	2360.5	5.17	5.83
8.09	259.98	9.54E+06	2790.7	5.29	5.70
8.56	260.45	1.10E+07	3179.6	5.25	5.74

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
INFLOW : ID= 2 (8078)	297.56	25.51	11.00	164.19	1.38
OUTFLOW : ID= 1 (6198)	297.56	25.18	11.00	164.19	1.37

CALIB	Area (ha)	Dir. Conn.(%)
STANDHVD (1981)	104.70	20.00
ID= 1 DT=15.0 min	Total Imp(%) = 44.00	

Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
46.07		58.63
2.00		5.00
0.50		0.50
835.46		40.00
0.013		0.250
53.00		70.14
18.00		30.00
14.49 (ii)		26.82 (ii)
15.00		30.00
0.07		0.04

PEAK FLOW (cms)	12.960 (iii)
3.05	10.00
10.00	10.25
210.00	173.41
212.00	212.00
0.99	0.77

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 76.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHVD (1982)	285.90	82.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res.(N)= 1.50
	U.H. Tp(hrs)= 0.81	

Unit Hyd Qpeak (cms) = 6.048

PEAK FLOW (cms) = 21.778 (i)
 TIME TO PEAK (hrs) = 11.000
 RUNOFF VOLUME (mm) = 160.789
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.758

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1981):	104.70	12.960	10.00	173.41
+ ID2= 2 (1982):	285.90	21.778	11.00	160.79
=====				
ID = 3 (8080):	390.60	32.648	11.00	164.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8082)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6198):	297.56	25.181	11.00	164.19
+ ID2= 2 (8080):	390.60	32.648	11.00	164.17
=====				
ID = 3 (8082):	688.16	57.830	11.00	164.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8084)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8076):	3081.83	244.105	11.25	168.16
+ ID2= 2 (8082):	688.16	57.830	11.00	164.18
=====				
ID = 3 (8084):	3769.99	299.326	11.25	167.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6200)	ROUTING TIME STEP (min)
INS 2 -> OUT 1	15.00

----- DATA FOR SECTION (2001.0) -----

Distance	Elevation	Manning
0.00	252.47	0.0500
10.38	252.29	0.0500
20.76	252.28	0.0500
41.52	247.66	0.0500
62.28	247.47	0.0500
80.45	247.34	0.0500
90.83	246.85	0.0500
103.81	245.51	0.0500
106.00	244.50	0.0500
108.99	244.05	0.0500 / 0.0300 Main Channel
109.00	243.50	0.0300 Main Channel
114.00	243.50	0.0300 Main Channel
114.01	244.05	0.0300 / 0.0500 Main Channel
118.99	244.50	0.0500
132.36	245.09	0.0500
150.52	245.83	0.0500
199.83	249.24	0.0500
207.62	250.16	0.0500
236.16	253.11	0.0500
256.92	253.83	0.0500

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.28	243.77	.365E+04	0.7	0.54	81.42
0.55	244.05	.730E+04	2.2	0.81	54.59
1.05	244.55	.197E+05	7.2	0.97	45.51
1.54	245.04	.463E+05	17.2	0.99	44.76
2.04	245.54	.895E+05	34.8	1.03	42.81
2.53	246.03	.152E+06	62.8	1.09	40.43
3.03	246.53	.232E+06	104.7	1.20	36.91
3.52	247.02	.328E+06	159.7	1.29	34.19
4.02	247.52	.450E+06	223.4	1.32	33.56
4.51	248.01	.627E+06	326.9	1.38	31.99
5.01	248.51	.820E+06	463.0	1.50	29.52
5.50	249.00	.103E+07	626.0	1.62	27.30
6.00	249.50	.124E+07	820.1	1.75	25.24
6.49	249.99	.147E+07	1044.4	1.89	23.42
6.99	250.49	.170E+07	1292.6	2.01	21.95
7.48	250.98	.195E+07	1565.6	2.13	20.71
7.98	251.48	.220E+07	1864.5	2.25	19.65
8.47	251.97	.246E+07	2189.4	2.36	18.73
8.97	252.47	.274E+07	2427.5	2.35	18.80

<---- hydrograph ----> <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (8084)	3769.99	299.33	11.25	167.44	4.38	1.36
OUTFLOW : ID= 1 (6200)	3769.99	282.22	11.50	167.44	4.30	1.35

CALIB STANDBYD (2001)	Area (ha)	Imp(%)	Dir. Conn.(%)
ID= 1 DT=15.0 min	6.00	50.00	23.00

Surface Area (ha)	Impervious (mm)	Pervious (i)
3.00	3.00	5.00

Average Slope (%)	0.50	0.50	
Length (m)	200.00	40.00	
Mannings n	0.013	0.250	
Max.Eff.Inten.(mm/hr)	53.00	78.99	
over (min)	15.00	30.00	
Storage Coeff. (min)	6.15 (ii)	17.90 (ii)	
Unit Hyd. Tpeak (min)	15.00	30.00	
Unit Hyd. peak (cms)	0.10	0.05	
*****TOTALS*****			
PEAK FLOW (cms)	0.20	0.62	0.826 (iii)
TIME TO PEAK (hrs)	10.00	10.00	10.00
RUNOFF VOLUME (mm)	210.00	180.21	187.06
TOTAL RAINFALL (mm)	212.00	212.00	212.00
RUNOFF COEFFICIENT	0.99	0.85	0.88

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CN* = 83.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDBYD (2002)	Area (ha)	Imp(%)	Dir. Conn.(%)
ID= 1 DT=15.0 min	109.30	22.00	10.00

Surface Area (ha)	Impervious (mm)	Pervious (i)
24.05	85.25	5.00
2.00	5.00	5.00
0.50	40.00	0.50
853.62	40.00	0.250

Max.Eff.Inten.(mm/hr)	53.00	57.99	
over (min)	15.00	30.00	
Storage Coeff. (min)	14.68 (ii)	27.98 (ii)	
Unit Hyd. Tpeak (min)	15.00	30.00	
Unit Hyd. peak (cms)	0.07	0.04	
*****TOTALS*****			
PEAK FLOW (cms)	1.59	11.97	13.439 (iii)
TIME TO PEAK (hrs)	10.00	10.25	10.00
RUNOFF VOLUME (mm)	210.00	171.27	175.14
TOTAL RAINFALL (mm)	212.00	212.00	212.00
RUNOFF COEFFICIENT	0.99	0.81	0.83

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CN* = 83.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8086)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (2001):	6.00	0.826	10.00	187.06
+ ID2= 2 (2002):	109.30	13.439	10.00	175.14
=====				
ID = 3 (8086):	115.30	14.264	10.00	175.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8088)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6200):	3769.99	282.220	11.50	167.44
+ ID2= 2 (8086):	115.30	14.264	10.00	175.76
=====				
ID = 3 (8088):	3885.29	289.780	11.50	167.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8038)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8034):	5220.81	198.568	13.00	130.87
+ ID2= 2 (8088):	3885.29	289.780	11.50	167.68
=====				
ID = 3 (8038):	9106.10	466.285	11.75	146.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (1365) |
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (1362.0) ----->

Distance	Elevation	Manning
0.00	252.00	0.0400
2.35	244.60	0.0400
39.94	243.14	0.0400
58.74	242.65	0.0400
91.63	242.03	0.0400
101.03	241.69	0.0400
103.38	241.66	0.0400
105.73	240.76	0.0400
105.93	240.59	0.0400
109.18	240.28	0.0400 / 0.0300
110.43	239.81	0.0300
111.68	239.81	0.0300
113.43	240.09	0.0300 / 0.0400
118.43	240.50	0.0400
138.62	241.68	0.0400
152.72	241.60	0.0400
166.82	242.88	0.0400
185.62	249.09	0.0400
197.36	251.00	0.0400
232.61	252.28	0.0400

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.28	240.09	.118E+04	0.1	0.20	142.09
0.91	240.72	.114E+05	2.4	0.35	79.92
1.53	241.34	.352E+05	9.4	0.45	62.17
2.16	241.97	.831E+05	22.8	0.46	60.69
2.79	242.60	.172E+06	54.2	0.53	52.74
3.41	243.22	.297E+06	111.4	0.63	44.49
4.04	243.85	.445E+06	194.1	0.74	38.22
4.67	244.48	.612E+06	301.1	0.83	33.88
5.29	245.10	.793E+06	443.0	0.94	29.82
5.92	245.73	.976E+06	611.3	1.06	26.60
6.55	246.36	.116E+07	802.0	1.17	24.12
7.18	246.99	.135E+07	1014.0	1.27	22.16
7.80	247.61	.154E+07	1246.2	1.37	20.56
8.43	248.24	.173E+07	1498.0	1.46	19.24
9.06	248.87	.192E+07	1768.8	1.55	18.12
9.68	249.49	.212E+07	2049.6	1.63	17.24
10.31	250.12	.232E+07	2344.7	1.71	16.49
10.94	250.75	.252E+07	2658.5	1.78	15.83
11.56	251.37	.274E+07	2931.2	1.81	15.56

hydrograph ----->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8038) 9106.10	466.28	11.75	146.58	5.38	0.95
OUTFLOW : ID= 1 (1365) 9106.10	445.61	12.25	146.58	5.32	0.95

CALIB STANDARD (1321) |
ID= 1 DT=15.0 min | Area (ha)= 41.70
Total Imp(%)= 55.00 Dir. Com.(%)= 35.00

Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
22.94		18.77
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	527.26	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	53.00	74.30
over (min)	15.00	30.00
Storage Coeff. (min)	10.89 (ii)	23.04 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04
PEAK FLOW (cms)	2.14	3.51
TIME TO PEAK (hrs)	10.00	10.00
RUNOFF VOLUME (mm)	210.00	181.34
TOTAL RAINFALL (mm)	212.00	212.00
RUNOFF COEFFICIENT	0.99	0.86

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
TO THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (1322) |
ID= 1 DT=15.0 min | Area (ha)= 249.10
Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.83

Unit Hyd Qpeak (cms)= 5.130
PEAK FLOW (cms)= 19.675 (i)
TIME TO PEAK (hrs)= 11.000
RUNOFF VOLUME (mm)= 173.516
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.818

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8090) |
1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)

ID= 1 (1321):	41.70	5.653	10.00	191.37
+ ID= 2 (1322):	249.10	19.675	11.00	173.52
ID = 3 (8090):	290.80	24.112	11.00	176.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0134) |
ID= 1 DT=15.0 min | Area (ha)= 155.95
Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
U.H. Tp(hrs)= 0.64

Unit Hyd Qpeak (cms)= 4.167
PEAK FLOW (cms)= 12.996 (i)
TIME TO PEAK (hrs)= 11.000
RUNOFF VOLUME (mm)= 157.783
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.744

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8092) |
1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)

ID= 1 (0134):	155.95	12.996	11.00	157.78
+ ID= 2 (8090):	290.80	24.112	11.00	176.08
ID = 3 (8092):	446.75	37.108	11.00	169.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (1366) |
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (1361.0) ----->

Distance	Elevation	Manning
0.00	262.81	0.0400
37.23	260.54	0.0400
79.12	256.98	0.0400
111.70	254.74	0.0400
172.20	252.76	0.0400
251.32	248.82	0.0400
255.97	248.75	0.0400
260.43	248.76	0.0400
265.43	248.49	0.0400
268.68	248.18	0.0400 / 0.0300
269.93	247.71	0.0300
271.18	247.71	0.0300
272.93	247.99	0.0300
277.93	248.40	0.0300 / 0.0400
283.90	248.88	0.0400
307.17	249.60	0.0400
321.13	251.59	0.0400
363.01	257.08	0.0400
400.25	257.58	0.0400
460.75	261.91	0.0400

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.47	248.18	.881E+04	1.3	0.76	114.66
1.19	248.80	.742E+05	17.3	1.22	71.44
1.92	249.63	.278E+06	78.3	1.48	59.26
2.64	250.35	.589E+06	210.3	1.87	46.69
3.36	251.07	.974E+06	414.3	2.23	39.19

4.08	251.79	.143E+07	696.2	2.55	34.32
4.81	252.52	.197E+07	1063.0	2.83	30.86
5.53	253.24	.259E+07	1505.7	3.06	28.62
6.25	253.96	.331E+07	2051.3	3.26	26.85
6.97	254.68	.413E+07	2720.9	3.46	25.30
7.70	255.41	.504E+07	3584.5	3.73	23.43
8.42	256.13	.601E+07	4580.9	4.00	21.87
9.14	256.85	.704E+07	5707.9	4.25	20.57
9.86	257.57	.818E+07	6936.1	4.26	20.53
10.59	258.30	.945E+07	8115.0	4.50	19.42
11.31	259.02	1.08E+08	9759.2	4.74	18.45
12.03	259.74	1.22E+08	11569.5	4.96	17.61
12.75	260.46	1.37E+08	13550.6	5.18	16.87
13.48	261.19	1.53E+08	15636.4	5.37	16.29

----- hydrograph -----< pipe / channel ->
 AREA QPEAK TPEAK R.V. MAX DEPTH
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8092) 446.75 37.11 11.00 169.69 1.43 1.30
 OUTFLOW : ID= 1 (1366) 446.75 31.61 11.75 169.69 1.36 1.27

ID1= 1 (1361): 102.30 13.379 10.00 187.29
 + ID2= 2 (1362): 410.10 46.320 10.50 171.68
 =====
 ID = 3 (8096): 512.40 58.816 10.25 174.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8094)
 1 + 2 = 3 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (1366): 446.75 31.607 11.75 169.69
 + ID2= 2 (8096): 512.40 58.816 10.25 174.80
 =====
 ID = 3 (8094): 959.15 84.009 11.00 172.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8098)
 1 + 2 = 3 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (1365): 9106.10 445.614 12.25 146.58
 + ID2= 2 (8094): 959.15 84.009 11.00 172.42
 =====
 ID = 3 (8098): 10065.25 508.068 11.75 149.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB NASHYD (0142) Area (ha)= 396.54 Curve Number (CN)= 79.6
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.32
 Unit Hyd Qpeak (cms)= 5.133
 PEAK FLOW (cms)= 22.695 (i)
 TIME TO PEAK (hrs)= 11.500
 RUNOFF VOLUME (mm)= 156.413
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.738

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB NASHYD (0138) Area (ha)= 414.79 Curve Number (CN)= 42.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.39
 Unit Hyd Qpeak (cms)= 5.087
 PEAK FLOW (cms)= 12.049 (i)
 TIME TO PEAK (hrs)= 12.000
 RUNOFF VOLUME (mm)= 76.344
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.360

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB STANDHYD (0140) Area (ha)= 158.58 Dir. Conn.(%)= 21.00
 ID= 1 DT=15.0 min Total Imp(%)= 24.00
 IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 38.06 120.52
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1028.20 40.00
 Mannings n = 0.013 0.250
 Max.Eff.Inten.(mm/hr)= 53.00 39.86
 over (min) 15.00 45.00
 Storage Coeff. (min)= 16.41 (ii) 31.87 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.07 0.03
 PEAK FLOW (cms)= 4.81 11.13
 TIME TO PEAK (hrs)= 10.00 10.50
 RUNOFF VOLUME (mm)= 210.00 117.48
 TOTAL RAINFALL (mm)= 212.00 212.00
 RUNOFF COEFFICIENT = 0.99 0.55 0.65

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 60.7 Ia = Dep. Storage (Above)

 CALIB STANDHYD (1362) Area (ha)= 410.10 Dir. Conn.(%)= 11.00
 ID= 1 DT=15.0 min Total Imp(%)= 21.00
 IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 86.12 323.98
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1653.48 40.00
 Mannings n = 0.013 0.250
 Max.Eff.Inten.(mm/hr)= 53.00 55.91
 over (min) 15.00 45.00
 Storage Coeff. (min)= 21.83 (ii) 35.32 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.06 0.03
 PEAK FLOW (cms)= 6.32 41.16
 TIME TO PEAK (hrs)= 10.00 10.50
 RUNOFF VOLUME (mm)= 210.00 166.94
 TOTAL RAINFALL (mm)= 212.00 212.00
 RUNOFF COEFFICIENT = 0.99 0.79

 TOTALS
 PEAK FLOW (cms)= 6.32 41.16 46.320 (iii)
 TIME TO PEAK (hrs)= 10.00 10.50
 RUNOFF VOLUME (mm)= 210.00 166.94 171.68
 TOTAL RAINFALL (mm)= 212.00 212.00 212.00
 RUNOFF COEFFICIENT = 0.99 0.79 0.81

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 81.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB STANDHYD (1361) Area (ha)= 102.30 Dir. Conn.(%)= 27.00
 ID= 1 DT=15.0 min Total Imp(%)= 55.00
 IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 56.27 46.03
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 825.83 40.00
 Mannings n = 0.013 0.250
 Max.Eff.Inten.(mm/hr)= 53.00 82.93
 over (min) 15.00 30.00
 Storage Coeff. (min)= 14.39 (ii) 25.92 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.07 0.04
 PEAK FLOW (cms)= 4.02 9.37
 TIME TO PEAK (hrs)= 10.00 10.50
 RUNOFF VOLUME (mm)= 210.00 178.89
 TOTAL RAINFALL (mm)= 212.00 212.00
 RUNOFF COEFFICIENT = 0.99 0.84

 TOTALS
 PEAK FLOW (cms)= 4.02 9.37 13.379 (iii)
 TIME TO PEAK (hrs)= 10.00 10.25
 RUNOFF VOLUME (mm)= 210.00 178.89 187.29
 TOTAL RAINFALL (mm)= 212.00 212.00 212.00
 RUNOFF COEFFICIENT = 0.99 0.84 0.88

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 81.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8096)
 1 + 2 = 3 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8102)						
1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0138):			414.79	12.049	12.00	76.34
+ ID2= 2 (0140):			158.58	14.915	10.25	136.91
=====						
ID = 3 (8102):			573.37	25.356	11.00	93.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6142) |
 IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1421.0) ----->

Distance	Elevation	Manning	
0.00	295.50	0.0400	
7.25	295.36	0.0400	
14.50	295.22	0.0400	
21.75	295.22	0.0400	
29.00	294.23	0.0400	
52.56	286.45	0.0400	
67.06	281.44	0.0400	
77.94	278.28	0.0400	
82.81	277.20	0.0400 / 0.0300	Main Channel
83.21	276.80	0.0300	Main Channel
88.81	276.80	0.0300	Main Channel
96.41	276.80	0.0300	Main Channel
96.81	277.20	0.0300 / 0.0400	Main Channel
103.31	278.96	0.0400	
108.75	280.16	0.0400	
117.81	282.73	0.0400	
135.94	290.30	0.0400	
159.50	292.14	0.0400	
174.00	293.07	0.0400	
179.44	293.37	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.40	277.20	.238E+05	6.9	1.27	57.28
1.25	278.05	.869E+05	50.9	2.50	29.13
2.10	278.90	.179E+06	133.4	3.26	22.36
2.95	279.75	.293E+06	256.7	3.83	19.03
3.80	280.60	.432E+06	426.0	4.31	16.90
4.66	281.46	.593E+06	644.3	4.75	15.35
5.51	282.31	.776E+06	915.3	5.16	14.12
6.36	283.16	.978E+06	1242.2	5.55	13.12
7.21	284.01	1.20E+07	1626.2	5.94	12.27
8.06	284.86	1.43E+07	2065.6	6.30	11.57
8.91	285.71	1.69E+07	2562.4	6.64	10.97
9.76	286.56	1.96E+07	3118.0	6.97	10.46
10.61	287.41	2.24E+07	3733.3	7.28	10.02
11.46	288.26	2.55E+07	4411.9	7.57	9.62
12.31	289.11	2.87E+07	5155.6	7.86	9.27
13.17	289.97	3.21E+07	5966.6	8.14	8.96
14.02	290.82	3.57E+07	6705.0	8.22	8.87
14.87	291.67	3.98E+07	7494.3	8.24	8.88
15.72	292.52	4.44E+07	8405.3	8.28	8.80

<---- hydrograph ----> <-pipe / channel->

	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8102)	573.37	25.36	11.00	93.10	0.76	1.60
OUTFLOW: ID= 1 (6142)	573.37	22.96	11.25	93.10	0.71	1.55

ADD HYD (8104)						
1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0142):			396.54	22.695	11.50	156.41
+ ID2= 2 (6142):			573.37	22.960	11.25	93.10
=====						
ID = 3 (8104):			969.91	45.531	11.25	118.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0144) | Area (ha)= 135.69 Curve Number (CN)= 87.6
 NASHVD (0144) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.71

Unit Hyd Peak (cms)= 3.240

PEAK FLOW (cms)= 11.438 (1)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 173.409
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.818

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8106)						
1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0144):			135.69	11.438	11.00	173.41
+ ID2= 2 (8104):			969.91	45.531	11.25	118.98
=====						
ID = 3 (8106):			1105.60	56.676	11.25	125.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6146) |
 IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1461.0) ----->

Distance	Elevation	Manning	
0.00	262.00	0.0350	
39.88	257.13	0.0350	
123.69	257.51	0.0350	
183.07	256.83	0.0350	
202.86	255.73	0.0350	
212.75	254.73	0.0350	
222.65	252.86	0.0350	
232.44	250.70	0.0350 / 0.0300	Main Channel
236.44	250.20	0.0300	Main Channel
236.84	249.80	0.0300	Main Channel
242.44	249.80	0.0300	Main Channel
250.04	249.80	0.0300	Main Channel
250.44	250.20	0.0300	Main Channel
252.33	250.57	0.0300 / 0.0350	Main Channel
286.97	253.52	0.0350	
331.50	255.80	0.0350	
380.98	255.55	0.0350	
445.30	258.40	0.0350	
455.19	259.82	0.0350	
489.83	262.22	0.0350	

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.39	250.19	.169E+05	5.4	1.04	52.00
0.77	250.57	.373E+05	16.6	1.44	37.46
1.44	251.24	.911E+05	60.3	2.14	25.19
2.11	251.91	1.69E+06	136.3	2.61	20.64
2.79	252.59	2.70E+06	249.7	2.99	18.05
3.46	253.26	3.96E+06	405.4	3.31	16.28
4.13	253.93	5.49E+06	603.3	3.56	15.15
4.80	254.60	7.37E+06	857.8	3.77	14.31
5.48	255.28	9.63E+06	1174.6	3.95	13.67
6.15	255.95	1.28E+07	1449.5	3.66	14.74
6.82	256.62	1.73E+07	2046.4	3.83	14.07
7.49	257.29	2.27E+07	2665.2	3.81	14.17
8.17	257.97	3.10E+07	3705.2	3.87	13.93
8.84	258.64	4.00E+07	5127.4	4.15	12.99
9.51	259.31	4.92E+07	6858.6	4.51	11.95
10.18	259.98	5.87E+07	8815.5	4.86	11.09
10.86	260.66	6.84E+07	10954.6	5.18	10.41
11.53	261.33	7.85E+07	13324.3	5.49	9.82
12.20	262.00	8.90E+07	15923.2	5.79	9.31

<---- hydrograph ----> <-pipe / channel->

	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8106)	1105.60	56.68	11.25	125.66	1.39	2.06
OUTFLOW: ID= 1 (6146)	1105.60	56.10	11.50	125.66	1.38	2.04

STANDHYD (1461) | Area (ha)= 70.50
 ID= 1 DT=15.0 min | Total Imp(%)= 36.00 Dir. Conn.(%)= 15.00

CALIB (0144) | Surface Area (ha)= 25.38 Pervious (i)= 45.12
 NASHVD (0144) | Dep. Storage (mm)= 2.00 Pervious (i)= 5.00
 ID= 1 DT=15.0 min | Average Slope (%)= 0.50 Pervious (i)= 0.50
 Length (m)= 685.57 Pervious (i)= 40.00
 Mannings n = 0.013 Pervious (i)= 0.250
 Max.Eff.Inten.(mm/hr)= 53.00 Pervious (i)= 68.57

over (min) 15.00 30.00
Storage Coeff. (min)= 12.87 (ii) 25.31 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04

TOTALS
PEAK FLOW (cms)= 1.55 7.64 9.190 (iii)
TIME TO PEAK (hrs)= 10.00 10.00 10.00
RUNOFF VOLUME (mm)= 210.00 182.91 186.97
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.86 0.88

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 86.9 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |
| STANDHYD (1462) | Area (ha)= 125.00
| ID= 1 DT=15.0 min | Total Imp(%)= 29.00 Dir. Conn.(%)= 12.00

Surface Area (ha)= 36.25 IMPERVIOUS 88.75 PERVIOUS (i)
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 912.87 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 53.00 63.78
over (min) 15.00 30.00
Storage Coeff. (min)= 15.28 (ii) 28.09 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.07 0.04

TOTALS
PEAK FLOW (cms)= 2.18 13.74 15.807 (iii)
TIME TO PEAK (hrs)= 10.00 10.25 10.00
RUNOFF VOLUME (mm)= 210.00 181.07 184.54
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.85 0.87

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 86.9 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| ADD HYD (8099) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
+ ID1= 1 (1461): 70.50 9.190 10.00 186.97
+ ID2= 2 (1462): 125.00 15.807 10.00 184.54
=====

ID = 3 (8099): 195.50 24.997 10.00 185.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ADD HYD (8108) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
+ ID1= 1 (6146): 1105.60 56.099 11.50 125.66
+ ID2= 2 (8099): 195.50 24.997 10.00 185.42
=====

ID = 3 (8108): 1301.10 72.949 11.00 134.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ADD HYD (8100) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
+ ID1= 1 (8098): 10065.25 508.068 11.75 149.04
+ ID2= 2 (8108): 1301.10 72.949 11.00 134.64
=====

ID = 3 (8100): 11366.35 574.816 11.50 147.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ROUTE CHN (6202) |
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (2021.0) ----->

Distance	Elevation	Manning
0.00	245.48	0.0400
29.68	242.80	0.0400
74.30	240.39	0.0400
155.81	237.12	0.0400
200.33	234.48	0.0400
244.85	234.67	0.0400
259.69	233.96	0.0400
274.53	233.84	0.0400
281.68	233.40	0.0400
285.78	232.40	0.0400 / 0.0300
286.78	231.40	0.0300
296.78	231.40	0.0300
299.78	232.40	0.0300 / 0.0400
311.62	233.74	0.0400
333.88	234.34	0.0400
497.11	235.32	0.0400
571.31	237.32	0.0400
675.18	239.10	0.0400
704.86	242.71	0.0400
734.54	242.62	0.0400

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.50	231.90	.221E+05	4.7	0.85	78.67
1.00	232.40	.482E+05	15.3	1.28	52.40
1.60	233.00	.913E+05	38.3	1.68	39.77
2.20	233.60	.154E+06	72.9	1.90	35.24
2.80	234.20	.279E+06	124.2	1.79	37.10
3.40	234.80	.599E+06	214.3	1.44	46.59
4.01	235.41	.125E+07	445.0	1.43	46.90
4.61	236.01	.205E+07	838.4	1.64	40.81
5.21	236.61	.293E+07	1361.6	1.86	35.99
5.81	237.21	.389E+07	2011.2	2.08	32.23
6.41	237.81	.494E+07	2753.5	2.24	29.91
7.01	238.41	.612E+07	3635.0	2.39	28.04
7.61	239.01	.741E+07	4672.5	2.53	26.43
8.21	239.61	.880E+07	5999.3	2.74	24.44
8.82	240.22	.102E+08	7510.0	2.94	22.72
9.42	240.82	.117E+08	9201.9	3.15	21.23
10.02	241.42	.132E+08	11059.7	3.35	19.95
10.62	242.02	.148E+08	13073.4	3.54	18.87
11.22	242.62	.164E+08	15242.1	3.73	17.93

----- hydrograph -----> <- pipe / channel ->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8100) ***** 574.82 11.50 147.39 4.20 1.49
OUTFLOW : ID= 1 (6202) ***** 543.85 12.25 147.39 4.16 1.47

| ADD HYD (8112) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
+ ID1= 1 (6202): 11366.35 543.852 12.25 147.39
+ ID2= 2 (8110): 2468.50 57.751 10.00 193.91
=====

ID = 3 (8112): 11834.85 561.495 12.00 149.23

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| CALIB |
| STANDHYD (1662) | Area (ha)= 158.10
| ID= 1 DT=15.0 min | Total Imp(%)= 58.00 Dir. Conn.(%)= 30.00

Surface Area (ha)= 91.70 IMPERVIOUS 66.40 PERVIOUS (i)
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1026.65 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 53.00 86.80
over (min) 15.00 30.00
Storage Coeff. (min)= 16.40 (ii) 27.72 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.07 0.04

TOTALS
PEAK FLOW (cms)= 6.85 14.04 20.818 (iii)
TIME TO PEAK (hrs)= 10.00 10.25 10.00
RUNOFF VOLUME (mm)= 210.00 186.30 184.81
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.89 0.92

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 86.9 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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| CALIB |
| STANHYD (1661) | Area (ha)= 276.60
| ID= 1 DT=15.0 min | Total Imp(%)= 40.00 Dir. Conn.(%)= 21.00
-----
| IMPERVIOUS | PERVIOUS (i) | |
| Surface Area (ha)= 110.64 | 165.96 |
| Dep. Storage (mm)= 2.00 | 5.00 |
| Average Slope (%)= 0.50 | 0.50 |
| Length (m)= 1357.94 | 40.00 |
| Mannings n = 0.013 | 0.250 |
| Max.Eff.Inten.(mm/hr)= 53.00 | 67.95 |
| over (min) | 15.00 | 45.00 |
| Storage Coeff. (min)= 19.39 (ii) | 31.88 (ii) |
| Unit Hyd. Tpeak (min)= 15.00 | 45.00 |
| Unit Hyd. peak (cms)= 0.06 | 0.03 |
|
| PEAK FLOW (cms)= 8.26 | 26.21 | *TOTALS*
| TIME TO PEAK (hrs)= 10.00 | 10.25 | 33.279 (iii)
| RUNOFF VOLUME (mm)= 210.00 | 182.69 | 188.42
| TOTAL RAINFALL (mm)= 212.00 | 212.00 | 212.00
| RUNOFF COEFFICIENT = 0.99 | 0.86 | 0.89
  
```

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 86.9 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
| ADD HYD (8134) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm) |
| ID= 1 (1661): 276.60 33.279 10.25 188.42
| + ID= 2 (1662): 158.10 20.818 10.00 194.81
| ID = 3 (8134): 434.70 53.459 10.00 190.75
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| CALIB |
| STANHYD (1642) | Area (ha)= 132.50
| ID= 1 DT=15.0 min | Total Imp(%)= 20.00 Dir. Conn.(%)= 15.00
-----
| IMPERVIOUS | PERVIOUS (i) | |
| Surface Area (ha)= 26.50 | 106.00 |
| Dep. Storage (mm)= 2.00 | 5.00 |
| Average Slope (%)= 0.50 | 0.50 |
| Length (m)= 939.86 | 40.00 |
| Mannings n = 0.013 | 0.250 |
| Max.Eff.Inten.(mm/hr)= 53.00 | 53.94 |
| over (min) | 15.00 | 30.00 |
| Storage Coeff. (min)= 15.55 (ii) | 29.25 (ii) |
| Unit Hyd. Tpeak (min)= 15.00 | 30.00 |
| Unit Hyd. peak (cms)= 0.07 | 0.04 |
|
| PEAK FLOW (cms)= 2.88 | 13.76 | *TOTALS*
| TIME TO PEAK (hrs)= 10.00 | 10.25 | 16.461 (iii)
| RUNOFF VOLUME (mm)= 210.00 | 174.76 | 180.04
| TOTAL RAINFALL (mm)= 212.00 | 212.00 | 212.00
| RUNOFF COEFFICIENT = 0.99 | 0.82 | 0.85
  
```

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 86.1 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| STANHYD (1641) | Area (ha)= 101.70
| ID= 1 DT=15.0 min | Total Imp(%)= 70.00 Dir. Conn.(%)= 52.00
-----
| IMPERVIOUS | PERVIOUS (i) |
| Surface Area (ha)= 71.19 | 30.51 |
| Dep. Storage (mm)= 2.00 | 5.00 |
  
```

```

Average Slope (%)= 0.50 0.50
Length (m)= 823.41 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 53.00 83.02
over (min) | 15.00 | 30.00
Storage Coeff. (min)= 14.37 (ii) | 25.89 (ii) |
Unit Hyd. Tpeak (min)= 15.00 | 30.00 |
Unit Hyd. peak (cms)= 0.07 | 0.04 |
  
```

```

*TOTALS*
PEAK FLOW (cms)= 7.70 6.23 13.930 (iii)
TIME TO PEAK (hrs)= 10.00 10.00 10.00
RUNOFF VOLUME (mm)= 210.00 186.05 198.50
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.88 0.94
  
```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 86.1 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8132) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm) |
| ID= 1 (1641): 101.70 13.930 10.00 198.50
| + ID= 2 (1642): 132.50 16.461 10.00 180.04
| ID = 3 (8132): 234.20 30.390 10.00 188.06
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| CALIB |
| NASHYD (0162) | Area (ha)= 190.14 Curve Number (CN)= 81.1
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.79
|
| Unit Hyd Qpeak (cms)= 4.104
| PEAK FLOW (cms)= 14.489 (i)
| TIME TO PEAK (hrs)= 11.000
| RUNOFF VOLUME (mm)= 158.645
| TOTAL RAINFALL (mm)= 212.000
| RUNOFF COEFFICIENT = 0.748
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0160) | Area (ha)= 318.82 Curve Number (CN)= 78.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.45
|
| Unit Hyd Qpeak (cms)= 3.754
| PEAK FLOW (cms)= 16.939 (i)
| TIME TO PEAK (hrs)= 11.750
| RUNOFF VOLUME (mm)= 152.875
| TOTAL RAINFALL (mm)= 212.000
| RUNOFF COEFFICIENT = 0.721
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0152) | Area (ha)= 124.37 Curve Number (CN)= 76.8
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 0.89
|
| Unit Hyd Qpeak (cms)= 2.372
| PEAK FLOW (cms)= 8.500 (i)
| TIME TO PEAK (hrs)= 11.000
| RUNOFF VOLUME (mm)= 149.204
| TOTAL RAINFALL (mm)= 212.000
| RUNOFF COEFFICIENT = 0.704
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHYD (0148) | Area (ha)= 417.89 Curve Number (CN)= 43.7
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.18
  
```

Unit Hyd Qpeak (cms)= 6.067
 PEAK FLOW (cms)= 13.949 (1)
 TIME TO PEAK (hrs)= 11.750
 RUNOFF VOLUME (mm)= 79.563
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.375

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0150) | Area (ha)= 105.88 Curve Number (CN)= 33.1
 NASHYD (0150) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.58

Unit Hyd Qpeak (cms)= 3.138

PEAK FLOW (cms)= 3.900 (1)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 58.110
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.274

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8116) |
 1 + 2 = 3 |
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0148): 417.89 13.949 11.75 79.56
 + ID2= 2 (0150): 105.88 3.900 11.00 58.11
 ID= 3 (8116): 523.77 17.584 11.25 75.23

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6152) |
 IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (1521.0) ----->

Distance	Elevation	Manning
0.00	290.00	0.0500
15.34	286.87	0.0500
46.03	284.84	0.0500
88.22	283.07	0.0500
126.57	282.11	0.0500
153.42	280.34	0.0500
157.26	280.07	0.0500
161.09	279.72	0.0500
167.76	278.94	0.0500 /0.0300 Main Channel
168.01	278.60	0.0300 Main Channel
168.76	278.60	0.0300 Main Channel
169.51	278.60	0.0300 Main Channel
169.76	278.90	0.0300 /0.0500 Main Channel
173.76	279.40	0.0500
180.27	279.69	0.0500
184.11	279.93	0.0500
218.62	281.86	0.0500
260.82	283.20	0.0500
360.54	289.25	0.0500
379.72	289.80	0.0500

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.30	278.90	1.14E+04	0.4	0.80	45.70
0.87	279.47	9.32E+04	4.2	0.98	37.22
1.45	280.05	3.53E+05	17.5	1.09	33.68
2.02	280.62	8.31E+05	47.8	1.26	29.01
2.59	281.19	1.55E+06	102.9	1.46	25.06
3.17	281.77	2.50E+06	189.3	1.66	22.03
3.74	282.34	3.75E+06	298.5	1.75	20.91
4.32	282.92	5.47E+06	456.5	1.83	19.98
4.89	283.49	7.67E+06	705.7	2.02	18.12
5.46	284.06	1.02E+07	1038.0	2.24	16.34
6.04	284.64	1.30E+07	1444.1	2.45	14.98
6.61	285.21	1.61E+07	1940.1	2.66	13.79
7.18	285.78	1.94E+07	2522.6	2.86	12.80
7.76	286.36	2.29E+07	3187.4	3.06	11.98
8.33	286.93	2.67E+07	3942.3	3.25	11.28
8.91	287.51	3.06E+07	4824.9	3.46	10.58
9.48	288.08	3.47E+07	5795.7	3.67	9.99
10.05	288.65	3.90E+07	6855.4	3.86	9.48
10.63	289.23	4.34E+07	8005.0	4.05	9.04

hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8116) 523.77 17.58 11.25 75.23 1.45 1.09

OUTFLOW: ID= 1 (6152) 523.77 16.91 12.00 75.23 1.42 1.08

ADD HYD (8122) |
 1 + 2 = 3 |
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0152): 124.37 8.500 11.00 149.20
 + ID2= 2 (6152): 523.77 16.911 12.00 75.23
 ID= 3 (8122): 648.14 24.854 11.75 89.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0158) | Area (ha)= 178.59 Curve Number (CN)= 78.0
 NASHYD (0158) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.94

Unit Hyd Qpeak (cms)= 3.230

PEAK FLOW (cms)= 12.027 (1)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 152.069
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.717

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0156) | Area (ha)= 83.49 Curve Number (CN)= 65.3
 NASHYD (0156) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.62

Unit Hyd Qpeak (cms)= 2.306

PEAK FLOW (cms)= 5.849 (1)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 122.694
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.579

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0154) | Area (ha)= 200.32 Curve Number (CN)= 57.1
 NASHYD (0154) | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.97

Unit Hyd Qpeak (cms)= 3.537

PEAK FLOW (cms)= 9.854 (1)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 106.551
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.503

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8118) |
 1 + 2 = 3 |
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0154): 200.32 9.854 11.25 106.55
 + ID2= 2 (0156): 83.49 5.849 11.00 122.69
 ID= 3 (8118): 283.81 15.612 11.00 111.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6158) |
 IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (1581.0) ----->

Distance	Elevation	Manning
0.00	290.00	0.0400
19.20	288.88	0.0400
48.00	288.26	0.0400
119.99	283.68	0.0400
167.99	282.07	0.0400
196.79	281.32	0.0400
210.79	280.90	0.0400
219.79	280.38	0.0400 /0.0300 Main Channel

220.04	279.65	0.0300	Main Channel
220.79	279.65	0.0300	Main Channel
221.54	279.65	0.0300	Main Channel
221.79	280.39	0.0300 / 0.0400	Main Channel
225.79	280.59	0.0400	
235.19	280.88	0.0400	
254.39	281.50	0.0400	
273.58	282.56	0.0400	
302.38	283.42	0.0400	
359.98	284.03	0.0400	
436.77	289.37	0.0400	
475.17	290.37	0.0400	

268.97 261.92 0.0400

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.50	245.80	120E+05	2.9	1.18	68.31
1.00	246.30	242E+05	8.5	1.70	47.47
1.92	247.22	825E+05	31.7	1.86	43.34
2.84	248.14	248E+06	102.5	2.00	40.39
3.76	249.06	506E+06	247.7	2.37	34.03
4.68	249.98	832E+06	486.8	2.83	28.49
5.60	250.90	121E+07	814.9	3.27	24.71
6.52	251.82	180E+07	1157.1	3.11	25.97
7.44	252.74	253E+07	1840.2	3.52	22.92
8.36	253.66	331E+07	2727.1	3.98	20.25
9.29	254.59	414E+07	3782.6	4.43	18.22
10.21	255.51	499E+07	5010.7	4.86	16.61
11.13	256.43	589E+07	6397.4	5.26	15.33
12.05	257.35	681E+07	7932.8	5.64	14.31
12.97	258.27	777E+07	9621.2	5.99	13.46
13.89	259.19	877E+07	11469.4	6.33	12.74
14.81	260.11	980E+07	13478.4	6.66	12.12
15.73	261.03	109E+08	15651.7	6.97	11.58
16.65	261.95	120E+08	17492.5	7.07	11.42

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.37	280.01	973E+03	0.5	0.86	31.78
0.73	280.38	210E+04	1.5	1.18	23.16
1.30	280.95	149E+05	8.6	0.95	28.91
1.86	281.51	579E+05	40.0	1.13	24.12
2.43	282.08	133E+06	118.5	1.46	18.75
2.99	282.64	236E+06	254.6	1.77	15.46
3.56	283.21	369E+06	450.3	2.00	13.67
4.13	283.78	542E+06	702.7	2.13	12.86
4.69	284.34	767E+06	1125.2	2.41	11.36
5.26	284.91	101E+07	1697.1	2.75	9.93
5.82	285.47	127E+07	2379.1	3.07	8.91
6.39	286.04	155E+07	3171.4	3.36	8.14
6.95	286.60	184E+07	4075.5	3.64	7.53
7.52	287.17	215E+07	5093.1	3.89	7.03
8.09	287.74	247E+07	6226.5	4.13	6.62
8.65	288.30	281E+07	7461.2	4.36	6.28
9.22	288.87	318E+07	8635.1	4.46	6.13
9.78	289.43	357E+07	10143.6	4.67	5.86
10.35	290.00	398E+07	11608.8	4.79	5.71

----- hydrograph -----

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8124)	1110.54	51.43	11.50	105.09	2.18
OUTFLOW : ID= 1 (6160)	1110.54	48.54	12.25	105.09	2.13

----- hydrograph -----

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8118)	283.81	15.61	11.00	111.30	1.42
OUTFLOW : ID= 1 (6158)	283.81	14.89	11.50	111.30	1.41

ADD HYD (8126)

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0160):	318.50	16.698	11.75	152.88
+ ID2= 2 (6160):	1110.54	48.540	12.25	105.09
ID = 3 (8126):	1429.36	65.050	12.25	115.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8128)

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0162):	190.14	14.489	11.00	158.64
+ ID2= 2 (8126):	1429.36	65.050	12.25	115.75
ID = 3 (8128):	1619.50	77.698	12.00	120.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8130)

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8128):	1619.50	77.698	12.00	120.78
+ ID2= 2 (8132):	234.20	30.390	10.00	188.06
ID = 3 (8130):	1853.70	94.554	11.00	129.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8120)

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0158):	178.59	12.027	11.00	152.07
+ ID2= 2 (6158):	283.81	14.891	11.50	111.30
ID = 3 (8120):	462.40	26.734	11.50	127.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8124)

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8120):	462.40	26.734	11.50	127.05
+ ID2= 2 (8122):	648.14	24.854	11.75	89.42
ID = 3 (8124):	1110.54	51.435	11.50	105.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6160)
IN= 2 -> OUT= 1
Routing time step (min)'= 15.00

----- DATA FOR SECTION (1601.0) -----

Distance	Elevation	Manning
0.00	261.02	0.0400
29.89	254.30	0.0400
40.75	252.58	0.0400
62.49	251.30	0.0400
114.11	250.94	0.0400
130.41	248.58	0.0400
141.28	247.50	0.0400
154.86	246.87	0.0400
155.20	246.70	0.0400
160.20	246.30	0.0400 / 0.0300
160.30	245.30	0.0300
165.20	245.30	0.0300
165.30	246.30	0.0300 / 0.0400
168.45	246.96	0.0400
195.62	249.20	0.0400
203.77	250.82	0.0400
225.50	256.78	0.0400
244.52	261.14	0.0400
255.39	261.95	0.0400

ROUTE CHN (6166)
IN= 2 -> OUT= 1
Routing time step (min)'= 15.00

----- DATA FOR SECTION (1661.0) -----

Distance	Elevation	Manning
0.00	248.36	0.0500
41.67	246.98	0.0500
95.82	244.93	0.0500
149.98	242.42	0.0500
191.64	241.88	0.0500
224.97	239.90	0.0500
229.13	239.35	0.0500
233.30	237.77	0.0500
237.47	236.57	0.0500
240.63	235.60	0.0500 / 0.0300
241.63	234.10	0.0300
247.13	234.10	0.0300
247.53	235.60	0.0300 / 0.0500
254.13	236.25	0.0500
266.63	236.77	0.0500
283.29	237.84	0.0500
291.63	238.89	0.0500

329.12 244.66 0.0500
 370.78 249.55 0.0500
 412.44 253.17 0.0500

----- TRAVEL TIME TABLE -----

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.75	234.85	130B+05	6.0	1.36	36.45
1.50	235.60	276E+05	18.1	1.95	25.41
2.25	236.35	544E+05	40.0	2.18	22.69
3.00	237.10	117E+06	80.1	2.04	24.32
3.75	237.85	213E+06	148.9	2.07	23.89
4.50	238.60	334E+06	255.1	2.27	21.85
5.25	239.35	473E+06	395.8	2.49	19.90
6.00	240.10	631E+06	563.8	2.65	18.66
6.75	240.85	824E+06	773.2	2.79	17.77
7.50	241.60	106E+07	1037.9	2.92	16.97
8.26	242.36	135E+07	1326.7	2.92	16.94
9.01	243.11	173E+07	1758.5	3.01	16.44
9.76	243.86	217E+07	2285.9	3.13	15.81
10.51	244.61	265E+07	2905.5	3.26	15.17
11.26	245.36	318E+07	3610.1	3.37	14.67
12.01	246.11	377E+07	4417.4	3.48	14.21
12.76	246.86	441E+07	5348.0	3.60	13.75
13.51	247.61	512E+07	6394.0	3.71	13.34
14.26	248.36	589E+07	7580.3	3.82	12.95

<--- hydrograph ---> <-pipe / channel-->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW: ID= 2 (8130)	1853.70	94.55	11.00	129.28	3.16
OUTFLOW: ID= 1 (6166)	1853.70	90.42	11.50	129.28	3.11

----- ADD HYD (8136) -----

1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6166)			1853.70	90.419	11.50	129.28
+ ID2= 2 (8134)			434.70	53.459	10.00	190.75

ID = 3 (8136)			2288.40	133.764	11.00	140.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ADD HYD (8114) -----

1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8112)			11834.85	561.495	12.00	149.23
+ ID2= 2 (8136)			2288.40	133.764	11.00	140.96

ID = 3 (8114)			14123.25	672.198	12.00	147.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- CALIB -----

STANDHYD (2062)	Area	(ha)=	250.50	Total Imp(%)=	62.00	Dir. Conn.(%)=	39.00
ID= 1 DT=15.0 min	-----						
Surface Area	(ha)=	155.31	95.19				
Dep. Storage	(mm)=	2.00	5.00				
Average Slope	(%)=	0.50	0.50				
Length	(m)=	1292.28	40.00				
Mannings n	=	0.013	0.250				
Max.Eff.Inten.(mm/hr)=	53.00	83.82					
over (min)	15.00	45.00					
Storage Coeff. (min)=	18.83 (ii)	30.31 (ii)					
Unit Hyd. Tpeak (min)=	15.00	45.00					
Unit Hyd. peak (cms)=	0.06	0.03					
			TOTALS				
PEAK FLOW (cms)=	13.94	18.79	31.533 (iii)				
TIME TO PEAK (hrs)=	10.00	10.25	10.00				
RUNOFF VOLUME (mm)=	210.00	189.83	197.70				
TOTAL RAINFALL (mm)=	212.00	212.00	212.00				
RUNOFF COEFFICIENT =	0.99	0.90	0.93				

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 88.3 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB -----

----- CALIB -----

STANDHYD (2061)	Area	(ha)=	25.00	Total Imp(%)=	93.00	Dir. Conn.(%)=	60.00
ID= 1 DT=15.0 min	-----						

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	23.25	1.75
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	408.25	40.00
Mannings n =	0.013	0.250
Max.Eff.Inten.(mm/hr)=	53.00	302.44
over (min)	15.00	30.00
Storage Coeff. (min)=	9.43 (ii)	16.30 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.05
TOTALS		
PEAK FLOW (cms)=	2.21	1.42
TIME TO PEAK (hrs)=	10.00	10.00
RUNOFF VOLUME (mm)=	210.00	205.40
TOTAL RAINFALL (mm)=	212.00	212.00
RUNOFF COEFFICIENT =	0.99	0.97

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 88.3 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- ADD HYD (8144) -----

1	2	3	AREA	QPEAK	TPEAK	R.V.
			(ha)	(cms)	(hrs)	(mm)
ID1= 1 (2061)			25.00	3.621	10.00	208.16
+ ID2= 2 (2062)			250.50	31.533	10.00	197.70

ID = 3 (8144)			275.50	35.154	10.00	198.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- CALIB -----

STANDHYD (2042)	Area	(ha)=	122.90	Total Imp(%)=	60.00	Dir. Conn.(%)=	33.00
ID= 1 DT=15.0 min	-----						

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	73.74	49.16
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	905.17	40.00
Mannings n =	0.013	0.250
Max.Eff.Inten.(mm/hr)=	53.00	84.46
over (min)	15.00	30.00
Storage Coeff. (min)=	15.20 (ii)	26.65 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.07	0.04
TOTALS		
PEAK FLOW (cms)=	5.88	10.14
TIME TO PEAK (hrs)=	10.00	10.25
RUNOFF VOLUME (mm)=	210.00	173.51
TOTAL RAINFALL (mm)=	212.00	212.00
RUNOFF COEFFICIENT =	0.99	0.82

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 78.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB -----

STANDHYD (2041)	Area	(ha)=	201.50	Total Imp(%)=	36.00	Dir. Conn.(%)=	20.00
ID= 1 DT=15.0 min	-----						

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	72.54	128.96
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1159.02	40.00
Mannings n =	0.013	0.250
Max.Eff.Inten.(mm/hr)=	53.00	61.20
over (min)	15.00	45.00
Storage Coeff. (min)=	17.64 (ii)	30.65 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.06	0.03
TOTALS		
PEAK FLOW (cms)=	5.78	18.36
23.198 (iii)		

TIME TO PEAK (hrs)= 10.00 10.50 10.25
 RUNOFF VOLUME (mm)= 210.00 163.07 172.45
 TOTAL RAINFALL (mm)= 212.00 212.00 212.00
 RUNOFF COEFFICIENT = 0.99 0.77 0.81

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
CN* = 78.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8140)
 1 + 2 = 3
 AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (2041): 201.50 23.198 10.25 172.45
 + ID2= 2 (2042): 122.90 15.966 10.00 185.55
 ID = 3 (8140): 324.40 38.738 10.00 177.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6206)
 IN= 2--> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (2061.0) -----
 Distance Elevation Manning
 0.00 253.07 0.0350
 13.98 252.89 0.0350
 27.95 251.99 0.0350
 34.94 252.18 0.0350
 41.93 252.02 0.0350
 73.38 250.35 0.0350
 94.34 247.09 0.0350
 111.81 244.79 0.0350
 115.31 244.24 0.0350
 117.55 244.20 0.0350 / 0.0300 Main Channel
 118.80 243.60 0.0300 Main Channel
 122.50 243.60 0.0300 Main Channel
 122.50 244.60 0.0300 / 0.0350 Main Channel
 146.76 244.74 0.0350
 164.23 247.26 0.0350
 213.15 248.92 0.0350
 241.10 249.51 0.0350
 272.55 250.87 0.0350
 300.50 252.26 0.0350
 345.93 253.67 0.0350

----- TRAVEL TIME TABLE -----
 DEPTH (m) ELEV (m) VOLUME (cu.m.) FLOW RATE (cms) VELOCITY (m/s) TRAV. TIME (min)
 0.30 243.90 3.10E+04 1.4 1.23 36.25
 0.60 244.20 6.76E+04 4.6 1.81 24.64
 1.12 244.72 2.21E+05 17.1 2.07 21.60
 1.64 245.24 7.54E+05 64.3 2.28 19.60
 2.17 245.77 1.39E+06 149.8 2.88 15.50
 2.69 246.29 2.14E+06 273.3 3.43 13.04
 3.21 246.81 2.99E+06 436.1 3.91 11.43
 3.73 247.33 3.95E+06 632.6 4.30 10.40
 4.25 247.85 5.11E+06 836.4 4.39 10.18
 4.77 248.37 6.53E+06 1114.9 4.58 9.76
 5.30 248.90 8.22E+06 1472.1 4.81 9.30
 5.82 249.42 1.02E+07 1872.2 4.91 9.10
 6.34 249.94 1.26E+07 2444.6 5.22 8.56
 6.86 250.46 1.51E+07 3113.1 5.52 8.09
 7.38 250.98 1.80E+07 3843.5 5.74 7.79
 7.90 251.50 2.11E+07 4717.8 6.00 7.45
 8.43 252.03 2.45E+07 5701.4 6.24 7.16
 8.95 252.55 2.84E+07 6727.5 6.36 7.03
 9.47 253.07 3.26E+07 7885.3 6.48 6.90

----- hydrograph ----- <-pipe / channel-->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8140) 324.40 38.74 10.00 177.42 1.36 2.16
 OUTFLOW : ID= 1 (6206) 324.40 36.50 10.50 177.42 1.34 2.15

ADD HYD (8142)
 1 + 2 = 3
 AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (6206): 324.40 36.502 10.50 177.42
 + ID2= 2 (8144): 275.50 35.154 10.00 198.65
 ID = 3 (8142): 599.90 69.147 10.25 187.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8138)
 1 + 2 = 3
 AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (8114): 14123.25 672.198 12.00 147.89
 + ID2= 2 (8142): 599.90 69.147 10.25 187.17
 ID = 3 (8138): 14723.15 709.429 11.50 149.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6208)
 IN= 2--> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (2081.0) -----
 Distance Elevation Manning
 0.00 249.19 0.1000
 73.11 244.13 0.1000
 121.86 243.39 0.1000
 243.71 240.01 0.1000
 353.38 238.75 0.1000
 450.87 235.40 0.1000
 499.61 231.40 0.1000
 511.80 230.96 0.1000
 523.98 230.12 0.1000
 534.50 227.80 0.1000 / 0.0500 Main Channel
 536.50 226.70 0.0500 Main Channel
 546.50 226.70 0.0500 Main Channel
 548.50 227.80 0.0500 / 0.1000 Main Channel
 572.72 230.25 0.1000
 682.40 230.50 0.1000
 804.25 233.95 0.1000
 926.11 235.83 0.1000
 1047.96 240.70 0.1000
 1145.45 243.26 0.1000
 1206.38 247.03 0.1000

----- TRAVEL TIME TABLE -----
 DEPTH (m) ELEV (m) VOLUME (cu.m.) FLOW RATE (cms) VELOCITY (m/s) TRAV. TIME (min)
 0.55 227.25 1.11E+05 2.0 0.33 92.44
 1.10 227.80 2.42E+05 6.5 0.50 61.56
 2.23 228.93 7.00E+05 26.0 0.68 44.91
 3.36 230.06 1.50E+06 60.8 0.74 41.04
 4.49 231.19 4.46E+06 130.8 0.54 56.83
 5.62 232.32 9.25E+06 287.4 0.57 53.66
 6.76 233.46 1.52E+07 522.9 0.63 48.32
 7.89 234.59 2.23E+07 827.2 0.68 44.91
 9.02 235.72 3.12E+07 1219.5 0.72 42.61
 10.15 236.85 4.18E+07 1780.6 0.78 39.11
 11.28 237.98 5.37E+07 2479.2 0.85 36.07
 12.41 239.11 6.69E+07 3282.9 0.90 33.95
 13.54 240.24 8.24E+07 4228.6 0.94 32.47
 14.67 241.37 9.97E+07 5384.6 0.99 30.87
 15.81 242.51 1.19E+08 6698.4 1.03 29.56
 16.94 243.64 1.40E+08 8235.2 1.08 28.25
 18.07 244.77 1.62E+08 10104.2 1.14 26.73
 19.20 245.90 1.85E+08 12251.5 1.21 25.21
 20.33 247.03 2.09E+08 14597.0 1.28 23.89

----- hydrograph ----- <-pipe / channel-->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8138) ***** 709.43 11.50 149.49 7.45 0.66
 OUTFLOW : ID= 1 (6208) ***** 659.64 12.25 149.49 7.27 0.65

CALIB
 STANDHYD (2082) | Area (ha)= 426.60
 ID= 1 DT=15.0 min | Total Imp(%)= 64.00 Dir. Conn.(%)= 37.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 273.02 153.58
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1686.42 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 53.00 90.86
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 22.09 (ii) 33.20 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.06 0.03

TOTALS
 PEAK FLOW (cms)= 22.08 32.24 51.934 (iii)
 TIME TO PEAK (hrs)= 10.00 10.50 10.00

RUNOFF VOLUME (mm)= 210.00 186.64 195.28
 TOTAL RAINFALL (mm)= 212.00 212.00 212.00
 RUNOFF COEFFICIENT = 0.99 0.88 0.92

568.65 236.24 0.0800
 661.06 241.85 0.0800
 703.71 246.04 0.0800

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 85.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (2081) | Area (ha)= 265.70
 ID= 1 DT=15.0 min | Total Imp(%)= 35.00 Dir. Conn.(%)= 20.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	93.00	172.71
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1330.91	40.00
Mannings n	0.013	0.250
Max.Eff.Inten.(mm/hr)	53.00	62.81
over (min)	15.00	45.00
Storage Coeff. (min)	19.16 (ii)	32.05 (iii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.06	0.03

PEAK FLOW (cms)= 7.57 25.15 **TOTALS*
 TIME TO PEAK (hrs)= 10.00 10.50 31.576 (iii)
 RUNOFF VOLUME (mm)= 210.00 177.36 183.89
 TOTAL RAINFALL (mm)= 212.00 212.00 212.00
 RUNOFF COEFFICIENT = 0.99 0.84 0.87

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 85.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8148)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (2081):	265.70	31.576	10.25	183.89
+ ID2= 2 (2082):	426.60	51.934	10.00	195.28
ID = 3 (8148):	692.30	83.086	10.25	190.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8146)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6208):	14723.15	659.640	12.25	149.49
+ ID2= 2 (8148):	692.30	83.086	10.25	190.91
ID = 3 (8146):	15415.45	694.057	12.00	151.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6210) | Routing time step (min)'= 15.00
 IN= 2--> OUT= 1 |

<----- DATA FOR SECTION (2101.0) ----->

Distance	Elevation	Manning
0.00	241.81	0.0800
28.43	241.78	0.0800
85.30	237.04	0.0800
149.27	231.32	0.0800
170.60	230.10	0.0800
255.89	229.37	0.0800
277.22	228.68	0.0800
284.33	228.42	0.0800
291.43	228.16	0.0800
297.76	227.00	0.0800 / 0.0350 Main Channel
298.76	225.00	0.0350 Main Channel
312.76	225.00	0.0350 Main Channel
313.76	227.00	0.0350 / 0.0800 Main Channel
326.97	229.50	0.0800
348.30	230.29	0.0800
376.73	229.76	0.0800
490.46	235.75	0.0800

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.67	225.67	141E+05	6.6	0.69	35.86
1.33	225.33	289E+05	20.4	1.04	23.65
2.00	227.00	443E+05	39.3	1.31	18.81
2.93	227.93	730E+05	77.8	1.57	15.64
3.85	228.85	123E+06	131.1	1.57	15.65
4.78	229.78	228E+06	205.6	1.33	18.51
5.70	230.70	503E+06	356.7	1.05	23.49
6.63	231.63	846E+06	608.7	1.06	23.17
7.55	232.55	123E+07	946.4	1.14	21.65
8.48	233.48	165E+07	1367.0	1.22	20.13
9.41	234.41	211E+07	1871.8	1.31	18.80
10.33	235.33	261E+07	2463.2	1.40	17.65
11.26	236.26	317E+07	2999.4	1.40	17.63
12.18	237.18	384E+07	3848.3	1.48	16.63
13.11	238.11	454E+07	4806.0	1.56	15.75
14.03	239.03	528E+07	5876.9	1.64	14.97
14.96	239.96	605E+07	7062.7	1.72	14.29
15.88	240.88	687E+07	8365.0	1.80	13.68
16.81	241.81	771E+07	9506.9	1.82	13.52

<----- hydrograph -----> <-pipe / channel-->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8146) *****	694.06	12.00	151.35	6.86	1.08
OUTFLOW : ID= 1 (6210) *****	681.04	12.25	151.35	6.82	1.08

CALIB
 NASHYD (0210) | Area (ha)= 218.27 Curve Number (CN)= 91.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.08

Unit Hyd Qpeak (cms)= 3.454
 PEAK FLOW (cms)= 15.594 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 182.915
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.863

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8150)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0210):	218.27	15.594	11.25	182.92
+ ID2= 2 (6210):	15415.45	681.044	12.25	151.35
ID = 3 (8150):	15633.72	695.155	12.25	151.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0218) | Area (ha)= 152.25 Curve Number (CN)= 84.1
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.79

Unit Hyd Qpeak (cms)= 3.278
 PEAK FLOW (cms)= 11.927 (i)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 165.604
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.781

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0216) | Area (ha)= 145.77 Curve Number (CN)= 88.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 0.52

Unit Hyd Qpeak (cms)= 4.768
 PEAK FLOW (cms)= 13.676 (i)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 172.603
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.814

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB		Area (ha)= 56.00	
STANDHYD (2121)		Total Imp(%)= 93.00 Dir. Conn.(%)= 75.00	
ID= 1 DT=15.0 min			
		IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	52.08	3.92	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (k)=	0.50	0.50	
Length (m)=	611.01	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	53.00	188.05	
over (min)=	15.00	30.00	
Storage Coeff. (min)=	12.01 (ii)	20.32 (iii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.08	0.05	
TOTALS			
PEAK FLOW (cms)=	6.15	1.91	8.062 (iii)
TIME TO PEAK (hrs)=	10.00	10.00	
RUNOFF VOLUME (mm)=	210.00	197.96	206.99
TOTAL RAINFALL (mm)=	212.00	212.00	212.00
RUNOFF COEFFICIENT =	0.99	0.93	0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB		Area (ha)= 272.40	
STANDHYD (2122)		Total Imp(%)= 29.00 Dir. Conn.(%)= 23.00	
ID= 1 DT=15.0 min			
		IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	79.00	193.40	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (k)=	0.50	0.50	
Length (m)=	1347.59	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	53.00	54.48	
over (min)=	15.00	45.00	
Storage Coeff. (min)=	19.31 (ii)	32.94 (iii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.06	0.03	
TOTALS			
PEAK FLOW (cms)=	8.91	24.28	31.777 (iii)
TIME TO PEAK (hrs)=	10.00	10.50	10.25
RUNOFF VOLUME (mm)=	210.00	170.90	179.89
TOTAL RAINFALL (mm)=	212.00	212.00	212.00
RUNOFF COEFFICIENT =	0.99	0.81	0.85

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8152)		AREA		QPEAK		TPEAK		R.V.	
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)				
ID1= 1 (2121):	56.00	8.062	10.00	206.99					
+ ID2= 2 (2122):	272.40	31.777	10.25	179.89					

ID = 3 (8152):	328.40	39.312	10.00	184.51					

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CUN (6214)		ROUTING TIME STEP (min)'= 15.00	
IN= 2---- OUT= 1			
----- DATA FOR SECTION (2141.0) -----			
Distance	Elevation	Manning	
0.00	259.29	0.0500	
15.33	258.91	0.0500	
38.33	258.57	0.0500	
65.16	257.83	0.0500	
95.82	253.44	0.0500	
111.15	251.21	0.0500	
130.32	249.67	0.0500	

149.48	248.51	0.0500	
150.66	248.50	0.0500 / 0.0370	Main Channel
150.81	247.50	0.0370	Main Channel
154.66	247.50	0.0370	Main Channel
155.66	248.50	0.0370 / 0.0500	Main Channel
160.98	248.50	0.0500	
187.81	249.78	0.0500	
199.31	252.13	0.0500	
245.30	254.04	0.0500	
264.46	253.99	0.0500	
298.96	255.23	0.0500	
329.62	257.02	0.0500	
379.45	258.82	0.0500	

TRAVEL TIME TABLE					
DEPTH (m)	ELEV (m)	VOLUME (cu. m)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	248.00	6.48E+04	2.1	1.00	52.30
1.00	248.50	1.39E+05	6.4	1.44	36.35
1.51	249.11	5.70E+05	22.6	1.24	42.01
2.21	249.71	1.43E+06	63.6	1.39	37.61
2.82	250.32	2.63E+06	142.9	1.70	30.69
3.43	250.93	4.03E+06	257.2	2.00	26.11
4.04	251.54	5.62E+06	410.4	2.29	22.82
4.64	252.14	7.35E+06	601.3	2.56	20.37
5.25	252.75	9.33E+06	791.0	2.66	19.67
5.86	253.36	1.17E+07	1038.0	2.79	18.74
6.46	253.96	1.44E+07	1344.4	2.93	17.82
7.07	254.57	1.78E+07	1664.1	2.93	17.83
7.68	255.18	2.17E+07	2140.2	3.10	16.87
8.28	255.78	2.59E+07	2722.9	3.30	15.83
8.89	256.39	3.04E+07	3387.1	3.50	14.94
9.50	257.00	3.51E+07	4132.7	3.69	14.16
10.11	257.61	4.02E+07	4913.0	3.83	13.64
10.71	258.21	4.58E+07	5648.3	3.87	13.50
11.32	258.82	5.21E+07	6343.1	3.82	13.68

		<---- hydrograph ---->		<-pipe / channel->			
		AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8152)	328.40	39.31	10.00	184.51	1.85	1.30	
OUTFLOW : ID= 1 (6214)	328.40	34.32	11.00	184.51	1.78	1.28	

CALIB		Area (ha)= 316.95	
STANDHYD (0214)		Total Imp(%)= 25.00 Dir. Conn.(%)= 9.00	
ID= 1 DT=15.0 min			
		IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	79.24	237.71	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (k)=	0.50	0.50	
Length (m)=	1453.62	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	53.00	59.77	
over (min)=	15.00	45.00	
Storage Coeff. (min)=	20.20 (ii)	33.35 (iii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.06	0.03	
TOTALS			
PEAK FLOW (cms)=	4.04	32.61	35.849 (iii)
TIME TO PEAK (hrs)=	10.00	10.50	
RUNOFF VOLUME (mm)=	210.00	164.82	168.88
TOTAL RAINFALL (mm)=	212.00	212.00	212.00
RUNOFF COEFFICIENT =	0.99	0.78	0.80

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 79.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8154)		AREA		QPEAK		TPEAK		R.V.	
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)				
ID1= 1 (0214):	316.95	35.849	10.50	168.88					
+ ID2= 2 (6214):	328.40	34.323	11.00	184.51					

ID = 3 (8154):	645.35	68.345	10.50	176.84					

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8156)

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0216):	145.77	13.676	11.00	172.60
+ ID2= 2 (8154):	645.35	68.345	10.50	176.84
ID = 3 (8156):	791.12	81.502	10.75	176.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6218) |
| ID= 2 ---> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2181.0) ----->

Distance	Elevation	Manning	
0.00	256.38	0.0450	
7.70	254.85	0.0450	
15.40	253.20	0.0450	
19.25	252.29	0.0450	
23.10	251.17	0.0450	
78.94	236.16	0.0450	
80.87	235.80	0.0450	
82.79	235.45	0.0450	
84.57	234.60	0.0450 /0.0300	Main Channel
85.07	233.60	0.0300	Main Channel
88.57	233.60	0.0300	Main Channel
93.57	233.60	0.0300	Main Channel
94.07	234.60	0.0300 /0.0450	Main Channel
96.27	235.13	0.0450	
98.19	235.20	0.0450	
100.12	235.32	0.0450	
138.63	239.95	0.0450	
180.98	244.80	0.0450	
188.69	245.00	0.0450	
190.61	256.00	0.0450	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
1.00	234.60	.181E+05	6.0	0.66	50.64
2.13	235.73	.518E+05	23.8	0.93	36.27
3.25	236.85	.119E+06	60.0	1.02	33.03
4.38	237.98	.217E+06	120.6	1.12	29.99
5.51	239.11	.346E+06	210.5	1.23	27.39
6.63	240.23	.506E+06	334.1	1.33	25.22
7.76	241.36	.697E+06	495.6	1.43	23.44
8.88	242.48	.920E+06	699.8	1.53	21.91
10.01	243.61	1.17E+07	950.8	1.63	20.59
11.14	244.74	1.46E+07	1252.5	1.73	19.45
12.26	245.86	1.78E+07	1619.6	1.83	18.37
13.39	246.99	2.12E+07	2078.5	1.97	17.01
14.52	248.12	2.47E+07	2589.0	2.11	15.88
15.64	249.24	2.82E+07	3149.7	2.25	14.94
16.77	250.37	3.19E+07	3759.8	2.37	14.14
17.89	251.49	3.56E+07	4419.5	2.50	13.44
19.02	252.62	3.95E+07	5126.4	2.61	12.84
20.15	253.75	4.35E+07	5871.6	2.72	12.34
21.27	254.87	4.76E+07	6663.2	2.82	11.89

<----- hydrograph -----> <-pipe / channel->					
AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8156)	791.12	81.50	10.75	176.06	3.65
OUTFLOW : ID= 1 (6218)	791.12	74.98	11.00	176.06	3.54

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0218):	152.25	11.927	11.00	165.60
+ ID2= 2 (6218):	791.12	74.976	11.00	176.06
ID = 3 (8158):	943.37	86.903	11.00	174.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8150):	15633.72	695.155	12.25	151.79
+ ID2= 2 (8158):	943.37	86.903	11.00	174.37
ID = 3 (8151):	16577.09	762.042	12.00	153.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6220) |
| ID= 2 ---> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2201.0) ----->

Distance	Elevation	Manning	
0.00	245.59	0.0600	
5.76	245.45	0.0600	
23.03	242.27	0.0600	
57.58	232.35	0.0600	
86.37	226.18	0.0600	
109.41	224.31	0.0600	
115.17	223.86	0.0600	
120.92	222.78	0.0600	
126.68	221.93	0.0600	
128.06	221.50	0.0600 /0.0300	Main Channel
129.56	220.00	0.0300	Main Channel
141.86	220.00	0.0300	Main Channel
143.06	221.50	0.0300 /0.0600	Main Channel
149.72	223.77	0.0600	
155.47	225.09	0.0600	
253.36	229.21	0.0600	
333.98	227.50	0.0600	
454.90	229.25	0.0600	
500.97	233.11	0.0600	
570.07	237.42	0.0600	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.75	220.75	.262E+05	5.5	0.57	79.44
1.50	221.50	.555E+05	17.5	0.86	52.89
2.44	222.44	.103E+06	42.7	1.14	40.03
3.37	223.37	.171E+06	79.4	1.27	35.98
4.31	224.31	.263E+06	128.1	1.33	34.22
5.25	225.25	.393E+06	191.7	1.34	34.19
6.18	226.18	.594E+06	276.7	1.28	35.77
7.12	227.12	.872E+06	399.9	1.26	36.33
8.06	228.06	1.27E+07	542.7	1.17	38.92
8.99	228.99	1.99E+07	770.0	1.06	43.01
9.93	229.93	2.97E+07	1151.8	1.06	42.95
10.86	230.86	4.00E+07	1652.9	1.13	40.33
11.80	231.80	5.07E+07	2251.8	1.22	37.53
12.74	232.74	6.18E+07	2945.5	1.31	34.97
13.67	233.67	7.33E+07	3726.8	1.39	32.79
14.61	234.61	8.53E+07	4591.0	1.48	30.96
15.55	235.55	9.77E+07	5544.1	1.55	29.37
16.48	236.48	1.11E+08	6586.7	1.63	27.99
17.42	237.42	1.24E+08	7719.2	1.71	26.77

<----- hydrograph -----> <-pipe / channel->					
AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8151)	762.04	12.00	153.08	8.96	1.07
OUTFLOW : ID= 1 (6220)	700.18	12.50	153.08	8.72	1.09

CALIB NASHVD (0220) | Area (ha)= 169.10 Curve Number (CN)= 86.9
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.50

Unit Hyd Qpeak (cms)= 1.923

PEAK FLOW (cms)= 9.727 (i)

TIME TO PEAK (hrs)= 11.750

RUNOFF VOLUME (mm)= 173.713

TOTAL RAINFALL (mm)= 212.000

RUNOFF COEFFICIENT = 0.819

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8160) |
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0220): 169.10 9.727 11.75 173.71
+ ID2= 2 (6220): 16577.09 700.182 12.50 153.08
ID = 3 (8160): 16746.19 709.374 12.50 153.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHVD (0226) | Area (ha)= 237.58 Curve Number (CN)= 88.3
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.05

Unit Hyd Qpeak (cms)= 3.852

PEAK FLOW (cms)= 16.809 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 176.368
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.832

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
STANDHYD (2221) | Area (ha)= 145.20
                  | Total Imp(%)= 36.00 Dir. Conn.(%)= 19.00
                  |-----|
                  | IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 52.27 92.93
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 983.87 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 53.00 65.59
                    over (min)= 15.00 30.00
Storage Coeff. (min)= 15.98 (ii) 28.65 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.07 0.04

PEAK FLOW (cms)= 3.99 14.76
TIME TO PEAK (hrs)= 10.00 10.25
RUNOFF VOLUME (mm)= 210.00 184.67
TOTAL RAINFALL (mm)= 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.87
  
```

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 88.4 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
STANDHYD (2222) | Area (ha)= 74.30
                  | Total Imp(%)= 78.00 Dir. Conn.(%)= 41.00
                  |-----|
                  | IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 57.95 16.35
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 703.80 40.00
Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 53.00 141.33
                    over (min)= 15.00 30.00
Storage Coeff. (min)= 13.07 (ii) 22.59 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04

PEAK FLOW (cms)= 4.45 5.89
TIME TO PEAK (hrs)= 10.00 10.00
RUNOFF VOLUME (mm)= 210.00 198.40
TOTAL RAINFALL (mm)= 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.94
  
```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 88.4 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8164) |
1 + 2 = 3 |
            | AREA QPEAK TPEAK R.V.
            | (ha) (cms) (hrs) (mm)
            |-----|
ID1= 1 (2221): 145.20 18.621 10.00 189.49
+ ID2= 2 (2222): 74.30 10.337 10.00 203.16
*****
ID = 3 (8164): 219.50 28.958 10.00 194.11
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB
STANDHYD (0224) | Area (ha)= 140.45
                  | Total Imp(%)= 34.00 Dir. Conn.(%)= 26.00
                  |-----|
                  | IMPERVIOUS PERVIOUS (i)
  
```

Surface Area (ha)= 47.75 92.70
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 967.64 40.00
 Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 53.00 58.42
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 15.83 (ii) 29.09 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.07 0.04

```

PEAK FLOW (cms)= 5.28 13.09
TIME TO PEAK (hrs)= 10.00 10.25
RUNOFF VOLUME (mm)= 210.00 187.32
TOTAL RAINFALL (mm)= 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.88
  
```

 18.25 (iii)
 10.00
 193.22
 212.00
 0.91

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 91.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8166) |
1 + 2 = 3 |
            | AREA QPEAK TPEAK R.V.
            | (ha) (cms) (hrs) (mm)
            |-----|
ID1= 1 (0224): 140.45 18.253 10.00 193.22
+ ID2= 2 (8164): 219.50 28.958 10.00 194.11
*****
ID = 3 (8166): 359.95 47.211 10.00 193.76
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6226) |
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
  
```

<----- DATA FOR SECTION (2261.0) ----->

Distance	Elevation	Manning
0.00	245.35	0.0600
8.27	242.72	0.0600
16.54	240.01	0.0600
31.02	235.31	0.0600
41.36	233.14	0.0600
90.99	227.73	0.0600
93.06	227.50	0.0600
95.13	227.27	0.0600
95.83	227.00	0.0600
100.83	226.50	0.0600 / 0.0350 Main Channel
101.33	225.50	0.0350 Main Channel
101.83	226.50	0.0350 / 0.0600 Main Channel
105.83	227.00	0.0600
107.54	227.39	0.0600
109.60	227.62	0.0600
111.67	227.86	0.0600
146.83	231.76	0.0600
173.71	234.77	0.0600
186.12	238.23	0.0600
202.67	238.35	0.0600

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	226.00	403E+03	0.0	0.16	330.59
1.00	226.50	.161E+04	0.1	0.26	208.26
1.70	227.20	.173E+05	1.5	0.27	195.72
2.39	227.89	.541E+05	5.9	0.35	152.27
3.09	228.59	.119E+06	16.0	0.43	124.08
3.79	229.29	.212E+06	33.8	0.51	104.65
4.49	229.99	.334E+06	61.2	0.59	90.97
5.18	230.68	.484E+06	99.7	0.66	80.88
5.88	231.38	.663E+06	151.0	0.73	73.13
6.58	232.08	.870E+06	216.5	0.80	66.95
7.27	232.77	.111E+07	297.5	0.87	61.91
7.97	233.47	.137E+07	398.1	0.94	57.29
8.67	234.17	.165E+07	518.9	1.01	53.12
9.36	234.86	.196E+07	658.9	1.08	49.59
10.06	235.56	.238E+07	827.0	1.17	46.02
10.76	236.26	.282E+07	1015.4	1.25	42.96
11.46	236.96	.296E+07	1221.2	1.33	40.41
12.15	237.65	.332E+07	1444.6	1.40	38.26
12.85	238.35	.368E+07	1598.2	1.40	38.42

```

<---- hydrograph ----> <-pipe / channel->
CALIB
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8166) 359.95 47.21 10.00 193.76 4.13 0.55
OUTFLOW: ID= 1 (6226) 359.95 32.72 11.00 193.76 3.75 0.51
  
```

TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 152.445
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.719

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8168) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0226): 237.58 16.809 11.25 176.37
+ ID2= 2 (6226): 359.95 32.730 11.00 193.76
=====
ID = 3 (8168): 597.53 49.479 11.00 186.84
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8162) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8160): 16746.19 709.374 12.50 153.29
+ ID2= 2 (8168): 597.53 49.479 11.00 186.84
=====
ID = 3 (8162): 17343.72 744.520 12.25 154.44
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6228) |
| IN= 2--> OUT= 1 |
-----
Routing time step (min)= 15.00
  
```

```

<----- DATA FOR SECTION (2281.0) ----->
Distance Elevation Manning
0.00 245.00 0.0600
18.60 244.74 0.0600
37.20 242.47 0.0600
93.01 233.71 0.0600
125.57 226.95 0.0600
213.93 221.63 0.0600
218.58 221.15 0.0600
223.23 220.45 0.0600
225.03 220.55 0.0600 / 0.0300 Main Channel
225.53 218.05 0.0300 Main Channel
232.53 218.05 0.0300 Main Channel
240.03 218.05 0.0300 Main Channel
241.03 220.55 0.0300 / 0.0600 Main Channel
246.48 221.00 0.0600
251.13 221.27 0.0600
255.78 221.22 0.0600
325.54 225.25 0.0600
367.40 228.91 0.0600
404.60 231.75 0.0600
460.41 245.00 0.0600
  
```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV. TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
1.25 219.30 .338E+05 16.0 0.86 35.15
2.50 220.55 .696E+05 48.3 1.26 24.02
3.94 221.99 .168E+06 116.0 1.26 24.14
5.38 223.43 .392E+06 247.5 1.15 26.37
6.81 224.86 .743E+06 472.6 1.16 26.20
8.25 226.30 .122E+07 821.6 1.23 24.67
9.59 227.74 .179E+07 1320.1 1.34 22.59
11.13 229.18 .243E+07 1965.7 1.47 20.58
12.57 230.62 .313E+07 2744.2 1.59 19.02
14.01 232.06 .390E+07 3693.4 1.72 17.66
15.44 233.49 .472E+07 4824.1 1.86 16.30
16.88 234.93 .557E+07 6092.4 1.99 15.23
18.32 236.37 .646E+07 7503.3 2.11 14.35
19.76 237.81 .739E+07 9061.2 2.23 13.59
21.20 239.25 .836E+07 10767.6 2.34 12.94
22.64 240.69 .937E+07 12624.8 2.45 12.37
24.07 242.12 1.04E+08 14634.9 2.56 11.87
25.51 243.56 1.15E+08 16762.8 2.65 11.45
26.95 245.00 1.27E+08 18699.9 2.69 11.28
  
```

```

<---- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8162) ***** 744.52 12.25 154.44 7.94 1.21
OUTFLOW: ID= 1 (6228) ***** 718.75 13.00 154.44 7.85 1.21
  
```

```

-----
| CALIB (0228) |
| NASHYD (0228) | Area (ha)= 310.54 Curve Number (CN)= 78.0
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| U.H. Tp(hrs)= 1.12 |
  
```

Unit Hyd Qpeak (cms)= 4.751
 PEAK FLOW (cms)= 19.165 (i)

```

-----
| ADD HYD (8170) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0228): 310.54 19.165 11.25 152.45
+ ID2= 2 (6228): 17343.72 718.748 13.00 154.44
=====
ID = 3 (8170): 17654.26 734.174 12.50 154.41
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| CALIB (2302) |
| STANHYD (2302) | Area (ha)= 126.70
| ID= 1 DT=15.0 min | Total Imp(%)= 23.00 Dir. Conn.(%)= 9.00
  
```

```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 29.14 97.56
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 919.06 40.00
Mannings n = 0.013 0.250
  
```

```

Max.Eff.Inten.(mm/hr)= 53.00 59.79
over (min) 15.00 30.00
Storage Coeff. (min)= 13.34 (ii) 28.49 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.07 0.04
*TOTALS*
PEAK FLOW (cms)= 1.65 14.10 15.593 (iii)
TIME TO PEAK (hrs)= 10.00 10.25 10.00
RUNOFF VOLUME (mm)= 210.00 173.78 177.04
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.82 0.84
  
```

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 84.1 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB (2301) |
| STANHYD (2301) | Area (ha)= 78.80
| ID= 1 DT=15.0 min | Total Imp(%)= 50.00 Dir. Conn.(%)= 19.00
  
```

```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 39.40 39.40
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 724.80 40.00
Mannings n = 0.013 0.250
  
```

```

Max.Eff.Inten.(mm/hr)= 53.00 83.55
over (min) 15.00 30.00
Storage Coeff. (min)= 13.31 (ii) 24.80 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04
*TOTALS*
PEAK FLOW (cms)= 2.19 8.17 10.355 (iii)
TIME TO PEAK (hrs)= 10.00 10.00 10.00
RUNOFF VOLUME (mm)= 210.00 182.95 188.09
TOTAL RAINFALL (mm)= 212.00 212.00 212.00
RUNOFF COEFFICIENT = 0.99 0.86 0.89
  
```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 84.1 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8172) |
| 1 + 2 = 3 |
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
  
```

ID1= 1 (2301): 78.80 10.355 10.00 188.09
 + ID2= 2 (2302): 126.70 15.593 10.00 177.04
 =====
 ID = 3 (8172): 205.50 25.948 10.00 181.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB |
 NASHVD (0232) | Area (ha)= 314.80 Curve Number (CN)= 77.3
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.00

Unit Hyd Opeak (cms)= 5.391

PEAK FLOW (cms)= 20.468 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 150.609
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.710

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8173) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0232): 314.80 20.468 11.25 150.61
 + ID2= 2 (8172): 205.50 25.948 10.00 181.28
 =====
 ID = 3 (8173): 520.30 42.272 11.00 162.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8174) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8170): 17654.26 734.174 12.50 154.41
 + ID2= 2 (8173): 520.30 42.272 11.00 162.72
 =====
 ID = 3 (8174): 18174.56 786.520 12.25 154.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6234) |
 IIR= 2--> OUTF= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (2341.0) -----

Distance	Elevation	Manning
0.00	228.00	0.0550
42.86	223.21	0.0550
85.72	219.56	0.0550
107.15	219.42	0.0550
128.58	219.39	0.0550
214.30	219.30	0.0550
225.02	219.26	0.0550
235.73	219.24	0.0550
255.16	219.20	0.0550 / 0.0350 Main Channel
257.16	217.20	0.0350 Main Channel
303.16	217.20	0.0350 Main Channel
305.16	219.20	0.0350 / 0.0550 Main Channel
310.74	219.28	0.0550
321.45	219.27	0.0550
332.17	219.26	0.0550
395.46	219.29	0.0550
782.20	222.91	0.0550
878.64	227.43	0.0550
964.36	227.80	0.0550
1060.79	228.00	0.0550

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	217.70	.887E+05	7.2	0.31	206.54
1.00	218.20	.179E+06	22.7	0.48	131.74
1.50	218.70	.272E+06	44.5	0.62	101.75
2.00	219.20	.366E+06	71.8	0.75	84.97
2.59	219.79	.993E+06	136.1	0.52	121.59
3.17	220.37	.189E+07	258.6	0.52	121.80
3.76	220.96	.294E+07	434.7	0.56	112.82
4.35	221.55	.415E+07	665.9	0.61	103.88
4.93	222.13	.551E+07	955.0	0.66	96.22
5.52	222.72	.703E+07	1305.1	0.71	89.80
6.11	223.31	.868E+07	1744.9	0.77	82.91
6.69	223.89	.104E+08	2261.4	0.83	76.46
7.28	224.48	.121E+08	2838.0	0.89	71.12
7.87	225.07	.139E+08	3472.9	0.95	66.63

8.45 225.65 .157E+08 4165.0 1.01 62.82
 9.04 226.24 .176E+08 4913.5 1.07 58.54
 9.63 226.83 .194E+08 5717.7 1.12 56.68
 10.21 227.41 .214E+08 6577.1 1.17 54.18
 10.80 228.00 .235E+08 6919.5 1.12 56.61

----- hydrograph ----- <-pipe / channel-->
 CALIB | AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW: ID= 2 (8174) ***** 758.52 12.25 154.65 4.53 0.63
 OUTFLOW: ID= 1 (6234) ***** 654.80 13.75 154.65 4.31 0.61

CALIB |
 STANDHYD (0234) | Area (ha)= 267.16
 ID= 1 DT=15.0 min | Total Imp(%)= 22.00 Dir. Conn.(%)= 8.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 58.78 208.38
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1334.57 40.00
 Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 53.00 46.93
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 19.19 (ii) 33.67 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.06 0.03

PEAK FLOW (cms)= 3.04 22.07 *TOTALS*
 TIME TO PEAK (hrs)= 10.00 10.50 24.494 (iii)
 RUNOFF VOLUME (mm)= 210.00 120.77 127.91
 TOTAL RAINFALL (mm)= 212.00 212.00 212.00
 RUNOFF COEFFICIENT = 0.99 0.57 0.60

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8176) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0234): 267.16 24.494 10.50 127.91
 + ID2= 2 (6234): 18174.56 654.799 13.75 154.65
 =====
 ID = 3 (8176): 18441.72 655.562 13.75 154.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB |
 NASHVD (0238) | Area (ha)= 311.70 Curve Number (CN)= 71.5
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.56

Unit Hyd Opeak (cms)= 3.405

PEAK FLOW (cms)= 14.547 (i)
 TIME TO PEAK (hrs)= 12.000
 RUNOFF VOLUME (mm)= 138.280
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.652

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB |
 NASHVD (0236) | Area (ha)= 494.49 Curve Number (CN)= 72.5
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.24

Unit Hyd Opeak (cms)= 6.830

PEAK FLOW (cms)= 26.862 (i)
 TIME TO PEAK (hrs)= 11.500
 RUNOFF VOLUME (mm)= 140.203
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.661

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ROUTE CHN (6238) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
-----

```

```

----- DATA FOR SECTION (2381.0) -----
Distance Elevation Manning
0.00 241.54 0.0380
602.55 227.00 0.0380
1702.00 224.50 0.0380
1721.25 223.00 0.0380
1725.10 222.60 0.0380
1780.94 222.50 0.0380
1782.87 222.45 0.0380
1784.79 222.40 0.0380 /0.0300 Main Channel
1786.57 221.75 0.0300 Main Channel
1787.07 220.75 0.0300 Main Channel
1787.57 220.75 0.0300 Main Channel
1791.57 220.75 0.0300 Main Channel
1794.07 221.75 0.0300 Main Channel
1798.27 222.00 0.0300 Main Channel
1800.19 222.35 0.0300 /0.0380 Main Channel
1802.12 222.47 0.0380
1840.63 223.00 0.0380
1900.00 225.00 0.0380
1950.00 226.00 0.0380
2242.61 240.00 0.0380

```

```

----- TRAVEL TIME TABLE -----
DEPTH ELLEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.80 221.55 1.01E+05 5.2 1.14 32.36
1.60 222.35 2.98E+05 18.7 1.40 26.36
2.64 223.39 2.49E+06 151.8 1.35 27.33
3.68 224.43 6.13E+06 512.2 1.85 19.94
4.71 225.46 1.52E+07 1065.8 1.55 23.80
5.75 226.50 3.59E+07 2824.9 1.75 21.16
6.79 227.54 6.63E+07 6528.0 2.18 16.93
7.83 228.58 9.94E+07 12158.6 2.71 13.62
8.87 229.62 1.34E+08 19258.8 3.19 11.58
9.91 230.66 1.70E+08 27785.3 3.63 10.19
10.94 231.69 2.07E+08 37667.1 4.03 9.18
11.98 232.73 2.46E+08 48891.7 4.40 8.40
13.02 233.77 2.87E+08 61448.0 4.75 7.78
14.06 234.81 3.29E+08 75332.7 5.08 7.28
15.10 235.85 3.72E+08 90547.8 5.39 6.85
16.14 236.89 4.17E+08 107111.0 5.69 6.49
17.17 237.92 4.64E+08 125111.0 5.98 6.18
18.21 238.96 5.12E+08 144511.0 6.25 5.91
19.25 240.00 5.61E+08 165411.0 6.52 5.67

```

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (0236) 494.49 26.86 11.50 140.20 1.66 1.40
OUTFLOW : ID= 1 (6238) 494.49 26.35 12.00 140.20 1.66 1.40

```

```

----- ADD HYD (8180) -----
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
ID1= 1 (0238): 311.70 14.547 12.00 138.28
+ ID2= 2 (6238): 494.49 26.346 12.00 140.20
=====
ID = 3 (8180): 806.19 40.893 12.00 139.46
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

```

----- ADD HYD (8178) -----
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
ID1= 1 (8176): 18441.72 655.562 13.75 154.26
+ ID2= 2 (8180): 806.19 40.893 12.00 139.46
=====
ID = 3 (8178): 19247.91 697.352 13.75 153.64
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

```

-----
| ROUTE CHN (6240) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
-----

```

```

----- DATA FOR SECTION (2401.0) -----
Distance Elevation Manning
0.00 222.00 0.0450
11.46 221.00 0.0450
208.98 221.00 0.0450
404.04 220.97 0.0450
808.08 220.83 0.0450

```

```

905.60 220.17 0.0450
919.53 219.43 0.0450
933.47 219.22 0.0450
945.26 219.21 0.0450 /0.0300 Main Channel
946.36 217.81 0.0300 Main Channel
975.26 217.81 0.0300 Main Channel
1003.26 217.81 0.0300 Main Channel
1005.26 219.21 0.0300 /0.0450 Main Channel
1017.06 219.28 0.0450
1030.99 219.26 0.0450
1044.92 219.23 0.0450
1058.86 219.23 0.0450
1253.91 219.22 0.0450
1323.57 221.05 0.0450
1379.30 222.00 0.0450

```

```

----- TRAVEL TIME TABLE -----
DEPTH ELLEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.20 218.01 3.07E+05 2.2 0.20 227.52
0.40 218.21 6.17E+05 7.1 0.31 143.98
0.60 218.41 9.28E+05 14.0 0.41 110.33
0.80 218.61 1.24E+06 22.6 0.49 91.46
1.00 218.81 1.55E+06 32.8 0.57 79.15
1.20 219.01 1.88E+06 44.5 0.64 70.38
1.40 219.21 2.20E+06 57.5 0.70 63.77
1.63 219.44 4.13E+06 82.5 0.54 83.52
1.87 219.68 6.32E+06 120.9 0.51 87.03
2.10 219.91 8.58E+06 170.0 0.53 84.11
2.33 220.14 1.09E+07 228.7 0.56 79.62
2.56 220.37 1.34E+07 294.4 0.59 76.03
2.80 220.61 1.62E+07 371.1 0.61 72.75
3.03 220.84 1.92E+07 456.8 0.64 70.18
3.26 221.07 2.51E+07 552.1 0.59 75.74
3.49 221.30 3.33E+07 711.8 0.57 78.07
3.73 221.54 4.17E+07 907.0 0.58 76.62
3.96 221.77 5.02E+07 1134.1 0.61 73.70
4.19 222.00 5.87E+07 1390.9 0.64 70.34

```

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8178) 687.35 13.75 153.64 3.46 0.58
OUTFLOW : ID= 1 (6240) 626.30 14.75 153.64 3.37 0.58

```

```

----- CALIB (0240) -----
| NASHVD (0240) | Area (ha)= 434.37 Curve Number (CN)= 65.5
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----| U.H. Tp(hrs)= 3.60
Unit Hyd Qpeak (cms)= 2.058
PEAK FLOW (cms)= 9.581 (i)
TIME TO PEAK (hrs)= 13.250
RINFLOW VOLUME (mm)= 125.546
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.592
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

```

----- ADD HYD (8182) -----
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm)
ID1= 1 (0240): 434.37 9.581 13.25 125.55
+ ID2= 2 (6240): 19247.91 626.303 14.75 153.64
=====
ID = 3 (8182): 19682.28 635.522 14.75 153.02
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

```

----- CALIB (0242) -----
| NASHVD (0242) | Area (ha)= 657.88 Curve Number (CN)= 70.6
| ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
|-----| U.H. Tp(hrs)= 5.37
Unit Hyd Qpeak (cms)= 2.090
PEAK FLOW (cms)= 10.917 (i)
TIME TO PEAK (hrs)= 14.750
RINFLOW VOLUME (mm)= 136.883
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.646
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ID = 3 (8186) : 2248.87 672.190 15.25 152.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8184) |
| 1 + 2 = 3 |
-----
| ID1= 1 (0244): 657.88 10.917 14.75 136.88
+ ID2= 2 (8182): 19682.28 635.522 14.75 153.02
-----
| ID = 3 (8184): 20340.16 646.439 14.75 152.50
-----

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ROUTE CHN (6244) |
| IN= 2 -> OUT= 1 |
-----
Routing time step (min) = 15.00
-----
|<<----- DATA FOR SECTION (2441.0) ----->>|
| Distance | Elevation | Manning |
| 0.00 | 225.00 | 0.0450 |
| 2.50 | 220.10 | 0.0450 |
| 3.00 | 220.06 | 0.0450 |
| 3.64 | 220.07 | 0.0450 |
| 7.28 | 220.08 | 0.0450 |
| 10.91 | 220.09 | 0.0450 |
| 14.55 | 219.81 | 0.0450 |
| 18.19 | 219.29 | 0.0450 |
| 24.13 | 219.21 | 0.0450 / 0.0300 | Main Channel
| 24.63 | 217.81 | 0.0300 | Main Channel
| 69.13 | 217.81 | 0.0300 | Main Channel
| 114.13 | 217.81 | 0.0300 | Main Channel
| 115.13 | 219.21 | 0.0300 / 0.0450 | Main Channel
| 120.06 | 219.25 | 0.0450 |
| 123.70 | 219.32 | 0.0450 |
| 127.34 | 219.77 | 0.0450 |
| 130.98 | 219.88 | 0.0450 |
| 134.61 | 219.99 | 0.0450 |
| 350.00 | 220.18 | 0.0450 |
| 360.18 | 225.00 | 0.0450 |
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|<----- TRAVEL TIME TABLE ----->>|
| DEPTH | ELEV | VOLUME | FLOW RATE | VELOCITY | TRAV. TIME |
| (m) | (m) | (cu.m.) | (cms) | (m/s) | (min) |
| 0.25 | 218.16 | 1.08E+06 | 9.0 | 0.29 | 200.52 |
| 0.70 | 218.51 | 2.16E+06 | 28.4 | 0.45 | 126.89 |
| 1.05 | 218.86 | 3.25E+06 | 55.7 | 0.59 | 97.27 |
| 1.40 | 219.21 | 4.34E+06 | 89.7 | 0.71 | 80.65 |
| 1.79 | 219.60 | 5.74E+06 | 135.9 | 0.81 | 70.36 |
| 2.17 | 219.98 | 7.26E+06 | 190.8 | 0.90 | 63.38 |
| 2.56 | 220.37 | 1.11E+07 | 264.3 | 0.82 | 69.82 |
| 2.94 | 220.75 | 1.57E+07 | 371.7 | 0.81 | 70.36 |
| 3.33 | 221.14 | 2.03E+07 | 503.4 | 0.85 | 67.30 |
| 3.72 | 221.53 | 2.50E+07 | 656.6 | 0.90 | 63.40 |
| 4.10 | 221.91 | 2.96E+07 | 829.6 | 0.96 | 59.54 |
| 4.49 | 222.30 | 3.43E+07 | 1021.1 | 1.02 | 56.00 |
| 4.87 | 222.68 | 3.90E+07 | 1230.2 | 1.08 | 52.83 |
| 5.26 | 223.07 | 4.37E+07 | 1456.2 | 1.14 | 50.01 |
| 5.65 | 223.46 | 4.84E+07 | 1698.4 | 1.20 | 47.51 |
| 6.03 | 223.84 | 5.31E+07 | 1956.3 | 1.26 | 45.28 |
| 6.42 | 224.23 | 5.79E+07 | 2229.3 | 1.32 | 43.28 |
| 6.80 | 224.61 | 6.26E+07 | 2517.0 | 1.38 | 41.48 |
| 7.19 | 225.00 | 6.74E+07 | 2819.2 | 1.44 | 39.85 |
-----
|<----- hydrograph ----->> |<- pipe / channel ->| | | | |
| AREA | QPEAK | TPEAK | R.V. | MAX DEPTH | MAX VEL |
| (ha) | (cms) | (hrs) | (mm) | (m) | (m/s) |
INFLOW : ID= 2 (8184) ***** 646.44 14.75 152.50 3.69 0.90
OUTFLOW : ID= 1 (6244) ***** 618.27 15.75 152.50 3.62 0.89
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| CALIB (0244) | Area (ha)=1908.71 Curve Number (CN)= 78.3
| NRSHYD | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
| ID= 1 DT=15.0 min | U.H. Tp(hrs)= 2.20
-----
Unit Hyd Qpeak (cms)= 14.812
PEAK FLOW (cms)= 77.149 (i)
TIME TO PEAK (hrs)= 12.000
RUNOFF VOLUME (mm)= 153.983
TOTAL RAINFALL (mm)= 212.000
RUNOFF COEFFICIENT = 0.726
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
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| ADD HYD (8186) |
| 1 + 2 = 3 |
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| ID1= 1 (0244): 1908.71 77.149 12.00 153.98
+ ID2= 2 (6244): 20340.16 618.266 15.75 152.50
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| READ HYD (8190) | AREA (ha)=*****
| DT=15.0 min | TPEAK (hrs)= 12.00
| ID= 3 (8186) | VOLUME (mm)= 158.54
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Fileame: S:\2010 Projects\D-E-E (E10)\WR\M10-487 East Gwillimbury SWM Master Plan\Calsc\Hydrologic Models\Exis
Comments:
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| HOWEVER THE TABLE BELOW AND HYDROGRAPH FILES DO. |
| ACTUAL PEAK FLOW: PEAK ( 500.76 ) + BASE ( 0.00 ) = 500.76 (cms). |
| TIME | FLOW | TIME | FLOW | TIME | FLOW | TIME | FLOW |
| hrs | cms | hrs | cms | hrs | cms | hrs | cms |
0.00 0.000 0.00 0.000 76.00 8.988 152.00 1.007 228.00 0.105 304.00 0.010
0.25 0.000 76.25 8.918 152.25 1.000 228.25 0.104 304.25 0.010
0.50 0.332 76.50 8.848 152.50 0.993 228.50 0.103 304.50 0.010
0.75 0.773 76.75 8.780 152.75 0.985 228.75 0.102 304.75 0.010
1.00 1.201 77.00 8.713 153.00 0.978 229.00 0.101 305.00 0.010
1.25 1.492 77.25 8.646 153.25 0.971 229.25 0.101 305.25 0.010
1.50 1.846 77.50 8.580 153.50 0.964 229.50 0.100 305.50 0.010
1.75 2.243 77.75 8.515 153.75 0.957 229.75 0.099 305.75 0.010
2.00 2.705 78.00 8.451 154.00 0.950 230.00 0.098 306.00 0.010
2.25 3.507 78.25 8.387 154.25 0.943 230.25 0.098 306.25 0.010
2.50 4.499 78.50 8.325 154.50 0.936 230.50 0.097 306.50 0.010
2.75 5.741 78.75 8.263 154.75 0.929 230.75 0.096 306.75 0.009
3.00 7.214 79.00 8.201 155.00 0.923 231.00 0.096 307.00 0.009
3.25 10.063 79.25 8.141 155.25 0.916 231.25 0.095 307.25 0.009
3.50 13.769 79.50 8.080 155.50 0.909 231.50 0.094 307.50 0.009
3.75 18.535 79.75 8.021 155.75 0.903 231.75 0.093 307.75 0.009
4.00 24.207 80.00 7.962 156.00 0.896 232.00 0.093 308.00 0.009
4.25 31.608 80.25 7.904 156.25 0.889 232.25 0.092 308.25 0.009
4.50 40.406 80.50 7.846 156.50 0.883 232.50 0.091 308.50 0.009
4.75 50.718 80.75 7.789 156.75 0.876 232.75 0.091 308.75 0.009
5.00 62.413 81.00 7.732 157.00 0.870 233.00 0.090 309.00 0.009
5.25 74.388 81.25 7.676 157.25 0.864 233.25 0.089 309.25 0.009
5.50 86.853 81.50 7.621 157.50 0.857 233.50 0.089 309.50 0.009
5.75 99.647 81.75 7.566 157.75 0.851 233.75 0.088 309.75 0.009
6.00 113.006 82.00 7.511 158.00 0.845 234.00 0.087 310.00 0.009
6.25 129.808 82.25 7.457 158.25 0.839 234.25 0.087 310.25 0.008
6.50 149.127 82.50 7.403 158.50 0.833 234.50 0.086 310.50 0.008
6.75 167.626 82.75 7.350 158.75 0.826 234.75 0.085 310.75 0.008
7.00 186.671 83.00 7.297 159.00 0.820 235.00 0.085 311.00 0.008
7.25 204.144 83.25 7.245 159.25 0.814 235.25 0.084 311.25 0.008
7.50 221.084 83.50 7.193 159.50 0.808 235.50 0.083 311.50 0.008
7.75 237.378 83.75 7.142 159.75 0.803 235.75 0.083 311.75 0.008
8.00 250.318 84.00 7.091 160.00 0.797 236.00 0.082 312.00 0.008
8.25 262.490 84.25 7.040 160.25 0.791 236.25 0.081 312.25 0.008
8.50 272.937 84.50 6.990 160.50 0.785 236.50 0.081 312.50 0.008
8.75 282.661 84.75 6.940 160.75 0.779 236.75 0.080 312.75 0.008
9.00 292.533 85.00 6.891 161.00 0.774 237.00 0.080 313.00 0.008
9.25 314.661 85.25 6.842 161.25 0.768 237.25 0.079 313.25 0.008
9.50 342.089 85.50 6.794 161.50 0.762 237.50 0.078 313.50 0.008
9.75 367.676 85.75 6.745 161.75 0.757 237.75 0.078 313.75 0.008
10.00 394.096 86.00 6.698 162.00 0.751 238.00 0.077 314.00 0.008
10.25 416.533 86.25 6.650 162.25 0.746 238.25 0.077 314.25 0.007
10.50 438.008 86.50 6.603 162.50 0.740 238.50 0.076 314.50 0.007
10.75 456.344 86.75 6.556 162.75 0.735 238.75 0.076 314.75 0.007
11.00 474.685 87.00 6.510 163.00 0.729 239.00 0.075 315.00 0.007
11.25 485.126 87.25 6.464 163.25 0.724 239.25 0.074 315.25 0.007
11.50 492.251 87.50 6.418 163.50 0.719 239.50 0.074 315.50 0.007
11.75 496.660 87.75 6.373 163.75 0.713 239.75 0.073 315.75 0.007
12.00 500.763 88.00 6.328 164.00 0.708 240.00 0.073 316.00 0.007
12.25 500.059 88.25 6.283 164.25 0.703 240.25 0.072 316.25 0.007
12.50 496.428 88.50 6.239 164.50 0.698 240.50 0.072 316.50 0.007
12.75 490.031 88.75 6.195 164.75 0.693 240.75 0.071 316.75 0.007
13.00 483.539 89.00 6.151 165.00 0.688 241.00 0.071 317.00 0.007
13.25 476.896 89.25 6.108 165.25 0.683 241.25 0.070 317.25 0.007
13.50 470.127 89.50 6.064 165.50 0.678 241.50 0.069 317.50 0.007
13.75 463.287 89.75 6.022 165.75 0.673 241.75 0.069 317.75 0.007
14.00 456.319 90.00 5.979 166.00 0.668 242.00 0.068 318.00 0.007
14.25 449.220 90.25 5.937 166.25 0.663 242.25 0.068 318.25 0.007
14.50 442.202 90.50 5.895 166.50 0.658 242.50 0.067 318.50 0.007
14.75 435.404 90.75 5.854 166.75 0.653 242.75 0.067 318.75 0.007
15.00 428.881 91.00 5.812 167.00 0.648 243.00 0.066 319.00 0.006
15.25 422.654 91.25 5.771 167.25 0.643 243.25 0.066 319.25 0.006
15.50 416.681 91.50 5.731 167.50 0.639 243.50 0.065 319.50 0.006
15.75 410.932 91.75 5.690 167.75 0.634 243.75 0.065 319.75 0.006
16.00 405.390 92.00 5.650 168.00 0.629 244.00 0.064 320.00 0.006
16.25 400.041 92.25 5.611 168.25 0.625 244.25 0.064 320.25 0.006
16.50 394.877 92.50 5.571 168.50 0.620 244.50 0.063 320.50 0.006
16.75 389.887 92.75 5.532 168.75 0.616 244.75 0.063 320.75 0.006
17.00 385.063 93.00 5.493 169.00 0.611 245.00 0.062 321.00 0.006
17.25 380.402 93.25 5.454 169.25 0.607 245.25 0.062 321.25 0.006
17.50 375.903 93.50 5.416 169.50 0.602 245.50 0.062 321.50 0.006
17.75 371.558 93.75 5.378 169.75 0.598 245.75 0.061 321.75 0.006
18.00 367.360 94.00 5.340 170.00 0.593 246.00 0.061 322.00 0.006
18.25 363.301 94.25 5.302 170.25 0.589 246.25 0.060 322.25 0.006
18.50 359.374 94.50 5.265 170.50 0.585 246.50 0.060 322.50 0.006
18.75 355.576 94.75 5.228 170.75 0.580 246.75 0.059 322.75 0.006
19.00 351.870 95.00 5.191 171.00 0.576 247.00 0.059 323.00 0.006
19.25 348.266 95.25 5.154 171.25 0.572 247.25 0.058 323.25 0.006
19.50 344.551 95.50 5.118 171.50 0.568 247.50 0.058 323.50 0.006
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67.25	15.765	143.25	1.301	219.25	0.136	295.25	0.013	371.25	0.001
67.50	14.723	143.50	1.291	219.50	0.135	295.50	0.013	371.50	0.001
67.75	13.932	143.75	1.282	219.75	0.134	295.75	0.013	371.75	0.001
68.00	13.324	144.00	1.272	220.00	0.133	296.00	0.013	372.00	0.001
68.25	12.849	144.25	1.263	220.25	0.132	296.25	0.013	372.25	0.001
68.50	12.473	144.50	1.254	220.50	0.131	296.50	0.013	372.50	0.001
68.75	12.164	144.75	1.245	220.75	0.130	296.75	0.013	372.75	0.001
69.00	11.910	145.00	1.236	221.00	0.129	297.00	0.013	373.00	0.001
69.25	11.694	145.25	1.227	221.25	0.128	297.25	0.013	373.25	0.001
69.50	11.507	145.50	1.218	221.50	0.127	297.50	0.013	373.50	0.001
69.75	11.341	145.75	1.209	221.75	0.126	297.75	0.012	373.75	0.001
70.00	11.192	146.00	1.200	222.00	0.125	298.00	0.012	374.00	0.001
70.25	11.055	146.25	1.192	222.25	0.124	298.25	0.012	374.25	0.001
70.50	10.928	146.50	1.183	222.50	0.124	298.50	0.012	374.50	0.001
70.75	10.809	146.75	1.174	222.75	0.123	298.75	0.012	374.75	0.001
71.00	10.695	147.00	1.166	223.00	0.122	299.00	0.012	375.00	0.001
71.25	10.587	147.25	1.157	223.25	0.121	299.25	0.012	375.25	0.001
71.50	10.484	147.50	1.149	223.50	0.120	299.50	0.012	375.50	0.001
71.75	10.383	147.75	1.141	223.75	0.119	299.75	0.012	375.75	0.001
72.00	10.286	148.00	1.132	224.00	0.118	300.00	0.012	376.00	0.001
72.25	10.192	148.25	1.124	224.25	0.117	300.25	0.012	376.25	0.001
72.50	10.100	148.50	1.116	224.50	0.116	300.50	0.011	376.50	0.001
72.75	10.010	148.75	1.108	224.75	0.115	300.75	0.011	376.75	0.001
73.00	9.922	149.00	1.100	225.00	0.115	301.00	0.011	377.00	0.001
73.25	9.837	149.25	1.092	225.25	0.114	301.25	0.011	377.25	0.001
73.50	9.752	149.50	1.084	225.50	0.113	301.50	0.011	377.50	0.001
73.75	9.670	149.75	1.076	225.75	0.112	301.75	0.011	377.75	0.001
74.00	9.589	150.00	1.068	226.00	0.111	302.00	0.011	378.00	0.001
74.25	9.509	150.25	1.060	226.25	0.110	302.25	0.011	378.25	0.001
74.50	9.431	150.50	1.053	226.50	0.109	302.50	0.011	378.50	0.001
74.75	9.354	150.75	1.045	226.75	0.109	302.75	0.011	378.75	0.001
75.00	9.279	151.00	1.037	227.00	0.108	303.00	0.011	379.00	0.001
75.25	9.204	151.25	1.030	227.25	0.107	303.25	0.011	379.25	0.001
75.50	9.131	151.50	1.022	227.50	0.106	303.50	0.010		
75.75	9.059	151.75	1.015	227.75	0.105	303.75	0.010		

ADD HYD (8188)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID= 1	(8186): 22248.87	672.190	15.25	152.62
+ ID= 2	(8190): 1908.71	77.149	12.00	153.98
ID = 3	(8188): 24157.57	730.927	15.00	152.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB				
NASHYD (4462)				
ID= 1 DT=15.0 min	Area	(ha)=1366.10	Curve Number	(CN)= 78.4
	Ia	(mm)= 5.00	# of Linear Res.(N)=	1.50
	U.H. Tp(hrs)=	4.22		
Unit Hyd Qpeak	(cms)=	5.524		
PEAK FLOW	(cms)=	31.915 (i)		
TIME TO PEAK	(hrs)=	13.500		
RUNOFF VOLUME	(mm)=	154.517		
TOTAL RAINFALL	(mm)=	212.000		
RUNOFF COEFFICIENT	=	0.729		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB				
STANHYD (4461)				
ID= 1 DT=15.0 min	Area	(ha)= 108.90	Curve Number	(CN)= 70.2
	Total Imp(%)=	50.00	Dir. Conn.(%)=	30.00
	IMPERVIOUS	PERVIOUS (i)		
Surface Area	(ha)=	54.45		
Dep. Storage	(mm)=	2.00		
Average Slope	(%)=	0.50		
Length	(m)=	852.06		
Mannings n	=	0.013		
Max.Eff.Inten.(mm/hr)=	53.00	69.43		
over (min)	15.00	30.00		
Storage Coeff. (min)=	14.66 (ii)	27.04 (ii)		
Unit Hyd. Tpeak (min)=	15.00	30.00		
Unit Hyd. peak (cms)=	0.07	0.04		
			TOTALS	
PEAK FLOW	(cms)=	4.75	9.19	13.863 (iii)
TIME TO PEAK	(hrs)=	10.00	10.25	
RUNOFF VOLUME	(mm)=	210.00	167.34	180.14
TOTAL RAINFALL	(mm)=	212.00	212.00	
RUNOFF COEFFICIENT	=	0.99	0.79	0.85

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 78.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8192)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID= 1	(4461): 108.90	13.863	10.00	180.14
+ ID= 2	(4462): 1366.10	31.915	13.50	154.52
ID = 3	(8192): 1475.00	37.049	11.00	156.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8194)				
1 + 2 = 3				
	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID= 1	(8188): 24157.57	730.927	15.00	152.73
+ ID= 2	(8192): 1475.00	37.049	11.00	156.41
ID = 3	(8194): 25632.58	762.090	15.00	152.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB				
NASHYD (0442)				
ID= 1 DT=15.0 min	Area	(ha)= 117.26	Curve Number	(CN)= 62.6
	Ia	(mm)= 5.00	# of Linear Res.(N)=	1.50
	U.H. Tp(hrs)=	1.17		
Unit Hyd Qpeak	(cms)=	1.705		
PEAK FLOW	(cms)=	5.668 (i)		
TIME TO PEAK	(hrs)=	11.500		
RUNOFF VOLUME	(mm)=	118.479		
TOTAL RAINFALL	(mm)=	212.000		
RUNOFF COEFFICIENT	=	0.559		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB				
NASHYD (0440)				
ID= 1 DT=15.0 min	Area	(ha)= 226.35	Curve Number	(CN)= 77.8
	Ia	(mm)= 5.00	# of Linear Res.(N)=	1.50
	U.H. Tp(hrs)=	1.11		
Unit Hyd Qpeak	(cms)=	3.481		
PEAK FLOW	(cms)=	13.978 (i)		
TIME TO PEAK	(hrs)=	11.250		
RUNOFF VOLUME	(mm)=	151.978		
TOTAL RAINFALL	(mm)=	212.000		
RUNOFF COEFFICIENT	=	0.717		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB				
NASHYD (0438)				
ID= 1 DT=15.0 min	Area	(ha)= 130.70	Curve Number	(CN)= 70.2
	Ia	(mm)= 5.00	# of Linear Res.(N)=	1.50
	U.H. Tp(hrs)=	0.60		
Unit Hyd Qpeak	(cms)=	3.738		
PEAK FLOW	(cms)=	9.909 (i)		
TIME TO PEAK	(hrs)=	11.000		
RUNOFF VOLUME	(mm)=	133.127		
TOTAL RAINFALL	(mm)=	212.000		
RUNOFF COEFFICIENT	=	0.628		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB				
NASHYD (0436)				
ID= 1 DT=15.0 min	Area	(ha)= 187.51	Curve Number	(CN)= 73.8
	Ia	(mm)= 5.00	# of Linear Res.(N)=	1.50
	U.H. Tp(hrs)=	0.73		
Unit Hyd Qpeak	(cms)=	4.391		
PEAK FLOW	(cms)=	13.681 (i)		
TIME TO PEAK	(hrs)=	11.000		
RUNOFF VOLUME	(mm)=	161.839		
TOTAL RAINFALL	(mm)=	212.000		
RUNOFF COEFFICIENT	=	0.669		

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0434)			
Area	(ha)	= 56.64	Curve Number (CN)= 66.0
Ia	(mm)	= 5.00	# of Linear Res.(N)= 1.50
U.H. Tp	(hrs)	= 0.52	

Unit Hyd Qpeak (cms) = 1.867

PEAK FLOW (cms) = 4.268 (1)
 TIME TO PEAK (hrs) = 11.000
 RUNOFF VOLUME (mm) = 123.411
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.582

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8206)				
1	2	3		
ID= 1 (0434)	Area (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
+ ID2= 2 (0438)	56.64	4.268	11.00	123.41
	187.51	13.681	11.00	141.84
ID = 3 (8206)	244.15	17.949	11.00	137.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6438)	
IN= 2	OUT= 1
Routing time step (min) = 15.00	

----- DATA FOR SECTION (4381.0) -----

Distance	Elevation	Manning
0.00	256.00	0.0600
9.23	255.50	0.0600
27.69	255.00	0.0600
50.77	254.90	0.0600
64.62	254.85	0.0600
78.47	252.26	0.0600
96.93	249.44	0.0600
129.24	245.94	0.0600
145.40	245.76	0.0600
146.20	245.80	0.0600 / 0.0350
146.70	245.30	0.0350
147.70	245.30	0.0350
148.20	245.80	0.0350 / 0.0600
150.01	245.78	0.0600
152.32	245.79	0.0600
163.86	246.72	0.0600
175.40	249.07	0.0600
186.94	253.15	0.0600
196.17	255.38	0.0600
228.48	255.55	0.0600

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.50	245.80	.221E+04	0.9	1.02	40.44
1.01	246.31	.352E+05	14.0	0.99	41.77
1.53	246.83	.841E+05	47.3	1.39	29.61
2.04	247.34	.144E+06	102.0	1.76	23.49
2.55	247.85	.213E+06	177.3	2.07	19.99
3.07	248.37	.291E+06	274.4	2.34	17.66
3.58	248.88	.378E+06	394.6	2.58	15.97
4.09	249.39	.474E+06	541.4	2.83	14.60
4.61	249.91	.578E+06	719.9	3.08	13.38
5.12	250.42	.688E+06	923.8	3.33	12.40
5.63	250.93	.803E+06	1153.3	3.56	11.61
6.14	251.44	.925E+06	1408.6	3.77	10.95
6.66	251.96	.105E+07	1690.6	3.98	10.38
7.17	252.47	.119E+07	2002.3	4.18	9.88
7.68	252.98	.133E+07	2345.4	4.38	9.43
8.20	253.50	.147E+07	2710.1	4.56	9.05
8.71	254.01	.162E+07	3101.0	4.73	8.72
9.22	254.52	.178E+07	3521.8	4.90	8.43
9.74	255.04	.195E+07	3467.6	4.39	9.39

----- hydrograph ----- <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8206)	244.15	17.95	11.00	137.56	1.07	1.02
OUTFLOW: ID= 1 (6438)	244.15	16.32	11.50	137.56	1.05	1.01

ADD HYD (8208)				
1	2	3		
Area	QPEAK	TPEAK	R.V.	

	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0438)	130.70	9.909	11.00	133.13
+ ID2= 2 (6438)	244.15	16.323	11.50	137.56
ID = 3 (8208)	374.85	25.529	11.25	136.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8210)				
1	2	3		
Area	QPEAK	TPEAK	R.V.	
ID1= 1 (0440)	226.35	13.978	11.25	151.98
+ ID2= 2 (8208)	374.85	25.529	11.25	136.02
ID = 3 (8210)	601.20	39.507	11.25	142.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0432)			
Area	(ha)	= 114.21	Curve Number (CN)= 72.1
Ia	(mm)	= 5.00	# of Linear Res.(N)= 1.50
U.H. Tp	(hrs)	= 1.21	

Unit Hyd Qpeak (cms) = 1.617

PEAK FLOW (cms) = 6.257 (1)
 TIME TO PEAK (hrs) = 11.500
 RUNOFF VOLUME (mm) = 139.271
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.657

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0430)			
Area	(ha)	= 111.64	Curve Number (CN)= 73.5
Ia	(mm)	= 5.00	# of Linear Res.(N)= 1.50
U.H. Tp	(hrs)	= 0.52	

Unit Hyd Qpeak (cms) = 3.687

PEAK FLOW (cms) = 9.236 (1)
 TIME TO PEAK (hrs) = 11.000
 RUNOFF VOLUME (mm) = 139.631
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.659

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0428)			
Area	(ha)	= 50.53	Curve Number (CN)= 70.1
Ia	(mm)	= 5.00	# of Linear Res.(N)= 1.50
U.H. Tp	(hrs)	= 0.50	

Unit Hyd Qpeak (cms) = 1.725

PEAK FLOW (cms) = 4.056 (1)
 TIME TO PEAK (hrs) = 11.000
 RUNOFF VOLUME (mm) = 132.025
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.623

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0426)			
Area	(ha)	= 247.17	Curve Number (CN)= 78.0
Ia	(mm)	= 5.00	# of Linear Res.(N)= 1.50
U.H. Tp	(hrs)	= 0.98	

Unit Hyd Qpeak (cms) = 4.315

PEAK FLOW (cms) = 16.360 (1)
 TIME TO PEAK (hrs) = 11.250
 RUNOFF VOLUME (mm) = 152.157
 TOTAL RAINFALL (mm) = 212.000
 RUNOFF COEFFICIENT = 0.718

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0424)			
Area	(ha)	= 49.57	Curve Number (CN)= 77.4
Ia	(mm)	= 5.00	# of Linear Res.(N)= 1.50
U.H. Tp	(hrs)	= 0.37	

Unit Hyd Qpeak (cms)= 2.293
 PEAK FLOW (cms)= 4.565 (1)
 TIME TO PEAK (hrs)= 11.000
 RUNOFF VOLUME (mm)= 145.658
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.687

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8198)
1 + 2 = 3
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0424): 49.57 4.565 11.00 145.66
+ ID2= 2 (0426): 247.17 16.360 11.25 152.16
=====
ID = 3 (8198): 296.74 20.886 11.00 151.07
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6428)
IN= 2 -> OUT= 1
Routing time step (min)'= 15.00
  
```

```

<----- DATA FOR SECTION (4281.0) ----->
Distance Elevation Manning
0.00 246.54 0.0900
8.80 246.29 0.0900
30.78 246.03 0.0900
41.78 243.97 0.0900
63.77 237.16 0.0900
85.76 232.53 0.0900
96.75 230.16 0.0900
109.94 228.40 0.0900
112.14 228.28 0.0900 / 0.0450 Main Channel
112.84 228.00 0.0450 Main Channel
113.34 227.50 0.0450 Main Channel
114.34 227.50 0.0450 Main Channel
114.84 228.00 0.0450 Main Channel
116.54 228.29 0.0450 / 0.0900 Main Channel
118.74 228.40 0.0900
120.94 228.52 0.0900
149.52 229.51 0.0900
169.31 230.69 0.0900
186.00 234.27 0.0900
217.69 243.56 0.0900
  
```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.78 228.28 .205E+04 1.6 0.97 21.60
1.58 229.08 .202E+05 16.6 1.02 20.37
2.39 229.89 .665E+05 64.8 1.22 17.11
3.19 230.69 .133E+06 158.1 1.49 14.05
4.00 231.50 .213E+06 308.6 1.82 11.48
4.80 232.30 .299E+06 504.9 2.11 9.89
5.61 233.11 .394E+06 746.6 2.37 8.79
6.41 233.91 .496E+06 1034.5 2.61 7.99
7.21 234.71 .605E+06 1373.3 2.84 7.35
8.02 235.52 .722E+06 1763.3 3.06 6.82
8.82 236.32 .844E+06 2202.0 3.27 6.39
9.63 237.13 .974E+06 2690.5 3.46 6.03
10.43 237.93 .111E+07 3239.5 3.66 5.71
11.23 238.73 .125E+07 3839.0 3.84 5.43
12.04 239.54 .140E+07 4489.1 4.03 5.18
12.84 240.34 .155E+07 5190.5 4.20 4.97
13.65 241.15 .170E+07 5943.8 4.36 4.78
14.45 241.95 .187E+07 6749.8 4.53 4.61
15.26 242.76 .204E+07 7609.4 4.68 4.46
  
```

```

<----- hydrograph -----> <-pipe / channel->
AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8198) 296.74 20.89 11.00 151.07 1.66 1.04
OUTFLOW : ID= 1 (6428) 296.74 20.20 11.25 151.07 1.64 1.04
  
```

```

ADD HYD (8200)
1 + 2 = 3
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0428): 50.53 4.056 11.00 132.02
+ ID2= 2 (6428): 296.74 20.204 11.25 151.07
=====
ID = 3 (8200): 347.27 24.019 11.25 148.30
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8202)
1 + 2 = 3
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0430): 111.64 9.236 11.00 139.63
+ ID2= 2 (8200): 347.27 24.019 11.25 148.30
=====
ID = 3 (8202): 458.91 32.909 11.00 146.19
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6432)
IN= 2 -> OUT= 1
Routing time step (min)'= 15.00
  
```

```

<----- DATA FOR SECTION (4321.0) ----->
Distance Elevation Manning
0.00 221.00 0.0600
15.29 219.85 0.0600
45.86 219.86 0.0600
76.44 219.96 0.0600
110.84 220.23 0.0600
112.00 220.15 0.0600
114.66 220.09 0.0600
118.48 219.73 0.0600
120.80 219.70 0.0600 / 0.0350 Main Channel
121.30 219.20 0.0350 Main Channel
122.30 219.20 0.0350 Main Channel
122.80 219.70 0.0350 / 0.0600 Main Channel
129.95 220.11 0.0600
175.81 220.24 0.0600
214.03 220.55 0.0600
252.25 220.61 0.0600
290.47 220.70 0.0600
328.69 221.04 0.0600
347.80 221.38 0.0600
378.37 221.04 0.0600
  
```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.08 219.28 .185E+03 0.0 0.23 149.38
0.17 219.37 .399E+03 0.1 0.34 100.62
0.25 219.45 .641E+03 0.1 0.42 80.98
0.33 219.53 .911E+03 0.2 0.49 69.79
0.42 219.62 .121E+04 0.3 0.55 62.35
0.50 219.70 .154E+04 0.5 0.68 56.91
0.61 219.81 .267E+04 0.7 0.56 61.28
0.71 219.91 .401E+04 1.3 0.30 112.96
0.82 220.02 .549E+04 3.4 0.28 122.71
0.92 220.12 .712E+04 6.8 0.31 109.98
1.03 220.23 .974E+04 12.2 0.33 102.05
1.14 220.34 .122E+05 21.3 0.39 87.59
1.24 220.44 .153E+05 33.0 0.44 77.01
1.35 220.55 .196E+05 47.2 0.49 69.22
1.46 220.66 .248E+05 62.3 0.51 66.48
1.56 220.76 .310E+05 83.4 0.55 62.06
1.67 220.87 .376E+05 108.8 0.59 57.56
1.77 220.97 .444E+05 137.7 0.64 53.77
1.88 221.08 .517E+05 163.6 0.65 52.70
  
```

```

<----- hydrograph -----> <-pipe / channel->
AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8202) 458.91 32.91 11.00 146.19 1.24 0.44
OUTFLOW : ID= 1 (6432) 458.91 27.86 12.00 146.19 1.20 0.42
  
```

```

ADD HYD (8204)
1 + 2 = 3
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0432): 114.21 6.257 11.50 139.27
+ ID2= 2 (6432): 458.91 27.859 12.00 146.19
=====
ID = 3 (8204): 573.12 34.014 12.00 144.81
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8212)
1 + 2 = 3
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8204): 573.12 34.014 12.00 144.81
+ ID2= 2 (8210): 601.20 39.507 11.25 142.03
=====
ID = 3 (8212): 1174.32 72.704 11.50 143.39
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN= (6442) |
 IN= 2--> OUF= 1 | Routing time step (min)'= 15.00

----- DATA FOR SECTION (4421.0) -----

Distance	Elevation	Manning
0.00	221.00	0.0350
26.73	220.80	0.0350
53.47	220.60	0.0350
66.83	220.40	0.0350
80.20	220.20	0.0350
347.52	220.00	0.0350
354.21	219.59	0.0350
360.89	219.50	0.0350
367.57	219.08	0.0350
368.76	219.00	0.0350 /0.0300 Main Channel
369.26	218.50	0.0300 Main Channel
374.26	218.50	0.0300 Main Channel
374.76	219.00	0.0300 /0.0350 Main Channel
380.94	220.33	0.0350
387.62	219.62	0.0350
394.31	219.23	0.0350
454.45	219.07	0.0350
514.60	219.09	0.0350
660.00	219.19	0.0350
661.63	221.00	0.0350

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.12	218.62	.100E+04	0.2	0.28	92.89
0.25	218.75	.205E+04	0.6	0.43	60.09
0.37	218.87	.315E+04	1.1	0.55	46.99
0.50	219.00	.430E+04	1.8	0.66	39.67
0.63	219.13	.541E+05	3.4	0.37	69.99
0.77	219.27	.652E+05	13.7	0.33	79.14
0.90	219.40	.124E+06	35.5	0.45	58.02
1.03	219.53	.183E+06	65.7	0.56	46.45
1.17	219.67	.245E+06	103.8	0.66	39.30
1.30	219.80	.307E+06	149.3	0.76	34.30
1.43	219.93	.371E+06	201.6	0.85	30.65
1.57	220.07	.440E+06	256.5	0.91	28.56
1.70	220.20	.541E+06	325.7	0.94	27.70
1.83	220.33	.653E+06	415.2	0.98	26.62
1.97	220.47	.787E+06	520.3	1.03	25.21
2.10	220.60	.913E+06	638.5	1.09	23.82
2.23	220.73	.104E+07	767.2	1.15	22.62
2.37	220.87	.117E+07	908.0	1.21	21.54
2.50	221.00	.131E+07	1060.7	1.27	20.57

----- Hydrograph ----- <-pipe / channel->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8212)	1174.32	72.70	11.50	143.39	1.06	0.58
OUTFLOW : ID= 1 (6442)	1174.32	69.52	12.00	143.39	1.05	0.57

ADD HYD (8214) |
 1 + 2 = 3 |

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0442):	117.26	5.668	11.50	118.48
+ ID2= 2 (6442):	1174.32	69.524	12.00	143.39
ID = 3 (8214):	1291.58	75.109	12.00	141.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8196) |
 1 + 2 = 3 |

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8194):	25632.58	762.090	15.00	152.94
+ ID2= 2 (8214):	1291.58	75.109	12.00	141.12
ID = 3 (8196):	26924.15	799.848	14.75	152.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB |
 NASHVD (0444) | Area (ha)= 221.65 Curve Number (CN)= 75.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res.(N)= 1.50
 U.H. Tp(hrs)= 1.03

Unit Hyd Qpeak (cms)= 3.656

PEAK FLOW (cms)= 13.744 (i)
 TIME TO PEAK (hrs)= 11.250
 RUNOFF VOLUME (mm)= 145.486
 TOTAL RAINFALL (mm)= 212.000
 RUNOFF COEFFICIENT = 0.686

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8216) |
 1 + 2 = 3 |

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0444):	221.65	13.744	11.25	145.49
+ ID2= 2 (8196):	26924.15	799.848	14.75	152.38
ID = 3 (8216):	27145.80	805.767	14.50	152.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

APPENDIX D
Future Conditions Hydrologic Input Parameters

APPENDIX E
Unit Flow And Unit Storage Calculations

Catchment No.	Subwatershed	TOTAL AREA (ha)	TIMP (%)	Reservoir ID	Storm Event	Rating Curve	
						Discharge (m ³ /s)	Active Storage (ha-m)
Mount Albert Settlement Area							
3B	Black River	280.1	22%	525	25 mm Storm 1:2 Year Storm 1:5 Year Storm 1:10 Year Storm 1:25 Year Storm 1:50 Year Storm 1:100 Year Storm	0.95 5.43 10.11 14.00 18.74 23.52 27.39	2.2068 2.3823 3.7195 4.7295 6.0462 7.3268 8.5786
4B	Black River	92.3	44%	526	25 mm Storm 1:2 Year Storm 1:5 Year Storm 1:10 Year Storm 1:25 Year Storm 1:50 Year Storm 1:100 Year Storm	0.31 1.79 3.33 4.62 6.18 7.75 9.03	1.1099 1.2815 1.6636 1.9904 2.3852 2.7331 3.0736
TOTAL		372.4					
Queensville Settlement Area							
1M	Maskingonge River	487.7	45%	8511	25 mm Storm 1:2 Year Storm 1:5 Year Storm 1:10 Year Storm 1:25 Year Storm 1:50 Year Storm 1:100 Year Storm	1.66 9.46 17.61 24.39 32.63 40.97 47.70	5.9307 6.8569 8.8657 10.5911 12.6704 14.4958 16.2820
244	East Holland	143.2	34%	9265	25 mm Storm 1:2 Year Storm 1:5 Year Storm 1:10 Year Storm 1:25 Year Storm 1:50 Year Storm 1:100 Year Storm	0.49 2.78 5.17 7.16 9.58 12.03 14.01	1.4537 1.6401 2.2743 2.7857 3.4259 4.0179 4.5968
236	East Holland	411.9	23%	5236	25 mm Storm 1:2 Year Storm 1:5 Year Storm 1:10 Year Storm 1:25 Year Storm 1:50 Year Storm 1:100 Year Storm	1.40 7.99 14.87 20.60 27.56 34.60 40.29	3.3659 3.6596 5.6082 7.0920 9.0165 10.8770 12.6956
238	East Holland	4.7	21%	9267	25 mm Storm 1:2 Year Storm 1:5 Year Storm 1:10 Year Storm 1:25 Year Storm 1:50 Year Storm 1:100 Year Storm	0.02 0.09 0.17 0.23 0.31 0.39 0.46	0.0364 0.0393 0.0616 0.0785 0.1005 0.1220 0.1429
232	East Holland	1.8	20%	9269	25 mm Storm 1:2 Year Storm 1:5 Year Storm 1:10 Year Storm 1:25 Year Storm 1:50 Year Storm 1:100 Year Storm	0.01 0.04 0.07 0.09 0.12 0.15 0.18	0.0136 0.0146 0.0234 0.0300 0.0386 0.0471 0.0554
7B	Black River	156.6	39%	530	25 mm Storm 1:2 Year Storm 1:5 Year Storm 1:10 Year Storm 1:25 Year Storm 1:50 Year Storm 1:100 Year Storm	0.53 3.04 5.65 7.83 10.48 13.16 15.32	1.7357 1.9828 2.6540 3.2109 3.8961 4.5152 5.1208
216	East Holland	3.5	30%	9272	25 mm Storm 1:2 Year Storm 1:5 Year Storm 1:10 Year Storm 1:25 Year Storm	0.01 0.07 0.13 0.18 0.24	0.0333 0.0372 0.0532 0.0659 0.0820

					1:50 Year Storm	0.30	0.0971
					1:100 Year Storm	0.35	0.1119
TOTAL		1209.6					
Holland Landing Settlement Area							
240	East Holland	88.9	20%	9273	25 mm Storm	0.30	0.6746
					1:2 Year Storm	1.73	0.7225
					1:5 Year Storm	3.21	1.1511
					1:10 Year Storm	4.45	1.4723
					1:25 Year Storm	5.95	1.8930
					1:50 Year Storm	7.47	2.3047
					1:100 Year Storm	8.70	2.7071
242	East Holland	5.4	17%	9274	25 mm Storm	0.02	0.0379
					1:2 Year Storm	0.10	0.0399
					1:5 Year Storm	0.19	0.0663
					1:10 Year Storm	0.27	0.0857
					1:25 Year Storm	0.36	0.1115
					1:50 Year Storm	0.45	0.1369
					1:100 Year Storm	0.53	0.1618
342	West Holland	249.4	45%	9425	25 mm Storm	0.85	3.0375
					1:2 Year Storm	4.84	3.5126
					1:5 Year Storm	9.00	4.5390
					1:10 Year Storm	12.47	5.4211
					1:25 Year Storm	16.68	6.4838
					1:50 Year Storm	20.95	7.4161
					1:100 Year Storm	24.39	8.3284
422	West Holland	66.8	29%	5422	25 mm Storm	0.23	0.6171
					1:2 Year Storm	1.30	0.6860
					1:5 Year Storm	2.41	0.9909
					1:10 Year Storm	3.34	1.2303
					1:25 Year Storm	4.47	1.5350
					1:50 Year Storm	5.61	1.8228
					1:100 Year Storm	6.53	2.1041
228	East Holland	130.5	23%	9275	25 mm Storm	0.44	1.0592
					1:2 Year Storm	2.53	1.1502
					1:5 Year Storm	4.71	1.7686
					1:10 Year Storm	6.53	2.2389
					1:25 Year Storm	8.73	2.8494
					1:50 Year Storm	10.96	3.4402
					1:100 Year Storm	12.76	4.0178
226	East Holland	10.5	0%	9276	25 mm Storm	0.04	0.0402
					1:2 Year Storm	0.20	0.0341
					1:5 Year Storm	0.38	0.0910
					1:10 Year Storm	0.53	0.1297
					1:25 Year Storm	0.71	0.1837
					1:50 Year Storm	0.89	0.2402
					1:100 Year Storm	1.03	0.2954
220	East Holland	57.8	21%	9278	25 mm Storm	0.20	0.4503
					1:2 Year Storm	1.12	0.4850
					1:5 Year Storm	2.09	0.7618
					1:10 Year Storm	2.89	0.9704
					1:25 Year Storm	3.87	1.2427
					1:50 Year Storm	4.86	1.5080
					1:100 Year Storm	5.65	1.7674
230	East Holland	175.0	25%	9280	25 mm Storm	0.59	1.4722
					1:2 Year Storm	3.39	1.6096
					1:5 Year Storm	6.32	2.4308
					1:10 Year Storm	8.75	3.0604
					1:25 Year Storm	11.71	3.8736
					1:50 Year Storm	14.70	4.6556
					1:100 Year Storm	17.11	5.4201
232	East Holland	184.0	24%	9270	25 mm Storm	0.63	1.5211
					1:2 Year Storm	3.57	1.6576
					1:5 Year Storm	6.64	2.5250
					1:10 Year Storm	9.20	3.1874
					1:25 Year Storm	12.31	4.0450
					1:50 Year Storm	15.45	4.8723
					1:100 Year Storm	17.99	5.6811

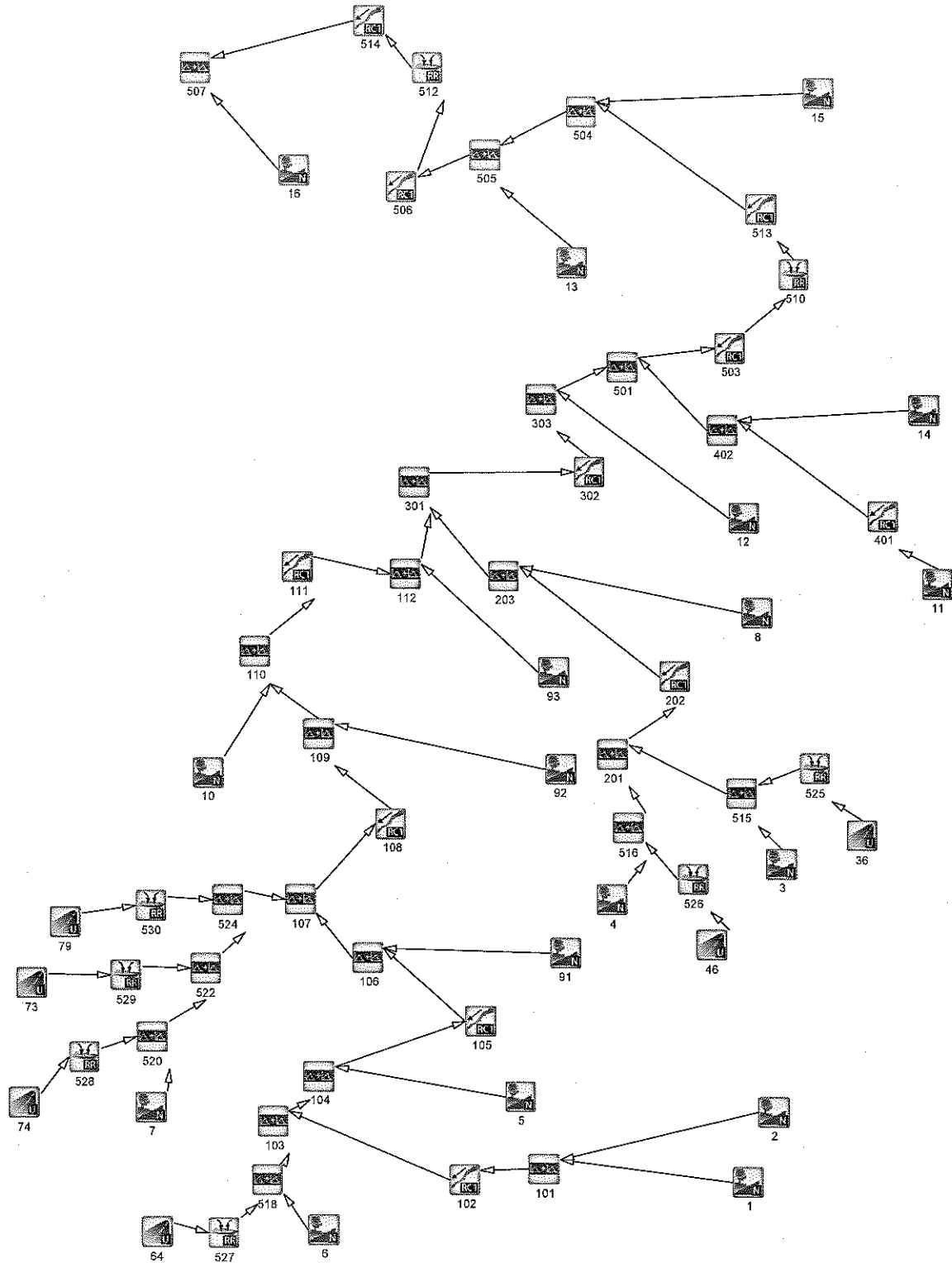
234	East Holland	266.1	14%	9281	25 mm Storm 1:2 Year Storm 1:5 Year Storm 1:10 Year Storm 1:25 Year Storm 1:50 Year Storm 1:100 Year Storm	0.90 5.16 9.61 13.31 17.80 22.35 26.03	1.7152 1.7685 3.0976 4.0635 5.3537 6.6446 7.9060
238	East Holland	90.2	13%	9268	25 mm Storm 1:2 Year Storm 1:5 Year Storm 1:10 Year Storm 1:25 Year Storm 1:50 Year Storm 1:100 Year Storm	0.31 1.75 3.26 4.51 6.04 7.58 8.83	0.5624 0.5748 1.0285 1.3564 1.7959 2.2374 2.6688
TOTAL		1324.6					
Sharon Settlement Area							
218	East Holland	69.3	22%	9282	25 mm Storm 1:2 Year Storm 1:5 Year Storm 1:10 Year Storm 1:25 Year Storm 1:50 Year Storm 1:100 Year Storm	0.24 1.35 2.50 3.47 4.64 5.82 6.78	0.5431 0.5855 0.9171 1.1672 1.4936 1.8113 2.1218
210	East Holland	11.1	30%	9284	25 mm Storm 1:2 Year Storm 1:5 Year Storm 1:10 Year Storm 1:25 Year Storm 1:50 Year Storm 1:100 Year Storm	0.04 0.22 0.40 0.56 0.74 0.93 1.09	0.1045 0.1166 0.1670 0.2067 0.2572 0.3046 0.3510
214	East Holland	158.3	26%	9286	25 mm Storm 1:2 Year Storm 1:5 Year Storm 1:10 Year Storm 1:25 Year Storm 1:50 Year Storm 1:100 Year Storm	0.54 3.07 5.72 7.92 10.59 13.30 15.49	1.3792 1.5175 2.2534 2.8224 3.5534 4.2519 4.9349
7B	Black River	178.9	27%	529	25 mm Storm 1:2 Year Storm 1:5 Year Storm 1:10 Year Storm 1:25 Year Storm 1:50 Year Storm 1:100 Year Storm	0.61 3.47 6.46 8.95 11.97 15.03 17.50	1.5855 1.7498 2.5771 3.2195 4.0426 4.8266 5.5931
216	East Holland	69.7	27%	9271	25 mm Storm 1:2 Year Storm 1:5 Year Storm 1:10 Year Storm 1:25 Year Storm 1:50 Year Storm 1:100 Year Storm	0.24 1.35 2.52 3.49 4.66 5.86 6.82	0.6230 0.6885 1.0102 1.2605 1.5808 1.8854 2.1832
TOTAL		487.4					
Type D Green Lane and Employment Expansion Settlement Area							
416	West Holland	89.3	7%	9427	25 mm Storm 1:2 Year Storm 1:5 Year Storm 1:10 Year Storm 1:25 Year Storm 1:50 Year Storm 1:100 Year Storm	0.30 1.73 3.22 4.47 5.98 7.50 8.74	0.4554 0.4375 0.9021 1.2282 1.6736 2.1302 2.5763
418	West Holland	8.7	30%	9428	25 mm Storm 1:2 Year Storm 1:5 Year Storm 1:10 Year Storm 1:25 Year Storm 1:50 Year Storm	0.03 0.17 0.31 0.43 0.58 0.73	0.0818 0.0912 0.1306 0.1618 0.2012 0.2384

					1:100 Year Storm	0.85	0.2747
414	West Holland	2.1	30%	9429	25 mm Storm	0.01	0.0195
					1:2 Year Storm	0.04	0.0218
					1:5 Year Storm	0.07	0.0312
					1:10 Year Storm	0.10	0.0386
					1:25 Year Storm	0.14	0.0480
					1:50 Year Storm	0.17	0.0569
					1:100 Year Storm	0.20	0.0655
224	East Holland	87.7	70%	9287	25 mm Storm	0.30	1.4858
					1:2 Year Storm	1.70	1.7770
					1:5 Year Storm	3.17	2.0741
					1:10 Year Storm	4.39	2.3776
					1:25 Year Storm	5.87	2.7087
					1:50 Year Storm	7.37	2.9557
					1:100 Year Storm	8.58	3.1980
222	East Holland	43.2	72%	9288	25 mm Storm	0.15	0.7428
					1:2 Year Storm	0.84	0.8895
					1:5 Year Storm	1.56	1.0342
					1:10 Year Storm	2.16	1.1836
					1:25 Year Storm	2.89	1.3455
					1:50 Year Storm	3.63	1.4651
					1:100 Year Storm	4.23	1.5823
208	East Holland	56.0	75%	9289	25 mm Storm	0.19	0.9984
					1:2 Year Storm	1.09	1.1991
					1:5 Year Storm	2.02	1.3813
					1:10 Year Storm	2.80	1.5745
					1:25 Year Storm	3.75	1.7809
					1:50 Year Storm	4.71	1.9291
					1:100 Year Storm	5.48	2.0745
226	East Holland	117.8	75%	9277	25 mm Storm	0.40	2.0988
					1:2 Year Storm	2.28	2.5207
					1:5 Year Storm	4.25	2.9037
					1:10 Year Storm	5.89	3.3097
					1:25 Year Storm	7.88	3.7436
					1:50 Year Storm	9.89	4.0552
					1:100 Year Storm	11.52	4.3609
210	East Holland	192.7	73%	9283	25 mm Storm	0.66	2.0554
					1:2 Year Storm	3.74	2.4643
					1:5 Year Storm	6.96	2.8540
					1:10 Year Storm	9.63	3.2607
					1:25 Year Storm	12.89	3.6991
					1:50 Year Storm	16.19	4.0191
					1:100 Year Storm	18.84	4.3330
220	East Holland	13.3	75%	9279	25 mm Storm	0.05	0.2376
					1:2 Year Storm	0.26	0.2854
					1:5 Year Storm	0.48	0.3287
					1:10 Year Storm	0.67	0.3747
					1:25 Year Storm	0.89	0.4238
					1:50 Year Storm	1.12	0.4591
					1:100 Year Storm	1.30	0.4937
214	East Holland	93.1	75%	9285	25 mm Storm	0.32	1.6583
					1:2 Year Storm	1.81	1.9916
					1:5 Year Storm	3.36	2.2943
					1:10 Year Storm	4.65	2.6150
					1:25 Year Storm	6.23	2.9579
					1:50 Year Storm	7.82	3.2041
					1:100 Year Storm	9.10	3.4456
6B	Black River	32.3	75%	527	25 mm Storm	0.11	0.5752
					1:2 Year Storm	0.63	0.6908
					1:5 Year Storm	1.17	0.7958
					1:10 Year Storm	1.61	0.9070
					1:25 Year Storm	2.16	1.0260
					1:50 Year Storm	2.71	1.1114
					1:100 Year Storm	3.16	1.1951
7B	Black River	135.7	75%	528	25 mm Storm	0.46	2.4187
					1:2 Year Storm	2.63	2.9048
					1:5 Year Storm	4.90	3.3462
					1:10 Year Storm	6.79	3.8141

					1:25 Year Storm	9.08	4.3142
					1:50 Year Storm	11.40	4.6733
					1:100 Year Storm	13.27	5.0255
212	East Holland	4.4	75%	9290	25 mm Storm	0.01	0.0778
					1:2 Year Storm	0.08	0.0934
					1:5 Year Storm	0.16	0.1076
					1:10 Year Storm	0.22	0.1226
					1:25 Year Storm	0.29	0.1387
					1:50 Year Storm	0.37	0.1503
					1:100 Year Storm	0.43	0.1616
TOTAL		876.3					

APPENDIX F
Future Conditions Hydrologic Model Output

BLACK RIVER SUBWATERSHED



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V V I SSSS U U A L
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
W I SSSS UUUU A A LLLLL

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000 TTTT TTTT H H Y Y M M 000 TM
0 0 T T H H Y Y M M 0 0
0 0 T T H H Y Y M M 0 0 Company Serial
000 T T H H Y Y M M 000

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***** D E T A I L E D O U T P U T *****

Input filename: C:\Program Files (x86)\Visual Otthymo 2.4\VO2\vo1n.dat
 Output filename: C:\Users\aman1\cu\AppData\Local\Temp\6ab08fc7-e8e6-4b3d-8177-e9629b714f53\Scenario.ou
 Summary filename: C:\Users\aman1\cu\AppData\Local\Temp\6ab08fc7-e8e6-4b3d-8177-e9629b714f53\Scenario.sum

DATE: 08/29/2012 TIME: 01:49:24

USER:

COMMENTS: _____

 ** SIMULATION NUMBER: 1 **

READ STORM File name: C:\Users\aman1\cu\AppData\Local\Temp\6ab08fc7-e8e6-4b3d-8177-e9629b714f53\3fe6a33e
 Ptotal = 51.24 mm Comments: 2yr/24hr

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	0.00	6.50	4.35	12.75	1.79
0.50	0.26	6.75	4.35	13.00	1.79
0.75	0.26	7.00	4.35	13.25	1.79
1.00	0.26	7.25	4.35	13.50	1.79
1.25	0.26	7.50	4.35	13.75	1.79
1.50	0.26	7.75	4.35	14.00	1.79
1.75	0.26	8.00	4.35	14.25	1.79
2.00	0.26	8.25	4.35	14.50	1.02
2.25	0.26	8.50	11.78	14.75	1.02
2.50	0.26	8.75	11.78	15.00	1.02
2.75	0.26	9.00	11.78	15.25	1.02
3.00	0.26	9.25	11.78	15.50	1.02
3.25	0.26	9.50	11.78	15.75	1.02
3.50	0.26	9.75	11.78	16.00	1.02
3.75	0.26	10.00	11.78	16.25	1.02
4.00	0.26	10.25	11.78	16.50	0.51
4.25	0.26	10.50	3.33	16.75	0.51
4.50	1.54	10.75	3.33	17.00	0.51
4.75	1.54	11.00	3.33	17.25	0.51
5.00	1.54	11.25	3.33	17.50	0.51
5.25	1.54	11.50	3.33	17.75	0.51
5.50	1.54	11.75	3.33	18.00	0.51
5.75	1.54	12.00	3.33	18.25	0.51
6.00	1.54	12.25	3.33	18.50	0.26
6.25	1.54	12.50	1.79	18.75	0.26

CALIB NASHYD (0016) Area (ha)=3094.49 Curve Number (CN)= 74.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 18.10

Unit Hyd Opeak (cms)= 6.704

PEAK FLOW (cms)= 4.001 (i)
 TIME TO PEAK (hrs)= 30.000
 RUNOFF VOLUME (mm)= 15.782
 TOTAL RAINFALL (mm)= 51.240
 RUNOFF COEFFICIENT = 0.308

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0013) Area (ha)=1627.59 Curve Number (CN)= 65.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 15.50

Unit Hyd Opeak (cms)= 4.118

PEAK FLOW (cms)= 1.790 (i)
 TIME TO PEAK (hrs)= 27.500
 RUNOFF VOLUME (mm)= 11.683
 TOTAL RAINFALL (mm)= 51.240
 RUNOFF COEFFICIENT = 0.228

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0015) Area (ha)=1059.88 Curve Number (CN)= 73.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 9.30

Unit Hyd Opeak (cms)= 4.469

PEAK FLOW (cms)= 2.358 (i)
 TIME TO PEAK (hrs)= 20.750
 RUNOFF VOLUME (mm)= 15.252
 TOTAL RAINFALL (mm)= 51.240
 RUNOFF COEFFICIENT = 0.298

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0012) Area (ha)=1501.52 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 11.50

Unit Hyd Opeak (cms)= 5.120

PEAK FLOW (cms)= 1.873 (i)
 TIME TO PEAK (hrs)= 23.250
 RUNOFF VOLUME (mm)= 10.248
 TOTAL RAINFALL (mm)= 51.240
 RUNOFF COEFFICIENT = 0.200

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0093) Area (ha)= 381.73 Curve Number (CN)= 69.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 14.90

Unit Hyd Opeak (cms)= 1.005

PEAK FLOW (cms)= 0.496 (i)
 TIME TO PEAK (hrs)= 26.750
 RUNOFF VOLUME (mm)= 13.334
 TOTAL RAINFALL (mm)= 51.240
 RUNOFF COEFFICIENT = 0.260

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0010) Area (ha)=2842.76 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 10.80

Unit Hyd Qpeak (cms) = 10.322

PEAK FLOW (cms) = 4.127 (i)
TIME TO PEAK (hrs) = 22.500
RUNOFF VOLUME (mm) = 11.306
TOTAL RAINFALL (mm) = 51.240
RUNOFF COEFFICIENT = 0.221

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0005) | Area (ha)=1444.47 Curve Number (CN)= 60.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
U.H. Tp(hrs)= 8.00

Unit Hyd Qpeak (cms) = 7.080

PEAK FLOW (cms) = 2.348 (i)
TIME TO PEAK (hrs) = 19.500
RUNOFF VOLUME (mm) = 9.918
TOTAL RAINFALL (mm) = 51.240
RUNOFF COEFFICIENT = 0.194

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0001) | Area (ha)=2873.64 Curve Number (CN)= 61.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
U.H. Tp(hrs)= 11.00

Unit Hyd Qpeak (cms) = 10.244

PEAK FLOW (cms) = 3.721 (i)
TIME TO PEAK (hrs) = 22.750
RUNOFF VOLUME (mm) = 10.248
TOTAL RAINFALL (mm) = 51.240
RUNOFF COEFFICIENT = 0.200

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0002) | Area (ha)= 988.72 Curve Number (CN)= 58.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
U.H. Tp(hrs)= 9.10

Unit Hyd Qpeak (cms) = 4.261

PEAK FLOW (cms) = 1.357 (i)
TIME TO PEAK (hrs) = 20.750
RUNOFF VOLUME (mm) = 9.289
TOTAL RAINFALL (mm) = 51.240
RUNOFF COEFFICIENT = 0.181

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0101) |
1 + 2 = 3
ID1= 1 (0001): 2873.64 3.721 22.75 10.25
+ ID2= 2 (0002): 988.72 1.357 20.75 9.29
ID = 3 (0101): 3862.36 5.044 22.25 10.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0102) |
IN= 2----> OUT= 1 Routing time step (min)' = 15.00

----- DATA FOR SECTION (1.1) -----
Distance Elevation Manning
0.00 250.00 0.0700
43.00 240.00 0.0700 / 0.0350 Main Channel
44.00 239.20 0.0350 Main Channel

45.00 239.20 0.0350 Main Channel
46.00 240.00 0.0350 / 0.0700 Main Channel
63.50 241.00 0.0700
71.00 250.00 0.0700

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.40	239.60	.642E+04	0.5	0.80	221.73
0.80	240.00	.171E+05	1.8	1.15	155.20
1.39	240.59	.764E+05	8.0	1.12	159.62
1.98	241.18	.214E+06	23.7	1.19	150.23
2.56	241.76	.385E+06	51.8	1.44	123.91
3.15	242.35	.575E+06	90.1	1.68	106.38
3.74	242.94	.784E+06	138.5	1.89	94.40
4.33	243.53	.101E+07	196.9	2.08	85.71
4.92	244.12	.126E+07	265.6	2.26	79.08
5.51	244.71	.153E+07	344.7	2.42	73.81
6.09	245.29	.181E+07	434.5	2.57	69.50
6.68	245.88	.212E+07	535.2	2.71	65.89
7.27	246.47	.244E+07	647.3	2.84	62.81
7.86	247.06	.278E+07	771.1	2.97	60.13
8.45	247.65	.314E+07	906.8	3.09	57.77
9.04	248.24	.352E+07	1054.8	3.20	55.67
9.62	248.82	.392E+07	1215.5	3.32	53.79
10.21	249.41	.434E+07	1389.1	3.42	52.09
10.80	250.00	.478E+07	1576.1	3.53	50.53

hydrograph
AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW: ID= 2 (0101) 3862.36 5.04 22.25 10.00 1.11 1.13
OUTFLOW: ID= 1 (0102) 3862.36 4.79 25.00 10.00 1.08 1.13

CALIB NASHYD (0006) | Area (ha)=1472.14 Curve Number (CN)= 73.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
U.H. Tp(hrs)= 12.20

Unit Hyd Qpeak (cms) = 4.732

PEAK FLOW (cms) = 2.606 (i)
TIME TO PEAK (hrs) = 24.000
RUNOFF VOLUME (mm) = 15.252
TOTAL RAINFALL (mm) = 51.240
RUNOFF COEFFICIENT = 0.298

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (0064) | Area (ha)= 32.30
ID= 1 DT=15.0 min | Total Imp(%)= 75.00 Dir. Conn.(%)= 70.00

IMPERVIOUS PVIOUS (i)
Surface Area (ha)= 24.22 8.07
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 464.04 40.00
Mannings n = 0.013 0.250
Max. Eff. Inten. (mm/hr)= 11.78 5.87
over (min) 15.00 45.00
Storage Coeff. (min)= 15.09 (ii) 37.04 (ii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.07 0.03

TOTALS
PEAK FLOW (cms)= 0.74 0.10 0.843 (iii)
TIME TO PEAK (hrs)= 10.25 10.50 10.25
RUNOFF VOLUME (mm)= 50.24 15.59 39.84
TOTAL RAINFALL (mm)= 51.24 51.24 51.24
RUNOFF COEFFICIENT = 0.98 0.30 0.78

(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0527)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.6100	10.9070
0.1100	10.5752	2.1600	11.0260
0.6300	10.6908	2.7100	11.1114
1.1700	10.7958	3.1600	11.1951

INFLOW: ID= 2 (0064)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
32.300	32.300	0.843	10.25	39.84
OUTFLOW: ID= 1 (0527)	32.300	0.013	24.50	33.47

PEAK FLOW REDUCTION [Qout/Qin] (%) = 1.51
TIME SHIFT OF PEAK FLOW (min) = 855.00
MAXIMUM STORAGE USED (ha.m.) = 1.2200

ADD HYD (0518)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0006)	1472.14	2.606	24.00	15.25
+ ID2= 2 (0527)	32.30	0.013	24.50	33.47
ID = 3 (0518)	1504.44	2.619	24.00	15.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0103)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0102)	3862.36	4.787	25.00	10.00
+ ID2= 2 (0518)	1504.44	2.619	24.00	15.64
ID = 3 (0103)	5366.80	7.394	24.75	11.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0104)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0005)	1444.47	2.348	19.50	9.92
+ ID2= 2 (0103)	5366.80	7.394	24.75	11.58
ID = 3 (0104)	6811.27	9.346	23.25	11.23

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0105)
IN= 2--> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning	
0.00	237.00	0.1000	
330.00	235.00	0.1000 / 0.0350	Main Channel
331.00	234.20	0.0350	Main Channel
332.00	234.20	0.0350	Main Channel
333.00	235.00	0.0350 / 0.1000	Main Channel
425.00	235.10	0.1000	
698.00	236.00	0.1000	
1101.00	239.00	0.1000	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.13	234.33	.490E+03	0.1	0.64	82.66
0.27	234.47	.112E+04	0.3	0.93	56.72
0.40	234.60	.189E+04	0.7	1.14	45.94
0.53	234.73	.280E+04	1.2	1.33	39.64
0.67	234.87	.385E+04	1.8	1.49	35.34
0.80	235.00	.504E+04	2.6	1.63	32.16

0.95	235.15	.442E+05	6.3	0.45	116.42
1.11	235.31	.128E+06	17.7	0.44	120.51
1.26	235.46	.247E+06	38.2	0.49	107.48
1.42	235.62	.400E+06	69.8	0.55	95.60
1.57	235.77	.589E+06	114.0	0.61	86.08
1.72	235.92	.812E+06	172.6	0.67	78.45
1.88	236.08	.107E+07	250.3	0.74	71.19
2.03	236.23	.135E+07	347.8	0.81	64.71
2.18	236.38	.165E+07	461.9	0.88	59.65
2.34	236.54	.198E+07	593.4	0.95	55.58
2.49	236.69	.233E+07	742.8	1.01	52.21
2.65	236.85	.270E+07	910.6	1.06	49.36
2.80	237.00	.309E+07	1097.6	1.12	46.91

INFLOW: ID= 2 (0104)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
6811.27	6811.27	9.35	23.25	11.23	0.99	0.45
OUTFLOW: ID= 1 (0105)	6811.27	9.07	25.75	11.23	0.99	0.45

CALIB NASHYD (0091)
ID= 1 DT=15.0 min

Area (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
281.97	0.689	17.00	11.68	0.99	0.45
U. H. Tp (hrs) = 5.80					

Unit Hyd Opeak (cms) = 1.906

PEAK FLOW (cms) = 0.689 (i)
TIME TO PEAK (hrs) = 17.000
RUNOFF VOLUME (mm) = 11.683
TOTAL RAINFALL (mm) = 51.240
RUNOFF COEFFICIENT = 0.228

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0106)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0091)	281.97	0.689	17.00	11.68
+ ID2= 2 (0105)	6811.27	9.067	25.75	11.23
ID = 3 (0106)	7093.24	9.368	25.25	11.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0007)
ID= 1 DT=15.0 min

Area (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
902.64	3.308	10.70	11.23	0.99	0.45
U. H. Tp (hrs) = 10.70					

Unit Hyd Opeak (cms) = 3.308

PEAK FLOW (cms) = 1.786 (i)
TIME TO PEAK (hrs) = 22.250
RUNOFF VOLUME (mm) = 15.252
TOTAL RAINFALL (mm) = 51.240
RUNOFF COEFFICIENT = 0.298

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (0074)
ID= 1 DT=15.0 min

Area Total	(ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
135.70	135.70	75.00	70.00	70.00	70.00	70.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) = 101.78	33.92
Dep. Storage (mm) = 1.00	5.00
Average Slope (%) = 1.00	2.00
Length (m) = 951.14	40.00
Mannings n = 0.013	0.250
Max. Eff. Inten. (mm/hr) = 11.78	4.04
over (min) = 30.00	60.00
Storage Coeff. (min) = 23.22 (ii)	48.70 (ii)

Unit Hyd. Tpeak (min)= 30.00 60.00
 Unit Hyd. peak (cms)= 0.04 0.02

PEAK FLOW (cms)= 3.09 0.27 3.338 (iii)
 TIME TO PEAK (hrs)= 10.25 10.75 10.25
 RUNOFF VOLUME (mm)= 50.24 10.72 38.38
 TOTAL RAINFALL (mm)= 51.24 51.24 51.24
 RUNOFF COEFFICIENT = 0.98 0.21 0.75

TOTALS
 (iii)

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 57.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0528)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	6.7900	9.8141
0.4600	8.4187	9.0800	10.3142
2.6300	8.9048	11.4000	10.6733
4.9000	9.3462	13.2700	11.0255

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0074)	135.700	3.338	10.25	38.38
OUTFLOW: ID= 1 (0528)	135.700	0.235	16.75	38.35

PEAK FLOW REDUCTION [Qout/Qin] (%) = 7.05
 TIME SHIFT OF PEAK FLOW (min) = 390.00
 MAXIMUM STORAGE USED (ha.m.) = 4.3050

ADD HYD (0520)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0007):	902.64	1.786	22.25	15.25
+ ID2= 2 (0528):	135.70	0.235	16.75	38.35
=====				
ID = 3 (0520):	1038.34	2.009	22.25	18.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDBYD (0073)
 ID= 1 DT=15.0 min

Area (ha) = 178.90
 Total Imp (%) = 27.00
 Dir. Conn. (%) = 9.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	48.30	130.60
Dep. Storage (mm)	1.00	5.00
Average Slope (%)	1.00	2.00
Length (m)	1092.09	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 11.78 4.47
 over (min) = 30.00 60.00
 Storage Coeff. (min) = 25.23 (ii) 49.68 (ii)
 Unit Hyd. Tpeak (min) = 30.00 60.00
 Unit Hyd. peak (cms) = 0.04 0.02

PEAK FLOW (cms) = 0.52 1.14 *TOTALS*
 TIME TO PEAK (hrs) = 10.25 10.75 1.588 (iii)
 RUNOFF VOLUME (mm) = 50.24 11.45 10.50
 TOTAL RAINFALL (mm) = 51.24 51.24 14.94
 RUNOFF COEFFICIENT = 0.98 0.22 51.24 0.29

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 58.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0529)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	8.9500	9.2195
0.6100	7.5855	11.9700	10.0426
3.4700	7.7498	15.0300	10.8266
6.4600	8.5771	17.5000	11.5931

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0073)	178.900	1.588	10.50	14.94
OUTFLOW: ID= 1 (0529)	178.900	0.165	17.50	14.93

PEAK FLOW REDUCTION [Qout/Qin] (%) = 10.40
 TIME SHIFT OF PEAK FLOW (min) = 420.00
 MAXIMUM STORAGE USED (ha.m.) = 2.0538

ADD HYD (0522)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0520):	1038.34	2.009	22.25	18.27
+ ID2= 2 (0529):	178.90	0.165	17.50	14.93
=====				
ID = 3 (0522):	1217.24	2.165	22.00	17.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDBYD (0079)
 ID= 1 DT=15.0 min

Area (ha) = 156.60
 Total Imp (%) = 39.00
 Dir. Conn. (%) = 30.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	61.07	95.53
Dep. Storage (mm)	1.00	5.00
Average Slope (%)	1.00	2.00
Length (m)	1021.76	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 11.78 5.77
 over (min) = 30.00 60.00
 Storage Coeff. (min) = 24.24 (ii) 46.32 (ii)
 Unit Hyd. Tpeak (min) = 30.00 60.00
 Unit Hyd. peak (cms) = 0.04 0.02

PEAK FLOW (cms) = 1.53 1.11 *TOTALS*
 TIME TO PEAK (hrs) = 10.25 10.50 2.574 (iii)
 RUNOFF VOLUME (mm) = 50.24 16.01 26.28
 TOTAL RAINFALL (mm) = 51.24 51.24 51.24
 RUNOFF COEFFICIENT = 0.98 0.31 0.51

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 71.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0530)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	7.8300	9.2109
0.5300	7.7357	10.4800	9.8961
3.0400	7.9828	13.1600	10.5152
5.6500	8.6540	15.3200	11.1208

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0079)	156.600	2.574	10.25	26.28
OUTFLOW: ID= 1 (0530)	156.600	0.224	17.00	26.26

PEAK FLOW REDUCTION [Qout/Qin] (%) = 8.69
 TIME SHIFT OF PEAK FLOW (min) = 405.00
 MAXIMUM STORAGE USED (ha.m.) = 3.2668

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0522):	1217.24	2.165	22.00	17.78
+ ID2= 2 (0530):	156.60	0.224	17.00	26.26
=====				
ID = 3 (0524):	1373.84	2.377	22.00	18.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0106):	7093.24	9.368	25.25	11.25
+ ID2= 2 (0524):	1373.84	2.377	22.00	18.75
=====				
ID = 3 (0107):	8467.08	11.639	24.75	12.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0108)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

Distance	Elevation	Manning	
0.00	234.00	0.1000	
400.00	231.00	0.1000 / 0.0350	Main Channel
401.00	229.70	0.0350	Main Channel
402.00	229.70	0.0350	Main Channel
404.00	230.50	0.0350 / 0.1000	Main Channel
495.00	231.00	0.1000	
1234.00	256.00	0.1000	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.20	229.90	.151E+04	0.1	0.24	395.61
0.40	230.10	.377E+04	0.2	0.35	274.62
0.60	230.30	.677E+04	0.5	0.43	222.06
0.80	230.50	.105E+05	0.9	0.50	190.52
1.03	230.73	.437E+05	2.0	0.27	356.49
1.27	230.97	.134E+06	5.0	0.21	448.69
1.50	231.20	.278E+06	11.4	0.23	405.59
1.73	231.43	.473E+06	21.6	0.26	364.80
1.97	231.67	.718E+06	36.0	0.29	332.76
2.20	231.90	.101E+07	55.1	0.31	306.87
2.43	232.13	.136E+07	79.4	0.33	285.44
2.67	232.37	.176E+07	109.6	0.36	267.34
2.90	232.60	.221E+07	145.9	0.38	251.83
3.13	232.83	.270E+07	189.0	0.40	238.35
3.37	233.07	.325E+07	239.3	0.42	226.51
3.60	233.30	.385E+07	297.1	0.44	216.02
3.83	233.53	.450E+07	363.0	0.46	206.64
4.07	233.77	.520E+07	437.3	0.48	198.20
4.30	234.00	.595E+07	520.5	0.50	190.55

INFLOW	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
ID= 2 (0107)	8467.08	11.64	24.75	12.46	1.51	0.23
OUTFLOW: ID= 1 (0108)	8467.08	9.50	30.50	12.46	1.43	0.23

CALIB NASHYD (0092)
ID= 1 DT=15.0 min
Area (ha)=1102.32
Curve Number (CN)= 64.0
U. H. Tp (hrs)= 16.50
of Linear Res. (N)= 3.10

Unit Hyd Opeak (cms)= 2.620
PEAK FLOW (cms)= 1.109 (i)
TIME TO PEAK (hrs)= 28.500
RUNOFF VOLUME (mm)= 11.306
TOTAL RAINFALL (mm)= 51.240

RUNOFF COEFFICIENT = 0.221

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0092):	1102.32	1.109	28.50	11.31
+ ID2= 2 (0108):	8467.08	9.503	30.50	12.46
=====				
ID = 3 (0109):	9569.40	10.598	30.50	12.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0010):	2842.76	4.127	22.50	11.31
+ ID2= 2 (0109):	9569.40	10.598	30.50	12.33
=====				
ID = 3 (0110):	12412.16	13.836	28.50	12.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0111)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

Distance	Elevation	Manning	
0.00	236.00	0.1000	
275.00	232.00	0.1000	
1039.00	230.00	0.1000 / 0.0350	Main Channel
1040.00	229.20	0.0350	Main Channel
1041.00	229.20	0.0350	Main Channel
1043.00	230.00	0.0350 / 0.1000	Main Channel
1117.00	230.60	0.1000	
1325.00	234.00	0.1000	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.20	229.40	.985E+03	0.0	0.14	431.34
0.40	229.60	.251E+04	0.1	0.20	298.03
0.60	229.80	.457E+04	0.3	0.25	239.97
0.80	230.00	.717E+04	0.6	0.29	205.17
1.07	230.27	.754E+05	2.2	0.11	561.90
1.33	230.53	.272E+06	8.6	0.11	527.12
1.60	230.80	.594E+06	22.7	0.14	436.37
1.87	231.07	.103E+07	46.2	0.16	371.32
2.13	231.33	.158E+07	80.7	0.18	325.64
2.40	231.60	.224E+07	127.8	0.20	291.74
2.67	231.87	.301E+07	189.0	0.22	265.46
2.93	232.13	.389E+07	271.8	0.25	238.27
3.20	232.40	.481E+07	376.4	0.28	212.79
3.47	232.67	.576E+07	496.0	0.31	193.49
3.73	232.93	.674E+07	630.4	0.33	178.30
4.00	233.20	.776E+07	779.4	0.36	165.99
4.27	233.47	.881E+07	943.0	0.38	155.79
4.53	233.73	.990E+07	1121.1	0.41	147.16
4.80	234.00	.110E+08	1313.9	0.43	139.76

INFLOW	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
ID= 2 (0110)	13.84	28.50	12.10	1.43	0.12	
OUTFLOW: ID= 1 (0111)	11.17	35.25	12.10	1.38	0.12	

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0093):	381.73	0.496	26.75	13.33
+ ID2= 2 (0111):	12412.16	11.167	35.25	12.10

 ID = 3 (0112): 12793.89 11.564 35.00 12.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB NASHYD (0008) Area (ha)=1549.97 Curve Number (CN)= 60.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 9.10

Unit Hyd Qpeak (cms)= 6.679

PEAK FLOW (cms)= 2.273 (i)
 TIME TO PEAK (hrs)= 20.750
 RUNOFF VOLUME (mm)= 9.918
 TOTAL RAINFALL (mm)= 51.240
 RUNOFF COEFFICIENT = 0.194

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB NASHYD (0003) Area (ha)=2674.44 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 13.10

Unit Hyd Qpeak (cms)= 8.006

PEAK FLOW (cms)= 3.296 (i)
 TIME TO PEAK (hrs)= 25.000
 RUNOFF VOLUME (mm)= 11.306
 TOTAL RAINFALL (mm)= 51.240
 RUNOFF COEFFICIENT = 0.221

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB STANDHYD (0036) Area (ha)= 280.10 Dir. Conn. (%)= 9.00
 ID= 1 DT=15.0 min Total Imp(%)= 22.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	61.62	218.48
Dep. Storage (mm)=	1.00	5.00
Average Slope (%)=	1.00	2.00
Length (m)=	1366.50	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	11.78	5.76
over (min)	30.00	60.00
Storage Coeff. (min)=	28.86 (ii)	50.96 (ii)
Unit Hyd. Tpeak (min)=	30.00	60.00
Unit Hyd. peak (cms)=	0.04	0.02

TOTALS
 PEAK FLOW (cms)= 0.81 2.47 3.183 (iii)
 TIME TO PEAK (hrs)= 10.25 10.75 10.50
 RUNOFF VOLUME (mm)= 50.24 15.72 18.83
 TOTAL RAINFALL (mm)= 51.24 51.24 51.24
 RUNOFF COEFFICIENT = 0.98 0.31 0.37

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR (0525)
 IN= 2----> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	14.0000	14.7295
0.9500	12.2068	18.7400	16.0462
5.4300	12.3823	23.5200	17.3268
10.1100	13.7195	27.3900	18.5786

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (0036)	280.100	3.183	10.50	18.83
OUTFLOW: ID= 1 (0525)	280.100	0.318	17.50	18.81

PEAK FLOW REDUCTION [Qout/ Qin] (%) = 9.98
 TIME SHIFT OF PEAK FLOW (min) = 420.00
 MAXIMUM STORAGE USED (ha. m.) = 4.0809

 ADD HYD (0515)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0003):	2674.44	3.296	25.00	11.31
+ ID2= 2 (0525):	280.10	0.318	17.50	18.81

ID = 3 (0515):	2954.54	3.583	25.00	12.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB NASHYD (0004) Area (ha)=2580.39 Curve Number (CN)= 53.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 10.70

Unit Hyd Qpeak (cms)= 9.457

PEAK FLOW (cms)= 2.624 (i)
 TIME TO PEAK (hrs)= 22.500
 RUNOFF VOLUME (mm)= 7.876
 TOTAL RAINFALL (mm)= 51.240
 RUNOFF COEFFICIENT = 0.154

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB STANDHYD (0046) Area (ha)= 92.30 Dir. Conn. (%)= 33.00
 ID= 1 DT=15.0 min Total Imp(%)= 44.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	40.61	51.69
Dep. Storage (mm)=	1.00	5.00
Average Slope (%)=	1.00	2.00
Length (m)=	784.43	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	11.78	4.55
over (min)	15.00	45.00
Storage Coeff. (min)=	20.68 (ii)	44.97 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.06	0.03

TOTALS
 PEAK FLOW (cms)= 0.99 0.49 1.470 (iii)
 TIME TO PEAK (hrs)= 10.25 10.50 10.25
 RUNOFF VOLUME (mm)= 50.24 12.12 24.70
 TOTAL RAINFALL (mm)= 51.24 51.24 51.24
 RUNOFF COEFFICIENT = 0.98 0.24 0.48

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR (0526)
 IN= 2----> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.6200	51.9904
0.3100	51.1099	6.1800	52.3852
1.7900	51.2815	7.7500	52.7331
3.3300	51.6636	9.0300	53.0736

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)

INFLOW : ID= 2 (0046) 92.300 1.470 10.25 24.70
 OUTFLOW: ID= 1 (0526) 92.300 0.013 25.00 16.20

PEAK FLOW REDUCTION [Qout/Qin] (%) = 0.91
 TIME SHIFT OF PEAK FLOW (min) = 885.00
 MAXIMUM STORAGE USED (ha. m.) = 2.2080

ADD HYD	(0516)	AREA	OPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0004):	2580.39	2.624	22.50	7.88	
+ ID2= 2 (0526):	92.30	0.013	25.00	16.20	
ID = 3 (0516):	2672.69	2.638	22.50	8.16	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(0201)	AREA	OPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0515):	2954.54	3.583	25.00	12.02	
+ ID2= 2 (0516):	2672.69	2.638	22.50	8.16	
ID = 3 (0201):	5627.23	6.172	23.75	10.19	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0202)
 IN= 2--> OUT= 1 Routing time step (min)' = 15.00

Distance	Elevation	Manning	
0.00	239.00	0.1000	
690.00	230.00	0.1000 / 0.0350	Main Channel
691.00	229.20	0.0350	Main Channel
692.00	229.20	0.0350	Main Channel
693.00	229.50	0.0350 / 0.1000	Main Channel
1190.00	230.00	0.1000	
1417.00	255.00	0.1000	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.30	229.50	.347E+04	0.4	0.84	136.57
0.80	230.00	.864E+06	45.2	0.36	318.78
1.30	230.50	.265E+07	274.1	0.71	161.20
1.80	231.00	.459E+07	651.8	0.97	117.24
2.30	231.50	.667E+07	1165.3	1.20	95.34
2.80	232.00	.889E+07	1810.2	1.40	81.89
3.30	232.50	.113E+08	2585.1	1.57	72.65
3.80	233.00	.138E+08	3490.4	1.73	65.85
4.30	233.50	.165E+08	4527.4	1.89	60.59
4.80	234.00	.193E+08	5697.8	2.03	56.38
5.30	234.50	.222E+08	7003.8	2.16	52.91
5.80	235.00	.253E+08	8447.7	2.28	50.00
6.30	235.50	.286E+08	10032.3	2.40	47.52
6.80	236.00	.320E+08	11760.1	2.52	45.36
7.30	236.50	.356E+08	13634.1	2.63	43.46
7.80	237.00	.393E+08	15657.0	2.73	41.78
8.30	237.50	.431E+08	17831.7	2.84	40.28
8.80	238.00	.471E+08	20161.3	2.93	38.92
9.30	238.50	.512E+08	22648.7	3.03	37.69

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0201)	5627.23	6.17	23.75	10.19	0.36
OUTFLOW: ID= 1 (0202)	5627.23	5.91	27.00	10.19	0.36

ADD HYD	(0203)	AREA	OPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	

ID1= 1 (0008): 1549.97 2.273 20.75 9.92
 + ID2= 2 (0202): 5627.23 5.909 27.00 10.19
 ID = 3 (0203): 7177.20 7.774 25.00 10.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(0301)	AREA	OPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0112):	12793.89	11.564	35.00	12.13	
+ ID2= 2 (0203):	7177.20	7.774	25.00	10.13	
ID = 3 (0301):	19971.09	17.701	30.75	11.41	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0302)
 IN= 2--> OUT= 1 Routing time step (min)' = 15.00

Distance	Elevation	Manning	
0.00	240.00	0.0700	
110.00	239.00	0.0700	
161.00	238.00	0.0700	
250.00	237.00	0.0700	
420.00	236.00	0.0700	
560.00	235.00	0.0700	
589.00	234.40	0.0700 / 0.0350	Main Channel
589.10	233.60	0.0350	Main Channel
594.00	233.60	0.0350	Main Channel
594.10	234.40	0.0350 / 0.0700	Main Channel
615.00	235.00	0.0700	
740.00	236.00	0.0700	
860.00	236.50	0.0700	
1340.00	237.00	0.0700	
1460.00	238.00	0.0700	
1470.00	239.00	0.0700	
1510.00	240.00	0.0700	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.27	233.87	.770E+04	1.0	0.74	132.49
0.53	234.13	.155E+05	2.9	1.10	88.48
0.80	234.40	.234E+05	5.5	1.37	71.19
1.15	234.75	.636E+05	11.6	1.07	91.11
1.50	235.10	.169E+06	24.5	0.85	114.97
1.85	235.45	.431E+06	54.5	0.74	131.62
2.20	235.80	.882E+06	117.3	0.78	125.39
2.55	236.15	.153E+07	218.3	0.83	117.06
2.90	236.50	.246E+07	375.3	0.89	109.30
3.25	236.85	.394E+07	579.0	0.86	113.45
3.60	237.20	.612E+07	1010.3	0.97	101.01
3.95	237.55	.851E+07	1631.3	1.12	86.99
4.30	237.90	.111E+08	2391.8	1.27	77.05
4.65	238.25	.137E+08	3337.5	1.42	68.52
5.00	238.60	.164E+08	4438.7	1.58	61.71
5.35	238.95	.192E+08	5671.0	1.73	56.40
5.70	239.30	.220E+08	6955.1	1.85	52.75
6.05	239.65	.249E+08	8355.2	1.96	49.76
6.40	240.00	.280E+08	9884.7	2.07	47.18

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0301)	*****	17.70	30.75	11.41	1.32
OUTFLOW: ID= 1 (0302)	*****	17.48	33.25	11.41	0.96

ADD HYD	(0303)	AREA	OPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0012):	1501.52	1.873	23.25	10.25	
+ ID2= 2 (0302):	19971.09	17.480	33.25	11.41	

ID = 3 (0303): 21472.61 18.736 32.50 11.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha)=3099.17 Ia (mm)= 5.00 U.H. Tp(hrs)= 11.10	Curve Number (CN)= 57.0 # of Linear Res. (N)= 3.10
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Unit Hyd Qpeak (cms)= 10.949

PEAK FLOW (cms)= 3.491 (i)
TIME TO PEAK (hrs)= 23.000
RUNOFF VOLUME (mm)= 8.989
TOTAL RAINFALL (mm)= 51.240
RUNOFF COEFFICIENT = 0.175

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (0401) IN= 2--> OUT= 1	Routing time step (min)'= 15.00
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DATA FOR SECTION (1.1) ----->			
Distance	Elevation	Manning	
0.00	236.00	0.0500	
72.00	235.00	0.0500	
492.00	230.00	0.0500	
581.00	229.20	0.0500 / 0.0350	Main Channel
582.00	228.40	0.0350	Main Channel
583.00	228.40	0.0350	Main Channel
584.00	229.20	0.0350 / 0.0500	Main Channel
588.00	229.20	0.0500	
607.00	230.00	0.0500	
627.00	235.00	0.0500	

TRAVEL TIME TABLE ----->						
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)	
0.27	228.67	.302E+04	0.1	0.21	666.69	
0.53	228.93	.756E+04	0.3	0.30	465.92	
0.80	229.20	.136E+05	0.6	0.37	377.99	
1.16	229.56	.111E+06	3.0	0.23	616.66	
1.52	229.92	.358E+06	11.5	0.27	519.91	
1.89	230.29	.740E+06	30.1	0.35	410.09	
2.25	230.65	.122E+07	59.5	0.41	342.49	
2.61	231.01	.180E+07	100.6	0.47	298.58	
2.97	231.37	.248E+07	154.6	0.53	267.22	
3.34	231.74	.326E+07	222.9	0.58	243.40	
3.70	232.10	.413E+07	306.6	0.63	224.51	
4.06	232.46	.510E+07	406.9	0.68	209.05	
4.42	232.82	.617E+07	524.8	0.72	196.10	
4.79	233.19	.734E+07	661.4	0.77	185.06	
5.15	233.55	.861E+07	817.9	0.81	175.49	
5.51	233.91	.998E+07	995.2	0.85	167.10	
5.87	234.27	.114E+08	1194.3	0.89	159.67	
6.24	234.64	.130E+08	1416.3	0.93	153.03	
6.60	235.00	.147E+08	1662.2	0.96	147.05	

<---- hydrograph ----->				<-pi pe / channel -->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0011) 3099.17	3.49	23.00	8.99	1.18	0.23
OUTFLOW: ID= 1 (0401) 3099.17	2.54	32.75	8.99	1.09	0.25

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha)=2163.09 Ia (mm)= 5.00 U.H. Tp(hrs)= 25.10	Curve Number (CN)= 69.0 # of Linear Res. (N)= 3.10
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Unit Hyd Qpeak (cms)= 3.379

PEAK FLOW (cms)= 1.737 (i)
TIME TO PEAK (hrs)= 37.000
RUNOFF VOLUME (mm)= 13.334
TOTAL RAINFALL (mm)= 51.240
RUNOFF COEFFICIENT = 0.260

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0402) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0014):	2163.09	1.737	37.00	13.33
+ ID2= 2 (0401):	3099.17	2.543	32.75	8.99
ID = 3 (0402):	5262.26	4.239	33.75	10.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0501) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0303):	21472.61	18.736	32.50	11.33
+ ID2= 2 (0402):	5262.26	4.239	33.75	10.77
ID = 3 (0501):	26734.87	22.955	32.75	11.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0503) IN= 2--> OUT= 1	Routing time step (min)'= 15.00
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DATA FOR SECTION (1.1) ----->			
Distance	Elevation	Manning	
0.00	237.00	0.0700	
425.00	235.00	0.0700	
445.00	234.00	0.0700	
480.00	233.50	0.0700	
1535.00	233.50	0.0700 / 0.0350	Main Channel
1536.50	232.70	0.0350	Main Channel
1537.50	232.70	0.0350	Main Channel
1540.00	233.50	0.0350 / 0.0700	Main Channel
1605.00	233.20	0.0700	
1610.00	233.10	0.0700	
2135.00	233.10	0.0700	
2440.00	233.50	0.0700	
2505.00	233.50	0.0700	
2530.00	234.00	0.0700	
2535.00	235.00	0.0700	
2540.00	236.00	0.0700	
2605.00	238.00	0.0700	
2645.00	241.00	0.0700	

TRAVEL TIME TABLE ----->						
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)	
0.20	232.90	.385E+03	0.2	0.55	38.92	
0.40	233.10	.103E+04	0.6	0.80	26.75	
0.60	233.30	.159E+06	39.5	0.32	67.02	
0.80	233.50	.366E+06	133.3	0.47	45.74	
1.03	233.73	.977E+06	428.7	0.56	37.97	
1.27	233.97	.160E+07	914.3	0.74	29.09	
1.50	234.20	.222E+07	1556.9	0.90	23.78	
1.73	234.43	.285E+07	2336.2	1.05	20.32	
1.97	234.67	.348E+07	3240.2	1.20	17.88	
2.20	234.90	.411E+07	4260.9	1.33	16.07	
2.43	235.13	.474E+07	5359.1	1.45	14.75	
2.67	235.37	.539E+07	6537.6	1.56	13.74	
2.90	235.60	.605E+07	7819.4	1.66	12.90	
3.13	235.83	.673E+07	9203.8	1.76	12.19	
3.37	236.07	.743E+07	10683.6	1.85	11.59	
3.60	236.30	.814E+07	12247.8	1.93	11.07	
3.83	236.53	.887E+07	13912.8	2.01	10.62	
4.07	236.77	.961E+07	15679.0	2.09	10.22	
4.30	237.00	.104E+08	17546.8	2.17	9.85	

<---- hydrograph ----->				<-pi pe / channel -->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0501) *****	22.95	32.75	11.22	0.51	0.43
OUTFLOW: ID= 1 (0503) *****	22.89	33.75	11.22	0.51	0.43

RESERVOIR (0510)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	27.8300	83.9600
3.8000	14.0500	*****	101.7400
11.0800	33.6500	*****	145.2700
20.8900	61.8900	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
*****	22.891	33.75	11.22
*****	19.996	41.25	11.22

PEAK FLOW REDUCTION [Qout/Qin]= 87.35
TIME SHIFT OF PEAK FLOW (min)=450.00
MAXIMUM STORAGE USED (ha. m.) = 59.3163

ROUTE CHN (0513)
IN= 2--> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning	
0.00	237.00	0.0700	
425.00	235.00	0.0700	
445.00	234.00	0.0700	
480.00	233.50	0.0700	
1535.00	233.50	0.0700 / 0.0350	Main Channel
1536.50	232.70	0.0350	Main Channel
1537.50	232.70	0.0350	Main Channel
1540.00	233.50	0.0350 / 0.0700	Main Channel
1605.00	233.20	0.0700	
1610.00	233.10	0.0700	
2135.00	233.10	0.0700	
2440.00	233.50	0.0700	
2505.00	233.50	0.0700	
2530.00	234.00	0.0700	
2535.00	235.00	0.0700	
2540.00	236.00	0.0700	
2605.00	238.00	0.0700	
2645.00	241.00	0.0700	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.20	232.90	.291E+03	0.2	0.55	29.43
0.40	233.10	.777E+03	0.6	0.80	20.23
0.60	233.30	.120E+06	39.5	0.32	50.68
0.80	233.50	.277E+06	133.3	0.47	34.59
1.03	233.73	.739E+06	428.7	0.56	28.72
1.27	233.97	.121E+07	914.3	0.74	22.00
1.50	234.20	.168E+07	1556.9	0.90	17.98
1.73	234.43	.215E+07	2336.2	1.05	15.37
1.97	234.67	.263E+07	3240.2	1.20	13.52
2.20	234.90	.311E+07	4260.9	1.33	12.15
2.43	235.13	.359E+07	5359.1	1.45	11.15
2.67	235.37	.408E+07	6537.6	1.56	10.39
2.90	235.60	.458E+07	7819.4	1.66	9.76
3.13	235.83	.509E+07	9203.8	1.76	9.22
3.37	236.07	.562E+07	10683.6	1.85	8.76
3.60	236.30	.615E+07	12247.8	1.93	8.37
3.83	236.53	.670E+07	13912.8	2.01	8.03
4.07	236.77	.727E+07	15679.0	2.09	7.73
4.30	237.00	.784E+07	17546.8	2.17	7.45

<---- hydrograph ----> <-pi pe / channel -->

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
*****	20.00	41.25	11.22	0.50	0.46
*****	19.98	42.00	11.22	0.50	0.46

ADD HYD (0504)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1059.88	2.358	20.75	15.25
26734.87	19.977	42.00	11.22

ID = 3 (0504): 27794.75 20.293 41.50 11.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0505)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1627.59	1.790	27.50	11.68
27794.75	20.293	41.50	11.37

ID = 3 (0505): 29422.34 21.377 41.00 11.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0506)
IN= 2--> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning	
0.00	251.00	0.0500	
323.00	238.00	0.0500	
400.00	229.00	0.0500	
545.00	228.00	0.0500	
602.00	226.00	0.0500	
604.00	225.80	0.0500 / 0.0350	Main Channel
605.00	225.00	0.0350	Main Channel
609.00	225.00	0.0350	Main Channel
610.00	225.80	0.0350 / 0.0500	Main Channel
614.00	226.00	0.0500	
650.00	227.00	0.0500	
810.00	229.50	0.0500	
1855.00	240.00	0.0500	
2617.00	259.00	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.80	225.80	.292E+05	3.5	0.87	139.36
2.13	227.13	.441E+06	50.6	0.84	145.20
3.45	228.45	.197E+07	262.2	0.97	125.42
4.78	229.78	.557E+07	971.2	1.27	95.57
6.11	231.11	.106E+08	2326.5	1.61	75.63
7.43	232.43	.169E+08	4425.6	1.90	63.77
8.76	233.76	.247E+08	7361.8	2.17	55.90
10.08	235.08	.338E+08	11229.7	2.42	50.22
11.41	236.41	.444E+08	16121.0	2.65	45.87
12.74	237.74	.563E+08	22124.8	2.86	42.40
14.06	239.06	.697E+08	29005.2	3.03	40.02
15.39	240.39	.846E+08	37433.0	3.23	37.66
16.72	241.72	.101E+09	47908.6	3.47	34.97
18.04	243.04	.117E+09	59742.1	3.71	32.73
19.37	244.37	.135E+09	72949.0	3.94	30.83
20.69	245.69	.153E+09	87549.6	4.16	29.20
22.02	247.02	.173E+09	*****	4.37	27.79
23.35	248.35	.193E+09	*****	4.58	26.55
24.67	249.67	.214E+09	*****	4.77	25.45

<---- hydrograph ----> <-pi pe / channel -->

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
*****	21.38	41.00	11.39	1.30	0.86
*****	21.14	43.25	11.39	1.30	0.86

RESERVOIR (0512)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	*****	16.2850
27.2000	3.7650	*****	20.9160
76.8000	7.7410	*****	25.9120
*****	11.0650	*****	36.6600
*****	11.9150	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
*****	*****	*****	*****

INFLOW : ID= 2 (0506) ***** 21.143 43.25 11.39
 OUTFLOW: ID= 1 (0512) ***** 21.138 43.50 11.39

PEAK FLOW REDUCTION [Qout/Qin] (%) = 99.98
 TIME SHI FT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 2.9259

ROUTE CHN (0514)
 IN= 2--> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning	
0.00	230.00	0.0500	
400.00	229.00	0.0500	
545.00	228.00	0.0500	
602.00	226.00	0.0500	
604.00	225.80	0.0500 / 0.0350	Main Channel
605.00	225.00	0.0350	Main Channel
609.00	225.00	0.0350	Main Channel
610.00	225.80	0.0350 / 0.0500	Main Channel
614.00	226.00	0.0500	
650.00	227.00	0.0500	
810.00	229.50	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.20	225.20	243E+04	0.3	0.39	121.91
0.40	225.40	515E+04	1.1	0.59	80.58
0.60	225.60	815E+04	2.1	0.74	64.02
0.80	225.80	114E+05	3.5	0.87	54.69
1.05	226.05	184E+05	6.1	0.94	50.48
1.29	226.29	346E+05	10.3	0.85	55.99
1.54	226.54	620E+05	17.4	0.80	59.38
1.79	226.79	101E+06	28.3	0.81	59.18
2.03	227.03	151E+06	43.8	0.83	57.35
2.28	227.28	215E+06	63.9	0.85	55.99
2.53	227.53	295E+06	91.4	0.89	53.82
2.77	227.77	391E+06	127.0	0.93	51.35
3.02	228.02	504E+06	170.6	0.97	49.24
3.27	228.27	644E+06	215.5	0.96	49.84
3.51	228.51	821E+06	279.7	0.97	48.92
3.76	228.76	103E+07	363.1	1.00	47.47
4.01	229.01	128E+07	466.2	1.04	45.89
4.25	229.25	159E+07	572.0	1.03	46.41
4.50	229.50	198E+07	719.1	1.04	45.95

<----- hydrograph ----->

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0512) *****	21.14	43.50	11.39	1.62	0.80
OUTFLOW: ID= 1 (0514) *****	21.09	44.50	11.39	1.62	0.80

ADD HYD (0507)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0016):	3094.49	4.001	30.00	15.78
+ ID2= 2 (0514):	29422.34	21.094	44.50	11.39
ID = 3 (0507):	32516.83	23.806	42.75	11.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ** SIMULATION NUMBER: 2 **

READ STORM File name: C:\Users\aman\cu\AppData\Local\Temp\6ab08fc7-e8e6-4b3d-8177-e9629b714f53\783dd254
 Ptotal = 72.50 mm Comments: 5yr/24hr

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.25	0.00	6.50	6.16	12.75	2.54	19.00	0.36

0.50	0.36	6.75	6.16	13.00	2.54	19.25	0.36
0.75	0.36	7.00	6.16	13.25	2.54	19.50	0.36
1.00	0.36	7.25	6.16	13.50	2.54	19.75	0.36
1.25	0.36	7.50	6.16	13.75	2.54	20.00	0.36
1.50	0.36	7.75	6.16	14.00	2.54	20.25	0.36
1.75	0.36	8.00	6.16	14.25	2.54	20.50	0.36
2.00	0.36	8.25	6.16	14.50	1.45	20.75	0.36
2.25	0.36	8.50	16.68	14.75	1.45	21.00	0.36
2.50	0.36	8.75	16.68	15.00	1.45	21.25	0.36
2.75	0.36	9.00	16.68	15.25	1.45	21.50	0.36
3.00	0.36	9.25	16.68	15.50	1.45	21.75	0.36
3.25	0.36	9.50	16.68	15.75	1.45	22.00	0.36
3.50	0.36	9.75	16.68	16.00	1.45	22.25	0.36
3.75	0.36	10.00	16.68	16.25	1.45	22.50	0.36
4.00	0.36	10.25	16.68	16.50	0.73	22.75	0.36
4.25	0.36	10.50	4.71	16.75	0.73	23.00	0.36
4.50	2.18	10.75	4.71	17.00	0.73	23.25	0.36
4.75	2.18	11.00	4.71	17.25	0.73	23.50	0.36
5.00	2.18	11.25	4.71	17.50	0.73	23.75	0.36
5.25	2.18	11.50	4.71	17.75	0.73	24.00	0.36
5.50	2.18	11.75	4.71	18.00	0.73	24.25	0.36
5.75	2.18	12.00	4.71	18.25	0.73		
6.00	2.18	12.25	4.71	18.50	0.36		
6.25	2.18	12.50	2.54	18.75	0.36		

CALIB NASHYD (0016) Area (ha)=3094.49 Curve Number (CN)= 74.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U.H. Tp(hrs)= 18.10

Unit Hyd Opeak (cms)= 6.704

PEAK FLOW (cms)= 7.377 (i)
 TIME TO PEAK (hrs)= 29.750
 RUNOFF VOLUME (mm)= 29.068
 TOTAL RAINFALL (mm)= 72.500
 RUNOFF COEFFICIENT = 0.401

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0013) Area (ha)=1627.59 Curve Number (CN)= 65.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U.H. Tp(hrs)= 15.50

Unit Hyd Opeak (cms)= 4.118

PEAK FLOW (cms)= 3.423 (i)
 TIME TO PEAK (hrs)= 27.250
 RUNOFF VOLUME (mm)= 22.305
 TOTAL RAINFALL (mm)= 72.500
 RUNOFF COEFFICIENT = 0.308

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0015) Area (ha)=1059.88 Curve Number (CN)= 73.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U.H. Tp(hrs)= 9.30

Unit Hyd Opeak (cms)= 4.469

PEAK FLOW (cms)= 4.382 (i)
 TIME TO PEAK (hrs)= 20.500
 RUNOFF VOLUME (mm)= 28.222
 TOTAL RAINFALL (mm)= 72.500
 RUNOFF COEFFICIENT = 0.389

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0012) Area (ha)=1501.52 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U.H. Tp(hrs)= 11.50

Unit Hyd Qpeak (cms) = 5.120

PEAK FLOW (cms) = 3.633 (i)
TIME TO PEAK (hrs) = 23.250
RUNOFF VOLUME (mm) = 19.819
TOTAL RAINFALL (mm) = 72.500
RUNOFF COEFFICIENT = 0.273

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0093) Area (ha) = 381.73 Curve Number (CN) = 69.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 3.10
U.H. Tp(hrs) = 14.90

Unit Hyd Qpeak (cms) = 1.005

PEAK FLOW (cms) = 0.936 (i)
TIME TO PEAK (hrs) = 26.750
RUNOFF VOLUME (mm) = 25.087
TOTAL RAINFALL (mm) = 72.500
RUNOFF COEFFICIENT = 0.346

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0010) Area (ha) = 2842.76 Curve Number (CN) = 64.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 3.10
U.H. Tp(hrs) = 10.80

Unit Hyd Qpeak (cms) = 10.322

PEAK FLOW (cms) = 7.933 (i)
TIME TO PEAK (hrs) = 22.250
RUNOFF VOLUME (mm) = 21.658
TOTAL RAINFALL (mm) = 72.500
RUNOFF COEFFICIENT = 0.299

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0005) Area (ha) = 1444.47 Curve Number (CN) = 60.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 3.10
U.H. Tp(hrs) = 8.00

Unit Hyd Qpeak (cms) = 7.080

PEAK FLOW (cms) = 4.575 (i)
TIME TO PEAK (hrs) = 19.250
RUNOFF VOLUME (mm) = 19.238
TOTAL RAINFALL (mm) = 72.500
RUNOFF COEFFICIENT = 0.265

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0001) Area (ha) = 2873.64 Curve Number (CN) = 61.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 3.10
U.H. Tp(hrs) = 11.00

Unit Hyd Qpeak (cms) = 10.244

PEAK FLOW (cms) = 7.220 (i)
TIME TO PEAK (hrs) = 22.500
RUNOFF VOLUME (mm) = 19.819
TOTAL RAINFALL (mm) = 72.500
RUNOFF COEFFICIENT = 0.273

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0002) Area (ha) = 988.72 Curve Number (CN) = 58.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 3.10
U.H. Tp(hrs) = 9.10

Unit Hyd Qpeak (cms) = 4.261

PEAK FLOW (cms) = 2.658 (i)
TIME TO PEAK (hrs) = 20.500
RUNOFF VOLUME (mm) = 18.121
TOTAL RAINFALL (mm) = 72.500
RUNOFF COEFFICIENT = 0.250

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0101)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0001):	2873.64	7.220	22.50	19.82
+ ID2= 2 (0002):	988.72	2.658	20.50	18.12
=====				
ID = 3 (0101):	3862.36	9.878	22.00	19.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0102)
IN= 2---> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning	
0.00	250.00	0.0700	
43.00	240.00	0.0700 / 0.0350	Main Channel
44.00	239.20	0.0350	Main Channel
45.00	239.20	0.0350	Main Channel
46.00	240.00	0.0350 / 0.0700	Main Channel
63.50	241.00	0.0700	
71.00	250.00	0.0700	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.40	239.60	.642E+04	0.5	0.80	221.73
0.80	240.00	.171E+05	1.8	1.15	155.20
1.39	240.59	.764E+05	8.0	1.12	159.62
1.98	241.18	.214E+06	23.7	1.19	150.23
2.56	241.76	.385E+06	51.8	1.44	123.91
3.15	242.35	.575E+06	90.1	1.68	106.38
3.74	242.94	.784E+06	138.5	1.89	94.40
4.33	243.53	.101E+07	196.9	2.08	85.71
4.92	244.12	.126E+07	265.6	2.26	79.08
5.51	244.71	.153E+07	344.7	2.42	73.81
6.09	245.29	.181E+07	434.5	2.57	69.50
6.68	245.88	.212E+07	535.2	2.71	65.89
7.27	246.47	.244E+07	647.3	2.84	62.81
7.86	247.06	.278E+07	771.1	2.97	60.13
8.45	247.65	.314E+07	906.8	3.09	57.77
9.04	248.24	.352E+07	1054.8	3.20	55.67
9.62	248.82	.392E+07	1215.5	3.32	53.79
10.21	249.41	.434E+07	1389.1	3.42	52.09
10.80	250.00	.478E+07	1576.1	3.53	50.53

<---- hydrograph ----> <- pipe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (0101)	3862.36	9.81	22.00	19.38	1.46	1.12
OUTFLOW: ID= 1 (0102)	3862.36	9.30	24.75	19.38	1.44	1.12

CALIB NASHYD (0006) Area (ha) = 1472.14 Curve Number (CN) = 73.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 3.10
U.H. Tp(hrs) = 12.20

Unit Hyd Qpeak (cms) = 4.732

PEAK FLOW (cms) = 4.836 (i)
TIME TO PEAK (hrs) = 23.750
RUNOFF VOLUME (mm) = 28.222
TOTAL RAINFALL (mm) = 72.500
RUNOFF COEFFICIENT = 0.389

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (0064) ID= 1 DT=15.0 min			
Area (ha)	=	32.30	
Total Imp (%)	=	75.00	Dir. Conn. (%) = 70.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	24.22	8.07
Dep. Storage (mm)	1.00	5.00
Average Slope (%)	1.00	2.00
Length (m)	464.04	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	=	16.68	10.54
over (min)	=	15.00	45.00
Storage Coeff. (min)	=	13.13 (ii)	30.50 (ii)
Unit Hyd. Tpeak (min)	=	15.00	45.00
Unit Hyd. peak (cms)	=	0.08	0.03

		TOTALS	(iii)
PEAK FLOW (cms)	=	1.05	0.20
TIME TO PEAK (hrs)	=	10.25	10.25
RUNOFF VOLUME (mm)	=	71.50	28.57
TOTAL RAINFALL (mm)	=	72.50	72.50
RUNOFF COEFFICIENT	=	0.99	0.39

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0527) IN= 2--> OUT= 1 DT= 15.0 min			
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.6100	10.9070
0.1100	10.5752	2.1600	11.0260
0.6300	10.6908	2.7100	11.1114
1.1700	10.7958	3.1600	11.1951

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (0064)	32.300	1.247	10.25	58.62
OUTFLOW: ID= 1 (0527)	32.300	0.019	24.50	49.24

PEAK FLOW REDUCTION [Qout/Qin] (%) = 1.50
TIME SHIFT OF PEAK FLOW (min) = 855.00
MAXIMUM STORAGE USED (ha.m.) = 1.7950

ADD HYD (0518) 1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0006):	1472.14	4.836	23.75	28.22
+ ID2= 2 (0527):	32.30	0.019	24.50	49.24
ID = 3 (0518):	1504.44	4.855	23.75	28.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0103) 1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0102):	3862.36	9.297	24.75	19.38
+ ID2= 2 (0518):	1504.44	4.855	23.75	28.67
ID = 3 (0103):	5366.80	14.129	24.50	21.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0104) 1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0005):	1444.47	4.575	19.25	19.24
+ ID2= 2 (0103):	5366.80	14.129	24.50	21.99
ID = 3 (0104):	6811.27	17.936	23.25	21.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0105) IN= 2--> OUT= 1		Routing time step (min) = 15.00
-------------------------------------	--	---------------------------------

<----- DATA FOR SECTION (1.1) ----->					
Distance	Elevation	Manning			
0.00	237.00	0.1000			
330.00	235.00	0.1000 / 0.0350	Main	Channel	
331.00	234.20	0.0350	Main	Channel	
332.00	234.20	0.0350	Main	Channel	
333.00	235.00	0.0350 / 0.1000	Main	Channel	
425.00	235.10	0.1000			
698.00	236.00	0.1000			
1101.00	239.00	0.1000			

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.13	234.33	.490E+03	0.1	0.64	82.66
0.27	234.47	.112E+04	0.3	0.93	56.72
0.40	234.60	.189E+04	0.7	1.14	45.94
0.53	234.73	.280E+04	1.2	1.33	39.64
0.67	234.87	.385E+04	1.8	1.49	35.34
0.80	235.00	.504E+04	2.6	1.63	32.16
0.95	235.15	.442E+05	6.3	0.45	116.42
1.11	235.31	.128E+06	17.7	0.44	120.51
1.26	235.46	.247E+06	38.2	0.49	107.48
1.42	235.62	.400E+06	69.8	0.55	95.60
1.57	235.77	.589E+06	114.0	0.61	86.08
1.72	235.92	.812E+06	172.6	0.67	78.45
1.88	236.08	.107E+07	250.3	0.74	71.19
2.03	236.23	.135E+07	347.8	0.81	64.71
2.18	236.38	.165E+07	461.9	0.88	59.65
2.34	236.54	.198E+07	593.4	0.95	55.58
2.49	236.69	.233E+07	742.8	1.01	52.21
2.65	236.85	.270E+07	910.6	1.06	49.36
2.80	237.00	.309E+07	1097.6	1.12	46.91

<----- hydrograph ----->						
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (0104)	6811.27	17.94	23.25	21.41	1.11	0.44
OUTFLOW: ID= 1 (0105)	6811.27	17.40	25.50	21.40	1.10	0.44

CALIB NASHYD (0091) ID= 1 DT=15.0 min			
Area (ha)	=	281.97	Curve Number (CN) = 65.0
Ia (mm)	=	5.00	# of Linear Res. (N) = 3.10
U.H. Tp (hrs)	=	5.80	

Unit Hyd Opeak (cms) = 1.906

PEAK FLOW (cms)	=	1.324 (i)
TIME TO PEAK (hrs)	=	16.750
RUNOFF VOLUME (mm)	=	22.305
TOTAL RAINFALL (mm)	=	72.500
RUNOFF COEFFICIENT	=	0.308

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0106) 1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0091):	281.97	1.324	16.75	22.31
+ ID2= 2 (0105):	6811.27	17.399	25.50	21.40
ID = 3 (0106):	7093.24	17.973	25.00	21.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (0073)
ID= 1 DT=15.0 mi n

Area (ha)=	902.64	Curve Number (CN)=	73.0
Ia (mm)=	5.00	# of Linear Res. (N)=	3.10
U. H. Tp (hrs)=	10.70		

Unit Hyd Qpeak (cms)=	3.308
PEAK FLOW (cms)=	3.318 (i)
TIME TO PEAK (hrs)=	22.000
RUNOFF VOLUME (mm)=	28.222
TOTAL RAINFALL (mm)=	72.500
RUNOFF COEFFICIENT =	0.389

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (0074)
ID= 1 DT=15.0 mi n

Area (ha)=	135.70	Dir. Conn. (%)=	70.00
Total Imp (%)=	75.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	101.78	33.92
Dep. Storage (mm)=	1.00	5.00
Average Slope (%)=	1.00	2.00
Length (m)=	951.14	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	16.68	7.60
over (min)	15.00	45.00
Storage Coeff. (min)=	20.20 (ii)	39.98 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.06	0.03

TOTALS
(iii)

PEAK FLOW (cms)=	4.39	0.56	4.947
TIME TO PEAK (hrs)=	10.25	10.50	10.25
RUNOFF VOLUME (mm)=	71.50	20.48	56.19
TOTAL RAINFALL (mm)=	72.50	72.50	72.50
RUNOFF COEFFICIENT =	0.99	0.28	0.78

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 57.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0528)
IN= 2---> OUT= 1
DT= 15.0 mi n

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	6.7900	9.8141
	0.4600	8.4187	9.0800	10.3142
	2.6300	8.9048	11.4000	10.6733
	4.9000	9.3462	13.2700	11.0255
INFLOW : ID= 2 (0074)	135.700	4.947	10.25	56.19
OUTFLOW: ID= 1 (0528)	135.700	0.345	16.50	56.16

PEAK FLOW REDUCTION [Qout/Qin] (%) = 6.96
TIME SHIFT OF PEAK FLOW (min) = 375.00
MAXIMUM STORAGE USED (ha. m.) = 6.3063

ADD HYD (0520)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0073):	902.64	3.318	22.00	28.22
+ ID2= 2 (0528):	135.70	0.345	16.50	56.16
ID = 3 (0520):	1038.34	3.645	22.00	31.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (0073)
ID= 1 DT=15.0 mi n

Area (ha)=	178.90	Dir. Conn. (%)=	9.00
Total Imp (%)=	27.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	48.30	130.60
Dep. Storage (mm)=	1.00	5.00
Average Slope (%)=	1.00	2.00
Length (m)=	1092.09	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	16.68	8.36
over (min)	15.00	45.00
Storage Coeff. (min)=	21.95 (ii)	41.00 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.06	0.03

TOTALS
(iii)

PEAK FLOW (cms)=	0.74	2.38	3.079
TIME TO PEAK (hrs)=	10.25	10.50	10.25
RUNOFF VOLUME (mm)=	71.50	21.71	26.19
TOTAL RAINFALL (mm)=	72.50	72.50	72.50
RUNOFF COEFFICIENT =	0.99	0.30	0.36

***** WARNING: FOR AREAS WITH IMPVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 58.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0529)
IN= 2---> OUT= 1
DT= 15.0 mi n

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	8.9500	9.2195
	0.6100	7.5855	11.9700	10.0426
	3.4700	7.7498	15.0300	10.8266
	6.4600	8.5771	17.5000	11.5931
INFLOW : ID= 2 (0073)	178.900	3.079	10.25	26.19
OUTFLOW: ID= 1 (0529)	178.900	0.291	17.00	26.18

PEAK FLOW REDUCTION [Qout/Qin] (%) = 9.45
TIME SHIFT OF PEAK FLOW (min) = 405.00
MAXIMUM STORAGE USED (ha. m.) = 3.6175

ADD HYD (0522)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0520):	1038.34	3.645	22.00	31.87
+ ID2= 2 (0529):	178.90	0.291	17.00	26.18
ID = 3 (0522):	1217.24	3.917	22.00	31.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (0079)
ID= 1 DT=15.0 mi n

Area (ha)=	156.60	Dir. Conn. (%)=	30.00
Total Imp (%)=	39.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	61.07	95.53
Dep. Storage (mm)=	1.00	5.00
Average Slope (%)=	1.00	2.00
Length (m)=	1021.76	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	16.68	10.34
over (min)	15.00	45.00
Storage Coeff. (min)=	21.09 (ii)	38.59 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00

Unit Hyd. peak (cms) = 0.06 0.03
 PEAK FLOW (cms) = 2.17 2.22 *TOTALS*
 TIME TO PEAK (hrs) = 10.25 10.50 4.375 (iii)
 RUNOFF VOLUME (mm) = 71.50 29.29 41.95
 TOTAL RAINFALL (mm) = 72.50 72.50 72.50
 RUNOFF COEFFICIENT = 0.99 0.40 0.58

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 71.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0530)			
IN= 2---> OUT= 1			
DT= 15.0 min			
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	7.8300	9.2109
0.5300	7.7357	10.4800	9.8961
3.0400	7.9828	13.1600	10.5152
5.6500	8.6540	15.3200	11.1208

INFLOW : ID= 2 (0079) 156.600 4.375 10.25 41.95
 OUTFLOW: ID= 1 (0530) 156.600 0.358 16.75 41.93

PEAK FLOW REDUCTION [Out/Oin] (%) = 8.19
 TIME SHIFT OF PEAK FLOW (min) = 390.00
 MAXIMUM STORAGE USED (ha.m.) = 5.2312

ADD HYD (0524)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0522):	1217.24	3.917	22.00	31.04
+ ID2= 2 (0530):	156.60	0.358	16.75	41.93
=====				
ID = 3 (0524):	1373.84	4.256	21.75	32.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0107)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0106):	7093.24	17.973	25.00	21.44
+ ID2= 2 (0524):	1373.84	4.256	21.75	32.28
=====				
ID = 3 (0107):	8467.08	22.043	24.50	23.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0108)
 IN= 2---> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1.1) ----->				
Distance	Elevation	Manning		
0.00	234.00	0.1000		
400.00	231.00	0.1000 / 0.0350	Main Channel	
401.00	229.70	0.0350	Main Channel	
402.00	229.70	0.0350	Main Channel	
404.00	230.50	0.0350 / 0.1000	Main Channel	
495.00	231.00	0.1000		
1234.00	256.00	0.1000		

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.20	229.90	.151E+04	0.1	0.24	395.61
0.40	230.10	.377E+04	0.2	0.35	274.62
0.60	230.30	.677E+04	0.5	0.43	222.06
0.80	230.50	.105E+05	0.9	0.50	190.52
1.03	230.73	.437E+05	2.0	0.27	356.49

1.27 230.97 .134E+06 5.0 0.21 448.69
 1.50 231.20 .278E+06 11.4 0.23 405.59
 1.73 231.43 .473E+06 21.6 0.26 364.80
 1.97 231.67 .718E+06 36.0 0.29 332.76
 2.20 231.90 .101E+07 55.1 0.31 306.87
 2.43 232.13 .136E+07 79.4 0.33 285.44
 2.67 232.37 .176E+07 109.6 0.36 267.34
 2.90 232.60 .221E+07 145.9 0.38 251.83
 3.13 232.83 .270E+07 189.0 0.40 238.35
 3.37 233.07 .325E+07 239.3 0.42 226.51
 3.60 233.30 .385E+07 297.1 0.44 216.02
 3.83 233.53 .450E+07 363.0 0.46 206.64
 4.07 233.77 .520E+07 437.3 0.48 198.20
 4.30 234.00 .595E+07 520.5 0.50 190.55

<---- hydrograph ---->						
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0107)	8467.08	22.04	24.50	23.20	1.74	0.26
OUTFLOW: ID= 1 (0108)	8467.08	18.53	29.75	23.20	1.66	0.25

CALIB NASHYD (0092)
 ID= 1 DT=15.0 min Area (ha)=1102.32 Curve Number (CN)= 64.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U.H. Tp(hrs)= 16.50

Unit Hyd Qpeak (cms) = 2.620
 PEAK FLOW (cms) = 2.128 (i)
 TIME TO PEAK (hrs) = 28.250
 RUNOFF VOLUME (mm) = 21.658
 TOTAL RAINFALL (mm) = 72.500
 RUNOFF COEFFICIENT = 0.299

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0109)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0092):	1102.32	2.128	28.25	21.66
+ ID2= 2 (0108):	8467.08	18.527	29.75	23.20
=====				
ID = 3 (0109):	9569.40	20.640	29.50	23.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0110)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0010):	2842.76	7.933	22.25	21.66
+ ID2= 2 (0109):	9569.40	20.640	29.50	23.02
=====				
ID = 3 (0110):	12412.16	27.046	27.75	22.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0111)
 IN= 2---> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1.1) ----->				
Distance	Elevation	Manning		
0.00	236.00	0.1000		
275.00	232.00	0.1000		
1039.00	230.00	0.1000 / 0.0350	Main Channel	
1040.00	229.20	0.0350	Main Channel	
1041.00	229.20	0.0350	Main Channel	
1043.00	230.00	0.0350 / 0.1000	Main Channel	
1117.00	230.60	0.1000		
1325.00	234.00	0.1000		

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.20	229.90	.151E+04	0.1	0.24	395.61
0.40	230.10	.377E+04	0.2	0.35	274.62
0.60	230.30	.677E+04	0.5	0.43	222.06
0.80	230.50	.105E+05	0.9	0.50	190.52
1.03	230.73	.437E+05	2.0	0.27	356.49

0.20	229.40	.985E+03	0.0	0.14	431.34
0.40	229.60	.251E+04	0.1	0.20	298.03
0.60	229.80	.457E+04	0.3	0.25	239.97
0.80	230.00	.717E+04	0.6	0.29	205.17
1.07	230.27	.754E+05	2.2	0.11	561.90
1.33	230.53	.272E+06	8.6	0.11	527.12
1.60	230.80	.594E+06	22.7	0.14	436.37
1.87	231.07	.103E+07	46.2	0.16	371.32
2.13	231.33	.158E+07	80.7	0.18	325.64
2.40	231.60	.224E+07	127.8	0.20	291.74
2.67	231.87	.301E+07	189.0	0.22	265.46
2.93	232.13	.389E+07	271.8	0.25	238.27
3.20	232.40	.481E+07	376.4	0.28	212.79
3.47	232.67	.576E+07	496.0	0.31	193.49
3.73	232.93	.674E+07	630.4	0.33	178.30
4.00	233.20	.776E+07	779.4	0.36	165.99
4.27	233.47	.881E+07	943.0	0.38	155.79
4.53	233.73	.990E+07	1121.1	0.41	147.16
4.80	234.00	.110E+08	1313.9	0.43	139.76

----- hydrograph -----						-<- pi pe / channel ->	
AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL		
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW: ID= 2 (0110)	27.05	27.75	22.71	1.65	0.14		
OUTFLOW: ID= 1 (0111)	22.52	33.50	22.71	1.60	0.14		

ADD HYD (0112)
1 + 2 = 3

AREA	OPEAK	TPEAK	R. V.	
(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0093):	381.73	0.936	26.75	25.09
+ ID2= 2 (0111):	12412.16	22.516	33.50	22.71
=====				
ID = 3 (0112):	12793.89	23.314	33.50	22.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0008)
ID= 1 DT=15.0 min

Area (ha)=1549.97	Curve Number (CN)= 60.0
Ia (mm)= 5.00	# of Linear Res. (N)= 3.10
U. H. Tp(hrs)= 9.10	

Unit Hyd Qpeak (cms)= 6.679
PEAK FLOW (cms)= 4.426 (i)
TIME TO PEAK (hrs)= 20.500
RUNOFF VOLUME (mm)= 19.238
TOTAL RAINFALL (mm)= 72.500
RUNOFF COEFFICIENT = 0.265

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0003)
ID= 1 DT=15.0 min

Area (ha)=2674.44	Curve Number (CN)= 64.0
Ia (mm)= 5.00	# of Linear Res. (N)= 3.10
U. H. Tp(hrs)= 13.10	

Unit Hyd Qpeak (cms)= 8.006
PEAK FLOW (cms)= 6.328 (i)
TIME TO PEAK (hrs)= 24.750
RUNOFF VOLUME (mm)= 21.658
TOTAL RAINFALL (mm)= 72.500
RUNOFF COEFFICIENT = 0.299

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (0036)
ID= 1 DT=15.0 min

Area Total Imp(%)= 280.10	Dir. Conn. (%)= 9.00
Imp(%)= 22.00	

IMPERVIOUS	PERVIOUS (i)
(ha)= 61.62	218.48
Dep. Storage (mm)= 1.00	5.00
Average Slope (%)= 1.00	2.00

Length (m)= 1366.50	40.00
Mannings n = 0.013	0.250
Max. Eff. Inten. (mm/hr)= 16.68	10.34
over (mi n) = 30.00	45.00
Storage Coeff. (mi n)= 25.11 (ii)	42.60 (ii)
Unit Hyd. Tpeak (mi n)= 30.00	45.00
Unit Hyd. peak (cms)= 0.04	0.03
PEAK FLOW (cms)= 1.16	4.97
TIME TO PEAK (hrs)= 10.25	10.50
RUNOFF VOLUME (mm)= 71.50	28.81
TOTAL RAINFALL (mm)= 72.50	72.50
RUNOFF COEFFICIENT = 0.99	0.40

TOTALS	6.058 (iii)
6.058	10.25
32.65	32.65
72.50	72.50
0.45	0.45

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0525)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	14.0000	14.7295
0.9500	12.2068	18.7400	16.0462
5.4300	12.3823	23.5200	17.3268
10.1100	13.7195	27.3900	18.5786

AREA	OPEAK	TPEAK	R. V.	
(ha)	(cms)	(hrs)	(mm)	
INFLOW: ID= 2 (0036)	280.100	6.058	10.25	32.65
OUTFLOW: ID= 1 (0525)	280.100	0.554	17.25	32.64

PEAK FLOW REDUCTION [Qout/Qin] (%) = 9.14
TIME SHIFT OF PEAK FLOW (min) = 420.00
MAXIMUM STORAGE USED (ha. m.) = 7.1186

ADD HYD (0515)
1 + 2 = 3

AREA	OPEAK	TPEAK	R. V.	
(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0003):	2674.44	6.328	24.75	21.66
+ ID2= 2 (0525):	280.10	0.554	17.25	32.64
=====				
ID = 3 (0515):	2954.54	6.825	24.75	22.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0004)
ID= 1 DT=15.0 min

Area (ha)=2580.39	Curve Number (CN)= 53.0
Ia (mm)= 5.00	# of Linear Res. (N)= 3.10
U. H. Tp(hrs)= 10.70	

Unit Hyd Qpeak (cms)= 9.457
PEAK FLOW (cms)= 5.202 (i)
TIME TO PEAK (hrs)= 22.250
RUNOFF VOLUME (mm)= 15.564
TOTAL RAINFALL (mm)= 72.500
RUNOFF COEFFICIENT = 0.215

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (0046)
ID= 1 DT=15.0 min

Area Total Imp(%)= 92.30	Dir. Conn. (%)= 33.00
Imp(%)= 44.00	

IMPERVIOUS	PERVIOUS (i)
(ha)= 40.61	51.69
Dep. Storage (mm)= 1.00	5.00
Average Slope (%)= 1.00	2.00

Length (m) = 784.43 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 16.68 8.46
 over (min) = 15.00 45.00

Storage Coeff. (min) = 18.00 (ii) 36.95 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.06 0.03

PEAK FLOW (cms) = 1.41 0.98 *TOTALS*
 TIME TO PEAK (hrs) = 10.25 10.50 2.375 (iii)
 RUNOFF VOLUME (mm) = 71.50 22.88 38.92
 TOTAL RAINFALL (mm) = 72.50 72.50 72.50
 RUNOFF COEFFICIENT = 0.99 0.32 0.54

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 61.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0526)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.6200	51.9904
0.3100	51.1099	6.1800	52.3852
1.7900	51.2815	7.7500	52.7331
3.3300	51.6636	9.0300	53.0736

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (0046)	92.300	2.375	10.25	38.92
OUTFLOW: ID= 1 (0526)	92.300	0.021	25.00	25.53

PEAK FLOW REDUCTION [Qout/Qin] (%) = 0.89
 TIME SHIFT OF PEAK FLOW (min) = 885.00
 MAXIMUM STORAGE USED (ha.m.) = 3.4808

ADD HYD (0516)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0004):	2580.39	5.202	22.25	15.56
+ ID2= 2 (0526):	92.30	0.021	25.00	25.53
ID= 3 (0516):	2672.69	5.223	22.25	15.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0201)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0515):	2954.54	6.825	24.75	22.70
+ ID2= 2 (0516):	2672.69	5.223	22.25	15.91
ID= 3 (0201):	5627.23	11.954	23.50	19.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0202)
 IN= 2--> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning
0.00	239.00	0.1000
690.00	230.00	0.1000 / 0.0350
691.00	229.20	0.0350
692.00	229.20	0.0350
693.00	229.50	0.0350 / 0.1000
1190.00	230.00	0.1000
1417.00	255.00	0.1000

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.30	229.50	.347E+04	0.4	0.84	136.57
0.80	230.00	.864E+06	45.2	0.36	318.78
1.30	230.50	.265E+07	274.1	0.71	161.20
1.80	231.00	.459E+07	651.8	0.97	117.24
2.30	231.50	.667E+07	1165.3	1.20	95.34
2.80	232.00	.889E+07	1810.2	1.40	81.89
3.30	232.50	.113E+08	2585.1	1.57	72.65
3.80	233.00	.138E+08	3490.4	1.73	65.85
4.30	233.50	.165E+08	4527.4	1.89	60.59
4.80	234.00	.193E+08	5697.8	2.03	56.38
5.30	234.50	.222E+08	7003.8	2.16	52.91
5.80	235.00	.253E+08	8447.7	2.28	50.00
6.30	235.50	.286E+08	10032.3	2.40	47.52
6.80	236.00	.320E+08	11760.1	2.52	45.36
7.30	236.50	.356E+08	13634.1	2.63	43.46
7.80	237.00	.393E+08	15657.0	2.73	41.78
8.30	237.50	.431E+08	17831.7	2.84	40.28
8.80	238.00	.471E+08	20161.3	2.93	38.92
9.30	238.50	.512E+08	22648.7	3.03	37.69

<---- hydrograph ----> <- pipe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0201)	5627.23	11.95	23.50	19.47	0.43	0.62
OUTFLOW: ID= 1 (0202)	5627.23	11.29	27.25	19.47	0.42	0.63

ADD HYD (0203)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0008):	1549.97	4.426	20.50	19.24
+ ID2= 2 (0202):	5627.23	11.291	27.25	19.47
ID= 3 (0203):	7177.20	14.797	25.25	19.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0301)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0112):	12793.89	23.314	33.50	22.78
+ ID2= 2 (0203):	7177.20	14.797	25.25	19.42
ID= 3 (0301):	19971.09	35.834	30.50	21.57

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0302)
 IN= 2--> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning
0.00	240.00	0.0700
110.00	239.00	0.0700
161.00	238.00	0.0700
250.00	237.00	0.0700
420.00	236.00	0.0700
560.00	235.00	0.0700
589.00	234.40	0.0700 / 0.0350
589.10	233.60	0.0350
594.00	233.60	0.0350
594.10	234.40	0.0350 / 0.0700
615.00	235.00	0.0700
740.00	236.00	0.0700
860.00	236.50	0.0700
1340.00	237.00	0.0700
1460.00	238.00	0.0700
1470.00	239.00	0.0700
1510.00	240.00	0.0700

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.27	233.87	.770E+04	1.0	0.74	132.49

0.53	234.13	.155E+05	2.9	1.10	88.48
0.80	234.40	.234E+05	5.5	1.37	71.19
1.15	234.75	.636E+05	11.6	1.07	91.11
1.50	235.10	.169E+06	24.5	0.85	114.97
1.85	235.45	.431E+06	54.5	0.74	131.62
2.20	235.80	.882E+06	117.3	0.78	125.39
2.55	236.15	.153E+07	218.3	0.83	117.06
2.90	236.50	.246E+07	375.3	0.89	109.30
3.25	236.85	.394E+07	579.0	0.86	113.45
3.60	237.20	.612E+07	1010.3	0.97	101.01
3.95	237.55	.851E+07	1631.3	1.12	86.99
4.30	237.90	.111E+08	2391.8	1.27	77.05
4.65	238.25	.137E+08	3337.5	1.42	68.52
5.00	238.60	.164E+08	4438.7	1.58	61.71
5.35	238.95	.192E+08	5671.0	1.73	56.40
5.70	239.30	.220E+08	6955.1	1.85	52.75
6.05	239.65	.249E+08	8355.2	1.96	49.76
6.40	240.00	.280E+08	9884.7	2.07	47.18

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0301)	*****	35.83	30.50	21.57	1.63	0.80
OUTFLOW: ID= 1 (0302)	*****	35.20	32.75	21.57	1.62	0.81

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0012):	1501.52	3.633	23.25	19.82
+ ID2= 2 (0302):	19971.09	35.200	32.75	21.57
=====				
ID = 3 (0303):	21472.61	37.646	32.25	21.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0011)	3099.17	57.0
ID= 1 DT=15.0 mi n	U. H. Tp(hrs)= 5.00	# of Linear Res. (N)= 3.10

Unit Hyd Qpeak (cms) = 10.949

PEAK FLOW (cms) = 6.850 (i)
 TIME TO PEAK (hrs) = 22.750
 RUNOFF VOLUME (mm) = 17.584
 TOTAL RAINFALL (mm) = 72.500
 RUNOFF COEFFICIENT = 0.243

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (0401)	Routing time step (min)'
IN= 2--> OUT= 1	15.00

Distance	Elevation	Manning
0.00	236.00	0.0500
72.00	235.00	0.0500
492.00	230.00	0.0500
581.00	229.20	0.0500 / 0.0350
582.00	228.40	0.0350
583.00	228.40	0.0350
584.00	229.20	0.0350 / 0.0500
588.00	229.20	0.0500
607.00	230.00	0.0500
627.00	235.00	0.0500

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.27	228.67	.302E+04	0.1	0.21	666.69
0.53	228.93	.756E+04	0.3	0.30	465.92
0.80	229.20	.136E+05	0.6	0.37	377.99
1.16	229.56	.111E+06	3.0	0.23	616.66
1.52	229.92	.358E+06	11.5	0.27	519.91
1.89	230.29	.740E+06	30.1	0.35	410.09
2.25	230.65	.122E+07	59.5	0.41	342.49

2.61	231.01	.180E+07	100.6	0.47	298.58
2.97	231.37	.248E+07	154.6	0.53	267.22
3.34	231.74	.326E+07	222.9	0.58	243.40
3.70	232.10	.413E+07	306.6	0.63	224.51
4.06	232.46	.510E+07	406.9	0.68	209.05
4.42	232.82	.617E+07	524.8	0.72	196.10
4.79	233.19	.734E+07	661.4	0.77	185.06
5.15	233.55	.861E+07	817.9	0.81	175.49
5.51	233.91	.998E+07	995.2	0.85	167.10
5.87	234.27	.114E+08	1194.3	0.89	159.67
6.24	234.64	.130E+08	1416.3	0.93	153.03
6.60	235.00	.147E+08	1662.2	0.96	147.05

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0011)	3099.17	6.85	22.75	17.58	1.33	0.25
OUTFLOW: ID= 1 (0401)	3099.17	4.86	30.50	17.58	1.24	0.24

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0014)	2163.09	69.0
ID= 1 DT=15.0 mi n	U. H. Tp(hrs)= 5.00	# of Linear Res. (N)= 3.10

Unit Hyd Qpeak (cms) = 3.379

PEAK FLOW (cms) = 3.271 (i)
 TIME TO PEAK (hrs) = 36.750
 RUNOFF VOLUME (mm) = 25.087
 TOTAL RAINFALL (mm) = 72.500
 RUNOFF COEFFICIENT = 0.346

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0014):	2163.09	3.271	36.75	25.09
+ ID2= 2 (0401):	3099.17	4.859	30.50	17.58
=====				
ID = 3 (0402):	5262.26	7.958	32.25	20.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0303):	21472.61	37.646	32.25	21.45
+ ID2= 2 (0402):	5262.26	7.958	32.25	20.67
=====				
ID = 3 (0501):	26734.87	45.604	32.25	21.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0503)	Routing time step (min)'
IN= 2--> OUT= 1	15.00

Distance	Elevation	Manning
0.00	237.00	0.0700
425.00	235.00	0.0700
445.00	234.00	0.0700
480.00	233.50	0.0700
1535.00	233.50	0.0700 / 0.0350
1536.50	232.70	0.0350
1537.50	232.70	0.0350
1540.00	233.50	0.0350 / 0.0700
1605.00	233.20	0.0700
1610.00	233.10	0.0700
2135.00	233.10	0.0700
2440.00	233.50	0.0700
2505.00	233.50	0.0700
2530.00	234.00	0.0700

2535.00 235.00 0.0700
 2540.00 236.00 0.0700
 2605.00 238.00 0.0700
 2645.00 241.00 0.0700

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.20	232.90	385E+03	0.2	0.55	38.92
0.40	233.10	103E+04	0.6	0.80	26.75
0.60	233.30	159E+06	39.5	0.32	67.02
0.80	233.50	366E+06	133.3	0.47	45.74
1.03	233.73	977E+06	428.7	0.56	37.97
1.27	233.97	160E+07	914.3	0.74	29.09
1.50	234.20	222E+07	1556.9	0.90	23.78
1.73	234.43	285E+07	2336.2	1.05	20.32
1.97	234.67	348E+07	3240.2	1.20	17.88
2.20	234.90	411E+07	4260.9	1.33	16.07
2.43	235.13	474E+07	5359.1	1.45	14.75
2.67	235.37	539E+07	6537.6	1.56	13.74
2.90	235.60	605E+07	7819.4	1.66	12.90
3.13	235.83	673E+07	9203.8	1.76	12.19
3.37	236.07	743E+07	10683.6	1.85	11.59
3.60	236.30	814E+07	12247.8	1.93	11.07
3.83	236.53	887E+07	13912.8	2.01	10.62
4.07	236.77	961E+07	15679.0	2.09	10.22
4.30	237.00	104E+08	17546.8	2.17	9.85

<----- hydrograph -----> <-pi pe / channel ->

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0501)	*****	45.60	32.25	21.30	0.61
OUTFLOW: ID= 1 (0503)	*****	45.46	33.00	21.30	0.33

RESERVOIR (0510)
 IN= 2--> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	27.8300	83.9600
3.8000	14.0500	*****	101.7400
11.0800	33.6500	*****	145.2700
20.8900	61.8900	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (0503)	*****	45.462	33.00
OUTFLOW: ID= 1 (0510)	*****	47.061	30.75

PEAK FLOW REDUCTION [Qout/Oi n](%)=103.52
 TIME SHIFT OF PEAK FLOW (mi n)=*****
 MAXIMUM STORAGE USED (ha. m.)= 84.4722

**** WARNING : HYDROGRAPH PEAK WAS NOT REDUCED.
 CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.

ROUTE CHN (0513)
 IN= 2--> OUT= 1

Routing time step (mi n)' = 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning	
0.00	237.00	0.0700	
425.00	235.00	0.0700	
445.00	234.00	0.0700	
480.00	233.50	0.0700	
1535.00	233.50	0.0700 /0.0350	Main Channel
1536.50	232.70	0.0350	Main Channel
1537.50	232.70	0.0350	Main Channel
1540.00	233.50	0.0350 /0.0700	Main Channel
1605.00	233.20	0.0700	
1610.00	233.10	0.0700	
2135.00	233.10	0.0700	
2440.00	233.50	0.0700	
2505.00	233.50	0.0700	
2530.00	234.00	0.0700	
2535.00	235.00	0.0700	
2540.00	236.00	0.0700	
2605.00	238.00	0.0700	
2645.00	241.00	0.0700	

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.20	232.90	291E+03	0.2	0.55	29.43
0.40	233.10	777E+03	0.6	0.80	20.23
0.60	233.30	120E+06	39.5	0.32	50.68
0.80	233.50	277E+06	133.3	0.47	34.59
1.03	233.73	739E+06	428.7	0.56	28.72
1.27	233.97	121E+07	914.3	0.74	22.00
1.50	234.20	168E+07	1556.9	0.90	17.98
1.73	234.43	215E+07	2336.2	1.05	15.37
1.97	234.67	263E+07	3240.2	1.20	13.52
2.20	234.90	311E+07	4260.9	1.33	12.15
2.43	235.13	359E+07	5359.1	1.45	11.15
2.67	235.37	408E+07	6537.6	1.56	10.39
2.90	235.60	458E+07	7819.4	1.66	9.76
3.13	235.83	509E+07	9203.8	1.76	9.22
3.37	236.07	562E+07	10683.6	1.85	8.76
3.60	236.30	615E+07	12247.8	1.93	8.37
3.83	236.53	670E+07	13912.8	2.01	8.03
4.07	236.77	727E+07	15679.0	2.09	7.73
4.30	237.00	784E+07	17546.8	2.17	7.45

<----- hydrograph -----> <-pi pe / channel ->

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0510)	*****	47.06	30.75	21.29	0.62
OUTFLOW: ID= 1 (0513)	*****	45.10	34.75	21.29	0.61

ADD HYD (0504)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0015):	1059.88	4.382	20.50
+ ID2= 2 (0513):	26734.87	45.105	34.75
=====			
ID = 3 (0504):	27794.75	46.651	34.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0505)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0013):	1627.59	3.423	27.25
+ ID2= 2 (0504):	27794.75	46.651	34.25
=====			
ID = 3 (0505):	29422.34	49.590	34.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0506)
 IN= 2--> OUT= 1

Routing time step (mi n)' = 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning	
0.00	251.00	0.0500	
323.00	238.00	0.0500	
400.00	229.00	0.0500	
545.00	228.00	0.0500	
602.00	226.00	0.0500	
604.00	225.80	0.0500 /0.0350	Main Channel
605.00	225.00	0.0350	Main Channel
609.00	225.00	0.0350	Main Channel
610.00	225.80	0.0350 /0.0500	Main Channel
614.00	226.00	0.0500	
650.00	227.00	0.0500	
810.00	229.50	0.0500	
1855.00	240.00	0.0500	
2617.00	259.00	0.0500	

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.80	225.80	292E+05	3.5	0.87	139.36
2.13	227.13	441E+06	50.6	0.84	145.20
3.45	228.45	197E+07	262.2	0.97	125.42
4.78	229.78	557E+07	971.2	1.27	95.57

6.11	231.11	.106E+08	2326.5	1.61	75.63
7.43	232.43	.169E+08	4425.6	1.90	63.77
8.76	233.76	.247E+08	7361.8	2.17	55.90
10.08	235.08	.338E+08	11229.7	2.42	50.22
11.41	236.41	.444E+08	16121.0	2.65	45.87
12.74	237.74	.563E+08	22124.8	2.86	42.40
14.06	239.06	.697E+08	29005.2	3.03	40.02
15.39	240.39	.846E+08	37433.0	3.23	37.66
16.72	241.72	.101E+09	47908.6	3.47	34.97
18.04	243.04	.117E+09	59742.1	3.71	32.73
19.37	244.37	.135E+09	72949.0	3.94	30.83
20.69	245.69	.153E+09	87549.6	4.16	29.20
22.02	247.02	.173E+09	*****	4.37	27.79
23.35	248.35	.193E+09	*****	4.58	26.55
24.67	249.67	.214E+09	*****	4.77	25.45

<---- hydrograph ---->						<-pi pe / channel -->	
AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL		
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW : ID= 2 (0505)	*****	49.59	34.25	21.60	2.10	0.84	
OUTFLOW: ID= 1 (0506)	*****	46.71	37.75	21.60	2.01	0.84	

RESERVOIR (0512)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	*****	16.2850
27.2000	3.7650	*****	20.9160
76.8000	7.7410	*****	25.9120
*****	11.0650	*****	36.6600
*****	11.9150	0.0000	0.0000

AREA	OPEAK	TPEAK	R. V.	
(ha)	(cms)	(hrs)	(mm)	
INFLOW : ID= 2 (0506)	*****	46.708	37.75	21.60
OUTFLOW: ID= 1 (0512)	*****	46.695	38.00	21.60

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 99.97
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha.m.) = 5.3279

ROUTE CHN (0514)
IN= 2--> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1.1) ----->					
Distance	Elevation	Manning			
0.00	230.00	0.0500			
400.00	229.00	0.0500			
545.00	228.00	0.0500			
602.00	226.00	0.0500			
604.00	225.80	0.0500 / 0.0350	Main	Channel	
605.00	225.00	0.0350	Main	Channel	
609.00	225.00	0.0350	Main	Channel	
610.00	225.80	0.0350 / 0.0500	Main	Channel	
614.00	226.00	0.0500			
650.00	227.00	0.0500			
810.00	229.50	0.0500			

----- TRAVEL TIME TABLE -----					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(mi n)
0.20	225.20	.243E+04	0.3	0.39	121.91
0.40	225.40	.515E+04	1.1	0.59	80.58
0.60	225.60	.815E+04	2.1	0.74	64.02
0.80	225.80	.114E+05	3.5	0.87	54.69
1.05	226.05	.184E+05	6.1	0.94	50.48
1.29	226.29	.246E+05	10.3	0.85	55.99
1.54	226.54	.320E+05	17.4	0.80	59.38
1.79	226.79	.401E+05	28.3	0.81	59.18
2.03	227.03	.491E+05	43.8	0.83	57.35
2.28	227.28	.591E+05	63.9	0.85	55.99
2.53	227.53	.701E+05	91.4	0.89	53.82
2.77	227.77	.821E+05	127.0	0.93	51.35
3.02	228.02	.951E+05	170.6	0.97	49.24
3.27	228.27	.109E+06	215.5	0.96	49.84
3.51	228.51	.129E+06	279.7	0.97	48.92
3.76	228.76	.151E+06	363.1	1.00	47.47
4.01	229.01	.176E+06	466.2	1.04	45.89
4.25	229.25	.204E+06	572.0	1.03	46.41

4.50	229.50	.198E+07	719.1	1.04	45.95		
<---- hydrograph ---->						<-pi pe / channel -->	
AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL		
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW : ID= 2 (0512)	*****	46.70	38.00	21.60	2.07	0.83	
OUTFLOW: ID= 1 (0514)	*****	46.42	39.00	21.60	2.06	0.83	

ADD HYD. (0507)					
1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0016):	3094.49	7.377	29.75	29.07	
+ ID2= 2 (0514):	29422.34	46.423	39.00	21.60	
=====	=====	=====	=====	=====	
ID = 3 (0507):	32516.83	52.511	38.75	22.31	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

** SIMULATION NUMBER: 3 **

READ STORM | File name: C:\Users\aman\lucy\AppData
Local\Temp\
6ab08Fc7-e8e6-4b3d-8177-e9629b714f53\6f96d923
Ptotal = 86.66 mm | Comments: 10yr/24hr

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	0.00	6.50	7.37	12.75	3.03	19.00	0.43
0.50	0.43	6.75	7.37	13.00	3.03	19.25	0.43
0.75	0.43	7.00	7.37	13.25	3.03	19.50	0.43
1.00	0.43	7.25	7.37	13.50	3.03	19.75	0.43
1.25	0.43	7.50	7.37	13.75	3.03	20.00	0.43
1.50	0.43	7.75	7.37	14.00	3.03	20.25	0.43
1.75	0.43	8.00	7.37	14.25	3.03	20.50	0.43
2.00	0.43	8.25	7.37	14.50	1.73	20.75	0.43
2.25	0.43	8.50	19.94	14.75	1.73	21.00	0.43
2.50	0.43	8.75	19.94	15.00	1.73	21.25	0.43
2.75	0.43	9.00	19.94	15.25	1.73	21.50	0.43
3.00	0.43	9.25	19.94	15.50	1.73	21.75	0.43
3.25	0.43	9.50	19.94	15.75	1.73	22.00	0.43
3.50	0.43	9.75	19.94	16.00	1.73	22.25	0.43
3.75	0.43	10.00	19.94	16.25	1.73	22.50	0.43
4.00	0.43	10.25	19.94	16.50	0.87	22.75	0.43
4.25	0.43	10.50	5.64	16.75	0.87	23.00	0.43
4.50	2.60	10.75	5.64	17.00	0.87	23.25	0.43
4.75	2.60	11.00	5.64	17.25	0.87	23.50	0.43
5.00	2.60	11.25	5.64	17.50	0.87	23.75	0.43
5.25	2.60	11.50	5.64	17.75	0.87	24.00	0.43
5.50	2.60	11.75	5.64	18.00	0.87	24.25	0.43
5.75	2.60	12.00	5.64	18.25	0.87		
6.00	2.60	12.25	5.64	18.50	0.43		
6.25	2.60	12.50	3.03	18.75	0.43		

CALIB NASHYD (0016) | Area (ha)=3094.49 | Curve Number (CN)= 74.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res. (N)= 3.10
U.H. Tp(hrs)= 18.10

Unit Hyd Opeak (cms)= 6.704

PEAK FLOW (cms)= 9.907 (i)
TIME TO PEAK (hrs)= 29.750
RUNOFF VOLUME (mm)= 39.018
TOTAL RAINFALL (mm)= 86.660
RUNOFF COEFFICIENT = 0.450

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0013) | Area (ha)=1627.59 | Curve Number (CN)= 65.0

| ID= 1 DT=15.0 min | I a (mm)= 5.00 # of Li near Res. (N)= 3.10
 U. H. Tp(hrs)= 15.50

Uni t Hyd Qpeak (cms)= 4.118
 PEAK FLOW (cms)= 4.688 (i)
 TIME TO PEAK (hrs)= 27.250
 RUNOFF VOLUME (mm)= 30.529
 TOTAL RAINFALL (mm)= 86.660
 RUNOFF COEFFICIENT = 0.352

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0015) Area (ha)=1059.88 Curve Number (CN)= 73.0
 ID= 1 DT=15.0 min I a (mm)= 5.00 # of Li near Res. (N)= 3.10
 U. H. Tp(hrs)= 9.30

Uni t Hyd Qpeak (cms)= 4.469
 PEAK FLOW (cms)= 5.908 (i)
 TIME TO PEAK (hrs)= 20.500
 RUNOFF VOLUME (mm)= 37.974
 TOTAL RAINFALL (mm)= 86.660
 RUNOFF COEFFICIENT = 0.438

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0012) Area (ha)=1501.52 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min I a (mm)= 5.00 # of Li near Res. (N)= 3.10
 U. H. Tp(hrs)= 11.50

Uni t Hyd Qpeak (cms)= 5.120
 PEAK FLOW (cms)= 5.015 (i)
 TIME TO PEAK (hrs)= 23.000
 RUNOFF VOLUME (mm)= 27.323
 TOTAL RAINFALL (mm)= 86.660
 RUNOFF COEFFICIENT = 0.315

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0093) Area (ha)= 381.73 Curve Number (CN)= 69.0
 ID= 1 DT=15.0 min I a (mm)= 5.00 # of Li near Res. (N)= 3.10
 U. H. Tp(hrs)= 14.90

Uni t Hyd Qpeak (cms)= 1.005
 PEAK FLOW (cms)= 1.271 (i)
 TIME TO PEAK (hrs)= 26.500
 RUNOFF VOLUME (mm)= 34.061
 TOTAL RAINFALL (mm)= 86.660
 RUNOFF COEFFICIENT = 0.393

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0010) Area (ha)=2842.76 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min I a (mm)= 5.00 # of Li near Res. (N)= 3.10
 U. H. Tp(hrs)= 10.80

Uni t Hyd Qpeak (cms)= 10.322
 PEAK FLOW (cms)= 10.894 (i)
 TIME TO PEAK (hrs)= 22.250
 RUNOFF VOLUME (mm)= 29.699
 TOTAL RAINFALL (mm)= 86.660
 RUNOFF COEFFICIENT = 0.343

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB |

NASHYD (0005) Area (ha)=1444.47 Curve Number (CN)= 60.0
 ID= 1 DT=15.0 min I a (mm)= 5.00 # of Li near Res. (N)= 3.10
 U. H. Tp(hrs)= 8.00

Uni t Hyd Qpeak (cms)= 7.080
 PEAK FLOW (cms)= 6.332 (i)
 TIME TO PEAK (hrs)= 19.250
 RUNOFF VOLUME (mm)= 26.568
 TOTAL RAINFALL (mm)= 86.660
 RUNOFF COEFFICIENT = 0.307

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0001) Area (ha)=2873.64 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min I a (mm)= 5.00 # of Li near Res. (N)= 3.10
 U. H. Tp(hrs)= 11.00

Uni t Hyd Qpeak (cms)= 10.244
 PEAK FLOW (cms)= 9.967 (i)
 TIME TO PEAK (hrs)= 22.500
 RUNOFF VOLUME (mm)= 27.323
 TOTAL RAINFALL (mm)= 86.660
 RUNOFF COEFFICIENT = 0.315

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0002) Area (ha)= 988.72 Curve Number (CN)= 58.0
 ID= 1 DT=15.0 min I a (mm)= 5.00 # of Li near Res. (N)= 3.10
 U. H. Tp(hrs)= 9.10

Uni t Hyd Qpeak (cms)= 4.261
 PEAK FLOW (cms)= 3.689 (i)
 TIME TO PEAK (hrs)= 20.500
 RUNOFF VOLUME (mm)= 25.108
 TOTAL RAINFALL (mm)= 86.660
 RUNOFF COEFFICIENT = 0.290

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0101)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0001):	2873.64	9.967	22.50	27.32
+ ID2= 2 (0002):	988.72	3.689	20.50	25.11

ID = 3 (0101):	3862.36	13.565	21.75	26.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0102) Routing time step (min)' = 15.00
 IN= 2---> OUT= 1

<----- DATA FOR SECTION (1.1) ----->				
Distance	Elevation	Manning		
0.00	250.00	0.0700		
43.00	240.00	0.0700 / 0.0350	Main Channel	
44.00	239.20	0.0350	Main Channel	
45.00	239.20	0.0350	Main Channel	
46.00	240.00	0.0350 / 0.0700	Main Channel	
63.50	241.00	0.0700		
71.00	250.00	0.0700		

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.40	239.60	.642E+04	0.5	0.80	221.73
0.80	240.00	.171E+05	1.8	1.15	155.20
1.39	240.59	.764E+05	8.0	1.12	159.62
1.98	241.18	.214E+06	23.7	1.19	150.23
2.56	241.76	.385E+06	51.8	1.44	123.91

3.15	242.35	.575E+06	90.1	1.68	106.38
3.74	242.94	.784E+06	138.5	1.89	94.40
4.33	243.53	.101E+07	196.9	2.08	85.71
4.92	244.12	.126E+07	265.6	2.26	79.08
5.51	244.71	.153E+07	344.7	2.42	73.81
6.09	245.29	.181E+07	434.5	2.57	69.50
6.68	245.88	.212E+07	535.2	2.71	65.89
7.27	246.47	.244E+07	647.3	2.84	62.81
7.86	247.06	.278E+07	771.1	2.97	60.13
8.45	247.65	.314E+07	906.8	3.09	57.77
9.04	248.24	.352E+07	1054.8	3.20	55.67
9.62	248.82	.392E+07	1215.5	3.32	53.79
10.21	249.41	.434E+07	1389.1	3.42	52.09
10.80	250.00	.478E+07	1576.1	3.53	50.53

		<---- hydrograph ---->				<- pi pe / channel ->	
	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL	
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (0101)	3862.36	13.56	21.75	26.76	1.60	1.14	
OUTFLOW: ID= 1 (0102)	3862.36	12.88	24.75	26.76	1.57	1.14	

CALIB NASHYD (0006)	Area (ha)=1472.14	Curve Number (CN)= 73.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res. (N)= 3.10
	U. H. Tp(hrs)= 12.20	

Unit Hyd Qpeak (cms)=	4.732
PEAK FLOW (cms)=	6.515 (i)
TIME TO PEAK (hrs)=	23.500
RUNOFF VOLUME (mm)=	37.974
TOTAL RAINFALL (mm)=	86.660
RUNOFF COEFFICIENT =	0.438

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (0064)	Area (ha)= 32.30	Dir. Conn. (%)= 70.00
ID= 1 DT=15.0 min	Total Imp(%)= 75.00	

Surface Area (ha)=	24.22	PERVIOUS (i)	8.07
Dep. Storage (mm)=	1.00		5.00
Average Slope (%)=	1.00		2.00
Length (m)=	464.04		40.00
Mannings n =	0.013		0.250
Max. Eff. Inten. (mm/hr)=	19.94		13.97
over (min)=	15.00		30.00
Storage Coeff. (min)=	12.23 (ii)		27.74 (ii)
Unit Hyd. Tpeak (min)=	15.00		30.00
Unit Hyd. peak (cms)=	0.08		0.04

TOTALS			
PEAK FLOW (cms)=	1.25	0.28	1.537 (iii)
TIME TO PEAK (hrs)=	10.25	10.25	10.25
RUNOFF VOLUME (mm)=	85.66	38.32	71.46
TOTAL RAINFALL (mm)=	86.66	86.66	86.66
RUNOFF COEFFICIENT =	0.99	0.44	0.82

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0527)			
IN= 2--> OUT= 1			
DT= 15.0 min			
OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	1.6100	10.9070
0.1100	10.5752	2.1600	11.0260
0.6300	10.6908	2.7100	11.1114
1.1700	10.7958	3.1600	11.1951
AREA	OPEAK	TPEAK	R. V.

INFLOW : ID= 2 (0064)	(ha)	(cms)	(hrs)	(mm)
OUTFLOW: ID= 1 (0527)	32.300	1.537	10.25	71.46
	32.300	0.023	24.50	60.03

PEAK FLOW REDUCTION [Qout/ Qin] (%) = 1.48
 TIME SHIFT OF PEAK FLOW (min) = 855.00
 MAXIMUM STORAGE USED (ha. m.) = 2.1883

ADD HYD (0518)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0006):	1472.14	6.515	23.50	37.97
+ ID2= 2 (0527):	32.30	0.023	24.50	60.03
ID = 3 (0518):	1504.44	6.538	23.50	38.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0103)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0102):	3862.36	12.877	24.75	26.76
+ ID2= 2 (0518):	1504.44	6.538	23.50	38.45
ID = 3 (0103):	5366.80	19.385	24.25	30.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0104)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0005):	1444.47	6.332	19.25	26.57
+ ID2= 2 (0103):	5366.80	19.385	24.25	30.03
ID = 3 (0104):	6811.27	24.675	23.00	29.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0105)		
IN= 2--> OUT= 1		
	Routing time step (min) = 15.00	

<----- DATA FOR SECTION (1.1) ----->		
Distance	Elevation	Manning
0.00	237.00	0.1000
330.00	235.00	0.1000 / 0.0350
331.00	234.20	0.0350
332.00	234.20	0.0350
333.00	235.00	0.0350 / 0.1000
425.00	235.10	0.1000
698.00	236.00	0.1000
1101.00	239.00	0.1000

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.13	234.33	.490E+03	0.1	0.64	82.66
0.27	234.47	.112E+04	0.3	0.93	56.72
0.40	234.60	.189E+04	0.7	1.14	45.94
0.53	234.73	.280E+04	1.2	1.33	39.64
0.67	234.87	.385E+04	1.8	1.49	35.34
0.80	235.00	.504E+04	2.6	1.63	32.16
0.95	235.15	.642E+05	6.3	0.45	116.42
1.11	235.31	.128E+06	17.7	0.44	120.51
1.26	235.46	.247E+06	38.2	0.49	107.48
1.42	235.62	.400E+06	69.8	0.55	95.60
1.57	235.77	.589E+06	114.0	0.61	86.08
1.72	235.92	.812E+06	172.6	0.67	78.45
1.88	236.08	.107E+07	250.3	0.74	71.19
2.03	236.23	.135E+07	347.8	0.81	64.71
2.18	236.38	.165E+07	461.9	0.88	59.65
2.34	236.54	.198E+07	593.4	0.95	55.58
2.49	236.69	.233E+07	742.8	1.01	52.21
2.65	236.85	.270E+07	910.6	1.06	49.36

2.80 237.00 .309E+07 1097.6 1.12 46.91

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0104)	6811.27	24.68	23.00	29.30	1.16	0.45
OUTFLOW: ID= 1 (0105)	6811.27	24.05	25.00	29.30	1.15	0.45

CALIB NASHYD (0091) ID= 1 DT=15.0 min	Area (ha)= 281.97 Ia (mm)= 5.00 U.H. Tp(hrs)= 5.80	Curve Number (CN)= 65.0 # of Linear Res. (N)= 3.10
--	--	---

Unit Hyd Qpeak (cms)= 1.906
 PEAK FLOW (cms)= 1.818 (i)
 TIME TO PEAK (hrs)= 16.750
 RUNOFF VOLUME (mm)= 30.529
 TOTAL RAINFALL (mm)= 86.660
 RUNOFF COEFFICIENT = 0.352

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0106) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0091):	281.97	1.818	16.75	30.53
+ ID2= 2 (0105):	6811.27	24.050	25.00	29.30
=====				
ID = 3 (0106):	7093.24	24.889	24.50	29.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0007) ID= 1 DT=15.0 min	Area (ha)= 902.64 Ia (mm)= 5.00 U.H. Tp(hrs)= 10.70	Curve Number (CN)= 73.0 # of Linear Res. (N)= 3.10
--	---	---

Unit Hyd Qpeak (cms)= 3.308
 PEAK FLOW (cms)= 4.471 (i)
 TIME TO PEAK (hrs)= 22.000
 RUNOFF VOLUME (mm)= 37.974
 TOTAL RAINFALL (mm)= 86.660
 RUNOFF COEFFICIENT = 0.438

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (0074) ID= 1 DT=15.0 min	Area Total (ha)= 135.70 Imp(%)= 75.00	Dir. Conn. (%)= 70.00
--	--	-----------------------

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	101.78	33.92
Dep. Storage (mm)=	1.00	5.00
Average Slope (%)=	1.00	2.00
Length (m)=	951.14	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 19.94 10.35
 over (min) 15.00 45.00
 Storage Coeff. (min)= 18.81 (ii) 36.29 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.06 0.03

TOTALS
 6.038 (iii)

PEAK FLOW (cms)= 5.26 0.79
 TIME TO PEAK (hrs)= 10.25 10.25
 RUNOFF VOLUME (mm)= 85.66 28.10
 TOTAL RAINFALL (mm)= 86.66 86.66
 RUNOFF COEFFICIENT = 0.99 0.32 0.79

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 57.0 Ia = Dep. Storage (Above)

- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0528) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	6.7900	9.8141
	0.4600	8.4187	9.0800	10.3142
	2.6300	8.9048	11.4000	10.6733
	4.9000	9.3462	13.2700	11.0255

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0074)	135.700	6.038	10.25	68.39
OUTFLOW: ID= 1 (0528)	135.700	0.419	16.50	68.36

PEAK FLOW REDUCTION [Qout/Qin] (%) = 6.94
 TIME SHIFT OF PEAK FLOW (min) = 375.00
 MAXIMUM STORAGE USED (ha. m.) = 7.6758

ADD HYD (0520) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0007):	902.64	4.471	22.00	37.97
+ ID2= 2 (0528):	135.70	0.419	16.50	68.36
=====				
ID = 3 (0520):	1038.34	4.869	22.00	41.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (0073) ID= 1 DT=15.0 min	Area Total (ha)= 178.90 Imp(%)= 27.00	Dir. Conn. (%)= 9.00
--	--	----------------------

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	48.30	130.60
Dep. Storage (mm)=	1.00	5.00
Average Slope (%)=	1.00	2.00
Length (m)=	1092.09	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 19.94 11.33
 over (min) 15.00 45.00
 Storage Coeff. (min)= 20.44 (ii) 37.30 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.06 0.03

TOTALS
 4.182 (iii)

PEAK FLOW (cms)= 0.89 3.32
 TIME TO PEAK (hrs)= 10.25 10.50
 RUNOFF VOLUME (mm)= 85.66 29.67
 TOTAL RAINFALL (mm)= 86.66 86.66
 RUNOFF COEFFICIENT = 0.99 0.34 0.40

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 58.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0529) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	8.9500	9.2195
	0.6100	7.5855	11.9700	10.0426
	3.4700	7.7498	15.0300	10.8266
	6.4600	8.5771	17.5000	11.5931

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
--	--------------	----------------	----------------	--------------

INFLOW : ID= 2 (0073) 178.900 4.182 10.25 34.71
 OUTFLOW: ID= 1 (0529) 178.900 0.386 17.00 34.69

PEAK FLOW REDUCTION [Qout/Qin] (%) = 9.24
 TIME SHIFT OF PEAK FLOW (min) = 405.00
 MAXIMUM STORAGE USED (ha. m.) = 4.8027

ADD HYD	(0522)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0520):	1038.34	4.869	22.00	41.94	
+ ID2= 2 (0529):	178.90	0.386	17.00	34.69	
ID = 3 (0522):	1217.24	5.231	21.75	40.88	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANHYD (0079)
 ID= 1 DT=15.0 min
 Area (ha) = 156.60
 Total Imp(%) = 39.00
 Dir. Conn. (%) = 30.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 61.07 95.53
 Dep. Storage (mm) = 1.00 5.00
 Average Slope (%) = 1.00 2.00
 Length (m) = 1021.76 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 19.94 13.67
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 19.64 (ii) 35.28 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.06 0.03

TOTALS

PEAK FLOW (cms) = 2.60 3.03 5.624 (iii)
 TIME TO PEAK (hrs) = 10.25 10.25
 RUNOFF VOLUME (mm) = 85.66 39.22 53.15
 TOTAL RAINFALL (mm) = 86.66 86.66
 RUNOFF COEFFICIENT = 0.99 0.45 0.61

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 71.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0530)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2---> OUT= 1	(cms)	(ha. m.)	(cms)	(ha. m.)
DT= 15.0 min	0.0000	0.0000	7.8300	9.2109
	0.5300	7.7357	10.4800	9.8961
	3.0400	7.9828	13.1600	10.5152
	5.6500	8.6540	15.3200	11.1208

INFLOW : ID= 2 (0079) 156.600 5.624 10.25 53.15
 OUTFLOW: ID= 1 (0530) 156.600 0.455 16.75 53.13

PEAK FLOW REDUCTION [Qout/Qin] (%) = 8.08
 TIME SHIFT OF PEAK FLOW (min) = 390.00
 MAXIMUM STORAGE USED (ha. m.) = 6.6360

ADD HYD	(0524)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0522):	1217.24	5.231	21.75	40.88	
+ ID2= 2 (0530):	156.60	0.455	16.75	53.13	
ID = 3 (0524):	1373.84	5.660	21.75	42.28	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(0107)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0106):	7093.24	24.889	24.50	29.35	
+ ID2= 2 (0524):	1373.84	5.660	21.75	42.28	
ID = 3 (0107):	8467.08	30.338	24.25	31.44	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0108)
 IN= 2---> OUT= 1
 Routing time step (min) = 15.00

Distance	Elevation	Manning	Main Channel
0.00	234.00	0.1000	
400.00	231.00	0.1000 / 0.0350	Main Channel
401.00	229.70	0.0350	Main Channel
402.00	229.70	0.0350	Main Channel
404.00	230.50	0.0350 / 0.1000	Main Channel
495.00	231.00	0.1000	
1234.00	256.00	0.1000	

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)
0.20	229.90	.151E+04	0.1	0.24	395.61
0.40	230.10	.377E+04	0.2	0.35	274.62
0.60	230.30	.677E+04	0.5	0.43	222.06
0.80	230.50	.105E+05	0.9	0.50	190.52
1.03	230.73	.437E+05	2.0	0.27	356.49
1.27	230.97	.134E+06	5.0	0.21	448.69
1.50	231.20	.278E+06	11.4	0.23	405.59
1.73	231.43	.473E+06	21.6	0.26	364.80
1.97	231.67	.718E+06	36.0	0.29	332.76
2.20	231.90	.101E+07	55.1	0.31	306.87
2.43	232.13	.136E+07	79.4	0.33	285.44
2.67	232.37	.176E+07	109.6	0.36	267.34
2.90	232.60	.221E+07	145.9	0.38	251.83
3.13	232.83	.270E+07	189.0	0.40	238.35
3.37	233.07	.325E+07	239.3	0.42	226.51
3.60	233.30	.385E+07	297.1	0.44	216.02
3.83	233.53	.450E+07	363.0	0.46	206.64
4.07	233.77	.520E+07	437.3	0.48	198.20
4.30	234.00	.595E+07	520.5	0.50	190.55

INFLOW : ID= 2 (0107)	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
OUTFLOW: ID= 1 (0108)	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
	8467.08	30.34	24.25	31.44	1.88	0.28
	8467.08	25.66	29.25	31.44	1.80	0.27

CALIB NASHYD (0092)
 ID= 1 DT=15.0 min
 Area (ha) = 1102.32
 Ia (mm) = 5.00
 U.H. Tp(hrs) = 16.50
 Curve Number (CN) = 64.0
 # of Linear Res. (N) = 3.10

Unit Hyd Opeak (cms) = 2.620
 PEAK FLOW (cms) = 2.919 (i)
 TIME TO PEAK (hrs) = 28.250
 RUNOFF VOLUME (mm) = 29.698
 TOTAL RAINFALL (mm) = 86.660
 RUNOFF COEFFICIENT = 0.343

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	(0109)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0092):	1102.32	2.919	28.25	29.70	
+ ID2= 2 (0108):	8467.08	25.661	29.25	31.44	

 ID = 3 (0109): 9569.40 28.570 29.25 31.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (0110)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0010):	2842.76	10.894	22.25	29.70
+ ID2= 2 (0109):	9569.40	28.570	29.25	31.24

ID = 3 (0110):	12412.16	37.523	27.50	30.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (0111)
 IN= 2---> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning	
0.00	236.00	0.1000	
275.00	232.00	0.1000	
1039.00	230.00	0.1000 / 0.0350	Main Channel
1040.00	229.20	0.0350	Main Channel
1041.00	229.20	0.0350	Main Channel
1043.00	230.00	0.0350 / 0.1000	Main Channel
1117.00	230.60	0.1000	
1325.00	234.00	0.1000	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.20	229.40	.985E+03	0.0	0.14	431.34
0.40	229.60	.251E+04	0.1	0.20	298.03
0.60	229.80	.457E+04	0.3	0.25	239.97
0.80	230.00	.717E+04	0.6	0.29	205.17
1.07	230.27	.754E+05	2.2	0.11	561.90
1.33	230.53	.272E+06	8.6	0.11	527.12
1.60	230.80	.594E+06	22.7	0.14	436.37
1.87	231.07	.103E+07	46.2	0.16	371.32
2.13	231.33	.158E+07	80.7	0.18	325.64
2.40	231.60	.224E+07	127.8	0.20	291.74
2.67	231.87	.301E+07	189.0	0.22	265.46
2.93	232.13	.389E+07	271.8	0.25	238.27
3.20	232.40	.481E+07	376.4	0.28	212.79
3.47	232.67	.576E+07	496.0	0.31	193.49
3.73	232.93	.674E+07	630.4	0.33	178.30
4.00	233.20	.776E+07	779.4	0.36	165.99
4.27	233.47	.881E+07	943.0	0.38	155.79
4.53	233.73	.990E+07	1121.1	0.41	147.16
4.80	234.00	.110E+08	1313.9	0.43	139.76

<----- hydrograph -----> <--pi pe / channel -->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0110)	*****	37.52	27.50	30.89	1.77	0.15
OUTFLOW: ID= 1 (0111)	*****	31.41	32.75	30.89	1.70	0.14

 ADD HYD (0112)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0093):	381.73	1.271	26.50	34.06
+ ID2= 2 (0111):	12412.16	31.411	32.75	30.89

ID = 3 (0112):	12793.89	32.530	32.50	30.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB NASHYD (0008)
 ID= 1 DT=15.0 min

Area (ha)=1549.97
 la (mm)= 5.00
 U. H. Tp(hrs)= 9.10

Curve Number (CN)= 60.0
 # of Linear Res. (N)= 3.10

Unit Hyd Opeak (cms)= 6.679

PEAK FLOW (cms)= 6.123 (i)
 TIME TO PEAK (hrs)= 20.500
 RUNOFF VOLUME (mm)= 26.568
 TOTAL RAI NFALL (mm)= 86.660
 RUNOFF COEFFICIENT = 0.307

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB NASHYD (0003)
 ID= 1 DT=15.0 min

Area (ha)=2674.44
 la (mm)= 5.00
 U. H. Tp(hrs)= 13.10

Curve Number (CN)= 64.0
 # of Linear Res. (N)= 3.10

Unit Hyd Opeak (cms)= 8.006

PEAK FLOW (cms)= 8.685 (i)
 TIME TO PEAK (hrs)= 24.750
 RUNOFF VOLUME (mm)= 29.698
 TOTAL RAI NFALL (mm)= 86.660
 RUNOFF COEFFICIENT = 0.343

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB STANDHYD (0036)
 ID= 1 DT=15.0 min

Area (ha)= 280.10
 Total Imp(%)= 22.00
 Dir. Conn.(%)= 9.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	61.62	218.48
Dep. Storage (mm)	1.00	5.00
Average Slope (%)	1.00	2.00
Length (m)	1366.50	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr)	= 19.94	13.70
over (min)	30.00	45.00
Storage Coeff. (min)	= 23.38 (ii)	39.01 (ii)
Unit Hyd. Tpeak (min)	= 30.00	45.00
Unit Hyd. peak (cms)	= 0.04	0.03

PEAK FLOW (cms)	= 1.39	6.78	*TOTALS*
TIME TO PEAK (hrs)	= 10.25	10.50	8.147 (iii)
RUNOFF VOLUME (mm)	= 85.66	38.62	42.86
TOTAL RAI NFALL (mm)	= 86.66	86.66	86.66
RUNOFF COEFFICIENT	= 0.99	0.45	0.49

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.0 la = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR (0525)
 IN= 2---> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	14.0000	14.7295
	0.9500	12.2068	18.7400	16.0462
	5.4300	12.3823	23.5200	17.3268
	10.1100	13.7195	27.3900	18.5786

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (0036)	280.100	8.147	10.25	42.86
OUTFLOW: ID= 1 (0525)	280.100	0.729	17.00	42.85

PEAK FLOW REDUCTION [Qout/Qin](%)= 8.94
 TIME SHIFT OF PEAK FLOW (min)=405.00
 MAXIMUM STORAGE USED (ha. m.)= 9.3626

ADD HYD	(0515)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0003)	2674.44	8.685	24.75	29.70	
+ ID2= 2 (0525)	280.10	0.729	17.00	42.85	

ID = 3 (0515)	2954.54	9.337	24.50	30.94	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	NASHYD	(0004)	Area	(ha)=2580.39	Curve Number	(CN)= 53.0
ID= 1	DT=15.0	min	Ia	(mm)= 5.00	# of Linear Res.	(N)= 3.10
			U.H.	TP(hrs)= 10.70		

Unit Hyd Qpeak (cms)= 9.457

PEAK FLOW (cms)= 7.272 (i)
 TIME TO PEAK (hrs)= 22.250
 RUNOFF VOLUME (mm)= 21.728
 TOTAL RAINFALL (mm)= 86.660
 RUNOFF COEFFICIENT = 0.251

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD	(0046)	Area	(ha)= 92.30	Dir. Conn. (%)= 33.00
ID= 1	DT=15.0	min	Total Imp(%)= 44.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	40.61	51.69
Dep. Storage (mm)=	1.00	5.00
Average Slope (%)=	1.00	2.00
Length (m)=	784.43	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 19.94 over (min)= 15.00
 Storage Coeff. (min)= 16.76 (ii)
 Unit Hyd. Tpeak (min)= 15.00
 Unit Hyd. peak (cms)= 0.07

			TOTALS
PEAK FLOW (cms)=	1.69	1.35	3.037 (iii)
TIME TO PEAK (hrs)=	10.25	10.50	10.25
RUNOFF VOLUME (mm)=	85.66	31.18	49.16
TOTAL RAINFALL (mm)=	86.66	86.66	86.66
RUNOFF COEFFICIENT =	0.99	0.36	0.57

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0526)	IN= 2--> OUT= 1	DT= 15.0	min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
				0.0000	0.0000	4.6200	51.9904
				0.3100	51.1099	6.1800	52.3852
				1.7900	51.2815	7.7500	52.7331
				3.3300	51.6636	9.0300	53.0736

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (0046)	92.300	3.037	10.25	49.16
OUTFLOW: ID= 1 (0526)	92.300	0.027	25.00	32.25

PEAK FLOW REDUCTION [Qout/Qin] (%) = 0.88
 TIME SHIFT OF PEAK FLOW (min) = 885.00
 MAXIMUM STORAGE USED (ha.m.) = 4.3966

ADD HYD	(0516)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3					

	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0004)	2580.39	7.272	22.25	21.73
+ ID2= 2 (0526)	92.30	0.027	25.00	32.25

ID = 3 (0516)	2672.69	7.298	22.25	22.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(0201)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0515)		2954.54	9.337	24.50	30.94
+ ID2= 2 (0516)		2672.69	7.298	22.25	22.09

ID = 3 (0201)		5627.23	16.506	23.50	26.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0202)
 IN= 2--> OUT= 1 Routing time step (min) = 15.00

Distance	Elevation	Manning	
0.00	239.00	0.1000	Main Channel
690.00	230.00	0.0350	Main Channel
691.00	229.20	0.0350	Main Channel
692.00	229.20	0.0350	Main Channel
693.00	229.50	0.0350	Main Channel
1190.00	230.00	0.1000	Main Channel
1417.00	255.00	0.1000	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.30	229.50	.347E+04	0.4	0.84	136.57
0.80	230.00	.864E+06	45.2	0.36	318.78
1.30	230.50	.265E+07	274.1	0.71	161.20
1.80	231.00	.459E+07	651.8	0.97	117.24
2.30	231.50	.667E+07	1165.3	1.20	95.34
2.80	232.00	.889E+07	1810.2	1.40	81.89
3.30	232.50	.113E+08	2585.1	1.57	72.65
3.80	233.00	.138E+08	3490.4	1.73	65.85
4.30	233.50	.165E+08	4527.4	1.89	60.59
4.80	234.00	.193E+08	5697.8	2.03	56.38
5.30	234.50	.222E+08	7003.8	2.16	52.91
5.80	235.00	.253E+08	8447.7	2.28	50.00
6.30	235.50	.286E+08	10032.3	2.40	47.52
6.80	236.00	.320E+08	11760.1	2.52	45.36
7.30	236.50	.356E+08	13634.1	2.63	43.46
7.80	237.00	.393E+08	15657.0	2.73	41.78
8.30	237.50	.431E+08	17831.7	2.84	40.28
8.80	238.00	.471E+08	20161.3	2.93	38.92
9.30	238.50	.512E+08	22648.7	3.03	37.69

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (0201)	5627.23	16.51	23.50	26.74	0.48	0.57
OUTFLOW: ID= 1 (0202)	5627.23	15.43	27.75	26.74	0.47	0.58

ADD HYD	(0203)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0008)		1549.97	6.123	20.50	26.57
+ ID2= 2 (0202)		5627.23	15.426	27.75	26.74

ID = 3 (0203)		7177.20	20.149	25.50	26.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(0301)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3					


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-----
              (ha)      (cms)      (hrs)      (mm)
ID1= 1 (0112): 12793.89 32.530 32.50 30.98
+ ID2= 2 (0203): 7177.20 20.149 25.50 26.70
-----
ID = 3 (0301): 19971.09 50.293 30.00 29.44

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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ROUTE CHN (0302)
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
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<----- DATA FOR SECTION ( 1.1) ----->
Distance      Elevation      Manning
0.00           240.00          0.0700
110.00         239.00          0.0700
161.00         238.00          0.0700
250.00         237.00          0.0700
420.00         236.00          0.0700
560.00         235.00          0.0700
589.00         234.40          0.0700 /0.0350 Main Channel
589.10         233.60          0.0350 Main Channel
594.00         233.60          0.0350 Main Channel
594.10         234.40          0.0350 /0.0700 Main Channel
615.00         235.00          0.0700
740.00         236.00          0.0700
860.00         236.50          0.0700
1340.00        237.00          0.0700
1460.00        238.00          0.0700
1470.00        239.00          0.0700
1510.00        240.00          0.0700

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<----- TRAVEL TIME TABLE ----->
DEPTH      ELEV      VOLUME      FLOW RATE      VELOCITY      TRAV. TIME
(m)         (m)        (cu. m.)    (cms)           (m/s)         (mi n)
0.27 233.87 .770E+04 1.0 0.74 132.49
0.53 234.13 .155E+05 2.9 1.10 88.48
0.80 234.40 .234E+05 5.5 1.37 71.19
1.15 234.75 .636E+05 11.6 1.07 91.11
1.50 235.10 .169E+06 24.5 0.85 114.97
1.85 235.45 .431E+06 54.5 0.74 131.62
2.20 235.80 .882E+06 117.3 0.78 125.39
2.55 236.15 .153E+07 218.3 0.83 117.06
2.90 236.50 .246E+07 375.3 0.89 109.30
3.25 236.85 .394E+07 579.0 0.86 113.45
3.60 237.20 .612E+07 1010.3 0.97 101.01
3.95 237.55 .851E+07 1631.3 1.12 86.99
4.30 237.90 .111E+08 2391.8 1.27 77.05
4.65 238.25 .137E+08 3337.5 1.42 68.52
5.00 238.60 .164E+08 4438.7 1.58 61.71
5.35 238.95 .192E+08 5671.0 1.73 56.40
5.70 239.30 .220E+08 6955.1 1.85 52.75
6.05 239.65 .249E+08 8355.2 1.96 49.76
6.40 240.00 .280E+08 9884.7 2.07 47.18

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<---- hydrograph ----> <--pi pe / channel -->
AREA      OPEAK      TPEAK      R. V.      MAX DEPTH      MAX VEL
(ha)      (cms)      (hrs)      (mm)      (m)            (m/s)
INFLOW : ID= 2 (0301) ***** 50.29 30.00 29.44 1.80 0.75
OUTFLOW: ID= 1 (0302) ***** 49.19 32.75 29.44 1.79 0.76

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ADD HYD (0303)
1 + 2 = 3 |
              AREA      OPEAK      TPEAK      R. V.
              (ha)      (cms)      (hrs)      (mm)
ID1= 1 (0012): 1501.52 5.015 23.00 27.32
+ ID2= 2 (0302): 19971.09 49.192 32.75 29.44
-----
ID = 3 (0303): 21472.61 52.575 31.75 29.30

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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CALLIB (0011)
NASHYD | Area (ha)=3099.17 Curve Number (CN)= 57.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
                  U. H. Tp(hrs)= 11.10

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Unit Hyd Opeak (cms)= 10.949

```

PEAK FLOW      (cms)= 9.517 (i)
TIME TO PEAK  (hrs)= 22.750
RUNOFF VOLUME (mm)= 24.402
TOTAL RAINFALL (mm)= 86.660
RUNOFF COEFFICIENT = 0.282

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(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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-----
ROUTE CHN (0401)
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
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<----- DATA FOR SECTION ( 1.1) ----->
Distance      Elevation      Manning
0.00           236.00          0.0500
72.00          235.00          0.0500
492.00         230.00          0.0500
581.00         229.20          0.0500 /0.0350 Main Channel
582.00         228.40          0.0350 Main Channel
583.00         228.40          0.0350 Main Channel
584.00         229.20          0.0350 /0.0500 Main Channel
588.00         229.20          0.0500
607.00         230.00          0.0500
627.00         235.00          0.0500

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<----- TRAVEL TIME TABLE ----->
DEPTH      ELEV      VOLUME      FLOW RATE      VELOCITY      TRAV. TIME
(m)         (m)        (cu. m.)    (cms)           (m/s)         (mi n)
0.27 228.67 .302E+04 0.1 0.21 666.69
0.53 228.93 .756E+04 0.3 0.30 465.92
0.80 229.20 .136E+05 0.6 0.37 377.99
1.16 229.56 .111E+06 3.0 0.23 616.66
1.52 229.92 .358E+06 11.5 0.27 519.91
1.89 230.29 .740E+06 30.1 0.35 410.09
2.25 230.65 .122E+07 59.5 0.41 342.49
2.61 231.01 .180E+07 100.6 0.47 298.58
2.97 231.37 .248E+07 154.6 0.53 267.22
3.34 231.74 .326E+07 222.9 0.58 243.40
3.70 232.10 .413E+07 306.6 0.63 224.51
4.06 232.46 .510E+07 406.9 0.68 209.05
4.42 232.82 .617E+07 524.8 0.72 196.10
4.79 233.19 .734E+07 661.4 0.77 185.06
5.15 233.55 .861E+07 817.9 0.81 175.49
5.51 233.91 .998E+07 995.2 0.85 167.10
5.87 234.27 .114E+08 1194.3 0.89 159.67
6.24 234.64 .130E+08 1416.3 0.93 153.03
6.60 235.00 .147E+08 1662.2 0.96 147.05

```

```

<---- hydrograph ----> <--pi pe / channel -->
AREA      OPEAK      TPEAK      R. V.      MAX DEPTH      MAX VEL
(ha)      (cms)      (hrs)      (mm)      (m)            (m/s)
INFLOW : ID= 2 (0011) 3099.17 9.52 22.75 24.40 1.44 0.26
OUTFLOW: ID= 1 (0401) 3099.17 6.86 30.00 24.40 1.33 0.25

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```

-----
CALLIB (0014)
NASHYD | Area (ha)=2163.09 Curve Number (CN)= 69.0
ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
                  U. H. Tp(hrs)= 25.10

```

Unit Hyd Opeak (cms)= 3.379

```

PEAK FLOW      (cms)= 4.442 (i)
TIME TO PEAK  (hrs)= 36.750
RUNOFF VOLUME (mm)= 34.061
TOTAL RAINFALL (mm)= 86.660
RUNOFF COEFFICIENT = 0.393

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
ADD HYD (0402)
1 + 2 = 3 |
              AREA      OPEAK      TPEAK      R. V.
              (ha)      (cms)      (hrs)      (mm)
ID1= 1 (0014): 2163.09 4.442 36.75 34.06
+ ID2= 2 (0401): 3099.17 6.858 30.00 24.40
-----
ID = 3 (0402): 5262.26 11.022 31.50 28.37

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	(0501)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0303):	21472.61	52.575	31.75	29.30	
+ ID2= 2 (0402):	5262.26	11.022	31.50	28.37	

ID = 3 (0501):	26734.87	63.596	31.75	29.11	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0503)
IN= 2--> OUT= 1 Routing time step (mi n)'= 15.00

<----- DATA FOR SECTION (1.1) ----->					
Di stance	El evation	Manni ng			
0.00	237.00	0.0700			
425.00	235.00	0.0700			
445.00	234.00	0.0700			
480.00	233.50	0.0700			
1535.00	233.50	0.0700 / 0.0350	Main Channel		
1536.50	232.70	0.0350	Main Channel		
1537.50	232.70	0.0350	Main Channel		
1540.00	233.50	0.0350 / 0.0700	Main Channel		
1605.00	233.20	0.0700			
1610.00	233.10	0.0700			
2135.00	233.10	0.0700			
2440.00	233.50	0.0700			
2505.00	233.50	0.0700			
2530.00	234.00	0.0700			
2535.00	235.00	0.0700			
2540.00	236.00	0.0700			
2605.00	238.00	0.0700			
2645.00	241.00	0.0700			

<----- TRAVEL TIME TABLE ----->						
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCIT Y (m/s)	TRAV. TIME (mi n)	
0.20	232.90	385E+03	0.2	0.55	38.92	
0.40	233.10	103E+04	0.6	0.80	26.75	
0.60	233.30	159E+06	39.5	0.32	67.02	
0.80	233.50	366E+06	133.3	0.47	45.74	
1.03	233.73	977E+06	428.7	0.56	37.97	
1.27	233.97	160E+07	914.3	0.74	29.09	
1.50	234.20	222E+07	1556.9	0.90	23.78	
1.73	234.43	285E+07	2336.2	1.05	20.32	
1.97	234.67	348E+07	3240.2	1.20	17.88	
2.20	234.90	411E+07	4260.9	1.33	16.07	
2.43	235.13	474E+07	5359.1	1.45	14.75	
2.67	235.37	539E+07	6537.6	1.56	13.74	
2.90	235.60	605E+07	7819.4	1.66	12.90	
3.13	235.83	673E+07	9203.8	1.76	12.19	
3.37	236.07	743E+07	10683.6	1.85	11.59	
3.60	236.30	814E+07	12247.8	1.93	11.07	
3.83	236.53	887E+07	13912.8	2.01	10.62	
4.07	236.77	961E+07	15679.0	2.09	10.22	
4.30	237.00	104E+08	17546.8	2.17	9.85	

<----- hydrograph ----->						<- pi pe / channel ->	
INFLOW : ID= 2 (0501)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
OUTFLOW: ID= 1 (0503)	63.43	63.60	31.75	29.11	0.65	0.35	
		63.43	32.75	29.11	0.65	0.35	

RESERVOIR (0510)			
IN= 2--> OUT= 1			
DT= 15.0 mi n			
OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	27.8300	83.9600
3.8000	14.0500	*****	101.7400
11.0800	33.6500	*****	145.2700
20.8900	61.8900	0.0000	0.0000
AREA	OPEAK	TPEAK	R. V.

INFLOW : ID= 2 (0503)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (0510)	63.427	63.425	32.75	29.11
	63.425	32.75	29.11	

PEAK FLOW REDUCTION [Qout/Qi n](%)=100.00
TIME SHIF T OF PEAK FLOW (mi n)= 0.00
MAXI MUM STORAGE USED (ha. m.)= 84.9941

ROUTE CHN (0513)
IN= 2--> OUT= 1 Routing time step (mi n)'= 15.00

<----- DATA FOR SECTION (1.1) ----->					
Di stance	El evation	Manni ng			
0.00	237.00	0.0700			
425.00	235.00	0.0700			
445.00	234.00	0.0700			
480.00	233.50	0.0700			
1535.00	233.50	0.0700 / 0.0350	Main Channel		
1536.50	232.70	0.0350	Main Channel		
1537.50	232.70	0.0350	Main Channel		
1540.00	233.50	0.0350 / 0.0700	Main Channel		
1605.00	233.20	0.0700			
1610.00	233.10	0.0700			
2135.00	233.10	0.0700			
2440.00	233.50	0.0700			
2505.00	233.50	0.0700			
2530.00	234.00	0.0700			
2535.00	235.00	0.0700			
2540.00	236.00	0.0700			
2605.00	238.00	0.0700			
2645.00	241.00	0.0700			

<----- TRAVEL TIME TABLE ----->						
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCIT Y (m/s)	TRAV. TIME (mi n)	
0.20	232.90	291E+03	0.2	0.55	29.43	
0.40	233.10	777E+03	0.6	0.80	20.23	
0.60	233.30	120E+06	39.5	0.32	50.68	
0.80	233.50	277E+06	133.3	0.47	34.59	
1.03	233.73	739E+06	428.7	0.56	28.72	
1.27	233.97	121E+07	914.3	0.74	22.00	
1.50	234.20	168E+07	1556.9	0.90	17.98	
1.73	234.43	215E+07	2336.2	1.05	15.37	
1.97	234.67	263E+07	3240.2	1.20	13.52	
2.20	234.90	311E+07	4260.9	1.33	12.15	
2.43	235.13	359E+07	5359.1	1.45	11.15	
2.67	235.37	408E+07	6537.6	1.56	10.39	
2.90	235.60	458E+07	7819.4	1.66	9.76	
3.13	235.83	509E+07	9203.8	1.76	9.22	
3.37	236.07	562E+07	10683.6	1.85	8.76	
3.60	236.30	615E+07	12247.8	1.93	8.37	
3.83	236.53	670E+07	13912.8	2.01	8.03	
4.07	236.77	727E+07	15679.0	2.09	7.73	
4.30	237.00	784E+07	17546.8	2.17	7.45	

<----- hydrograph ----->						<- pi pe / channel ->	
INFLOW : ID= 2 (0510)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
OUTFLOW: ID= 1 (0513)	63.427	63.35	32.75	29.11	0.65	0.35	
	63.425	33.25	29.11		0.65	0.35	

ADD HYD (0504)					
1 + 2 = 3					
ID1= 1 (0015):	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
+ ID2= 2 (0513):	26734.87	63.349	33.25	29.11	

ID = 3 (0504):	27794.75	65.746	33.00	29.45	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0505)					
1 + 2 = 3					
ID1= 1 (0013):	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
	1627.59	4.688	27.25	30.53	

+ ID2= 2 (0504): 27794.75 65.746 33.00 29.45
 ID = 3 (0505): 29422.34 69.986 32.50 29.51

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0506)
 IN= 2--> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning	
0.00	251.00	0.0500	
323.00	238.00	0.0500	
400.00	229.00	0.0500	
545.00	228.00	0.0500	
602.00	226.00	0.0500	
604.00	225.80	0.0500 / 0.0350	Main Channel
605.00	225.00	0.0350	Main Channel
609.00	225.00	0.0350	Main Channel
610.00	225.80	0.0350 / 0.0500	Main Channel
614.00	226.00	0.0500	
650.00	227.00	0.0500	
810.00	229.50	0.0500	
1855.00	240.00	0.0500	
2617.00	259.00	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.80	225.80	.292E+05	3.5	0.87	139.36
2.13	227.13	.441E+06	50.6	0.84	145.20
3.45	228.45	.197E+07	262.2	0.97	125.42
4.78	229.78	.557E+07	971.2	1.27	95.57
6.11	231.11	.106E+08	2326.5	1.61	75.63
7.43	232.43	.169E+08	4425.6	1.90	63.77
8.76	233.76	.247E+08	7361.8	2.17	55.90
10.08	235.08	.338E+08	11229.7	2.42	50.22
11.41	236.41	.444E+08	16121.0	2.65	45.87
12.74	237.74	.563E+08	22124.8	2.86	42.40
14.06	239.06	.697E+08	29005.2	3.03	40.02
15.39	240.39	.846E+08	37433.0	3.23	37.66
16.72	241.72	.101E+09	47908.6	3.47	34.97
18.04	243.04	.117E+09	59742.1	3.71	32.73
19.37	244.37	.135E+09	72949.0	3.94	30.83
20.69	245.69	.153E+09	87549.6	4.16	29.20
22.02	247.02	.173E+09	*****	4.37	27.79
23.35	248.35	.193E+09	*****	4.58	26.55
24.67	249.67	.214E+09	*****	4.77	25.45

<---- hydrograph ---->

<-pi pe / channel ->

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (0505)	*****	69.99	32.50	29.51	2.25
OUTFLOW: ID= 1 (0506)	*****	67.56	35.75	29.51	2.23

RESERVOIR (0512)
 IN= 2--> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	*****	16.2850
27.2000	3.7650	*****	20.9160
76.8000	7.7410	*****	25.9120
*****	11.0650	*****	36.6600
*****	11.9150	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (0506)	*****	67.563	35.75
OUTFLOW: ID= 1 (0512)	*****	67.549	36.00

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 99.98
 TIME SHI FT OF PEAK FLOW (mi n) = 15.00
 MAXI MUM STORAGE USED (ha. m.) = 6.9998

ROUTE CHN (0514)
 IN= 2--> OUT= 1

Routing time step (mi n)' = 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning	
0.00	230.00	0.0500	
400.00	229.00	0.0500	
545.00	228.00	0.0500	
602.00	226.00	0.0500	
604.00	225.80	0.0500 / 0.0350	Main Channel
605.00	225.00	0.0350	Main Channel
609.00	225.00	0.0350	Main Channel
610.00	225.80	0.0350 / 0.0500	Main Channel
614.00	226.00	0.0500	
650.00	227.00	0.0500	
810.00	229.50	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.20	225.20	.243E+04	0.3	0.39	121.91
0.40	225.40	.515E+04	1.1	0.59	80.58
0.60	225.60	.815E+04	2.1	0.74	64.02
0.80	225.80	.114E+05	3.5	0.87	54.69
1.05	226.05	.184E+05	6.1	0.94	50.48
1.29	226.29	.246E+05	10.3	0.85	55.99
1.54	226.54	.320E+05	17.4	0.80	59.38
1.79	226.79	.401E+05	28.3	0.81	59.18
2.03	227.03	.488E+05	43.8	0.83	57.35
2.28	227.28	.581E+05	63.9	0.85	55.99
2.53	227.53	.680E+05	91.4	0.89	53.82
2.77	227.77	.784E+05	127.0	0.93	51.35
3.02	228.02	.894E+05	170.6	0.97	49.24
3.27	228.27	.101E+06	215.5	0.96	49.84
3.51	228.51	.114E+06	279.7	0.97	48.92
3.76	228.76	.128E+06	363.1	1.00	47.47
4.01	229.01	.143E+06	466.2	1.04	45.89
4.25	229.25	.159E+06	572.0	1.03	46.41
4.50	229.50	.178E+06	719.1	1.04	45.95

<---- hydrograph ---->

<-pi pe / channel ->

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (0512)	*****	67.55	36.00	29.51	2.31
OUTFLOW: ID= 1 (0514)	*****	67.27	37.00	29.51	2.31

ADD HYD (0507)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0016):	3094.49	9.907	29.75
+ ID2= 2 (0514):	29422.34	67.270	37.00
ID = 3 (0507):	32516.83	76.017	36.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ** SIMULATION NUMBER: 4 **

READ STORM

File name: C:\Users\aman\lucv\AppData\Local\Temp\6ab08Fc7-e8e6-4b3d-8177-e9629b714f53\64daf3b4
 Ptotal=104.50 mm
 Comments: 25yr/24hr

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.25	0.00	6.50	8.88	12.75	3.66	19.00	0.52
0.50	0.52	6.75	8.88	13.00	3.66	19.25	0.52
0.75	0.52	7.00	8.88	13.25	3.66	19.50	0.52
1.00	0.52	7.25	8.88	13.50	3.66	19.75	0.52
1.25	0.52	7.50	8.88	13.75	3.66	20.00	0.52
1.50	0.52	7.75	8.88	14.00	3.66	20.25	0.52
1.75	0.52	8.00	8.88	14.25	3.66	20.50	0.52
2.00	0.52	8.25	8.88	14.50	2.09	20.75	0.52
2.25	0.52	8.50	24.04	14.75	2.09	21.00	0.52
2.50	0.52	8.75	24.04	15.00	2.09	21.25	0.52
2.75	0.52	9.00	24.04	15.25	2.09	21.50	0.52
3.00	0.52	9.25	24.04	15.50	2.09	21.75	0.52
3.25	0.52	9.50	24.04	15.75	2.09	22.00	0.52

3.50	0.52	9.75	24.04	16.00	2.09	22.25	0.52
3.75	0.52	10.00	24.04	16.25	2.09	22.50	0.52
4.00	0.52	10.25	24.04	16.50	1.05	22.75	0.52
4.25	0.52	10.50	6.79	16.75	1.05	23.00	0.52
4.50	3.14	10.75	6.79	17.00	1.05	23.25	0.52
4.75	3.14	11.00	6.79	17.25	1.05	23.50	0.52
5.00	3.14	11.25	6.79	17.50	1.05	23.75	0.52
5.25	3.14	11.50	6.79	17.75	1.05	24.00	0.52
5.50	3.14	11.75	6.79	18.00	1.05	24.25	0.52
5.75	3.14	12.00	6.79	18.25	1.05		
6.00	3.14	12.25	6.79	18.50	0.52		
6.25	3.14	12.50	3.66	18.75	0.52		

CALIB NASHYD (0016) Area (ha)=3094.49 Curve Number (CN)= 74.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 18.10

Unit Hyd Qpeak (cms)= 6.704

PEAK FLOW (cms)= 13.323 (i)
 TIME TO PEAK (hrs)= 29.500
 RUNOFF VOLUME (mm)= 52.453
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.502

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0013) Area (ha)=1627.59 Curve Number (CN)= 65.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 15.50

Unit Hyd Qpeak (cms)= 4.118

PEAK FLOW (cms)= 6.437 (i)
 TIME TO PEAK (hrs)= 27.000
 RUNOFF VOLUME (mm)= 41.902
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.401

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0015) Area (ha)=1059.88 Curve Number (CN)= 73.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 9.30

Unit Hyd Qpeak (cms)= 4.469

PEAK FLOW (cms)= 7.975 (i)
 TIME TO PEAK (hrs)= 20.500
 RUNOFF VOLUME (mm)= 51.179
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.490

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0012) Area (ha)=1501.52 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 11.50

Unit Hyd Qpeak (cms)= 5.120

PEAK FLOW (cms)= 6.945 (i)
 TIME TO PEAK (hrs)= 23.000
 RUNOFF VOLUME (mm)= 37.803
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.362

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0093) Area (ha)= 381.73 Curve Number (CN)= 69.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 14.90

Unit Hyd Qpeak (cms)= 1.005

PEAK FLOW (cms)= 1.731 (i)
 TIME TO PEAK (hrs)= 26.500
 RUNOFF VOLUME (mm)= 46.346
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.444

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0010) Area (ha)=2842.76 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 10.80

Unit Hyd Qpeak (cms)= 10.322

PEAK FLOW (cms)= 15.001 (i)
 TIME TO PEAK (hrs)= 22.250
 RUNOFF VOLUME (mm)= 40.847
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.391

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0005) Area (ha)=1444.47 Curve Number (CN)= 60.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 8.00

Unit Hyd Qpeak (cms)= 7.080

PEAK FLOW (cms)= 8.792 (i)
 TIME TO PEAK (hrs)= 19.250
 RUNOFF VOLUME (mm)= 36.827
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.352

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0001) Area (ha)=2873.64 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 11.00

Unit Hyd Qpeak (cms)= 10.244

PEAK FLOW (cms)= 13.804 (i)
 TIME TO PEAK (hrs)= 22.500
 RUNOFF VOLUME (mm)= 37.803
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.362

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0002) Area (ha)= 988.72 Curve Number (CN)= 58.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 9.10

Unit Hyd Qpeak (cms)= 4.261

PEAK FLOW (cms)= 5.139 (i)
 TIME TO PEAK (hrs)= 20.500
 RUNOFF VOLUME (mm)= 34.930
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.334

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0001):	2873.64	13.804	22.50	37.80
+ ID2= 2 (0002):	988.72	5.139	20.50	34.93
=====				
ID = 3 (0101):	3862.36	18.820	21.75	37.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0102)
IN= 2--> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1.1) ----->				
Distance	Elevation	Manning		
0.00	250.00	0.0700		
43.00	240.00	0.0700 / 0.0350	Main	Channel
44.00	239.20	0.0350	Main	Channel
45.00	239.20	0.0350	Main	Channel
46.00	240.00	0.0350 / 0.0700	Main	Channel
63.50	241.00	0.0700		
71.00	250.00	0.0700		

<----- TRAVEL TIME TABLE ----->						
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)	
0.40	239.60	.642E+04	0.5	0.80	221.73	
0.80	240.00	.171E+05	1.8	1.15	155.20	
1.39	240.59	.764E+05	8.0	1.12	159.62	
1.98	241.18	.214E+06	23.7	1.19	150.23	
2.56	241.76	.385E+06	51.8	1.44	123.91	
3.15	242.35	.575E+06	90.1	1.68	106.38	
3.74	242.94	.784E+06	138.5	1.89	94.40	
4.33	243.53	.101E+07	196.9	2.08	85.71	
4.92	244.12	.126E+07	265.6	2.26	79.08	
5.51	244.71	.153E+07	344.7	2.42	73.81	
6.09	245.29	.181E+07	434.5	2.57	69.50	
6.68	245.88	.212E+07	535.2	2.71	65.89	
7.27	246.47	.244E+07	647.3	2.84	62.81	
7.86	247.06	.278E+07	771.1	2.97	60.13	
8.45	247.65	.314E+07	906.8	3.09	57.77	
9.04	248.24	.352E+07	1054.8	3.20	55.67	
9.62	248.82	.392E+07	1215.5	3.32	53.79	
10.21	249.41	.434E+07	1389.1	3.42	52.09	
10.80	250.00	.478E+07	1576.1	3.53	50.53	

<---- hydrograph ---->					<- pi pe / channel ->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (0101)	3862.36	18.82	21.75	37.07	1.79	1.16
OUTFLOW: ID= 1 (0102)	3862.36	17.90	24.50	37.07	1.76	1.16

CALIB NASHYD (0006)
ID= 1 DT=15.0 min Area (ha)=1472.14 Curve Number (CN)= 73.0
U.H. Tp(hrs)= 12.20 # of Linear Res. (N)= 3.10

Unit Hyd Opeak (cms)= 4.732
PEAK FLOW (cms)= 8.790 (i)
TIME TO PEAK (hrs)= 23.500
RUNOFF VOLUME (mm)= 51.179
TOTAL RAINFALL (mm)= 104.500
RUNOFF COEFFICIENT = 0.490

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (0064)
ID= 1 DT=15.0 min Area (ha)= 32.30 Total Imp(%)= 75.00 Dir. Conn.(%)= 70.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 24.22 8.07
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00

Length (m)= 464.04 40.00
Mannings n = 0.013 0.250
Max. Eff. Inten. (mm/hr)= 24.04 18.92
over (min)= 15.00 30.00
Storage Coeff. (min)= 11.35 (ii) 25.08 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04
TOTALS
PEAK FLOW (cms)= 1.51 0.39 1.896 (iii)
TIME TO PEAK (hrs)= 10.25 10.25 10.25
RUNOFF VOLUME (mm)= 103.50 51.51 87.90
TOTAL RAINFALL (mm)= 104.50 104.50 104.50
RUNOFF COEFFICIENT = 0.99 0.49 0.84

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0527)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.6100	10.9070
0.1100	10.5752	2.1600	11.0260
0.6300	10.6908	2.7100	11.1114
1.1700	10.7958	3.1600	11.1951

INFLOW : ID= 2 (0064)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
32.300	32.300	1.896	10.25	87.90
OUTFLOW: ID= 1 (0527)	32.300	0.028	24.50	73.84

PEAK FLOW REDUCTION [Qout/Qin](%)= 1.48
TIME SHIFT OF PEAK FLOW (min)=855.00
MAXIMUM STORAGE USED (ha.m.)= 2.6919

ADD HYD (0518)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (0006):	1472.14	8.790	23.50	51.18
+ ID2= 2 (0527):	32.30	0.028	24.50	73.84
=====				
ID = 3 (0518):	1504.44	8.817	23.50	51.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0103)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (0102):	3862.36	17.900	24.50	37.07
+ ID2= 2 (0518):	1504.44	8.817	23.50	51.67
=====				
ID = 3 (0103):	5366.80	26.684	24.25	41.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0104)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (0005):	1444.47	8.792	19.25	36.83
+ ID2= 2 (0103):	5366.80	26.684	24.25	41.16
=====				
ID = 3 (0104):	6811.27	34.055	22.75	40.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0105)
IN= 2--> OUT= 1

Routing time step (min)' = 15.00

DATA FOR SECTION (1.1)				
Distance	Elevation	Manning		
0.00	237.00	0.1000		
330.00	235.00	0.1000 / 0.0350	Main Channel	
331.00	234.20	0.0350	Main Channel	
332.00	234.20	0.0350	Main Channel	
333.00	235.00	0.0350 / 0.1000	Main Channel	
425.00	235.10	0.1000		
698.00	236.00	0.1000		
1101.00	239.00	0.1000		

TRAVEL TIME TABLE						
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)	
0.13	234.33	.490E+03	0.1	0.64	82.66	
0.27	234.47	.112E+04	0.3	0.93	56.72	
0.40	234.60	.189E+04	0.7	1.14	45.94	
0.53	234.73	.280E+04	1.2	1.33	39.64	
0.67	234.87	.385E+04	1.8	1.49	35.34	
0.80	235.00	.504E+04	2.6	1.63	32.16	
0.95	235.15	.442E+05	6.3	0.45	116.42	
1.11	235.31	.128E+06	17.7	0.44	120.51	
1.26	235.46	.247E+06	38.2	0.49	107.48	
1.42	235.62	.400E+06	69.8	0.55	95.60	
1.57	235.77	.589E+06	114.0	0.61	86.08	
1.72	235.92	.812E+06	172.6	0.67	78.45	
1.88	236.08	.107E+07	250.3	0.74	71.19	
2.03	236.23	.135E+07	347.8	0.81	64.71	
2.18	236.38	.165E+07	461.9	0.88	59.65	
2.34	236.54	.198E+07	593.4	0.95	55.58	
2.49	236.69	.233E+07	742.8	1.01	52.21	
2.65	236.85	.270E+07	910.6	1.06	49.36	
2.80	237.00	.309E+07	1097.6	1.12	46.91	

hydrograph						
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0104)	6811.27	34.05	22.75	40.24	1.23	0.48
OUTFLOW: ID= 1 (0105)	6811.27	33.32	24.50	40.24	1.22	0.47

CALIB NASHYD (0091) Area (ha)= 281.97 Curve Number (CN)= 65.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
U.H. Tp(hrs)= 5.80

Unit Hyd Opeak (cms)= 1.906
PEAK FLOW (cms)= 2.502 (i)
TIME TO PEAK (hrs)= 16.750
RUNOFF VOLUME (mm)= 41.902
TOTAL RAINFALL (mm)= 104.500
RUNOFF COEFFICIENT = 0.401

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0106) 1 + 2 = 3
ID1= 1 (0091): 281.97 2.502 16.75 41.90
+ ID2= 2 (0105): 6811.27 33.316 24.50 40.24
ID = 3 (0106): 7093.24 34.517 24.25 40.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0007) Area (ha)= 902.64 Curve Number (CN)= 73.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
U.H. Tp(hrs)= 10.70

Unit Hyd Opeak (cms)= 3.308
PEAK FLOW (cms)= 6.033 (i)

TIME TO PEAK (hrs)= 22.000
RUNOFF VOLUME (mm)= 51.179
TOTAL RAINFALL (mm)= 104.500
RUNOFF COEFFICIENT = 0.490

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (0074) Area (ha)= 135.70
ID= 1 DT=15.0 min Total Imp(%)= 75.00 Dir. Conn.(%)= 70.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 101.78 33.92
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 951.14 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 24.04 14.14
over (min)= 15.00 45.00
Storage Coeff. (min)= 17.45 (ii) 32.89 (ii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.07 0.03

PEAK FLOW (cms)= 6.34 1.11 *TOTALS*
TIME TO PEAK (hrs)= 10.25 10.25 7.447 (iii)
RUNOFF VOLUME (mm)= 103.50 38.72 84.06
TOTAL RAINFALL (mm)= 104.50 104.50 104.50
RUNOFF COEFFICIENT = 0.99 0.37 0.80

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 57.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0528) IN= 2--> OUT= 1 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	6.7900	9.8141
	0.4600	8.4187	9.0800	10.3142
	2.6300	8.9048	11.4000	10.6733
	4.9000	9.3462	13.2700	11.0255

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (0074)	135.700	7.447	10.25	84.06
OUTFLOW: ID= 1 (0528)	135.700	1.411	13.00	84.03

PEAK FLOW REDUCTION [Qout/Qin](%)= 18.95
TIME SHIFT OF PEAK FLOW (min)= 165.00
MAXIMUM STORAGE USED (ha. m.)= 8.6339

ADD HYD (0520) 1 + 2 = 3
ID1= 1 (0007): 902.64 6.033 22.00 51.18
+ ID2= 2 (0528): 135.70 1.411 13.00 84.03
ID = 3 (0520): 1038.34 6.474 21.75 55.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (0073) Area (ha)= 178.90
ID= 1 DT=15.0 min Total Imp(%)= 27.00 Dir. Conn.(%)= 9.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 48.30 130.60
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 1092.09 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 24.04 15.82
over (min) 15.00 45.00
Storage Coeff. (min)= 18.96 (ii) 33.72 (ii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.06 0.03

PEAK FLOW (cms)= 1.07 4.64
TIME TO PEAK (hrs)= 10.25 10.25
RUNOFF VOLUME (mm)= 103.50 40.71
TOTAL RAINFALL (mm)= 104.50 104.50
RUNOFF COEFFICIENT = 0.99 0.39

TOTALS
5.716 (iii)
10.25
46.36
104.50
0.44

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 58.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0529)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	8.9500	9.2195
0.6100	7.5855	11.9700	10.0426
3.4700	7.7498	15.0300	10.8266
6.4600	8.5771	17.5000	11.5931

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (0073)	178.900	5.716	10.25	46.36
OUTFLOW: ID= 1 (0529)	178.900	0.517	17.00	46.34

PEAK FLOW REDUCTION [Qout/Qin] (%) = 9.04
TIME SHIFT OF PEAK FLOW (min) = 405.00
MAXIMUM STORAGE USED (ha.m.) = 6.4258

ADD HYD (0522)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0520):	1038.34	6.474	21.75	55.47
+ ID2= 2 (0529):	178.90	0.517	17.00	46.34
=====				
ID = 3 (0522):	1217.24	6.958	21.75	54.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (0079)
ID= 1 DT=15.0 min

Area (ha) = 156.60
Total Imp (%) = 39.00
Dir. Conn. (%) = 30.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	61.07	95.53
Dep. Storage (mm)	1.00	5.00
Average Slope (%)	1.00	2.00
Length (m)	1021.76	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 24.04 18.46
over (min) 15.00 45.00
Storage Coeff. (min)= 18.22 (ii) 32.09 (ii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.06 0.03

PEAK FLOW (cms)= 3.13 4.14
TIME TO PEAK (hrs)= 10.25 10.25
RUNOFF VOLUME (mm)= 103.50 52.63
TOTAL RAINFALL (mm)= 104.50 104.50
RUNOFF COEFFICIENT = 0.99 0.50

TOTALS
7.278 (iii)
10.25
67.89
104.50
0.65

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 71.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0530)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	7.8300	9.2109
0.5300	7.7357	10.4800	9.8961
3.0400	7.9828	13.1600	10.5152
5.6500	8.6540	15.3200	11.1208

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (0079)	156.600	7.278	10.25	67.89
OUTFLOW: ID= 1 (0530)	156.600	1.441	13.50	67.87

PEAK FLOW REDUCTION [Qout/Qin] (%) = 19.80
TIME SHIFT OF PEAK FLOW (min) = 195.00
MAXIMUM STORAGE USED (ha.m.) = 7.8256

ADD HYD (0524)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0522):	1217.24	6.958	21.75	54.13
+ ID2= 2 (0530):	156.60	1.441	13.50	67.87
=====				
ID = 3 (0524):	1373.84	7.461	21.75	55.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0107)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0106):	7093.24	34.517	24.25	40.31
+ ID2= 2 (0524):	1373.84	7.461	21.75	55.70
=====				
ID = 3 (0107):	8467.08	41.734	24.00	42.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0108)
IN= 2---> OUT= 1

Routing time step (min)' = 15.00

Distance	Elevation	Manning	
0.00	234.00	0.1000	
400.00	231.00	0.1000 / 0.0350	Main Channel
401.00	229.70	0.0350	Main Channel
402.00	229.70	0.0350	Main Channel
404.00	230.50	0.0350 / 0.1000	Main Channel
495.00	231.00	0.1000	
1234.00	256.00	0.1000	

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.20	229.90	.151E+04	0.1	0.24	395.61
0.40	230.10	.377E+04	0.2	0.35	274.62
0.60	230.30	.677E+04	0.5	0.43	222.06
0.80	230.50	.105E+05	0.9	0.50	190.52
1.03	230.73	.437E+05	2.0	0.27	356.49
1.27	230.97	.134E+06	5.0	0.21	448.69
1.50	231.20	.278E+06	11.4	0.23	405.59
1.73	231.43	.473E+06	21.6	0.26	364.80
1.97	231.67	.718E+06	36.0	0.29	332.76
2.20	231.90	.101E+07	55.1	0.31	306.87
2.43	232.13	.136E+07	79.4	0.33	285.44
2.67	232.37	.176E+07	109.6	0.36	267.34
2.90	232.60	.221E+07	145.9	0.38	251.83
3.13	232.83	.270E+07	189.0	0.40	238.35
3.37	233.07	.325E+07	239.3	0.42	226.51
3.60	233.30	.385E+07	297.1	0.44	216.02
3.83	233.53	.450E+07	363.0	0.46	206.64

4.07 233.77 .520E+07 437.3 0.48 198.20
 4.30 234.00 .595E+07 520.5 0.50 190.55

<---- hydrograph ----> <-pi pe / channel ->
 AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0107) 8467.08 41.73 24.00 42.80 2.04 0.29
 OUTFLOW: ID= 1 (0108) 8467.08 35.81 28.75 42.80 1.96 0.28

CALIB NASHYD (0092) Area (ha)=1102.32 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U.H. Tp(hrs)= 16.50

Unit Hyd Qpeak (cms)= 2.620

PEAK FLOW (cms)= 4.017 (i)
 TIME TO PEAK (hrs)= 28.000
 RUNOFF VOLUME (mm)= 40.847
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.391

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0109) | AREA OPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0092): 1102.32 4.017 28.00 40.85
 + ID2= 2 (0108): 8467.08 35.806 28.75 42.80
 ID = 3 (0109): 9569.40 39.818 28.75 42.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0110) | AREA OPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0010): 2842.76 15.001 22.25 40.85
 + ID2= 2 (0109): 9569.40 39.818 28.75 42.58
 ID = 3 (0110): 12412.16 52.383 27.00 42.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0111) | Routing time step (min)' = 15.00
 IN= 2----> OUT= 1

<----- DATA FOR SECTION (1.1) ----->
 Distance Elevation Manning
 0.00 236.00 0.1000
 275.00 232.00 0.1000
 1039.00 230.00 0.1000 /0.0350 Main Channel
 1040.00 229.20 0.0350 Main Channel
 1041.00 229.20 0.0350 Main Channel
 1043.00 230.00 0.0350 /0.1000 Main Channel
 1117.00 230.60 0.1000
 1325.00 234.00 0.1000

<----- TRAVEL TIME TABLE ----->
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV. TIME
 (m) (m) (cu. m.) (cms) (m/s) (mi n)
 0.20 229.40 .985E+03 0.0 0.14 431.34
 0.40 229.60 .251E+04 0.1 0.20 298.03
 0.60 229.80 .457E+04 0.3 0.25 239.97
 0.80 230.00 .717E+04 0.6 0.29 205.17
 1.07 230.27 .754E+05 2.2 0.11 561.90
 1.33 230.53 .272E+06 8.6 0.11 527.12
 1.60 230.80 .594E+06 22.7 0.14 436.37
 1.87 231.07 .103E+07 46.2 0.16 371.32
 2.13 231.33 .158E+07 80.7 0.18 325.64
 2.40 231.60 .224E+07 127.8 0.20 291.74
 2.67 231.87 .301E+07 189.0 0.22 265.46
 2.93 232.13 .389E+07 271.8 0.25 238.27

3.20 232.40 .481E+07 376.4 0.28 212.79
 3.47 232.67 .576E+07 496.0 0.31 193.49
 3.73 232.93 .674E+07 630.4 0.33 178.30
 4.00 233.20 .776E+07 779.4 0.36 165.99
 4.27 233.47 .881E+07 943.0 0.38 155.79
 4.53 233.73 .990E+07 1121.1 0.41 147.16
 4.80 234.00 .110E+08 1313.9 0.43 139.76

<---- hydrograph ----> <-pi pe / channel ->
 AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0110) ***** 52.38 27.00 42.18 1.91 0.16
 OUTFLOW: ID= 1 (0111) ***** 44.61 32.00 42.18 1.85 0.16

ADD HYD (0112) | AREA OPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0093): 381.73 1.731 26.50 46.35
 + ID2= 2 (0111): 12412.16 44.607 32.00 42.18
 ID = 3 (0112): 12793.89 46.168 31.50 42.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0008) Area (ha)=1549.97 Curve Number (CN)= 60.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U.H. Tp(hrs)= 9.10

Unit Hyd Qpeak (cms)= 6.679

PEAK FLOW (cms)= 8.499 (i)
 TIME TO PEAK (hrs)= 20.250
 RUNOFF VOLUME (mm)= 36.827
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.352

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0003) Area (ha)=2674.44 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U.H. Tp(hrs)= 13.10

Unit Hyd Qpeak (cms)= 8.006

PEAK FLOW (cms)= 11.954 (i)
 TIME TO PEAK (hrs)= 24.750
 RUNOFF VOLUME (mm)= 40.847
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.391

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (0036) Area (ha)= 280.10
 ID= 1 DT=15.0 min Total Imp(%)= 22.00 Di r. Conn. (%)= 9.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 61.62 218.48
 Dep. Storage (mm)= 1.00 5.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 1366.50 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 24.04 18.53
 over (mi n) 15.00 45.00
 Storage Coeff. (mi n)= 21.69 (ii) 35.55 (ii)
 Unit Hyd. Tpeak (mi n)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.06 0.03

PEAK FLOW (cms)= 1.68 9.30 *TOTALS*
 TIME TO PEAK (hrs)= 10.25 10.25 10.980 (iii)
 RUNOFF VOLUME (mm)= 103.50 51.90 56.54

TOTAL RAINFALL (mm)= 104.50 104.50 104.50
 RUNOFF COEFFICIENT = 0.99 0.50 0.54

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0525)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	14.0000	14.7295
0.9500	12.2068	18.7400	16.0462
5.4300	12.3823	23.5200	17.3268
10.1100	13.7195	27.3900	18.5786

INFLOW : ID= 2 (0036)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
280.100	280.100	10.980	10.25	56.54
OUTFLOW: ID= 1 (0525)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
280.100	280.100	1.433	15.75	56.53

PEAK FLOW REDUCTION [Qout/Oin] (%) = 13.05
 TIME SHIFT OF PEAK FLOW (min) = 330.00
 MAXIMUM STORAGE USED (ha.m.) = 12.2284

ADD HYD (0515)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0003):	2674.44	11.954	24.75	40.85
+ ID2= 2 (0525):	280.10	1.433	15.75	56.53
ID = 3 (0515):	2954.54	12.803	24.50	42.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0004)
 ID= 1 DT=15.0 min

Area (ha)=2580.39 Curve Number (CN)= 53.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U.H. Tp(hrs)= 10.70

Unit Hyd Qpeak (cms)= 9.457

PEAK FLOW (cms)= 10.213 (i)
 TIME TO PEAK (hrs)= 22.250
 RUNOFF VOLUME (mm)= 30.486
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.292

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (0046)
 ID= 1 DT=15.0 min

Area (ha)= 92.30
 Total Imp(%)= 44.00 Dir. Conn. (%) = 33.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	40.61	51.69
Dep. Storage	1.00	5.00
Average Slope	1.00	2.00
Length	784.43	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 24.04 15.87
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 15.55 (ii) 30.29 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.07 0.03

TOTALS
 (cms)= 2.03 1.89
 (hrs)= 10.25 10.25
 (mm)= 103.50 42.64 62.72

TOTAL RAINFALL (mm)= 104.50 104.50 104.50
 RUNOFF COEFFICIENT = 0.99 0.41 0.60

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0526)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.6200	51.9904
0.3100	51.1099	6.1800	52.3852
1.7900	51.2815	7.7500	52.7331
3.3300	51.6636	9.0300	53.0736

INFLOW : ID= 2 (0046)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
92.300	92.300	3.926	10.25	62.72
OUTFLOW: ID= 1 (0526)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
92.300	92.300	0.034	25.00	41.15

PEAK FLOW REDUCTION [Qout/Oin] (%) = 0.87
 TIME SHIFT OF PEAK FLOW (min) = 885.00
 MAXIMUM STORAGE USED (ha.m.) = 5.6105

ADD HYD (0516)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0004):	2580.39	10.213	22.25	30.49
+ ID2= 2 (0526):	92.30	0.034	25.00	41.15
ID = 3 (0516):	2672.69	10.247	22.25	30.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0201)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0515):	2954.54	12.803	24.50	42.33
+ ID2= 2 (0516):	2672.69	10.247	22.25	30.85
ID = 3 (0201):	5627.23	22.871	23.25	36.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0202)
 IN= 2---> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning
0.00	239.00	0.1000
690.00	230.00	0.1000 / 0.0350
691.00	229.20	0.0350
692.00	229.20	0.0350
693.00	229.50	0.0350 / 0.1000
1190.00	230.00	0.1000
1417.00	255.00	0.1000

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.30	229.50	.347E+04	0.4	0.84	136.57
0.80	230.00	.864E+06	45.2	0.36	318.78
1.30	230.50	.265E+07	274.1	0.71	161.20
1.80	231.00	.459E+07	651.8	0.97	117.24
2.30	231.50	.667E+07	1165.3	1.20	95.34
2.80	232.00	.889E+07	1810.2	1.40	81.89
3.30	232.50	.119E+08	2585.1	1.57	72.65
3.80	233.00	.138E+08	3490.4	1.73	65.85
4.30	233.50	.165E+08	4527.4	1.89	60.59
4.80	234.00	.193E+08	5697.8	2.03	56.38

5.30	234.50	.222E+08	7003.8	2.16	52.91
5.80	235.00	.253E+08	8447.7	2.28	50.00
6.30	235.50	.286E+08	10032.3	2.40	47.52
6.80	236.00	.320E+08	11760.1	2.52	45.36
7.30	236.50	.356E+08	13634.1	2.63	43.46
7.80	237.00	.393E+08	15657.0	2.73	41.78
8.30	237.50	.431E+08	17831.7	2.84	40.28
8.80	238.00	.471E+08	20161.3	2.93	38.92
9.30	238.50	.512E+08	22648.7	3.03	37.69

<---- hydrograph ---->					<-pi pe / channel ->	
AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL	
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW: ID= 2 (0201)	5627.23	22.87	23.25	36.88	0.55	0.50
OUTFLOW: ID= 1 (0202)	5627.23	21.09	28.25	36.88	0.53	0.52

ADD HYD (0203)				
1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0008):	1549.97	8.499	20.25	36.83
+ ID2= 2 (0202):	5627.23	21.089	28.25	36.88
=====				
ID = 3 (0203):	7177.20	27.409	25.75	36.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0301)				
1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0112):	12793.89	46.168	31.50	42.30
+ ID2= 2 (0203):	7177.20	27.409	25.75	36.87
=====				
ID = 3 (0301):	19971.09	71.147	29.75	40.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0302)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->			
Distance	Elevation	Manning	
0.00	240.00	0.0700	
110.00	239.00	0.0700	
161.00	238.00	0.0700	
250.00	237.00	0.0700	
420.00	236.00	0.0700	
560.00	235.00	0.0700	
589.00	234.40	0.0700 / 0.0350	Main Channel
589.10	233.60	0.0350	Main Channel
594.00	233.60	0.0350	Main Channel
594.10	234.40	0.0350 / 0.0700	Main Channel
615.00	235.00	0.0700	
740.00	236.00	0.0700	
860.00	236.50	0.0700	
1340.00	237.00	0.0700	
1460.00	238.00	0.0700	
1470.00	239.00	0.0700	
1510.00	240.00	0.0700	

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(mi n)
0.27	233.87	.770E+04	1.0	0.74	132.49
0.53	234.13	.155E+05	2.9	1.10	88.48
0.80	234.40	.234E+05	5.5	1.37	71.19
1.15	234.75	.636E+05	11.6	1.07	91.11
1.50	235.10	.169E+06	24.5	0.85	114.97
1.85	235.45	.431E+06	54.5	0.74	131.62
2.20	235.80	.882E+06	117.3	0.78	125.39
2.55	236.15	.153E+07	218.3	0.83	117.06
2.90	236.50	.246E+07	375.3	0.89	109.30
3.25	236.85	.394E+07	579.0	0.86	113.45
3.60	237.20	.612E+07	1010.3	0.97	101.01
3.95	237.55	.851E+07	1631.3	1.12	86.99
4.30	237.90	.111E+08	2391.8	1.27	77.05

4.65	238.25	.137E+08	3337.5	1.42	68.52
5.00	238.60	.164E+08	4438.7	1.58	61.71
5.35	238.95	.192E+08	5671.0	1.73	56.40
5.70	239.30	.220E+08	6955.1	1.85	52.75
6.05	239.65	.249E+08	8355.2	1.96	49.76
6.40	240.00	.280E+08	9884.7	2.07	47.18

<---- hydrograph ---->						<-pi pe / channel ->	
AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL		
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW: ID= 2 (0301)	*****	71.15	29.75	40.35	1.94	0.75	
OUTFLOW: ID= 1 (0302)	*****	69.76	32.00	40.35	1.93	0.75	

ADD HYD (0303)				
1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0012):	1501.52	6.945	23.00	37.80
+ ID2= 2 (0302):	19971.09	69.758	32.00	40.35
=====				
ID = 3 (0303):	21472.61	74.634	31.50	40.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0011) Area (ha)=3099.17 Curve Number (CN)= 57.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
U.H. Tp(hrs)= 11.10

Unit Hyd Opeak (cms)= 10.949

PEAK FLOW (cms)= 13.278 (i)
TIME TO PEAK (hrs)= 22.500
RUNOFF VOLUME (mm)= 34.008
TOTAL RAINFALL (mm)= 104.500
RUNOFF COEFFICIENT = 0.325

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (0401)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->			
Distance	Elevation	Manning	
0.00	236.00	0.0500	
72.00	235.00	0.0500	
492.00	230.00	0.0500	
581.00	229.20	0.0500 / 0.0350	Main Channel
582.00	228.40	0.0350	Main Channel
583.00	228.40	0.0350	Main Channel
584.00	229.20	0.0350 / 0.0500	Main Channel
588.00	229.20	0.0500	
607.00	230.00	0.0500	
627.00	235.00	0.0500	

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(mi n)
0.27	228.67	.302E+04	0.1	0.21	666.69
0.53	228.93	.756E+04	0.3	0.30	465.92
0.80	229.20	.136E+05	0.6	0.37	377.99
1.16	229.56	.111E+06	3.0	0.23	616.66
1.52	229.92	.358E+06	11.5	0.27	519.91
1.89	230.29	.740E+06	30.1	0.35	410.09
2.25	230.65	.122E+07	59.5	0.41	342.49
2.61	231.01	.180E+07	100.6	0.47	298.58
2.97	231.37	.248E+07	154.6	0.53	267.22
3.34	231.74	.326E+07	222.9	0.58	243.40
3.70	232.10	.413E+07	306.6	0.63	224.51
4.06	232.46	.510E+07	406.9	0.68	209.05
4.42	232.82	.617E+07	524.8	0.72	196.10
4.79	233.19	.734E+07	661.4	0.77	185.06
5.15	233.55	.861E+07	817.9	0.81	175.49
5.51	233.91	.998E+07	995.2	0.85	167.10
5.87	234.27	.114E+08	1194.3	0.89	159.67
6.24	234.64	.130E+08	1416.3	0.93	153.03
6.60	235.00	.147E+08	1662.2	0.96	147.05

AREA <---- hydrograph ----> <-pi pe / channel ->
 OPEAK TPEAK R. V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0011) 3099.17 13.28 22.50 34.01 1.56 0.28
 OUTFLOW: ID= 1 (0401) 3099.17 9.81 29.00 34.01 1.45 0.26

CALIB NASHYD (0014) Area (ha)=2163.09 Curve Number (CN)= 69.0
 ID= 1 DT=15.0 mi n Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U.H. Tp(hrs)= 25.10

Unit Hyd Qpeak (cms)= 3.379

PEAK FLOW (cms)= 6.045 (i)
 TIME TO PEAK (hrs)= 36.500
 RUNOFF VOLUME (mm)= 46.346
 TOTAL RAINFALL (mm)= 104.500
 RUNOFF COEFFICIENT = 0.444

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0402) |
 1 + 2 = 3 | AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0014): 2163.09 6.045 36.50 46.35
 + ID2= 2 (0401): 3099.17 9.807 29.00 34.01
 ID = 3 (0402): 5262.26 15.378 31.00 39.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0501) |
 1 + 2 = 3 | AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0303): 21472.61 74.634 31.50 40.17
 + ID2= 2 (0402): 5262.26 15.378 31.00 39.08
 ID = 3 (0501): 26734.87 90.004 31.25 39.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0503) |
 IN= 2---> OUT= 1 | Routing time step (mi n)' = 15.00

<----- DATA FOR SECTION (1.1) ----->
 Di stance El evation Manning
 0.00 237.00 0.0700
 425.00 235.00 0.0700
 445.00 234.00 0.0700
 480.00 233.50 0.0700
 1535.00 233.50 0.0700 /0.0350 Mai n Channel
 1536.50 232.70 0.0350 Mai n Channel
 1537.50 232.70 0.0350 Mai n Channel
 1540.00 233.50 0.0350 /0.0700 Mai n Channel
 1605.00 233.20 0.0700
 1610.00 233.10 0.0700
 2135.00 233.10 0.0700
 2440.00 233.50 0.0700
 2505.00 233.50 0.0700
 2530.00 234.00 0.0700
 2535.00 235.00 0.0700
 2540.00 236.00 0.0700
 2605.00 238.00 0.0700
 2645.00 241.00 0.0700

<----- TRAVEL TIME TABLE ----->
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV. TIME
 (m) (m) (cu. m.) (cms) (m/s) (mi n)
 0.20 232.90 385E+03 0.2 0.55 38.92
 0.40 233.10 103E+04 0.6 0.80 26.75
 0.60 233.30 159E+06 39.5 0.32 67.02
 0.80 233.50 366E+06 133.3 0.47 45.74

1.03 233.73 .977E+06 428.7 0.56 37.97
 1.27 233.97 .160E+07 914.3 0.74 29.09
 1.50 234.20 .222E+07 1556.9 0.90 23.78
 1.73 234.43 .285E+07 2336.2 1.05 20.32
 1.97 234.67 .348E+07 3240.2 1.20 17.88
 2.20 234.90 .411E+07 4260.9 1.33 16.07
 2.43 235.13 .474E+07 5359.1 1.45 14.75
 2.67 235.37 .539E+07 6537.6 1.56 13.74
 2.90 235.60 .605E+07 7819.4 1.66 12.90
 3.13 235.83 .673E+07 9203.8 1.76 12.19
 3.37 236.07 .743E+07 10683.6 1.85 11.59
 3.60 236.30 .814E+07 12247.8 1.93 11.07
 3.83 236.53 .887E+07 13912.8 2.01 10.62
 4.07 236.77 .961E+07 15679.0 2.09 10.22
 4.30 237.00 .104E+08 17546.8 2.17 9.85

AREA <---- hydrograph ----> <-pi pe / channel ->
 OPEAK TPEAK R. V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0501) ***** 90.00 31.25 39.96 0.71 0.39
 OUTFLOW: ID= 1 (0503) ***** 89.85 32.00 39.96 0.71 0.38

RESERVOIR (0510) |
 IN= 2---> OUT= 1 |
 DT= 15.0 mi n |
 OUTFLOW STORAGE OUTFLOW STORAGE
 (cms) (ha. m.) (cms) (ha. m.)
 0.0000 0.0000 27.8300 83.9600
 3.8000 14.0500 ***** 101.7400
 11.0800 33.6500 ***** 145.2700
 20.8900 61.8900 0.0000 0.0000

AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (0503) ***** 89.848 32.00 39.96
 OUTFLOW: ID= 1 (0510) ***** 89.848 32.00 39.96

PEAK FLOW REDUCTION [Qout/Qi n](%)=100.00
 TIME SHIF T OF PEAK FLOW (mi n)= 0.00
 MAXIMUM STORAGE USED (ha. m.) = 85.7617

**** WARNING : HYDROGRAPH PEAK WAS NOT REDUCED.
 CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.

ROUTE CHN (0513) |
 IN= 2---> OUT= 1 | Routing time step (mi n)' = 15.00

<----- DATA FOR SECTION (1.1) ----->
 Di stance El evation Manning
 0.00 237.00 0.0700
 425.00 235.00 0.0700
 445.00 234.00 0.0700
 480.00 233.50 0.0700
 1535.00 233.50 0.0700 /0.0350 Mai n Channel
 1536.50 232.70 0.0350 Mai n Channel
 1537.50 232.70 0.0350 Mai n Channel
 1540.00 233.50 0.0350 /0.0700 Mai n Channel
 1605.00 233.20 0.0700
 1610.00 233.10 0.0700
 2135.00 233.10 0.0700
 2440.00 233.50 0.0700
 2505.00 233.50 0.0700
 2530.00 234.00 0.0700
 2535.00 235.00 0.0700
 2540.00 236.00 0.0700
 2605.00 238.00 0.0700
 2645.00 241.00 0.0700

<----- TRAVEL TIME TABLE ----->
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV. TIME
 (m) (m) (cu. m.) (cms) (m/s) (mi n)
 0.20 232.90 291E+03 0.2 0.55 29.43
 0.40 233.10 777E+03 0.6 0.80 20.23
 0.60 233.30 120E+06 39.5 0.32 50.68
 0.80 233.50 277E+06 133.3 0.47 34.59
 1.03 233.73 739E+06 428.7 0.56 28.72
 1.27 233.97 121E+07 914.3 0.74 22.00
 1.50 234.20 168E+07 1556.9 0.90 17.98
 1.73 234.43 215E+07 2336.2 1.05 15.37
 1.97 234.67 263E+07 3240.2 1.20 13.52
 2.20 234.90 311E+07 4260.9 1.33 12.15

2.43	235.13	.359E+07	5359.1	1.45	11.15
2.67	235.37	.408E+07	6537.6	1.56	10.39
2.90	235.60	.458E+07	7819.4	1.66	9.76
3.13	235.83	.509E+07	9203.8	1.76	9.22
3.37	236.07	.562E+07	10683.6	1.85	8.76
3.60	236.30	.615E+07	12247.8	1.93	8.37
3.83	236.53	.670E+07	13912.8	2.01	8.03
4.07	236.77	.727E+07	15679.0	2.09	7.73
4.30	237.00	.784E+07	17546.8	2.17	7.45

<--- hydrograph --->					<-pi pe / channel -->	
AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL	
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (0510)	*****	89.85	32.00	39.95	0.71	0.38
OUTFLOW: ID= 1 (0513)	*****	89.81	32.75	39.95	0.71	0.38

ADD HYD (0504)				
1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0015):	1059.88	7.975	20.50	51.18
+ ID2= 2 (0513):	26734.87	89.812	32.75	39.95

ID = 3 (0504):	27794.75	93.295	32.00	40.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0505)				
1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0013):	1627.59	6.437	27.00	41.90
+ ID2= 2 (0504):	27794.75	93.295	32.00	40.38

ID = 3 (0505):	29422.34	99.280	31.75	40.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0506)		Routing time step (min)' = 15.00		
IN= 2-->	OUT= 1			

<--- DATA FOR SECTION (1.1) --->				
Distance	Elevation	Manning		
0.00	251.00	0.0500		
323.00	238.00	0.0500		
400.00	229.00	0.0500		
545.00	228.00	0.0500		
602.00	226.00	0.0500		
604.00	225.80	0.0500 / 0.0350	Main Channel	
605.00	225.00	0.0350	Main Channel	
609.00	225.00	0.0350	Main Channel	
610.00	225.80	0.0350 / 0.0500	Main Channel	
614.00	226.00	0.0500		
650.00	227.00	0.0500		
810.00	229.50	0.0500		
1855.00	240.00	0.0500		
2617.00	259.00	0.0500		

<--- TRAVEL TIME TABLE --->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(mi n)
0.80	225.80	.292E+05	3.5	0.87	139.36
2.13	227.13	.441E+06	50.6	0.84	145.20
3.45	228.45	.197E+07	262.2	0.97	125.42
4.78	229.78	.557E+07	971.2	1.27	95.57
6.11	231.11	.106E+08	2326.5	1.61	75.63
7.43	232.43	.169E+08	4425.6	1.90	63.77
8.76	233.76	.247E+08	7361.8	2.17	55.90
10.08	235.08	.338E+08	11229.7	2.42	50.22
11.41	236.41	.444E+08	16121.0	2.65	45.87
12.74	237.74	.563E+08	22124.8	2.86	42.40
14.06	239.06	.697E+08	29005.2	3.03	40.02
15.39	240.39	.846E+08	37433.0	3.23	37.66
16.72	241.72	.101E+09	47908.6	3.47	34.97
18.04	243.04	.117E+09	59742.1	3.71	32.73
19.37	244.37	.135E+09	72949.0	3.94	30.83
20.69	245.69	.153E+09	87549.6	4.16	29.20

22.02	247.02	.173E+09	*****	4.37	27.79
23.35	248.35	.193E+09	*****	4.58	26.55
24.67	249.67	.214E+09	*****	4.77	25.45

<--- hydrograph --->					<-pi pe / channel -->	
AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL	
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (0505)	*****	99.28	31.75	40.47	2.43	0.86
OUTFLOW: ID= 1 (0506)	*****	96.54	34.50	40.47	2.41	0.86

RESERVOIR (0512)				
IN= 2-->	OUT= 1			
DT= 15.0 min	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha. m.)	(cms)	(ha. m.)
	0.0000	0.0000	*****	16.2850
	27.2000	3.7650	*****	20.9160
	76.8000	7.7410	*****	25.9120
	*****	11.0650	*****	36.6600
	*****	11.9150	0.0000	0.0000

INFLOW : ID= 2 (0506)				
AREA	OPEAK	TPEAK	R. V.	
(ha)	(cms)	(hrs)	(mm)	
*****	96.537	34.50	40.47	
OUTFLOW: ID= 1 (0512)	*****	96.517	34.50	40.47

PEAK FLOW REDUCTION [Out/Oi n] (%) = 99.98
 TIME SHIFT OF PEAK FLOW (mi n) = 0.00
 MAXIMUM STORAGE USED (ha. m.) = 9.0397

ROUTE CHN (0514)		Routing time step (min)' = 15.00		
IN= 2-->	OUT= 1			

<--- DATA FOR SECTION (1.1) --->				
Distance	Elevation	Manning		
0.00	230.00	0.0500		
400.00	229.00	0.0500		
545.00	228.00	0.0500		
602.00	226.00	0.0500		
604.00	225.80	0.0500 / 0.0350	Main Channel	
605.00	225.00	0.0350	Main Channel	
609.00	225.00	0.0350	Main Channel	
610.00	225.80	0.0350 / 0.0500	Main Channel	
614.00	226.00	0.0500		
650.00	227.00	0.0500		
810.00	229.50	0.0500		

<--- TRAVEL TIME TABLE --->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(mi n)
0.20	225.20	.243E+04	0.3	0.39	121.91
0.40	225.40	.515E+04	1.1	0.59	80.58
0.60	225.60	.815E+04	2.1	0.74	64.02
0.80	225.80	.114E+05	3.5	0.87	54.69
1.05	226.05	.184E+05	6.1	0.94	50.48
1.29	226.29	.246E+05	10.3	0.85	55.99
1.54	226.54	.320E+05	17.4	0.80	59.38
1.79	226.79	.401E+05	28.3	0.81	59.18
2.03	227.03	.491E+05	43.8	0.83	57.35
2.28	227.28	.591E+05	63.9	0.85	55.99
2.53	227.53	.701E+05	91.4	0.89	53.82
2.77	227.77	.821E+05	127.0	0.93	51.35
3.02	228.02	.941E+05	170.6	0.97	49.24
3.27	228.27	.106E+06	215.5	0.96	49.84
3.51	228.51	.121E+06	279.7	0.97	48.92
3.76	228.76	.135E+06	363.1	1.00	47.47
4.01	229.01	.153E+06	466.2	1.04	45.89
4.25	229.25	.171E+06	572.0	1.03	46.41
4.50	229.50	.198E+06	719.1	1.04	45.95

<--- hydrograph --->					<-pi pe / channel -->	
AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL	
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (0512)	*****	96.52	34.50	40.47	2.56	0.89
OUTFLOW: ID= 1 (0514)	*****	96.18	35.50	40.46	2.56	0.89

ADD HYD (0507)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0016):	3094.49	13.323	29.50	52.45
ID2= 2 (0514):	29422.34	96.177	35.50	40.46
ID = 3 (0507):	32516.83	108.431	35.00	41.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ** SIMULATION NUMBER: 5 **

READ STORM
 Ptotal =117.80 mm

Filename: C:\Users\aman\cu\AppData\Local\Temp\6ab08Fc7-e8e6-4b3d-8177-e9629b714f53\081bb95F
 Comments: 50yr/24hr

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.25	0.00	6.50	10.01	12.75	4.12	19.00	0.59
0.50	0.59	6.75	10.01	13.00	4.12	19.25	0.59
0.75	0.59	7.00	10.01	13.25	4.12	19.50	0.59
1.00	0.59	7.25	10.01	13.50	4.12	19.75	0.59
1.25	0.59	7.50	10.01	13.75	4.12	20.00	0.59
1.50	0.59	7.75	10.01	14.00	4.12	20.25	0.59
1.75	0.59	8.00	10.01	14.25	4.12	20.50	0.59
2.00	0.59	8.25	10.01	14.50	2.36	20.75	0.59
2.25	0.59	8.50	27.09	14.75	2.36	21.00	0.59
2.50	0.59	8.75	27.09	15.00	2.36	21.25	0.59
2.75	0.59	9.00	27.09	15.25	2.36	21.50	0.59
3.00	0.59	9.25	27.09	15.50	2.36	21.75	0.59
3.25	0.59	9.50	27.09	15.75	2.36	22.00	0.59
3.50	0.59	9.75	27.09	16.00	2.36	22.25	0.59
3.75	0.59	10.00	27.09	16.25	2.36	22.50	0.59
4.00	0.59	10.25	27.09	16.50	1.18	22.75	0.59
4.25	0.59	10.50	7.66	16.75	1.18	23.00	0.59
4.50	3.53	10.75	7.66	17.00	1.18	23.25	0.59
4.75	3.53	11.00	7.66	17.25	1.18	23.50	0.59
5.00	3.53	11.25	7.66	17.50	1.18	23.75	0.59
5.25	3.53	11.50	7.66	17.75	1.18	24.00	0.59
5.50	3.53	11.75	7.66	18.00	1.18	24.25	0.59
5.75	3.53	12.00	7.66	18.25	1.18		
6.00	3.53	12.25	7.66	18.50	0.59		
6.25	3.53	12.50	4.12	18.75	0.59		

CALIB NASHYD (0016)
 ID= 1 DT=15.0 mi n

Area (ha)=3094.49
 Curve Number (CN)= 74.0
 Ia (mm)= 5.00 # of Li near Res. (N)= 3.10
 U. H. Tp(hrs)= 18.10

Uni t Hyd Qpeak (cms)= 6.704
 PEAK FLOW (cms)= 15.998 (i)
 TIME TO PEAK (hrs)= 29.500
 RUNOFF VOLUME (mm)= 62.976
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.535

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0013)
 ID= 1 DT=15.0 mi n

Area (ha)=1627.59
 Curve Number (CN)= 65.0
 Ia (mm)= 5.00 # of Li near Res. (N)= 3.10
 U. H. Tp(hrs)= 15.50

Uni t Hyd Qpeak (cms)= 4.118
 PEAK FLOW (cms)= 7.834 (i)
 TIME TO PEAK (hrs)= 27.000
 RUNOFF VOLUME (mm)= 50.983
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.433

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0015)
 ID= 1 DT=15.0 mi n

Area (ha)=1059.88
 Curve Number (CN)= 73.0
 Ia (mm)= 5.00 # of Li near Res. (N)= 3.10
 U. H. Tp(hrs)= 9.30

Uni t Hyd Qpeak (cms)= 4.469
 PEAK FLOW (cms)= 9.598 (i)
 TIME TO PEAK (hrs)= 20.250
 RUNOFF VOLUME (mm)= 61.544
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.522

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0012)
 ID= 1 DT=15.0 mi n

Area (ha)=1501.52
 Curve Number (CN)= 61.0
 Ia (mm)= 5.00 # of Li near Res. (N)= 3.10
 U. H. Tp(hrs)= 11.50

Uni t Hyd Qpeak (cms)= 5.120
 PEAK FLOW (cms)= 8.498 (i)
 TIME TO PEAK (hrs)= 23.000
 RUNOFF VOLUME (mm)= 46.236
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.392

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0093)
 ID= 1 DT=15.0 mi n

Area (ha)= 381.73
 Curve Number (CN)= 69.0
 Ia (mm)= 5.00 # of Li near Res. (N)= 3.10
 U. H. Tp(hrs)= 14.90

Uni t Hyd Qpeak (cms)= 1.005
 PEAK FLOW (cms)= 2.094 (i)
 TIME TO PEAK (hrs)= 26.250
 RUNOFF VOLUME (mm)= 56.073
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.476

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0010)
 ID= 1 DT=15.0 mi n

Area (ha)=2842.76
 Curve Number (CN)= 64.0
 Ia (mm)= 5.00 # of Li near Res. (N)= 3.10
 U. H. Tp(hrs)= 10.80

Uni t Hyd Qpeak (cms)= 10.322
 PEAK FLOW (cms)= 18.288 (i)
 TIME TO PEAK (hrs)= 22.000
 RUNOFF VOLUME (mm)= 49.766
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.422

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0005)
 ID= 1 DT=15.0 mi n

Area (ha)=1444.47
 Curve Number (CN)= 60.0
 Ia (mm)= 5.00 # of Li near Res. (N)= 3.10
 U. H. Tp(hrs)= 8.00

Uni t Hyd Qpeak (cms)= 7.080
 PEAK FLOW (cms)= 10.776 (i)
 TIME TO PEAK (hrs)= 19.250
 RUNOFF VOLUME (mm)= 45.099
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.383

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0001) Area (ha)=2873.64 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 11.00

Unit Hyd Opeak (cms)= 10.244

PEAK FLOW (cms)= 16.894 (i)
 TIME TO PEAK (hrs)= 22.250
 RUNOFF VOLUME (mm)= 46.236
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.392

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0002) Area (ha)= 988.72 Curve Number (CN)= 58.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 9.10

Unit Hyd Opeak (cms)= 4.261

PEAK FLOW (cms)= 6.314 (i)
 TIME TO PEAK (hrs)= 20.250
 RUNOFF VOLUME (mm)= 42.880
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.364

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	(0101)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3					
ID1= 1 (0001):		2873.64	16.894	22.25	46.24
+ ID2= 2 (0002):		988.72	6.314	20.25	42.88
=====					
ID = 3 (0101):		3862.36	23.056	21.75	45.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0102) IN= 2--> OUT= 1 Routing time step (min)' = 15.00

Distance	Elevation	Manning	
0.00	250.00	0.0700	
43.00	240.00	0.0700 / 0.0350	Main Channel
44.00	239.20	0.0350	Main Channel
45.00	239.20	0.0350	Main Channel
46.00	240.00	0.0350 / 0.0700	Main Channel
63.50	241.00	0.0700	
71.00	250.00	0.0700	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.40	239.60	.642E+04	0.5	0.80	221.73
0.80	240.00	.171E+05	1.8	1.15	155.20
1.39	240.59	.764E+05	8.0	1.12	159.62
1.98	241.18	.214E+06	23.7	1.19	150.23
2.56	241.76	.385E+06	51.8	1.44	123.91
3.15	242.35	.575E+06	90.1	1.68	106.38
3.74	242.94	.784E+06	138.5	1.89	94.40
4.33	243.53	.101E+07	196.9	2.08	85.71
4.92	244.12	.126E+07	265.6	2.26	79.08
5.51	244.71	.153E+07	344.7	2.42	73.81
6.09	245.29	.181E+07	434.5	2.57	69.50
6.68	245.88	.212E+07	535.2	2.71	65.89
7.27	246.47	.244E+07	647.3	2.84	62.81
7.86	247.06	.278E+07	771.1	2.97	60.13
8.45	247.65	.314E+07	906.8	3.09	57.77
9.04	248.24	.352E+07	1054.8	3.20	55.67
9.62	248.82	.392E+07	1215.5	3.32	53.79

10.21 249.41 .434E+07 1389.1 3.42 52.09
 10.80 250.00 .478E+07 1576.1 3.53 50.53

AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)

INFLOW : ID= 2 (0101) 3862.36 23.06 21.75 45.38
 OUTFLOW: ID= 1 (0102) 3862.36 21.96 24.25 45.38 1.95 1.18

CALIB NASHYD (0006) Area (ha)=1472.14 Curve Number (CN)= 73.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 12.20

Unit Hyd Opeak (cms)= 4.732

PEAK FLOW (cms)= 10.574 (i)
 TIME TO PEAK (hrs)= 23.500
 RUNOFF VOLUME (mm)= 61.544
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.522

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (0064) Area (ha)= 32.30
 ID= 1 DT=15.0 min Total Imp(%)= 75.00 Dir. Conn.(%)= 70.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	24.22	8.07
Dep. Storage (mm)=	1.00	5.00
Average Slope (%)=	1.00	2.00
Length (m)=	464.04	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	27.09	22.47
over (min)	15.00	30.00
Storage Coeff. (min)=	10.82 (ii)	23.64 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04
PEAK FLOW (cms)=	1.70	0.46
TIME TO PEAK (hrs)=	10.25	10.25
RUNOFF VOLUME (mm)=	116.80	61.86
TOTAL RAINFALL (mm)=	117.80	117.80
RUNOFF COEFFICIENT =	0.99	0.53

TOTALS
 2.166 (iii)
 10.25
 100.32
 117.80
 0.85

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0527) IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	1.6100	10.9070
0.1100	10.5752	2.1600	11.0260
0.6300	10.6908	2.7100	11.1114
1.1700	10.7958	3.1600	11.1951

AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)

INFLOW : ID= 2 (0064) 32.300 2.166 10.25 100.32
 OUTFLOW: ID= 1 (0527) 32.300 0.032 24.50 84.27

PEAK FLOW REDUCTION [Qout/Qin] (%) = 1.48
 TIME SHIFT OF PEAK FLOW (min) = 855.00
 MAXIMUM STORAGE USED (ha. m.) = 3.0721

ADD HYD (0518)

1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0006):	1472.14	10.574	23.50	61.54
+ ID2= 2 (0527):	32.30	0.032	24.50	84.27

ID = 3 (0518):	1504.44	10.606	23.50	62.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0103) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0102):	3862.36	21.964	24.25	45.38
+ ID2= 2 (0518):	1504.44	10.606	23.50	62.03

ID = 3 (0103):	5366.80	32.536	24.00	50.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0104) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0005):	1444.47	10.776	19.25	45.10
+ ID2= 2 (0103):	5366.80	32.536	24.00	50.05

ID = 3 (0104):	6811.27	41.604	22.75	49.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0105)
IN= 2----> OUT= 1

Routing time step (mi n)' = 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning	
0.00	237.00	0.1000	
330.00	235.00	0.1000 / 0.0350	Main Channel
331.00	234.20	0.0350	Main Channel
332.00	234.20	0.0350	Main Channel
333.00	235.00	0.0350 / 0.1000	Main Channel
425.00	235.10	0.1000	
698.00	236.00	0.1000	
1101.00	239.00	0.1000	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.13	234.33	.490E+03	0.1	0.64	82.66
0.27	234.47	.112E+04	0.3	0.93	56.72
0.40	234.60	.189E+04	0.7	1.14	45.94
0.53	234.73	.280E+04	1.2	1.33	39.64
0.67	234.87	.385E+04	1.8	1.49	35.34
0.80	235.00	.504E+04	2.6	1.63	32.16
0.95	235.15	.442E+05	6.3	0.45	116.42
1.11	235.31	.128E+06	17.7	0.44	120.51
1.26	235.46	.247E+06	38.2	0.49	107.48
1.42	235.62	.400E+06	69.8	0.55	95.60
1.57	235.77	.589E+06	114.0	0.61	86.08
1.72	235.92	.812E+06	172.6	0.67	78.45
1.88	236.08	.107E+07	250.3	0.74	71.19
2.03	236.23	.135E+07	347.8	0.81	64.71
2.18	236.38	.165E+07	461.9	0.88	59.65
2.34	236.54	.198E+07	593.4	0.95	55.58
2.49	236.69	.233E+07	742.8	1.01	52.21
2.65	236.85	.270E+07	910.6	1.06	49.36
2.80	237.00	.309E+07	1097.6	1.12	46.91

<---- hydrograph ---->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0104)	6811.27	41.60	22.75	49.00	1.28	0.49
OUTFLOW: ID= 1 (0105)	6811.27	40.76	24.50	49.00	1.27	0.49

CALIB

NASHYD (0091) ID= 1 DT=15.0 mi n	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	Curve Number (CN)	# of Li near Res. (N)
	281.97	5.00	5.80	65.0	65.0	3.10

Uni t Hyd Qpeak (cms)= 1.906

PEAK FLOW (cms)=	3.048 (i)
TIME TO PEAK (hrs)=	16.500
RUNOFF VOLUME (mm)=	50.983
TOTAL RAINFALL (mm)=	117.800
RUNOFF COEFFICIENT =	0.433

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0106) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0091):	281.97	3.048	16.50	50.98
+ ID2= 2 (0105):	6811.27	40.761	24.50	49.00

ID = 3 (0106):	7093.24	42.238	24.00	49.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0007) ID= 1 DT=15.0 mi n	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	Curve Number (CN)	# of Li near Res. (N)
	902.64	5.00	10.70	73.0	73.0	3.10

Uni t Hyd Qpeak (cms)= 3.308

PEAK FLOW (cms)=	7.261 (i)
TIME TO PEAK (hrs)=	21.750
RUNOFF VOLUME (mm)=	61.544
TOTAL RAINFALL (mm)=	117.800
RUNOFF COEFFICIENT =	0.522

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (0074) ID= 1 DT=15.0 mi n	Area (ha)	Imp(%)	Dir. Conn. (%)
	135.70	75.00	70.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	101.78	33.92
Dep. Storage	1.00	5.00
Average Slope	1.00	2.00
Length	951.14	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	27.09	17.59
over (mi n)	15.00	45.00
Storage Coeff. (mi n)=	16.64 (ii)	30.78 (ii)
Uni t Hyd. Tpeak (mi n)=	15.00	45.00
Uni t Hyd. peak (cms)=	0.07	0.03

TOTALS

PEAK FLOW (cms)=	7.14	1.37	8.516 (iii)
TIME TO PEAK (hrs)=	10.25	10.25	10.25
RUNOFF VOLUME (mm)=	116.80	47.24	95.93
TOTAL RAINFALL (mm)=	117.80	117.80	117.80
RUNOFF COEFFICIENT =	0.99	0.40	0.81

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

CN* = 57.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0528) IN= 2----> OUT= 1 DT= 15.0 mi n	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	6.7900	9.8141
	0.4600	8.4187	9.0800	10.3142

2.6300	8.9048	11.4000	10.6733
4.9000	9.3462	13.2700	11.0255

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (0074)	135.700	8.516	10.25	95.93
OUTFLOW: ID= 1 (0528)	135.700	2.541	12.25	95.90

PEAK FLOW REDUCTION [Qout/Qin] (%) = 29.84
 TIME SHIFT OF PEAK FLOW (min) = 120.00
 MAXIMUM STORAGE USED (ha.m.) = 8.8863

ADD HYD (0520)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0007):	902.64	7.261	21.75	61.54
+ ID2= 2 (0528):	135.70	2.541	12.25	95.90
=====				
ID = 3 (0520):	1038.34	7.705	21.75	66.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (0073)
 ID= 1 DT=15.0 min

Area (ha) = 178.90
 Total Imp(%) = 27.00
 Di. r. Conn. (%) = 9.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	48.30	130.60
Dep. Storage	1.00	5.00
Average Slope	1.00	2.00
Length	1092.09	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) =	27.09	19.08
over (min) =	15.00	45.00
Storage Coeff. (min) =	18.08 (ii)	31.77 (ii)
Unit Hyd. Tpeak (min) =	15.00	45.00
Unit Hyd. peak (cms) =	0.06	0.03

TOTALS
 PEAK FLOW (cms) = 1.21
 TIME TO PEAK (hrs) = 10.25
 RUNOFF VOLUME (mm) = 116.80
 TOTAL RAINFALL (mm) = 117.80
 RUNOFF COEFFICIENT = 0.99

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 58.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0529)
 IN= 2 ---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	8.9500	9.2195
0.6100	7.5855	11.9700	10.0426
3.4700	7.7498	15.0300	10.8266
6.4600	8.5771	17.5000	11.5931

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (0073)	178.900	6.938	10.25	55.60
OUTFLOW: ID= 1 (0529)	178.900	0.896	15.50	55.58

PEAK FLOW REDUCTION [Qout/Qin] (%) = 12.91
 TIME SHIFT OF PEAK FLOW (min) = 315.00
 MAXIMUM STORAGE USED (ha.m.) = 7.6022

ADD HYD (0522)
 1 + 2 = 3

AREA	OPEAK	TPEAK	R. V.
------	-------	-------	-------

	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0520):	1038.34	7.705	21.75	66.03
+ ID2= 2 (0529):	178.90	0.896	15.50	55.58
=====				
ID = 3 (0522):	1217.24	8.276	21.75	64.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (0079)
 ID= 1 DT=15.0 min

Area (ha) = 156.60
 Total Imp(%) = 39.00
 Di. r. Conn. (%) = 30.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	61.07	95.53
Dep. Storage	1.00	5.00
Average Slope	1.00	2.00
Length	1021.76	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) =	27.09	21.89
over (min) =	15.00	45.00
Storage Coeff. (min) =	17.37 (ii)	30.33 (ii)
Unit Hyd. Tpeak (min) =	15.00	45.00
Unit Hyd. peak (cms) =	0.07	0.03

TOTALS
 PEAK FLOW (cms) = 3.53
 TIME TO PEAK (hrs) = 10.25
 RUNOFF VOLUME (mm) = 116.80
 TOTAL RAINFALL (mm) = 117.80
 RUNOFF COEFFICIENT = 0.99

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 71.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0530)
 IN= 2 ---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	7.8300	9.2109
0.5300	7.7357	10.4800	9.8961
3.0400	7.9828	13.1600	10.5152
5.6500	8.6540	15.3200	11.1208

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (0079)	156.600	8.548	10.25	79.23
OUTFLOW: ID= 1 (0530)	156.600	2.979	12.00	79.21

PEAK FLOW REDUCTION [Qout/Qin] (%) = 34.84
 TIME SHIFT OF PEAK FLOW (min) = 105.00
 MAXIMUM STORAGE USED (ha.m.) = 7.9779

ADD HYD (0524)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0522):	1217.24	8.276	21.75	64.50
+ ID2= 2 (0530):	156.60	2.979	12.00	79.21
=====				
ID = 3 (0524):	1373.84	8.783	21.50	66.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0107)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0106):	7093.24	42.238	24.00	49.08
+ ID2= 2 (0524):	1373.84	8.783	21.50	66.17
=====				
ID = 3 (0107):	8467.08	50.763	23.75	51.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0108)
IN= 2--> OUT= 1 Routing time step (min)' = 15.00

DATA FOR SECTION (1.1)				
Distance	Elevation	Manning		
0.00	234.00	0.1000	0.0350	Main Channel
400.00	231.00	0.1000	0.0350	Main Channel
401.00	229.70	0.0350		Main Channel
402.00	229.70	0.0350		Main Channel
404.00	230.50	0.0350	0.1000	Main Channel
495.00	231.00	0.1000		
1234.00	256.00	0.1000		

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.20	229.90	.151E+04	0.1	0.24	395.61
0.40	230.10	.377E+04	0.2	0.35	274.62
0.60	230.30	.677E+04	0.5	0.43	222.06
0.80	230.50	.105E+05	0.9	0.50	190.52
1.03	230.73	.437E+05	2.0	0.27	356.49
1.27	230.97	.134E+06	5.0	0.21	448.69
1.50	231.20	.278E+06	11.4	0.23	405.59
1.73	231.43	.473E+06	21.6	0.26	364.80
1.97	231.67	.718E+06	36.0	0.29	332.76
2.20	231.90	.101E+07	55.1	0.31	306.87
2.43	232.13	.136E+07	79.4	0.33	285.44
2.67	232.37	.176E+07	109.6	0.36	267.34
2.90	232.60	.221E+07	145.9	0.38	251.83
3.13	232.83	.270E+07	189.0	0.40	238.35
3.37	233.07	.325E+07	239.3	0.42	226.51
3.60	233.30	.385E+07	297.1	0.44	216.02
3.83	233.53	.450E+07	363.0	0.46	206.64
4.07	233.77	.520E+07	437.3	0.48	198.20
4.30	234.00	.595E+07	520.5	0.50	190.55

		hydrograph			pipe / channel	
INFLOW	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
ID= 2 (0107)	8467.08	50.76	23.75	51.85	2.15	0.30
OUTFLOW: ID= 1 (0108)	8467.08	43.81	28.25	51.85	2.06	0.29

CALIB NASHYD (0092)
ID= 1 DT=15.0 min Area (ha)=1102.32 Curve Number (CN)= 64.0
of Linear Res. (N)= 3.10
U.H. Tp(hrs)= 16.50

Unit Hyd Opeak (cms)= 2.620

PEAK FLOW (cms)= 4.895 (i)
TIME TO PEAK (hrs)= 28.000
RUNOFF VOLUME (mm)= 49.766
TOTAL RAINFALL (mm)= 117.800
RUNOFF COEFFICIENT = 0.422

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0109)
1 + 2 = 3 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 1 (0092): 1102.32 4.895 28.00 49.77
+ ID2= 2 (0108): 8467.08 43.808 28.25 51.85
ID = 3 (0109): 9569.40 48.702 28.25 51.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0110)
1 + 2 = 3 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 1 (0010): 2842.76 18.288 22.00 49.77

+ ID2= 2 (0109): 9569.40 48.702 28.25 51.61
ID = 3 (0110): 12412.16 64.261 26.75 51.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0111)
IN= 2--> OUT= 1 Routing time step (min)' = 15.00

DATA FOR SECTION (1.1)				
Distance	Elevation	Manning		
0.00	236.00	0.1000		
275.00	232.00	0.1000		
1039.00	230.00	0.1000	0.0350	Main Channel
1040.00	229.20	0.0350		Main Channel
1041.00	229.20	0.0350		Main Channel
1043.00	230.00	0.0350	0.1000	Main Channel
1117.00	230.60	0.1000		
1325.00	234.00	0.1000		

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.20	229.40	.985E+03	0.0	0.14	431.34
0.40	229.60	.251E+04	0.1	0.20	298.03
0.60	229.80	.457E+04	0.3	0.25	239.97
0.80	230.00	.717E+04	0.6	0.29	205.17
1.07	230.27	.754E+05	2.2	0.11	561.90
1.33	230.53	.272E+06	8.6	0.11	527.12
1.60	230.80	.594E+06	22.7	0.14	436.37
1.87	231.07	.103E+07	46.2	0.16	371.32
2.13	231.33	.158E+07	80.7	0.18	325.64
2.40	231.60	.224E+07	127.8	0.20	291.74
2.67	231.87	.301E+07	189.0	0.22	265.46
2.93	232.13	.389E+07	271.8	0.25	238.27
3.20	232.40	.481E+07	376.4	0.28	212.79
3.47	232.67	.576E+07	496.0	0.31	193.49
3.73	232.93	.674E+07	630.4	0.33	178.30
4.00	233.20	.776E+07	779.4	0.36	165.99
4.27	233.47	.881E+07	943.0	0.38	155.79
4.53	233.73	.990E+07	1121.1	0.41	147.16
4.80	234.00	.110E+08	1313.9	0.43	139.76

		hydrograph			pipe / channel	
INFLOW	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
ID= 2 (0110)	*****	64.26	26.75	51.19	2.01	0.17
OUTFLOW: ID= 1 (0111)	*****	54.95	31.50	51.18	1.93	0.17

ADD HYD (0112)
1 + 2 = 3 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 1 (0093): 381.73 2.094 26.25 56.07
+ ID2= 2 (0111): 12412.16 54.953 31.50 51.18
ID = 3 (0112): 12793.89 56.884 31.00 51.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0008)
ID= 1 DT=15.0 min Area (ha)=1549.97 Curve Number (CN)= 60.0
of Linear Res. (N)= 3.10
U.H. Tp(hrs)= 9.10

Unit Hyd Opeak (cms)= 6.679

PEAK FLOW (cms)= 10.418 (i)
TIME TO PEAK (hrs)= 20.250
RUNOFF VOLUME (mm)= 45.099
TOTAL RAINFALL (mm)= 117.800
RUNOFF COEFFICIENT = 0.383

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0003) Area (ha)=2674.44 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 13.10

Unit Hyd Qpeak (cms)= 8.006
 PEAK FLOW (cms)= 14.570 (i)
 TIME TO PEAK (hrs)= 24.500
 RUNOFF VOLUME (mm)= 49.766
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.422

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (0036) Area (ha)= 280.10
 ID= 1 DT=15.0 min Total Imp(%)= 22.00 Dir. Conn.(%)= 9.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	61.62	218.48
Dep. Storage	1.00	5.00
Average Slope	1.00	2.00
Length	1366.50	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	27.09	22.00
over (min)	15.00	45.00
Storage Coeff. (min)	20.68 (ii)	33.62 (ii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.06	0.03

			TOTALS
PEAK FLOW (cms)	1.89	11.29	13.187 (iii)
TIME TO PEAK (hrs)	10.25	10.25	10.25
RUNOFF VOLUME (mm)	116.80	62.30	67.21
TOTAL RAINFALL (mm)	117.80	117.80	117.80
RUNOFF COEFFICIENT	0.99	0.53	0.57

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0525) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)	
	0.0000	0.0000	14.0000	14.7295	
	0.9500	12.2068	18.7400	16.0462	
	5.4300	12.3823	23.5200	17.3268	
	10.1100	13.7195	27.3900	18.5786	
		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (0036)		280.100	13.187	10.25	67.21
OUTFLOW: ID= 1 (0525)		280.100	4.848	12.50	67.20

PEAK FLOW REDUCTION [Qout/Oin](%)= 36.77
 TIME SHIFT OF PEAK FLOW (min)=135.00
 MAXIMUM STORAGE USED (ha.m.)= 12.3739

ADD HYD (0515) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0003):	2674.44	14.570	24.50	49.77
+ ID2= 2 (0525):	280.10	4.848	12.50	67.20
ID = 3 (0515):	2954.54	15.432	24.50	51.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0004) Area (ha)=2580.39 Curve Number (CN)= 53.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 10.70

Unit Hyd Qpeak (cms)= 9.457
 PEAK FLOW (cms)= 12.615 (i)
 TIME TO PEAK (hrs)= 22.000
 RUNOFF VOLUME (mm)= 37.639
 TOTAL RAINFALL (mm)= 117.800
 RUNOFF COEFFICIENT = 0.320

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (0046) Area (ha)= 92.30
 ID= 1 DT=15.0 min Total Imp(%)= 44.00 Dir. Conn.(%)= 33.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	40.61	51.69
Dep. Storage	1.00	5.00
Average Slope	1.00	2.00
Length	784.43	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	27.09	19.10
over (min)	15.00	30.00
Storage Coeff. (min)	14.82 (ii)	28.51 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.07	0.04

			TOTALS
PEAK FLOW (cms)	2.29	2.43	4.718 (iii)
TIME TO PEAK (hrs)	10.25	10.25	10.25
RUNOFF VOLUME (mm)	116.80	51.77	73.23
TOTAL RAINFALL (mm)	117.80	117.80	117.80
RUNOFF COEFFICIENT	0.99	0.44	0.62

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0526) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)	
	0.0000	0.0000	4.6200	51.9904	
	0.3100	51.1099	6.1800	52.3852	
	1.7900	51.2815	7.7500	52.7331	
	3.3300	51.6636	9.0300	53.0736	
		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (0046)		92.300	4.718	10.25	73.23
OUTFLOW: ID= 1 (0526)		92.300	0.040	24.75	48.05

PEAK FLOW REDUCTION [Qout/Oin](%)= 0.84
 TIME SHIFT OF PEAK FLOW (min)=870.00
 MAXIMUM STORAGE USED (ha.m.)= 6.5521

ADD HYD (0516) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0004):	2580.39	12.615	22.00	37.64
+ ID2= 2 (0526):	92.30	0.040	24.75	48.05
ID = 3 (0516):	2672.69	12.654	22.00	38.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(0201)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1	(0515): 2954.54	15.432	24.50	51.42	
+ ID2= 2	(0516): 2672.69	12.654	22.00	38.00	
=====					
ID = 3	(0201): 5627.23	27.865	23.25	45.04	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0202)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->					
Distance	Elevation	Manning			
0.00	239.00	0.1000			
690.00	230.00	0.1000 / 0.0350	Main Channel		
691.00	229.20	0.0350	Main Channel		
692.00	229.20	0.0350	Main Channel		
693.00	229.50	0.0350 / 0.1000	Main Channel		
1190.00	230.00	0.1000			
1417.00	255.00	0.1000			

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(mi n)
0.30	229.50	347E+04	0.4	0.84	136.57
0.80	230.00	864E+06	45.2	0.36	318.78
1.30	230.50	265E+07	274.1	0.71	161.20
1.80	231.00	459E+07	651.8	0.97	117.24
2.30	231.50	667E+07	1165.3	1.20	95.34
2.80	232.00	889E+07	1810.2	1.40	81.89
3.30	232.50	113E+08	2585.1	1.57	72.65
3.80	233.00	138E+08	3490.4	1.73	65.85
4.30	233.50	165E+08	4527.4	1.89	60.59
4.80	234.00	193E+08	5697.8	2.03	56.38
5.30	234.50	222E+08	7003.8	2.16	52.91
5.80	235.00	253E+08	8447.7	2.28	50.00
6.30	235.50	286E+08	10032.3	2.40	47.52
6.80	236.00	320E+08	11760.1	2.52	45.36
7.30	236.50	356E+08	13634.1	2.63	43.46
7.80	237.00	393E+08	15657.0	2.73	41.78
8.30	237.50	431E+08	17831.7	2.84	40.28
8.80	238.00	471E+08	20161.3	2.93	38.92
9.30	238.50	512E+08	22648.7	3.03	37.69

<---- hydrograph ---->						<-pi pe / channel ->	
AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL		
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW : ID= 2 (0201)	5627.23	27.86	23.25	45.04	0.61	0.46	
OUTFLOW: ID= 1 (0202)	5627.23	25.47	28.50	45.04	0.58	0.48	

ADD HYD	(0203)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1	(0008): 1549.97	10.418	20.25	45.10	
+ ID2= 2	(0202): 5627.23	25.466	28.50	45.04	
=====					
ID = 3	(0203): 7177.20	33.031	25.75	45.05	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(0301)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1	(0112): 12793.89	56.884	31.00	51.33	
+ ID2= 2	(0203): 7177.20	33.031	25.75	45.05	
=====					
ID = 3	(0301): 19971.09	87.663	29.50	49.08	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0302)

IN= 2--> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->					
Distance	Elevation	Manning			
0.00	240.00	0.0700			
110.00	239.00	0.0700			
161.00	238.00	0.0700			
250.00	237.00	0.0700			
420.00	236.00	0.0700			
560.00	235.00	0.0700			
589.00	234.40	0.0700 / 0.0350	Main Channel		
589.10	233.60	0.0350	Main Channel		
594.00	233.60	0.0350	Main Channel		
594.10	234.40	0.0350 / 0.0700	Main Channel		
615.00	235.00	0.0700			
740.00	236.00	0.0700			
860.00	236.50	0.0700			
1340.00	237.00	0.0700			
1460.00	238.00	0.0700			
1470.00	239.00	0.0700			
1510.00	240.00	0.0700			

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(mi n)
0.27	233.87	.770E+04	1.0	0.74	132.49
0.53	234.13	.155E+05	2.9	1.10	88.48
0.80	234.40	234E+05	5.5	1.37	71.19
1.15	234.75	636E+05	11.6	1.07	91.11
1.50	235.10	169E+06	24.5	0.85	114.97
1.85	235.45	431E+06	54.5	0.74	131.62
2.20	235.80	882E+06	117.3	0.78	125.39
2.55	236.15	153E+07	218.3	0.83	117.06
2.90	236.50	246E+07	375.3	0.89	109.30
3.25	236.85	394E+07	579.0	0.86	113.45
3.60	237.20	612E+07	1010.3	0.97	101.01
3.95	237.55	851E+07	1631.3	1.12	86.99
4.30	237.90	111E+08	2391.8	1.27	77.05
4.65	238.25	137E+08	3337.5	1.42	68.52
5.00	238.60	164E+08	4438.7	1.58	61.71
5.35	238.95	192E+08	5671.0	1.73	56.40
5.70	239.30	220E+08	6955.1	1.85	52.75
6.05	239.65	249E+08	8355.2	1.96	49.76
6.40	240.00	280E+08	9884.7	2.07	47.18

<---- hydrograph ---->						<-pi pe / channel ->	
AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL		
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW : ID= 2 (0301)	*****	87.66	29.50	49.08	2.03	0.76	
OUTFLOW: ID= 1 (0302)	*****	85.97	31.75	49.07	2.02	0.76	

ADD HYD	(0303)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1	(0012): 1501.52	8.498	23.00	46.24	
+ ID2= 2	(0302): 19971.09	85.975	31.75	49.07	
=====					
ID = 3	(0303): 21472.61	92.059	31.00	48.88	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0011) Area (ha)=3099.17 Curve Number (CN)= 57.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Li near Res. (N)= 3.10
U. H. Tp(hrs)= 11.10

Unit Hyd Opeak (cms)= 10.949

PEAK FLOW (cms)= 16.328 (i)
TIME TO PEAK (hrs)= 22.500
RUNOFF VOLUME (mm)= 41.798
TOTAL RAINFALL (mm)= 117.800
RUNOFF COEFFICIENT = 0.355

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (0401)
IN= 2----> OUT= 1 Routing time step (min)' = 15.00

Distance	Elevation	Manning
0.00	236.00	0.0500
72.00	235.00	0.0500
492.00	230.00	0.0500
591.00	229.20	0.0500 / 0.0350
582.00	228.40	0.0350
583.00	228.40	0.0350
584.00	229.20	0.0350 / 0.0500
588.00	229.20	0.0500
607.00	230.00	0.0500
627.00	235.00	0.0500

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.27	228.67	.302E+04	0.1	0.21	666.69
0.53	228.93	.756E+04	0.3	0.30	465.92
0.80	229.20	.136E+05	0.6	0.37	377.99
1.16	229.56	.111E+06	3.0	0.23	616.66
1.52	229.92	.358E+06	11.5	0.27	519.91
1.89	230.29	.740E+06	30.1	0.35	410.09
2.25	230.65	.122E+07	59.5	0.41	342.49
2.61	231.01	.180E+07	100.6	0.47	298.58
2.97	231.37	.248E+07	154.6	0.53	267.22
3.34	231.74	.326E+07	222.9	0.58	243.40
3.70	232.10	.413E+07	306.6	0.63	224.51
4.06	232.46	.510E+07	406.9	0.68	209.05
4.42	232.82	.617E+07	524.8	0.72	196.10
4.79	233.19	.734E+07	661.4	0.77	185.06
5.15	233.55	.861E+07	817.9	0.81	175.49
5.51	233.91	.998E+07	995.2	0.85	167.10
5.87	234.27	.114E+08	1194.3	0.89	159.67
6.24	234.64	.130E+08	1416.3	0.93	153.03
6.60	235.00	.147E+08	1662.2	0.96	147.05

INFLOW	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
ID= 2 (0011)	3099.17	16.33	22.50	41.80	1.62	0.29
OUTFLOW: ID= 1 (0401)	3099.17	12.26	29.25	41.80	1.54	0.27

CALIB NASHYD (0014)
ID= 1 DT=15.0 min
Area (ha)=2163.09
Curve Number (CN)= 69.0
U.H. Tp(hrs)= 25.10
of Linear Res. (N)= 3.10

Unit Hyd Qpeak (cms)= 3.379

PEAK FLOW (cms)= 7.314 (i)
TIME TO PEAK (hrs)= 36.500
RUNOFF VOLUME (mm)= 56.073
TOTAL RAINFALL (mm)= 117.800
RUNOFF COEFFICIENT = 0.476

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0402)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0014):	2163.09	7.314	36.50	56.07
+ ID2= 2 (0401):	3099.17	12.262	29.25	41.80
ID= 3 (0402):	5262.26	19.035	31.00	47.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0501)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0303):	21472.61	92.059	31.00	48.88
+ ID2= 2 (0402):	5262.26	19.035	31.00	47.66

ID= 3 (0501): 26734.87 111.094 31.00 48.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0503)
IN= 2----> OUT= 1 Routing time step (min)' = 15.00

Distance	Elevation	Manning
0.00	237.00	0.0700
425.00	235.00	0.0700
445.00	234.00	0.0700
480.00	233.50	0.0700
1535.00	233.50	0.0700 / 0.0350
1536.50	232.70	0.0350
1537.50	232.70	0.0350
1540.00	233.50	0.0350 / 0.0700
1605.00	233.20	0.0700
1610.00	233.10	0.0700
2135.00	233.10	0.0700
2440.00	233.50	0.0700
2505.00	233.50	0.0700
2530.00	234.00	0.0700
2535.00	235.00	0.0700
2540.00	236.00	0.0700
2605.00	238.00	0.0700
2645.00	241.00	0.0700

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.20	232.90	.385E+03	0.2	0.55	38.92
0.40	233.10	.103E+04	0.6	0.80	26.75
0.60	233.30	.159E+06	39.5	0.32	67.02
0.80	233.50	.366E+06	133.3	0.47	45.74
1.03	233.73	.977E+06	428.7	0.56	37.97
1.27	233.97	.160E+07	914.3	0.74	29.09
1.50	234.20	.222E+07	1556.9	0.90	23.78
1.73	234.43	.285E+07	2336.2	1.05	20.32
1.97	234.67	.348E+07	3240.2	1.20	17.88
2.20	234.90	.411E+07	4260.9	1.33	16.07
2.43	235.13	.474E+07	5359.1	1.45	14.75
2.67	235.37	.539E+07	6537.6	1.56	13.74
2.90	235.60	.605E+07	7819.4	1.66	12.90
3.13	235.83	.673E+07	9203.8	1.76	12.19
3.37	236.07	.743E+07	10683.6	1.85	11.59
3.60	236.30	.814E+07	12247.8	1.93	11.07
3.83	236.53	.887E+07	13912.8	2.01	10.62
4.07	236.77	.961E+07	15679.0	2.09	10.22
4.30	237.00	.104E+08	17546.8	2.17	9.85

INFLOW	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
ID= 2 (0501)	*****	111.09	31.00	48.64	0.75	0.42
OUTFLOW: ID= 1 (0503)	*****	110.91	31.50	48.64	0.75	0.42

RESERVOIR (0510)
IN= 2----> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	27.8300	83.9600
3.8000	14.0500	*****	101.7400
11.0800	33.6500	*****	145.2700
20.8900	61.8900	0.0000	0.0000

INFLOW	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 2 (0503)	*****	110.913	31.50	48.64
OUTFLOW: ID= 1 (0510)	*****	110.909	31.50	48.63

PEAK FLOW REDUCTION [Qout/Qin] (%)=100.00
TIME SHIFT OF PEAK FLOW (min)= 0.00
MAXIMUM STORAGE USED (ha. m.)= 86.3737

ROUTE CHN (0513)

IN= 2---> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning
0.00	237.00	0.0700
425.00	235.00	0.0700
445.00	234.00	0.0700
480.00	233.50	0.0700
1535.50	232.70	0.0700 /0.0350
1537.50	232.70	0.0350
1540.00	233.50	0.0350 /0.0700
1605.00	233.20	0.0700
1610.00	233.10	0.0700
2135.00	233.10	0.0700
2440.00	233.50	0.0700
2505.00	233.50	0.0700
2530.00	234.00	0.0700
2535.00	235.00	0.0700
2540.00	236.00	0.0700
2605.00	238.00	0.0700
2645.00	241.00	0.0700

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.20	232.90	291E+03	0.2	0.55	29.43
0.40	233.10	777E+03	0.6	0.80	20.23
0.60	233.30	120E+06	39.5	0.32	50.68
0.80	233.50	277E+06	133.3	0.47	34.59
1.03	233.73	739E+06	428.7	0.56	28.72
1.27	233.97	121E+07	914.3	0.74	22.00
1.50	234.20	168E+07	1556.9	0.90	17.98
1.73	234.43	215E+07	2336.2	1.05	15.37
1.97	234.67	263E+07	3240.2	1.20	13.52
2.20	234.90	311E+07	4260.9	1.33	12.15
2.43	235.13	359E+07	5359.1	1.45	11.15
2.67	235.37	408E+07	6537.6	1.56	10.39
2.90	235.60	458E+07	7819.4	1.66	9.76
3.13	235.83	509E+07	9203.8	1.76	9.22
3.37	236.07	562E+07	10683.6	1.85	8.76
3.60	236.30	615E+07	12247.8	1.93	8.37
3.83	236.53	670E+07	13912.8	2.01	8.03
4.07	236.77	727E+07	15679.0	2.09	7.73
4.30	237.00	784E+07	17546.8	2.17	7.45

<---- hydrograph ----> <-pi pe / channel ->

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (0510)	110.91	31.50	48.63	0.75	0.42
OUTFLOW: ID= 1 (0513)	110.82	32.00	48.63	0.75	0.42

ADD HYD (0504) | 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0015):	1059.88	9.598	20.25	61.54
+ ID2= 2 (0513):	26734.87	110.824	32.00	48.63
ID = 3 (0504):	27794.75	115.469	31.00	49.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0505) | 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0013):	1627.59	7.834	27.00	50.98
+ ID2= 2 (0504):	27794.75	115.469	31.00	49.13
ID = 3 (0505):	29422.34	122.892	31.00	49.23

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0506) | IN= 2---> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning
0.00	251.00	0.0500
323.00	238.00	0.0500
400.00	229.00	0.0500
545.00	228.00	0.0500
602.00	226.00	0.0500
604.00	225.80	0.0500 /0.0350
605.00	225.00	0.0350
609.00	225.00	0.0350
610.00	225.80	0.0350 /0.0500
614.00	226.00	0.0500
650.00	227.00	0.0500
810.00	229.50	0.0500
1855.00	240.00	0.0500
2617.00	259.00	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.80	225.80	292E+05	3.5	0.87	139.36
2.13	227.13	441E+06	50.6	0.84	145.20
3.45	228.45	197E+07	262.2	0.97	125.42
4.78	229.78	557E+07	971.2	1.27	95.57
6.11	231.11	106E+08	2326.5	1.61	75.63
7.43	232.43	169E+08	4425.6	1.90	63.77
8.76	233.76	247E+08	7361.8	2.17	55.90
10.08	235.08	338E+08	11229.7	2.42	50.22
11.41	236.41	444E+08	16121.0	2.65	45.87
12.74	237.74	563E+08	22124.8	2.86	42.40
14.06	239.06	697E+08	29005.2	3.03	40.02
15.39	240.39	846E+08	37433.0	3.23	37.66
16.72	241.72	101E+09	47908.6	3.47	34.97
18.04	243.04	117E+09	59742.1	3.71	32.73
19.37	244.37	135E+09	72949.0	3.94	30.83
20.69	245.69	153E+09	87549.6	4.16	29.20
22.02	247.02	173E+09	*****	4.37	27.79
23.35	248.35	193E+09	*****	4.58	26.55
24.67	249.67	214E+09	*****	4.77	25.45

<---- hydrograph ----> <-pi pe / channel ->

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (0505)	122.89	31.00	49.23	2.58	0.88
OUTFLOW: ID= 1 (0506)	119.70	33.75	49.23	2.55	0.88

RESERVOIR (0512) | IN= 2---> OUT= 1 | DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	*****	16.2850
27.2000	3.7650	*****	20.9160
76.8000	7.7410	*****	25.9120
*****	11.0650	*****	36.6600
*****	11.9150	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (0506)	119.702	33.75	49.23
OUTFLOW: ID= 1 (0512)	119.688	34.00	49.23

PEAK FLOW REDUCTION [Qout/Qin] (%) = 99.99
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 10.5640

ROUTE CHN (0514) | IN= 2---> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning
0.00	230.00	0.0500
400.00	229.00	0.0500
545.00	228.00	0.0500
602.00	226.00	0.0500
604.00	225.80	0.0500 /0.0350
605.00	225.00	0.0350
609.00	225.00	0.0350
610.00	225.80	0.0350 /0.0500
614.00	226.00	0.0500

650.00 227.00 0.0500
810.00 229.50 0.0500

TRAVEL TIME TABLE						
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)	
0.20	225.20	.243E+04	0.3	0.39	121.91	
0.40	225.40	.515E+04	1.1	0.59	80.58	
0.60	225.60	.815E+04	2.1	0.74	64.02	
0.80	225.80	.114E+05	3.5	0.87	54.69	
1.05	226.05	.184E+05	6.1	0.94	50.48	
1.29	226.29	.346E+05	10.3	0.85	55.99	
1.54	226.54	.620E+05	17.4	0.80	59.38	
1.79	226.79	.101E+06	28.3	0.81	59.18	
2.03	227.03	.151E+06	43.8	0.83	57.35	
2.28	227.28	.215E+06	63.9	0.85	55.99	
2.53	227.53	.295E+06	91.4	0.89	53.82	
2.77	227.77	.391E+06	127.0	0.93	51.35	
3.02	228.02	.504E+06	170.6	0.97	49.24	
3.27	228.27	.644E+06	215.5	0.96	49.84	
3.51	228.51	.821E+06	279.7	0.97	48.92	
3.76	228.76	.103E+07	363.1	1.00	47.47	
4.01	229.01	.128E+07	466.2	1.04	45.89	
4.25	229.25	.159E+07	572.0	1.03	46.41	
4.50	229.50	.198E+07	719.1	1.04	45.95	

		<--- hydrograph --->			<--- pipe / channel --->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (0512)	*****	119.69	34.00	49.23	2.72	0.92
OUTFLOW: ID= 1 (0514)	*****	119.32	34.75	49.23	2.71	0.92

ADD HYD (0507)					
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
ID1= 1 (0016):	3094.49	15.998	29.50	62.98	
+ ID2= 2 (0514):	29422.34	119.323	34.75	49.23	
ID = 3 (0507):	32516.83	134.289	34.50	50.54	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

** SIMULATION NUMBER: 6 **

READ STORM
Ptotal =130.88 mm

Filename: C:\Users\aman\cu\AppData\Local\Temp\6ab08Fc7-e8e6-4b3d-8177-e9629b714f53\1ade81b1
Comments: 100yr/24hr

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.25	0.00	6.50	11.13	12.75	4.58	19.00	0.65
0.50	0.65	6.75	11.13	13.00	4.58	19.25	0.65
0.75	0.65	7.00	11.13	13.25	4.58	19.50	0.65
1.00	0.65	7.25	11.13	13.50	4.58	19.75	0.65
1.25	0.65	7.50	11.13	13.75	4.58	20.00	0.65
1.50	0.65	7.75	11.13	14.00	4.58	20.25	0.65
1.75	0.65	8.00	11.13	14.25	4.58	20.50	0.65
2.00	0.65	8.25	11.13	14.50	2.62	20.75	0.65
2.25	0.65	8.50	30.11	14.75	2.62	21.00	0.65
2.50	0.65	8.75	30.11	15.00	2.62	21.25	0.65
2.75	0.65	9.00	30.11	15.25	2.62	21.50	0.65
3.00	0.65	9.25	30.11	15.50	2.62	21.75	0.65
3.25	0.65	9.50	30.11	15.75	2.62	22.00	0.65
3.50	0.65	9.75	30.11	16.00	2.62	22.25	0.65
3.75	0.65	10.00	30.11	16.25	2.62	22.50	0.65
4.00	0.65	10.25	30.11	16.50	1.31	22.75	0.65
4.25	0.65	10.50	8.51	16.75	1.31	23.00	0.65
4.50	3.93	10.75	8.51	17.00	1.31	23.25	0.65
4.75	3.93	11.00	8.51	17.25	1.31	23.50	0.65
5.00	3.93	11.25	8.51	17.50	1.31	23.75	0.65
5.25	3.93	11.50	8.51	17.75	1.31	24.00	0.65
5.50	3.93	11.75	8.51	18.00	1.31	24.25	0.65
5.75	3.93	12.00	8.51	18.25	1.31		
6.00	3.93	12.25	8.51	18.50	0.65		

6.25 3.93 | 12.50 4.58 | 18.75 0.65 |

CALIB NASHYD (0016)			
ID= 1 DT=15.0 mi n	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
	3094.49	74.0	3.10
	Ia (mm)= 5.00		
	U. H. Tp(hrs)= 18.10		

Unit Hyd Opeak (cms) = 6.704

PEAK FLOW (cms) = 18.718 (i)
TIME TO PEAK (hrs) = 29.500
RUNOFF VOLUME (mm) = 73.659
TOTAL RAINFALL (mm) = 130.880
RUNOFF COEFFICIENT = 0.563

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0013)			
ID= 1 DT=15.0 mi n	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
	1627.59	65.0	3.10
	Ia (mm)= 5.00		
	U. H. Tp(hrs)= 15.50		

Unit Hyd Opeak (cms) = 4.118

PEAK FLOW (cms) = 9.275 (i)
TIME TO PEAK (hrs) = 27.000
RUNOFF VOLUME (mm) = 60.331
TOTAL RAINFALL (mm) = 130.880
RUNOFF COEFFICIENT = 0.461

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0015)			
ID= 1 DT=15.0 mi n	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
	1059.88	73.0	3.10
	Ia (mm)= 5.00		
	U. H. Tp(hrs)= 9.30		

Unit Hyd Opeak (cms) = 4.469

PEAK FLOW (cms) = 11.257 (i)
TIME TO PEAK (hrs) = 20.250
RUNOFF VOLUME (mm) = 72.084
TOTAL RAINFALL (mm) = 130.880
RUNOFF COEFFICIENT = 0.551

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0012)			
ID= 1 DT=15.0 mi n	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
	1501.52	61.0	3.10
	Ia (mm)= 5.00		
	U. H. Tp(hrs)= 11.50		

Unit Hyd Opeak (cms) = 5.120

PEAK FLOW (cms) = 10.114 (i)
TIME TO PEAK (hrs) = 22.750
RUNOFF VOLUME (mm) = 54.968
TOTAL RAINFALL (mm) = 130.880
RUNOFF COEFFICIENT = 0.420

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0093)			
ID= 1 DT=15.0 mi n	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
	381.73	69.0	3.10
	Ia (mm)= 5.00		
	U. H. Tp(hrs)= 14.90		

Unit Hyd Opeak (cms) = 1.005

PEAK FLOW (cms) = 2.468 (i)
TIME TO PEAK (hrs) = 26.250
RUNOFF VOLUME (mm) = 66.025
TOTAL RAINFALL (mm) = 130.880

RUNOFF COEFFICIENT = 0.504

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB NASHYD (0010) Area (ha)=2842.76 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 10.80
 Unit Hyd Opeak (cms)= 10.322
 PEAK FLOW (cms)= 21.693 (i)
 TIME TO PEAK (hrs)= 22.000
 RUNOFF VOLUME (mm)= 58.960
 TOTAL RAINFALL (mm)= 130.880
 RUNOFF COEFFICIENT = 0.450

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB NASHYD (0005) Area (ha)=1444.47 Curve Number (CN)= 60.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 8.00
 Unit Hyd Opeak (cms)= 7.080
 PEAK FLOW (cms)= 12.846 (i)
 TIME TO PEAK (hrs)= 19.000
 RUNOFF VOLUME (mm)= 53.676
 TOTAL RAINFALL (mm)= 130.880
 RUNOFF COEFFICIENT = 0.410

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB NASHYD (0001) Area (ha)=2873.64 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 11.00
 Unit Hyd Opeak (cms)= 10.244
 PEAK FLOW (cms)= 20.108 (i)
 TIME TO PEAK (hrs)= 22.250
 RUNOFF VOLUME (mm)= 54.968
 TOTAL RAINFALL (mm)= 130.880
 RUNOFF COEFFICIENT = 0.420

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB NASHYD (0002) Area (ha)= 988.72 Curve Number (CN)= 58.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 9.10
 Unit Hyd Opeak (cms)= 4.261
 PEAK FLOW (cms)= 7.542 (i)
 TIME TO PEAK (hrs)= 20.250
 RUNOFF VOLUME (mm)= 51.147
 TOTAL RAINFALL (mm)= 130.880
 RUNOFF COEFFICIENT = 0.391

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (0101) AREA OPEAK TPEAK R. V.
 1 + 2 = 3 (ha) (cms) (hrs) (mm)
 ID1= 1 (0001): 2873.64 20.108 22.25 54.97
 + ID2= 2 (0002): 988.72 7.542 20.25 51.15
 =====
 ID = 3 (0101): 3862.36 27.467 21.75 53.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (0102) Routing time step (min)'= 15.00
 IN= 2--> OUT= 1

<----- DATA FOR SECTION (1.1) ----->
 Distance Elevation Manning
 0.00 250.00 0.0700
 43.00 240.00 0.0700 /0.0350 Main Channel
 44.00 239.20 0.0350 Main Channel
 45.00 239.20 0.0350 Main Channel
 46.00 240.00 0.0350 /0.0700 Main Channel
 63.50 241.00 0.0700
 71.00 250.00 0.0700

<----- TRAVEL TIME TABLE ----->
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV. TIME
 (m) (m) (cu. m.) (cms) (m/s) (mi n)
 0.40 239.60 .642E+04 0.5 0.80 221.73
 0.80 240.00 .171E+05 1.8 1.15 155.20
 1.39 240.59 .764E+05 8.0 1.12 159.62
 1.98 241.18 .214E+06 23.7 1.19 150.23
 2.56 241.76 .385E+06 51.8 1.44 123.91
 3.15 242.35 .575E+06 90.1 1.68 106.38
 3.74 242.94 .784E+06 138.5 1.89 94.40
 4.33 243.53 .101E+07 196.9 2.08 85.71
 4.92 244.12 .126E+07 265.6 2.26 79.08
 5.51 244.71 .153E+07 344.7 2.42 73.81
 6.09 245.29 .181E+07 434.5 2.57 69.50
 6.68 245.88 .212E+07 535.2 2.71 65.89
 7.27 246.47 .244E+07 647.3 2.84 62.81
 7.86 247.06 .278E+07 771.1 2.97 60.13
 8.45 247.65 .314E+07 906.8 3.09 57.77
 9.04 248.24 .352E+07 1054.8 3.20 55.67
 9.62 248.82 .392E+07 1215.5 3.32 53.79
 10.21 249.41 .434E+07 1389.1 3.42 52.09
 10.80 250.00 .478E+07 1576.1 3.53 50.53

<---- hydrograph ----> <- pipe / channel ->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0101) 3862.36 27.47 21.75 53.99 2.06 1.22
 OUTFLOW: ID= 1 (0102) 3862.36 26.23 24.25 53.99 2.02 1.20

 CALIB STANDHYD (0006) Area (ha)=1472.14 Curve Number (CN)= 73.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 12.20

Unit Hyd Opeak (cms)= 4.732
 PEAK FLOW (cms)= 12.396 (i)
 TIME TO PEAK (hrs)= 23.250
 RUNOFF VOLUME (mm)= 72.084
 TOTAL RAINFALL (mm)= 130.880
 RUNOFF COEFFICIENT = 0.551

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB STANDHYD (0064) Area (ha)= 32.30
 ID= 1 DT=15.0 min Total Imp(%)= 75.00 Di r. Conn.(%)= 70.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 24.22 8.07
 Dep. Storage (mm)= 1.00 5.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 464.04 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 30.11 26.05
 over (mi n) 15.00 30.00
 Storage Coeff. (mi n)= 10.37 (ii) 22.46 (ii)
 Unit Hyd. Tpeak (mi n)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.04

TOTALS
 PEAK FLOW (cms)= 1.89 0.54 2.435 (iii)
 TIME TO PEAK (hrs)= 10.25 10.25 10.25

RUNOFF VOLUME (mm)= 129.88 72.39 112.63
 TOTAL RAINFALL (mm)= 130.88 130.88 130.88
 RUNOFF COEFFICIENT = 0.99 0.55 0.86

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
 CN* = 69.0 Ia = Dep Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0527)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.6100	10.9070
0.1100	10.5752	2.1600	11.0260
0.6300	10.6908	2.7100	11.1114
1.1700	10.7958	3.1600	11.1951

INFLOW : ID= 2 (0064)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (0527)	32.300	2.435	10.25	112.63
	32.300	0.036	24.50	94.62

PEAK FLOW REDUCTION [Qout/Oin] (%) = 1.47
 TIME SHIFT OF PEAK FLOW (min) = 855.00
 MAXIMUM STORAGE USED (ha.m.) = 3.4492

ADD HYD (0518)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0006):	1472.14	12.396	23.25	72.08
+ ID2= 2 (0527):	32.30	0.036	24.50	94.62
ID = 3 (0518):	1504.44	12.432	23.25	72.57

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0103)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0102):	3862.36	26.232	24.25	53.99
+ ID2= 2 (0518):	1504.44	12.432	23.25	72.57
ID = 3 (0103):	5366.80	38.637	24.00	59.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0104)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0005):	1444.47	12.846	19.00	53.68
+ ID2= 2 (0103):	5366.80	38.637	24.00	59.20
ID = 3 (0104):	6811.27	49.521	22.75	58.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0105)
 IN= 2--> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning
0.00	237.00	0.1000
330.00	235.00	0.1000 / 0.0350
331.00	234.20	0.0350
332.00	234.20	0.0350
333.00	235.00	0.0350 / 0.1000
425.00	235.10	0.1000

698.00 236.00 0.1000
 1101.00 239.00 0.1000

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.13	234.33	.490E+03	0.1	0.64	82.66
0.27	234.47	.112E+04	0.3	0.93	56.72
0.40	234.60	.189E+04	0.7	1.14	45.94
0.53	234.73	.280E+04	1.2	1.33	39.64
0.67	234.87	.385E+04	1.8	1.49	35.34
0.80	235.00	.504E+04	2.6	1.63	32.16
0.95	235.15	.442E+05	6.3	0.45	116.42
1.11	235.31	.128E+06	17.7	0.44	120.51
1.26	235.46	.247E+06	38.2	0.49	107.48
1.42	235.62	.400E+06	69.8	0.55	95.60
1.57	235.77	.589E+06	114.0	0.61	86.08
1.72	235.92	.812E+06	172.6	0.67	78.45
1.88	236.08	.107E+07	250.3	0.74	71.19
2.03	236.23	.135E+07	347.8	0.81	64.71
2.18	236.38	.165E+07	461.9	0.88	59.65
2.34	236.54	.198E+07	593.4	0.95	55.58
2.49	236.69	.233E+07	742.8	1.01	52.21
2.65	236.85	.270E+07	910.6	1.06	49.36
2.80	237.00	.309E+07	1097.6	1.12	46.91

<----- hydrograph ----->

INFLOW : ID= 2 (0104)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW: ID= 1 (0105)	6811.27	49.52	22.75	58.03	1.32	0.51
	6811.27	48.54	24.25	58.03	1.31	0.51

CALIB NASHYD (0091)
 ID= 1 DT=15.0 min

Area (ha) = 281.97
 Ia (mm) = 5.00
 U.H. Tp(hrs) = 5.80
 Curve Number (CN) = 65.0
 # of Linear Res. (N) = 3.10

Unit Hyd Qpeak (cms) = 1.906

PEAK FLOW (cms) = 3.616 (i)
 TIME TO PEAK (hrs) = 16.500
 RUNOFF VOLUME (mm) = 60.331
 TOTAL RAINFALL (mm) = 130.880
 RUNOFF COEFFICIENT = 0.461

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0106)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0091):	281.97	3.616	16.50	60.33
+ ID2= 2 (0105):	6811.27	48.543	24.25	58.03
ID = 3 (0106):	7093.24	50.320	24.00	58.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0007)
 ID= 1 DT=15.0 min

Area (ha) = 902.64
 Ia (mm) = 5.00
 U.H. Tp(hrs) = 10.70
 Curve Number (CN) = 73.0
 # of Linear Res. (N) = 3.10

Unit Hyd Qpeak (cms) = 3.308

PEAK FLOW (cms) = 8.514 (i)
 TIME TO PEAK (hrs) = 21.750
 RUNOFF VOLUME (mm) = 72.084
 TOTAL RAINFALL (mm) = 130.880
 RUNOFF COEFFICIENT = 0.551

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (0074)

Area (ha) = 135.70

| ID= 1 DT=15.0 min | Total Imp(%)= 75.00 Di r. Conn.(%)= 70.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	101.78	33.92
Dep. Storage (mm)=	1.00	5.00
Average Slope (%)=	1.00	2.00
Length (m)=	951.14	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	30.11	20.74
over (min)=	15.00	30.00
Storage Coeff. (min)=	15.95 (ii)	29.19 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.07	0.04

TOTALS
 PEAK FLOW (cms)= 7.94 1.72 9.663 (iii)
 TIME TO PEAK (hrs)= 10.25 10.25 10.25
 RUNOFF VOLUME (mm)= 129.88 56.06 107.74
 TOTAL RAINFALL (mm)= 130.88 130.88 130.88
 RUNOFF COEFFICIENT = 0.99 0.43 0.82

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 57.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0528)
 IN= 2---> OUT= 1
 DT= 15.0 min

	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha. m.)	(cms)	(ha. m.)
	0.0000	0.0000	6.7900	9.8141
	0.4600	8.4187	9.0800	10.3142
	2.6300	8.9048	11.4000	10.6733
	4.9000	9.3462	13.2700	11.0255
	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0074)	135.700	9.663	10.25	107.74
OUTFLOW: ID= 1 (0528)	135.700	3.944	10.75	107.70

PEAK FLOW REDUCTION [Qout/Qin] (%) = 40.82
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 9.1759

ADD HYD (0520)
 1 + 2 = 3

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0007):	902.64	8.514	21.75	72.08
+ ID2= 2 (0528):	135.70	3.944	10.75	107.70
ID = 3 (0520):	1038.34	8.960	21.75	76.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (0073)
 ID= 1 DT=15.0 min

Area (ha)= 178.90
 Total Imp(%)= 27.00 Di r. Conn.(%)= 9.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	48.30	130.60
Dep. Storage (mm)=	1.00	5.00
Average Slope (%)=	1.00	2.00
Length (m)=	1092.09	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	30.11	22.44
over (min)=	15.00	45.00
Storage Coeff. (min)=	17.33 (ii)	30.16 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.07	0.03

TOTALS
 PEAK FLOW (cms)= 1.35 6.86 8.205 (iii)
 TIME TO PEAK (hrs)= 10.25 10.25 10.25
 RUNOFF VOLUME (mm)= 129.88 58.65 65.06
 TOTAL RAINFALL (mm)= 130.88 130.88 130.88

PEAK FLOW (cms)= 1.35 6.86 8.205 (iii)
 TIME TO PEAK (hrs)= 10.25 10.25 10.25
 RUNOFF VOLUME (mm)= 129.88 58.65 65.06
 TOTAL RAINFALL (mm)= 130.88 130.88 130.88

RUNOFF COEFFICIENT = 0.99 0.45 0.50

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 58.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0529)
 IN= 2---> OUT= 1
 DT= 15.0 min

	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha. m.)	(cms)	(ha. m.)
	0.0000	0.0000	8.9500	9.2195
	0.6100	7.5855	11.9700	10.0426
	3.4700	7.7498	15.0300	10.8266
	6.4600	8.5771	17.5000	11.5931

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0073)	178.900	8.205	10.25	65.06
OUTFLOW: ID= 1 (0529)	178.900	2.679	12.50	65.05

PEAK FLOW REDUCTION [Qout/Qin] (%) = 32.65
 TIME SHIFT OF PEAK FLOW (min) = 135.00
 MAXIMUM STORAGE USED (ha. m.) = 7.7087

ADD HYD (0522)
 1 + 2 = 3

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0520):	1038.34	8.960	21.75	76.74
+ ID2= 2 (0529):	178.90	2.679	12.50	65.05
ID = 3 (0522):	1217.24	9.537	21.75	75.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (0079)
 ID= 1 DT=15.0 min

Area (ha)= 156.60
 Total Imp(%)= 39.00 Di r. Conn.(%)= 30.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	61.07	95.53
Dep. Storage (mm)=	1.00	5.00
Average Slope (%)=	1.00	2.00
Length (m)=	1021.76	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	30.11	25.35
over (min)=	15.00	30.00
Storage Coeff. (min)=	16.65 (ii)	28.87 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.07	0.04

TOTALS
 PEAK FLOW (cms)= 3.93 6.11 10.038 (iii)
 TIME TO PEAK (hrs)= 10.25 10.25 10.25
 RUNOFF VOLUME (mm)= 129.88 73.79 90.62
 TOTAL RAINFALL (mm)= 130.88 130.88 130.88
 RUNOFF COEFFICIENT = 0.99 0.56 0.69

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 71.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0530)
 IN= 2---> OUT= 1
 DT= 15.0 min

	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha. m.)	(cms)	(ha. m.)
	0.0000	0.0000	7.8300	9.2109
	0.5300	7.7357	10.4800	9.8961

	3.0400	7.9828	13.1600	10.5152
	5.6500	8.6540	15.3200	11.1208
	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (0079)	156.600	10.038	10.25	90.62
OUTFLOW: ID= 1 (0530)	156.600	4.166	11.25	90.60

PEAK FLOW REDUCTION [Qout/Qin] (%) = 41.50
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 8.2840

ADD HYD	(0524)			
1 + 2 = 3				
	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0522):	1217.24	9.537	21.75	75.02
+ ID2= 2 (0530):	156.60	4.166	11.25	90.60
ID= 3 (0524):	1373.84	11.309	12.50	76.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(0107)			
1 + 2 = 3				
	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0106):	7093.24	50.320	24.00	58.12
+ ID2= 2 (0524):	1373.84	11.309	12.50	76.80
ID= 3 (0107):	8467.08	60.090	23.75	61.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0108)
 IN= 2---> OUT= 1 Routing time step (min)' = 15.00

DATA FOR SECTION (1.1)				
Distance	Elevation	Manning		
0.00	234.00	0.1000	0.0350	Main Channel
400.00	231.00	0.1000	0.0350	Main Channel
401.00	229.70	0.0350		Main Channel
402.00	229.70	0.0350		Main Channel
404.00	230.50	0.0350	0.1000	Main Channel
495.00	231.00	0.1000		
1234.00	256.00	0.1000		

TRAVEL TIME TABLE					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)
0.20	229.90	.151E+04	0.1	0.24	395.61
0.40	230.10	.377E+04	0.2	0.35	274.62
0.60	230.30	.677E+04	0.5	0.43	222.06
0.80	230.50	.105E+05	0.9	0.50	190.52
1.03	230.73	.437E+05	2.0	0.27	356.49
1.27	230.97	.134E+06	5.0	0.21	448.69
1.50	231.20	.278E+06	11.4	0.23	405.59
1.73	231.43	.473E+06	21.6	0.26	364.80
1.97	231.67	.718E+06	36.0	0.29	332.76
2.20	231.90	.101E+07	55.1	0.31	306.87
2.43	232.13	.136E+07	79.4	0.33	285.44
2.67	232.37	.176E+07	109.6	0.36	267.34
2.90	232.60	.221E+07	145.9	0.38	251.83
3.13	232.83	.270E+07	189.0	0.40	238.35
3.37	233.07	.325E+07	239.3	0.42	226.51
3.60	233.30	.385E+07	297.1	0.44	216.02
3.83	233.53	.450E+07	363.0	0.46	206.64
4.07	233.77	.520E+07	437.3	0.48	198.20
4.30	234.00	.595E+07	520.5	0.50	190.55

		<---- hydrograph ---->			<- pi pe / channel ->	
	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW: ID= 2 (0107)	8467.08	60.09	23.75	61.15	2.25	0.31
OUTFLOW: ID= 1 (0108)	8467.08	52.31	27.75	61.15	2.17	0.31

CALIB NASHYD (0092)
 ID= 1 DT=15.0 min Area (ha)=1102.32 Curve Number (CN)= 64.0
 U. H. Tp(hrs)= 5.00 # of Li near Res. (N)= 3.10
 U. H. Tp(hrs)= 16.50

Unit Hyd Opeak (cms) = 2.620

PEAK FLOW (cms) = 5.802 (i)
 TIME TO PEAK (hrs) = 28.000
 RUNOFF VOLUME (mm) = 58.960
 TOTAL RAINFALL (mm) = 130.880
 RUNOFF COEFFICIENT = 0.450

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	(0109)			
1 + 2 = 3				
	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0092):	1102.32	5.802	28.00	58.96
+ ID2= 2 (0108):	8467.08	52.312	27.75	61.15
ID= 3 (0109):	9569.40	58.113	27.75	60.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(0110)			
1 + 2 = 3				
	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0010):	2842.76	21.693	22.00	58.96
+ ID2= 2 (0109):	9569.40	58.113	27.75	60.89
ID= 3 (0110):	12412.16	76.728	26.50	60.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0111)
 IN= 2---> OUT= 1 Routing time step (min)' = 15.00

DATA FOR SECTION (1.1)				
Distance	Elevation	Manning		
0.00	236.00	0.1000		
275.00	232.00	0.1000		
1039.00	230.00	0.1000	0.0350	Main Channel
1040.00	229.20	0.0350		Main Channel
1041.00	229.20	0.0350		Main Channel
1043.00	230.00	0.0350	0.1000	Main Channel
1117.00	230.60	0.1000		
1325.00	234.00	0.1000		

TRAVEL TIME TABLE					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)
0.20	229.40	.985E+03	0.0	0.14	431.34
0.40	229.60	.251E+04	0.1	0.20	298.03
0.60	229.80	.457E+04	0.3	0.25	239.97
0.80	230.00	.717E+04	0.6	0.29	205.17
1.07	230.27	.754E+05	2.2	0.11	561.90
1.33	230.53	.272E+06	8.6	0.11	527.12
1.60	230.80	.594E+06	22.7	0.14	436.37
1.87	231.07	.103E+07	46.2	0.16	371.32
2.13	231.33	.158E+07	80.7	0.18	325.64
2.40	231.60	.224E+07	127.8	0.20	291.74
2.67	231.87	.301E+07	189.0	0.22	265.46
2.93	232.13	.389E+07	271.8	0.25	238.27
3.20	232.40	.481E+07	376.4	0.28	212.79
3.47	232.67	.576E+07	496.0	0.31	193.49
3.73	232.93	.674E+07	630.4	0.33	178.30
4.00	233.20	.776E+07	779.4	0.36	165.99
4.27	233.47	.881E+07	943.0	0.38	155.79
4.53	233.73	.990E+07	1121.1	0.41	147.16
4.80	234.00	.110E+08	1313.9	0.43	139.76

		<---- hydrograph ---->			<- pi pe / channel ->	
	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)

INFLOW : ID= 2 (0110) ***** 76.73 26.50 60.45 2.10 0.18
 OUTFLOW: ID= 1 (0111) ***** 65.99 30.75 60.45 2.02 0.17

ADD HYD (0112)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0093):	381.73	2.468	26.25	66.03
+ ID2= 2 (0111):	12412.16	65.987	30.75	60.45
ID = 3 (0112):	12793.89	68.291	30.50	60.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0008)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	1549.97	60.0
	U. H. Tp (hrs) = 5.00	# of Linear Res. (N) = 3.10

Unit Hyd Qpeak (cms) = 6.679
 PEAK FLOW (cms) = 12.417 (i)
 TIME TO PEAK (hrs) = 20.250
 RUNOFF VOLUME (mm) = 53.676
 TOTAL RAINFALL (mm) = 130.880
 RUNOFF COEFFICIENT = 0.410

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0003)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	2674.44	64.0
	U. H. Tp (hrs) = 5.00	# of Linear Res. (N) = 3.10

Unit Hyd Qpeak (cms) = 8.006
 PEAK FLOW (cms) = 17.276 (i)
 TIME TO PEAK (hrs) = 24.500
 RUNOFF VOLUME (mm) = 58.960
 TOTAL RAINFALL (mm) = 130.880
 RUNOFF COEFFICIENT = 0.450

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (0036)	Area Total (ha)	Imp (%)	Dir. Conn. (%)
ID= 1 DT=15.0 min	280.10	22.00	9.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	61.62	218.48
Dep. Storage	1.00	5.00
Average Slope	1.00	2.00
Length	1366.50	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (min)	30.11 / 15.00	25.49 / 45.00
Storage Coeff. (min)	19.83 (ii)	32.02 (ii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.06	0.03

TOTALS
 PEAK FLOW (cms) = 2.11 13.33 15.438 (iii)
 TIME TO PEAK (hrs) = 10.25 10.25 10.25
 RUNOFF VOLUME (mm) = 129.88 72.88 78.01
 TOTAL RAINFALL (mm) = 130.88 130.88 130.88
 RUNOFF COEFFICIENT = 0.99 0.56 0.60

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0525)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	14.0000	14.7295
0.9500	12.2068	18.7400	16.0462
5.4300	12.3823	23.5200	17.3268
10.1100	13.7195	27.3900	18.5786

INFLOW : ID= 2 (0036)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
280.100	280.100	15.438	10.25	78.01
OUTFLOW: ID= 1 (0525)	280.100	6.135	11.75	78.00

PEAK FLOW REDUCTION [Qout/Qin] (%) = 39.74
 TIME SHIFT OF PEAK FLOW (min) = 90.00
 MAXIMUM STORAGE USED (ha. m.) = 12.5889

ADD HYD (0515)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0003):	2674.44	17.276	24.50	58.96
+ ID2= 2 (0525):	280.10	6.135	11.75	78.00
ID = 3 (0515):	2954.54	18.149	24.50	60.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0004)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	2580.39	53.0
	U. H. Tp (hrs) = 5.00	# of Linear Res. (N) = 3.10

Unit Hyd Qpeak (cms) = 9.457
 PEAK FLOW (cms) = 15.143 (i)
 TIME TO PEAK (hrs) = 22.000
 RUNOFF VOLUME (mm) = 45.129
 TOTAL RAINFALL (mm) = 130.880
 RUNOFF COEFFICIENT = 0.345

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (0046)	Area Total (ha)	Imp (%)	Dir. Conn. (%)
ID= 1 DT=15.0 min	92.30	44.00	33.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	40.61	51.69
Dep. Storage	1.00	5.00
Average Slope	1.00	2.00
Length	784.43	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (min)	30.11 / 15.00	22.40 / 30.00
Storage Coeff. (min)	14.21 (ii)	27.05 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.07	0.04

TOTALS
 PEAK FLOW (cms) = 2.55 2.89 5.432 (iii)
 TIME TO PEAK (hrs) = 10.25 10.25 10.25
 RUNOFF VOLUME (mm) = 129.88 61.17 83.84
 TOTAL RAINFALL (mm) = 130.88 130.88 130.88
 RUNOFF COEFFICIENT = 0.99 0.47 0.64

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (0526)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.6200	51.9904
0.3100	51.1099	6.1800	52.3852
1.7900	51.2815	7.7500	52.7331
3.3300	51.6636	9.0300	53.0736

INFLOW : ID= 2 (0046)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
	92.300	5.432	10.25	83.84
OUTFLOW: ID= 1 (0526)	92.300	0.046	24.75	55.02

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 0.84
TIME SHIFT OF PEAK FLOW (min) = 870.00
MAXIMUM STORAGE USED (ha. m.) = 7.5018

ADD HYD (0516)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0004):	2580.39	15.143	22.00	45.13
+ ID2= 2 (0526):	92.30	0.046	24.75	55.02
ID = 3 (0516):	2672.69	15.188	22.00	45.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0201)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0515):	2954.54	18.149	24.50	60.76
+ ID2= 2 (0516):	2672.69	15.188	22.00	45.47
ID = 3 (0201):	5627.23	33.068	23.25	53.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0202)
IN= 2--> OUT= 1

Routing time step (min)' = 15.00

Distance	Elevation	Manning	Channel
0.00	239.00	0.1000	
690.00	230.00	0.1000 / 0.0350	Main Channel
691.00	229.20	0.0350	Main Channel
692.00	229.20	0.0350	Main Channel
693.00	229.50	0.0350 / 0.1000	Main Channel
1190.00	230.00	0.1000	
1417.00	255.00	0.1000	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.30	229.50	347E+04	0.4	0.84	136.57
0.80	230.00	864E+06	45.2	0.36	318.78
1.30	230.50	265E+07	274.1	0.71	161.20
1.80	231.00	459E+07	651.8	0.97	117.24
2.30	231.50	667E+07	1165.3	1.20	95.34
2.80	232.00	889E+07	1810.2	1.40	81.89
3.30	232.50	113E+08	2585.1	1.57	72.65
3.80	233.00	138E+08	3490.4	1.73	65.85
4.30	233.50	165E+08	4527.4	1.89	60.59
4.80	234.00	193E+08	5697.8	2.03	56.38
5.30	234.50	222E+08	7003.8	2.16	52.91
5.80	235.00	253E+08	8447.7	2.28	50.00
6.30	235.50	286E+08	10032.3	2.40	47.52
6.80	236.00	320E+08	11760.1	2.52	45.36
7.30	236.50	356E+08	13634.1	2.63	43.46
7.80	237.00	393E+08	15657.0	2.73	41.78
8.30	237.50	431E+08	17831.7	2.84	40.28
8.80	238.00	471E+08	20161.3	2.93	38.92
9.30	238.50	512E+08	22648.7	3.03	37.69

INFLOW : ID= 2 (0201)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
	5627.23	33.07	23.25	53.50	0.66	0.42
OUTFLOW: ID= 1 (0202)	5627.23	29.88	28.75	53.50	0.63	0.45

ADD HYD (0203)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0008):	1549.97	12.417	20.25	53.68
+ ID2= 2 (0202):	5627.23	29.881	28.75	53.50
ID = 3 (0203):	7177.20	38.709	26.00	53.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0301)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0112):	12793.89	68.291	30.50	60.62
+ ID2= 2 (0203):	7177.20	38.709	26.00	53.54
ID = 3 (0301):	19971.09	105.016	29.25	58.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0302)
IN= 2--> OUT= 1

Routing time step (min)' = 15.00

Distance	Elevation	Manning	Channel
0.00	240.00	0.0700	
110.00	239.00	0.0700	
161.00	238.00	0.0700	
250.00	237.00	0.0700	
420.00	236.00	0.0700	
560.00	235.00	0.0700	
589.00	234.40	0.0700 / 0.0350	Main Channel
589.10	233.60	0.0350	Main Channel
594.00	233.60	0.0350	Main Channel
594.10	234.40	0.0350 / 0.0700	Main Channel
615.00	235.00	0.0700	
740.00	236.00	0.0700	
860.00	236.50	0.0700	
1340.00	237.00	0.0700	
1460.00	238.00	0.0700	
1470.00	239.00	0.0700	
1510.00	240.00	0.0700	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.27	233.87	770E+04	1.0	0.74	132.49
0.53	234.13	155E+05	2.9	1.10	88.48
0.80	234.40	234E+05	5.5	1.37	71.19
1.15	234.75	636E+05	11.6	1.07	91.11
1.50	235.10	169E+06	24.5	0.85	114.97
1.85	235.45	431E+06	54.5	0.74	131.62
2.20	235.80	882E+06	117.3	0.78	125.39
2.55	236.15	153E+07	218.3	0.83	117.06
2.90	236.50	246E+07	375.3	0.89	109.30
3.25	236.85	394E+07	579.0	0.86	113.45
3.60	237.20	612E+07	1010.3	0.97	101.01
3.95	237.55	851E+07	1631.3	1.12	86.99
4.30	237.90	111E+08	2391.8	1.27	77.05
4.65	238.25	137E+08	3337.5	1.42	68.52
5.00	238.60	164E+08	4438.7	1.58	61.71
5.35	238.95	192E+08	5671.0	1.73	56.40
5.70	239.30	220E+08	6955.1	1.85	52.75
6.05	239.65	249E+08	8355.2	1.96	49.76
6.40	240.00	280E+08	9884.7	2.07	47.18

INFLOW : ID= 2 (0201)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
	5627.23	33.07	23.25	53.50	0.66	0.42
OUTFLOW: ID= 1 (0202)	5627.23	29.88	28.75	53.50	0.63	0.45

INFLOW : ID= 2 (0301) ***** 105.02 29.25 58.07 2.13 0.77
 OUTFLOW: ID= 1 (0302) ***** 103.05 31.25 58.07 2.12 0.77

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0012):	1501.52	10.114	22.75	54.97
+ ID2= 2 (0302):	19971.09	103.054	31.25	58.07
ID = 3 (0303):	21472.61	110.394	30.75	57.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0011) Area (ha)=3099.17 Curve Number (CN)= 57.0
 ID= 1 DT=15.0 mi n I a (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 11.10

Unit Hyd Qpeak (cms) = 10.949
 PEAK FLOW (cms) = 19.517 (i)
 TIME TO PEAK (hrs) = 22.500
 RUNOFF VOLUME (mm) = 49.909
 TOTAL RAINFALL (mm) = 130.880
 RUNOFF COEFFICIENT = 0.381

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (0401) Routing time step (min)' = 15.00
 IN= 2----> OUT= 1

Distance	Elevation	Manning
0.00	236.00	0.0500
72.00	235.00	0.0500
492.00	230.00	0.0500
581.00	229.20	0.0500 / 0.0350
582.00	228.40	0.0350
583.00	228.40	0.0350
584.00	229.20	0.0350 / 0.0500
588.00	229.20	0.0500
607.00	230.00	0.0500
627.00	235.00	0.0500

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.27	228.67	.302E+04	0.1	0.21	666.69
0.53	228.93	.756E+04	0.3	0.30	465.92
0.80	229.20	.136E+05	0.6	0.37	377.99
1.16	229.56	.111E+06	3.0	0.23	616.66
1.52	229.92	.358E+06	11.5	0.27	519.91
1.89	230.29	.740E+06	30.1	0.35	410.09
2.25	230.65	.122E+07	59.5	0.41	342.49
2.61	231.01	.180E+07	100.6	0.47	298.58
2.97	231.37	.248E+07	154.6	0.53	267.22
3.34	231.74	.326E+07	222.9	0.58	243.40
3.70	232.10	.413E+07	306.6	0.63	224.51
4.06	232.46	.510E+07	406.9	0.68	209.05
4.42	232.82	.617E+07	524.8	0.72	196.10
4.79	233.19	.734E+07	661.4	0.77	185.06
5.15	233.55	.861E+07	817.9	0.81	175.49
5.51	233.91	.998E+07	995.2	0.85	167.10
5.87	234.27	.114E+08	1194.3	0.89	159.67
6.24	234.64	.130E+08	1416.3	0.93	153.03
6.60	235.00	.147E+08	1662.2	0.96	147.05

AREA hydrograph <----> <--pi pe / channel-->
 INFLOW : ID= 2 (0011) 3099.17 19.52 22.50 49.91 1.68 0.30
 OUTFLOW: ID= 1 (0401) 3099.17 14.77 29.00 49.91 1.59 0.28

CALIB NASHYD (0014) Area (ha)=2163.09 Curve Number (CN)= 69.0
 ID= 1 DT=15.0 mi n I a (mm)= 5.00 # of Linear Res. (N)= 3.10
 U. H. Tp(hrs)= 25.10

Unit Hyd Qpeak (cms) = 3.379
 PEAK FLOW (cms) = 8.614 (i)
 TIME TO PEAK (hrs) = 36.500
 RUNOFF VOLUME (mm) = 66.025
 TOTAL RAINFALL (mm) = 130.880
 RUNOFF COEFFICIENT = 0.504

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0014):	2163.09	8.614	36.50	66.03
+ ID2= 2 (0401):	3099.17	14.771	29.00	49.91
ID = 3 (0402):	5262.26	22.690	30.75	56.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0303):	21472.61	110.394	30.75	57.85
+ ID2= 2 (0402):	5262.26	22.690	30.75	56.53
ID = 3 (0501):	26734.87	133.084	30.75	57.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0503) Routing time step (min)' = 15.00
 IN= 2----> OUT= 1

Distance	Elevation	Manning
0.00	237.00	0.0700
425.00	235.00	0.0700
445.00	234.00	0.0700
480.00	233.50	0.0700
1535.00	233.50	0.0700 / 0.0350
1536.50	232.70	0.0350
1537.50	232.70	0.0350
1540.00	233.50	0.0350 / 0.0700
1605.00	233.20	0.0700
1610.00	233.10	0.0700
2135.00	233.10	0.0700
2440.00	233.50	0.0700
2505.00	233.50	0.0700
2530.00	234.00	0.0700
2535.00	235.00	0.0700
2540.00	236.00	0.0700
2605.00	238.00	0.0700
2645.00	241.00	0.0700

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.20	232.90	.385E+03	0.2	0.55	38.92
0.40	233.10	.103E+04	0.6	0.80	26.75
0.60	233.30	.159E+06	39.5	0.32	67.02
0.80	233.50	.366E+06	133.3	0.47	45.74
1.03	233.73	.977E+06	428.7	0.56	37.97
1.27	233.97	.160E+07	914.3	0.74	29.09
1.50	234.20	.222E+07	1556.9	0.90	23.78
1.73	234.43	.285E+07	2336.2	1.05	20.32
1.97	234.67	.348E+07	3240.2	1.20	17.88
2.20	234.90	.411E+07	4260.9	1.33	16.07
2.43	235.13	.474E+07	5359.1	1.45	14.75
2.67	235.37	.539E+07	6537.6	1.56	13.74
2.90	235.60	.605E+07	7819.4	1.66	12.90
3.13	235.83	.673E+07	9203.8	1.76	12.19

3.37	236.07	.743E+07	10683.6	1.85	11.59
3.60	236.30	.814E+07	12247.8	1.93	11.07
3.83	236.53	.887E+07	13912.8	2.01	10.62
4.07	236.77	.961E+07	15679.0	2.09	10.22
4.30	237.00	.104E+08	17546.8	2.17	9.85

<--- hydrograph --->						<-pi pe / channel ->		
AREA	OPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL			
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)			
INFLOW : ID= 2 (0501)	*****	133.08	30.75	57.59				
OUTFLOW: ID= 1 (0503)	*****	133.24	31.00	57.59	0.80	0.47		

RESERVOIR (0510)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	27.8300	83.9600
3.8000	14.0500	*****	101.7400
11.0800	33.6500	*****	145.2700
20.8900	61.8900	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0503)	*****	133.237	31.00
OUTFLOW: ID= 1 (0510)	*****	133.204	31.25

PEAK FLOW REDUCTION [Qout/Qin] (%) = 99.97
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha. m.) = 87.0230

ROUTE CHN (0513)
IN= 2--> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1.1) ----->					
Distance	Elevation	Manning			
0.00	237.00	0.0700			
425.00	235.00	0.0700			
445.00	234.00	0.0700			
480.00	233.50	0.0700			
1536.50	232.70	0.0700 / 0.0350	Main Channel		
1537.50	232.70	0.0350	Main Channel		
1540.00	233.50	0.0350	Main Channel		
1605.00	233.20	0.0700	Main Channel		
1610.00	233.10	0.0700			
2135.00	233.10	0.0700			
2440.00	233.50	0.0700			
2505.00	233.50	0.0700			
2530.00	234.00	0.0700			
2535.00	235.00	0.0700			
2540.00	236.00	0.0700			
2605.00	238.00	0.0700			
2645.00	241.00	0.0700			

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.20	232.90	.291E+03	0.2	0.55	29.43
0.40	233.10	.777E+03	0.6	0.80	20.23
0.60	233.30	.120E+06	39.5	0.32	50.68
0.80	233.50	.277E+06	133.3	0.47	34.59
1.03	233.73	.739E+06	428.7	0.56	28.72
1.27	233.97	.121E+07	914.3	0.74	22.00
1.50	234.20	.168E+07	1556.9	0.90	17.98
1.73	234.43	.215E+07	2336.2	1.05	15.37
1.97	234.67	.263E+07	3240.2	1.20	13.52
2.20	234.90	.311E+07	4260.9	1.33	12.15
2.43	235.13	.359E+07	5359.1	1.45	11.15
2.67	235.37	.408E+07	6537.6	1.56	10.39
2.90	235.60	.458E+07	7819.4	1.66	9.76
3.13	235.83	.509E+07	9203.8	1.76	9.22
3.37	236.07	.562E+07	10683.6	1.85	8.76
3.60	236.30	.615E+07	12247.8	1.93	8.37
3.83	236.53	.670E+07	13912.8	2.01	8.03
4.07	236.77	.727E+07	15679.0	2.09	7.73
4.30	237.00	.784E+07	17546.8	2.17	7.45

<--- hydrograph --->						<-pi pe / channel ->		
AREA	OPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL			
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)			
INFLOW : ID= 2 (0505)	*****	147.56	30.25	58.26	2.73	0.89		
OUTFLOW: ID= 1 (0506)	*****	143.99	33.25	58.26	2.70	0.89		

INFLOW : ID= 2 (0510)	*****	133.20	31.25	57.59	0.80	0.47
OUTFLOW: ID= 1 (0513)	*****	133.07	31.50	57.59	0.80	0.46

ADD HYD (0504)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0015):	1059.88	11.257	20.25
+ ID2= 2 (0513):	26734.87	133.072	31.50
=====			
ID = 3 (0504):	27794.75	138.630	30.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0505)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0013):	1627.59	9.275	27.00
+ ID2= 2 (0504):	27794.75	138.630	30.25
=====			
ID = 3 (0505):	29422.34	147.564	30.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (0506)
IN= 2--> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1.1) ----->			
Distance	Elevation	Manning	
0.00	251.00	0.0500	
323.00	238.00	0.0500	
400.00	229.00	0.0500	
545.00	228.00	0.0500	
602.00	226.00	0.0500	
604.00	225.80	0.0500 / 0.0350	Main Channel
605.00	225.00	0.0350	Main Channel
609.00	225.00	0.0350	Main Channel
610.00	225.80	0.0350 / 0.0500	Main Channel
614.00	226.00	0.0500	
650.00	227.00	0.0500	
810.00	229.50	0.0500	
1855.00	240.00	0.0500	
2617.00	259.00	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.80	225.80	.292E+05	3.5	0.87	139.36
2.13	227.13	.441E+06	50.6	0.84	145.20
3.45	228.45	.197E+07	262.2	0.97	125.42
4.78	229.78	.557E+07	971.2	1.27	95.57
6.11	231.11	.106E+08	2326.5	1.61	75.63
7.43	232.43	.169E+08	4425.6	1.90	63.77
8.76	233.76	.247E+08	7361.8	2.17	55.90
10.08	235.08	.338E+08	11229.7	2.42	50.22
11.41	236.41	.444E+08	16121.0	2.65	45.87
12.74	237.74	.563E+08	22124.8	2.86	42.40
14.06	239.06	.697E+08	29005.2	3.03	40.02
15.39	240.39	.846E+08	37433.0	3.23	37.66
16.72	241.72	.101E+09	47908.6	3.47	34.97
18.04	243.04	.117E+09	59742.1	3.71	32.73
19.37	244.37	.135E+09	72949.0	3.94	30.83
20.69	245.69	.153E+09	87549.6	4.16	29.20
22.02	247.02	.173E+09	*****	4.37	27.79
23.35	248.35	.193E+09	*****	4.58	26.55
24.67	249.67	.214E+09	*****	4.77	25.45

<--- hydrograph --->						<-pi pe / channel ->		
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)			
INFLOW : ID= 2 (0505)	*****	147.56	30.25	58.26	2.73	0.89		
OUTFLOW: ID= 1 (0506)	*****	143.99	33.25	58.26	2.70	0.89		

RESERVOIR (0512)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	*****	16.2850
27.2000	3.7650	*****	20.9160
76.8000	7.7410	*****	25.9120
*****	11.0650	*****	36.6600
*****	11.9150	0.0000	0.0000

INFLOW : ID= 2 (0506)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
*****	143.991	33.25	58.26	
OUTFLOW: ID= 1 (0512)	*****	144.070	33.25	58.26

PEAK FLOW REDUCTION [Qout/Qin](%)=100.05
 TIME SHIFT OF PEAK FLOW (min)= 0.00
 MAXIMUM STORAGE USED (ha.m.)= 11.1202

**** WARNING : HYDROGRAPH PEAK WAS NOT REDUCED.
 CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.

ROUTE CHN (0514)
 IN= 2--> OUT= 1

Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1.1) ----->

Distance	Elevation	Manning	
0.00	230.00	0.0500	
400.00	229.00	0.0500	
545.00	228.00	0.0500	
602.00	226.00	0.0500	
604.00	225.80	0.0500 / 0.0350	Main Channel
605.00	225.00	0.0350	Main Channel
609.00	225.00	0.0350	Main Channel
610.00	225.80	0.0350 / 0.0500	Main Channel
614.00	226.00	0.0500	
650.00	227.00	0.0500	
810.00	229.50	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.20	225.20	.243E+04	0.3	0.39	121.91
0.40	225.40	.518E+04	1.1	0.59	80.58
0.60	225.60	.815E+04	2.1	0.74	64.02
0.80	225.80	.114E+05	3.5	0.87	54.69
1.05	226.05	.184E+05	6.1	0.94	50.48
1.29	226.29	.346E+05	10.3	0.85	55.99
1.54	226.54	.620E+05	17.4	0.80	59.38
1.79	226.79	.101E+06	28.3	0.81	59.18
2.03	227.03	.151E+06	43.8	0.83	57.35
2.28	227.28	.215E+06	63.9	0.85	55.99
2.53	227.53	.295E+06	91.4	0.89	53.82
2.77	227.77	.391E+06	127.0	0.93	51.35
3.02	228.02	.504E+06	170.6	0.97	49.24
3.27	228.27	.644E+06	215.5	0.96	49.84
3.51	228.51	.821E+06	279.7	0.97	48.92
3.76	228.76	.103E+07	363.1	1.00	47.47
4.01	229.01	.128E+07	466.2	1.04	45.89
4.25	229.25	.159E+07	572.0	1.03	46.41
4.50	229.50	.198E+07	719.1	1.04	45.95

<---- hydrograph ----> <- pipe / channel ->

INFLOW : ID= 2 (0512)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
*****	144.07	33.25	58.26	2.87	0.94	
OUTFLOW: ID= 1 (0514)	*****	143.57	34.00	58.26	2.86	0.94

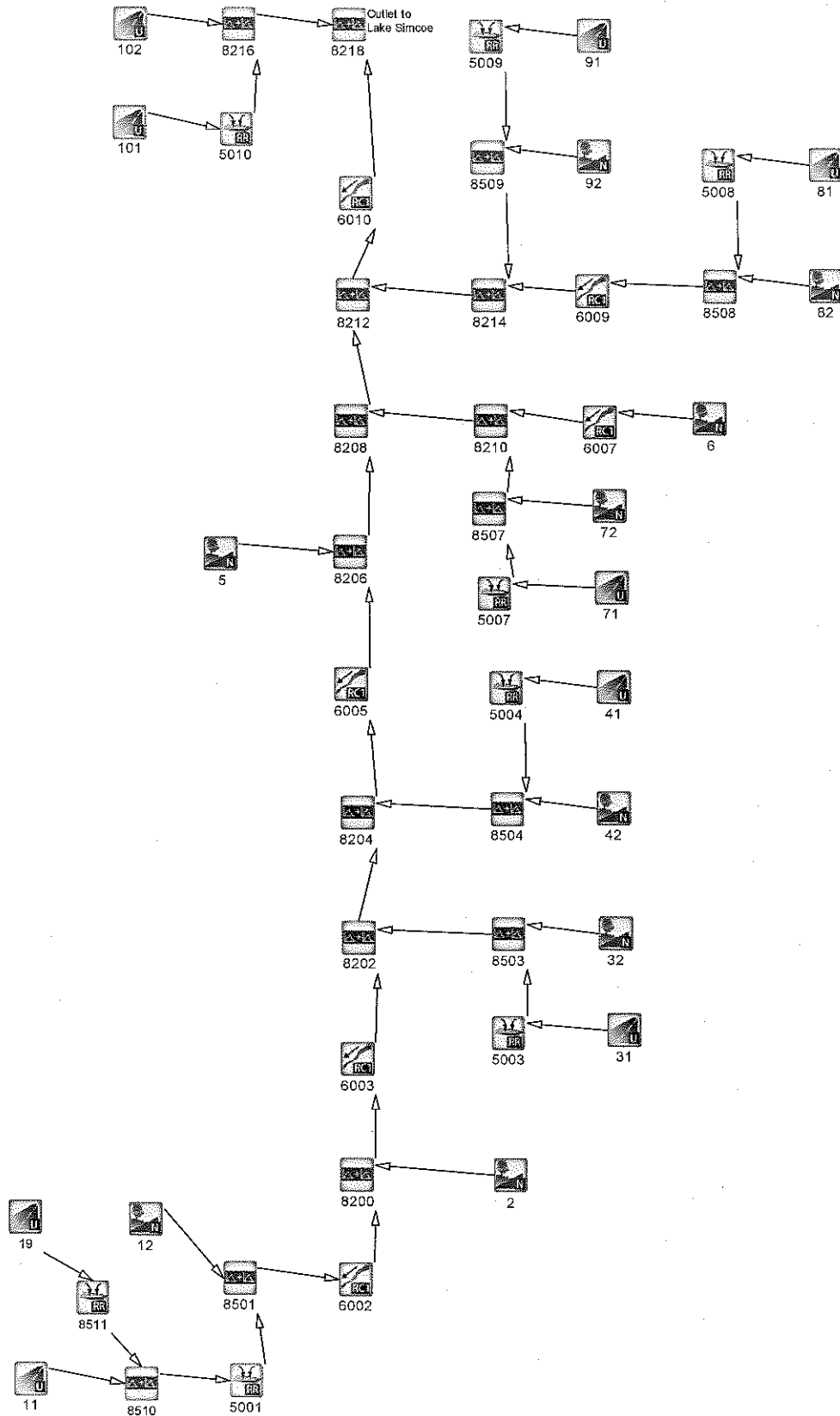
ADD HYD (0507)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0016):	3094.49	18.718	29.50	73.66
+ ID2= 2 (0514):	29422.34	143.567	34.00	58.26
=====				
ID = 3 (0507):	32516.83	161.383	33.50	59.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

MASKINONGE RIVER SUBWATERSHED



**FUTURE
MASKINONGE RIVER 2-100 YEAR STORMS**

```
V V I SSSS U U A L
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
WV I SSSS UUUU A A LLLL

OOO TTTT TTTT H H Y Y M M OOO TM
O O T T H H Y Y MM MM O O
O O T T H H Y Y M M O O
OOO T T H H Y Y M M OOO Company Serial
```

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***** DETAILED OUTPUT *****

Input filename: C:\program files\Visual Ortymo 2.4r\VO2\voind.dat
Output filename: C:\Users\jscott\AppData\Local\Temp\ad939110-f7bb-4285-9226-bce91a941061\Scenario.out
Summary filename: C:\Users\jscott\AppData\Local\Temp\ad939110-f7bb-4285-9226-bce91a941061\Scenario.sum

DATE: 08/22/2012 TIME: 02:19:42

USER:

COMMENTS: _____

** SIMULATION NUMBER: 1 **

```
READ STORM | Filename: C:\Users\jscott\AppData
            | Local\Temp\
            | ad939110-f7bb-4285-9226-bce91a941061\343e2ee8
            | Comments: 2-Year 12-Hour SCS II Design Storm
-----
```

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.05	3.25	1.68	6.25	7.56	9.25	1.47
0.50	1.05	3.50	1.68	6.50	7.56	9.50	1.47
0.75	1.05	3.75	1.68	6.75	7.56	9.75	1.47
1.00	1.05	4.00	1.68	7.00	7.56	10.00	1.47
1.25	1.05	4.25	1.68	7.25	7.56	10.25	1.47
1.50	1.05	4.50	1.68	7.50	7.56	10.50	1.47
1.75	1.05	4.75	1.68	7.75	7.56	10.75	1.47
2.00	1.05	5.00	1.68	8.00	7.56	11.00	1.47
2.25	1.26	5.25	5.04	8.25	14.40	11.25	1.47
2.50	1.26	5.50	5.04	8.50	14.40	11.50	1.47
2.75	1.26	5.75	5.04	8.75	14.40	11.75	1.47
3.00	1.26	6.00	5.04	9.00	14.40	12.00	1.47

```
CALIB | STANHYD (0082) | Area (ha)= 10.53
      | ID= 1 DT=15.0 min | Total Imp(%)= 45.00 Dir. Con. (%)= 15.00
```

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	4.74	5.79
Dep. Storage (mm)=	0.80	2.50
Average Slope (%)=	1.00	2.00
Length (m)=	264.95	40.00
Mannings n	0.113	0.250

Max. Eff. Inten. (mm/hr)=	55.44	26.42	
over (min)	15.00	30.00	
Storage Coeff. (min)=	5.80 (ii)	17.82 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.10	0.05	

PEAK FLOW (cms)=	0.23	0.23	0.373 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	41.20	11.45	15.91
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.98	0.27	0.38

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 61.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
RESERVOIR (5009)
IN= 2--> OUT= 1
DT= 15.0 min
```

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0360	0.1287	0.8850	0.3136
	0.2040	0.1489	1.0300	0.3521
	0.3800	0.1922	1.4300	0.3621
	0.5270	0.2294	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0081)	10.530	0.373	6.00	15.91
OUTFLOW: ID= 1 (5008)	10.530	0.032	8.25	15.83

	PEAK FLOW REDUCTION [Qout/Qin] (%)	TIME SHIFT OF PEAK FLOW (min)	MAXIMUM STORAGE USED (ha.m.)
	8.46	135.00	0.1129

```
CALIB | NASHVD (0082) | Area (ha)= 402.65 Curve Number (CN)= 63.3
      | ID= 1 DT=15.0 min | Ia (mm)= 2.50 # of Linear Res. (N)= 3.50
      | U.N. Tp (hrs)= 1.63
```

Unit Hyd Qpeak (cms)= 10.622

PEAK FLOW (cms)=	2.300 (i)
TIME TO PEAK (hrs)=	7.750
RUNOFF VOLUME (mm)=	8.355
TOTAL RAINFALL (mm)=	42.000
RUNOFF COEFFICIENT =	0.199

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```
ADD HYD (8508)
1 + 2 = 3
```

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 1 (0082):	402.65	2.300	7.75	8.35
+ ID2 = 2 (5008):	10.53	0.032	8.25	15.83
ID = 3 (8508):	413.18	2.332	7.75	8.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
ROUTE CHN (6009)
IN= 2--> OUT= 1
Routing time step (min)= 15.00
```

```
<----- DATA FOR SECTION ( 9.1) ----->
```

Distance	Elevation	Manning
0.00	232.41	0.0600
16.80	232.21	0.0600
28.00	231.55	0.0600
117.61	229.36	0.0600
235.21	228.08	0.0600
296.81	226.96	0.0600
316.00	226.17	0.0600
326.00	226.17	0.0600
332.00	226.04	0.0600
335.50	225.16	0.0350 Main Channel
336.00	225.08	0.0350 Main Channel
337.00	225.08	0.0350 Main Channel
337.90	225.21	0.0350 Main Channel
339.90	225.93	0.0350 / 0.0600 Main Channel
345.00	226.28	0.0600
358.42	226.57	0.0600
403.22	227.73	0.0600
448.02	229.07	0.0600
492.82	230.51	0.0600
554.43	232.28	0.0600

```
<----- TRAVEL TIME TABLE ----->
```

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.28	225.36	255E+04	0.3	0.37	160.52
0.57	225.65	719E+04	1.1	0.55	107.34
0.85	225.93	138E+05	2.7	0.70	85.08
1.25	226.33	405E+05	7.8	0.69	86.59
1.64	226.72	109E+06	20.0	0.65	91.15
2.04	227.12	216E+06	41.2	0.68	87.34
2.44	227.52	372E+06	74.8	0.72	82.92

2.83	227.91	.580E+06	125.6	0.77	77.03
3.23	228.31	.842E+06	193.5	0.82	72.57
3.63	228.71	.117E+07	284.7	0.87	68.66
4.02	229.10	.157E+07	405.4	0.92	64.71
4.42	229.50	.204E+07	564.5	0.99	60.30
4.82	229.90	.256E+07	766.1	1.07	55.64
5.22	230.30	.311E+07	1001.2	1.15	51.82
5.61	230.69	.371E+07	1269.8	1.22	48.68
6.01	231.09	.435E+07	1573.3	1.29	46.05
6.41	231.49	.503E+07	1914.5	1.36	43.77
6.80	231.88	.575E+07	2314.0	1.44	41.39
7.20	232.28	.650E+07	2741.3	1.50	39.49

<---- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8508) 413.18 2.33 7.75 8.55 0.78 0.66
 OUTFLOW: ID= 1 (6009) 413.18 1.65 8.75 8.54 0.66 0.60

CALIB
 STANDHYD (0091) Area (ha)= 11.32
 ID= 1 DT=15.0 min Total Imp(%)= 50.00 Dir. Conn.(%)= 20.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	5.66		
Dep. Storage (mm)=	0.80	2.50	
Average Slope (%)=	1.00	2.00	
Length (m)=	274.71	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	55.44	26.42	
over (min)	15.00	30.00	
Storage Coeff. (min)=	5.93 (ii)	17.95 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.10	0.05	
TOTALS			
PEAK FLOW (cms)=	0.33	0.22	0.468 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	41.20	11.08	17.10
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.98	0.26	0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING:FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5009)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.7570	0.3065
0.0380	0.1497	0.9510	0.3465
0.2190	0.1748	1.1070	0.3857
0.4090	0.2196	1.5070	0.3957
0.5670	0.2594	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
11.320	0.468	6.00	17.10
11.320	0.034	8.25	17.01

PEAK FLOW REDUCTION [Qout/Qin](%)= 7.23
 TIME SHIFT OF PEAK FLOW (min)=135.00
 MAXIMUM STORAGE USED (ha.m.)= 0.1334

CALIB
 NASHYD (0092) Area (ha)= 451.25 Curve Number (CN)= 60.8
 ID= 1 DT=15.0 min Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
 U.H. Tp(hrs)= 2.11

Unit Hyd Qpeak (cms)= 9.199

PEAK FLOW (cms)=	1.951 (i)
TIME TO PEAK (hrs)=	8.250
RUNOFF VOLUME (mm)=	7.676
TOTAL RAINFALL (mm)=	42.000
RUNOFF COEFFICIENT =	0.183

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8509)
 1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
451.25	1.951	8.25	7.68
11.32	0.034	8.25	17.01
462.57	1.985	8.25	7.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8214)
 1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
413.18	1.651	8.75	8.54
462.57	1.985	8.25	7.90
875.75	3.589	8.50	8.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (0011) Area (ha)= 2.54
 ID= 1 DT=15.0 min Total Imp(%)= 45.00 Dir. Conn.(%)= 15.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	1.14		
Dep. Storage (mm)=	0.80	2.50	
Average Slope (%)=	1.00	2.00	
Length (m)=	130.13	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	55.44	24.41	
over (min)	15.00	30.00	
Storage Coeff. (min)=	3.79 (ii)	16.20 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.11	0.05	
TOTALS			
PEAK FLOW (cms)=	0.06	0.05	0.091 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	41.20	10.61	15.20
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.98	0.25	0.36

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING:FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0019) Area (ha)= 487.70
 ID= 1 DT=15.0 min Total Imp(%)= 45.00 Dir. Conn.(%)= 35.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	219.46	268.24	
Dep. Storage (mm)=	0.80	5.00	
Average Slope (%)=	1.00	2.00	
Length (m)=	1803.15	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	55.44	21.17	
over (min)	15.00	45.00	
Storage Coeff. (min)=	18.34 (ii)	31.48 (ii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.06	0.03	
TOTALS			
PEAK FLOW (cms)=	17.47	5.76	19.769 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	41.20	11.76	22.06
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.98	0.28	0.53

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (8511)
 IN= 2--> OUT= 1

DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	24.3900	10.5911
	1.6600	5.9307	32.6300	12.6704
	9.4600	6.8569	40.9700	14.4958
	17.6100	8.8657	47.7000	16.2820

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0019)	487.700	19.769	6.00	22.06
OUTFLOW : ID= 1 (8511)	487.700	5.103	7.25	22.06

PEAK FLOW REDUCTION [Qout/Qin](%) = 25.82
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha.m.) = 6.3679

ADD HYD (8510)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0011):	2.54	0.091	6.00	15.20
+ ID2= 2 (8511):	487.70	5.103	7.25	22.06
ID = 3 (8510):	490.24	5.116	7.25	22.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5001)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1	0.0000	0.0000	4.6830	11.8098
DT= 15.0 min	0.0540	0.1960	5.4380	13.3066
	1.2810	6.8008	6.1990	14.9402
	2.5090	8.6337	6.5990	14.9502
	3.4630	9.9865	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8510)	490.240	5.116	7.25	22.02
OUTFLOW : ID= 1 (5001)	490.240	0.900	17.50	22.02

PEAK FLOW REDUCTION [Qout/Qin](%) = 17.59
 TIME SHIFT OF PEAK FLOW (min) = 615.00
 MAXIMUM STORAGE USED (ha.m.) = 4.7497

CALIB	Area (ha)	Curve Number (CN)
NASHVD (0012)	630.33	11.8098
ID= 1 DT=15.0 min	Ia (mm) = 2.50	# of Linear Res.(N) = 3.50
	U.H. Tp(hrs) = 2.24	

Unit Hyd Qpeak (cms) = 12.094

PEAK FLOW (cms) = 2.619 (i)
 TIME TO PEAK (hrs) = 8.500
 RUNOFF VOLUME (mm) = 7.728
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.184

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8501)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0012):	630.33	2.619	8.50	7.73
+ ID2= 2 (5001):	490.24	0.900	17.50	22.02
ID = 3 (8501):	1120.57	3.161	8.50	13.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6002)	Routing time step (min) = 15.00
------------------	---------------------------------

Distance	Elevation	Manning
0.00	235.70	0.0700
15.10	235.42	0.0700
30.21	235.01	0.0700
37.76	234.49	0.0700
45.31	231.43	0.0700
56.64	231.55	0.0700
98.18	229.87	0.0700
117.06	229.64	0.0700

124.61	229.03	0.0700 / 0.0300	Main Channel
128.38	228.69	0.0300	Main Channel
132.16	228.58	0.0300	Main Channel
143.49	228.98	0.0300 / 0.0700	Main Channel
158.59	229.36	0.0700	
181.25	229.58	0.0700	
200.13	229.96	0.0700	
237.89	231.19	0.0700	
268.09	232.60	0.0700	
302.08	233.40	0.0700	
332.29	234.25	0.0700	
373.82	235.68	0.0700	

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.20	228.78	639E+04	0.3	0.26	359.77
0.40	228.98	222E+05	1.6	0.40	227.92
0.79	229.37	842E+05	9.7	0.63	145.25
1.19	229.77	218E+06	25.5	0.65	142.16
1.58	230.16	441E+06	53.8	0.67	136.74
1.98	230.56	715E+06	94.9	0.73	125.69
2.37	230.95	104E+07	148.9	0.79	116.12
2.76	231.34	141E+07	217.2	0.85	107.88
3.16	231.74	183E+07	299.4	0.90	101.81
3.55	232.13	228E+07	403.8	0.97	94.28
3.95	232.53	276E+07	523.3	1.04	87.91
4.34	232.92	326E+07	650.4	1.10	83.62
4.74	233.32	380E+07	792.5	1.15	79.99
5.13	233.71	438E+07	954.7	1.20	76.48
5.52	234.10	499E+07	1134.7	1.25	73.30
5.92	234.50	563E+07	1334.6	1.31	70.34
6.31	234.89	631E+07	1540.4	1.35	68.23
6.71	235.29	702E+07	1746.8	1.37	67.00
7.10	235.68	780E+07	1958.9	1.38	66.34

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8501)	1120.57	3.16	8.50	13.98	0.48	0.43
OUTFLOW : ID= 1 (6002)	1120.57	1.95	11.75	13.98	0.42	0.41

CALIB	Area (ha)	Curve Number (CN)
NASHVD (0002)	869.77	11.8098
ID= 1 DT=15.0 min	Ia (mm) = 2.50	# of Linear Res.(N) = 3.50
	U.H. Tp(hrs) = 3.32	

Unit Hyd Qpeak (cms) = 11.293

PEAK FLOW (cms) = 2.458 (i)
 TIME TO PEAK (hrs) = 9.750
 RUNOFF VOLUME (mm) = 7.078
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.169

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8200)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0002):	869.77	2.458	9.75	7.08
+ ID2= 2 (6002):	1120.57	1.946	11.75	13.98
ID = 3 (8200):	1990.34	4.254	10.50	10.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6003)	Routing time step (min) = 15.00
------------------	---------------------------------

Distance	Elevation	Manning	
0.00	234.33	0.0700	
50.41	229.32	0.0700	
94.52	227.15	0.0700	
113.42	226.02	0.0700	
116.57	225.86	0.0700	
125.32	224.53	0.0700	
130.32	224.24	0.0700 / 0.0300	Main Channel
130.82	223.78	0.0300	Main Channel
131.82	223.63	0.0300	Main Channel
132.42	223.79	0.0300	Main Channel
133.42	224.43	0.0300 / 0.0700	Main Channel
163.83	226.19	0.0700	
189.03	227.19	0.0700	
229.99	228.02	0.0700	
270.95	228.65	0.0700	

286.70 229.01 0.0700
305.61 229.31 0.0700

TRAVEL TIME TABLE						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	(min)
0.20	223.83	564E+03	0.1	0.26	175.53	
0.41	224.04	168E+04	0.3	0.44	105.08	
0.61	224.24	309E+04	0.6	0.56	82.17	
0.93	224.56	847E+04	1.8	0.57	80.17	
1.24	224.87	209E+05	4.1	0.54	84.69	
1.56	225.19	399E+05	8.0	0.55	83.36	
1.88	225.51	654E+05	13.7	0.57	79.74	
2.19	225.82	976E+05	21.5	0.61	75.55	
2.51	226.14	138E+06	31.1	0.62	73.71	
2.83	226.46	188E+06	43.6	0.64	71.92	
3.15	226.78	250E+06	59.8	0.66	69.63	
3.46	227.09	324E+06	80.5	0.68	67.03	
3.78	227.41	411E+06	103.4	0.69	66.15	
4.10	227.73	516E+06	132.2	0.70	65.13	
4.41	228.04	642E+06	168.2	0.72	63.59	
4.73	228.36	788E+06	210.4	0.73	62.46	
5.05	228.68	959E+06	262.8	0.75	60.81	
5.36	228.99	115E+07	328.9	0.79	58.23	
5.68	229.31	136E+07	401.2	0.81	56.49	

hydrograph		<-pipe / channel->	
AREA	QPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8200) 1990.34	4.25	10.50	10.96
OUTFLOW: ID= 1 (6003) 1990.34	3.82	12.00	10.96

CALIB	Area (ha)	Dir. Com. (%)
STANDHYD (0031)	6.23	47.00
ID= 1 DT=15.0 min	Total Imp (%) = 65.00	

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)	4.05	2.18	
Dep. Storage (mm)	0.80	2.50	
Average Slope (%)	1.00	2.00	
Length (m)	203.80	40.00	
Mannings n	0.013	0.250	

Max. Eff. Inten. (mm/hr)	55.44	19.56
over (min)	15.00	30.00
Storage Coeff. (min)	4.96 (ii)	18.51 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.11	0.05

TOTALS

PEAK FLOW (cms)	0.44	0.06	0.475 (iii)
TIME TO PEAK (hrs)	6.00	6.25	6.00
RUNOFF VOLUME (mm)	41.20	8.75	24.00
TOTAL RAINFALL (mm)	42.00	42.00	42.00
RUNOFF COEFFICIENT	0.98	0.21	0.57

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 53.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5003)	OUTFLOW		STORAGE	
DT= 15.0 min	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	0.4170	0.1860
	0.0210	0.0993	0.5230	0.2047
	0.1210	0.1182	0.6090	0.2231
	0.2250	0.1402	1.0090	0.2331
	0.3120	0.1519	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0031)	6.230	0.475	6.00	24.00
OUTFLOW: ID= 1 (5003)	6.230	0.040	7.00	23.81

PEAK FLOW REDUCTION [Qout/Qin] (%)	8.32
TIME SHIFT OF PEAK FLOW (min)	60.00
MAXIMUM STORAGE USED (ha.m.)	0.1031

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0032)	461.62	57.8
ID= 1 DT=15.0 min	Ia (mm) = 2.50	# of Linear Res. (N) = 3.50
	U.H. Tp (hrs) = 2.40	

Unit Hyd Qpeak (cms) = 8.267

PEAK FLOW (cms) = 1.629 (i)
TIME TO PEAK (hrs) = 8.500
RUNOFF VOLUME (mm) = 6.936
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.165

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8503)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0032):	461.62	1.629	8.50	6.94
+ ID2= 2 (5003):	6.23	0.040	7.00	23.81
ID = 3 (8503):	467.85	1.654	8.50	7.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8202)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6003):	1990.34	3.825	12.00	10.96
+ ID2= 2 (8503):	467.85	1.654	8.50	7.16
ID = 3 (8202):	2458.19	4.741	11.00	10.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area (ha)	Dir. Com. (%)
STANDHYD (0041)	8.66	22.00
ID= 1 DT=15.0 min	Total Imp (%) = 52.00	

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)	4.50	4.16	
Dep. Storage (mm)	0.80	2.50	
Average Slope (%)	1.00	2.00	
Length (m)	240.28	40.00	
Mannings n	0.013	0.250	

Max. Eff. Inten. (mm/hr)	55.44	20.51
over (min)	15.00	30.00
Storage Coeff. (min)	5.47 (ii)	18.77 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.11	0.05

TOTALS

PEAK FLOW (cms)	0.28	0.12	0.357 (iii)
TIME TO PEAK (hrs)	6.00	6.25	6.00
RUNOFF VOLUME (mm)	41.20	8.56	15.74
TOTAL RAINFALL (mm)	42.00	42.00	42.00
RUNOFF COEFFICIENT	0.98	0.20	0.37

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 50.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5004)	OUTFLOW		STORAGE	
DT= 15.0 min	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	0.5790	0.2368
	0.0290	0.1168	0.7270	0.2669
	0.1680	0.1367	0.8470	0.2865
	0.3120	0.1706	1.2470	0.3065
	0.4330	0.2010	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0041)	8.660	0.357	6.00	15.74
OUTFLOW: ID= 1 (5004)	8.660	0.023	8.25	15.62

PEAK FLOW REDUCTION [Qout/Qin] (%)	6.56
TIME SHIFT OF PEAK FLOW (min)	135.00
MAXIMUM STORAGE USED (ha.m.)	0.0945

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0042)	376.06	53.3
ID= 1 DT=15.0 min	Ia (mm) = 2.50	# of Linear Res. (N) = 3.50
	U.H. Tp (hrs) = 1.62	

Unit Hyd Qpeak (cms)= 9.964

PEAK FLOW (cms)= 1.522 (1)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 5.954
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.142

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8504)
1 + 2 = 3
AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 1 (0042): 376.06 1.522 7.75 5.95
+ ID2= 2 (5004): 8.66 0.023 8.25 15.62
ID = 3 (8504): 384.72 1.545 7.75 6.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8204)
1 + 2 = 3
AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 1 (8202): 2458.19 4.741 11.00 10.24
+ ID2= 2 (8504): 384.72 1.545 7.75 6.17
ID = 3 (8204): 2842.91 5.312 10.50 9.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6005)
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

Table with columns: Distance, Elevation, Manning. Rows show channel data points from 0.00 to 185.20.

TRAVEL TIME TABLE
DEPT# ELEV (m) VOLUME (cu.m.) FLOW RATE (cms) VELOCITY (m/s) TRAV.TIME (min)

hydrograph <--> <-pipe / channel->
AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm) MAX DEPTH (m) MAX VEL (m/s)

CALIB NASHVD (0005)
Area (ha)= 340.11 Curve Number (CN)= 69.5
ID= 1 DT=15.0 min Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
U.H. Tp(hrs)= 2.45

Unit Hyd Qpeak (cms)= 5.972

PEAK FLOW (cms)= 1.781 (1)
TIME TO PEAK (hrs)= 8.750
RUNOFF VOLUME (mm)= 10.335
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.246

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8206)
1 + 2 = 3
AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 1 (0005): 340.11 1.781 8.75 10.34
+ ID2= 2 (6005): 2842.91 4.725 12.50 9.69
ID = 3 (8206): 3183.02 5.616 11.50 9.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHVD (0006)
Area (ha)=1375.77 Curve Number (CN)= 55.0
ID= 1 DT=15.0 min Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
U.H. Tp(hrs)= 3.90

Unit Hyd Qpeak (cms)= 15.192

PEAK FLOW (cms)= 3.064 (1)
TIME TO PEAK (hrs)= 10.500
RUNOFF VOLUME (mm)= 6.309
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.150

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6007)
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (7.1)
Distance Elevation Manning
0.00 225.91 0.0700
51.88 225.32 0.0700
135.69 224.23 0.0700
183.58 223.08 0.0700
211.52 223.28 0.0700
223.49 222.51 0.0700
231.43 220.69 0.0700
244.53 219.78 0.0700 / 0.0500 Main Channel
249.23 219.60 0.0500 Main Channel
249.93 219.46 0.0500 Main Channel
251.43 219.23 0.0500 Main Channel
253.50 219.70 0.0500 / 0.0700 Main Channel
261.10 219.90 0.0700
271.38 221.01 0.0700
283.36 223.77 0.0700
295.33 224.21 0.0700
331.25 226.04 0.0700
351.20 227.76 0.0700
371.16 227.49 0.0700
395.10 228.55 0.0700

TRAVEL TIME TABLE
DEPT# ELEV (m) VOLUME (cu.m.) FLOW RATE (cms) VELOCITY (m/s) TRAV.TIME (min)

6.31 225.54 .215E+07 523.9 0.90 68.38
 6.68 225.91 .257E+07 638.4 0.92 67.05

<---- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0006) 1375.77 3.06 10.50 6.31 0.88 0.35
 OUTFLOW: ID= 1 (6007) 1375.77 2.36 13.00 6.31 0.83 0.33

CALIB |
 STANDHYD (0071) | Area (ha)= 16.47
 ID= 1 DT=15.0 min | Total Imp(%)= 47.00 Dir. Conn.(%)= 17.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 7.74 8.73
 Dep. Storage (mm)= 0.80 2.50
 Average Slope (ft)= 1.00 2.00
 Length (m)= 331.36 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 55.44 28.11
 over (min) 15.00 30.00
 Storage Coeff. (min)= 6.64 (ii) 18.36 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.10 0.05

TOTALS
 PEAK FLOW (cms)= 0.40 0.36 0.626 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 41.20 12.00 16.96
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.98 0.29 0.40

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 62.9 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5007) |
 IN= 2--> OUT= 1 |
 DT= 15.0 min |
 OUTFLOW STORAGE OUTFLOW STORAGE
 (cms) (ha.m.) (cms) (ha.m.)
 0.0000 0.0000 1.1020 0.4341
 0.0560 0.2063 1.3830 0.4946
 0.3190 0.2394 1.6100 0.5538
 0.5940 0.3063 2.0100 0.5638
 0.8240 0.3645 0.0000 0.0000

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (0071) 16.470 0.626 6.00 16.96
 OUTFLOW: ID= 1 (5007) 16.470 0.051 8.25 16.90

PEAK FLOW REDUCTION [Qout/Qin](%)= 8.22
 TIME SHIFT OF PEAK FLOW (min)=135.00
 MAXIMUM STORAGE USED (ha.m.)= 0.1896

CALIB |
 NASHYD (0072) | Area (ha)= 539.30 Curve Number (CN)= 64.2
 ID= 1 DT=15.0 min | Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
 U.H. Tp(hrs)= 2.74

Unit Hyd Qpeak (cms)= 8.477

PEAK FLOW (cms)= 2.154 (i)
 TIME TO PEAK (hrs)= 9.000
 RUNOFF VOLUME (mm)= 8.614
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.205

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8507) |
 1 + 2 = 3 |
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0072): 539.30 2.154 9.00 8.61
 + ID2= 2 (5007): 16.47 0.051 8.25 16.90
 ID = 3 (8507): 555.77 2.205 9.00 8.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8210) |
 1 + 2 = 3 |
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (6007): 1375.77 2.364 13.00 6.31
 + ID2= 2 (8507): 555.77 2.205 9.00 8.86
 ID = 3 (8210): 1931.54 3.658 11.25 7.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8208) |
 1 + 2 = 3 |
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8206): 3183.02 5.616 11.50 9.76
 + ID2= 2 (8210): 1931.54 3.658 11.25 7.04
 ID = 3 (8208): 5114.56 9.271 11.25 8.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8212) |
 1 + 2 = 3 |
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8208): 5114.56 9.271 11.25 8.73
 + ID2= 2 (8214): 875.75 3.589 8.50 8.21
 ID = 3 (8212): 5990.31 11.676 10.00 8.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6010) |
 IN= 2--> OUT= 1 | Routing time step (min)= 15.00

DATA FOR SECTION (10.1) ----->

Distance	Elevation	Manning
0.00	223.78	0.0400
10.97	224.70	0.0400
60.32	222.23	0.0400
104.19	219.90	0.0400
126.13	219.00	0.0400
134.36	218.90	0.0400 / 0.0330
137.10	218.41	0.0330
139.84	218.30	0.0330
142.58	216.76	0.0330
145.32	216.74	0.0330
148.07	216.71	0.0330
150.81	217.11	0.0330
153.55	218.40	0.0330
156.29	219.01	0.0330 / 0.0400
159.03	219.52	0.0400
167.26	219.89	0.0400
170.00	220.09	0.0400
183.71	221.31	0.0400
200.16	223.39	0.0400
271.45	224.04	0.0400

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.36	217.07	.680E+04	0.9	0.36	129.75
0.73	217.44	.165E+05	3.4	0.58	81.12
1.09	217.81	.278E+05	7.3	0.75	63.18
1.46	218.17	.405E+05	12.6	0.88	53.42
1.83	218.54	.561E+05	17.7	0.90	52.70
2.19	218.90	.764E+05	26.2	0.97	48.52
2.64	219.35	.118E+06	44.8	1.07	44.04
3.08	219.79	.180E+06	71.5	1.13	41.83
3.53	220.24	.264E+06	109.3	1.17	40.26
3.97	220.68	.366E+06	159.0	1.23	38.34
4.42	221.13	.485E+06	221.7	1.30	36.43
4.87	221.58	.620E+06	298.8	1.37	34.58
5.31	222.02	.771E+06	391.1	1.44	32.85
5.76	222.47	.937E+06	498.7	1.51	31.30
6.21	222.92	.112E+07	622.7	1.58	29.93
6.65	223.36	.132E+07	764.2	1.65	28.69
7.10	223.81	.155E+07	878.9	1.60	29.46
7.54	224.25	.186E+07	1057.1	1.61	29.35
7.99	224.70	.220E+07	1291.1	1.67	28.35

<---- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8212) 5990.31 11.68 10.00 8.66 1.39 0.86

OUTFLOW: ID= 1 (6010) 5990.31 11.27 11.00 8.66 1.36 0.84

```

CALIB
STANDHYD (0102) | Area (ha)= 211.49
ID= 1 DT=15.0 min | Total Imp(%)= 47.00 Dir. Conn.(%)= 26.00
-----
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 99.40 112.09
Dep. Storage (mm)= 0.80 2.50
Average Slope (%)= 1.00 2.00
Length (m)= 1187.41 40.00
Mannings n = 0.013 0.250
Max. Eff. Inten.(mm/hr)= 55.44 18.16
over (min)= 15.00 30.00
Storage Coeff. (min)= 14.27 (ii) 28.24 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.07 0.04
*TOTALS*
PEAK FLOW (cms)= 6.30 2.40 7.765 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 41.20 8.80 17.22
TOTAL RAINFALL (mm)= 42.00 42.00 42.00
RUNOFF COEFFICIENT = 0.98 0.21 0.41
  
```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 55.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
STANDHYD (0101) | Area (ha)= 133.27
ID= 1 DT=15.0 min | Total Imp(%)= 47.00 Dir. Conn.(%)= 26.00
-----
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 62.64 70.63
Dep. Storage (mm)= 0.80 2.50
Average Slope (%)= 1.00 2.00
Length (m)= 942.59 40.00
Mannings n = 0.013 0.250
Max. Eff. Inten.(mm/hr)= 55.44 18.16
over (min)= 15.00 30.00
Storage Coeff. (min)= 12.43 (ii) 26.39 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04
*TOTALS*
PEAK FLOW (cms)= 4.19 1.57 5.150 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 41.20 8.80 17.22
TOTAL RAINFALL (mm)= 42.00 42.00 42.00
RUNOFF COEFFICIENT = 0.98 0.21 0.41
  
```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 55.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

RESERVOIR (5010)
IN= 2--> OUT= 1
DT= 15.0 min
-----
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
0.0000 0.0000 | 6.8040 3.7679
1.3970 1.4119 | 8.2460 4.4558
3.2730 2.3774 | 10.2760 4.7276
5.2480 3.0364 | 10.6760 4.7376
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (0101) 133.270 5.150 6.00 17.22
OUTFLOW: ID= 1 (5010) 133.270 1.105 7.00 17.22
  
```

```

PEAK FLOW REDUCTION [Qout/Qin](%)= 21.46
TIME SHIFT OF PEAK FLOW (min)= 60.00
MAXIMUM STORAGE USED (ha.m.)= 1.1208
  
```

```

ADD HYD (8216)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
  
```

```

ID= 1 (0102): 211.49 7.765 6.00 17.22
+ ID2= 2 (5010): 133.27 1.105 7.00 17.22
=====
ID = 3 (8216): 344.76 8.316 6.00 17.22
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8218)
1 + 2 = 3
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (6010): 5990.31 11.275 11.00 8.66
+ ID2= 2 (8216): 344.76 8.316 6.00 17.22
=====
ID = 3 (8218): 6335.07 12.096 10.75 9.12
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

** SIMULATION NUMBER: 2 **

```

READ STORM | Filename: C:\Users\jscott\AppData
| | ata\Local\Temp\
| | ad939110-f7bb-4285-9226-bce91a941061\fc72785f
| Ptotal= 54.40 mm | Comments: 5-Year 12-Hour SCS II Design Storm
-----
TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
0.25 1.36 | 3.25 2.18 | 6.25 9.79 | 9.25 1.90
0.50 1.36 | 3.50 2.18 | 6.50 9.79 | 9.50 1.90
0.75 1.36 | 3.75 2.18 | 6.75 4.35 | 9.75 1.90
1.00 1.36 | 4.00 2.18 | 7.00 4.35 | 10.00 1.90
1.25 1.36 | 4.25 3.26 | 7.25 3.26 | 10.25 1.09
1.50 1.36 | 4.50 3.26 | 7.50 3.26 | 10.50 1.09
1.75 1.36 | 4.75 4.35 | 7.75 3.26 | 10.75 1.09
2.00 1.36 | 5.00 4.35 | 8.00 3.26 | 11.00 1.09
2.25 1.63 | 5.25 6.53 | 8.25 1.90 | 11.25 1.09
2.50 1.63 | 5.50 6.53 | 8.50 1.90 | 11.50 1.09
2.75 1.63 | 5.75 26.11 | 8.75 1.90 | 11.75 1.09
3.00 1.63 | 6.00 71.81 | 9.00 1.90 | 12.00 1.09
  
```

```

CALIB
STANDHYD (0081) | Area (ha)= 10.53
ID= 1 DT=15.0 min | Total Imp(%)= 45.00 Dir. Conn.(%)= 15.00
-----
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 4.74 5.79
Dep. Storage (mm)= 0.80 2.50
Average Slope (%)= 1.00 2.00
Length (m)= 264.95 40.00
Mannings n = 0.013 0.250
Max. Eff. Inten.(mm/hr)= 71.81 41.78
over (min)= 15.00 30.00
Storage Coeff. (min)= 5.23 (ii) 15.24 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.11 0.05
*TOTALS*
PEAK FLOW (cms)= 0.30 0.38 0.546 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 53.60 18.00 23.34
TOTAL RAINFALL (mm)= 54.40 54.40 54.40
RUNOFF COEFFICIENT = 0.99 0.33 0.43
  
```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 61.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

RESERVOIR (5008)
IN= 2--> OUT= 1
DT= 15.0 min
-----
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
0.0000 0.0000 | 0.7050 0.2743
0.0360 0.1287 | 0.8850 0.3136
0.2040 0.1489 | 1.0300 0.3521
0.3800 0.1922 | 1.4300 0.3621
0.5270 0.2294 | 0.0000 0.0000
  
```

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (0081) 10.53 0.546 6.00 23.34
 OUTFLOW: ID= 1 (5008) 10.53 0.142 6.75 23.25

PEAK FLOW REDUCTION [Qout/Qin](%)= 26.03
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 0.1422

CALIB (0082) Area (ha)= 402.65 Curve Number (CN)= 63.3
 NASHYD (0082) Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 1.63

Unit Hyd Qpeak (cms)= 10.622

PEAK FLOW (cms)= 3.750 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 13.526
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.249

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8508) 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0082): 402.65 3.750 7.75 13.53
 + ID2= 2 (5008): 10.53 0.142 6.75 23.25
 ID = 3 (8508): 413.18 3.836 7.50 13.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6009) |
 | I#= 2 -> OUT= 1 |
 | DT= 15.0 min |
 Routing time step (min)'= 15.00

----- DATA FOR SECTION (9.1) -----

Distance	Elevation	Manning
0.00	232.41	0.0600
16.80	232.21	0.0600
28.00	231.55	0.0600
117.61	229.36	0.0600
235.21	228.08	0.0600
296.81	226.96	0.0600
316.00	226.17	0.0600
326.00	226.17	0.0600
332.00	226.04	0.0600 / 0.0350 Main Channel
335.50	225.16	0.0350 Main Channel
336.00	225.08	0.0350 Main Channel
337.00	225.08	0.0350 Main Channel
337.90	225.21	0.0350 Main Channel
339.90	225.93	0.0350 / 0.0600 Main Channel
345.00	226.28	0.0600
358.42	226.57	0.0600
403.22	227.73	0.0600
448.02	229.07	0.0600
492.82	230.51	0.0600
554.43	232.28	0.0600

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.28	225.36	.255E+04	0.3	0.37	160.52
0.57	225.65	.719E+04	1.1	0.55	107.34
0.85	225.93	.138E+05	2.7	0.70	85.08
1.25	226.33	.405E+05	7.8	0.69	86.59
1.64	226.72	.109E+06	20.0	0.65	91.15
2.04	227.12	.216E+06	41.2	0.68	87.34
2.44	227.52	.372E+06	74.8	0.72	82.92
2.83	227.91	.580E+06	125.6	0.77	77.03
3.23	228.31	.842E+06	193.5	0.82	72.57
3.63	228.71	.117E+07	284.7	0.87	68.66
4.02	229.10	.157E+07	405.4	0.92	64.71
4.42	229.50	.204E+07	564.5	0.99	60.30
4.82	229.90	.256E+07	766.1	1.07	55.64
5.22	230.30	.311E+07	1001.2	1.15	51.82
5.61	230.69	.371E+07	1269.8	1.22	48.68
6.01	231.09	.435E+07	1573.3	1.29	46.05
6.41	231.49	.503E+07	1914.5	1.36	43.77
6.80	231.88	.575E+07	2314.0	1.44	41.39
7.20	232.28	.650E+07	2741.3	1.50	39.49

----- hydrograph -----> <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8508)	413.18	3.84	7.50	13.77	0.94
OUTFLOW: ID= 1 (6009)	413.18	2.89	8.75	13.77	0.87

CALIB (0091) Area (ha)= 11.32
 STANHYD (0091) Total Imp(%)= 50.00 Dir. Conn.(%)= 20.00
 ID= 1 DT=15.0 min

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 5.66 5.66
 Dep. Storage (mm)= 0.80 2.50
 Average Slope (%)= 1.00 2.00
 Length (m)= 274.71 40.00
 Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 71.81 41.85
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 5.35 (ii) 15.35 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.11 0.05

PEAK FLOW (cms)= 0.43 0.37 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.25 0.671 (iii)
 RUNOFF VOLUME (mm)= 53.60 17.45 24.68
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.99 0.32 0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING:FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.7 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5009) |
 | IN= 2 -> OUT= 1 |
 | DT= 15.0 min |
 OUTFLOW STORAGE OUTFLOW STORAGE
 (cms) (ha.m.) (cms) (ha.m.)
 0.0000 0.0000 0.7570 0.3065
 0.0380 0.1497 0.9510 0.3465
 0.2190 0.1748 1.1070 0.3857
 0.4090 0.2196 1.5070 0.3957
 0.5670 0.2594 0.0000 0.0000

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (0091) 11.32 0.671 6.00 24.68
 OUTFLOW: ID= 1 (5009) 11.32 0.149 6.75 24.59

PEAK FLOW REDUCTION [Qout/Qin](%)= 22.17
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 0.1658

CALIB (0092) Area (ha)= 451.25 Curve Number (CN)= 60.8
 NASHYD (0092) Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
 ID= 1 DT=15.0 min U.H. Tp(hrs)= 2.11

Unit Hyd Qpeak (cms)= 9.199

PEAK FLOW (cms)= 3.196 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 12.490
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.230

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8509) 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0092): 451.25 3.196 8.25 12.49
 + ID2= 2 (5009): 11.32 0.149 6.75 24.59
 ID = 3 (8509): 462.57 3.262 8.25 12.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8214) 1 + 2 = 3
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (6009): 413.18 2.890 8.75 13.77

+ ID2= 2 (8509): 462.57 3.262 8.25 12.79
 ID = 3 (8214): 875.75 6.067 8.50 13.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB		Area (ha)=	2.54
STANDHYD (0011)		Total Imp(%)=	45.00
ID= 1 DT=15.0 min		Dir. Conn.(%)=	15.00
IMPERVIOUS PERVIOUS (i)			
Surface Area (ha)=	1.14		1.40
Dep. Storage (mm)=	0.80		2.50
Average Slope (%)=	1.00		2.00
Length (m)=	130.13		40.00
Manning's n =	0.013		0.250
Max.Eff.Inten.(mm/hr)=	71.01	38.82	
over (min)=	15.00	15.00	
Storage Coeff. (min)=	3.42 (ii)	13.72 (ii)	
Unit Hyd. Tpeak (min)=	15.00	15.00	
Unit Hyd. peak (cms)=	0.11	0.08	
TOTALS			
PEAK FLOW (cms)=	0.08	0.11	0.184 (iii)
TIME TO PEAK (hrs)=	6.00	6.00	6.00
RUNOFF VOLUME (mm)=	53.60	16.78	22.30
TOTAL RAINFALL (mm)=	54.40	54.40	54.40
RUNOFF COEFFICIENT =	0.99	0.31	0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB		Area (ha)=	487.70
STANDHYD (0019)		Total Imp(%)=	45.00
ID= 1 DT=15.0 min		Dir. Conn.(%)=	35.00
IMPERVIOUS PERVIOUS (i)			
Surface Area (ha)=	219.46		268.24
Dep. Storage (mm)=	0.80		5.00
Average Slope (%)=	1.00		2.00
Length (m)=	1803.15		40.00
Manning's n =	0.013		0.250
Max.Eff.Inten.(mm/hr)=	71.81	34.11	
over (min)=	15.00	30.00	
Storage Coeff. (min)=	16.54 (ii)	27.39 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.07	0.04	
TOTALS			
PEAK FLOW (cms)=	23.76	10.95	30.499 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	53.60	18.82	30.99
TOTAL RAINFALL (mm)=	54.40	54.40	54.40
RUNOFF COEFFICIENT =	0.99	0.35	0.57

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (8511)		AREA	QPEAK	TPEAK	R.V.
IN= 2--> OUT= 1		(ha)	(cms)	(hrs)	(mm)
DT= 15.0 min					
OUTFLOW (cms)		STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)	
0.0000	0.0000		24.3900	10.5911	
1.6600	5.9307		32.6300	12.6704	
9.4600	6.8569		40.9700	14.4958	
17.6100	8.8657		47.7000	16.2820	
INFLow : ID= 2 (0019) 487.700 30.499 6.00 30.99					
OUTFLOW: ID= 1 (8511) 487.700 11.878 6.75 30.99					

PEAK FLOW REDUCTION [Qout/Qin](%)= 38.94
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 7.5114

ADD HYD (8510)		AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)
ID= 1 (0011):		2.54	0.184	6.00	22.30
+ ID2= 2 (8511):		487.70	11.878	6.75	30.99
ID = 3 (8510):		490.24	11.902	6.75	30.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5001)		OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1		(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min					
		0.0000	0.0000	4.6830	11.8298
		0.0540	0.1960	5.4380	13.3066
		1.2810	6.8008	6.1990	14.9402
		2.5090	8.6037	6.5990	14.9502
		3.4630	9.9865	0.0000	0.0000
INFLow : ID= 2 (8510) 490.240 11.902 6.75 30.94					
OUTFLOW: ID= 1 (5001) 490.240 1.441 13.25 30.94					
PEAK FLOW REDUCTION [Qout/Qin](%)= 12.11					
TIME SHIFT OF PEAK FLOW (min)=390.00					
MAXIMUM STORAGE USED (ha.m.)= 7.0359					

CALIB		Area (ha)=	630.33	Curve Number (CN)=	61.0
NASHYD (0012)		Ia (mm)=	2.50	# of Linear Res.(N)=	3.50
ID= 1 DT=15.0 min		U.H. Tp(hrs)=	2.24		

Unit Hyd Qpeak (cms)= 12.094

PEAK FLOW (cms)= 4.285 (i)
 TIME TO PEAK (hrs)= 8.500
 RUNOFF VOLUME (mm)= 12.570
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.231

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8501)		AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)
ID= 1 (0012):		630.33	4.285	8.50	12.57
+ ID2= 2 (5001):		490.24	1.441	13.25	30.94
ID = 3 (8501):		1120.57	5.449	8.50	20.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CN (6002)		ROUTING TIME STEP (min)=	15.00
IN= 2--> OUT= 1			
<----- DATA FOR SECTION (2.1) ----->			
Distance	Elevation	Manning	
0.00	235.70	0.0700	
15.10	235.42	0.0700	
30.21	235.01	0.0700	
37.76	234.49	0.0700	
45.31	231.43	0.0700	
56.64	231.55	0.0700	
98.18	229.87	0.0700	
117.06	229.64	0.0700	
124.61	229.03	0.0700 / 0.0300	Main Channel
128.38	228.69	0.0300	Main Channel
132.16	228.58	0.0300	Main Channel
143.49	228.98	0.0300 / 0.0700	Main Channel
158.59	229.36	0.0700	
181.25	229.58	0.0700	
200.13	229.96	0.0700	
237.89	231.19	0.0700	
268.09	232.60	0.0700	
302.08	233.40	0.0700	
332.29	234.25	0.0700	
373.82	235.68	0.0700	

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.20	228.78	.639E+04	0.3	0.26	359.77
0.40	228.98	.222E+05	1.6	0.40	227.92

0.79	229.37	.842E+05	9.7	0.63	145.25
1.19	229.77	.218E+06	25.5	0.65	142.16
1.58	230.16	.441E+06	53.8	0.67	136.74
1.98	230.56	.715E+06	94.9	0.73	125.69
2.37	231.95	.104E+07	148.9	0.79	115.12
2.76	231.34	.141E+07	217.2	0.85	107.88
3.16	231.74	.183E+07	299.4	0.90	101.81
3.55	232.13	.228E+07	403.8	0.97	94.28
3.95	232.53	.276E+07	523.3	1.04	87.91
4.34	232.92	.326E+07	650.4	1.10	83.62
4.74	233.32	.380E+07	792.5	1.15	79.99
5.13	233.71	.438E+07	954.7	1.20	76.48
5.52	234.10	.499E+07	1134.7	1.25	73.30
5.92	234.50	.563E+07	1334.6	1.31	70.34
6.31	234.89	.631E+07	1540.4	1.35	68.23
6.71	235.29	.702E+07	1746.8	1.37	67.00
7.10	235.68	.780E+07	1958.9	1.38	66.34

3.78	227.41	.411E+06	103.4	0.59	66.15
4.10	227.73	.516E+06	132.2	0.70	65.13
4.41	228.04	.642E+06	168.2	0.72	63.59
4.73	228.36	.788E+06	210.4	0.73	62.46
5.05	228.68	.959E+06	262.8	0.75	60.81
5.36	228.99	1.15E+07	328.9	0.79	58.23
5.68	229.31	1.36E+07	401.2	0.81	56.49

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLW : ID= 2 (8501) 1120.57 5.45 8.50 20.61 0.59 0.49
OUTFLOW: ID= 1 (6002) 1120.57 3.38 11.25 20.61 0.48 0.44

```

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLW : ID= 2 (8200) 1990.34 7.22 10.25 16.66 1.50 0.55
OUTFLOW: ID= 1 (6003) 1990.34 6.49 11.75 16.66 1.44 0.55

```

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLW : ID= 2 (8501) 1120.57 5.45 8.50 20.61 0.59 0.49
OUTFLOW: ID= 1 (6002) 1120.57 3.38 11.25 20.61 0.48 0.44

```

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLW : ID= 2 (8200) 1990.34 7.22 10.25 16.66 1.50 0.55
OUTFLOW: ID= 1 (6003) 1990.34 6.49 11.75 16.66 1.44 0.55

```

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLW : ID= 2 (8501) 1120.57 5.45 8.50 20.61 0.59 0.49
OUTFLOW: ID= 1 (6002) 1120.57 3.38 11.25 20.61 0.48 0.44

```

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLW : ID= 2 (8200) 1990.34 7.22 10.25 16.66 1.50 0.55
OUTFLOW: ID= 1 (6003) 1990.34 6.49 11.75 16.66 1.44 0.55

```

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLW : ID= 2 (8501) 1120.57 5.45 8.50 20.61 0.59 0.49
OUTFLOW: ID= 1 (6002) 1120.57 3.38 11.25 20.61 0.48 0.44

```

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLW : ID= 2 (8200) 1990.34 7.22 10.25 16.66 1.50 0.55
OUTFLOW: ID= 1 (6003) 1990.34 6.49 11.75 16.66 1.44 0.55

```

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLW : ID= 2 (8501) 1120.57 5.45 8.50 20.61 0.59 0.49
OUTFLOW: ID= 1 (6002) 1120.57 3.38 11.25 20.61 0.48 0.44

```

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLW : ID= 2 (8200) 1990.34 7.22 10.25 16.66 1.50 0.55
OUTFLOW: ID= 1 (6003) 1990.34 6.49 11.75 16.66 1.44 0.55

```

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLW : ID= 2 (8501) 1120.57 5.45 8.50 20.61 0.59 0.49
OUTFLOW: ID= 1 (6002) 1120.57 3.38 11.25 20.61 0.48 0.44

```

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLW : ID= 2 (8200) 1990.34 7.22 10.25 16.66 1.50 0.55
OUTFLOW: ID= 1 (6003) 1990.34 6.49 11.75 16.66 1.44 0.55

```

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLW : ID= 2 (8501) 1120.57 5.45 8.50 20.61 0.59 0.49
OUTFLOW: ID= 1 (6002) 1120.57 3.38 11.25 20.61 0.48 0.44

```

```

----- hydrograph ----- <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLW : ID= 2 (8200) 1990.34 7.22 10.25 16.66 1.50 0.55
OUTFLOW: ID= 1 (6003) 1990.34 6.49 11.75 16.66 1.44 0.55

```

ID = 3 (8503): 467.85 2.720 8.50 11.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8202)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (6003):	1990.34	6.492	11.75	16.66
+ ID2= 2 (8503):	467.85	2.720	8.50	11.63
ID = 3 (8202):	2458.19	8.039	11.00	15.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area (ha)	Imp(%)	Dir. Conn.(%)
STANDHYD (0041)	8.66	52.00	22.00
ID= 1 DT=15.0 min			

	IMPERVIOUS (ha)	PERVIOUS (i)	
Surface Area	4.50	4.16	
Dep. Storage	0.80	2.50	
Average Slope	1.00	2.00	
Length	240.28	40.00	
Manning's n	0.013	0.250	
Max. Eff. Inten. (mm/hr)	71.81	33.07	
over (min)	15.00	30.00	
Storage Coeff. (min)	4.94 (ii)	15.92 (ii)	
Unit Hyd. Tpeak (min)	15.00	30.00	
Unit Hyd. peak (cms)	0.11	0.05	
TOTALS			
PEAK FLOW (cms)	0.37	0.21	0.502 (iii)
TIME TO PEAK (hrs)	6.00	6.25	6.00
RUNOFF VOLUME (mm)	53.60	13.72	22.49
TOTAL RAINFALL (mm)	54.40	54.40	54.40
RUNOFF COEFFICIENT	0.99	0.25	0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 50.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5004)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
ID= 2 -> OUT= 1				
DT= 15.0 min				
	0.0000	0.0000	0.5790	0.2368
	0.0290	0.1168	0.7270	0.2669
	0.1680	0.1367	0.8470	0.2965
	0.3120	0.1706	1.2470	0.3065
	0.4330	0.2010	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0041)	8.660	0.502	6.00	22.49
OUTFLOW: ID= 1 (5004)	8.660	0.075	7.00	22.38

PEAK FLOW REDUCTION [Qout/Qin](%) = 14.86
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 0.1233

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0042)	376.06	53.3
ID= 1 DT=15.0 min		
U.H. Tp(hrs)=	1.62	# of Linear Res.(N)= 3.50

Unit Hyd Qpeak (cms) = 9.964

PEAK FLOW (cms)	2.524 (i)
TIME TO PEAK (hrs)	7.750
RUNOFF VOLUME (mm)	9.815
TOTAL RAINFALL (mm)	54.400
RUNOFF COEFFICIENT	0.180

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8504)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0042):	376.06	2.524	7.75	9.82

+ ID2= 2 (5004): 8.66 0.075 7.00 22.38

ID = 3 (8504): 384.72 2.580 7.50 10.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8204)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8202):	2458.19	8.039	11.00	15.70
+ ID2= 2 (8504):	384.72	2.580	7.50	10.10
ID = 3 (8204):	2842.91	8.977	10.50	14.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6005)	Routing time step (min)
IN= 2 -> OUT= 1	15.00

Distance	Elevation	Manning
0.00	225.02	0.0400
20.58	224.79	0.0400
35.54	223.93	0.0400
48.64	223.09	0.0400
59.86	222.15	0.0400
67.35	221.11	0.0400
76.41	219.63	0.0400 / 0.0350
82.81	218.53	0.0350
85.41	218.50	0.0350
86.78	218.50	0.0350
88.15	218.50	0.0350
93.45	218.71	0.0350
103.45	218.94	0.0350 / 0.0400
109.45	219.25	0.0400
114.11	220.15	0.0400
123.47	221.73	0.0400
136.56	223.41	0.0400
153.40	224.15	0.0400
170.24	224.55	0.0400
185.20	224.54	0.0400

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.22	218.72	640E+04	0.5	0.28	204.26
0.44	218.94	198E+05	2.2	0.39	147.20
0.77	219.27	507E+05	9.3	0.63	91.02
1.10	219.60	880E+05	20.9	0.82	70.26
1.43	219.93	130E+06	37.7	1.00	57.26
1.76	220.26	175E+06	59.1	1.16	49.43
2.09	220.59	226E+06	85.1	1.30	44.19
2.42	220.92	280E+06	115.8	1.43	40.35
2.75	221.25	340E+06	151.3	1.54	37.40
3.08	221.58	404E+06	191.9	1.64	35.06
3.40	221.90	473E+06	237.5	1.73	33.19
3.73	222.23	548E+06	288.1	1.82	31.68
4.06	222.56	629E+06	344.1	1.89	30.48
4.39	222.89	718E+06	406.8	1.96	29.43
4.72	223.22	815E+06	476.1	2.02	28.52
5.05	223.55	920E+06	547.9	2.06	28.00
5.38	223.88	104E+07	624.5	2.07	27.74
5.71	224.21	117E+07	710.0	2.09	27.54
6.04	224.54	133E+07	800.8	2.08	27.62

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8204)	2842.91	8.98	10.50	14.94	0.76
OUTFLOW: ID= 1 (6005)	2842.91	8.40	11.50	14.94	0.73

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0005)	340.11	69.5
ID= 1 DT=15.0 min		
U.H. Tp(hrs)=	2.45	# of Linear Res.(N)= 3.50

Unit Hyd Qpeak (cms) = 5.972

PEAK FLOW (cms)	2.859 (i)
TIME TO PEAK (hrs)	8.500
RUNOFF VOLUME (mm)	16.488
TOTAL RAINFALL (mm)	54.400
RUNOFF COEFFICIENT	0.303

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8206)
1 + 2 = 3
AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 1 (0005): 340.11 2.859 8.50 16.49
+ ID2= 2 (6005): 2842.91 8.397 11.50 14.94
=====
ID = 3 (8206): 3183.02 10.125 11.00 15.11

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB (8006)
Area (ha)=1375.77 Curve Number (CN)= 55.0
ID= 1 DT=15.0 min Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
U.H. Tp(hrs)= 3.90

```

Unit Hyd Qpeak (cms)= 15.192

```

PEAK FLOW (cms)= 5.052 (i)
TIME TO PEAK (hrs)= 10.500
RUNOFF VOLUME (mm)= 10.371
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.191

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ROUTE CHN (6007)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

```

DATA FOR SECTION (7.1) ----->

Distance	Elevation	Manning
0.00	225.91	0.0700
51.88	225.32	0.0700
135.69	224.23	0.0700
183.58	223.08	0.0700
211.52	223.28	0.0700
223.49	222.51	0.0700
231.43	220.69	0.0700
244.53	219.78	0.0700 /0.0500 Main Channel
249.23	219.60	0.0500 Main Channel
249.93	219.46	0.0500 Main Channel
251.43	219.23	0.0500 Main Channel
253.50	219.70	0.0500 /0.0700 Main Channel
261.10	219.90	0.0700
271.38	221.01	0.0700
283.36	223.77	0.0700
295.33	224.21	0.0700
331.25	226.04	0.0700
351.20	227.76	0.0700
371.16	227.49	0.0700
395.10	228.55	0.0700

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.23	219.46	112E+04	0.0	0.16	375.56
0.47	219.70	471E+04	0.3	0.22	277.80
0.84	220.07	267E+05	2.4	0.34	184.45
1.20	220.43	626E+05	7.4	0.44	140.97
1.57	220.80	110E+06	15.6	0.52	117.94
1.93	221.16	165E+06	27.4	0.61	100.71
2.30	221.53	225E+06	42.6	0.70	88.16
2.66	221.89	290E+06	60.9	0.78	79.20
3.03	222.26	358E+06	82.4	0.85	72.44
3.39	222.62	431E+06	106.0	0.91	67.79
3.76	222.99	513E+06	131.5	0.95	65.01
4.12	223.35	628E+06	149.2	0.88	70.14
4.49	223.72	788E+06	191.4	0.90	68.58
4.85	224.08	974E+06	232.1	0.88	69.94
5.22	224.45	120E+07	283.2	0.88	70.45
5.58	224.81	147E+07	347.5	0.88	70.32
5.95	225.18	178E+07	427.9	0.89	69.46
6.31	225.54	215E+07	523.9	0.90	68.38
6.68	225.91	257E+07	638.4	0.92	67.05

hydrograph -----> <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0006) 1375.77	5.05	10.50	10.37	1.03	0.38
OUTFLOW : ID= 1 (6007) 1375.77	3.93	12.75	10.37	0.94	0.36

```

CALIB (8071)
STANDHYD (8071)
ID= 1 DT=15.0 min
Area (ha)= 16.47
Total Imp(%)= 47.00 Dir. Conn.(%)= 17.00

```

Surface Area (ha)= IMPERVIOUS 7.74 PERVIOUS (i) 8.73

```

Dep. Storage (mm)= 0.80 2.50
Average Slope (%)= 1.00 2.00
Length (m)= 331.36 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 71.81 44.26
over (min)= 15.00 30.00
Storage Coeff. (min)= 5.98 (ii) 15.76 (iii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.10 0.05

```

```

*TOTALS*
PEAK FLOW (cms)= 0.53 0.60 0.913 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 53.60 18.78 24.70
TOTAL RAINFALL (mm)= 54.40 54.40 54.40
RUNOFF COEFFICIENT = 0.99 0.35 0.45

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 62.9 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

RESERVOIR (5007)
IN= 2--> OUT= 1
DT= 15.0 min
OUTFLOW (cms) STORAGE (ha.m.) OUTFLOW (cms) STORAGE (ha.m.)
0.0000 0.0000 1.1020 0.4341
0.0560 0.2063 1.3830 0.4946
0.3190 0.2394 1.6100 0.5538
0.5940 0.3063 2.0100 0.5638
0.8240 0.3645 0.0000 0.0000

```

```

AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
INFLOW : ID= 2 (0071) 16.470 0.913 6.00 24.70
OUTFLOW : ID= 1 (5007) 16.470 0.251 6.75 24.64

```

PEAK FLOW REDUCTION [Qout/qin](%)= 27.53
TIME SHIFT OF PEAK FLOW (min)= 45.00
MAXIMUM STORAGE USED (ha.m.)= 0.2313

```

CALIB (0072)
NASHYD (0072)
ID= 1 DT=15.0 min
Area (ha)= 539.30 Curve Number (CN)= 64.2
Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
U.H. Tp(hrs)= 2.74

```

Unit Hyd Qpeak (cms)= 8.477

```

PEAK FLOW (cms)= 3.501 (i)
TIME TO PEAK (hrs)= 9.000
RUNOFF VOLUME (mm)= 13.918
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.256

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8507)
1 + 2 = 3
AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 1 (0072): 539.30 3.501 9.00 13.92
+ ID2= 2 (5007): 16.47 0.251 6.75 24.64
=====
ID = 3 (8507): 555.77 3.565 9.00 14.24

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8210)
1 + 2 = 3
AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 1 (6007): 1375.77 3.930 12.75 10.37
+ ID2= 2 (8507): 555.77 3.565 9.00 14.24
=====
ID = 3 (8210): 1931.54 6.172 11.00 11.48

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8208)

```

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8206):	3183.02	10.125	11.00	15.11
+ ID2= 2 (8210):	1931.54	6.172	11.00	11.48
=====				
ID = 3 (8208):	5114.56	16.297	11.00	13.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8212)	1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8208):	5114.56	16.297	11.00	13.74	
+ ID2= 2 (8214):	875.75	6.067	8.50	13.35	
=====					
ID = 3 (8212):	5990.31	20.335	10.00	13.67	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6010) | Routing time step (min)'= 15.00
IN= 2--> OUT= 1 |

DATA FOR SECTION (10.1) ----->					
Distance	Elevation	Manning			
0.00	223.78	0.0400			
10.97	224.70	0.0400			
60.32	222.23	0.0400			
104.19	219.90	0.0400			
126.13	219.00	0.0400			
134.36	218.90	0.0400 / 0.0330	Main Channel		
137.10	218.41	0.0330	Main Channel		
139.84	218.30	0.0330	Main Channel		
142.58	216.76	0.0330	Main Channel		
145.32	216.74	0.0330	Main Channel		
148.07	216.71	0.0330	Main Channel		
150.81	217.11	0.0330	Main Channel		
153.55	218.40	0.0330	Main Channel		
156.29	219.01	0.0330 / 0.0400	Main Channel		
159.03	219.52	0.0400			
167.26	219.89	0.0400			
170.00	220.09	0.0400			
183.71	221.31	0.0400			
200.16	223.39	0.0400			
271.45	224.04	0.0400			

<----- TRAVEL TIME TABLE ----->						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.36	217.07	680E+04	0.9	0.36	129.75	
0.73	217.44	165E+05	3.4	0.58	81.12	
1.09	217.81	278E+05	7.3	0.75	63.18	
1.46	218.17	405E+05	12.6	0.88	53.42	
1.83	218.54	561E+05	17.7	0.90	52.70	
2.19	218.90	764E+05	26.2	0.97	48.52	
2.64	219.35	118E+06	44.8	1.07	44.04	
3.08	219.79	180E+06	71.5	1.13	41.83	
3.53	220.24	264E+06	109.3	1.17	40.26	
3.97	220.68	366E+06	159.0	1.23	38.34	
4.42	221.13	485E+06	221.7	1.30	36.43	
4.87	221.58	620E+06	298.8	1.37	34.58	
5.31	222.02	771E+06	391.1	1.44	32.85	
5.76	222.47	937E+06	498.7	1.51	31.30	
6.21	222.92	112E+07	622.7	1.58	29.93	
6.65	223.36	132E+07	764.2	1.65	28.69	
7.10	223.81	155E+07	878.9	1.60	29.46	
7.54	224.25	186E+07	1057.1	1.61	29.35	
7.99	224.70	220E+07	1291.1	1.67	28.35	

<---- hydrograph ----> <-pipe / channel-->						
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8212)	5990.31	20.33	10.00	13.67	1.84	0.92
OUTFLOW : ID= 1 (6010)	5990.31	19.53	11.00	13.67	1.90	0.91

CALIB	STANDHYD (0101)	Area (ha)=	211.49
ID= 1 DT=15.0 min	Total Imp(%)=	47.00	Dir. Conn.(%)= 26.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	99.40	112.09
Dep. Storage (mm)=	0.80	2.50
Average Slope (%)=	1.00	2.00
Length (m)=	1187.41	40.00
Mannings n =	0.013	0.250
Max.Eff.Inten.(mm/hr)=	71.81	29.30
over (min)	15.00	30.00

Storage Coeff. (min)=	12.87 (ii)	24.40 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.08	0.04	
=====			
PEAK FLOW (cms)=	8.50	4.19	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	11.103 (iii)
RUNOFF VOLUME (mm)=	53.60	14.11	24.38
TOTAL RAINFALL (mm)=	54.40	54.40	54.40
RUNOFF COEFFICIENT =	0.99	0.26	0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CN* = 55.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	STANDHYD (0101)	Area (ha)=	133.27
ID= 1 DT=15.0 min	Total Imp(%)=	47.00	Dir. Conn.(%)= 26.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	62.64	70.63	
Dep. Storage (mm)=	0.80	2.50	
Average Slope (%)=	1.00	2.00	
Length (m)=	942.59	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	71.81	29.30	
over (min)	15.00	30.00	
Storage Coeff. (min)=	11.21 (ii)	22.74 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.08	0.04	
=====			
PEAK FLOW (cms)=	5.63	2.73	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	7.331 (iii)
RUNOFF VOLUME (mm)=	53.60	14.11	24.38
TOTAL RAINFALL (mm)=	54.40	54.40	54.40
RUNOFF COEFFICIENT =	0.99	0.26	0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CN* = 55.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5010)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
IN= 2--> OUT= 1				
DT= 15.0 min				
	0.0000	0.0000	6.8040	3.7679
	1.3970	1.4119	8.2460	4.4558
	3.2730	2.3774	10.2760	4.7276
	5.2480	3.0364	10.6760	4.7376

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0101)	133.270	7.331	6.00	24.38
OUTFLOW : ID= 1 (5010)	133.270	1.704	7.00	24.38
=====				
PEAK FLOW REDUCTION [Qout/Qin](%)=	23.25			
TIME SHIFT OF PEAK FLOW (min)=	60.00			
MAXIMUM STORAGE USED (ha.m.)=	1.5829			

ADD HYD (8216)	1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0102):	211.49	11.103	6.00	24.38	
+ ID2= 2 (5010):	133.27	1.704	7.00	24.38	
=====					
ID = 3 (8216):	344.76	11.876	6.00	24.38	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8218)	1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6010):	5990.31	19.531	11.00	13.67	
+ ID2= 2 (8216):	344.76	11.876	6.00	24.38	
=====					
ID = 3 (8218):	6335.07	20.660	10.75	14.25	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ** SIMULATION NUMBER: 3 **

READ STORM
 Filename: C:\Users\jscott\AppData\Local\Temp\ad939110-17bb-4285-9226-bce91a941061\azf4f2dd
 Comments: 10-Year 12-Hour SCS II Design Storm

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.57	3.25	2.51	6.25	11.29	9.25	2.19
0.50	1.57	3.50	2.51	6.50	11.29	9.50	2.19
0.75	1.57	3.75	2.51	6.75	5.02	9.75	2.19
1.00	1.57	4.00	2.51	7.00	5.02	10.00	2.19
1.25	1.57	4.25	3.76	7.25	3.76	10.25	1.25
1.50	1.57	4.50	3.76	7.50	3.76	10.50	1.25
1.75	1.57	4.75	5.02	7.75	3.76	10.75	1.25
2.00	1.57	5.00	5.02	8.00	3.76	11.00	1.25
2.25	1.88	5.25	7.52	8.25	2.19	11.25	1.25
2.50	1.88	5.50	7.52	8.50	2.19	11.50	1.25
2.75	1.88	5.75	30.10	8.75	2.19	11.75	1.25
3.00	1.88	6.00	82.76	9.00	2.19	12.00	1.25

CALIB
 STANDHYD (0081) Area (ha)= 10.53
 ID= 1 DT=15.0 min Total Imp(%)= 45.00 Dir. Conn.(%)= 15.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	4.74	5.79
Dep. Storage (mm)	0.80	2.50
Average Slope (%)	1.00	2.00
Length (m)	264.95	40.00
Mannings n	0.013	0.250

Max.Eff.Inten.(mm/hr)= 82.76 53.26
 over (min) 15.00 15.00
 Storage Coeff. (min)= 4.94 (ii) 14.02 (iii)
 Unit Hyd. Tpeak (min)= 15.00 15.00
 Unit Hyd. peak (cms)= 0.11 0.07

 PEAK FLOW (cms)= 0.35 0.61 0.965 (iii)
 TIME TO PEAK (hrs)= 6.00 6.00 6.00
 RUNOFF VOLUME (mm)= 61.90 22.88 28.73
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.99 0.36 0.46

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 61.7 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5008)
 ID= 2 --> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.7050	0.2743
0.0360	0.1287	0.8850	0.3136
0.2040	0.1489	1.0300	0.3521
0.3800	0.1922	1.4300	0.3621
0.5270	0.2294	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
INFLOW: ID= 2 (0081)	10.530	0.965	6.00	28.73
OUTFLOW: ID= 1 (5008)	10.530	0.235	6.50	28.64

PEAK FLOW REDUCTION [Qout/qin](%)= 24.33
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 0.1573

CALIB
 NASHVD (0082) Area (ha)= 402.65 Curve Number (CN)= 63.3
 ID= 1 DT=15.0 min Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
 U.H. Tp(hrs)= 1.63

Unit Hyd Qpeak (cms)= 10.622

PEAK FLOW (cms)= 4.865 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 17.470
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.279

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8508)
 1 + 2 + 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 (0082):	402.65	4.865	7.50	17.47
+ ID= 2 (5008):	10.53	0.235	6.50	28.64
ID = 3 (8508):	413.18	4.967	7.50	17.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6009)
 IN= 2 --> OUT= 1 Routing time step (min)= 15.00

DATA FOR SECTION (9.1) ----->

Distance	Elevation	Manning
0.00	232.41	0.0600
16.80	232.21	0.0600
28.00	231.55	0.0600
117.61	229.36	0.0600
235.21	228.08	0.0600
296.81	226.96	0.0600
316.00	226.17	0.0600
326.00	226.17	0.0600
332.00	226.04	0.0600 / 0.0350
335.50	225.16	0.0350
335.00	225.08	0.0350
337.00	225.08	0.0350
337.90	225.21	0.0350
339.90	225.93	0.0350 / 0.0600
345.00	226.28	0.0600
358.42	226.57	0.0600
403.22	227.73	0.0600
448.02	229.07	0.0600
492.82	230.51	0.0600
554.43	232.28	0.0600

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.28	225.36	.255E+04	0.3	0.37	160.52
0.57	225.65	.719E+04	1.1	0.55	107.34
0.85	225.93	.138E+05	2.7	0.70	85.08
1.25	226.33	.405E+05	7.8	0.69	86.59
1.64	226.72	.109E+06	20.0	0.65	91.15
2.04	227.12	.215E+06	41.2	0.68	87.34
2.44	227.52	.372E+06	74.8	0.72	82.92
2.83	227.91	.580E+06	125.6	0.77	77.03
3.23	228.31	.842E+06	193.5	0.82	72.57
3.63	228.71	.117E+07	284.7	0.87	68.66
4.02	229.10	.157E+07	405.4	0.92	64.71
4.42	229.50	.204E+07	564.5	0.99	60.30
4.82	229.90	.255E+07	765.1	1.07	55.64
5.22	230.30	.311E+07	1001.2	1.15	51.82
5.61	230.69	.371E+07	1269.8	1.22	48.68
6.01	231.09	.435E+07	1573.3	1.29	46.05
6.41	231.49	.503E+07	1914.5	1.36	43.77
6.80	231.88	.575E+07	2314.0	1.44	41.39
7.20	232.28	.650E+07	2741.3	1.50	39.49

hydrograph -----> <- pipe / channel ->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW: ID= 2 (8508)	413.18	4.97	7.50	17.75	1.03	0.69
OUTFLOW: ID= 1 (6009)	413.18	3.70	8.75	17.75	0.93	0.70

CALIB
 STANDHYD (0091) Area (ha)= 11.32
 ID= 1 DT=15.0 min Total Imp(%)= 50.00 Dir. Conn.(%)= 20.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	5.66	5.66
Dep. Storage (mm)	0.80	2.50
Average Slope (%)	1.00	2.00
Length (m)	274.71	40.00
Mannings n	0.013	0.250

Max.Eff.Inten.(mm/hr)= 82.76 53.44
 over (min) 15.00 15.00
 Storage Coeff. (min)= 5.05 (ii) 14.12 (ii)

Unit Hyd. Tpeak (min)= 15.00 15.00
 Unit Hyd. peak (cms)= 0.11 0.07

TOTALS
 PEAK FLOW (cms)= 0.50 0.60 1.102 (iii)
 TIME TO PEAK (hrs)= 6.00 6.00 6.00
 RUNOFF VOLUME (mm)= 61.90 22.21 30.15
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.99 0.35 0.48

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR (5009)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.7570	0.3065
0.0380	0.1497	0.9510	0.3465
0.2190	0.1748	1.1070	0.3957
0.4090	0.2196	1.5070	0.3957
0.5670	0.2594	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
11.320	1.102	6.00	30.15
11.320	0.253	6.50	30.06

INFLOW : ID= 2 (0091) 11.320 1.102 6.00 30.15
 OUTFLOW: ID= 1 (5009) 11.320 0.253 6.50 30.06

PEAK FLOW REDUCTION [Qout/Qin](%)= 22.90
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 0.1835

 CALIB
 NASHYD (0092) Area (ha)= 451.25 Curve Number (CN)= 60.8
 ID= 1 DT=15.0 min Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
 U.H. Tp(hrs)= 2.11

Unit Hyd Qpeak (cms)= 9.199

PEAK FLOW (cms)= 4.155 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 16.182
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.258

- (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8509)
 1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
451.25	4.155	8.25	16.18
11.32	0.253	6.50	30.06
462.57	4.232	8.25	16.52

ID1= 1 (0092): 451.25 4.155 8.25 16.18
 + ID2= 2 (5009): 11.32 0.253 6.50 30.06
 ID = 3 (8509): 462.57 4.232 8.25 16.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8214)
 1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
413.18	3.704	8.75	17.95
462.57	4.232	8.25	16.52
875.75	7.821	8.50	17.10

ID1= 1 (6009): 413.18 3.704 8.75 17.95
 + ID2= 2 (8509): 462.57 4.232 8.25 16.52
 ID = 3 (8214): 875.75 7.821 8.50 17.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB
 STANDBYD (0011) Area (ha)= 2.54 Dir. Conn.(%)= 15.00
 ID= 1 DT=15.0 min Total Imp(%)= 45.00

Surface Area (ha)= 1.14 1.40
 Dep. Storage (mm)= 0.80 2.50
 Average Slope (%)= 1.00 2.00
 Length (m)= 130.13 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten.(mm/hr)= 82.76 49.67
 over (min)= 15.00 15.00
 Storage Coeff. (min)= 3.23 (ii) 12.56 (ii)
 Unit Hyd. Tpeak (min)= 15.00 15.00
 Unit Hyd. peak (cms)= 0.11 0.08

TOTALS
 PEAK FLOW (cms)= 0.09 0.14 0.232 (iii)
 TIME TO PEAK (hrs)= 6.00 6.00 6.00
 RUNOFF VOLUME (mm)= 61.90 21.40 27.47
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.99 0.34 0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB
 STANDBYD (0019) Area (ha)= 487.70 Dir. Conn.(%)= 35.00
 ID= 1 DT=15.0 min Total Imp(%)= 45.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 219.46 268.24
 Dep. Storage (mm)= 0.80 5.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 1803.15 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten.(mm/hr)= 82.76 43.75
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 15.63 (ii) 25.45 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.07 0.04

TOTALS
 PEAK FLOW (cms)= 28.08 14.65 37.239 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 61.90 24.07 37.31
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.99 0.38 0.60

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR (8511)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	24.3900	10.5811
1.6600	5.9307	32.6300	12.6704
9.4600	6.8569	40.9700	14.4958
17.6100	8.8657	47.7000	16.2820

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
487.700	37.239	6.00	37.31
487.700	15.418	6.75	37.31

INFLOW : ID= 2 (0019) 487.700 37.239 6.00 37.31
 OUTFLOW: ID= 1 (8511) 487.700 15.418 6.75 37.31

PEAK FLOW REDUCTION [Qout/Qin](%)= 41.40
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 8.4508

 ADD HYD (8510)
 1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
2.54	0.232	6.00	27.47
487.70	15.418	6.75	37.31
490.24	15.448	6.75	37.26

ID1= 1 (0011): 2.54 0.232 6.00 27.47
 + ID2= 2 (8511): 487.70 15.418 6.75 37.31
 ID = 3 (8510): 490.24 15.448 6.75 37.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 RESERVOIR (5001)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.6830	11.8298

0.0540	0.1960	5.4380	13.3066
1.2810	6.8008	6.1990	14.9402
2.5090	8.6037	6.5990	14.9502
3.4630	9.9865	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8510)	490.240	15.448	6.75	37.26
OUTFLOW: ID= 1 (5001)	490.240	2.458	9.50	37.25

PEAK FLOW REDUCTION [Qout/Qin](%) = 15.91
 TIME SHIFT OF PEAK FLOW (min)=165.00
 MAXIMUM STORAGE USED (ha.m.) = 8.5289

CALIB	Area	(ha)	QPEAK	TPEAK	R.V.
NASHYD (0012)	630.33	5.567	8.50	16.28	
ID= 1 DT=15.0 min	1a	(mm)	2.50	# of Linear Res. (N)	3.50
	U.H. Tp(hrs)		2.24		

Unit Hyd Qpeak (cms) = 12.094

PEAK FLOW (cms) = 5.567 (i)
 TIME TO PEAK (hrs) = 8.500
 RUNOFF VOLUME (mm) = 16.281
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.260

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8501)	Area	(ha)	QPEAK	TPEAK	R.V.
1 + 2 = 3	630.33	5.567	8.50	16.28	
ID= 1 (0012):	630.33	5.567	8.50	16.28	
+ ID= 2 (5001):	490.24	2.458	9.50	37.25	
ID = 3 (8501):	1120.57	7.897	8.50	25.46	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6002)	Routing time step (min)'
IN= 2--> OUT= 1	15.00

Distance	Elevation	Manning
0.00	235.70	0.0700
15.10	235.42	0.0700
30.21	235.01	0.0700
37.76	234.49	0.0700
45.31	231.43	0.0700
56.64	231.55	0.0700
98.18	229.87	0.0700
117.06	229.64	0.0700
124.61	229.03	0.0700 / 0.0300 Main Channel
128.38	228.69	0.0300 Main Channel
132.16	228.58	0.0300 Main Channel
143.49	228.98	0.0300 / 0.0700 Main Channel
158.59	229.36	0.0700
181.25	229.58	0.0700
200.13	229.96	0.0700
237.89	231.19	0.0700
268.09	232.60	0.0700
302.08	233.40	0.0700
332.29	234.25	0.0700
373.82	235.68	0.0700

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
0.20	228.78	.639E+04	0.3	0.26	359.77
0.40	228.98	.222E+05	1.6	0.40	227.92
0.79	229.37	.842E+05	9.7	0.63	145.25
1.19	229.77	.218E+06	25.5	0.65	142.16
1.58	230.16	.441E+06	53.8	0.67	136.74
1.98	230.56	.715E+06	94.9	0.73	125.69
2.37	230.95	.104E+07	148.9	0.79	116.12
2.76	231.34	.141E+07	217.2	0.85	107.88
3.16	231.74	.183E+07	299.4	0.90	101.81
3.55	232.13	.228E+07	403.8	0.97	94.28
3.95	232.53	.276E+07	523.3	1.04	87.91
4.34	232.92	.326E+07	650.4	1.10	83.62
4.74	233.32	.380E+07	792.5	1.15	79.99
5.13	233.71	.438E+07	954.7	1.20	76.48
5.52	234.10	.499E+07	1134.7	1.25	73.30
5.92	234.50	.563E+07	1334.6	1.31	70.34
6.31	234.89	.631E+07	1540.4	1.35	68.23
6.71	235.29	.702E+07	1746.8	1.37	67.00
7.10	235.68	.780E+07	1958.9	1.38	66.34

	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
INFLOW : ID= 2 (8501)	1120.57	7.90	8.50	25.46	0.71	0.56
OUTFLOW: ID= 1 (6002)	1120.57	5.02	11.00	25.46	0.56	0.47

CALIB	Area	(ha)	QPEAK	TPEAK	R.V.
NASHYD (0002)	869.77	5.253	9.75	15.03	
ID= 1 DT=15.0 min	1a	(mm)	2.50	# of Linear Res. (N)	3.50
	U.H. Tp(hrs)		3.32		

Unit Hyd Qpeak (cms) = 11.293

PEAK FLOW (cms) = 5.253 (i)
 TIME TO PEAK (hrs) = 9.750
 RUNOFF VOLUME (mm) = 15.029
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.240

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8200)	Area	(ha)	QPEAK	TPEAK	R.V.
1 + 2 = 3	869.77	5.253	9.75	15.03	
ID= 1 (0002):	869.77	5.253	9.75	15.03	
+ ID= 2 (6002):	1120.57	5.021	11.00	25.46	
ID = 3 (8200):	1990.34	10.072	10.25	20.90	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6003)	Routing time step (min)'
IN= 2--> OUT= 1	15.00

Distance	Elevation	Manning
0.00	234.33	0.0700
50.41	229.32	0.0700
94.82	227.15	0.0700
113.42	226.02	0.0700
116.57	225.86	0.0700
125.32	224.53	0.0700
130.32	224.24	0.0700 / 0.0300 Main Channel
130.82	223.78	0.0300 Main Channel
131.82	223.63	0.0300 Main Channel
132.42	223.79	0.0300 Main Channel
133.42	224.43	0.0300 / 0.0700 Main Channel
163.83	226.19	0.0700
189.03	227.19	0.0700
229.99	228.02	0.0700
270.95	228.65	0.0700
286.70	229.01	0.0700
305.61	229.31	0.0700

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME
0.20	223.83	.564E+03	0.1	0.26	175.53
0.41	224.04	.168E+04	0.3	0.44	105.08
0.61	224.24	.309E+04	0.6	0.56	82.17
0.93	224.56	.847E+04	1.8	0.57	80.17
1.24	224.87	.209E+05	4.1	0.54	84.69
1.56	225.19	.399E+05	8.0	0.55	83.36
1.88	225.51	.654E+05	13.7	0.57	79.74
2.19	225.82	.976E+05	21.5	0.61	75.55
2.51	226.14	.138E+06	31.1	0.62	73.71
2.83	226.46	.188E+06	43.6	0.64	71.92
3.15	226.78	.250E+06	59.8	0.66	69.63
3.46	227.09	.324E+06	80.5	0.68	67.03
3.78	227.41	.411E+06	103.4	0.69	66.15
4.10	227.73	.516E+06	132.2	0.70	65.13
4.41	228.04	.642E+06	168.2	0.72	63.59
4.73	228.36	.788E+06	210.4	0.73	62.46
5.05	228.68	.959E+06	262.8	0.75	60.81
5.36	228.99	.115E+07	328.9	0.79	58.23
5.68	229.31	.136E+07	401.2	0.81	56.49

	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
INFLOW : ID= 2 (8200)	1990.34	10.07	10.25	20.90	1.68	0.56
OUTFLOW: ID= 1 (6003)	1990.34	9.03	11.75	20.90	1.62	0.55

CALIB

STANDHYD (0031) Area (ha)= 6.23
 ID= 1 DT=15.0 min Total Imp(%)= 65.00 Dir. Conn.(%)= 47.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	4.05	2.18
Dep. Storage (mm)	0.80	2.50
Average Slope (%)	1.00	2.00
Length (m)	203.80	40.00
Mannings n	0.013	0.250
Max.Eff.Inten.(mm/hr)	82.76	40.67
over (min)	15.00	15.00
Storage Coeff. (min)	4.22 (ii)	14.54 (ii)
Unit Hyd. Tpeak (min)	15.00	15.00
Unit Hyd. peak (cms)	0.11	0.07
**TOTALS*		
PEAK FLOW (cms)	0.66	0.17
TIME TO PEAK (hrs)	6.00	6.00
RUNOFF VOLUME (mm)	61.90	18.02
TOTAL RAINFALL (mm)	62.70	62.70
RUNOFF COEFFICIENT	0.99	0.29

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 53.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5003)

IN= 2--> OUT= 1	DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
		0.0000	0.0000	0.4170	0.1860
		0.0210	0.0993	0.5230	0.2047
		0.1120	0.1182	0.6090	0.2231
		0.2250	0.1402	1.0090	0.2331
		0.3120	0.1619	0.0000	0.0000
		AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0031)		6.230	0.835	6.00	38.64
OUTFLOW: ID= 1 (5003)		6.230	0.211	6.25	38.45

PEAK FLOW REDUCTION [Qout/Qin](%)= 25.24
 TIME SHIFT OF PEAK FLOW (min)= 15.00
 MAXIMUM STORAGE USED (ha.m.)= 0.1378

CALIB NASHVD (0032)

Area (ha)= 461.62 Curve Number (CN)= 57.8
 ID= 1 DT=15.0 min Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
 U.H. Tp(hrs)= 2.40
 Unit Hyd Qpeak (cms)= 8.267

PEAK FLOW (cms)	3.500 (i)
TIME TO PEAK (hrs)	8.500
RUNOFF VOLUME (mm)	14.753
TOTAL RAINFALL (mm)	62.700
RUNOFF COEFFICIENT	0.235

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8503)

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0032):	461.62	3.500	8.50	14.75
+ ID2= 2 (5003):	6.23	0.211	6.25	38.45
ID = 3 (8503):	467.85	3.543	8.50	15.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8202)

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6003):	1990.34	9.026	11.75	20.90
+ ID2= 2 (8503):	467.85	3.543	8.50	15.07
ID = 3 (8202):	2458.19	11.040	11.00	19.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (0041)

Area (ha)= 8.66 Total Imp(%)= 52.00 Dir. Conn.(%)= 22.00
 ID= 1 DT=15.0 min

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	4.50	4.16
Dep. Storage (mm)	0.80	2.50
Average Slope (%)	1.00	2.00
Length (m)	240.28	40.00
Mannings n	0.013	0.250
Max.Eff.Inten.(mm/hr)	82.76	42.68
over (min)	15.00	15.00
Storage Coeff. (min)	4.66 (ii)	14.58 (ii)
Unit Hyd. Tpeak (min)	15.00	15.00
Unit Hyd. peak (cms)	0.11	0.07
**TOTALS*		

PEAK FLOW (cms)	0.43	0.35	0.772 (iii)
TIME TO PEAK (hrs)	6.00	6.00	6.00
RUNOFF VOLUME (mm)	61.90	17.65	27.38
TOTAL RAINFALL (mm)	62.70	62.70	62.70
RUNOFF COEFFICIENT	0.99	0.28	0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 50.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5004)

IN= 2--> OUT= 1	DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
		0.0000	0.0000	0.2000	0.2368
		0.0290	0.1168	0.7270	0.2669
		0.1680	0.1367	0.8470	0.2965
		0.3120	0.1706	1.2470	0.3065
		0.4330	0.2010	0.0000	0.0000
		AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0041)		8.660	0.772	6.00	27.38
OUTFLOW: ID= 1 (5004)		8.660	0.152	6.50	27.27

PEAK FLOW REDUCTION [Qout/Qin](%)= 19.67
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 0.1357

CALIB NASHVD (0042)

Area (ha)= 376.06 Curve Number (CN)= 53.3
 ID= 1 DT=15.0 min Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
 U.H. Tp(hrs)= 1.62
 Unit Hyd Qpeak (cms)= 9.964

PEAK FLOW (cms)	3.307 (i)
TIME TO PEAK (hrs)	7.750
RUNOFF VOLUME (mm)	12.818
TOTAL RAINFALL (mm)	62.700
RUNOFF COEFFICIENT	0.204

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8504)

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0042):	376.06	3.307	7.75	12.82
+ ID2= 2 (5004):	8.66	0.152	6.50	27.27
ID = 3 (8504):	384.72	3.380	7.50	13.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8204)

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8202):	2458.19	11.040	11.00	19.79
+ ID2= 2 (8504):	384.72	3.380	7.50	13.14
ID = 3 (8204):	2842.91	12.217	10.50	18.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6005) |
 IN= 2--> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (5.1) -----				
Distance	Elevation	Manning		
0.00	225.02	0.0400		
20.58	224.79	0.0400		
35.54	223.93	0.0400		
48.64	223.09	0.0400		
59.86	222.15	0.0400		
67.35	221.11	0.0400		
76.41	219.63	0.0400 / 0.0350	Main Channel	
82.81	218.53	0.0350	Main Channel	
85.41	218.50	0.0350	Main Channel	
86.78	218.50	0.0350	Main Channel	
88.15	218.50	0.0350	Main Channel	
93.45	218.71	0.0350	Main Channel	
103.45	218.94	0.0350 / 0.0400	Main Channel	
109.45	219.25	0.0400		
114.11	220.15	0.0400		
123.47	221.73	0.0400		
136.56	223.41	0.0400		
153.40	224.15	0.0400		
170.24	224.55	0.0400		
185.20	224.54	0.0400		

----- TRAVEL TIME TABLE -----					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.22	218.72	.640E+04	0.5	0.28	204.26
0.44	218.94	.198E+05	2.2	0.39	147.20
0.77	219.27	.507E+05	9.3	0.63	91.02
1.10	219.60	.880E+05	20.9	0.82	70.26
1.43	219.93	.130E+06	37.7	1.00	57.26
1.76	220.26	.175E+06	59.1	1.16	49.43
2.09	220.59	.235E+06	85.1	1.30	44.19
2.42	220.92	.280E+06	115.8	1.43	40.35
2.75	221.25	.340E+06	151.3	1.54	37.40
3.08	221.58	.404E+06	191.9	1.64	35.06
3.40	221.90	.473E+06	237.5	1.73	33.19
3.73	222.23	.548E+06	288.1	1.82	31.68
4.06	222.56	.629E+06	344.1	1.89	30.48
4.39	222.89	.718E+06	406.8	1.96	29.43
4.72	223.22	.815E+06	476.1	2.02	28.52
5.05	223.55	.920E+06	547.9	2.06	28.00
5.38	223.88	.104E+07	624.5	2.07	27.74
5.71	224.21	.117E+07	710.0	2.09	27.54
6.04	224.54	.133E+07	800.8	2.08	27.62

----- hydrograph -----		<-pipe / channel-->			
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8204) 2842.91	12.22	10.50	18.89	0.85	0.67
OUTFLOW: ID= 1 (6005) 2842.91	11.51	11.75	18.89	0.83	0.66

 CALIB |
 NASHVD (0005) | Area (ha)= 340.11 Curve Number (CN)= 69.5
 ID= 1 DT=15.0 min | Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
 U.H. Tp(hrs)= 2.45

Unit Hyd Qpeak (cms)= 5.972
 PEAK FLOW (cms)= 3.676 (i)
 TIME TO PEAK (hrs)= 8.500
 RUNOFF VOLUME (mm)= 21.111
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.337

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8206) |
 1 2 3 |
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID= 1 (0005): 340.11 3.676 8.50 21.11
 + ID= 2 (6005): 2842.91 11.510 11.75 18.89
 =====
 ID = 3 (8206): 3183.02 13.657 11.00 19.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB |
 NASHVD (0006) | Area (ha)=1375.77 Curve Number (CN)= 55.0
 ID= 1 DT=15.0 min | Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
 U.H. Tp(hrs)= 3.90

Unit Hyd Qpeak (cms)= 15.192

PEAK FLOW (cms)= 6.597 (i)
 TIME TO PEAK (hrs)= 10.500
 RUNOFF VOLUME (mm)= 13.522
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.216

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ROUTE CHN (6007) |
 IN= 2--> OUT= 1 | Routing time step (min)= 15.00

----- DATA FOR SECTION (7.1) -----				
Distance	Elevation	Manning		
0.00	225.91	0.0700		
51.88	225.32	0.0700		
135.69	224.23	0.0700		
183.58	223.08	0.0700		
211.52	223.28	0.0700		
223.49	222.51	0.0700		
231.43	220.69	0.0700		
244.53	219.78	0.0700 / 0.0500	Main Channel	
249.23	219.60	0.0500	Main Channel	
249.93	219.46	0.0500	Main Channel	
251.43	219.23	0.0500	Main Channel	
253.50	219.70	0.0500 / 0.0700	Main Channel	
261.10	219.90	0.0700		
271.38	221.01	0.0700		
283.36	223.77	0.0700		
295.33	224.21	0.0700		
331.25	226.04	0.0700		
351.20	227.76	0.0700		
371.16	227.49	0.0700		
395.10	228.55	0.0700		

----- TRAVEL TIME TABLE -----					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.23	219.46	.112E+04	0.0	0.16	375.56
0.47	219.70	.471E+04	0.3	0.22	277.80
0.84	220.07	.267E+05	2.4	0.34	184.45
1.20	220.43	.626E+05	7.4	0.44	140.97
1.57	220.80	.110E+06	15.6	0.52	117.94
1.93	221.16	.165E+06	27.4	0.61	100.71
2.30	221.53	.225E+06	42.6	0.70	88.16
2.66	221.89	.290E+06	60.9	0.78	79.20
3.03	222.26	.358E+06	82.4	0.85	72.44
3.39	222.62	.431E+06	106.0	0.91	67.79
3.76	222.99	.513E+06	131.5	0.95	65.01
4.12	223.35	.628E+06	149.2	0.88	70.14
4.49	223.72	.788E+06	191.4	0.90	68.58
4.85	224.08	.974E+06	232.1	0.88	69.94
5.22	224.45	.120E+07	283.2	0.88	70.45
5.58	224.81	.147E+07	347.5	0.88	70.32
5.95	225.18	.178E+07	427.9	0.89	69.46
6.31	225.54	.215E+07	523.9	0.90	68.38
6.68	225.91	.257E+07	638.4	0.92	67.05

----- hydrograph -----		<-pipe / channel-->			
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (0006) 1375.77	6.60	10.50	13.52	1.14	0.42
OUTFLOW: ID= 1 (6007) 1375.77	5.23	12.50	13.52	1.04	0.39

 CALIB |
 STANSHVD (0071) | Area (ha)= 16.47
 ID= 1 DT=15.0 min | Total Imp(%)= 47.00 Dir. Conn.(%)= 17.00

Surface Area (ha)= 7.74 IMPERVIOUS (i) 8.73
 Dep. Storage (mm)= 0.80 PERVIOUS (i) 2.50
 Average Slope (%)= 1.00 2.00
 Length (m)= 331.36 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten.(mm/hr)= 82.76 56.29
 over (min) 15.00 15.00
 Storage Coeff. (min)= 5.65 (ii) 14.54 (ii)
 Unit Hyd. Tpeak (min)= 15.00 15.00
 Unit Hyd. peak (cms)= 0.11 0.07
 TOTALS
 PEAK FLOW (cms)= 0.61 0.96 1.576 (iii)
 TIME TO PEAK (hrs)= 6.00 6.00 6.00
 RUNOFF VOLUME (mm)= 61.90 23.82 30.29
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.99 0.38 0.48

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN = 62.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
RESERVOIR (5007)
| IN= 2--> OUT= 1 |
| DT= 15.0 min |
-----
OUTFLOW STORAGE OUTFLOW STORAGE
(cms) (ha.m.) (cms) (ha.m.)
0.0000 0.0000 1.1020 0.4341
0.0560 0.2063 1.3830 0.4946
0.3190 0.2394 1.6100 0.5538
0.5940 0.3063 2.0100 0.5638
0.8240 0.3645 0.0000 0.0000
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (0071) 16.470 1.576 6.00 30.29
OUTFLOW : ID= 1 (5007) 16.470 0.394 6.50 30.23
-----
PEAK FLOW REDUCTION (Qout/Qin)(%)= 25.02
TIME SHIFT OF PEAK FLOW (min)= 30.00
MAXIMUM STORAGE USED (ha.m.)= 0.2587
-----

```

```

-----
CALIB
NASHYD (0072) Area (ha)= 539.30 Curve Number (CN)= 64.2
| ID= 1 DT=15.0 min | Ia (mm)= 2.50 # of Linear Res. (N)= 3.50
| U.H. Tp (hrs)= 2.74 |
-----
Unit Hyd Qpeak (cms)= 8.477
PEAK FLOW (cms)= 4.531 (i)
TIME TO PEAK (hrs)= 9.000
RUNOFF VOLUME (mm)= 17.955
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.286
-----
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
-----

```

```

-----
ADD HYD (8507)
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm) |
+ ID1= 1 (0072): 539.30 4.531 9.00 17.96
+ ID2= 2 (5007): 16.47 0.394 6.50 30.23
=====
ID = 3 (8507): 555.77 4.605 9.00 18.32
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
-----

```

```

-----
ADD HYD (8210)
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm) |
+ ID1= 1 (6007): 1375.77 5.227 12.50 13.52
+ ID2= 2 (8507): 555.77 4.605 9.00 18.32
=====
ID = 3 (8210): 1931.54 8.175 11.00 14.90
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
-----

```

```

-----
ADD HYD (8208)
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm) |
+ ID1= 1 (8206): 3183.02 13.657 11.00 19.13
+ ID2= 2 (8210): 1931.54 8.175 11.00 14.90
=====
ID = 3 (8208): 5114.56 21.832 11.00 17.53
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
-----

```

```

-----
ADD HYD (8212)
| 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
| (ha) (cms) (hrs) (mm) |
+ ID1= 1 (8208): 5114.56 21.832 11.00 17.53
+ ID2= 2 (8214): 875.75 7.821 8.50 17.10
=====

```

ID = 3 (8212): 5990.31 27.444 9.75 17.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ROUTE CHN (6010)
| IN= 2--> OUT= 1 | Routing time step (min)= 15.00
-----
<----- DATA FOR SECTION ( 10.1) ----->
Distance Elevation Manning
0.00 223.78 0.0400
10.97 224.70 0.0400
60.32 222.23 0.0400
104.19 219.90 0.0400
126.13 219.00 0.0400
134.36 218.90 0.0400 /0.0330 Main Channel
137.10 218.41 0.0330 Main Channel
139.84 218.30 0.0330 Main Channel
142.58 216.76 0.0330 Main Channel
145.32 216.74 0.0330 Main Channel
148.07 216.71 0.0330 Main Channel
150.81 217.11 0.0330 Main Channel
153.55 218.40 0.0330 Main Channel
156.29 219.01 0.0330 /0.0400 Main Channel
159.03 219.52 0.0400
167.26 219.89 0.0400
170.00 220.09 0.0400
183.71 221.31 0.0400
200.16 223.39 0.0400
271.45 224.04 0.0400
-----

```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
0.36 217.07 680E+04 0.9 0.36 129.75
0.73 217.44 1.65E+05 3.4 0.58 81.12
1.09 217.81 2.78E+05 7.3 0.75 63.18
1.46 218.17 4.05E+05 12.6 0.88 53.42
1.83 218.54 5.61E+05 17.7 0.90 52.70
2.19 218.90 7.64E+05 26.2 0.97 48.52
2.64 219.35 1.18E+06 44.8 1.07 44.04
3.08 219.79 1.60E+06 71.5 1.13 41.83
3.53 220.24 2.26E+06 109.3 1.17 40.26
3.97 220.68 3.66E+06 159.0 1.23 38.34
4.42 221.13 4.85E+06 221.7 1.30 36.43
4.87 221.58 6.00E+06 298.8 1.37 34.58
5.31 222.02 7.71E+06 391.1 1.44 32.85
5.76 222.47 9.97E+06 498.7 1.51 31.30
6.21 222.92 1.12E+07 623.7 1.58 29.93
6.65 223.36 1.32E+07 764.2 1.65 28.69
7.10 223.81 1.55E+07 878.9 1.60 29.46
7.54 224.25 1.86E+07 1057.1 1.61 29.35
7.99 224.70 2.20E+07 1291.1 1.67 28.35
-----

```

```

----- hydrograph ----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW : ID= 2 (8212) 5990.31 27.44 9.75 17.47 2.22 0.98
OUTFLOW : ID= 1 (6010) 5990.31 26.31 10.75 17.47 2.19 0.97
-----

```

```

-----
CALIB
STANDHYD (0102) Area (ha)= 211.49
| ID= 1 DT=15.0 min | Total Imp(%)= 47.00 Dir. Conn.(%)= 26.00
-----
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 99.40 112.09
Dep. Storage (mm)= 0.80 2.50
Average Slope (%)= 1.00 2.00
Length (m)= 1187.41 40.00
Mannings n = 0.013 0.250
-----
Max.Eff.Inten.(mm/hr)= 82.76 37.81
over (min) 15.00 30.00
Storage Coeff. (min)= 12.16 (ii) 22.58 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04
-----
*TOTALS*
PEAK FLOW (cms)= 10.00 5.63 13.533 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 61.90 16.15 29.52
TOTAL RAINFALL (mm)= 62.70 62.70 62.70
RUNOFF COEFFICIENT = 0.99 0.29 0.47
-----

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN = 55.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
CALIB
STANDHYD (0101) Area (ha)= 133.27
ID= 1 DT=15.0 min Total Imp(%)= 47.00 Dir. Conn.(%)= 26.00
-----
Surface Area (ha)= 62.64 IMPERVIOUS 70.63 PERVIOUS (i)
Dep. Storage (mm)= 0.60 2.50
Average Slope (%)= 1.00 2.00
Length (m)= 942.59 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 82.76 37.81
over (min)= 15.00 30.00
Storage Coeff. (min)= 10.59 (ii) 21.00 (iii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.09 0.05
*****
PEAK FLOW (cms)= 6.61 3.67 8.915 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 61.90 18.15 29.52
TOTAL RAINFALL (mm)= 62.70 62.70 62.70
RUNOFF COEFFICIENT = 0.99 0.29 0.47

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 55.9 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
RESERVOIR (5010)
IN= 2--> OUT= 1
DT= 15.0 min
-----
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
0.0000 0.0000 | 6.8040 3.7679
1.3970 1.4119 | 8.2460 4.4558
3.2730 2.3774 | 10.2760 4.7276
5.2480 3.0364 | 10.6760 4.7376
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (0101) 133.270 8.915 6.00 29.52
OUTFLOW : ID= 1 (5010) 133.270 2.290 6.75 29.52
-----
PEAK FLOW REDUCTION [Qout/Qin](%)= 25.69
TIME SHIFT OF PEAK FLOW (min)= 45.00
MAXIMUM STORAGE USED (ha.m.)= 1.8782

```

```

-----
ADD HYD (8216)
1 + 2 = 3
-----
ID1= 1 (0102): 211.49 13.533 6.00 29.52
+ ID2= 2 (5010): 133.27 2.290 6.75 29.52
=====
ID = 3 (8216): 344.76 14.466 6.00 29.52
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

```

-----
ADD HYD (8218)
1 + 2 = 3
-----
ID1= 1 (5010): 5990.31 26.314 10.75 17.47
+ ID2= 2 (8216): 344.76 14.466 6.00 29.52
=====
ID = 3 (8218): 6335.07 27.702 10.75 18.12
-----
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

 ** SIMULATION NUMBER: 4 **

```

-----
READ STORM
Filename: C:\Users\jascott\AppData
Local\Temp\
ad939110-f7bb-4285-9226-bce91a941061\2dd0affe
Ptotal= 73.10 mm
Comments: 25-Year 12-Hour SCS II Design Storm
-----
TIME RAIN | TIME RAIN | TIME RAIN | TIME RAIN
hrs mm/hr | hrs mm/hr | hrs mm/hr | hrs mm/hr
0.25 1.83 | 3.25 2.92 | 6.25 13.16 | 9.25 2.56
0.50 1.83 | 3.50 2.92 | 6.50 13.16 | 9.50 2.56

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```

0.75 1.83 | 3.75 2.92 | 6.75 5.85 | 9.75 2.56
1.00 1.83 | 4.00 2.92 | 7.00 5.85 | 10.00 2.56
1.25 1.83 | 4.25 4.39 | 7.25 4.39 | 10.25 1.46
1.50 1.83 | 4.50 4.39 | 7.50 4.39 | 10.50 1.46
1.75 1.83 | 4.75 5.85 | 7.75 4.39 | 10.75 1.46
2.00 1.83 | 5.00 5.85 | 8.00 4.39 | 11.00 1.46
2.25 2.19 | 5.25 8.77 | 8.25 2.56 | 11.25 1.46
2.50 2.19 | 5.50 8.77 | 8.50 2.56 | 11.50 1.46
2.75 2.19 | 5.75 35.09 | 8.75 2.56 | 11.75 1.46
3.00 2.19 | 6.00 96.49 | 9.00 2.56 | 12.00 1.46

```

```

-----
CALIB
STANDHYD (0081) Area (ha)= 10.53
ID= 1 DT=15.0 min Total Imp(%)= 45.00 Dir. Conn.(%)= 15.00
-----
Surface Area (ha)= 4.74 IMPERVIOUS 5.79 PERVIOUS (i)
Dep. Storage (mm)= 0.60 2.50
Average Slope (%)= 1.00 2.00
Length (m)= 264.95 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 96.49 68.78
over (min)= 15.00 15.00
Storage Coeff. (min)= 4.65 (ii) 12.85 (iii)
Unit Hyd. Tpeak (min)= 15.00 15.00
Unit Hyd. peak (cms)= 0.11 0.08
*****
PEAK FLOW (cms)= 0.41 0.83 1.238 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 72.30 29.45 35.88
TOTAL RAINFALL (mm)= 73.10 73.10 73.10
RUNOFF COEFFICIENT = 0.99 0.40 0.49

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 61.7 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
RESERVOIR (5008)
IN= 2--> OUT= 1
DT= 15.0 min
-----
OUTFLOW STORAGE | OUTFLOW STORAGE
(cms) (ha.m.) | (cms) (ha.m.)
0.0000 0.0000 | 0.7050 0.2743
0.0360 0.1287 | 0.8850 0.3136
0.2040 0.1489 | 1.0300 0.3521
0.3800 0.1922 | 1.4300 0.3621
0.5270 0.2294 | 0.0000 0.0000
-----
AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (0081) 10.530 1.238 6.00 35.88
OUTFLOW : ID= 1 (5008) 10.530 0.357 6.50 35.79
-----
PEAK FLOW REDUCTION [Qout/Qin](%)= 28.84
TIME SHIFT OF PEAK FLOW (min)= 30.00
MAXIMUM STORAGE USED (ha.m.)= 0.1888

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-----
CALIB
NASHYD (0082) Area (ha)= 402.65 Curve Number (CN)= 63.3
ID= 1 DT=15.0 min Ia (mm)= 2.50 # of Linear Res. (N)= 3.50
U.H. Tp(hrs)= 1.63
-----
Unit Hyd Tpeak (cms)= 10.622
PEAK FLOW (cms)= 6.407 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 22.980
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.313
-----
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

```

-----
ADD HYD (8508)
1 + 2 = 3
-----
ID1= 1 (0082): 402.65 6.407 7.50 22.88
+ ID2= 2 (5008): 10.53 0.357 6.50 35.79
=====
ID = 3 (8508): 413.18 6.568 7.50 23.21

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6009) |
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

DATA FOR SECTION (9.1)----->>>				
Distance	Elevation	Manning		
0.00	232.41	0.0600		
16.80	232.21	0.0600		
28.00	231.55	0.0600		
117.61	229.36	0.0600		
235.21	228.08	0.0600		
296.81	226.96	0.0600		
316.00	226.17	0.0600		
326.00	226.17	0.0600		
332.00	226.04	0.0600 / 0.0350	Main Channel	
335.50	225.16	0.0350	Main Channel	
336.00	225.08	0.0350	Main Channel	
337.00	225.08	0.0350	Main Channel	
337.90	225.21	0.0350	Main Channel	
339.90	225.93	0.0350 / 0.0600	Main Channel	
345.00	226.28	0.0600		
358.42	226.57	0.0600		
403.22	227.73	0.0600		
448.02	229.07	0.0600		
492.82	230.51	0.0600		
554.43	232.28	0.0600		

<----- TRAVEL TIME TABLE ----->						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV.TIME	
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)	
0.28	225.36	.255E+04	0.3	0.37	160.52	
0.57	225.65	.719E+04	1.1	0.55	107.34	
0.85	225.93	.138E+05	2.7	0.70	85.08	
1.25	226.33	.405E+05	7.8	0.69	86.59	
1.64	226.72	.109E+06	20.0	0.65	91.15	
2.04	227.12	.216E+06	41.2	0.68	87.34	
2.44	227.52	.372E+06	74.8	0.72	82.92	
2.83	227.91	.580E+06	125.6	0.77	77.03	
3.23	228.31	.842E+06	193.5	0.82	72.57	
3.63	228.71	.117E+07	284.7	0.87	68.66	
4.02	229.10	.157E+07	405.4	0.92	64.71	
4.42	229.50	.204E+07	564.5	0.99	60.30	
4.82	229.90	.258E+07	765.1	1.07	55.64	
5.22	230.30	.311E+07	1001.2	1.15	51.82	
5.61	230.69	.371E+07	1269.8	1.22	48.68	
6.01	231.09	.435E+07	1573.3	1.29	46.05	
6.41	231.49	.503E+07	1914.5	1.36	43.77	
6.80	231.88	.575E+07	2314.0	1.44	41.39	
7.20	232.28	.650E+07	2741.3	1.50	39.49	

<----- hydrograph ----->>> <-pipe / channel-->						
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8508)	413.18	6.57	7.50	23.21	1.15	0.69
OUTFLOW: ID= 1 (6009)	413.18	4.84	8.75	23.21	1.02	0.69

CALIB | STANDBY (0091) | Area (ha)= 11.32
ID= 1 DT=15.0 min | Total Imp(%)= 50.00 Dir. Conn.(%)= 20.00

IMPERVIOUS PERVIOUS (i)		
Surface Area (ha)=	5.66	5.66
Dep. Storage (mm)=	0.80	2.50
Average Slope (%)=	1.00	2.00
Length (m)=	274.71	40.00
Mannings n	=	0.013 0.250
Max.Eff.Inten.(mm/hr)=	96.49	69.13
over (min)	15.00	15.00
Storage Coeff. (min)=	4.75 (ii)	12.93 (ii)
Unit Hyd. Tpeak (min)=	15.00	15.00
Unit Hyd. peak (cms)=	0.11	0.08
**TOTALS*		
PEAK FLOW (cms)=	0.59	0.81
TIME TO PEAK (hrs)=	6.00	6.00
RUNOFF VOLUME (mm)=	72.30	28.64
TOTAL RAINFALL (mm)=	73.10	73.10
RUNOFF COEFFICIENT =	0.99	0.39

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 59.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5009)				
IM= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	0.7570	0.3065
	0.0380	0.1497	0.9510	0.3465
	0.2190	0.1748	1.1070	0.3857
	0.4090	0.2196	1.5070	0.3957
	0.5670	0.2594	0.0000	0.0000

INFLOW : ID= 2 (0091) 11.320 1.398 6.00 37.37
OUTFLOW: ID= 1 (5009) 11.320 0.389 6.50 37.28

PEAK FLOW REDUCTION [Qout/Qin](%)= 27.83
TIME SHIFT OF PEAK FLOW (min)= 30.00
MAXIMUM STORAGE USED (ha.m.)= 0.2176

CALIB | NASHVD (0092) | Area (ha)= 451.25 Curve Number (CN)= 60.8
ID= 1 DT=15.0 min | Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
U.H. Tp(hrs)= 2.11

Unit Hyd Qpeak (cms)= 9.199

PEAK FLOW (cms)= 5.482 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 21.268
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.291

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8509) | AREA QPEAK TPEAK R.V.
1 + 2 = 3 | (ha) (cms) (hrs) (mm)
ID1= 1 (0092): 451.25 5.482 8.25 21.27
+ ID2= 2 (5009): 11.32 0.389 6.50 37.28
=====

ID = 3 (8509): 462.57 5.580 8.25 21.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8214) | AREA QPEAK TPEAK R.V.
1 + 2 = 3 | (ha) (cms) (hrs) (mm)
ID1= 1 (6009): 413.18 4.844 8.75 23.21
+ ID2= 2 (8509): 462.57 5.580 8.25 21.66
=====

ID = 3 (8214): 875.75 10.270 8.50 22.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB | STANBYD (0011) | Area (ha)= 2.54
ID= 1 DT=15.0 min | Total Imp(%)= 45.00 Dir. Conn.(%)= 15.00

IMPERVIOUS PERVIOUS (i)		
Surface Area (ha)=	1.14	1.40
Dep. Storage (mm)=	0.80	2.50
Average Slope (%)=	1.00	2.00
Length (m)=	130.13	40.00
Mannings n	=	0.013 0.250
Max.Eff.Inten.(mm/hr)=	96.49	64.40
over (min)	15.00	15.00
Storage Coeff. (min)=	3.04 (ii)	11.45 (ii)
Unit Hyd. Tpeak (min)=	15.00	15.00
Unit Hyd. peak (cms)=	0.11	0.08
**TOTALS*		
PEAK FLOW (cms)=	0.10	0.20
TIME TO PEAK (hrs)=	6.00	6.00
RUNOFF VOLUME (mm)=	72.30	27.66
TOTAL RAINFALL (mm)=	73.10	73.10
RUNOFF COEFFICIENT =	0.99	0.38

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

CN* = 59.2 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANBYD (0019) | Area (ha)= 487.70
 |ID= 1 DT=15.0 min | Total Imp(%)= 45.00 Dir. Conn.(%)= 35.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	219.46	268.24
Dep. Storage (mm)=	0.80	5.00
Average Slope (%)=	1.00	2.00
Length (m)=	1803.15	40.00
Mannings n	0.013	0.250

Max.Eff.Inten.(mm/hr)= 96.49 56.67
 over (min) 15.00 30.00
 Storage Coeff. (min)= 14.69 (ii) 23.55 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.07 0.04

*****TOTALS*
 PEAK FLOW (cms)= 33.62 19.81 46.181 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 72.30 31.11 45.53
 TOTAL RAINFALL (mm)= 73.10 73.10
 RUNOFF COEFFICIENT = 0.99 0.43 0.62

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (8511)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	24.3900	10.5911
1.6600	5.9307	32.6300	12.6704
9.4600	6.8569	40.9700	14.4958
17.6100	8.8657	47.7000	16.2820

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
487.700	46.181	6.00	45.53
487.700	21.106	6.50	45.53

INFLOW : ID= 2 (0019) 487.700 46.181 6.00 45.53
 OUTFLOW: ID= 1 (8511) 487.700 21.106 6.50 45.53

PEAK FLOW REDUCTION [Qout/Qin](%)= 45.70
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 9.8672

ADD HYD (8510) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0011): 2.54 0.297 6.00 34.35
 + ID2= 2 (8511): 487.70 21.106 6.50 45.53
 ID = 3 (8510): 490.24 21.174 6.50 45.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5001)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.6830	11.8298
0.0540	0.1960	5.4380	13.3066
1.2810	6.8008	6.1990	14.9402
2.5090	8.6037	6.5990	14.9502
3.4630	9.9865	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
490.240	21.174	6.50	45.47
490.240	4.064	8.75	45.47

INFLOW : ID= 2 (8510) 490.240 21.174 6.50 45.47
 OUTFLOW: ID= 1 (5001) 490.240 4.064 8.75 45.47

PEAK FLOW REDUCTION [Qout/Qin](%)= 19.19
 TIME SHIFT OF PEAK FLOW (min)=135.00
 MAXIMUM STORAGE USED (ha.m.)= 10.8980

CALIB

NASHYD (8012) | Area (ha)= 630.33 Curve Number (CN)= 61.0
 |ID= 1 DT=15.0 min | Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
 U.H. Tp(hrs)= 2.24

Unit Hyd Qpeak (cms)= 12.094

PEAK FLOW (cms)= 7.344 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 21.393
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.293

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8501) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0012): 630.33 7.344 8.25 21.39
 + ID2= 2 (5001): 490.24 4.064 8.75 45.47
 ID = 3 (8501): 1120.57 11.379 8.50 31.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6002) | Routing time step (min)= 15.00
 IN= 2--> OUT= 1 |

----- DATA FOR SECTION (2.1) -----
 Distance Elevation Manning
 0.00 235.70 0.0700
 15.10 235.42 0.0700
 30.21 235.01 0.0700
 37.76 234.49 0.0700
 45.31 231.43 0.0700
 56.64 231.55 0.0700
 98.18 229.87 0.0700
 117.06 229.64 0.0700
 124.61 229.03 0.0700 /0.0300 Main Channel
 128.38 228.69 0.0300 Main Channel
 132.16 228.58 0.0300 Main Channel
 143.49 228.98 0.0300 /0.0700 Main Channel
 158.59 229.36 0.0700
 181.25 229.58 0.0700
 200.13 229.96 0.0700
 237.89 231.19 0.0700
 268.09 232.60 0.0700
 302.08 233.40 0.0700
 332.29 234.25 0.0700
 373.82 235.68 0.0700

----- TRAVEL TIME TABLE -----
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
 (m) (m) (cu.m.) (cms) (m/s) (min)
 0.20 228.78 .639E+04 0.3 0.26 359.77
 0.40 228.98 .222E+05 1.6 0.40 227.92
 0.79 229.37 .842E+05 9.7 0.63 145.25
 1.19 229.77 .218E+06 25.5 0.65 142.16
 1.58 230.16 .441E+06 53.8 0.67 136.74
 1.98 230.56 .715E+06 94.9 0.73 125.69
 2.37 230.95 1.004E+07 148.9 0.79 116.12
 2.76 231.34 .141E+07 217.2 0.85 107.88
 3.16 231.74 .183E+07 299.4 0.90 101.81
 3.55 232.13 .228E+07 403.8 0.97 94.28
 3.95 232.53 .276E+07 523.3 1.04 87.91
 4.34 232.92 .326E+07 650.4 1.10 83.62
 4.74 233.32 .380E+07 792.5 1.15 79.99
 5.13 233.71 .438E+07 954.7 1.20 76.48
 5.52 234.10 .499E+07 1134.7 1.25 73.30
 5.92 234.50 .563E+07 1334.6 1.31 70.34
 6.31 234.89 .631E+07 1540.4 1.35 68.23
 6.71 235.29 .702E+07 1746.8 1.37 67.00
 7.10 235.68 .780E+07 1958.9 1.38 66.34

----- hydrograph ----- <-pipe / channel-->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8501) 1120.57 11.38 8.50 31.92 0.84 0.63
 OUTFLOW: ID= 1 (6002) 1120.57 7.68 10.25 31.92 0.70 0.55

CALIB
 NASHYD (0002) | Area (ha)= 869.77 Curve Number (CN)= 58.4
 |ID= 1 DT=15.0 min | Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
 U.H. Tp(hrs)= 3.32

Unit Hyd Qpeak (cms)= 11.293

PEAK FLOW (cms)= 6.943 (i)

TIME TO PEAK (hrs)= 9.750
 RUNOFF VOLUME (mm)= 19.816
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.271

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8200)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0002):	869.77	6.943	9.75	19.82
+ ID2= 2 (6002):	1120.57	7.685	10.25	31.92
=====				
ID = 3 (8200):	1990.34	14.507	10.00	26.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6003)
 IM= 2 -> OUT= 1
 Routing time step (min)= 15.00

----- DATA FOR SECTION (3.1) -----

Distance	Elevation	Manning		
0.00	234.33	0.0700		
50.41	229.32	0.0700		
94.52	227.15	0.0700		
113.42	226.02	0.0700		
116.57	225.86	0.0700		
125.32	224.53	0.0700		
130.32	224.24	0.0700 / 0.0300	Main Channel	
130.82	223.78	0.0300	Main Channel	
131.82	223.63	0.0300	Main Channel	
132.42	223.79	0.0300	Main Channel	
133.42	224.43	0.0300 / 0.0700	Main Channel	
163.83	226.19	0.0700		
189.03	227.19	0.0700		
229.99	228.02	0.0700		
270.95	228.65	0.0700		
286.70	229.01	0.0700		
305.61	229.31	0.0700		

----- TRAVEL TIME TABLE -----

DEPTH	RLV	VOLUME	FLOW RATE	VELOCITY	TRAV TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.20	223.83	.564E+03	0.1	0.26	175.53
0.41	224.04	.168E+04	0.3	0.44	105.08
0.61	224.24	.309E+04	0.6	0.56	82.17
0.93	224.56	.847E+04	1.8	0.57	80.17
1.24	224.87	.209E+05	4.1	0.54	84.69
1.56	225.19	.399E+05	8.0	0.55	83.36
1.88	225.51	.654E+05	13.7	0.57	79.74
2.19	225.82	.976E+05	21.5	0.61	75.55
2.51	226.14	.138E+06	31.1	0.62	73.71
2.83	226.46	.188E+06	43.6	0.64	71.92
3.15	226.78	.250E+06	59.8	0.66	69.63
3.46	227.09	.324E+06	80.5	0.68	67.03
3.78	227.41	.411E+06	103.4	0.69	66.15
4.10	227.73	.516E+06	132.2	0.70	65.13
4.41	228.04	.642E+06	168.2	0.72	63.59
4.73	228.36	.788E+06	210.4	0.73	62.46
5.05	228.68	.959E+06	262.8	0.75	60.81
5.36	228.99	.115E+07	328.9	0.79	58.23
5.68	229.31	.136E+07	401.2	0.81	56.49

<---- hydrograph ----> <-pipe / channel->

	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8200) 1990.34	14.51	10.00	26.63	1.91	0.58	
OUTFLOW: ID= 1 (6003) 1990.34	12.81	11.50	26.63	1.83	0.57	

CALIB
 STANDHYD (0031)
 ID= 1 DT=15.0 min
 Total Imp(%)= 65.00 Dir. Conn.(%)= 47.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	4.05	2.18
Dep. Storage (mm)=	0.80	2.50
Average Slope (%)=	1.00	2.00
Length (m)=	203.80	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	53.23
over (min)	15.00	15.00
Storage Coeff. (min)=	3.97 (ii)	13.06 (ii)
Unit Hyd. Tpeak (min)=	15.00	15.00
Unit Hyd. peak (cms)=	0.11	0.08

PEAK FLOW (cms)= 0.77 0.24 *TOTALS* 1.011 (iii)

TIME TO PEAK (hrs)= 6.00 6.00 6.00
 RUNOFF VOLUME (mm)= 72.30 23.50 46.43
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.99 0.32 0.64

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 53.5 Is = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5003)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IM= 2 -> OUT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min	0.0000	0.0000	0.4170	0.1860
	0.0210	0.0993	0.5230	0.2047
	0.1210	0.1182	0.6090	0.2231
	0.2250	0.1402	1.0090	0.2331
	0.3120	0.1619	0.0000	0.0000

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0031)	6.230	1.011	6.00	46.43
OUTFLOW: ID= 1 (5003)	6.230	0.289	6.25	46.25

PEAK FLOW REDUCTION [Qout/Qin](%)= 28.59
 TIME SHIFT OF PEAK FLOW (min)= 15.00
 MAXIMUM STORAGE USED (ha.m.)= 0.1586

CALIB
 NASHYD (0032)
 ID= 1 DT=15.0 min
 U.H. Tp(hrs)= 2.40
 Area (ha)= 461.62 Curve Number (CN)= 57.8
 # of Linear Res. (N)= 3.50

Unit Hyd Qpeak (cms)= 8.267

PEAK FLOW (cms)= 4.636 (i)
 TIME TO PEAK (hrs)= 8.500
 RUNOFF VOLUME (mm)= 19.467
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.266

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8503)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0032):	461.62	4.636	8.50	19.47
+ ID2= 2 (5003):	6.23	0.289	6.25	46.25
=====				
ID = 3 (8503):	467.85	4.689	8.50	19.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8202)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6003):	1990.34	12.808	11.50	26.63
+ ID2= 2 (8503):	467.85	4.689	8.50	19.82
=====				
ID = 3 (8202):	2458.19	15.612	11.00	25.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (0041)
 ID= 1 DT=15.0 min
 Total Imp(%)= 52.00 Dir. Conn.(%)= 22.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	4.50	4.16
Dep. Storage (mm)=	0.80	2.50
Average Slope (%)=	1.00	2.00
Length (m)=	240.28	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 96.49 55.90
 over (min)= 15.00 15.00
 Storage Coeff. (min)= 4.39 (ii) 13.29 (ii)
 Unit Hyd. Tpeak (min)= 15.00 15.00
 Unit Hyd. peak (cms)= 0.11 0.08

 PEAK FLOW (cms) = 0.50 0.47
 TIME TO PEAK (hrs) = 6.00 6.00
 RUNOFF VOLUME (mm) = 72.30 23.03 33.87
 TOTAL RAINFALL (mm) = 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.99 0.32 0.46

*****TOTALS*

0.973 (iii)
 6.00
 33.87
 73.10
 0.46

86.78 218.50 0.0350 Main Channel
 89.15 218.50 0.0350 Main Channel
 93.45 218.71 0.0350 Main Channel
 103.45 218.94 0.0350 / 0.0400 Main Channel
 109.45 219.25 0.0400
 114.11 220.15 0.0400
 123.47 221.73 0.0400
 136.56 223.41 0.0400
 153.40 224.15 0.0400
 170.24 224.55 0.0400
 185.20 224.54 0.0400

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 50.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR (5004)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.5790	0.2368
0.0290	0.1168	0.7270	0.2669
0.1680	0.1367	0.8470	0.2965
0.3120	0.1706	1.2470	0.3065
0.4330	0.2010	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
8.660	0.973	6.00	33.87
8.660	0.243	6.50	33.75

PEAK FLOW (cms)	REDUCTION (Qout/Qin) (%)	TIME SHIFT OF PEAK FLOW (min)	MAXIMUM STORAGE USED (ha.m.)
24.98		30.00	0.1553

 CALIB (0042) Area (ha) = 376.06 Curve Number (CN) = 53.3
 ID= 1 DT=15.0 min Ia (mm) = 2.50 # of Linear Res.(N) = 3.50
 U.H. Tp(hrs) = 1.62
 Unit Hyd Qpeak (cms) = 9.964
 PEAK FLOW (cms) = 4.405 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 17.004
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.233
 (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8504)

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0042):	376.06	4.405	7.50	17.00
+ ID2= 2 (5004):	8.66	0.243	6.50	33.75
ID = 3 (8504):	384.72	4.512	7.50	17.38

 NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8204)

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8202):	2458.19	15.612	11.00	25.34
+ ID2= 2 (8504):	384.72	4.512	7.50	17.38
ID = 3 (8204):	2842.91	17.221	10.50	24.26

 NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6005)
 IN= 2--> OUT= 1
 Routing time step (min) = 15.00

 <----- DATA FOR SECTION (5.1) ----->

Distance	Elevation	Manning
0.00	225.02	0.0400
20.58	224.79	0.0400
35.54	223.93	0.0400
48.64	223.09	0.0400
59.86	222.15	0.0400
67.85	221.11	0.0400
76.41	219.63	0.0400 / 0.0350
82.81	218.53	0.0350
85.41	218.50	0.0350

 TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.22	218.72	.640E+04	0.5	0.28	204.26
0.44	218.94	.198E+05	2.2	0.39	147.20
0.77	219.27	.507E+05	9.3	0.63	91.02
1.10	219.60	.880E+05	20.9	0.82	70.26
1.43	219.93	.130E+06	37.7	1.00	57.26
1.76	220.26	.175E+06	59.1	1.16	49.43
2.09	220.59	.226E+06	85.1	1.30	44.19
2.42	220.92	.280E+06	115.8	1.43	40.35
2.75	221.25	.340E+06	151.3	1.54	37.40
3.08	221.58	.404E+06	191.9	1.64	35.06
3.40	221.90	.473E+06	237.5	1.73	33.19
3.73	222.23	.548E+06	288.1	1.82	31.68
4.06	222.56	.629E+06	344.1	1.89	30.48
4.39	222.89	.718E+06	406.8	1.96	29.43
4.72	223.22	.815E+06	476.1	2.02	28.52
5.05	223.55	.920E+06	547.9	2.06	28.00
5.38	223.88	.104E+07	624.5	2.07	27.74
5.71	224.21	.117E+07	710.0	2.09	27.54
6.04	224.54	.133E+07	800.8	2.08	27.62

 <---- hydrograph ----> <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
ID= 2 (8204) 2842.91	16.25	11.50	24.26	0.99	0.75
OUTFLOW: ID= 1 (6005) 2842.91	16.25	11.50	24.26	0.97	0.73

 CALIB (0005) Area (ha) = 340.11 Curve Number (CN) = 69.5
 ID= 1 DT=15.0 min Ia (mm) = 2.50 # of Linear Res.(N) = 3.50
 U.H. Tp(hrs) = 2.45
 Unit Hyd Qpeak (cms) = 5.972
 PEAK FLOW (cms) = 4.789 (i)
 TIME TO PEAK (hrs) = 8.500
 RUNOFF VOLUME (mm) = 27.377
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.375
 (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8206)

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0005):	340.11	4.789	8.50	27.38
+ ID2= 2 (6005):	2842.91	16.247	11.50	24.26
ID = 3 (8206):	3183.02	19.117	11.00	24.59

 NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB (0006) Area (ha) = 1375.77 Curve Number (CN) = 55.0
 ID= 1 DT=15.0 min Ia (mm) = 2.50 # of Linear Res.(N) = 3.50
 U.H. Tp(hrs) = 3.90
 Unit Hyd Qpeak (cms) = 15.192
 PEAK FLOW (cms) = 8.749 (i)
 TIME TO PEAK (hrs) = 10.500
 RUNOFF VOLUME (mm) = 17.902
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.245
 (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ROUTE CHN (6007)
 IN= 2--> OUT= 1
 Routing time step (min) = 15.00

 <----- DATA FOR SECTION (7.1) ----->

Distance	Elevation	Manning
0.00	225.81	0.0700
51.88	225.32	0.0700
135.69	224.23	0.0700
183.58	223.08	0.0700
211.52	223.28	0.0700
223.49	222.51	0.0700
231.43	220.69	0.0700
244.53	219.78	0.0700 / 0.0500
249.23	219.60	0.0500
249.93	219.46	0.0500
251.43	219.23	0.0500
253.50	219.70	0.0500 / 0.0700
261.10	219.90	0.0700
271.38	221.01	0.0700
283.36	223.77	0.0700
295.33	224.21	0.0700
331.25	226.04	0.0700
351.20	227.76	0.0700
371.16	227.49	0.0700
395.10	228.55	0.0700

0.3190	0.2394	1.6100	0.5538
0.5940	0.3063	2.0100	0.5638
0.8240	0.3645	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0071)	16.470	2.012	6.00	37.68
OUTFLOW: ID= 1 (5007)	16.470	0.594	6.50	37.62

PEAK FLOW REDUCTION [Qout/Qin](%) = 29.53
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 0.3104

CALIB (0072)	Area (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1	539.30	5.946	9.00	23.49

Unit Hyd Qpeak (cms) = 8.477

PEAK FLOW (cms) = 5.946 (1)
TIME TO PEAK (hrs) = 9.000
RUNOFF VOLUME (mm) = 23.485
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.321

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8507)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3	539.30	5.946	9.00	23.49
ID1 = 2 (5007):	16.47	0.594	6.50	37.62
ID = 3 (8507):	555.77	6.037	9.00	23.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8210)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3	1375.77	7.226	12.50	17.90
ID1 = 1 (6007):	1375.77	7.226	12.50	17.90
+ ID2 = 2 (8507):	555.77	6.037	9.00	23.90
ID = 3 (8210):	1931.54	11.041	11.00	19.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8208)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3	3183.02	19.117	11.00	24.59
ID1 = 1 (8206):	3183.02	19.117	11.00	24.59
+ ID2 = 2 (8210):	1931.54	11.041	11.00	19.63
ID = 3 (8208):	5114.56	30.158	11.00	22.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8212)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3	5114.56	30.158	11.00	22.72
ID1 = 1 (8208):	5114.56	30.158	11.00	22.72
+ ID2 = 2 (8214):	875.75	10.270	8.50	22.39
ID = 3 (8212):	5990.31	37.224	10.00	22.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6010)	Routing time step (min) = 15.00
IN= 2--> OUT= 1	

Distance	Elevation	Manning
0.00	223.78	0.0400
10.97	224.70	0.0400
60.32	222.23	0.0400
104.19	219.90	0.0400
126.13	219.00	0.0400
134.36	218.90	0.0400 / 0.0330

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.23	219.46	1.12E+04	0.0	0.16	375.56
0.47	219.70	4.71E+04	0.3	0.22	277.80
0.84	220.07	2.67E+05	2.4	0.34	184.45
1.20	220.43	6.26E+05	7.4	0.44	140.97
1.57	220.80	1.10E+06	15.6	0.52	117.94
1.93	221.16	1.65E+06	27.4	0.61	100.71
2.30	221.53	2.28E+06	42.6	0.70	88.16
2.66	221.89	2.90E+06	60.9	0.78	79.20
3.03	222.26	3.58E+06	82.4	0.85	72.44
3.39	222.62	4.31E+06	106.0	0.91	67.79
3.76	222.99	5.13E+06	131.5	0.95	65.01
4.12	223.35	6.28E+06	149.2	0.88	70.14
4.49	223.72	7.88E+06	191.4	0.90	68.58
4.85	224.08	9.74E+06	232.1	0.88	69.94
5.22	224.45	1.20E+07	283.2	0.88	70.45
5.58	224.81	1.47E+07	347.5	0.88	70.32
5.95	225.18	1.78E+07	427.9	0.89	69.46
6.31	225.54	2.15E+07	523.9	0.90	68.38
6.68	225.91	2.57E+07	638.4	0.92	67.05

<---- hydrograph ----> <-pipe / channel->

INFLOW : ID= 2 (0006)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
1375.77	8.75	10.50	17.90	1.26	0.45	
OUTFLOW: ID= 1 (6007)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
1375.77	7.23	12.50	17.90	1.18	0.43	

CALIB (0071)	Area (ha)	Total Imp(%)	Dir. Com.(%)
1	16.47	47.00	17.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) = 7.74	8.73
Dep. Storage (mm) = 0.80	2.50
Average Slope (ft) = 1.00	2.00
Length (m) = 331.36	40.00
Mannings n = 0.013	0.250
Max. Eff. Inten. (mm/hr) = 96.49	72.49
over (min) = 15.00	15.00
Storage Coeff. (min) = 5.32 (ii)	13.34 (iii)
Unit Hyd. Tpeak (min) = 15.00	15.00
Unit Hyd. peak (cms) = 0.11	0.08

TOTALS

PEAK FLOW (cms) = 0.72	1.29	2.012 (iii)
TIME TO PEAK (hrs) = 6.00	6.00	
RUNOFF VOLUME (mm) = 72.30	30.58	37.68
TOTAL RAINFALL (mm) = 73.10	73.10	73.10
RUNOFF COEFFICIENT = 0.99	0.42	0.52

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 62.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5007)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
DT= 15.0 min	0.0000	0.0000	1.1020	0.4341
	0.0560	0.2063	1.3830	0.4946

137.10	218.41	0.0330	Main Channel
139.84	218.30	0.0330	Main Channel
142.58	216.76	0.0330	Main Channel
145.32	216.74	0.0330	Main Channel
148.07	216.71	0.0330	Main Channel
150.81	217.11	0.0330	Main Channel
153.55	218.40	0.0330	Main Channel
156.29	219.01	0.0330 / 0.0400	Main Channel
159.03	219.52	0.0400	
167.26	219.89	0.0400	
170.00	220.09	0.0400	
183.71	221.31	0.0400	
200.16	223.39	0.0400	
271.45	224.04	0.0400	

----- TRAVEL TIME TABLE -----

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.36	217.07	.680E+04	0.9	0.36	129.75
0.73	217.44	.165E+05	3.4	0.58	81.12
1.09	217.81	.278E+05	7.3	0.75	63.18
1.46	218.17	.405E+05	12.6	0.88	53.42
1.83	218.54	.561E+05	17.7	0.90	52.70
2.19	218.90	.764E+05	26.2	0.97	48.52
2.64	219.35	.118E+06	44.8	1.07	44.04
3.08	219.79	.180E+06	71.5	1.13	41.83
3.53	220.24	.264E+06	109.3	1.17	40.26
3.97	220.68	.365E+06	159.0	1.23	38.94
4.42	221.13	.485E+06	221.7	1.30	36.43
4.87	221.58	.620E+06	298.8	1.37	34.58
5.31	222.02	.771E+06	391.1	1.44	32.85
5.76	222.47	.937E+06	498.7	1.51	31.30
6.21	222.92	.112E+07	622.7	1.58	29.93
6.65	223.36	.132E+07	764.2	1.65	28.69
7.10	223.81	.155E+07	878.9	1.60	29.46
7.54	224.25	.186E+07	1057.1	1.61	29.35
7.99	224.70	.220E+07	1291.1	1.67	28.35

----- hydrograph ----- <-pipe / channel->

AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8212) 5990.31	37.22	10.00	22.67	2.45	1.03
OUTFLOW : ID= 1 (6010) 5990.31	36.04	10.75	22.67	2.42	1.02

----- CALIB -----

STANDHYD (0102)	Area (ha) = 211.49	Dir. Conn.(%) = 26.00
ID= 1 DT=15.0 min	Total Imp(%) = 47.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	99.40	112.09
Dep. Storage (mm)	0.80	2.50
Average Slope (%)	1.00	2.00
Length (m)	1187.41	40.00
Mannings n	0.013	0.250

Max.Eff.Inten.(mm/hr)=	96.49	49.49
over (min)	15.00	30.00
Storage Coeff. (min)	11.44 (ii)	20.79 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.05

	TOTALS		
PEAK FLOW (cms)	11.92	7.66	16.775 (iii)
TIME TO PEAK (hrs)	6.00	6.25	6.00
RUNOFF VOLUME (mm)	72.30	23.67	36.31
TOTAL RAINFALL (mm)	73.10	73.10	73.10
RUNOFF COEFFICIENT	0.99	0.32	0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 55.9 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB -----

STANDHYD (0101)	Area (ha) = 133.27	Dir. Conn.(%) = 26.00
ID= 1 DT=15.0 min	Total Imp(%) = 47.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	62.64	70.63
Dep. Storage (mm)	0.80	2.50
Average Slope (%)	1.00	2.00
Length (m)	942.59	40.00
Mannings n	0.013	0.250

Max.Eff.Inten.(mm/hr)=	96.49	49.49
over (min)	15.00	30.00
Storage Coeff. (min)	9.96 (ii)	19.31 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00

Unit Hyd. peak (cms)	0.09	0.05	*TOTALS*
PEAK FLOW (cms)	7.85	4.99	11.023 (iii)
TIME TO PEAK (hrs)	6.00	6.25	6.00
RUNOFF VOLUME (mm)	72.30	23.67	36.31
TOTAL RAINFALL (mm)	73.10	73.10	73.10
RUNOFF COEFFICIENT	0.99	0.32	0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 55.9 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- RESERVOIR (5010) -----

DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	6.8040	3.7679
	1.3970	1.4119	8.2460	4.4558
	3.2730	2.3774	10.2760	4.7276
	5.2480	3.0364	10.6760	4.7376

INFLOW : ID= 2 (0101)	133.270	11.023	6.00	36.31
OUTFLOW : ID= 1 (5010)	133.270	3.048	6.75	36.31

PEAK FLOW REDUCTION [Qout/Qin](%) = 27.65
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha.m.) = 2.2644

----- ADD HYD (8216) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0102):	211.49	16.775	6.00	36.31
+ ID2= 2 (5010):	133.27	3.048	6.75	36.31
=====				
ID = 3 (8216):	344.76	17.920	6.00	36.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- ADD HYD (8218) -----

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6010):	5990.31	36.039	10.75	22.67
+ ID2= 2 (8216):	344.76	17.920	6.00	36.31
=====				
ID = 3 (8218):	6335.07	37.672	10.75	23.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

***** SIMULATION NUMBER: 5 *****

----- READ STORM -----

Filename:	C:\Users\jscott\AppData\Local\Temp\ad939110-f7bb-4285-9226-bce91a941061\7ae15a6b
Comments:	50-Year 12-Hour SCS II Design Storm
Ptotal:	80.80 mm

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	2.02	3.25	3.23	6.25	14.54	9.25	2.83
0.50	2.02	3.50	3.23	6.50	14.54	9.50	2.83
0.75	2.02	3.75	3.23	6.75	6.46	9.75	2.83
1.00	2.02	4.00	3.23	7.00	6.46	10.00	2.83
1.25	2.02	4.25	4.85	7.25	4.85	10.25	1.62
1.50	2.02	4.50	4.85	7.50	4.85	10.50	1.62
1.75	2.02	4.75	6.46	7.75	4.85	10.75	1.62
2.00	2.02	5.00	6.46	8.00	4.85	11.00	1.62
2.25	2.42	5.25	9.70	8.25	2.83	11.25	1.62
2.50	2.42	5.50	9.70	8.50	2.83	11.50	1.62
2.75	2.42	5.75	38.78	8.75	2.83	11.75	1.62
3.00	2.42	6.00	106.66	9.00	2.83	12.00	1.62

----- CALIB -----

STANDHYD (0081)	Area (ha) = 10.53	Dir. Conn.(%) = 15.00
ID= 1 DT=15.0 min	Total Imp(%) = 45.00	

```

-----
Surface Area (ha)= 4.74 5.79
Dep. Storage (mm)= 0.80 2.50
Average Slope (ft)= 1.00 2.00
Length (m)= 264.95 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 106.66 80.95
over (min) 15.00 15.00
Storage Coeff. (min)= 4.47 (ii) 12.15 (ii)
Unit Hyd. Tpeak (min)= 15.00 15.00
Unit Hyd. peak (cms)= 0.11 0.08

*TOTALS*
PEAK FLOW (cms)= 0.46 1.00 1.453 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 80.00 34.60 41.41
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.99 0.43 0.51

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 61.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
RESERVOIR (5008)
IN= 2--> OUT= 1
DT= 15.0 min

```

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.7050	0.2743
0.0360	0.1287	0.8850	0.3136
0.2040	0.1489	1.0300	0.3521
0.3800	0.1922	1.4300	0.3621
0.5270	0.2294	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0081)	10.530	1.453	6.00
OUTFLOW : ID= 1 (5008)	10.530	0.430	6.50

PEAK FLOW REDUCTION [Qout/Qin](%) = 29.60
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha.m.) = 0.2086

```

-----
CALIB
STANDHYD (0091)
ID= 1 DT=15.0 min

```

Area (ha)	Curve Number (CN)	# of Linear Res. (N)
402.65	63.3	3.50

U.H. Tp (hrs) = 1.63

Unit Hyd Qpeak (cms) = 10.622

PEAK FLOW (cms) = 7.640 (i)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 27.182
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.336

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
ADD HYD (8508)
1 + 2 = 3

```

ID	Area (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 1 (0082)	402.65	7.640	7.50	27.18
ID2 = 2 (5008)	10.53	0.430	6.50	41.33
ID = 3 (8508)	413.18	7.849	7.50	27.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
ROUTE CN (6009)
IN= 2--> OUT= 1
Routing time step (min) = 15.00

```

<----- DATA FOR SECTION (9.1) ----->

Distance	Elevation	Manning
0.00	232.41	0.0600
16.80	232.21	0.0600
28.00	231.55	0.0600
117.61	229.36	0.0600
235.21	228.08	0.0600
296.81	226.96	0.0600
316.00	226.17	0.0600

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.28	225.36	2558+04	0.3	0.37	160.52
0.57	225.65	7198+04	1.1	0.55	107.34
0.85	225.93	1388+05	2.7	0.70	85.08
1.25	226.33	4058+05	7.8	0.69	86.59
1.64	226.72	1098+06	20.0	0.65	91.15
2.04	227.12	2168+06	41.2	0.68	87.34
2.44	227.52	3728+06	74.8	0.72	82.92
2.83	227.91	5808+06	125.6	0.77	77.03
3.23	228.31	8428+06	193.5	0.82	72.57
3.63	228.71	1178+07	284.7	0.87	68.66
4.02	229.10	1578+07	405.4	0.92	64.71
4.42	229.50	2048+07	564.5	0.99	60.30
4.82	229.90	2568+07	766.1	1.07	55.64
5.22	230.30	3118+07	1001.2	1.15	51.82
5.61	230.69	3718+07	1269.8	1.22	48.68
6.01	231.09	4358+07	1573.3	1.29	46.05
6.41	231.49	5038+07	1914.5	1.36	43.77
6.80	231.88	5758+07	2314.0	1.44	41.39
7.20	232.28	6508+07	2741.3	1.50	39.49

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-----
TRAVEL TIME TABLE
DEPTH (m)  ELEV (m)  VOLUME (cu.m.)  FLOW RATE (cms)  VELOCITY (m/s)  TRAV. TIME (min)

```

<---- hydrograph ----> <-pipe / channel-->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8508)	413.18	7.85	7.50	27.54	1.25
OUTFLOW : ID= 1 (6009)	413.18	5.75	8.75	27.54	1.09

```

-----
CALIB
STANDHYD (0091)
ID= 1 DT=15.0 min

```

Area (ha) = 11.32
Total Imp(%) = 50.00
Dir. Conn.(%) = 20.00

```

-----
Surface Area (ha)= 5.66 5.66
Dep. Storage (mm)= 0.80 2.50
Average Slope (ft)= 1.00 2.00
Length (m)= 274.71 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 106.66 81.46
over (min) 15.00 15.00
Storage Coeff. (min)= 4.57 (ii) 12.23 (ii)
Unit Hyd. Tpeak (min)= 15.00 15.00
Unit Hyd. peak (cms)= 0.11 0.08

*TOTALS*
PEAK FLOW (cms)= 0.65 0.98 1.631 (iii)
TIME TO PEAK (hrs)= 6.00 6.00 6.00
RUNOFF VOLUME (mm)= 80.00 33.68 42.95
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.99 0.42 0.53

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 59.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
RESERVOIR (5009)
IN= 2--> OUT= 1
DT= 15.0 min

```

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.7570	0.3065
0.0380	0.1497	0.9510	0.3465
0.2190	0.1748	1.1070	0.3957
0.4090	0.2196	1.5070	0.3957
0.5670	0.2594	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0091)	11.320	1.631	6.00
OUTFLOW : ID= 1 (5009)	11.320	0.473	6.25

PEAK FLOW REDUCTION [Qout/Qin](%)= 29.02
 TIME SHIFT OF PEAK FLOW (min)= 15.00
 MAXIMUM STORAGE USED (ha.m.)= 0.2391

CALIB
 NASHVD (0092) Area (ha)= 451.25 Curve Number (CN)= 60.8
 ID= 1 DT=15.0 min Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
 U.H. Tp(hrs)= 2.11

Unit Hyd Qpeak (cms)= 9.199

PEAK FLOW (cms)= 6.545 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 25.328
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.313

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8509)
 1 + 2 = 3 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0092): 451.25 6.545 8.25 25.33
 + ID2= 2 (5009): 11.32 0.473 6.25 42.86
 ID = 3 (8509): 462.57 6.660 8.25 25.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8214)
 1 + 2 = 3 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (6009): 413.18 5.751 8.75 27.54
 + ID2= 2 (8509): 462.57 6.660 8.25 25.76
 ID = 3 (8214): 875.75 12.226 8.50 26.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (0011) Area (ha)= 2.54
 ID= 1 DT=15.0 min Total Imp(%)= 45.00 Dir. Conn.(%)= 15.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 1.14 1.40
 Dep. Storage (mm)= 0.80 2.50
 Average Slope (%)= 1.00 2.00
 Length (m)= 130.13 40.00
 Mannings n = 0.013 0.250

Max. Rff. Inten.(mm/hr)= 106.66 76.01
 over (min) 15.00 15.00
 Storage Coeff. (min)= 2.92 (ii) 10.79 (ii)
 Unit Hyd. Tpeak (min)= 15.00 15.00
 Unit Hyd. peak (cms)= 0.11 0.09

TOTALS

PEAK FLOW (cms)= 0.11 0.24 0.348 (iii)
 TIME TO PEAK (hrs)= 6.00 6.00 6.00
 RUNOFF VOLUME (mm)= 80.00 32.58 39.69
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.99 0.40 0.49

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.2 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (0019) Area (ha)= 487.70
 ID= 1 DT=15.0 min Total Imp(%)= 45.00 Dir. Conn.(%)= 35.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 219.46 268.24
 Dep. Storage (mm)= 0.80 5.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 1803.15 40.00
 Mannings n = 0.013 0.250

Max. Rff. Inten.(mm/hr)= 106.66 66.74
 over (min) 15.00 30.00
 Storage Coeff. (min)= 14.12 (ii) 22.41 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.07 0.04

TOTALS

PEAK FLOW (cms)= 37.78 23.95 53.114 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 80.00 36.61 51.80
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.99 0.45 0.64

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (8511)
 IN= 2--> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	24.3900	10.5911
	1.6600	5.9307	32.6300	12.6704
	9.4600	6.8569	40.9700	14.4958
	17.6100	8.8657	47.7000	16.2820

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (0019)	487.700	53.114	6.00	51.80
OUTFLOW: ID= 1 (8511)	487.700	24.685	6.50	51.79

PEAK FLOW REDUCTION [Qout/Qin](%)= 46.48
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 10.7627

ADD HYD (8510)
 1 + 2 = 3 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0011): 2.54 0.348 6.00 39.69
 + ID2= 2 (8511): 487.70 24.685 6.50 51.79
 ID = 3 (8510): 490.24 24.762 6.50 51.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5001)
 IN= 2--> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	4.6820	11.8298
	0.0540	0.1960	5.4380	13.3066
	1.2810	6.8008	6.1990	14.9402
	2.5090	8.6037	6.5990	14.9502
	3.4630	9.9865	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (8510)	490.240	24.762	6.50	51.73
OUTFLOW: ID= 1 (5001)	490.240	5.140	8.75	51.73

PEAK FLOW REDUCTION [Qout/Qin](%)= 20.76
 TIME SHIFT OF PEAK FLOW (min)=135.00
 MAXIMUM STORAGE USED (ha.m.)= 12.7469

CALIB
 NASHVD (0012) Area (ha)= 630.33 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
 U.H. Tp(hrs)= 2.24

Unit Hyd Qpeak (cms)= 12.094

PEAK FLOW (cms)= 8.770 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 25.472
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.315

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8501)	1	2	3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0012):	630.33	8.770	8.25	25.47			
+ ID2= 2 (5001):	490.24	5.140	8.75	51.73			
ID = 3 (8501):	1120.57	13.891	8.50	36.96			

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CIN (6002) |
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2.1) ----->			
Distance	Elevation	Manning	
0.00	235.70	0.0700	
15.10	235.42	0.0700	
30.21	235.01	0.0700	
37.76	234.49	0.0700	
45.31	231.43	0.0700	
56.64	231.55	0.0700	
98.18	229.87	0.0700	
117.06	229.64	0.0700	
124.61	229.03	0.0700 / 0.0300	Main Channel
128.38	228.69	0.0300	Main Channel
132.16	228.58	0.0300	Main Channel
143.49	228.98	0.0300 / 0.0700	Main Channel
158.59	229.36	0.0700	
181.25	229.58	0.0700	
200.13	229.96	0.0700	
237.89	231.19	0.0700	
268.09	232.60	0.0700	
302.08	233.40	0.0700	
332.29	234.25	0.0700	
373.82	235.68	0.0700	

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.20	228.78	.639E+04	0.3	0.26	359.77
0.40	228.98	.222E+05	1.6	0.40	227.92
0.79	229.37	.842E+05	9.7	0.63	145.25
1.19	229.77	.218E+06	25.5	0.65	142.16
1.58	230.16	.441E+06	53.8	0.67	136.74
1.98	230.56	.715E+06	94.9	0.73	125.69
2.37	230.95	.104E+07	148.9	0.79	116.12
2.76	231.34	.141E+07	217.2	0.85	107.88
3.16	231.74	.183E+07	299.4	0.90	101.81
3.55	232.13	.228E+07	403.8	0.97	94.28
3.95	232.53	.276E+07	523.3	1.04	87.91
4.34	232.92	.326E+07	650.4	1.10	83.62
4.74	233.32	.380E+07	792.5	1.15	79.99
5.13	233.71	.438E+07	954.7	1.20	76.48
5.52	234.10	.499E+07	1134.7	1.25	73.30
5.92	234.50	.563E+07	1334.6	1.31	70.34
6.31	234.89	.631E+07	1540.4	1.35	68.23
6.71	235.29	.702E+07	1746.8	1.37	67.00
7.10	235.68	.780E+07	1958.9	1.38	66.34

<---- hydrograph ----> <-pipe / channel-->						
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8501)	1120.57	13.89	8.50	36.96	0.90	0.64
OUTFLOW: ID= 1 (6002)	1120.57	9.98	10.50	36.96	0.80	0.63

CALIB	Area (ha)	Curve Number (CN)=
NASHVD (0002)	869.77	58.4
ID= 1 DT=15.0 min	Ia (mm)= 2.50	# of Linear Res. (N)= 3.50
	U.H. Tp (hrs)= 3.32	

Unit Hyd Qpeak (cms)= 11.293

PEAK FLOW (cms)= 8.299 (i)
 TIME TO PEAK (hrs)= 9.750
 RUNOFF VOLUME (mm)= 23.650
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.293

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8200)	1	2	3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0002):	869.77	8.299	9.75	23.65			
+ ID2= 2 (6002):	1120.57	9.981	10.50	36.96			
ID = 3 (8200):	1990.34	18.054	10.25	31.14			

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CIN (6003) |
IN= 2--> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3.1) ----->			
Distance	Elevation	Manning	
0.00	234.33	0.0700	
50.41	229.32	0.0700	
94.52	227.15	0.0700	
113.42	226.02	0.0700	
116.57	225.86	0.0700	
125.32	224.53	0.0700	
130.32	224.24	0.0700 / 0.0300	Main Channel
130.82	223.78	0.0300	Main Channel
131.82	223.63	0.0300	Main Channel
132.42	223.79	0.0300	Main Channel
133.42	224.43	0.0300 / 0.0700	Main Channel
163.83	226.19	0.0700	
189.03	227.19	0.0700	
229.99	228.02	0.0700	
270.95	228.65	0.0700	
286.70	229.01	0.0700	
305.61	229.31	0.0700	

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.20	228.83	.564E+03	0.1	0.26	175.53
0.41	224.04	.168E+04	0.3	0.44	105.08
0.61	224.24	.309E+04	0.6	0.56	82.17
0.93	224.56	.847E+04	1.8	0.57	80.17
1.24	224.87	.209E+05	4.1	0.54	84.69
1.56	225.19	.399E+05	8.0	0.55	83.36
1.88	225.51	.654E+05	13.7	0.57	79.74
2.19	225.82	.976E+05	21.5	0.61	75.55
2.51	226.14	.138E+06	31.1	0.62	73.71
2.83	226.46	.188E+06	43.6	0.64	71.92
3.15	226.78	.250E+06	59.8	0.66	69.63
3.46	227.09	.324E+06	80.5	0.68	67.03
3.78	227.41	.411E+06	103.4	0.69	66.15
4.10	227.73	.516E+06	132.2	0.70	65.13
4.41	228.04	.642E+06	168.2	0.72	63.59
4.73	228.36	.788E+06	210.4	0.73	62.46
5.05	228.68	.959E+06	262.8	0.75	60.81
5.36	228.99	.115E+07	328.9	0.79	58.23
5.68	229.31	.136E+07	401.2	0.81	56.49

<---- hydrograph ----> <-pipe / channel-->						
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8200)	1990.34	18.05	10.25	31.14	2.05	0.59
OUTFLOW: ID= 1 (6003)	1990.34	16.05	11.50	31.14	1.97	0.58

CALIB	Area (ha)	Curve Number (CN)=
STANDHYD (0031)	6.23	
ID= 1 DT=15.0 min	Total Imp(%)= 65.00	Dir. Conn.(%)= 47.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	4.05	2.18
Dep. Storage (mm)	0.80	2.50
Average Slope (%)	1.00	2.00
Length (m)	203.80	40.00
Mannings n	0.013	0.250

Max.Eff.Inten.(mm/hr)=	106.66	63.23
over (min)	15.00	15.00
Storage Coeff. (min)=	3.82 (ii)	12.29 (ii)
Unit Hyd. Tpeak (min)=	15.00	15.00
Unit Hyd. peak (cms)=	0.11	0.08

	TOTALS
PEAK FLOW (cms)=	0.86 0.29 1.147 (iii)
TIME TO PEAK (hrs)=	6.00 6.00 6.00
RUNOFF VOLUME (mm)=	80.00 27.55 52.36
TOTAL RAINFALL (mm)=	80.80 80.80 80.80
RUNOFF COEFFICIENT =	0.99 0.34 0.65

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 53.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5003) |
IN= 2--> OUT= 1 |

DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	0.4170	0.1860
	0.0210	0.0993	0.5230	0.2047
	0.1120	0.1162	0.6090	0.2231
	0.2250	0.1402	1.0090	0.2331
	0.3120	0.1619	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0031)	6.230	1.147	6.00	52.36
OUTFLOW : ID= 1 (5003)	6.230	0.352	6.25	52.17

PEAK FLOW REDUCTION [Qout/Qin](%)	TIME SHIFT OF PEAK FLOW (min)	MAXIMUM STORAGE USED (ha.m.)
30.70	15.00	0.1751

CALIB	Area (ha)	Curve Number (CN)
NASHVD (0032)	461.62	57.8
ID= 1 DT=15.0 min	Ia (mm)= 2.50	# of Linear Res.(N)= 3.50
	U.H. Tp(hrs)= 2.40	

Unit Hyd Qpeak (cms)	PEAK FLOW (cms)	TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
8.267	5.549 (1)	8.500	23.246	80.800	0.288

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8503)	Area (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0032):	461.62	5.549	8.50	23.25
+ ID2= 2 (5003):	6.23	0.352	6.25	52.17
ID = 3 (8503):	467.85	5.610	8.50	23.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8202)	Area (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (6003):	1990.34	16.052	11.50	31.14
+ ID2= 2 (8503):	467.85	5.610	8.50	23.63
ID = 3 (8202):	2458.19	19.403	11.00	29.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area (ha)	Total Imp(%)	Dir. Conn.(%)
STANDHYD (0041)	8.66	52.00	22.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	4.50	4.16
Dep. Storage (mm)	0.80	2.50
Average Slope (%)	1.00	2.00
Length (m)	240.28	40.00
Mannings n	0.013	0.250

	Max.Eff.Inten.(mm/hr)	over (min)	Storage Coeff. (min)	Unit Hyd. peak (cms)	*TOTALS*
	106.66	15.00	4.21 (ii)	0.11	1.131 (iii)
PEAK FLOW (cms)	0.55	0.58			
TIME TO PEAK (hrs)	6.00	6.00			
RUNOFF VOLUME (mm)	80.00	27.31			
TOTAL RAINFALL (mm)	80.80	80.80			
RUNOFF COEFFICIENT	0.99	0.34			

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 50.9 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5004)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1	0.0000	0.0000	0.5790	0.2368
DT= 15.0 min	0.0290	0.1168	0.7270	0.2669
	0.1680	0.1367	0.8470	0.2965
	0.3120	0.1706	1.2470	0.3065
	0.4330	0.2010	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (0041)	8.660	1.131	6.00	38.90
OUTFLOW : ID= 1 (5004)	8.660	0.314	6.25	38.79

PEAK FLOW REDUCTION [Qout/Qin](%)	TIME SHIFT OF PEAK FLOW (min)	MAXIMUM STORAGE USED (ha.m.)
27.72	15.00	0.1729

CALIB	Area (ha)	Curve Number (CN)
NASHVD (0042)	376.06	53.3
ID= 1 DT=15.0 min	Ia (mm)= 2.50	# of Linear Res.(N)= 3.50
	U.H. Tp(hrs)= 1.62	

Unit Hyd Qpeak (cms)	PEAK FLOW (cms)	TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
9.964	5.297 (1)	7.500	20.380	80.800	0.252

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8504)	Area (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0042):	376.06	5.297	7.50	20.38
+ ID2= 2 (5004):	8.66	0.314	6.25	38.79
ID = 3 (8504):	384.72	5.435	7.50	20.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8204)	Area (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8202):	2458.19	19.403	11.00	29.71
+ ID2= 2 (8504):	384.72	5.435	7.50	20.79
ID = 3 (8204):	2842.91	21.251	10.50	28.51

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CH (6005)	Routing time step (min)
IN= 2--> OUT= 1	15.00

Distance	Elevation	Manning
0.00	225.02	0.0400
20.58	224.79	0.0400
35.54	223.93	0.0400
48.64	223.09	0.0400
59.86	222.15	0.0400
67.35	221.11	0.0400
76.41	219.63	0.0400 / 0.0350
82.81	218.53	0.0350
85.41	218.50	0.0350
86.78	218.50	0.0350
88.15	218.50	0.0350
93.45	218.71	0.0350
103.45	218.94	0.0350 / 0.0400
109.45	219.25	0.0400
114.11	220.15	0.0400
123.47	221.73	0.0400
136.56	223.41	0.0400
153.40	224.15	0.0400
170.24	224.55	0.0400
185.20	224.54	0.0400

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV TIME (min)
0.22	218.72	.640E+04	0.5	0.28	204.26
0.44	218.94	.198E+05	2.2	0.39	147.20

0.77	219.27	.507E+05	9.3	0.63	91.02
1.10	219.60	.880E+05	20.9	0.82	70.26
1.43	219.93	.130E+06	37.7	1.00	57.26
1.76	220.26	.175E+06	59.1	1.16	49.43
2.09	220.59	.226E+06	85.1	1.30	44.19
2.42	220.92	.280E+06	115.8	1.43	40.35
2.75	221.25	.340E+06	151.3	1.54	37.40
3.08	221.58	.404E+06	191.9	1.64	35.06
3.40	221.90	.473E+06	237.5	1.73	33.19
3.73	222.23	.548E+06	288.1	1.82	31.68
4.06	222.56	.629E+06	344.1	1.89	30.48
4.39	222.89	.718E+06	406.8	1.96	29.43
4.72	223.22	.815E+06	476.1	2.02	28.52
5.05	223.55	.920E+06	547.9	2.06	28.00
5.38	223.88	1.04E+07	624.5	2.07	27.74
5.71	224.21	1.17E+07	710.0	2.09	27.54
6.04	224.54	1.33E+07	800.8	2.08	27.62

331.25	226.04	0.0700
351.20	227.76	0.0700
371.16	227.49	0.0700
395.10	228.55	0.0700

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.23	219.46	.112E+04	0.3	0.16	375.56
0.47	219.70	.471E+04	0.3	0.22	277.80
0.84	220.07	.267E+05	2.4	0.34	184.45
1.20	220.43	.626E+05	7.4	0.44	140.97
1.57	220.80	1.10E+06	15.6	0.52	117.94
1.93	221.16	1.65E+06	27.4	0.61	100.71
2.30	221.53	2.25E+06	42.6	0.70	88.16
2.66	221.89	2.90E+06	60.9	0.78	79.20
3.03	222.26	3.58E+06	83.4	0.85	72.44
3.39	222.62	4.31E+06	106.0	0.91	67.79
3.76	222.99	5.13E+06	131.5	0.95	65.01
4.12	223.35	6.08E+06	149.2	0.98	70.14
4.49	223.72	7.88E+06	191.4	0.90	68.58
4.85	224.08	9.74E+06	232.1	0.88	69.94
5.22	224.45	1.20E+07	283.2	0.88	70.45
5.58	224.81	1.47E+07	347.5	0.88	70.32
5.95	225.18	1.78E+07	427.9	0.89	69.46
6.31	225.54	2.15E+07	523.9	0.90	68.38
6.68	225.91	2.57E+07	638.4	0.92	67.05

----- hydrograph ----- <-pipe / channel->
 AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8204) 2842.91 21.25 10.50 28.51 1.11 0.82
 OUTFLOW : ID= 1 (6005) 2842.91 20.36 11.25 28.51 1.08 0.81

----- hydrograph ----- <-pipe / channel->
 AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0006) 1375.77 10.48 10.50 21.43 1.34 0.47
 OUTFLOW : ID= 1 (6007) 1375.77 8.68 12.25 21.43 1.26 0.45

 CALIB (0005) | Area (ha)= 340.11 | Curve Number (CN)= 69.5
 NASHVD (0005) | Ia (mm)= 2.50 | # of Linear Res.(N)= 3.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 2.45

 Unit Hyd Opeak (cms)= 5.972

 PEAK FLOW (cms)= 5.667 (i)
 TIME TO PEAK (hrs)= 8.500
 RUNOFF VOLUME (mm)= 32.308
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.400
 (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB (0071) | Area (ha)= 16.47 | Dir. Com.(%)= 17.00
 STASHVD (0071) | Total Imp(%)= 47.00
ID= 1 DT=15.0 min

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	8.74	8.73
Dep. Storage (mm)	0.80	2.50
Average Slope (%)	1.00	2.00
Length (m)	331.36	40.00
Mannings n	0.013	0.250
Max. Eff. Inten.(mm/hr)	106.66	85.16
over (min)	15.00	15.00
Storage Coeff. (min)	5.11 (ii)	12.63 (iii)
Unit Hyd. Tpeak (min)	15.00	15.00
Unit Hyd. peak (cms)	0.11	0.08

 ADD HYD (8206) |
1 * 2 = 3
 ID= 1 (0005): 340.11 5.667 8.50 32.31
 + ID= 2 (6005): 2842.91 20.362 11.25 28.51

 ID = 3 (8206): 3183.02 23.788 11.00 28.91

 NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 PEAK FLOW (cms)= 0.80 1.55 2.355 (iii)
 TIME TO PEAK (hrs)= 6.00 6.00 6.00
 RUNOFF VOLUME (mm)= 80.00 35.87 43.37
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.99 0.44 0.54
 ***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 62.9 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB (0006) | Area (ha)=1375.77 | Curve Number (CN)= 55.0
 NASHVD (0006) | Ia (mm)= 2.50 | # of Linear Res.(N)= 3.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 3.90

 Unit Hyd Opeak (cms)= 15.192

 PEAK FLOW (cms)= 10.483 (i)
 TIME TO PEAK (hrs)= 10.500
 RUNOFF VOLUME (mm)= 21.428
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.265
 (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR (5007) |
 IN= 2--> OUT= 1 |
DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0000	1.1020	0.4341
0.0560	0.2063	1.3830	0.4946	
0.3190	0.2394	1.6100	0.5538	
0.5940	0.3063	2.0100	0.5538	
0.8240	0.3645	0.0000	0.0000	

INFLOW : ID= 2 (0071) 16.470 2.355 6.00 43.37
 OUTFLOW : ID= 1 (5007) 16.470 0.706 6.50 43.32

 PEAK FLOW REDUCTION [Qout/Qin](%)= 29.96
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 0.3407

 ROUTE CHN (6007) |
 IN= 2--> OUT= 1 | Routing time step (min)= 15.00

 <----- DATA FOR SECTION (7.1) ----->

Distance	Elevation	Manning
0.00	225.91	0.0700
51.88	225.32	0.0700
135.69	224.23	0.0700
183.58	223.08	0.0700
211.52	223.28	0.0700
223.49	222.51	0.0700
231.43	220.69	0.0700
244.53	219.78	0.0700 / 0.0500 Main Channel
249.23	219.60	0.0500 Main Channel
249.93	219.46	0.0500 Main Channel
251.43	219.23	0.0500 Main Channel
253.50	219.70	0.0500 / 0.0700 Main Channel
261.10	219.80	0.0700
271.38	221.01	0.0700
283.36	223.77	0.0700
295.33	224.21	0.0700

 CALIB (0072) | Area (ha)= 539.30 | Curve Number (CN)= 64.2
NASHVD (0072)

|ID= 1 DT=15.0 min | Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
 U.H. Tp(hrs)= 2.74

Unit Hyd Qpeak (cms)= 8.477

PEAK FLOW (cms)= 7.072 (i)
 TIME TO PEAK (hrs)= 9.000
 RUNOFF VOLUME (mm)= 27.876
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.345

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8507) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8007): 539.30 7.072 9.00 27.88
 + ID2= 2 (5007): 16.47 0.706 6.50 43.32
 =====
 ID = 3 (8507): 555.77 7.177 9.00 28.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8210) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (6007): 1375.77 8.678 12.25 21.43
 + ID2= 2 (8507): 555.77 7.177 9.00 28.33
 =====
 ID = 3 (8210): 1931.54 13.384 10.75 23.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8208) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8206): 3183.02 23.788 11.00 28.91
 + ID2= 2 (8210): 1931.54 13.384 10.75 23.41
 =====
 ID = 3 (8208): 5114.56 37.161 10.75 26.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8212) |
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8208): 5114.56 37.161 10.75 26.84
 + ID2= 2 (8214): 875.75 12.226 8.50 26.60
 =====
 ID = 3 (8212): 5990.31 45.539 10.25 26.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6010) |
 IIR 2--> ODF 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (10.1) ----->

Distance	Elevation	Manning	
0.00	223.78	0.0400	
10.97	224.70	0.0400	
60.32	222.23	0.0400	
104.19	219.90	0.0400	
126.13	219.00	0.0400	
134.36	218.90	0.0400 /0.0330	Main Channel
137.10	218.41	0.0330	Main Channel
139.84	218.30	0.0330	Main Channel
142.58	216.76	0.0330	Main Channel
145.32	216.74	0.0330	Main Channel
148.07	216.71	0.0330	Main Channel
150.81	217.11	0.0330	Main Channel
153.55	218.40	0.0330	Main Channel
156.29	219.01	0.0330 /0.0400	Main Channel
159.03	219.52	0.0400	
167.26	219.89	0.0400	
170.00	220.09	0.0400	
183.71	221.31	0.0400	
200.16	223.39	0.0400	
271.45	224.04	0.0400	

<----- TRAVEL TIME TABLE ----->
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME

(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.36	217.07	680E+04	0.9	0.36	129.75
0.73	217.44	165E+05	3.4	0.58	81.12
1.09	217.81	278E+05	7.3	0.75	63.18
1.46	218.17	405E+05	15.6	0.88	53.42
1.83	218.54	561E+05	17.7	0.90	52.70
2.19	218.90	764E+05	26.2	0.97	48.52
2.64	219.35	118E+06	44.8	1.07	44.04
3.08	219.79	180E+06	71.5	1.13	41.83
3.53	220.24	264E+06	109.3	1.17	40.26
3.97	220.68	366E+06	159.0	1.23	38.34
4.42	221.13	485E+06	221.7	1.30	36.43
4.87	221.58	620E+06	298.8	1.37	34.58
5.31	222.02	771E+06	391.1	1.44	32.85
5.76	222.47	937E+06	498.7	1.51	31.30
6.21	222.92	112E+07	622.7	1.58	29.93
6.65	223.36	132E+07	764.2	1.65	28.69
7.10	223.81	155E+07	878.9	1.60	29.46
7.54	224.25	186E+07	1057.1	1.61	29.35
7.99	224.70	220E+07	1291.1	1.67	28.35

<----- hydrograph -----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8212) 5990.31 45.53 10.25 26.80 2.65 1.07
 OUTFLOW: ID= 1 (6010) 5990.31 44.25 10.75 26.80 2.62 1.07

 CALIB |
 STANDBYD (0102) | Area (ha)= 211.49
 ID= 1 DT=15.0 min | Total Imp(%)= 47.00 Dir. Conn.(%)= 26.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 99.40 112.09
 Dep. Storage (mm)= 0.80 2.50
 Average Slope (%)= 1.00 2.00
 Length (m)= 1187.41 40.00
 Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 106.66 58.78
 over (min) 15.00 30.00
 Storage Coeff. (min)= 10.99 (ii) 19.72 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.05

 PEAK FLOW (cms)= 13.35 9.33 19.305 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 80.00 28.05 41.56
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.99 0.35 0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 55.9 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB |
 STANDBYD (0101) | Area (ha)= 133.27
 ID= 1 DT=15.0 min | Total Imp(%)= 47.00 Dir. Conn.(%)= 26.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 62.64 70.63
 Dep. Storage (mm)= 0.80 2.50
 Average Slope (%)= 1.00 2.00
 Length (m)= 942.59 40.00
 Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 106.66 58.78
 over (min) 15.00 30.00
 Storage Coeff. (min)= 9.27 (ii) 18.30 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.05

 PEAK FLOW (cms)= 8.78 6.07 12.665 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 80.00 28.05 41.56
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.99 0.35 0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 55.9 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5010)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	6.8040	3.7679
1.3970	1.4119	8.2460	4.4558
3.2730	2.3774	10.2760	4.7276
5.2480	3.0364	10.6760	4.7376

AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
INFLOW : ID= 2 (0101) 133.270 12.665 6.00 41.56
OUTFLOW : ID= 1 (5010) 133.270 3.789 6.75 41.56

PEAK FLOW REDUCTION [Qout/Gin] (%) = 29.92
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha.m.) = 2.5674

ADD HYD (8216)

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0102):	211.49	19.305	6.00	41.56
+ ID2= 2 (5010):	133.27	3.789	6.75	41.56
ID = 3 (8216):	344.76	20.614	6.00	41.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8218)

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0101):	5990.31	44.253	10.75	26.80
+ ID2= 2 (8216):	344.76	20.614	6.00	41.56
ID = 3 (8218):	6335.07	46.059	10.75	27.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

** SIMULATION NUMBER: 6 **

READ STORM

Filename: C:\Users\jascott\AppData\Local\Temp\ad939110-f7bb-4285-9226-bce91a941061\7d312e97
Comments: 100-Year 12-Hour SCS II Design Storm

TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)
0.25	2.21	3.25	3.54	6.25	15.93	9.25	3.10
0.50	2.21	3.50	3.54	6.50	15.93	9.50	3.10
0.75	2.21	3.75	3.54	6.75	7.08	9.75	3.10
1.00	2.21	4.00	3.54	7.00	7.08	10.00	3.10
1.25	2.21	4.25	5.31	7.25	5.31	10.25	1.77
1.50	2.21	4.50	5.31	7.50	5.31	10.50	1.77
1.75	2.21	4.75	7.08	7.75	5.31	10.75	1.77
2.00	2.21	5.00	7.08	8.00	5.31	11.00	1.77
2.25	2.65	5.25	10.62	8.25	3.10	11.25	1.77
2.50	2.65	5.50	10.62	8.50	3.10	11.50	1.77
2.75	2.65	5.75	42.48	8.75	3.10	11.75	1.77
3.00	2.65	6.00	116.82	9.00	3.10	12.00	1.77

CALIB

STANDHYD (0081) Area (ha) = 10.53
ID= 1 DT=15.0 min Total Imp(%) = 45.00 Dir. Conn.(%) = 15.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 4.74 5.79
Dep. Storage (mm) = 0.80 2.50
Average Slope (%) = 1.00 2.00
Length (m) = 264.95 40.00
Mannings n = 0.013 0.250
Max. Eff. Inten. (mm/hr) = 116.82 93.61
over (min) = 15.00 15.00
Storage Coeff. (min) = 4.31 (ii) 11.55 (ii)
Unit Hyd. Tpeak (min) = 15.00 15.00
Unit Hyd. Tpeak (cms) = 0.11 0.08
TOTALS
PEAK FLOW (cms) = 0.50 1.18 1.679 (iii)
TIME TO PEAK (hrs) = 6.00 6.00 6.00
RUNOFF VOLUME (mm) = 87.70 39.96 47.12

TOTAL RAINFALL (mm) = 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.99 0.45 0.53

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 61.7 Is = Dep. Storage (ABOVE)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5008)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.7050	0.2743
0.0360	0.1287	0.8850	0.3136
0.2040	0.1489	1.0300	0.3521
0.3800	0.1922	1.4300	0.3621
0.5270	0.2294	0.0000	0.0000

AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
INFLOW : ID= 2 (0081) 10.530 1.679 6.00 47.12
OUTFLOW : ID= 1 (5008) 10.530 0.518 6.25 47.04

PEAK FLOW REDUCTION [Qout/Gin] (%) = 30.83
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha.m.) = 0.2299

CALIB

NASHYD (0082) Area (ha) = 402.65 Curve Number (CN) = 63.3
ID= 1 DT=15.0 min Ia (mm) = 2.50 # of Linear Res. (N) = 3.50
U.H. Tp (hrs) = 1.63

Unit Hyd Qpeak (cms) = 10.622

PEAK FLOW (cms) = 8.941 (i)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 31.709
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.358

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8508)

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
+ ID2= 1 (0082):	402.65	8.941	7.50	31.71
+ ID2= 2 (5008):	10.53	0.518	6.25	47.04
ID = 3 (8508):	413.18	9.181	7.50	32.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6009)

IN= 2--> OUT= 1 Routing time step (min) = 15.00

----- DATA FOR SECTION (9.1) -----

Distance	Elevation	Manning
0.00	232.41	0.0600
16.80	232.21	0.0600
28.00	231.55	0.0600
117.61	229.36	0.0600
235.21	228.08	0.0600
296.81	226.96	0.0600
316.00	226.17	0.0600
326.00	226.17	0.0600
332.00	226.04	0.0600 / 0.0350
335.50	225.16	0.0350
336.00	225.08	0.0350
337.00	225.08	0.0350
337.90	225.21	0.0350
339.90	225.93	0.0350 / 0.0600
345.00	226.28	0.0600
358.42	226.57	0.0600
403.22	227.73	0.0600
448.02	229.07	0.0600
492.82	230.51	0.0600
554.43	232.28	0.0600

----- TRAVEL TIME TABLE -----
DEPTH (m) ELEV (m) VOLUME (cu.m.) FLOW RATE (cms) VELOCITY (m/s) TRAV. TIME (min)

0.28	225.36	.255E+04	0.3	0.37	160.52
0.57	225.65	.719E+04	1.1	0.55	107.34
0.85	225.93	.138E+05	2.7	0.70	85.08
1.25	226.33	.405E+05	7.8	0.69	86.59
1.64	226.72	.109E+06	20.0	0.65	91.15
2.04	227.12	.216E+06	41.2	0.68	87.34
2.44	227.52	.372E+06	74.8	0.72	82.92
2.83	227.91	.580E+06	125.6	0.77	77.03
3.23	228.31	.842E+06	193.5	0.82	72.57
3.63	228.71	.117E+07	284.7	0.87	68.66
4.02	229.10	.157E+07	405.4	0.92	64.71
4.42	229.50	.204E+07	564.5	0.99	60.30
4.82	229.90	.256E+07	765.1	1.07	55.64
5.22	230.30	.311E+07	1001.2	1.15	51.82
5.61	230.69	.371E+07	1269.8	1.22	48.68
6.01	231.09	.435E+07	1573.3	1.29	46.05
6.41	231.49	.503E+07	1914.5	1.36	43.77
6.80	231.88	.575E+07	2314.0	1.44	41.39
7.20	232.28	.650E+07	2741.3	1.50	39.49

<---- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8508) 413.18 9.18 7.50 32.10 1.29 0.68
 OUTFLOW: ID= 1 (6009) 413.18 6.72 8.75 32.10 1.16 0.69

CALIB
 STANHYD (0091) | Area (ha)= 11.32
 ID= 1 DT=15.0 min | Total Imp(%)= 50.00 Dir. Conn.(%)= 20.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 5.66 5.66
 Dep. Storage (mm)= 0.80 2.50
 Average Slope (%)= 1.00 2.00
 Length (m)= 274.71 40.00
 Mannings n = 0.013 0.250
 Max.Eff.Inten.(mm/hr)= 116.82 94.31
 over (min) 15.00 15.00
 Storage Coeff. (min)= 4.40 (ii) 11.63 (ii)
 Unit Hyd. Tpeak (min)= 15.00 15.00
 Unit Hyd. peak (cms)= 0.11 0.08

****TOTALS*
 PEAK FLOW (cms)= 0.72 1.15 1.874 (iii)
 TIME TO PEAK (hrs)= 6.00 6.00 6.00
 RUNOFF VOLUME (mm)= 87.70 38.94 48.69
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.99 0.44 0.55

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5009)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.7570	0.3365
0.0380	0.1497	0.9510	0.3465
0.2190	0.1748	1.1070	0.3857
0.4090	0.2196	1.5070	0.3957
0.5670	0.2594	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
11.320	1.874	6.00	48.69
11.320	0.571	6.25	48.61

PEAK FLOW REDUCTION [Qout/Qin](%)= 30.49
 TIME SHIFT OF PEAK FLOW (min)= 15.00
 MAXIMUM STORAGE USED (ha.m.)= 0.2623

CALIB
 NASHYD (0092) | Area (ha)= 451.25 Curve Number (CN)= 60.8
 ID= 1 DT=15.0 min | Ia (mm)= 2.50 # of Linear Res. (N)= 3.50
 U.H. Tp(hrs)= 2.11

Unit Hyd Qpeak (cms)= 9.199

PEAK FLOW (cms)= 7.670 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 29.613

TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.335

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8509)
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0092): 451.25 7.670 8.25 29.61
 + ID2= 2 (5009): 11.32 0.571 6.25 48.61
 ID= 3 (8509): 462.57 7.804 8.25 30.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8214)
 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (6009): 413.18 6.718 8.75 32.10
 + ID2= 2 (8509): 462.57 7.804 8.25 30.08
 ID= 3 (8214): 875.75 14.300 8.50 31.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANHYD (8011) | Area (ha)= 2.54
 ID= 1 DT=15.0 min | Total Imp(%)= 45.00 Dir. Conn.(%)= 15.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 1.14 1.40
 Dep. Storage (mm)= 0.80 2.50
 Average Slope (%)= 1.00 2.00
 Length (m)= 130.13 40.00
 Mannings n = 0.013 0.250
 Max.Eff.Inten.(mm/hr)= 116.82 88.14
 over (min) 15.00 15.00
 Storage Coeff. (min)= 2.81 (ii) 10.24 (ii)
 Unit Hyd. Tpeak (min)= 15.00 15.00
 Unit Hyd. peak (cms)= 0.11 0.09

****TOTALS*
 PEAK FLOW (cms)= 0.12 0.28 0.402 (iii)
 TIME TO PEAK (hrs)= 6.00 6.00 6.00
 RUNOFF VOLUME (mm)= 87.70 37.71 45.21
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.99 0.43 0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANHYD (0019) | Area (ha)= 487.70
 ID= 1 DT=15.0 min | Total Imp(%)= 45.00 Dir. Conn.(%)= 35.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 219.46 268.24
 Dep. Storage (mm)= 0.80 5.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 1803.15 40.00
 Mannings n = 0.013 0.250
 Max.Eff.Inten.(mm/hr)= 116.82 77.17
 over (min) 15.00 30.00
 Storage Coeff. (min)= 13.61 (ii) 21.44 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.05

****TOTALS*
 PEAK FLOW (cms)= 41.99 28.32 60.283 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 87.70 42.11 58.19
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.99 0.48 0.66

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
RESERVOIR (8511)
IN= 2--> OUT= 1
DT= 15.0 min
-----
      OUTFLOW STORAGE | OUTFLOW STORAGE
      (cms) (ha.m.) | (cms) (ha.m.)
-----
0.0000 0.0000 | 24.3900 10.5911
1.6600 5.9307 | 32.6300 12.6704
9.4600 6.8569 | 40.9700 14.4958
17.6100 8.8657 | 47.7000 16.2820
-----
      AREA QPEAK TPEAK R.V.
      (ha) (cms) (hrs) (mm)
-----
INFLOW : ID= 2 (0019) 487.700 60.283 6.00 58.19
OUTFLOW : ID= 1 (8511) 487.700 28.752 6.50 58.19
-----
      PEAK FLOW REDUCTION [Qout/Qin](%) = 47.70
      TIME SHIFT OF PEAK FLOW (min) = 30.00
      MAXIMUM STORAGE USED (ha.m.) = 11.7540
  
```

```

-----
ADD HYD (8510)
1 + 2 = 3
-----
      AREA QPEAK TPEAK R.V.
      (ha) (cms) (hrs) (mm)
-----
ID1= 1 (0011): 2.54 0.402 6.00 45.21
+ ID2= 2 (8511): 487.70 28.752 6.50 58.19
=====
ID = 3 (8510): 490.24 28.838 6.50 58.13
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
RESERVOIR (5001)
IN= 2--> OUT= 1
DT= 15.0 min
-----
      OUTFLOW STORAGE | OUTFLOW STORAGE
      (cms) (ha.m.) | (cms) (ha.m.)
-----
0.0000 0.0000 | 4.6830 11.8298
0.0540 0.1960 | 5.4380 13.3066
1.2810 6.8008 | 6.1990 14.9402
2.5090 8.6937 | 6.5990 14.9502
3.4630 9.9865 | 0.0000 0.0000
-----
      AREA QPEAK TPEAK R.V.
      (ha) (cms) (hrs) (mm)
-----
INFLOW : ID= 2 (8510) 490.240 28.838 6.50 58.13
OUTFLOW : ID= 1 (5001) 490.240 6.070 8.75 58.12
-----
      PEAK FLOW REDUCTION [Qout/Qin](%) = 21.05
      TIME SHIFT OF PEAK FLOW (min) = 135.00
      MAXIMUM STORAGE USED (ha.m.) = 14.6955
  
```

```

-----
CALIB (0012)
Area (ha) = 630.33 Curve Number (CN) = 61.0
ID= 1 DT=15.0 min | Ia (mm) = 2.50 # of Linear Res.(N) = 3.50
U.H. Tp(hrs) = 2.24
-----
Unit Hyd Qpeak (cms) = 12.094
-----
PEAK FLOW (cms) = 10.278 (1)
TIME TO PEAK (hrs) = 8.250
RUNOFF VOLUME (mm) = 29.776
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.336
  
```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
ADD HYD (8501)
1 + 2 = 3
-----
ID1= 1 (0012): 630.33 10.278 8.25 29.78
+ ID2= 2 (5001): 490.24 6.070 8.75 58.12
=====
ID = 3 (8501): 1120.57 16.320 8.50 42.18
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ROUTE CHN (6002)
IN= 2--> OUT= 1
Routing time step (min)'= 15.00
  
```

```

<----- DATA FOR SECTION ( 2.1) ----->
Distance Elevation Manning
  
```

```

0.00 235.70 0.0700
15.10 235.42 0.0700
30.21 235.01 0.0700
37.76 234.49 0.0700
45.31 231.43 0.0700
56.64 231.55 0.0700
98.18 229.87 0.0700
117.06 229.64 0.0700
124.61 229.33 0.0700 /0.0300 Main Channel
128.38 228.69 0.0300 Main Channel
132.16 228.58 0.0300 Main Channel
143.49 228.98 0.0300 /0.0700 Main Channel
158.59 229.36 0.0700
181.25 229.58 0.0700
200.13 229.96 0.0700
237.89 231.19 0.0700
268.09 232.60 0.0700
302.08 233.40 0.0700
332.29 234.25 0.0700
373.82 235.68 0.0700
  
```

```

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME
(m) (m) (cu.m.) (cms) (m/s) (min)
-----
0.20 228.78 6.39E+04 0.3 0.26 359.77
0.40 228.98 2.22E+05 1.6 0.40 227.92
0.79 229.37 8.42E+05 9.7 0.63 145.25
1.19 229.77 2.18E+06 25.5 0.65 142.16
1.58 230.16 4.41E+06 53.8 0.67 136.74
1.98 230.56 7.15E+06 94.9 0.73 125.69
2.37 230.95 1.04E+07 148.9 0.79 116.12
2.76 231.34 1.41E+07 217.2 0.85 107.88
3.16 231.74 1.83E+07 299.4 0.90 101.81
3.55 232.13 2.28E+07 403.8 0.97 94.28
3.95 232.53 2.76E+07 523.3 1.04 87.91
4.34 232.92 3.26E+07 650.4 1.10 83.62
4.74 233.32 3.80E+07 792.5 1.15 79.99
5.13 233.71 4.38E+07 954.7 1.20 76.48
5.52 234.10 4.99E+07 1134.7 1.25 73.30
5.92 234.50 5.63E+07 1334.6 1.31 70.34
6.31 234.89 6.31E+07 1540.4 1.35 68.23
6.71 235.29 7.02E+07 1746.8 1.37 67.00
7.10 235.68 7.80E+07 1958.9 1.38 66.34
  
```

```

<----- hydrograph -----> <-pipe / channel->
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
-----
INFLOW : ID= 2 (8501) 1120.57 16.32 8.50 42.18 0.96 0.64
OUTFLOW : ID= 1 (6002) 1120.57 11.62 10.75 42.18 0.84 0.63
  
```

```

-----
CALIB (0002)
Area (ha) = 869.77 Curve Number (CN) = 58.4
ID= 1 DT=15.0 min | Ia (mm) = 2.50 # of Linear Res.(N) = 3.50
U.H. Tp(hrs) = 3.32
  
```

```

Unit Hyd Qpeak (cms) = 11.293
-----
PEAK FLOW (cms) = 9.737 (1)
TIME TO PEAK (hrs) = 9.750
RUNOFF VOLUME (mm) = 27.708
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.313
  
```

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
ADD HYD (8200)
1 + 2 = 3
-----
ID1= 1 (0002): 869.77 9.737 9.75 27.71
+ ID2= 2 (6002): 1120.57 11.624 10.75 42.18
=====
ID = 3 (8200): 1990.34 21.077 10.25 35.85
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
ROUTE CHN (6003)
IN= 2--> OUT= 1
Routing time step (min)'= 15.00
  
```

```

<----- DATA FOR SECTION ( 3.1) ----->
Distance Elevation Manning
0.00 234.33 0.0700
50.41 229.32 0.0700
94.52 227.15 0.0700
113.42 226.02 0.0700
116.57 225.86 0.0700
125.32 224.53 0.0700
130.32 224.24 0.0700 /0.0300 Main Channel
  
```

130.82	223.78	0.0300	Main Channel
131.82	223.63	0.0300	Main Channel
132.42	223.79	0.0300	Main Channel
133.42	224.43	0.0300 / 0.0700	Main Channel
163.83	226.19	0.0700	
189.03	227.19	0.0700	
229.99	228.02	0.0700	
270.95	228.65	0.0700	
286.70	229.31	0.0700	
305.61	229.31	0.0700	

TRAVEL TIME TABLE					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.20	223.83	.564E+03	0.1	0.26	175.53
0.41	224.04	.168E+04	0.3	0.44	105.08
0.61	224.24	.309E+04	0.6	0.56	82.17
0.93	224.56	.847E+04	1.8	0.57	80.17
1.24	224.87	.209E+05	4.1	0.54	84.69
1.56	225.19	.399E+05	8.0	0.55	83.36
1.88	225.51	.654E+05	13.7	0.57	79.74
2.19	225.82	.976E+05	21.5	0.61	75.55
2.51	226.14	.138E+06	31.1	0.62	73.71
2.83	226.46	.188E+06	43.6	0.64	71.92
3.15	226.78	.250E+06	59.8	0.66	69.63
3.46	227.09	.324E+06	80.5	0.68	67.03
3.78	227.41	.411E+06	103.4	0.69	66.15
4.10	227.73	.516E+06	132.2	0.70	65.13
4.41	228.04	.642E+06	168.2	0.72	63.59
4.73	228.36	.788E+06	210.4	0.73	62.46
5.05	228.68	.959E+06	262.8	0.75	60.81
5.36	228.99	.115E+07	328.9	0.79	58.23
5.68	229.31	.136E+07	401.2	0.81	56.49

<--- hydrograph --->						<--- pipe / channel --->	
	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW :	ID= 2 (8200) 1990.34	21.08	10.25	35.85	2.18	0.60	
OUTFLOW :	ID= 1 (6003) 1990.34	19.03	11.50	35.85	2.09	0.60	

CALIB	STANDHYD (0031)	Area (ha)= 6.23	Dir. Conn.(%)= 47.00
ID= 1 DT=15.0 min	Total Imp(%)= 65.00		

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)=	4.05	2.18	
Dep. Storage (mm)=	0.80	2.50	
Average Slope (%)=	1.00	2.00	
Length (m)=	203.80	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	116.82	73.76	
over (min)=	15.00	15.00	
Storage Coeff. (min)=	3.68 (ii)	11.65 (ii)	
Unit Hyd. Tpeak (min)=	15.00	15.00	
Unit Hyd. peak (cms)=	0.11	0.08	
			TOTALS
PEAK FLOW (cms)=	0.94	0.35	1.286 (iii)
TIME TO PEAK (hrs)=	6.00	6.00	
RUNOFF VOLUME (mm)=	87.70	32.42	58.40
TOTAL RAINFALL (mm)=	88.50	88.50	88.50
RUNOFF COEFFICIENT =	0.99	0.37	0.66

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 53.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5003)					
IN= 2--> OUT= 1					
DT= 15.0 min					
	OUTFLOW	STORAGE	OUTFLOW	STORAGE	
	(cms)	(ha.m.)	(cms)	(ha.m.)	
	0.0000	0.0000	0.4170	0.1860	
	0.0210	0.0993	0.5230	0.2047	
	0.1210	0.1182	0.6090	0.2231	
	0.2250	0.1402	1.0090	0.2331	
	0.3120	0.1619	0.0000	0.0000	
	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
INFLOW :	ID= 2 (0031) 6.230	1.286	6.00	58.40	
OUTFLOW :	ID= 1 (5003) 6.230	0.420	6.25	58.21	
	PEAK FLOW REDUCTION [Qout/Qin](%)=	32.64			
	TIME SHIFT OF PEAK FLOW (min)=	15.00			
	MAXIMUM STORAGE USED (ha.m.)=	0.1922			

CALIB	NASHVD (0032)	Area (ha)= 461.62	Curve Number (CN)= 57.8
ID= 1 DT=15.0 min	Is (mm)= 2.50	# of Linear Res.(N)= 3.50	
	U.H. Tp(hrs)= 2.40		

Unit Hyd Qpeak (cms)= 8.267

PEAK FLOW (cms)= 6.519 (i)

TIME TO PEAK (hrs)= 8.500

RUNOFF VOLUME (mm)= 27.247

TOTAL RAINFALL (mm)= 88.500

RUNOFF COEFFICIENT = 0.308

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8503)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID= 1 (0032):	461.62	6.519	8.50	27.25	
+ ID2= 2 (5003):	6.23	0.420	6.25	58.21	
ID = 3 (8503):	467.85	6.589	8.50	27.66	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8202)					
1 + 2 = 3	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID= 1 (6003):	1990.34	19.032	11.50	35.85	
+ ID2= 2 (8503):	467.85	6.589	8.50	27.66	
ID = 3 (8202):	2458.19	22.919	10.75	34.29	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	STANDHYD (0041)	Area (ha)= 8.66	Dir. Conn.(%)= 22.00
ID= 1 DT=15.0 min	Total Imp(%)= 52.00		

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)=	4.50	4.16	
Dep. Storage (mm)=	0.80	2.50	
Average Slope (%)=	1.00	2.00	
Length (m)=	240.28	40.00	
Mannings n =	0.013	0.250	
Max.Eff.Inten.(mm/hr)=	116.82	77.54	
over (min)=	15.00	15.00	
Storage Coeff. (min)=	4.06 (ii)	11.88 (ii)	
Unit Hyd. Tpeak (min)=	15.00	15.00	
Unit Hyd. peak (cms)=	0.11	0.08	
			TOTALS
PEAK FLOW (cms)=	0.61	0.69	1.298 (iii)
TIME TO PEAK (hrs)=	6.00	6.00	6.00
RUNOFF VOLUME (mm)=	87.70	31.81	44.10
TOTAL RAINFALL (mm)=	88.50	88.50	88.50
RUNOFF COEFFICIENT =	0.99	0.36	0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 50.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5004)					
IN= 2--> OUT= 1					
DT= 15.0 min					
	OUTFLOW	STORAGE	OUTFLOW	STORAGE	
	(cms)	(ha.m.)	(cms)	(ha.m.)	
	0.0000	0.0000	0.5790	0.2368	
	0.0290	0.1168	0.7270	0.2669	
	0.1680	0.1367	0.8470	0.2965	
	0.3120	0.1706	1.2470	0.3065	
	0.4330	0.2010	0.0000	0.0000	
	AREA	QPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
INFLOW :	ID= 2 (0041) 8.660	1.298	6.00	44.10	
OUTFLOW :	ID= 1 (5004) 8.660	0.376	6.25	43.99	
	PEAK FLOW REDUCTION [Qout/Qin](%)=	28.98			
	TIME SHIFT OF PEAK FLOW (min)=	15.00			

MAXIMUM STORAGE USED (ha.m.) = 0.1877

CALIB (0002) Area (ha) = 376.06 Curve Number (CN) = 53.3
ID= 1 DT=15.0 min Ia (mm) = 2.50 # of Linear Res.(N) = 3.50
U.H. Tp(hrs) = 1.62

Unit Hyd Qpeak (cms) = 9.964
PEAK FLOW (cms) = 6.249 (1)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 23.972
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.271

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8504)
1 + 2 = 3
ID1= 1 (0042): 376.06 6.249 7.50 23.97
+ ID2= 2 (5004): 8.66 0.376 6.25 43.99
ID = 3 (8504): 384.72 6.420 7.50 24.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8204)
1 + 2 = 3
ID1= 1 (8202): 2458.29 22.919 10.75 34.29
+ ID2= 2 (8504): 384.72 6.420 7.50 24.42
ID = 3 (8204): 2842.91 25.169 10.50 32.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6005)
IN= 2--> OUT= 1
Routing time step (min)'= 15.00

Table with columns: Distance, Elevation, Manning, and Main Channel. Rows show data points from 0.00 to 185.20.

TRAVEL TIME TABLE
DEPTH (m), ELEV (m), VOLUME (cu.m.), FLOW RATE (cms), VELOCITY (m/s), TRAV.TIME (min)
0.22 218.72 640E+04 0.5 0.28 204.26

hydrograph
AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
INFLOW : ID= 2 (8204) 2842.91 25.17 10.50 32.96 1.16 0.86
OUTFLOW: ID= 1 (6005) 2842.91 24.09 11.25 32.96 1.16 0.85

CALIB (0005) Area (ha) = 340.11 Curve Number (CN) = 69.5
ID= 1 DT=15.0 min Ia (mm) = 2.50 # of Linear Res.(N) = 3.50
U.H. Tp(hrs) = 2.45

Unit Hyd Qpeak (cms) = 5.972
PEAK FLOW (cms) = 6.586 (1)
TIME TO PEAK (hrs) = 8.500
RUNOFF VOLUME (mm) = 37.455
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.423

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8206)
1 + 2 = 3
ID1= 1 (0005): 340.11 6.586 8.50 37.45
+ ID2= 2 (6005): 2842.91 24.091 11.25 32.96
ID = 3 (8206): 3183.02 28.151 10.75 33.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0006) Area (ha)=1375.77 Curve Number (CN)= 55.0
ID= 1 DT=15.0 min Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
U.H. Tp(hrs)= 3.90

Unit Hyd Qpeak (cms)= 15.192
PEAK FLOW (cms)= 12.327 (1)
TIME TO PEAK (hrs)= 10.500
RUNOFF VOLUME (mm)= 25.172
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.284

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6007)
IN= 2--> OUT= 1
Routing time step (min)'= 15.00

Table with columns: Distance, Elevation, Manning, and Main Channel. Rows show data points from 0.00 to 395.10.

TRAVEL TIME TABLE
DEPTH (m), ELEV (m), VOLUME (cu.m.), FLOW RATE (cms), VELOCITY (m/s), TRAV.TIME (min)
0.23 219.46 112E+04 0.0 0.16 375.56

3.39	222.62	.431E+06	106.0	0.91	67.79
3.76	222.99	.532E+06	131.5	0.95	65.01
4.12	223.35	.628E+06	149.2	0.88	70.14
4.49	223.72	.788E+06	191.4	0.90	68.58
4.85	224.08	.974E+06	232.1	0.88	69.94
5.22	224.45	1.20E+07	283.2	0.88	70.45
5.58	224.81	1.47E+07	347.5	0.88	70.32
5.95	225.18	1.78E+07	427.9	0.89	69.46
6.31	225.54	2.15E+07	523.9	0.90	68.38
6.68	225.91	2.57E+07	638.4	0.92	67.05

<---- hydrograph ----> <-pipe / channel->
 AREA QPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0006) 1375.77 12.33 10.50 25.17 1.42 0.49
 OUTFLOW : ID= 1 (6007) 1375.77 10.26 12.25 25.17 1.32 0.46

CALIB
 STANDHYD (0071) Area (ha)= 16.47
 ID= 1 DT=15.0 min Total Imp(%)= 47.00 Dir. Com.(%)= 17.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 7.74 8.73
 Dep. Storage (mm)= 0.80 2.50
 Average Slope (%)= 1.00 2.00
 Length (m)= 331.36 40.00
 Mannings n = 0.013 0.250
 Max. Ref. Inten.(mm/hr)= 116.82 98.32
 over (min) 15.00 15.00
 Storage Coeff. (min)= 4.93 (ii) 12.03 (ii)
 Unit Hyd. Tpeak (min)= 15.00 15.00
 Unit Hyd. peak (cms)= 0.11 0.08
 TOTALS
 PEAK FLOW (cms)= 0.88 1.83 2.714 (iii)
 TIME TO PEAK (hrs)= 6.00 6.00
 RUNOFF VOLUME (mm)= 87.70 41.37 49.24
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.99 0.47 0.56

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
- CN* = 62.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5007)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.1020	0.4341
0.0560	0.2063	1.3830	0.4946
0.3190	0.2394	1.6100	0.5538
0.5940	0.3063	2.0100	0.5638
0.8240	0.3645	0.0000	0.0000

AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (0071) 16.470 2.714 6.00 49.24
 OUTFLOW : ID= 1 (5007) 16.470 0.851 6.25 49.19

PEAK FLOW REDUCTION [Qout/Qin](%)= 31.36
 TIME SHIFT OF PEAK FLOW (min)= 15.00
 MAXIMUM STORAGE USED (ha.m.)= 0.3762

CALIB
 NASHYD (0072) Area (ha)= 539.30 Curve Number (CN)= 64.2
 ID= 1 DT=15.0 min Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
 U.H. Tp(hrs)= 2.74

Unit Hyd Qpeak (cms)= 8.477

PEAK FLOW (cms)= 8.259 (i)
 TIME TO PEAK (hrs)= 9.000
 RUNOFF VOLUME (mm)= 32.491
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.367

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8507)

1 + 2 = 3 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0072): 539.30 8.259 9.00 32.49
 + ID2= 2 (5007): 16.47 0.851 6.25 49.19
 ID = 3 (8507): 555.77 8.378 9.00 32.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8210)
 1 + 2 = 3 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (6007): 1375.77 10.259 12.25 25.17
 + ID2= 2 (8507): 555.77 8.378 9.00 32.99
 ID = 3 (8210): 1931.54 15.793 10.75 27.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8208)
 1 + 2 = 3 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8206): 3183.02 28.151 10.75 33.44
 + ID2= 2 (8210): 1931.54 15.793 10.75 27.42
 ID = 3 (8208): 5114.56 43.943 10.75 31.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8212)
 1 + 2 = 3 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8208): 5114.56 43.943 10.75 31.17
 + ID2= 2 (8214): 875.75 14.300 8.50 31.03
 ID = 3 (8212): 5990.31 54.144 10.00 31.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6010)
 IN= 2--> OUT= 1 Routing time step (min)= 15.00

DATA FOR SECTION (10.1) ----->

Distance	Elevation	Manning
0.00	223.78	0.0400
10.97	224.70	0.0400
60.32	222.23	0.0400
104.19	219.90	0.0400
126.13	219.00	0.0400
134.36	218.90	0.0400 / 0.0330
137.10	218.41	0.0330
139.84	218.30	0.0330
142.58	216.76	0.0330
145.32	216.74	0.0330
148.07	216.71	0.0330
150.81	217.11	0.0330
153.55	218.40	0.0330
156.29	219.01	0.0330 / 0.0400
159.03	219.52	0.0400
167.26	219.89	0.0400
170.00	220.09	0.0400
183.71	221.31	0.0400
200.16	223.39	0.0400
271.45	224.04	0.0400

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.36	217.07	680E+04	0.9	0.36	129.75
0.73	217.44	1.65E+05	3.4	0.58	81.12
1.09	217.81	2.78E+05	7.3	0.75	63.18
1.46	218.17	4.05E+05	12.6	0.88	53.42
1.83	218.54	5.61E+05	17.7	0.90	52.70
2.19	218.90	7.64E+05	26.2	0.97	48.52
2.64	219.35	1.18E+06	44.8	1.07	44.04
3.08	219.79	1.80E+06	71.5	1.13	41.83
3.53	220.24	2.64E+06	109.3	1.17	40.26
3.97	220.68	3.66E+06	159.0	1.23	38.34
4.42	221.13	4.85E+06	221.7	1.30	36.43
4.87	221.58	6.20E+06	298.8	1.37	34.58
5.31	222.02	7.71E+06	391.1	1.44	32.85
5.76	222.47	9.37E+06	498.7	1.51	31.30
6.21	222.92	1.12E+07	622.7	1.58	29.93
6.65	223.36	1.32E+07	764.2	1.65	28.69

7.10	223.81	.155E+07	878.9	1.60	29.46
7.84	224.25	.186E+07	1057.1	1.61	28.35
7.99	224.70	.220E+07	1291.1	1.67	28.35

<---- hydrograph ----> <-pipe / channel-->

	AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8212) 5990.31	54.14	10.00	31.15	2.79	1.09	
OUTFLOW: ID= 1 (6010) 5990.31	52.39	10.75	31.15	2.76	1.09	

CALIB	Area (ha)= 211.49
STANDHYD (0102)	Total Imp(%)= 47.00
ID= 1 DT=15.0 min	Dir. Conn.(%)= 26.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	99.40	112.09
Dep. Storage (mm)=	0.80	2.50
Average Slope (%)=	1.00	2.00
Length (m)=	1187.41	40.00
Mannings n =	0.013	0.250
Max.Eff.Inten.(mm/hr)=	116.82	68.57
over (min)	15.00	30.00
Storage Coeff. (min)=	10.59 (ii)	18.80 (iii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.05
PEAK FLOW (cms)=	14.80	11.11
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	87.70	32.66
TOTAL RAINFALL (mm)=	88.50	88.50
RUNOFF COEFFICIENT =	0.99	0.37

TOTALS

21.936 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 55.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)= 133.27
STANDHYD (0101)	Total Imp(%)= 47.00
ID= 1 DT=15.0 min	Dir. Conn.(%)= 26.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	62.64	70.63
Dep. Storage (mm)=	0.80	2.50
Average Slope (%)=	1.00	2.00
Length (m)=	942.59	40.00
Mannings n =	0.013	0.250
Max.Eff.Inten.(mm/hr)=	116.82	68.57
over (min)	15.00	30.00
Storage Coeff. (min)=	9.22 (ii)	17.43 (iii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.05
PEAK FLOW (cms)=	9.72	7.22
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	87.70	32.66
TOTAL RAINFALL (mm)=	88.50	88.50
RUNOFF COEFFICIENT =	0.99	0.37

TOTALS

14.371 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 55.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5010)	
IN= 2--> OUT= 1	
DT= 15.0 min	

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	6.8040	3.7679
1.3970	1.4119	8.2460	4.4558
3.2730	2.3774	10.2760	4.7276
5.2480	3.0364	10.6760	4.7376

	AREA	QPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (0101)	133.270	14.371	6.00	46.97
OUTFLOW: ID= 1 (5010)	133.270	4.632	6.75	46.97

PEAK FLOW REDUCTION [Qout/qin](%)= 32.09
TIME SHIFT OF PEAK FLOW (min)= 45.00

MAXIMUM STORAGE USED (ha.m.)= 2.8601

ADD HYD (8216)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0102):	211.49	21.936	6.00	46.97
+ ID2= 2 (5010):	133.27	4.632	6.75	46.97
ID = 3 (8216):	344.76	23.489	6.00	46.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8218)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6010):	5990.31	52.388	10.75	31.15
+ ID2= 2 (8216):	344.76	23.489	6.00	46.97
ID = 3 (8218):	6335.07	54.358	10.75	32.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

**FUTURE
MASKINONGE RIVER REGIONAL STORM**

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V V I SSSS U U A L
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
WV I SSSS UUUU A A LLLL
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OOO TTTT TTTT H H Y Y M M OOO TM
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O O T T H H Y Y M M O O
OOO T T H H Y Y M M OOO Company Serial
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***** DETAILED OUTPUT *****

Input filename: C:\program files\Visual Ortymo 2.4r\VO2\voindat
Output filename: C:\Users\jscott\AppData\Local\Temp\0cf93cf3-cda7-49d3-bc01-11871e34e470\Scenario.out
Summary filename: C:\Users\jscott\AppData\Local\Temp\0cf93cf3-cda7-49d3-bc01-11871e34e470\Scenario.sum

DATE: 08/22/2012 TIME: 02:21:44

USER:

COMMENTS:

** SIMULATION NUMBER: 1 **

READ STORM | Filename: C:\Users\jscott\AppData\Local\Temp\0cf93cf3-cda7-49d3-bc01-11871e34e470\89c83670
| Total=212.00 mm | Comments: 12-Hour Hurricane Hazel

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	6.00	3.25	13.00	6.25	23.00	9.25	53.00
0.50	6.00	3.50	13.00	6.50	23.00	9.50	53.00
0.75	6.00	3.75	13.00	6.75	23.00	9.75	53.00
1.00	6.00	4.00	13.00	7.00	23.00	10.00	53.00
1.25	4.00	4.25	17.00	7.25	13.00	10.25	38.00
1.50	4.00	4.50	17.00	7.50	13.00	10.50	38.00
1.75	4.00	4.75	17.00	7.75	13.00	10.75	38.00
2.00	4.00	5.00	17.00	8.00	13.00	11.00	38.00
2.25	6.00	5.25	13.00	8.25	13.00	11.25	13.00
2.50	6.00	5.50	13.00	8.50	13.00	11.50	13.00
2.75	6.00	5.75	13.00	8.75	13.00	11.75	13.00
3.00	6.00	6.00	13.00	9.00	13.00	12.00	13.00

MODIFY STORM | MODIFYING PARAMETERS
| CASE# 1 | Multiplication Factor= 0.92
| | Time shift (min) = 0.00

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.250	5.52	3.250	11.96	6.250	21.16	9.25	48.76
0.500	5.52	3.500	11.96	6.500	21.16	9.50	48.76
0.750	5.52	3.750	11.96	6.750	21.16	9.75	48.76
1.000	5.52	4.000	11.96	7.000	21.16	10.00	48.76
1.250	3.68	4.250	15.64	7.250	11.96	10.25	34.96
1.500	3.68	4.500	15.64	7.500	11.96	10.50	34.96
1.750	3.68	4.750	15.64	7.750	11.96	10.75	34.96
2.000	3.68	5.000	15.64	8.000	11.96	11.00	34.96
2.250	5.52	5.250	11.96	8.250	11.96	11.25	11.96
2.500	5.52	5.500	11.96	8.500	11.96	11.50	11.96
2.750	5.52	5.750	11.96	8.750	11.96	11.75	11.96
3.000	5.52	6.000	11.96	9.000	11.96	12.00	11.96

CALIB | Area (ha)= 10.53
STANDHYD (0081) | Total Imp(%)= 45.00 Dir. Com.(%)= 15.00
| ID= 1 DT=15.0 min |

Surface Area (ha)= IMPERVIOUS 4.74 PERVIOUS (i) 5.79

Dep. Storage (mm)= 0.80 2.50
Average Slope (%)= 1.00 2.00
Length (m)= 264.95 40.00
Mannings n = 0.013 0.250
Max.Eff.Inten.(mm/hr)= 48.76 70.99
over (min)= 15.00 15.00
Storage Coeff. (min)= 6.11 (ii) 14.20 (iii)
Unit Hyd. Tpeak (min)= 15.00 15.00
Unit Hyd. peak (cms)= 0.10 0.07
TOTALS
PEAK FLOW (cms)= 0.21 1.12 1.338 (iii)
TIME TO PEAK (hrs)= 10.00 10.00
RUNOFF VOLUME (mm)= 194.24 157.26 162.80
TOTAL RAINFALL (mm)= 195.04 195.04 195.04
RUNOFF COEFFICIENT = 1.00 0.81 0.83

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 78.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0082) | Area (ha)= 402.65 Curve Number (CN)= 80.3
NASHYD (0082) | ID= 1 DT=15.0 min | Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
U.N. Tp (hrs)= 1.63

Unit Hyd Qpeak (cms)= 10.622

PEAK FLOW (cms)= 31.281 (i)
TIME TO PEAK (hrs)= 11.500
RUNOFF VOLUME (mm)= 145.473
TOTAL RAINFALL (mm)= 195.040
RUNOFF COEFFICIENT = 0.746

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8508) | AREA QPEAK TPEAK R.V.
1 + 2 = 3 | (ha) (cms) (hrs) (mm)
ID1 = 1 (0081): 10.53 1.338 10.00 162.80
+ ID2 = 2 (0082): 402.65 31.281 11.50 145.47
ID = 3 (8508): 413.18 31.687 11.50 145.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6009) | Routing time step (min)= 15.00
IN= 2--> OUT= 1 |

----- DATA FOR SECTION (9.1) -----

Distance	Elevation	Manning
0.00	232.41	0.0600
16.80	232.21	0.0600
28.00	231.55	0.0600
117.61	229.36	0.0600
235.21	228.08	0.0600
296.81	226.96	0.0600
316.00	226.17	0.0600
326.00	226.17	0.0600
332.00	226.04	0.0600 / 0.0350
335.50	225.16	0.0350
336.00	225.08	0.0350
337.00	225.08	0.0350
337.90	225.21	0.0350
339.90	225.93	0.0350 / 0.0600
345.00	226.28	0.0600
358.42	226.57	0.0600
403.22	227.73	0.0600
448.02	229.07	0.0600
492.82	230.51	0.0600
554.43	232.28	0.0600

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.28	225.36	.255E+04	0.3	0.37	160.52
0.57	225.65	.719E+04	1.1	0.55	107.34
0.85	225.93	1.38E+05	2.7	0.70	85.08
1.25	226.33	4.05E+05	7.8	0.69	86.59
1.64	226.72	1.09E+06	20.0	0.65	91.15
2.04	227.12	2.16E+06	41.2	0.68	87.34
2.44	227.52	3.72E+06	74.8	0.72	82.92

2.83	227.91	.580E+06	125.6	0.77	77.03
3.23	228.31	.842E+06	193.5	0.82	72.57
3.63	228.71	.117E+07	284.7	0.87	68.66
4.02	229.10	.157E+07	405.4	0.92	64.71
4.42	229.50	.204E+07	564.5	0.99	60.30
4.82	229.90	.256E+07	766.1	1.07	55.64
5.22	230.30	.311E+07	1001.2	1.15	51.82
5.61	230.69	.371E+07	1269.8	1.22	48.68
6.01	231.09	.435E+07	1573.3	1.29	46.05
6.41	231.49	.503E+07	1914.5	1.36	43.77
6.80	231.88	.575E+07	2314.0	1.44	41.39
7.20	232.28	.650E+07	2741.3	1.50	39.49

<---- hydrograph ----> <-pipe / channel->
 AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8508) 413.18 31.69 11.50 145.91 1.86 0.67
 OUTFLOW : ID= 1 (6009) 413.18 25.76 12.50 145.91 1.75 0.66

CALIB
 STANSHYD (0091) Area (ha)= 11.32
 ID= 1 DT=15.0 min Total Imp(%)= 50.00 Dir. Conn.(%)= 20.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 5.66 5.66
 Dep. Storage (mm)= 0.80 2.50
 Average Slope (%)= 1.00 2.00
 Length (m)= 274.71 40.00
 Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 48.76 73.33
 over (min) 15.00 15.00
 Storage Coeff. (min)= 6.24 (ii) 14.23 (ii)
 Unit Hyd. Tpeak (min)= 15.00 15.00
 Unit Hyd. peak (cms)= 0.10 0.07
 TOTALS
 PEAK FLOW (cms)= 0.31 1.13 1.441 (iii)
 TIME TO PEAK (hrs)= 10.00 10.00 10.00
 RUNOFF VOLUME (mm)= 194.24 156.60 164.13
 TOTAL RAINFALL (mm)= 195.04 195.04 195.04
 RUNOFF COEFFICIENT = 1.00 0.80 0.84

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING:FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 77.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0092) Area (ha)= 451.25 Curve Number (CN)= 78.0
 ID= 1 DT=15.0 min Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
 U.H. Tp(hrs)= 2.11

Unit Hyd Opeak (cms)= 9.199

PEAK FLOW (cms)= 30.405 (i)
 TIME TO PEAK (hrs)= 12.000
 RUNOFF VOLUME (mm)= 140.331
 TOTAL RAINFALL (mm)= 195.040
 RUNOFF COEFFICIENT = 0.719

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8509) |
 1 + 2 = 3 | AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0091): 11.32 1.441 10.00 164.13
 + ID2= 2 (0092): 451.25 30.405 12.00 140.33
 =====
 ID = 3 (8509): 462.57 30.778 12.00 140.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8214) |
 1 + 2 = 3 | AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (6009): 413.18 25.762 12.50 145.91
 + ID2= 2 (8509): 462.57 30.778 12.00 140.91
 =====
 ID = 3 (8214): 875.75 55.887 12.25 143.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0012) Area (ha)= 630.33 Curve Number (CN)= 78.0
 ID= 1 DT=15.0 min Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
 U.H. Tp(hrs)= 1.79

Unit Hyd Opeak (cms)= 15.195

PEAK FLOW (cms)= 45.996 (i)
 TIME TO PEAK (hrs)= 11.750
 RUNOFF VOLUME (mm)= 140.334
 TOTAL RAINFALL (mm)= 195.040
 RUNOFF COEFFICIENT = 0.720

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANSHYD (0011) Area (ha)= 2.54
 ID= 1 DT=15.0 min Total Imp(%)= 45.00 Dir. Conn.(%)= 15.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 1.14 1.40
 Dep. Storage (mm)= 0.80 2.50
 Average Slope (%)= 1.00 2.00
 Length (m)= 130.13 40.00
 Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 48.76 70.38
 over (min) 15.00 15.00
 Storage Coeff. (min)= 3.99 (ii) 12.11 (ii)
 Unit Hyd. Tpeak (min)= 15.00 15.00
 Unit Hyd. peak (cms)= 0.11 0.08
 TOTALS
 PEAK FLOW (cms)= 0.05 0.27 0.322 (iii)
 TIME TO PEAK (hrs)= 10.00 10.00 10.00
 RUNOFF VOLUME (mm)= 194.24 154.62 160.56
 TOTAL RAINFALL (mm)= 195.04 195.04 195.04
 RUNOFF COEFFICIENT = 1.00 0.79 0.82

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING:FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 77.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANSHYD (0019) Area (ha)= 487.70
 ID= 1 DT=15.0 min Total Imp(%)= 45.00 Dir. Conn.(%)= 35.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 219.46 268.24
 Dep. Storage (mm)= 1.00 5.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 1803.15 40.00
 Mannings n = 0.013 0.250

Max.Eff.Inten.(mm/hr)= 48.76 55.26
 over (min) 15.00 30.00
 Storage Coeff. (min)= 19.31 (ii) 28.26 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.06 0.04
 TOTALS

PEAK FLOW (cms)= 22.34 35.89 57.871 (iii)
 TIME TO PEAK (hrs)= 10.00 10.25 10.00
 RUNOFF VOLUME (mm)= 194.04 161.24 172.72
 TOTAL RAINFALL (mm)= 195.04 195.04 195.04
 RUNOFF COEFFICIENT = 0.99 0.83 0.89

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 86.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8510) |
 1 + 2 = 3 | AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0011): 2.54 0.322 10.00 160.56
 + ID2= 2 (0019): 487.70 57.871 10.00 172.72

 ID = 3 (8510): 490.24 58.193 10.00 172.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8501)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0012):	630.33	45.996	11.75	140.33
+ ID2= 2 (8510):	490.24	58.193	10.00	172.66
ID = 3 (8501):	1120.57	88.735	11.00	154.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6002) | Routing time step (min)'= 15.00
 IN= 2--> OUT= 1 |

----- DATA FOR SECTION (2.1) -----

Distance	Elevation	Manning
0.00	235.70	0.0700
15.10	235.42	0.0700
30.21	235.01	0.0700
37.76	234.49	0.0700
45.31	231.43	0.0700
56.64	231.55	0.0700
98.18	229.87	0.0700
117.06	229.64	0.0700
124.61	229.03	0.0700 /0.0300
128.38	228.69	0.0300
132.16	228.58	0.0300
143.49	228.98	0.0300 /0.0700
158.59	229.36	0.0700
181.25	229.58	0.0700
200.13	229.96	0.0700
237.89	231.19	0.0700
268.09	232.60	0.0700
302.08	233.40	0.0700
332.29	234.25	0.0700
373.82	235.68	0.0700

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.20	228.78	639E+04	0.3	0.26	359.77
0.40	228.98	222E+05	1.6	0.40	227.92
0.79	229.37	842E+05	9.7	0.63	145.25
1.19	229.77	218E+06	25.5	0.85	142.16
1.58	230.16	441E+06	53.8	0.67	136.74
1.98	230.56	715E+06	94.9	0.73	125.69
2.37	230.95	104E+07	148.9	0.79	116.12
2.76	231.34	141E+07	217.2	0.85	107.88
3.16	231.74	183E+07	299.4	0.90	101.81
3.55	232.13	228E+07	403.8	0.97	94.28
3.95	232.53	276E+07	523.3	1.04	87.91
4.34	232.92	326E+07	650.4	1.10	83.62
4.74	233.32	380E+07	792.5	1.15	79.99
5.13	233.71	438E+07	954.7	1.20	76.48
5.52	234.10	499E+07	1134.7	1.25	73.30
5.92	234.50	563E+07	1334.6	1.31	70.34
6.31	234.89	631E+07	1540.4	1.35	68.23
6.71	235.29	702E+07	1746.8	1.37	67.00
7.10	235.68	780E+07	1958.9	1.38	66.34

----- hydrograph ----- <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8501) 1120.57	88.73	11.00	154.48	1.22	0.72
OUTFLOW: ID= 1 (6002) 1120.57	63.08	12.00	154.47	1.67	0.68

CALIB (0002) | Area (ha)= 869.77 Curve Number (CN)= 76.4
 ID= 1 DT=15.0 min | Ia (mm)= 2.50 # of Linear Res. (N)= 3.50
 U.H. Tp (hrs)= 3.32

Unit Hyd Qpeak (cms)= 11.293

PEAK FLOW (cms)= 44.932 (i)
 TIME TO PEAK (hrs)= 13.000
 RINOFF VOLUME (mm)= 116.796
 TOTAL RAINFALL (mm)= 195.040
 RUNOFF COEFFICIENT = 0.701

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8200)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0002):	869.77	44.932	13.00	136.80
+ ID2= 2 (6002):	1120.57	63.076	12.00	154.47
ID = 3 (8200):	1990.34	104.731	12.25	146.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6003) | Routing time step (min)'= 15.00
 IN= 2--> OUT= 1 |

----- DATA FOR SECTION (3.1) -----

Distance	Elevation	Manning
0.00	234.33	0.0700
50.41	229.32	0.0700
94.52	227.15	0.0700
113.42	226.02	0.0700
116.57	225.86	0.0700
125.32	224.53	0.0700
130.32	224.24	0.0700 /0.0300
130.82	223.78	0.0300
131.82	223.63	0.0300
132.42	223.79	0.0300
133.42	224.43	0.0300 /0.0700
163.83	226.19	0.0700
189.03	227.19	0.0700
229.99	228.02	0.0700
270.95	228.65	0.0700
286.70	229.01	0.0700
305.61	229.31	0.0700

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.20	223.83	564E+03	0.1	0.26	175.53
0.41	224.04	168E+04	0.3	0.44	105.08
0.61	224.24	309E+04	0.6	0.56	82.17
0.93	224.56	847E+04	1.8	0.57	80.17
1.24	224.87	209E+05	4.1	0.54	84.69
1.56	225.19	399E+05	8.0	0.55	83.36
1.88	225.51	654E+05	13.7	0.57	79.74
2.19	225.82	976E+05	21.5	0.61	75.55
2.51	226.14	138E+06	31.1	0.62	73.71
2.83	226.46	188E+06	43.6	0.64	71.92
3.15	226.78	250E+06	59.8	0.66	69.63
3.46	227.09	324E+06	80.5	0.68	67.03
3.78	227.41	411E+06	103.4	0.69	66.15
4.10	227.73	516E+06	132.2	0.70	65.13
4.41	228.04	642E+06	168.2	0.72	63.59
4.73	228.36	788E+06	210.4	0.73	62.46
5.05	228.68	959E+06	262.8	0.75	60.81
5.36	228.99	115E+07	328.9	0.79	58.23
5.68	229.31	136E+07	401.2	0.81	56.49

----- hydrograph ----- <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8200) 1990.34	104.73	12.25	146.75	3.79	0.69
OUTFLOW: ID= 1 (6003) 1990.34	95.75	13.25	146.75	3.67	0.69

CALIB (0031) | Area (ha)= 6.23
 ID= 1 DT=15.0 min | Total Imp(%)= 65.00 Dir. Conn.(%)= 47.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)= 4.05	2.18
Dep. Storage (mm)= 0.80	2.50
Average Slope (%)= 1.00	2.00
Length (m)= 203.80	40.00
Mannings n = 0.013	0.250

Max. Eff. Inten. (mm/hr)= 48.76 66.65
 over (min)= 15.00 15.00
 Storage Coeff. (min)= 5.22 (ii) 13.52 (ii)
 Unit Hyd. Tpeak (min)= 15.00 15.00
 Unit Hyd. peak (cms)= 0.11 0.08

TOTALS

PEAK FLOW (cms)	TIME TO PEAK (hrs)	RINOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
0.40	0.40	0.794 (iii)	195.04	0.86
10.00	10.00	10.00	195.04	0.86
194.24	145.52	168.41	195.04	0.86
195.04	195.04	195.04	195.04	0.86
1.00	0.75			

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.5 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHVD (0032) Area (ha)= 461.62 Curve Number (CN)= 75.8
ID= 1 DT=15.0 min Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
U.H. Tp(hrs)= 2.40

Unit Hyd Qpeak (cms)= 8.267

PEAK FLOW (cms)= 28.389 (i)
TIME TO PEAK (hrs)= 12.250
RUNOFF VOLUME (mm)= 135.483
TOTAL RAINFALL (mm)= 195.040
RUNOFF COEFFICIENT = 0.695
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8503)
1 + 2 = 3 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0031): 6.23 0.794 10.00 168.41
+ ID2= 2 (0032): 461.62 28.389 12.25 135.48
ID= 3 (8503): 467.85 28.429 12.25 135.92
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8202)
1 + 2 = 3 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (6003): 1990.34 95.754 13.25 146.75
+ ID2= 2 (8503): 467.85 28.429 12.25 135.92
ID= 3 (8202): 2458.19 122.355 12.75 144.69
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

CALIB
STANDHYD (0041) Area (ha)= 8.66
ID= 1 DT=15.0 min Total Imp(%)= 52.00 Dir. Conn.(%)= 22.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 4.50 4.16
Dep. Storage (mm)= 0.80 2.50
Average Slope (%)= 1.00 2.00
Length (m)= 240.28 40.00
Manning's n = 0.013 0.250

Max. Eff. Inter.(mm/hr)= 48.76 70.97
over (min)= 15.00 15.00
Storage Coeff. (min)= 5.76 (ii) 13.86 (ii)
Unit Hyd. Tpeak (min)= 15.00 15.00
Unit Hyd. peak (cms)= 0.10 0.07

*TOTALS*
PEAK FLOW (cms)= 0.26 0.81 1.063 (iii)
TIME TO PEAK (hrs)= 10.00 10.00 10.00
RUNOFF VOLUME (mm)= 194.24 143.74 154.85
TOTAL RAINFALL (mm)= 195.04 195.04 195.04
RUNOFF COEFFICIENT = 1.00 0.74 0.79
  
```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
 CN* = 70.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB
NASHVD (0042) Area (ha)= 376.06 Curve Number (CN)= 72.3
ID= 1 DT=15.0 min Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
U.H. Tp(hrs)= 1.62

Unit Hyd Qpeak (cms)= 9.964

PEAK FLOW (cms)= 26.685 (i)
TIME TO PEAK (hrs)= 11.750
RUNOFF VOLUME (mm)= 127.907
TOTAL RAINFALL (mm)= 195.040
RUNOFF COEFFICIENT = 0.656
  
```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (8504)
1 + 2 = 3 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (0041): 8.66 1.063 10.00 154.85
+ ID2= 2 (0042): 376.06 26.685 11.75 127.91
ID= 3 (8504): 384.72 26.977 11.50 128.51
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ADD HYD (8204)
1 + 2 = 3 AREA QPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8202): 2458.19 122.355 12.75 144.69
+ ID2= 2 (8504): 384.72 26.977 11.50 128.51
ID= 3 (8204): 2842.91 144.331 12.50 142.50
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

ROUTE CHN (6005)
IB= 2 -> OUF= 1 Routing time step (min)= 15.00
  
```

----- DATA FOR SECTION (5.1) -----						
Distance	Elevation	Manning				
0.00	225.02	0.0400				
20.58	224.79	0.0400				
35.54	223.93	0.0400				
48.54	223.09	0.0400				
59.86	222.15	0.0400				
67.35	221.11	0.0400				
76.41	219.63	0.0400 / 0.0350	Main Channel			
82.81	218.53	0.0350	Main Channel			
85.41	218.50	0.0350	Main Channel			
86.78	218.50	0.0350	Main Channel			
88.15	218.50	0.0350	Main Channel			
93.45	218.71	0.0350	Main Channel			
103.45	218.94	0.0350 / 0.0400	Main Channel			
109.45	219.25	0.0400				
114.11	220.15	0.0400				
123.47	221.73	0.0400				
136.56	223.41	0.0400				
153.40	224.15	0.0400				
170.24	224.55	0.0400				
185.20	224.54	0.0400				

----- TRAVEL TIME TABLE -----					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.22	218.72	.640E+04	0.5	0.28	204.26
0.44	218.94	.198E+05	2.2	0.39	147.20
0.77	219.27	.507E+05	9.3	0.63	91.02
1.10	219.60	.880E+05	20.9	0.82	70.26
1.43	219.93	.130E+06	37.7	1.00	57.26
1.76	220.26	.175E+06	59.1	1.16	49.43
2.09	220.59	.226E+06	85.1	1.30	44.19
2.42	220.92	.280E+06	115.8	1.43	40.35
2.75	221.25	.340E+06	151.3	1.54	37.40
3.08	221.58	.404E+06	191.9	1.64	35.06
3.40	221.90	.473E+06	237.5	1.73	33.19
3.73	222.23	.548E+06	288.1	1.82	31.68
4.06	222.56	.629E+06	344.1	1.89	30.48
4.39	222.89	.718E+06	406.8	1.96	29.43
4.72	223.22	.815E+06	476.1	2.02	28.52
5.05	223.55	.920E+06	547.9	2.06	28.00
5.38	223.88	.104E+07	624.5	2.07	27.74
5.71	224.21	.117E+07	710.0	2.09	27.54
6.04	224.54	.133E+07	800.8	2.08	27.62

----- hydrograph -----							
		AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
		(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW :	ID= 2 (8204)	2842.91	144.33	12.50	142.50	2.68	1.52
OUTFLOW :	ID= 1 (6005)	2842.91	140.13	13.00	142.50	2.64	1.50

```

CALIB
NASHVD (0005) Area (ha)= 340.11 Curve Number (CN)= 84.5
ID= 1 DT=15.0 min Ia (mm)= 2.50 # of Linear Res.(N)= 3.50
U.H. Tp(hrs)= 2.45

Unit Hyd Qpeak (cms)= 5.972
  
```

PEAK FLOW (cms)= 22.803 (i)
 TIME TO PEAK (hrs)= 12.250
 RUNOFF VOLUME (mm)= 155.029
 TOTAL RAINFALL (mm)= 195.040
 RUNOFF COEFFICIENT = 0.795

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8206) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0005): 340.11 22.803 12.25 155.03 |
| + ID2= 2 (6005): 2842.91 140.128 13.00 142.50 |
| ===== |
| ID = 3 (8206): 3183.02 162.096 12.75 143.84 |
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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-----
| NASHVD (0006) | Area (ha)=1375.77 Curve Number (CN)= 74.0 |
| ID= 1 DT=15.0 min | Ia (mm)= 2.50 # of Linear Res.(N)= 3.50 |
|-----|
| U.H. Tp(hrs)= 3.90 |
  
```

Unit Hyd Qpeak (cms)= 15.192

PEAK FLOW (cms)= 61.979 (i)
 TIME TO PEAK (hrs)= 13.500
 RUNOFF VOLUME (mm)= 131.562
 TOTAL RAINFALL (mm)= 195.040
 RUNOFF COEFFICIENT = 0.675

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ROUTE CHN (6007) |
| IN= 2--> OUT= 1 | Routing time step (min)'= 15.00
  
```

<----- DATA FOR SECTION (7.1) ----->

Distance	Elevation	Manning	
0.00	225.91	0.0700	
51.88	225.32	0.0700	
135.69	224.23	0.0700	
183.58	223.08	0.0700	
211.52	223.28	0.0700	
223.49	222.51	0.0700	
231.43	220.69	0.0700	
244.53	219.78	0.0700 /0.0500	Main Channel
249.23	219.60	0.0500	Main Channel
249.93	219.46	0.0500	Main Channel
251.43	219.23	0.0500	Main Channel
253.50	219.70	0.0500 /0.0700	Main Channel
261.10	219.90	0.0700	
271.38	221.01	0.0700	
283.36	223.77	0.0700	
295.33	224.21	0.0700	
331.25	226.04	0.0700	
351.20	227.76	0.0700	
371.16	227.49	0.0700	
395.10	228.55	0.0700	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.23	219.46	.112E+04	0.0	0.16	375.56
0.47	219.70	.471E+04	0.3	0.22	277.80
0.84	220.07	.267E+05	2.4	0.34	184.45
1.20	220.43	.628E+05	7.4	0.44	140.97
1.57	220.80	.110E+06	15.6	0.52	117.94
1.93	221.16	.165E+06	27.4	0.61	100.71
2.30	221.53	.225E+06	42.6	0.70	88.16
2.66	221.89	.290E+06	60.9	0.78	79.20
3.03	222.26	.358E+06	82.4	0.85	72.44
3.39	222.62	.431E+06	106.0	0.91	67.79
3.76	222.99	.513E+06	131.5	0.95	65.01
4.12	223.35	.628E+06	149.2	0.88	70.14
4.49	223.72	.788E+06	191.4	0.90	68.58
4.85	224.08	.974E+06	232.1	0.88	69.94
5.22	224.45	.120E+07	283.2	0.88	70.45
5.58	224.81	.147E+07	347.5	0.88	70.32
5.95	225.18	.178E+07	427.9	0.89	69.46
6.31	225.54	.215E+07	523.9	0.90	68.38
6.68	225.91	.257E+07	638.4	0.92	67.05

<---- hydrograph ----> <-pipe / channel->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0006) 1375.77	61.98	13.50	131.56	2.68	0.78
OUTFLOW: ID= 1 (6007) 1375.77	57.11	14.25	131.56	2.58	0.76

```

-----
| CALIB |
| STANDBY (0071) | Area (ha)= 16.47 |
| ID= 1 DT=15.0 min | Total Imp(%)= 47.00 Dir. Corr.(%)= 17.00
  
```

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	8.74	8.73
Dep. Storage (mm)=	0.80	2.50
Average Slope (%)=	1.00	2.00
Length (m)=	331.36	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	48.76	72.49
Storage Coeff. (min)=	6.99 (ii)	15.01 (iii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.10	0.05
PEAK FLOW (cms)=	0.38	1.69
TIME TO PEAK (hrs)=	10.00	10.00
RUNOFF VOLUME (mm)=	194.24	159.75
TOTAL RAINFALL (mm)=	195.04	195.04
RUNOFF COEFFICIENT =	1.00	0.82

**TOTALS*

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.9 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| CALIB |
| NASHVD (0072) | Area (ha)= 539.30 Curve Number (CN)= 81.2 |
| ID= 1 DT=15.0 min | Ia (mm)= 2.50 # of Linear Res.(N)= 3.50 |
|-----|
| U.H. Tp(hrs)= 2.74 |
  
```

Unit Hyd Qpeak (cms)= 8.477

PEAK FLOW (cms)= 32.979 (i)
 TIME TO PEAK (hrs)= 12.500
 RUNOFF VOLUME (mm)= 147.493
 TOTAL RAINFALL (mm)= 195.040
 RUNOFF COEFFICIENT = 0.756

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
| ADD HYD (8507) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (0071): 16.47 2.070 10.00 165.61 |
| + ID2= 2 (0072): 539.30 32.979 12.50 147.49 |
| ===== |
| ID = 3 (8507): 555.77 33.108 12.50 148.03 |
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8210) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (6007): 1375.77 57.110 14.25 131.56 |
| + ID2= 2 (8507): 555.77 33.108 12.50 148.03 |
| ===== |
| ID = 3 (8210): 1931.54 84.235 13.50 136.30 |
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

-----
| ADD HYD (8208) |
| 1 + 2 = 3 |
|-----|
| AREA QPEAK TPEAK R.V. |
| (ha) (cms) (hrs) (mm) |
| ID1= 1 (8206): 3183.02 162.096 12.75 143.84 |
| + ID2= 2 (8210): 1931.54 84.235 13.50 136.30 |
| ===== |
| ID = 3 (8208): 5114.56 244.116 13.00 140.99 |
  
```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8212)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8208):	5114.56	244.116	13.00	140.99
+ ID2= 2 (8214):	875.75	55.887	12.25	143.27
=====				
ID = 3 (8212):	5990.31	296.527	12.75	141.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6010)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

DATA FOR SECTION (10.1)----->				
Distance	Elevation	Manning		
0.00	223.78	0.0400		
10.97	224.70	0.0400		
60.32	222.23	0.0400		
104.19	219.90	0.0400		
126.13	219.00	0.0400		
134.36	218.90	0.0400 / 0.0330	Main Channel	
137.10	218.41	0.0330	Main Channel	
139.84	218.30	0.0330	Main Channel	
142.58	216.76	0.0330	Main Channel	
145.32	216.74	0.0330	Main Channel	
148.07	216.71	0.0330	Main Channel	
150.81	217.11	0.0330	Main Channel	
153.55	218.40	0.0330	Main Channel	
156.29	219.01	0.0330 / 0.0400	Main Channel	
159.03	219.52	0.0400		
167.26	219.89	0.0400		
170.00	220.09	0.0400		
183.71	221.31	0.0400		
200.16	223.39	0.0400		
271.45	224.04	0.0400		

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV.TIME (min)
0.36	217.07	.680E+04	0.9	0.36	129.75
0.73	217.44	.165E+05	3.4	0.58	81.12
1.09	217.81	.278E+05	7.3	0.75	63.18
1.46	218.17	.405E+05	12.6	0.88	53.42
1.83	218.54	.561E+05	17.7	0.90	52.70
2.19	218.90	.764E+05	26.2	0.97	48.52
2.64	219.35	.118E+06	44.8	1.07	44.04
3.08	219.79	.180E+06	71.5	1.13	41.83
3.53	220.24	.264E+06	109.3	1.17	40.26
3.97	220.68	.366E+06	159.0	1.23	38.34
4.42	221.13	.485E+06	221.7	1.30	36.43
4.87	221.58	.620E+06	298.8	1.37	34.58
5.31	222.02	.771E+06	391.1	1.44	32.85
5.76	222.47	.937E+06	498.7	1.51	31.30
6.21	222.92	.112E+07	622.7	1.58	29.93
6.65	223.36	.132E+07	764.2	1.65	28.69
7.10	223.81	.155E+07	878.9	1.60	29.46
7.54	224.25	.186E+07	1057.1	1.61	29.35
7.99	224.70	.220E+07	1291.1	1.67	28.35

hydrograph		<-pipe / channel->	
AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8212) 5990.31	296.53	12.75	141.32
OUTFLOW: ID= 1 (6010) 5990.31	286.79	13.25	141.32

CALIB	Area (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
STANDHYD (0101)	133.27			
ID= 1 DT=15.0 min		47.00		26.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)	62.64	70.63	
Dep. Storage (mm)	0.80	2.50	
Average Slope (%)	1.00	2.00	
Length (m)	942.59	40.00	
Mannings n	0.013	0.250	
Max.Eff.Inten.(mm/hr)	48.76	61.82	
over (min)	15.00	30.00	
Storage Coeff. (min)	13.08 (ii)	21.64 (ii)	
Unit Hyd. Tpeak (min)	15.00	30.00	
Unit Hyd. peak (cms)	0.08	0.05	
TOTALS			
PEAK FLOW (cms)	4.66	11.00	15.63 (iii)
TIME TO PEAK (hrs)	10.00	10.00	10.00
RUNOFF VOLUME (mm)	194.24	146.91	159.21
TOTAL RAINFALL (mm)	195.04	195.04	195.04
RUNOFF COEFFICIENT	1.00	0.75	0.82

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 74.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
STANDHYD (0102)	211.49			
ID= 1 DT=15.0 min		47.00		26.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)	99.40	112.09	
Dep. Storage (mm)	0.80	2.50	
Average Slope (%)	1.00	2.00	
Length (m)	1187.41	40.00	
Mannings n	0.013	0.250	
Max.Eff.Inten.(mm/hr)	48.76	61.82	
over (min)	15.00	30.00	
Storage Coeff. (min)	15.03 (ii)	23.58 (ii)	
Unit Hyd. Tpeak (min)	15.00	30.00	
Unit Hyd. peak (cms)	0.07	0.04	
TOTALS			
PEAK FLOW (cms)	7.34	17.15	24.492 (iii)
TIME TO PEAK (hrs)	10.00	10.00	10.00
RUNOFF VOLUME (mm)	194.24	146.91	159.21
TOTAL RAINFALL (mm)	195.04	195.04	195.04
RUNOFF COEFFICIENT	1.00	0.75	0.82

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 74.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8216)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0101):	133.27	15.653	10.00	159.21
+ ID2= 2 (0102):	211.49	24.492	10.00	159.21
=====				
ID = 3 (8216):	344.76	40.145	10.00	159.21

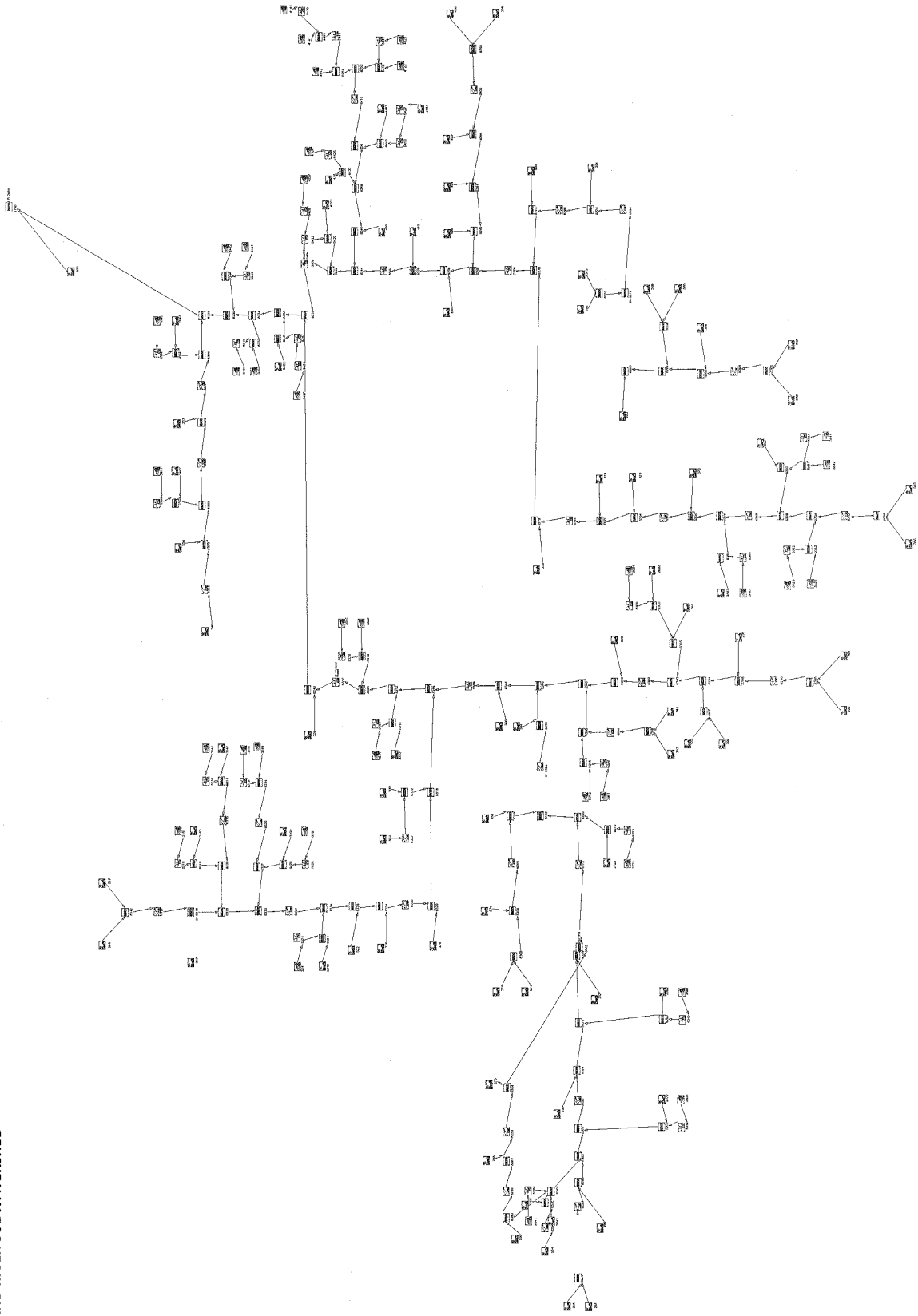
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8218)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (6010):	5990.31	286.787	13.25	141.32
+ ID2= 2 (8216):	344.76	40.145	10.00	159.21
=====				
ID = 3 (8218):	6335.07	287.335	13.25	142.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

WEST HOLLAND RIVER SUBWATERSHED



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V V I SSSS U U A L
V V I SS U U A A L
V V I SS U U A A A L
V V I SSSS U U A A L
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***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\Visual Otthymo 2.4\VO2\vo1.nat
 Output filename: C:\Users\aman\cu\AppData\Local\Temp\ec3b5098-78b3-448d-a2b0-630e907e78a7\Scenario.out
 Summary filename: C:\Users\aman\cu\AppData\Local\Temp\ec3b5098-78b3-448d-a2b0-630e907e78a7\Scenario.sum

DATE: 08/29/2012 TIME: 01:55:17
 USER:

COMMENTS: _____

 ** SIMULATION NUMBER: 1 **

READ STORM
 Ptotal = 42.00 mm
 Filename: C:\Users\aman\cu\AppData\Local\Temp\ec3b5098-78b3-448d-a2b0-630e907e78a7\1e1d252a5
 Comments: 2-Year 12-Hour SCS II Design Storm

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.05	3.25	1.68	6.25	7.56	9.25	1.47
0.50	1.05	3.50	1.68	6.50	7.56	9.50	1.47
0.75	1.05	3.75	1.68	6.75	3.36	9.75	1.47
1.00	1.05	4.00	1.68	7.00	3.36	10.00	1.47
1.25	1.05	4.25	2.52	7.25	2.52	10.25	0.84
1.50	1.05	4.50	2.52	7.50	2.52	10.50	0.84
1.75	1.05	4.75	3.36	7.75	2.52	10.75	0.84
2.00	1.05	5.00	3.36	8.00	2.52	11.00	0.84
2.25	1.26	5.25	5.04	8.25	1.47	11.25	0.84
2.50	1.26	5.50	5.04	8.50	1.47	11.50	0.84
2.75	1.26	5.75	20.16	8.75	1.47	11.75	0.84
3.00	1.26	6.00	55.44	9.00	1.47	12.00	0.84

CALIB
 STANDHYD (1662)
 ID= 1 DT=15.0 min
 Area (ha)= 141.06
 Total Imp(%)= 70.00 Di r. Conn.(%)= 38.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 98.74 42.32
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 969.74 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 55.44 58.61
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 15.56 (ii) 28.81 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.07 0.04
 PEAK FLOW (cms)= 5.92 2.92 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.25 7.759 (iii)
 6.00

RUNOFF VOLUME (mm)= 40.00 18.37 26.59
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.44 0.63

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (1661)
 ID= 1 DT=15.0 min
 Area (ha)= 293.64
 Total Imp(%)= 42.00 Di r. Conn.(%)= 23.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 123.33 170.31
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1399.14 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 37.80 16.49
 over (min)= 30.00 45.00
 Storage Coeff. (min)= 22.60 (ii) 44.60 (ii)
 Unit Hyd. Tpeak (min)= 30.00 45.00
 Unit Hyd. peak (cms)= 0.04 0.03
 PEAK FLOW (cms)= 5.28 3.90 *TOTALS*
 TIME TO PEAK (hrs)= 6.25 6.50 6.25 (iii)
 RUNOFF VOLUME (mm)= 40.00 13.37 19.49
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.32 0.46

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5166)
 IN= 2--> OUT= 1
 DT= 15.0 min
 OUTFLOW STORAGE OUTFLOW STORAGE
 (cms) (ha. m.) (cms) (ha. m.)
 0.0000 0.0000 7.2300 6.6504
 0.4310 3.5677 9.1580 7.5825
 1.6680 3.8178 10.9790 8.5125
 3.5970 4.8509 11.3790 8.5225
 5.3070 5.7381 0.0000 0.0000
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (1661) 293.640 8.110 6.25 19.49
 OUTFLOW: ID= 1 (5166) 293.640 1.685 8.25 19.49
 PEAK FLOW REDUCTION [Qout/Qin] (%) = 20.77
 TIME SHIFT OF PEAK FLOW (min) = 120.00
 MAXIMUM STORAGE USED (ha. m.) = 3.8268

ADD HYD (8134)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (1662): 141.06 7.759 6.00 26.59
 + ID2= 2 (5166): 293.64 1.685 8.25 19.49
 ID= 3 (8134): 434.70 7.864 6.00 21.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1642)
 ID= 1 DT=15.0 min
 Area (ha)= 120.35
 Total Imp(%)= 22.00 Di r. Conn.(%)= 17.00
 IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 26.48 93.87
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 895.73 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 55.44 10.44
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 14.84 (ii) 41.25 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.07 0.03

PEAK FLOW (cms)= 2.31 1.43
 TIME TO PEAK (hrs)= 6.00 6.50
 RUNOFF VOLUME (mm)= 40.00 10.81 15.77
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.26 0.38

TOTALS
 2.862 (iii)
 6.00
 15.77
 42.00
 0.38

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (1641)
 ID= 1 DT=15.0 min Area (ha)= 113.89
 Total Imp(%)= 71.00 Di r. Conn.(%)= 53.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	80.86	33.03
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	871.36	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	55.44	23.35
over (min)=	15.00	45.00
Storage Coeff. (min)=	14.60 (ii)	33.74 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.07	0.03
PEAK FLOW (cms)=	6.85	1.25
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	40.00	15.30
TOTAL RAINFALL (mm)=	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.36

TOTALS
 7.368 (iii)
 6.00
 28.39
 42.00
 0.68

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5164)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	8.4260	4.5513
0.3860	1.9228	9.2330	4.9251
3.3700	2.2339	9.9990	5.2982
4.5710	3.3613	10.3990	5.3082
5.7170	4.0568	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1641)	113.890	7.368	6.00	28.39
OUTFLOW: ID= 1 (5164)	113.890	1.446	7.00	28.38

PEAK FLOW REDUCTION [Qout/Qin] (%) = 19.63
 TIME SHIF T OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 2.0371

ADD HYD (8132)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1642):	120.35	2.862	6.00	15.77
+ ID2= 2 (5164):	113.89	1.446	7.00	28.38
ID = 3 (8132):	234.24	3.062	6.00	21.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0152)
 ID= 1 DT=15.0 min Area (ha)= 124.37 Curve Number (CN)= 59.0
 Ia (mm)= 5.00 # of Li near Res. (N)= 1.50
 U. H. Tp(hrs)= 0.89

Unit Hyd Opeak (cms)= 2.372

PEAK FLOW (cms)= 0.386 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 6.335
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.151

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (1481)
 ID= 1 DT=15.0 min Area (ha)= 61.75
 Total Imp(%)= 47.00 Di r. Conn.(%)= 17.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	29.02	32.73
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	641.61	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	55.44	2.78
over (min)=	15.00	60.00
Storage Coeff. (min)=	12.15 (ii)	57.01 (ii)
Unit Hyd. Tpeak (min)=	15.00	60.00
Unit Hyd. peak (cms)=	0.08	0.02
PEAK FLOW (cms)=	1.28	0.14
TIME TO PEAK (hrs)=	6.00	6.75
RUNOFF VOLUME (mm)=	40.00	2.66
TOTAL RAINFALL (mm)=	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.06

TOTALS
 1.319 (iii)
 6.00
 9.00
 42.00
 0.21

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 23.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5148)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.1310	1.6343
0.2090	0.7802	5.1870	1.8598
1.1960	0.9062	6.0380	2.0805
2.2280	1.1562	6.4380	2.0905
3.0910	1.3741	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1481)	61.750	1.319	6.00	9.00
OUTFLOW: ID= 1 (5148)	61.750	0.099	8.75	8.99

PEAK FLOW REDUCTION [Qout/Qin] (%) = 7.50
 TIME SHIF T OF PEAK FLOW (min) = 165.00
 MAXIMUM STORAGE USED (ha.m.) = 0.3697

 CALIB NASHYD (1482)
 ID= 1 DT=15.0 min

Area (ha)	= 356.14	Curve Number (CN)	= 31.3
Ia (mm)	= 5.00	# of Linear Res. (N)	= 1.50
U. H. Tp (hrs)	= 1.12		

Unit Hyd Qpeak (cms) = 5.438
 PEAK FLOW (cms) = 0.333 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 2.283
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.054

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (9148)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1482):	356.14	0.333	8.00	2.28
+ ID2= 2 (5148):	61.75	0.099	8.75	8.99

ID = 3 (9148):	417.89	0.431	8.00	3.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB STANDHYD (1501)
 ID= 1 DT=15.0 min

Area Total (ha)	= 0.16	Dir. Conn. (%)	= 15.00
Imp (%)	= 45.00		

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	= 0.07	0.09
Dep. Storage	= 2.00	5.00
Average Slope	= 0.50	0.50
Length	= 32.66	40.00
Mannings n	= 0.013	0.250
Max. Eff. Inten. (mm/hr)	= 55.44	1.50
over (min)	= 15.00	60.00
Storage Coeff. (min)	= 2.03 (ii)	59.42 (ii)
Unit Hyd. Tpeak (min)	= 15.00	60.00
Unit Hyd. peak (cms)	= 0.11	0.02

TOTALS
 PEAK FLOW (cms) = 0.00
 TIME TO PEAK (hrs) = 6.00
 RUNOFF VOLUME (mm) = 40.00
 TOTAL RAINFALL (mm) = 42.00
 RUNOFF COEFFICIENT = 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 16.4 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR (5150)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0110	0.0042
0.0010	0.0020	0.0140	0.0049
0.0030	0.0023	0.0160	0.0054
0.0060	0.0030	0.0160	0.0154
0.0080	0.0036	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1501)	0.160	0.004	6.00	7.11
OUTFLOW: ID= 1 (5150)	0.160	0.000	7.50	4.08

PEAK FLOW REDUCTION [Qout/Qin] (%) = 9.03
 TIME SHIFT OF PEAK FLOW (min) = 90.00

MAXIMUM STORAGE USED (ha. m.) = 0.0007

 CALIB NASHYD (1502)
 ID= 1 DT=15.0 min

Area (ha)	= 105.72	Curve Number (CN)	= 17.2
Ia (mm)	= 5.00	# of Linear Res. (N)	= 1.50
U. H. Tp (hrs)	= 0.58		

Unit Hyd Qpeak (cms) = 3.134
 PEAK FLOW (cms) = 0.075 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 1.062
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.025

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (9150)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1502):	105.72	0.075	6.75	1.06
+ ID2= 2 (5150):	0.16	0.000	7.50	4.08

ID = 3 (9150):	105.88	0.076	6.75	1.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8116)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9148):	417.89	0.431	8.00	3.27
+ ID2= 2 (9150):	105.88	0.076	6.75	1.07

ID = 3 (8116):	523.77	0.492	7.75	2.83

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6152)
 IN= 2--> OUT= 1
 Routing time step (min) = 15.00

<----- DATA FOR SECTION (1521.0) ----->

Distance	Elevation	Manning
0.00	290.00	0.0500
15.34	286.87	0.0500
46.03	284.84	0.0500
88.22	283.07	0.0500
126.57	282.11	0.0500
153.42	280.34	0.0500
157.26	280.07	0.0500
161.09	279.72	0.0500
167.76	278.94	0.0500 / 0.0300
168.01	278.60	0.0300
168.76	278.60	0.0300
169.51	278.60	0.0300
169.76	278.90	0.0300 / 0.0500
173.76	279.40	0.0500
180.27	279.69	0.0500
184.11	279.93	0.0500
218.62	281.86	0.0500
260.82	283.20	0.0500
360.54	289.25	0.0500
379.72	289.80	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.30	278.90	.114E+04	0.4	0.80	45.70
0.87	279.47	.932E+04	4.2	0.98	37.22
1.45	280.05	.353E+05	17.5	1.09	33.68
2.02	280.62	.831E+05	47.8	1.26	29.01
2.59	281.19	.155E+06	102.9	1.46	25.06
3.17	281.77	.250E+06	189.3	1.66	22.03
3.74	282.34	.375E+06	298.5	1.75	20.91
4.32	282.92	.547E+06	456.5	1.83	19.98

4.89	283.49	.767E+06	705.7	2.02	18.12
5.46	284.06	.102E+07	1038.0	2.24	16.34
6.04	284.64	.130E+07	1444.1	2.45	14.98
6.61	285.21	.161E+07	1940.1	2.66	13.79
7.18	285.78	.194E+07	2522.6	2.86	12.80
7.76	286.36	.229E+07	3187.4	3.06	11.98
8.33	286.93	.267E+07	3942.3	3.25	11.28
8.91	287.51	.306E+07	4824.9	3.46	10.58
9.48	288.08	.347E+07	5795.7	3.67	9.99
10.05	288.65	.390E+07	6855.4	3.86	9.48
10.63	289.23	.434E+07	8005.0	4.05	9.04

	<---- hydrograph ---->				<-pi pe / channel ->	
	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8116)	523.77	0.49	7.75	2.83	0.31	0.80
OUTFLOW: ID= 1 (6152)	523.77	0.46	8.75	2.83	0.31	0.80

ADD HYD (8122)					
1 + 2 = 3					
	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0152):	124.37	0.386	7.25	6.33	
+ ID2= 2 (6152):	523.77	0.463	8.75	2.83	
=====					
ID = 3 (8122):	648.14	0.817	8.00	3.50	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0158)			
ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
	178.59	61.0	1.50
	Ia (mm)= 5.00		
	U. H. Tp(hrs)= 0.94		

Unit Hyd Opeak (cms)=	3.230
PEAK FLOW (cms)=	0.571 (i)
TIME TO PEAK (hrs)=	7.250
RUNOFF VOLUME (mm)=	6.789
TOTAL RAINFALL (mm)=	42.000
RUNOFF COEFFICIENT =	0.162

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0156)			
ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
	83.49	45.0	1.50
	Ia (mm)= 5.00		
	U. H. Tp(hrs)= 0.62		

Unit Hyd Opeak (cms)=	2.306
PEAK FLOW (cms)=	0.208 (i)
TIME TO PEAK (hrs)=	6.750
RUNOFF VOLUME (mm)=	3.858
TOTAL RAINFALL (mm)=	42.000
RUNOFF COEFFICIENT =	0.092

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1541)			
ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	Dir. Conn. (%)
	11.15	75.00	70.00
	Total Imp(%)= 75.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	8.36	2.79
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	272.64	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	55.44	1.94
over (min)=	15.00	60.00
Storage Coeff. (min)=	7.27 (ii)	59.00 (ii)
Unit Hyd. Tpeak (min)=	15.00	60.00
Unit Hyd. peak (cms)=	0.10	0.02

TOTALS			
PEAK FLOW (cms)=	1.10	0.01	1.102 (iii)
TIME TO PEAK (hrs)=	6.00	6.75	6.00
RUNOFF VOLUME (mm)=	40.00	2.85	28.85
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.07	0.69

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 31.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5154)			
IN= 2--> OUT= 1	DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)
		0.0000	0.0000
		0.0380	0.1987
		0.2160	0.2386
		0.4020	0.2749
		0.5580	0.3133
		0.7460	0.3544
		0.9370	0.3839
		1.0900	0.4129
		1.4900	0.4229
		0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1541)	11.150	1.102	6.00	28.85
OUTFLOW: ID= 1 (5154)	11.150	0.122	6.75	28.73

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 11.07
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.2187

CALIB NASHYD (1542)			
ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
	189.17	39.1	1.50
	Ia (mm)= 5.00		
	U. H. Tp(hrs)= 0.95		

Unit Hyd Opeak (cms)=	3.396
PEAK FLOW (cms)=	0.273 (i)
TIME TO PEAK (hrs)=	7.500
RUNOFF VOLUME (mm)=	3.130
TOTAL RAINFALL (mm)=	42.000
RUNOFF COEFFICIENT =	0.075

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9154)					
1 + 2 = 3					
	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (1542):	189.17	0.273	7.50	3.13	
+ ID2= 2 (5154):	11.15	0.122	6.75	28.73	
=====					
ID = 3 (9154):	200.32	0.379	7.00	4.55	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8118)					
1 + 2 = 3					
	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0156):	83.49	0.208	6.75	3.86	
+ ID2= 2 (9154):	200.32	0.379	7.00	4.55	
=====					
ID = 3 (8118):	283.81	0.587	6.75	4.35	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6158)	
IN= 2--> OUT= 1	Routing time step (min) = 15.00

<----- DATA FOR SECTION (1581.0) ----->

Distance	Elevation	Manning		
0.00	290.00	0.0400		
19.20	288.88	0.0400		
48.00	288.26	0.0400		
119.99	283.68	0.0400		
167.99	282.07	0.0400		
196.79	281.32	0.0400		
219.79	280.90	0.0400		
219.79	280.38	0.0400 / 0.0300	Main Channel	
220.04	279.65	0.0300	Main Channel	
220.79	279.65	0.0300	Main Channel	
221.54	279.65	0.0300	Main Channel	
221.79	280.39	0.0300 / 0.0400	Main Channel	
225.79	280.59	0.0400		
235.19	280.88	0.0400		
254.39	281.50	0.0400		
273.58	282.56	0.0400		
302.38	283.42	0.0400		
359.98	284.03	0.0400		
436.77	289.37	0.0400		
475.17	290.37	0.0400		

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.37	280.01	973E+03	0.5	0.86	31.78
0.73	280.38	210E+04	1.5	1.18	23.16
1.30	280.95	149E+05	8.6	0.95	28.91
1.86	281.51	579E+05	40.0	1.13	24.12
2.43	282.08	133E+06	118.5	1.46	18.75
2.99	282.64	236E+06	254.6	1.77	15.46
3.56	283.21	369E+06	450.3	2.00	13.67
4.13	283.78	542E+06	702.7	2.13	12.86
4.69	284.34	767E+06	1125.2	2.41	11.36
5.26	284.91	101E+07	1697.1	2.75	9.93
5.82	285.47	127E+07	2379.1	3.07	8.91
6.39	286.04	155E+07	3171.4	3.36	8.14
6.95	286.60	184E+07	4075.5	3.64	7.53
7.52	287.17	215E+07	5093.1	3.89	7.03
8.09	287.74	247E+07	6226.5	4.13	6.62
8.65	288.30	281E+07	7461.2	4.36	6.28
9.22	288.87	318E+07	8635.1	4.46	6.13
9.78	289.43	357E+07	10143.6	4.67	5.86
10.35	290.00	398E+07	11608.8	4.79	5.71

		<---- hydrograph ---->			<- pi pe / channel ->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8118)	283.81	0.59	6.75	4.35	0.39	0.88
OUTFLOW: ID= 1 (6158)	283.81	0.53	7.75	4.35	0.37	0.87

ADD HYD (8120)		AREA (ha)		OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3						
ID1= 1 (0158)	178.59	0.571	7.25	6.79		
+ ID2= 2 (6158)	283.81	0.533	7.75	4.35		
ID = 3 (8120):	462.40	1.102	7.50	5.29		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8124)		AREA (ha)		OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3						
ID1= 1 (8120):	462.40	1.102	7.50	5.29		
+ ID2= 2 (8122):	648.14	0.817	8.00	3.50		
ID = 3 (8124):	1110.54	1.904	7.75	4.25		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6160)		Routing time step (min)'
IN= 2--> OUT= 1		15.00

<----- DATA FOR SECTION (1601.0) ----->

Distance	Elevation	Manning		
0.00	261.02	0.0400		
29.89	254.30	0.0400		
40.75	252.58	0.0400		
62.49	251.30	0.0400		
114.11	250.94	0.0400		
130.41	248.58	0.0400		
141.28	247.50	0.0400		
154.86	246.87	0.0400		
155.20	246.70	0.0400		
160.20	246.30	0.0400 / 0.0300	Main Channel	
160.30	245.30	0.0300	Main Channel	
165.20	245.30	0.0300	Main Channel	
165.30	246.30	0.0300 / 0.0400	Main Channel	
168.45	246.96	0.0400		
195.62	249.20	0.0400		
203.77	250.82	0.0400		
225.50	256.78	0.0400		
244.52	261.14	0.0400		
255.39	261.95	0.0400		
268.97	261.92	0.0400		

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	245.80	120E+05	2.9	1.18	68.31
1.00	246.30	242E+05	8.5	1.70	47.47
1.92	247.22	825E+05	31.7	1.86	43.34
2.84	248.14	248E+06	102.5	2.00	40.39
3.76	249.06	506E+06	247.7	2.37	34.03
4.68	249.98	832E+06	486.8	2.83	28.49
5.60	250.90	121E+07	814.9	3.27	24.71
6.52	251.82	180E+07	1157.1	3.11	25.97
7.44	252.74	253E+07	1840.2	3.52	22.92
8.36	253.66	331E+07	2727.1	3.98	20.25
9.29	254.59	414E+07	3782.6	4.43	18.22
10.21	255.51	499E+07	5010.7	4.86	16.61
11.13	256.43	589E+07	6397.4	5.26	15.33
12.05	257.35	681E+07	7932.8	5.64	14.31
12.97	258.27	777E+07	9621.2	5.99	13.46
13.89	259.19	877E+07	11469.4	6.33	12.74
14.81	260.11	980E+07	13478.4	6.66	12.12
15.73	261.03	109E+08	15651.7	6.97	11.58
16.65	261.95	120E+08	17492.5	7.07	11.42

		<---- hydrograph ---->			<- pi pe / channel ->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8124)	1110.54	1.90	7.75	4.25	0.33	1.18
OUTFLOW: ID= 1 (6160)	1110.54	1.66	9.25	4.25	0.28	1.18

CALIB STANDHYD (1601)		Area Total (ha)	Imp(%)	Di r. Conn. (%)
ID= 1 DT=15.0 min		19.03	75.00	69.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	14.27	4.76
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	356.18	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	55.44	7.84
over (min)	15.00	45.00
Storage Coeff. (min)	8.53 (ii)	38.16 (ii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.09	0.03

		TOTALS
PEAK FLOW (cms)	1.78	0.06
TIME TO PEAK (hrs)	6.00	6.50
RUNOFF VOLUME (mm)	40.00	7.15
TOTAL RAI NFALL (mm)	42.00	42.00
RUNOFF COEFFICIENT	0.95	0.17

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 55.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5160) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	1.2730	0.6034
	0.0640	0.3377	1.5980	0.6540
	0.3690	0.4055	1.8600	0.7037
	0.6870	0.4676	2.2600	0.7137
	0.9520	0.5332	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1601)	19.030	1.806	6.00	29.82
OUTFLOW: ID= 1 (5160)	19.030	0.232	6.75	29.75

PEAK FLOW REDUCTION [Oout/Oi n] (%) = 12.84
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 0.3763

CALIB NASHYD (1602) ID= 1 DT=15.0 min	Area (ha)	(ha) = 299.79	Curve Number (CN) = 60.3
	la	(mm) = 5.00	# of Linear Res. (N) = 1.50
	U. H.	TP(hrs) = 1.42	

Unit Hyd Opeak (cms) = 3.592
 PEAK FLOW (cms) = 0.693 (i)
 TIME TO PEAK (hrs) = 8.250
 RUNOFF VOLUME (mm) = 6.663
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.159

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9160) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1602):	299.79	0.693	8.25	6.66
+ ID2= 2 (5160):	19.03	0.232	6.75	29.75
=====				
ID = 3 (9160):	318.82	0.836	7.25	8.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8126) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6160):	1110.54	1.657	9.25	4.25
+ ID2= 2 (9160):	318.82	0.836	7.25	8.04
=====				
ID = 3 (8126):	1429.36	2.413	9.00	5.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1621) ID= 1 DT=15.0 min	Area Total	(ha) = 46.07	Imp(%) = 45.00	Dir. Conn. (%) = 20.00
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	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	20.73	25.34
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	554.20	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) over (min)	55.44	9.32
Storage Coeff. (min)	11.13 (ii)	38.76 (ii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.08	0.03

	(cms)			*TOTALS*
PEAK FLOW	1.16	0.36		1.300 (iii)
TIME TO PEAK	(hrs) = 6.00	6.50		6.00
RUNOFF VOLUME	(mm) = 40.00	7.23		13.78
TOTAL RAINFALL	(mm) = 42.00	42.00		42.00
RUNOFF COEFFICIENT	= 0.95	0.17		0.33

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 51.1 la = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5162) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	3.0820	1.1997
	0.1560	0.5629	3.8700	1.3716
	0.8930	0.6512	4.5050	1.5398
	1.6620	0.8407	4.9050	1.5498
	2.3060	1.0035	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1621)	46.070	1.300	6.00	13.78
OUTFLOW: ID= 1 (5162)	46.070	0.117	8.75	13.76

PEAK FLOW REDUCTION [Oout/Oi n] (%) = 8.99
 TIME SHIFT OF PEAK FLOW (min) = 165.00
 MAXIMUM STORAGE USED (ha. m.) = 0.4218

CALIB STANDHYD (1622) ID= 1 DT=15.0 min	Area Total	(ha) = 144.07	Imp(%) = 26.00	Dir. Conn. (%) = 11.00
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	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	37.46	106.61
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	980.03	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) over (min)	55.44	4.87
Storage Coeff. (min)	15.00	60.00
Unit Hyd. Tpeak (min)	15.66 (ii)	51.49 (ii)
Unit Hyd. peak (cms)	15.00	60.00
	0.07	0.02

	(cms)			*TOTALS*
PEAK FLOW	1.74	0.82		1.980 (iii)
TIME TO PEAK	(hrs) = 6.00	6.75		6.00
RUNOFF VOLUME	(mm) = 40.00	5.97		9.71
TOTAL RAINFALL	(mm) = 42.00	42.00		42.00
RUNOFF COEFFICIENT	= 0.95	0.14		0.23

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 51.1 la = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9162) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1622):	144.07	1.980	6.00	9.71
+ ID2= 2 (5162):	46.07	0.117	8.75	13.76
=====				
ID = 3 (9162):	190.14	2.026	6.00	10.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8128)	AREA	OPEAK	TPEAK	R. V.
1 + 2 =	3	(ha)	(cms)	(hrs)	(mm)
ID1= 1	(8126):	1429.36	2.413	9.00	5.09
+ ID2= 2	(8132):	190.14	2.026	6.00	10.69
ID = 3	(8128):	1619.50	2.924	8.00	5.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8130)	AREA	OPEAK	TPEAK	R. V.
1 + 2 =	3	(ha)	(cms)	(hrs)	(mm)
ID1= 1	(8128):	1619.50	2.924	8.00	5.75
+ ID2= 2	(8132):	234.24	3.062	6.00	21.90
ID = 3	(8130):	1853.74	5.549	6.00	7.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6166)
IN= 2---> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1661.0) ----->

Distance	Elevation	Manning	
0.00	248.36	0.0500	
41.67	246.98	0.0500	
95.82	244.93	0.0500	
149.98	242.42	0.0500	
191.64	241.88	0.0500	
224.97	239.90	0.0500	
229.13	239.35	0.0500	
233.30	237.77	0.0500	
237.47	236.57	0.0500	
240.63	235.60	0.0500 / 0.0300	Main Channel
241.63	234.10	0.0300	Main Channel
247.13	234.10	0.0300	Main Channel
247.53	235.60	0.0300 / 0.0500	Main Channel
254.13	236.25	0.0500	
266.63	236.77	0.0500	
283.29	237.84	0.0500	
291.63	238.89	0.0500	
329.12	244.66	0.0500	
370.78	249.55	0.0500	
412.44	253.17	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(mi n)
0.75	234.85	.130E+05	6.0	1.36	36.45
1.50	235.60	.276E+05	18.1	1.95	25.41
2.25	236.35	.544E+05	40.0	2.18	22.69
3.00	237.10	.117E+06	80.1	2.04	24.32
3.75	237.85	.213E+06	148.9	2.07	23.89
4.50	238.60	.334E+06	255.1	2.27	21.85
5.25	239.35	.473E+06	395.8	2.49	19.90
6.00	240.10	.631E+06	563.8	2.65	18.66
6.75	240.85	.824E+06	773.2	2.79	17.77
7.50	241.60	.106E+07	1037.9	2.92	16.97
8.26	242.36	.135E+07	1326.7	2.92	16.94
9.01	243.11	.173E+07	1758.5	3.01	16.44
9.76	243.86	.217E+07	2285.9	3.13	15.81
10.51	244.61	.265E+07	2909.5	3.26	15.17
11.26	245.36	.318E+07	3610.1	3.37	14.67
12.01	246.11	.377E+07	4417.4	3.48	14.21
12.76	246.86	.441E+07	5348.0	3.60	13.75
13.51	247.61	.512E+07	6394.0	3.71	13.34
14.26	248.36	.589E+07	7580.3	3.82	12.95

AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8130)	1853.74	5.55	6.00	7.79	1.36
OUTFLOW: ID= 1 (6166)	1853.74	4.83	7.50	7.79	0.61

ADD HYD	(8136)	AREA	OPEAK	TPEAK	R. V.
1 + 2 =	3	(ha)	(cms)	(hrs)	(mm)
ID1= 1	(6166):	1853.74	4.829	7.50	7.79
+ ID2= 2	(8134):	434.70	7.864	6.00	21.79
ID = 3	(8136):	2288.44	9.668	6.00	10.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (2021)
ID= 1 DT=15.0 mi n

Area	(ha)=	107.32	Di r. Conn. (%)=	27.00
Total	Imp(%)=	45.00		
Surface Area	(ha)=	48.29	PERVIOUS (i)	59.03
Dep. Storage	(mm)=	2.00		5.00
Average Slope	(%)=	0.50		0.50
Length	(m)=	845.85		40.00
Mannings n	=	0.013		0.250
Max. Eff. Inten. (mm/hr)=		55.44		16.90
over (mi n)		15.00		45.00
Storage Coeff. (mi n)=		14.34 (ii)		36.12 (ii)
Unit Hyd. Tpeak (mi n)=		15.00		45.00
Unit Hyd. peak (cms)=		0.07		0.03
PEAK FLOW (cms)=		3.31		1.56
TIME TO PEAK (hrs)=		6.00		6.50
RUNOFF VOLUME (mm)=		40.00		13.68
TOTAL RAINFALL (mm)=		42.00		42.00
RUNOFF COEFFICIENT =		0.95		0.33
			TOTALS	3.946 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5202)
IN= 2---> OUT= 1
DT= 15.0 mi n

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha. m.)	(cms)	(ha. m.)
0.0000	0.0000	5.1480	2.8069
0.3630	1.2717	6.4200	3.2412
1.5670	1.4796	7.4640	3.6910
2.7880	1.9296	7.8640	3.7010
3.8580	2.3404	0.0000	0.0000

INFLOW : ID= 2 (2021)	AREA	OPEAK	TPEAK	R. V.
ID= 1 (5202)	(ha)	(cms)	(hrs)	(mm)
	107.320	3.946	6.00	20.78
	107.320	0.819	7.50	20.78

PEAK FLOW REDUCTION [Out/Oi n] (%) = 20.74
TIME SHIFT OF PEAK FLOW (mi n) = 90.00
MAXIMUM STORAGE USED (ha. m.) = 1.3523

CALIB
STANDHYD (2022)
ID= 1 DT=15.0 mi n

Area	(ha)=	361.21	Di r. Conn. (%)=	36.00
Total	Imp(%)=	60.00		
Surface Area	(ha)=	216.73	PERVIOUS (i)	144.48
Dep. Storage	(mm)=	2.00		5.00
Average Slope	(%)=	0.50		0.50
Length	(m)=	1551.79		40.00
Mannings n	=	0.013		0.250
Max. Eff. Inten. (mm/hr)=		37.80		23.84
over (mi n)		30.00		45.00
Storage Coeff. (mi n)=		24.05 (ii)		43.03 (ii)

Unit Hyd. Tpeak (min)=	30.00	45.00	
Unit Hyd. peak (cms)=	0.04	0.03	
			TOTALS
PEAK FLOW (cms)=	9.90	4.87	13.487 (iii)
TIME TO PEAK (hrs)=	6.25	6.50	6.25
RUNOFF VOLUME (mm)=	40.00	15.77	24.49
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.38	0.58

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8110)				
1 + 2 = 3				
	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (2022):	361.21	13.487	6.25	24.49
+ ID2= 2 (5202):	107.32	0.819	7.50	20.78
=====				
ID = 3 (8110):	468.53	13.711	6.25	23.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (0102)			
ID= 1 DT=15.0 min			
Area (ha)=	466.00	Dir. Conn. (%)=	9.00
Total Imp(%)=	23.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	107.18	358.82
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1762.57	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	37.80	4.37	
over (min)	30.00	75.00	
Storage Coeff. (min)=	25.96 (ii)	63.38 (ii)	
Unit Hyd. Tpeak (min)=	30.00	75.00	
Unit Hyd. peak (cms)=	0.04	0.02	
			TOTALS
PEAK FLOW (cms)=	3.09	2.11	3.952 (iii)
TIME TO PEAK (hrs)=	6.25	7.00	6.25
RUNOFF VOLUME (mm)=	40.00	5.46	8.56
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.13	0.20

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 49.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0100)			
ID= 1 DT=15.0 min			
Area (ha)=	693.84	Curve Number (CN)=	40.0
Ia (mm)=	5.00	# of Linear Res. (N)=	1.50
U.H. Tp(hrs)=	1.95		

Unit Hyd. Qpeak (cms)=	6.087
PEAK FLOW (cms)=	0.615 (i)
TIME TO PEAK (hrs)=	9.500
RUNOFF VOLUME (mm)=	3.263
TOTAL RAINFALL (mm)=	42.000
RUNOFF COEFFICIENT =	0.078

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8000)	
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1 + 2 = 3				
	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0100):	693.84	0.615	9.50	3.26
+ ID2= 2 (0102):	466.00	3.952	6.25	8.56
=====				
ID = 3 (8000):	1159.84	4.251	6.25	5.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6104)	
IN= 2--> OUT= 1	
Routing time step (min)'= 15.00	

<----- DATA FOR SECTION (1041.0) ----->		
Distance	Elevation	Manning
0.00	330.16	0.0450
50.03	328.36	0.0450
89.34	326.33	0.0450
117.94	324.30	0.0450
160.82	322.75	0.0450
178.34	319.96	0.0450
183.34	319.81	0.0450
184.34	319.38	0.0450 / 0.0300
185.09	318.78	0.0300
185.84	318.72	0.0300
186.84	319.32	0.0300 / 0.0450
193.84	319.70	0.0450
198.84	320.38	0.0450
200.13	320.81	0.0450
218.00	322.49	0.0450
239.44	323.49	0.0450
250.17	323.96	0.0450
275.18	325.20	0.0450
310.92	326.47	0.0450
353.81	330.00	0.0450

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)
0.30	319.02	.624E+03	0.1	0.45	73.00
0.60	319.32	.181E+04	0.6	0.67	49.46
1.23	319.95	.128E+05	4.9	0.77	43.38
1.86	320.58	.396E+05	19.1	0.96	34.50
2.48	321.20	.757E+05	43.5	1.15	28.98
3.11	321.83	.125E+06	81.2	1.30	25.58
3.74	322.46	.187E+06	135.9	1.45	22.92
4.37	323.09	.269E+06	195.6	1.45	22.90
5.00	323.72	.387E+06	287.8	1.48	22.44
5.63	324.35	.545E+06	428.4	1.56	21.22
6.25	324.97	.735E+06	630.5	1.71	19.42
6.88	325.60	.952E+06	876.1	1.83	18.12
7.51	326.23	.120E+07	1177.0	1.95	17.03
8.14	326.86	.149E+07	1554.4	2.08	15.93
8.77	327.49	.179E+07	2009.7	2.23	14.88
9.40	328.12	.213E+07	2533.9	2.37	13.99
10.02	328.74	.249E+07	3112.6	2.49	13.31
10.65	329.37	.288E+07	3760.9	2.60	12.75
11.28	330.00	.330E+07	4494.8	2.71	12.23

	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8000)	1159.84	4.25	6.25	5.39	1.13	0.75
OUTFLOW: ID= 1 (6104)	1159.84	3.09	7.25	5.39	0.96	0.72

CALIB STANDHYD (1041)			
ID= 1 DT=15.0 min			
Area (ha)=	0.67	Dir. Conn. (%)=	70.00
Total Imp(%)=	75.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.50	0.17
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	66.83	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	55.44	2.85
over (min)	15.00	60.00
Storage Coeff. (min)=	3.13 (ii)	47.55 (ii)
Unit Hyd. Tpeak (min)=	15.00	60.00

Unit Hyd. peak (cms)= 0.11 0.02
 PEAK FLOW (cms)= 0.07 0.00
 TIME TO PEAK (hrs)= 6.00 6.75
 RUNOFF VOLUME (mm)= 40.00 3.52
 TOTAL RAINFALL (mm)= 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.08

TOTALS
 0.072 (iii)
 6.00
 29.00
 42.00
 0.69

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 36.5 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5104)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.0450	0.0213
0.0020	0.0120	0.0560	0.0231
0.0130	0.0144	0.0660	0.0248
0.0240	0.0165	0.4660	0.0348
0.0340	0.0189	0.0000	0.0000

INFLOW : ID= 2 (1041) AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 0.670 0.072 6.00 29.00
 OUTFLOW: ID= 1 (5104) 0.670 0.007 6.75 26.74

PEAK FLOW REDUCTION [Qout/Oin] (%) = 9.88
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 0.0132

CALIB NASHYD (1042)
 ID= 1 DT=15.0 min

Area (ha) = 527.14 Curve Number (CN) = 42.6
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp (hrs) = 1.34

Unit Hyd Opeak (cms) = 6.731
 PEAK FLOW (cms) = 0.682 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 3.586
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.085

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9104)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1042):	527.14	0.682	8.00	3.59
+ ID2= 2 (5104):	0.67	0.007	6.75	26.74
=====	=====	=====	=====	=====
ID = 3 (9104):	527.81	0.687	8.00	3.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8002)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6104):	1159.84	3.088	7.25	5.39
+ ID2= 2 (9104):	527.81	0.687	8.00	3.62
=====	=====	=====	=====	=====
ID = 3 (8002):	1687.65	3.732	7.25	4.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1061)

Area (ha) = 32.26

| ID= 1 DT=15.0 min | Total Imp(%)= 67.00 Di r. Conn.(%)= 51.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 21.61 10.65
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 463.75 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 55.44 11.63
 over (min) 15.00 45.00
 Storage Coeff. (min)= 10.00 (ii) 35.29 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.09 0.03

PEAK FLOW (cms)= 2.14 0.20
 TIME TO PEAK (hrs)= 6.00 6.50
 RUNOFF VOLUME (mm)= 40.00 8.73
 TOTAL RAINFALL (mm)= 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.21

TOTALS
 2.218 (iii)
 6.00
 24.68
 42.00
 0.59

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 56.4 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5106)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.1580	0.9744
0.1090	0.5252	2.7090	1.0694
0.6250	0.6260	3.1540	1.1625
1.1640	0.7386	3.5540	1.1725
1.6140	0.8505	0.0000	0.0000

INFLOW : ID= 2 (1061) AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 32.260 2.218 6.00 24.68
 OUTFLOW: ID= 1 (5106) 32.260 0.208 7.50 24.64

PEAK FLOW REDUCTION [Qout/Oin] (%) = 9.35
 TIME SHIFT OF PEAK FLOW (min) = 90.00
 MAXIMUM STORAGE USED (ha. m.) = 0.5448

CALIB NASHYD (1062)
 ID= 1 DT=15.0 min

Area (ha) = 227.27 Curve Number (CN) = 57.5
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp (hrs) = 1.09

Unit Hyd Opeak (cms) = 3.556
 PEAK FLOW (cms) = 0.579 (i)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 6.037
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.144

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9106)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1062):	227.27	0.579	7.75	6.04
+ ID2= 2 (5106):	32.26	0.208	7.50	24.64
=====	=====	=====	=====	=====
ID = 3 (9106):	259.53	0.784	7.50	8.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8004)

1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8002):	1687.65	3.732	7.25	4.84
+ ID2= 2 (9106):	259.53	0.784	7.50	8.35
=====				
ID = 3 (8004):	1947.18	4.505	7.25	5.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6108)
IN= 2---> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1081.0) ----->

Distance	Elevation	Manning	
0.00	326.06	0.0400	
12.06	325.74	0.0400	
30.15	324.28	0.0400	
59.27	320.35	0.0400	
72.37	317.60	0.0400	
87.45	314.52	0.0400	
93.48	313.91	0.0400	
105.54	313.55	0.0400	
106.17	313.40	0.0400 / 0.0300	Main Channel
110.62	313.04	0.0300	Main Channel
110.82	312.97	0.0300	Main Channel
111.57	313.08	0.0300	Main Channel
111.67	313.48	0.0300 / 0.0400	Main Channel
129.66	316.62	0.0400	
150.77	318.95	0.0400	
180.92	319.61	0.0400	
205.04	321.23	0.0400	
232.18	322.09	0.0400	
268.37	322.31	0.0400	
298.52	326.00	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.43	313.40	.244E+04	0.7	0.59	58.83
1.09	314.06	.214E+05	10.4	1.01	34.24
1.76	314.73	.600E+05	40.2	1.39	24.89
2.42	315.39	.109E+06	92.1	1.75	19.80
3.08	316.05	.169E+06	166.5	2.05	16.88
3.75	316.72	.237E+06	264.4	2.31	14.97
4.41	317.38	.318E+06	386.3	2.52	13.72
5.07	318.04	.411E+06	539.3	2.72	12.71
5.74	318.71	.517E+06	725.5	2.91	11.88
6.40	319.37	.642E+06	909.2	2.94	11.77
7.06	320.03	.806E+06	1179.3	3.04	11.38
7.72	320.69	.989E+06	1515.9	3.18	10.87
8.39	321.36	.119E+07	1895.5	3.30	10.49
9.05	322.02	.143E+07	2310.2	3.36	10.30
9.71	322.68	.172E+07	2763.2	3.33	10.39
10.38	323.35	.205E+07	3477.6	3.53	9.81
11.04	324.01	.238E+07	4284.3	3.73	9.27
11.70	324.67	.274E+07	5154.0	3.91	8.85
12.37	325.34	.311E+07	6104.0	4.08	8.48

<----- hydrograph -----> <-- pi pe / channel -->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8004)	1947.18	4.50	7.25	5.31	0.69	0.70
OUTFLOW: ID= 1 (6108)	1947.18	3.94	8.25	5.31	0.65	0.68

CALIB STANDHYD (1081)
ID= 1 DT=15.0 mi n
Area (ha)= 40.81
Total Imp(%)= 75.00
Di r. Conn.(%)= 70.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	30.61	10.20
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	521.60	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over	55.44 (min)	1.85 (min)
Storage Coeff. (min)	10.73 (ii)	63.48 (ii)
Unit Hyd. Tpeak (min)	15.00	75.00

Unit Hyd. peak (cms)=	0.09	0.02	*TOTALS*
PEAK FLOW (cms)=	3.63	0.03	3.641 (iii)
TIME TO PEAK (hrs)=	6.00	7.00	6.00
RUNOFF VOLUME (mm)=	40.00	2.72	28.81
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.06	0.69

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 30.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5108)
IN= 2---> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.7300	1.2970
0.1380	0.7271	3.4270	1.4050
0.7910	0.8733	3.9900	1.5110
1.4720	1.0061	4.3900	1.5210
2.0420	1.1467	0.0000	0.0000

INFLOW : ID= 2 (1081)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
40.810	40.810	3.641	6.00	28.81
OUTFLOW: ID= 1 (5108)	40.810	0.440	6.75	28.78

PEAK FLOW REDUCTION [Qout/Qi n] (%)	TIME SHIF T OF PEAK FLOW (mi n)	MAXIMUM STORAGE USED (ha. m.)
12.08	45.00	0.7956

CALIB NASHYD (1082)
ID= 1 DT=15.0 mi n
Area (ha)= 166.24
Ia (mm)= 5.00
U. H. Tp(hrs)= 0.89
Curve Number (CN)= 32.2
of Li near Res. (N)= 1.50

Unit Hyd Opeak (cms)=	3.171
PEAK FLOW (cms)=	0.189 (i)
TIME TO PEAK (hrs)=	7.250
RUNOFF VOLUME (mm)=	2.365
TOTAL RAINFALL (mm)=	42.000
RUNOFF COEFFICIENT =	0.056

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9108)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1082):	166.24	0.189	7.25	2.37
+ ID2= 2 (5108):	40.81	0.440	6.75	28.78
=====				
ID = 3 (9108):	207.05	0.620	6.75	7.57

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8006)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6108):	1947.18	3.939	8.25	5.31
+ ID2= 2 (9108):	207.05	0.620	6.75	7.57
=====				
ID = 3 (8006):	2154.23	4.359	8.25	5.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1101)
Area (ha)= 60.97

| ID= 1 DT=15.0 min | Total Imp(%)= 69.00 Dir. Conn.(%)= 55.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	42.07	18.90
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	637.55	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	55.44	9.81
over (min)	15.00	45.00
Storage Coeff. (min)=	12.10 (ii)	39.18 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03
		TOTALS
PEAK FLOW (cms)=	4.09	0.28
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	40.00	7.60
TOTAL RAINFALL (mm)=	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.18

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 52.7 Ia = Dep Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5110)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.0780	1.8615
0.2060	1.0120	5.1210	2.0372
1.1810	1.2083	5.9610	2.2096
2.2000	1.4181	6.3610	2.2196
3.0510	1.6293	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1101)	60.970	4.204	6.00	25.42
OUTFLOW: ID= 1 (5110)	60.970	0.431	7.25	25.40

PEAK FLOW REDUCTION [Qout/Oin] (%) = 10.25
TIME SHIFT OF PEAK FLOW (min) = 75.00
MAXIMUM STORAGE USED (ha.m.) = 1.0573

CALIB NASHYD (1102)
ID= 1 DT=15.0 min

Area (ha) = 262.95 Curve Number (CN) = 55.4
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U.H. Tp(hrs) = 1.29

Unit Hyd Opeak (cms) = 3.483

PEAK FLOW (cms) = 0.553 (i)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 5.629
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.134

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9110)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1102):	262.95	0.553	8.00	5.63
+ ID2= 2 (5110):	60.97	0.431	7.25	25.40
ID = 3 (9110):	323.92	0.959	7.50	9.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8008)

1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8006):	2154.23	4.359	8.25	5.52
+ ID2= 2 (9110):	323.92	0.959	7.50	9.35
ID = 3 (8008):	2478.15	5.274	8.00	6.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6112)
IN= 2---> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1121.0) ----->

Distance	Elevation	Manning
0.00	320.59	0.0400
13.48	317.93	0.0400
40.45	311.52	0.0400
64.04	307.68	0.0400
74.15	307.01	0.0400
91.00	307.22	0.0400
107.86	305.21	0.0400
111.23	304.86	0.0400
113.32	304.60	0.0400 / 0.0300
117.77	304.24	0.0300
117.97	304.17	0.0300
118.72	304.28	0.0300
118.82	304.68	0.0300 / 0.0400
124.71	304.76	0.0400
171.90	306.30	0.0400
205.60	307.24	0.0400
239.31	308.24	0.0400
262.90	310.64	0.0400
289.86	314.34	0.0400
333.68	317.66	0.0400

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.43	304.60	.372E+04	0.6	0.54	97.65
1.12	305.29	.463E+05	12.5	0.85	61.89
1.80	305.97	.149E+06	53.0	1.13	46.85
2.49	306.66	.311E+06	134.2	1.37	38.62
3.18	307.35	.554E+06	261.6	1.50	35.27
3.87	308.04	.894E+06	490.7	1.74	30.37
4.55	308.72	.129E+07	833.0	2.05	25.75
5.24	309.41	.171E+07	1269.8	2.36	22.39
5.93	310.10	.215E+07	1789.0	2.64	20.01
6.62	310.79	.261E+07	2392.7	2.90	18.21
7.30	311.47	.310E+07	3087.8	3.16	16.74
7.99	312.16	.361E+07	3873.6	3.40	15.52
8.68	312.85	.413E+07	4741.0	3.64	14.52
9.37	313.54	.467E+07	5689.3	3.86	13.69
10.05	314.22	.523E+07	6718.4	4.07	12.97
10.74	314.91	.581E+07	7770.7	4.24	12.46
11.43	315.60	.641E+07	8897.5	4.40	12.01
12.12	316.29	.704E+07	10113.2	4.55	11.61
12.80	316.97	.770E+07	11420.1	4.70	11.24

<----- hydrograph -----> <--pi pe / channel -->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8008)	2478.15	5.27	8.00	6.02	0.70	0.63
OUTFLOW: ID= 1 (6112)	2478.15	4.31	9.75	6.02	0.64	0.61

CALIB STANDHYD (1121)
ID= 1 DT=15.0 min

Area (ha) = 9.96 Dir. Conn.(%) = 25.00
Total Imp(%) = 55.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	5.48	4.48
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	257.68	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 55.44 3.72
over (min) 15.00 60.00
Storage Coeff. (min)= 7.03 (ii) 46.94 (ii)
Unit Hyd. Tpeak (min)= 15.00 60.00

Unit Hyd. peak (cms) = 0.10 0.02

PEAK FLOW (cms) = 0.35 0.03
 TIME TO PEAK (hrs) = 6.00 6.75
 RUNOFF VOLUME (mm) = 40.00 3.34
 TOTAL RAINFALL (mm) = 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.08

TOTALS
 0.361 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 26.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5112)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.6660	0.2787
0.0340	0.1406	0.8360	0.3122
0.1930	0.1653	0.9740	0.3450
0.3590	0.2034	1.3740	0.3550
0.4980	0.2383	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
9.960	0.361	6.00	12.50
9.960	0.021	8.50	12.40

INFLOW: ID= 2 (1121)
 OUTFLOW: ID= 1 (5112)

PEAK FLOW REDUCTION [Qout/Qin](%) = 5.75
 TIME SHIFT OF PEAK FLOW (min) = 150.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0860

CALIB NASHYD (1122)
 ID= 1 DT=15.0 min

Area (ha) = 404.93
 Ia (mm) = 5.00
 U.H. Tp(hrs) = 1.90

Curve Number (CN) = 31.1
 # of Linear Res. (N) = 1.50

Unit Hyd Opeak (cms) = 3.635

PEAK FLOW (cms) = 0.254 (i)
 TIME TO PEAK (hrs) = 9.500
 RUNOFF VOLUME (mm) = 2.274
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.054

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9112)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1122):	404.93	0.254	9.50	2.27
+ ID2= 2 (5112):	9.96	0.021	8.50	12.40
ID= 3 (9112):	414.89	0.275	9.25	2.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8010)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6112):	2478.15	4.307	9.75	6.02
+ ID2= 2 (9112):	414.89	0.275	9.25	2.52
ID= 3 (8010):	2893.04	4.581	9.75	5.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6114)
 IN= 2--> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (1141.0) ----->

Distance	Elevation	Manning
0.00	312.27	0.0500
14.07	309.70	0.0500
28.14	306.51	0.0500
35.17	305.81	0.0500
42.20	304.95	0.0500
52.75	304.24	0.0500
77.37	303.61	0.0500
98.47	301.33	0.0500
131.76	300.40	0.0500 / 0.0300
136.21	300.04	0.0300
136.41	299.97	0.0300
137.16	300.08	0.0300
137.26	300.48	0.0300 / 0.0500
140.68	300.53	0.0500
179.36	302.20	0.0500
214.53	303.82	0.0500
242.66	305.27	0.0500
288.38	307.66	0.0500
316.52	311.76	0.0500
348.17	313.48	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.43	300.40	.366E+04	0.7	0.59	88.31
1.05	301.02	.505E+05	12.1	0.75	69.49
1.68	301.65	.165E+06	52.7	0.99	52.28
2.30	302.27	.324E+06	130.3	1.26	41.40
2.93	302.90	.520E+06	247.1	1.48	35.08
3.55	303.52	.754E+06	406.8	1.68	30.91
4.18	304.15	.104E+07	586.1	1.76	29.55
4.80	304.77	.138E+07	856.0	1.93	26.96
5.43	305.40	.177E+07	1201.1	2.12	24.56
6.05	306.02	.219E+07	1612.2	2.30	22.63
6.68	306.65	.264E+07	2090.6	2.47	21.07
7.30	307.27	.313E+07	2653.0	2.64	19.65
7.93	307.90	.364E+07	3303.9	2.83	18.36
8.55	308.52	.417E+07	4053.9	3.03	17.14
9.18	309.15	.471E+07	4871.9	3.22	16.12
9.80	309.77	.527E+07	5756.2	3.41	15.25
10.43	310.40	.584E+07	6699.8	3.58	14.52
11.05	311.02	.642E+07	7710.5	3.74	13.89
11.68	311.65	.702E+07	8788.5	3.90	13.32

<---- hydrograph ----> <- pipe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8010)	2893.04	4.58	9.75	5.52	0.64	0.63
OUTFLOW: ID= 1 (6114)	2893.04	4.14	11.25	5.52	0.62	0.63

CALIB STANDHYD (1141)
 ID= 1 DT=15.0 min

Area Total (ha) = 19.45
 Imp(%) = 45.00
 Dir. Conn. (%) = 15.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	8.75	10.70
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	360.09	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 55.44 over (min) 15.00
 Storage Coeff. (min) = 8.59 (ii) 37.88 (ii)
 Unit Hyd. Tpeak (min) = 15.00
 Unit Hyd. peak (cms) = 0.09 0.03

TOTALS
 PEAK FLOW (cms) = 0.40 0.13 0.448 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 40.00 5.94 11.05
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.14 0.26

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

- CN* = 43.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5114)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2---> OUT= 1 DT= 15.0 min	0.0000	0.0000	1.3010	0.5065
	0.0660	0.2376	1.6340	0.5791
	0.3770	0.2749	1.9020	0.6501
	0.7020	0.3549	2.3020	0.6601
	0.9730	0.4237	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1141)	19.450	0.448	6.00	11.05
OUTFLOW: ID= 1 (5114)	19.450	0.039	8.75	11.00

PEAK FLOW REDUCTION [Qout/Qin] (%) = 8.80
 TIME SHIFT OF PEAK FLOW (min) = 165.00
 MAXIMUM STORAGE USED (ha.m.) = 0.1420

CALIB NASHYD (1142)	Area (ha)	Ia (mm)	U.H. Tp (hrs)	Curve Number (CN)	# of Linear Res. (N)
ID= 1 DT=15.0 min	268.19	5.00	1.09	51.3	1.50

Unit Hyd Qpeak (cms) = 4.197
 PEAK FLOW (cms) = 0.550 (i)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 4.878
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.116

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9114)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (1142):	268.19	0.550	7.75	4.88
+ ID2= 2 (5114):	19.45	0.039	8.75	11.00
ID = 3 (9114):	287.64	0.588	8.00	5.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8012)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (6114):	2893.04	4.137	11.25	5.52
+ ID2= 2 (9114):	287.64	0.588	8.00	5.29
ID = 3 (8012):	3180.68	4.524	11.00	5.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0116)	Area (ha)	Ia (mm)	U.H. Tp (hrs)	Curve Number (CN)	# of Linear Res. (N)
ID= 1 DT=15.0 min	206.43	5.00	0.80	64.0	1.50

Unit Hyd Qpeak (cms) = 4.394
 PEAK FLOW (cms) = 0.833 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 7.503
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.179

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8014)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0116):	206.43	0.833	7.00	7.50
+ ID2= 2 (8012):	3180.68	4.524	11.00	5.50
ID = 3 (8014):	3387.11	4.914	10.75	5.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6118)	Routing time step (min)
IN= 2---> OUT= 1	15.00

Distance	Elevation	Manning
0.00	307.18	0.0450
24.89	299.75	0.0450
37.34	296.39	0.0450
71.57	292.75	0.0450
96.46	291.12	0.0450
108.90	289.43	0.0450
112.02	288.86	0.0450
115.13	288.46	0.0450
117.24	288.32	0.0450 / 0.0350
117.49	288.15	0.0350
118.24	288.00	0.0350
118.99	288.20	0.0350
119.24	288.36	0.0350 / 0.0450
121.35	288.40	0.0450
149.35	289.36	0.0450
171.14	290.47	0.0450
208.47	293.14	0.0450
236.48	295.17	0.0450
280.04	299.48	0.0450
308.04	307.00	0.0450

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.32	288.32	.116E+04	0.2	0.62	81.92
1.30	289.30	.631E+05	22.2	1.07	47.43
2.29	290.29	.218E+06	117.5	1.64	30.95
3.27	291.27	.448E+06	314.6	2.15	23.72
4.25	292.25	.756E+06	625.6	2.53	20.13
5.24	293.24	.115E+07	1093.2	2.91	17.50
6.22	294.22	.161E+07	1731.6	3.28	15.50
7.20	295.20	.214E+07	2547.4	3.63	14.01
8.19	296.19	.274E+07	3584.9	4.00	12.72
9.17	297.17	.338E+07	4859.1	4.39	11.60
10.15	298.15	.407E+07	6341.5	4.76	10.70
11.13	299.13	.480E+07	8024.6	5.11	9.97
12.12	300.12	.556E+07	10006.6	5.49	9.27
13.10	301.10	.635E+07	12244.6	5.89	8.64
14.08	302.08	.716E+07	14681.7	6.26	8.13
15.07	303.07	.799E+07	17316.5	6.62	7.69
16.05	304.05	.884E+07	20148.1	6.96	7.31
17.03	305.03	.971E+07	23176.2	7.29	6.98
18.02	306.02	.106E+08	26400.8	7.61	6.69

INFLOW : ID= 2 (8014)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW: ID= 1 (6118)	3387.11	4.64	12.25	5.62	0.52	0.68

CALIB NASHYD (0118)	Area (ha)	Ia (mm)	U.H. Tp (hrs)	Curve Number (CN)	# of Linear Res. (N)
ID= 1 DT=15.0 min	340.53	5.00	1.12	45.0	1.50

Unit Hyd Qpeak (cms) = 5.200
 PEAK FLOW (cms) = 0.548 (i)
 TIME TO PEAK (hrs) = 8.000

RUNOFF VOLUME (mm)= 3.906
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.093

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0118):	340.53	0.548	8.00	3.91
+ ID2= 2 (6118):	3387.11	4.638	12.25	5.62

ID = 3 (8016):	3727.64	4.949	12.00	5.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6122)
 IN= 2--> OUT= 1 Routing time step (min)' = 15.00

----- DATA FOR SECTION (1221.0) -----

Distance	Elevation	Manning	
0.00	277.00	0.0500	
52.18	272.82	0.0500	
86.97	269.38	0.0500	
134.80	267.35	0.0500	
160.89	263.85	0.0500	
169.58	262.88	0.0500	
178.28	262.45	0.0500	
195.67	262.07	0.0500	
213.07	260.99	0.0500	
226.01	259.80	0.0500 / 0.0300	Main Channel
226.11	259.30	0.0300	Main Channel
230.11	259.30	0.0300	Main Channel
230.21	259.80	0.0300 / 0.0500	Main Channel
235.01	260.20	0.0500	
239.15	260.57	0.0500	
243.50	260.93	0.0500	
300.03	264.43	0.0500	
339.16	268.34	0.0500	
373.95	272.36	0.0500	
430.48	277.46	0.0500	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	259.80	.626E+04	2.5	1.23	41.35
1.41	260.71	.462E+05	21.8	1.44	35.27
2.31	261.61	.149E+06	79.6	1.63	31.19
3.22	262.52	.340E+06	200.1	1.80	28.34
4.12	263.42	.636E+06	446.8	2.15	23.71
5.03	264.33	.995E+06	819.9	2.52	20.23
5.93	265.23	.141E+07	1337.0	2.90	17.56
6.84	266.14	.187E+07	1984.8	3.25	15.67
7.74	267.04	.237E+07	2766.0	3.57	14.26
8.65	267.95	.292E+07	3596.9	3.76	13.53
9.55	268.85	.355E+07	4575.4	3.93	12.95
10.46	269.76	.427E+07	5814.9	4.16	12.23
11.36	270.66	.503E+07	7319.0	4.44	11.46
12.27	271.57	.584E+07	9009.7	4.71	10.81
13.17	272.47	.670E+07	10886.3	4.96	10.26
14.08	273.38	.761E+07	12893.2	5.17	9.84
14.98	274.28	.858E+07	15098.1	5.37	9.48
15.89	275.19	.961E+07	17527.6	5.57	9.14
16.79	276.09	.107E+08	20190.3	5.76	8.83

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8016)	3727.64	4.95	12.00	5.46	0.61	1.25
OUTFLOW: ID= 1 (6122)	3727.64	4.87	12.50	5.46	0.61	1.25

CALIB STANDHYD (1221)
 ID= 1 DT=15.0 min
 Area (ha)= 117.91
 Total Imp(%)= 44.00
 Dir. Conn.(%)= 16.00
 IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 51.88 66.03
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 886.60 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 55.44 4.50
 Storage over (min)= 15.00 60.00
 Unit Hyd. Tpeak (min)= 14.75 (ii) 51.72 (ii)
 Unit Hyd. peak (cms)= 15.00 60.00
 Unit Hyd. peak (cms)= 0.07 0.02
 PEAK FLOW (cms)= 2.13 0.47
 TIME TO PEAK (hrs)= 6.00 6.75
 RUNOFF VOLUME (mm)= 40.00 4.47
 TOTAL RAINFALL (mm)= 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.11

TOTALS
 2.269 (iii)
 6.00
 10.15
 42.00
 0.24

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 36.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5122)
 IN= 2--> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	7.8880	3.0390
	0.3990	1.4100	9.9040	3.4850
	2.2850	1.6269	11.5290	3.9214
	4.2550	2.1164	11.9290	3.9314
	5.9010	2.5338	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1221)	117.910	2.269	6.00	10.15
OUTFLOW: ID= 1 (5122)	117.910	0.220	9.00	10.15

PEAK FLOW REDUCTION [Qout/Qin] (%) = 9.70
 TIME SHIFT OF PEAK FLOW (min) = 180.00
 MAXIMUM STORAGE USED (ha. m.) = 0.7780

CALIB NASHYD (1222)
 ID= 1 DT=15.0 min
 Area (ha)= 246.95
 Curve Number (CN)= 43.8
 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.13

Unit Hyd Qpeak (cms)= 3.728
 PEAK FLOW (cms)= 0.377 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 3.740
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.089

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8018)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1222):	246.95	0.377	8.00	3.74
+ ID2= 2 (5122):	117.91	0.220	9.00	10.15

ID = 3 (8018):	364.86	0.591	8.00	5.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8020)
 1 + 2 = 3

	AREA	QPEAK	TPEAK	R. V.
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	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6122):	3727.64	4.866	12.50	5.46
+ ID2= 2 (8018):	364.86	0.591	8.00	5.81

ID = 3 (8020):	4092.50	5.252	12.50	5.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1261) ID= 1 DT=15.0 min	Area (ha)=	Imp(%)=	Di. r. Conn.(%)=
	50.19	36.00	26.00

	IMPERVIOUS (ha)=	PERVIOUS (i) (mm)=
Surface Area	18.07	32.12
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	578.45	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	55.44	4.05
over (min)	15.00	60.00
Storage Coeff. (min)=	11.42 (ii)	50.00 (ii)
Unit Hyd. Tpeak (min)=	15.00	60.00
Unit Hyd. peak (cms)=	0.08	0.02

	(cms)=	(hrs)=	(mm)=	(mm)=	
PEAK FLOW	1.63	0.21	42.00	42.00	*TOTALS* 1.685 (iii)
TIME TO PEAK	6.00	6.75	14.22	14.22	
RUNOFF VOLUME	40.00	5.17	42.00	42.00	
TOTAL RAINFALL	42.00	0.12	0.95	0.12	
RUNOFF COEFFICIENT					

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 48.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5126) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	3.3570	1.2179
	0.1700	0.5264	4.2160	1.4221
	0.9720	0.5968	4.9080	1.6217
	1.8110	0.8165	5.3080	1.6317
	2.5120	0.9954	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1261)	50.190	1.685	6.00	14.22
OUTFLOW: ID= 1 (5126)	50.190	0.147	8.25	14.21

PEAK FLOW REDUCTION [Qout/Qin] (%) = 8.72
 TIME SHIFT OF PEAK FLOW (min) = 135.00
 MAXIMUM STORAGE USED (ha. m.) = 0.4555

CALIB NASHYD (1262) ID= 1 DT=15.0 min	Area (ha)=	Curve Number (CN)=
	83.33	57.7
	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50
	U.H. Tp(hrs)= 0.57	

Unit Hyd Qpeak (cms)=	2.509
PEAK FLOW (cms)=	0.349 (i)
TIME TO PEAK (hrs)=	6.750
RUNOFF VOLUME (mm)=	5.988
TOTAL RAINFALL (mm)=	42.000
RUNOFF COEFFICIENT	= 0.143

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8024) 1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.
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	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1262):	83.33	0.349	6.75	5.99
+ ID2= 2 (5126):	50.19	0.147	8.25	14.21

ID = 3 (8024):	133.52	0.476	6.75	9.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0124) ID= 1 DT=15.0 min	Area (ha)=	Curve Number (CN)=
	121.43	43.0
	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50
	U.H. Tp(hrs)= 0.51	

Unit Hyd Qpeak (cms)= 4.049

PEAK FLOW (cms)=	0.324 (i)
TIME TO PEAK (hrs)=	6.500
RUNOFF VOLUME (mm)=	3.563
TOTAL RAINFALL (mm)=	42.000
RUNOFF COEFFICIENT	= 0.085

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8026) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0124):	121.43	0.324	6.50	3.56
+ ID2= 2 (8024):	133.52	0.476	6.75	9.08

ID = 3 (8026):	254.95	0.797	6.75	6.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6128) IN= 2--> OUT= 1	Routing time step (min)=
	15.00

DATA FOR SECTION (1281.0) ----->			
Distance	Elevation	Manning	
0.00	270.58	0.0400	
7.71	270.36	0.0400	
15.43	269.48	0.0400	
23.14	265.88	0.0400	
42.43	261.83	0.0400	
61.71	258.31	0.0400	
75.21	256.62	0.0400	
90.64	255.35	0.0400	
92.49	255.00	0.0400 / 0.0300	Main Channel
93.49	254.00	0.0300	Main Channel
94.49	253.90	0.0300	Main Channel
94.99	253.80	0.0300	Main Channel
95.49	253.70	0.0300	Main Channel
95.99	253.80	0.0300	Main Channel
98.49	255.00	0.0300 / 0.0400	Main Channel
100.28	255.44	0.0400	
134.99	257.45	0.0400	
148.49	259.67	0.0400	
167.77	263.42	0.0400	
190.92	270.00	0.0400	

TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.65	254.35	.439E+04	2.0	1.24	36.10
1.30	255.00	.131E+05	9.4	1.92	23.39
2.18	255.88	.433E+05	37.0	2.30	19.54
3.06	256.76	.131E+06	113.3	2.34	19.20
3.95	257.65	.273E+06	273.8	2.70	16.64
4.83	258.53	.453E+06	538.5	3.21	14.01
5.71	259.41	.658E+06	902.9	3.70	12.14
6.59	260.29	.886E+06	1368.4	4.16	10.79
7.48	261.18	.114E+07	1936.3	4.59	9.79
8.36	262.06	.141E+07	2610.9	4.99	9.01
9.24	262.94	.171E+07	3398.7	5.37	8.36
10.12	263.82	.202E+07	4309.6	5.75	7.81
11.01	264.71	.235E+07	5347.5	6.13	7.33
11.89	265.59	.270E+07	6501.0	6.48	6.93
12.77	266.47	.307E+07	7808.6	6.86	6.55

13.65	267.35	.345E+07	9250.5	7.23	6.21
14.54	268.24	.384E+07	10808.3	7.59	5.92
15.42	269.12	.424E+07	12482.5	7.94	5.66
16.30	270.00	.466E+07	14127.4	8.18	5.49

<---- hydrograph ---->					
	AREA	OPEAK	TPEAK	R. V.	<- pi pe / channel ->
	(ha)	(cms)	(hrs)	(mm)	MAX DEPTH MAX VEL
INFLOW : ID= 2 (8026)	254.95	0.80	6.75	6.45	(m) (m/s)
OUTFLOW: ID= 1 (6128)	254.95	0.70	7.50	6.45	0.26 1.24
					0.23 1.24

CALIB STANDHYD (1281)			
ID= 1 DT=15.0 min	Area (ha)=	55.87	Di r. Conn. (%)= 29.00
	Total Imp(%)=	50.00	

		IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	27.93		27.93
Dep. Storage (mm)=	2.00		5.00
Average Slope (%)=	0.50		0.50
Length (m)=	610.30		40.00
Mannings n =	0.013		0.250

Max. Eff. Inten. (mm/hr)=	55.44	9.80
over (min)	15.00	45.00
Storage Coeff. (min)=	11.79 (ii)	38.87 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03

TOTALS

PEAK FLOW (cms)=	2.00	0.42	2.160 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	40.00	7.76	17.11
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.18	0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 54.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5128)			
IN= 2--> OUT= 1			
DT= 15.0 min			

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.7380	1.5139
0.1890	0.7401	4.6930	1.7113
1.0830	0.8643	5.4630	1.9045
2.0160	1.0853	5.8630	1.9045
2.7960	1.2819	0.0000	0.0000

INFLOW : ID= 2 (1281)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5128)	55.870	2.160	6.00	17.11
	55.870	0.167	8.50	17.09

PEAK FLOW REDUCTION [Qout/Qi n] (%)=	7.71
TIME SHIFT OF PEAK FLOW (min)=	150.00
MAXIMUM STORAGE USED (ha. m.)=	0.6524

CALIB NASHYD (1282)			
ID= 1 DT=15.0 min	Area (ha)=	184.36	Curve Number (CN)= 62.0
	Ia (mm)=	5.00	# of Linear Res. (N)= 1.50
	U. H. Tp (hrs)=	0.62	

Unit Hyd Qpeak (cms)=	5.093
PEAK FLOW (cms)=	0.844 (i)
TIME TO PEAK (hrs)=	6.750
RUNOFF VOLUME (mm)=	6.957
TOTAL RAINFALL (mm)=	42.000
RUNOFF COEFFICIENT =	0.166

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9128)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1282):	184.36	0.844	6.75	6.96
+ ID2= 2 (5128):	55.87	0.167	8.50	17.09
=====				
ID = 3 (9128):	240.23	0.984	6.75	9.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8028)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6128):	254.95	0.703	7.50	6.45
+ ID2= 2 (9128):	240.23	0.984	6.75	9.31
=====				
ID = 3 (8028):	495.18	1.643	7.25	7.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8022)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8020):	4092.50	5.252	12.50	5.50
+ ID2= 2 (8028):	495.18	1.643	7.25	7.84
=====				
ID = 3 (8022):	4587.68	5.879	12.00	5.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1201)			
ID= 1 DT=15.0 min	Area (ha)=	176.35	Di r. Conn. (%)= 22.00
	Total Imp(%)=	45.00	

		IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	79.36		96.99
Dep. Storage (mm)=	2.00		5.00
Average Slope (%)=	0.50		0.50
Length (m)=	1084.28		40.00
Mannings n =	0.013		0.250

Max. Eff. Inten. (mm/hr)=	55.44	7.76
over (min)	15.00	60.00
Storage Coeff. (min)=	16.64 (ii)	46.39 (ii)
Unit Hyd. Tpeak (min)=	15.00	60.00
Unit Hyd. peak (cms)=	0.07	0.02

TOTALS

PEAK FLOW (cms)=	4.16	0.98	4.444 (iii)
TIME TO PEAK (hrs)=	6.00	6.75	6.00
RUNOFF VOLUME (mm)=	40.00	6.22	13.65
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.15	0.33

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 47.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5120)			
IN= 2--> OUT= 1			
DT= 15.0 min			

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	11.7960	4.5929
0.5970	2.1554	14.8120	5.2509
3.4170	2.4937	17.2430	5.8948
6.3630	3.2187	17.6430	5.9048
8.8260	3.8422	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
-----------	-------------	-------------	------------

INFLOW : ID= 2 (1201) 176.350 4.444 6.00 13.65
 OUTFLOW: ID= 1 (5120) 176.350 0.440 9.00 13.65

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 9.89
 TIME SHIFT OF PEAK FLOW (mi n) = 180.00
 MAXIMUM STORAGE USED (ha. m.) = 1.5885

CALIB STANDHYD (1202)
 ID= 1 DT=15.0 mi n
 Area (ha) = 70.67
 Total Imp(%) = 43.00 Di r. Conn.(%) = 20.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 30.39 40.28
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 686.39 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 55.44 5.84
 over (mi n) = 15.00 60.00
 Storage Coeff. (mi n) = 12.65 (ii) 45.97 (ii)
 Uni t Hyd. Tpeak (mi n) = 15.00 60.00
 Uni t Hyd. peak (cms) = 0.08 0.02

TOTALS
 PEAK FLOW (cms) = 1.70 0.40 1.815 (iii)
 TIME TO PEAK (hrs) = 6.00 6.75 6.00
 RUNOFF VOLUME (mm) = 40.00 6.15 12.92
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.15 0.31

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 47.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8030)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (1202): 70.67 1.815 6.00 12.92
 + ID2= 2 (5120): 176.35 0.440 9.00 13.65
 ID = 3 (8030): 247.02 1.981 6.00 13.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8032)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8022): 4587.68 5.879 12.00 5.75
 + ID2= 2 (8030): 247.02 1.981 6.00 13.44
 ID = 3 (8032): 4834.70 6.352 12.00 6.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6130)
 IN= 2---> OUT= 1
 Routing time step (mi n)' = 15.00

<----- DATA FOR SECTION (1301.0) ----->
 Di stance Elevati on Manni ng
 0.00 257.18 0.0350
 62.45 254.70 0.0350
 109.29 252.74 0.0350
 150.92 251.55 0.0350
 192.56 248.96 0.0350
 239.39 247.16 0.0350
 291.84 246.00 0.0350
 296.84 245.00 0.0350
 298.84 245.00 0.0350 /0.0300 Mai n Channel

299.24 244.60 0.0300 Mai n Channel
 301.84 244.60 0.0300 Mai n Channel
 303.44 244.60 0.0300 Mai n Channel
 303.84 245.00 0.0300 /0.0350 Mai n Channel
 306.84 245.00 0.0350
 309.84 246.00 0.0350
 317.46 247.22 0.0350
 348.68 254.83 0.0350
 359.09 256.70 0.0350
 452.76 258.23 0.0350
 515.22 258.33 0.0350

<----- TRAVEL TIME TABLE ----->
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV. TIME
 (m) (m) (cu. m.) (cms) (m/s) (mi n)
 0.40 245.00 .870E+04 1.2 0.64 124.12
 1.04 245.64 .468E+05 9.5 0.96 81.81
 1.68 246.28 .109E+06 25.6 1.11 70.86
 2.32 246.92 .257E+06 63.5 1.17 67.51
 2.96 247.56 .498E+06 142.7 1.36 58.18
 3.61 248.21 .800E+06 266.7 1.58 50.01
 4.25 248.85 .116E+07 437.4 1.78 44.24
 4.89 249.49 .157E+07 669.1 2.01 39.20
 5.53 250.13 .203E+07 954.9 2.23 35.36
 6.17 250.77 .252E+07 1294.4 2.43 32.42
 6.81 251.41 .305E+07 1689.6 2.62 30.07
 7.45 252.05 .363E+07 2090.7 2.73 28.93
 8.09 252.69 .429E+07 2553.1 2.82 27.98
 8.73 253.33 .501E+07 3138.4 2.96 26.60
 9.37 253.97 .579E+07 3803.8 3.11 25.36
 10.02 254.62 .662E+07 4549.3 3.25 24.25
 10.66 255.26 .751E+07 5364.9 3.38 23.32
 11.30 255.90 .845E+07 6265.7 3.51 22.49
 11.94 256.54 .946E+07 7260.1 3.63 21.72

<---- hydrograph ----> <-pi pe / channel ->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8032) 4834.70 6.35 12.00 6.14 0.80 0.81
 OUTFLOW: ID= 1 (6130) 4834.70 5.99 13.00 6.14 0.77 0.79

CALIB STANDHYD (1301)
 ID= 1 DT=15.0 mi n
 Area (ha) = 320.20
 Total Imp(%) = 64.00 Di r. Conn.(%) = 47.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 204.93 115.27
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 1461.05 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 37.80 21.77
 over (mi n) = 30.00 45.00
 Storage Coeff. (mi n) = 23.20 (ii) 42.88 (ii)
 Uni t Hyd. Tpeak (mi n) = 30.00 45.00
 Uni t Hyd. peak (cms) = 0.04 0.03

TOTALS
 PEAK FLOW (cms) = 11.64 3.56 14.247 (iii)
 TIME TO PEAK (hrs) = 6.25 6.50 6.25
 RUNOFF VOLUME (mm) = 40.00 15.65 27.10
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.37 0.65

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5130)
 IN= 2---> OUT= 1
 DT= 15.0 mi n
 OUTFLOW STORAGE OUTFLOW STORAGE
 (cms) (ha. m.) (cms) (ha. m.)
 0.0000 0.0000 18.6040 6.25
 1.1320 5.3237 22.6740 9.7083
 6.6360 6.1006 30.4650 12.2155
 10.3780 7.2341 31.8650 12.2255

14.1960 8.4233 | 0.0000 0.0000

INFLOW : ID= 2 (1301) 320.200 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 OUTFLOW: ID= 1 (5130) 320.200 14.247 6.25 27.10
 3.029 7.75 27.09

PEAK FLOW REDUCTION [Qout/Qin] (%) = 21.26
 TIME SHIFT OF PEAK FLOW (min) = 90.00
 MAXIMUM STORAGE USED (ha.m.) = 5.6020

CALIB NASHYD (1302) Area (ha) = 65.86 Curve Number (CN) = 80.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 0.69

Unit Hyd Opeak (cms) = 1.620

PEAK FLOW (cms) = 0.547 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 13.383
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.319

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8036) 1 + 2 = 3 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (1302): 65.86 0.547 6.75 13.38
 + ID2= 2 (5130): 320.20 3.029 7.75 27.09
 ID = 3 (8036): 386.06 3.513 7.75 24.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8034) 1 + 2 = 3 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (6130): 4834.70 5.990 13.00 6.14
 + ID2= 2 (8036): 386.06 3.513 7.75 24.75
 ID = 3 (8034): 5220.76 7.128 12.75 7.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1861) ID= 1 DT=15.0 min Area (ha) = 6.85
 Total Imp(%) = 51.00 Dir. Conn. (%) = 27.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 3.49 3.36
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 213.70 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 55.44 10.27
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 6.28 (ii) 32.87 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.10 0.03

TOTALS
 PEAK FLOW (cms) = 0.27 0.06 0.290 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 40.00 7.74 16.45
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.18 0.39

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 52.5 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5186) IN= 2--> OUT= 1 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.4580	0.1865
0.0230	0.0917	0.5750	0.2106
0.1330	0.1071	0.6700	0.2341
0.2470	0.1341	1.0700	0.2441
0.3430	0.1582	0.0000	0.0000

INFLOW : ID= 2 (1861) 6.850 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 OUTFLOW: ID= 1 (5186) 6.850 0.290 6.00 16.45
 0.019 8.50 16.30

PEAK FLOW REDUCTION [Qout/Qin] (%) = 6.68
 TIME SHIFT OF PEAK FLOW (min) = 150.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0774

CALIB STANDHYD (1862) ID= 1 DT=15.0 min Area (ha) = 344.94
 Total Imp(%) = 43.00 Dir. Conn. (%) = 23.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 148.32 196.62
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 1516.44 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 37.80 8.41
 over (min) = 30.00 60.00
 Storage Coeff. (min) = 23.72 (ii) 52.53 (ii)
 Unit Hyd. Tpeak (min) = 30.00 60.00
 Unit Hyd. peak (cms) = 0.04 0.02

PEAK FLOW (cms) = 6.08 2.01 *TOTALS*
 TIME TO PEAK (hrs) = 6.25 6.75 7.137 (iii)
 RUNOFF VOLUME (mm) = 40.00 7.04 6.25
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.17 0.35

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 52.5 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9186) 1 + 2 = 3 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (1862): 344.94 7.137 6.25 14.62
 + ID2= 2 (5186): 6.85 0.019 8.50 16.30
 ID = 3 (9186): 351.79 7.150 6.25 14.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1881) ID= 1 DT=15.0 min Area (ha) = 1.12
 Total Imp(%) = 75.00 Dir. Conn. (%) = 46.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 0.84 0.28
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 86.41 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 55.44 26.22
 over (min) = 15.00 30.00

Storage Coeff. (min)= 3.65 (ii) 21.92 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.11 0.04

PEAK FLOW (cms)= 0.08 0.02 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.25 0.089 (iii)
 RUNOFF VOLUME (mm)= 40.00 13.12 6.00
 TOTAL RAINFALL (mm)= 42.00 42.00 25.47
 RUNOFF COEFFICIENT = 0.95 0.31 42.00
 0.61

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5188)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0750	0.0357
0.0040	0.0200	0.0940	0.0386
0.0220	0.0240	0.1100	0.0416
0.0410	0.0277	0.5100	0.0516
0.0560	0.0315	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1881)	1.120	0.089	6.00	25.47
OUTFLOW: ID= 1 (5188)	1.120	0.006	8.00	24.35

PEAK FLOW REDUCTION [Qout/Qin] (%) = 6.60
 TIME SHIFT OF PEAK FLOW (min) = 120.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0204

CALIB STANDHYD (1882)
 ID= 1 DT=15.0 min

Area (ha) = 307.41
 Total Imp (%) = 41.00
 Di r. Conn. (%) = 25.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	126.04	181.37
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1431.57	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 37.80 9.41
 over (min) = 30.00 60.00
 Storage Coeff. (min) = 22.92 (ii) 50.45 (ii)
 Unit Hyd. Tpeak (min) = 30.00 60.00
 Unit Hyd. peak (cms) = 0.04 0.02

PEAK FLOW (cms) = 5.97 2.11 7.092 (iii)
 TIME TO PEAK (hrs) = 6.25 6.75 6.25
 RUNOFF VOLUME (mm) = 40.00 8.30 16.22
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.20 0.39

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9188)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1882):	307.41	7.092	6.25	16.22
+ ID2= 2 (5188):	1.12	0.006	8.00	24.35
ID = 3 (9188):	308.53	7.095	6.25	16.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8068)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9186):	351.79	7.150	6.25	14.65
+ ID2= 2 (9188):	308.53	7.095	6.25	16.25
ID = 3 (8068):	660.32	14.245	6.25	15.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1821)
 ID= 1 DT=15.0 min

Area (ha) = 111.80
 Total Imp (%) = 40.00
 Di r. Conn. (%) = 17.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	44.72	67.08
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	863.33	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 55.44 8.68
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 14.52 (ii) 42.96 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.07 0.03

PEAK FLOW (cms) = 2.16 0.84 *TOTALS*
 TIME TO PEAK (hrs) = 6.00 6.50 2.488 (iii)
 RUNOFF VOLUME (mm) = 40.00 7.09 12.68
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.17 0.30

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 52.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5182)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	5.7800	2.5656
0.3790	1.0207	7.3720	3.0829
1.6240	1.1939	8.7710	3.5956
3.0930	1.6631	9.1710	3.6056
4.3530	2.1032	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1821)	111.800	2.488	6.00	12.68
OUTFLOW: ID= 1 (5182)	111.800	0.323	8.50	12.68

PEAK FLOW REDUCTION [Qout/Qin] (%) = 12.96
 TIME SHIFT OF PEAK FLOW (min) = 150.00
 MAXIMUM STORAGE USED (ha.m.) = 0.8691

CALIB NASHYD (1822)
 ID= 1 DT=15.0 min

Area (ha) = 44.40
 Ia (mm) = 5.00
 U. H. Tp (hrs) = 0.79
 Curve Number (CN) = 57.0
 # of Linear Res. (N) = 1.50

Unit Hyd Opeak (cms) = 0.966

PEAK FLOW (cms) = 0.142 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 5.901
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.140

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1822):	44.40	0.142	7.00	5.90
+ ID2= 2 (5182):	111.80	0.323	8.50	12.68

ID = 3 (8062):	156.20	0.450	8.00	10.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1841) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	145.07 48.00	Dir. Conn. (%)=	19.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	69.63	75.44
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	983.43	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	55.44	12.58
over (min)	15.00	45.00
Storage Coeff. (min)=	15.70 (ii)	40.21 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.07	0.03

PEAK FLOW (cms)=	3.03	1.41	*TOTALS* 3.595 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	40.00	8.99	14.88
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.21	0.35

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 55.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5184) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	5.9270	2.3472
	0.2980	1.0088	7.4630	2.7585
	1.6660	1.1775	8.7490	3.1642
	3.1100	1.4809	9.1490	3.1742
	4.4100	1.8897	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1841)	145.070	3.595	6.00	14.88
OUTFLOW: ID= 1 (5184)	145.070	1.180	7.25	14.87

PEAK FLOW REDUCTION [Qout/Qin] (%) = 32.83
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha. m.) = 1.1233

CALIB STANDHYD (1842) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	117.45 42.00	Dir. Conn. (%)=	17.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	49.33	68.12
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	884.87	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	55.44	10.62
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over (min)	15.00	45.00
Storage Coeff. (min)=	14.73 (ii)	40.96 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.07	0.03

TOTALS	2.26	1.07	2.678 (iii)
PEAK FLOW (cms)=	2.26	1.07	2.678 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	40.00	8.30	13.69
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.20	0.33

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 55.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1842):	117.45	2.678	6.00	13.69
+ ID2= 2 (5184):	145.07	1.180	7.25	14.87

ID = 3 (8060):	262.52	2.815	6.00	14.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8060):	262.52	2.815	6.00	14.35
+ ID2= 2 (8062):	156.20	0.450	8.00	10.75

ID = 3 (8064):	418.72	3.018	6.00	13.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1781) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	55.50 39.00	Dir. Conn. (%)=	13.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	21.65	33.86
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	608.28	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	55.44	19.90
over (min)	15.00	45.00
Storage Coeff. (min)=	11.76 (ii)	32.17 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03

PEAK FLOW (cms)=	0.89	1.12	*TOTALS* 1.350 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	40.00	14.85	18.12
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.35	0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 74.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5178)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	6.1870	0.7248
0.1880	0.3394	9.3670	0.8313
1.7260	0.4003	10.7440	0.9285
3.2000	0.4933	11.1440	0.9385
4.6710	0.6039	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1781)	55.500	1.350	6.00	18.12
OUTFLOW: ID= 1 (5178)	55.500	1.203	6.75	18.11

PEAK FLOW REDUCTION [Qout/Oin](%)= 89.11
TIME SHIFT OF PEAK FLOW (min)= 45.00
MAXIMUM STORAGE USED (ha. m.)= 0.3896

CALIB NASHYD (1782)
ID= 1 DT=15.0 min

Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
219.91	1.541	7.00	12.22
5.00	1.203	6.75	18.11

Unit Hyd Opeak (cms)= 4.921

PEAK FLOW (cms)= 1.541 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 12.220
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.291

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8046)
1 + 2 = 3

ID	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1782):	219.91	1.541	7.00	12.22
+ ID2= 2 (5178):	55.50	1.203	6.75	18.11
=====				
ID = 3 (8046):	275.41	2.728	6.75	13.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0180)
ID= 1 DT=15.0 min

Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
226.65	0.714	8.00	7.81
5.00	1.203	6.75	18.11

Unit Hyd Opeak (cms)= 3.299

PEAK FLOW (cms)= 0.714 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 7.815
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.186

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8050)
1 + 2 = 3

ID	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0180):	226.65	0.714	8.00	7.81
+ ID2= 2 (8046):	275.41	2.728	6.75	13.41
=====				
ID = 3 (8050):	502.06	3.360	6.75	10.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1741)
ID= 1 DT=15.0 min

Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
56.30	1.489	6.00	15.04
35.00	0.213	8.25	15.00

Dir. Conn. (%)= 18.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	19.70	36.60
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	612.64	40.00
Mannings n =	0.013	0.250

	IMPERVIOUS	PERVIOUS (i)
Max. Eff. Inten. (mm/hr)=	55.44	10.87
over (min)	15.00	45.00
Storage Coeff. (min)=	11.82 (ii)	37.80 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03

TOTALS

	IMPERVIOUS	PERVIOUS (i)	TOTALS*
PEAK FLOW (cms)=	1.25	0.61	1.489 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	40.00	9.56	15.04
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.23	0.36

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5174)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	1.1370	1.3070
0.0580	0.5427	1.4280	1.5354
0.3290	0.6196	1.6620	1.7593
0.6130	0.8627	2.0620	1.7693
0.8510	1.0621	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1741)	56.300	1.489	6.00	15.04
OUTFLOW: ID= 1 (5174)	56.300	0.213	8.25	15.00

PEAK FLOW REDUCTION [Qout/Oin](%)= 14.28
TIME SHIFT OF PEAK FLOW (min)=135.00
MAXIMUM STORAGE USED (ha. m.)= 0.5868

CALIB NASHYD (1742)
ID= 1 DT=15.0 min

Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
82.80	0.484	6.75	9.55
5.00	1.203	6.75	18.11

Unit Hyd Opeak (cms)= 2.055

PEAK FLOW (cms)= 0.484 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 9.554
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.227

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8044)
1 + 2 = 3

ID	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1742):	82.80	0.484	6.75	9.55
+ ID2= 2 (5174):	56.30	0.213	8.25	15.00
=====				
ID = 3 (8044):	139.10	0.614	8.00	11.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB

NASHYD (0172) | Area (ha)= 202.23 | Curve Number (CN)= 78.0
 ID= 1 DT=15.0 min | I_a (mm)= 5.00 | # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.89

Unit Hyd Qpeak (cms)= 3.875

PEAK FLOW (cms)= 1.274 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 12.449
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.296

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0168) | Area (ha)= 247.13 | Curve Number (CN)= 52.0
 ID= 1 DT=15.0 min | I_a (mm)= 5.00 | # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.75

Unit Hyd Qpeak (cms)= 5.611

PEAK FLOW (cms)= 0.686 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 4.965
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.118

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0170) | Area (ha)= 166.91 | Curve Number (CN)= 65.0
 ID= 1 DT=15.0 min | I_a (mm)= 5.00 | # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.80

Unit Hyd Qpeak (cms)= 3.553

PEAK FLOW (cms)= 0.698 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 7.767
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.185

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	(8040)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3					
ID1= 1 (0168):	247.13	0.686	7.00	4.96	
+ ID2= 2 (0170):	166.91	0.698	7.00	7.77	
ID = 3 (8040):	414.04	1.384	7.00	6.09	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8042)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3					
ID1= 1 (0172):	202.23	1.274	7.25	12.45	
+ ID2= 2 (8040):	414.04	1.384	7.00	6.09	
ID = 3 (8042):	616.27	2.656	7.00	8.18	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6174) | Routing time step (min)' = 15.00
 IN= 2----> OUT= 1

Distance	Elevation	Manning
0.00	296.00	0.0450
13.36	288.94	0.0450
26.71	288.64	0.0450

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
51.19	287.96	0.0450			
73.45	287.32	0.0450			
86.81	286.48	0.0450			
102.39	285.08	0.0450			
110.73	282.76	0.0450			
115.73	281.06	0.0450 / 0.0350			Main Channel
115.74	280.35	0.0350			Main Channel
120.73	280.35	0.0350			Main Channel
120.74	281.10	0.0350 / 0.0450			Main Channel
125.74	282.64	0.0450			
135.78	286.87	0.0450			
149.13	289.58	0.0450			
162.49	292.29	0.0450			
175.84	293.95	0.0450			
195.88	295.13	0.0450			
213.68	296.02	0.0450			
220.36	296.09	0.0450			

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.71	281.06	.136E+05	6.0	1.69	37.89
1.50	281.85	.358E+05	22.5	2.42	26.43
2.28	282.63	.726E+05	53.8	2.85	22.47
3.07	283.42	.124E+06	104.1	3.24	19.79
3.86	284.21	.189E+06	176.7	3.60	17.81
4.64	284.99	.269E+06	274.9	3.94	16.26
5.43	285.78	.369E+06	390.8	4.07	15.73
6.21	286.56	.501E+06	551.6	4.23	15.15
7.00	287.35	.674E+06	750.5	4.28	14.97
7.79	288.14	.920E+06	993.3	4.15	15.44
8.57	288.92	.126E+07	1361.5	4.14	15.47
9.36	289.71	.167E+07	1986.0	4.56	14.04
10.15	290.50	.210E+07	2739.7	5.02	12.76
10.93	291.28	.254E+07	3615.0	5.47	11.70
11.72	292.07	.300E+07	4608.8	5.91	10.83
12.50	292.85	.347E+07	5693.3	6.30	10.16
13.29	293.64	.397E+07	6892.9	6.67	9.60
14.08	294.43	.450E+07	8150.4	6.97	9.19
14.86	295.21	.506E+07	9525.0	7.23	8.86

<--- hydrograph ---> <--- pi pe / channel --->

INFLOW	ID= 2 (8042)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW	ID= 1 (6174)	616.27	2.66	8.00	8.18	0.31	1.69
		616.27	2.45	8.00	8.18	0.29	1.69

ADD HYD	(8048)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3					
ID1= 1 (6174):	616.27	2.452	8.00	8.18	
+ ID2= 2 (8044):	139.10	0.614	8.00	11.76	
ID = 3 (8048):	755.37	3.066	8.00	8.84	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8052)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3					
ID1= 1 (8048):	755.37	3.066	8.00	8.84	
+ ID2= 2 (8050):	502.06	3.360	6.75	10.88	
ID = 3 (8052):	1257.43	5.628	7.25	9.65	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8058)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3					
ID1= 1 (8052):	1257.43	5.628	7.25	9.65	
+ ID2= 2 (8064):	418.72	3.018	6.00	13.00	
ID = 3 (8058):	1676.15	8.213	7.00	10.49	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (1762)
ID= 1 DT=15.0 min

Area (ha)	= 162.00	Dir. Conn. (%)	= 11.00
Total Imp (%)	= 26.00		

Surface Area (ha)	= 42.12	IMPERVIOUS (ha)	119.88	PERVIOUS (i)	
Dep. Storage (mm)	= 2.00		5.00		
Average Slope (%)	= 0.50		0.50		
Length (m)	= 1039.23		40.00		
Mannings n	= 0.013		0.250		

Max. Eff. Inten. (mm/hr)	= 55.44	8.28
over (min)	= 15.00	60.00
Storage Coeff. (min)	= 16.22 (ii)	45.20 (ii)
Unit Hyd. Tpeak (min)	= 15.00	60.00
Unit Hyd. peak (cms)	= 0.07	0.02

TOTALS
PEAK FLOW (cms) = 1.93 1.31 2.311 (iii)
TIME TO PEAK (hrs) = 6.00 6.75 6.00
RUNOFF VOLUME (mm) = 40.00 7.76 11.30
TOTAL RAINFALL (mm) = 42.00 42.00 42.00
RUNOFF COEFFICIENT = 0.95 0.18 0.27

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (1761)
ID= 1 DT=15.0 min

Area (ha)	= 48.80	Dir. Conn. (%)	= 21.00
Total Imp (%)	= 50.00		

Surface Area (ha)	= 24.40	IMPERVIOUS (ha)	24.40	PERVIOUS (i)	
Dep. Storage (mm)	= 2.00		5.00		
Average Slope (%)	= 0.50		0.50		
Length (m)	= 570.38		40.00		
Mannings n	= 0.013		0.250		

Max. Eff. Inten. (mm/hr)	= 55.44	14.34
over (min)	= 15.00	45.00
Storage Coeff. (min)	= 11.32 (ii)	34.58 (ii)
Unit Hyd. Tpeak (min)	= 15.00	45.00
Unit Hyd. peak (cms)	= 0.08	0.03

TOTALS
PEAK FLOW (cms) = 1.28 0.56 1.508 (iii)
TIME TO PEAK (hrs) = 6.00 6.50 6.00
RUNOFF VOLUME (mm) = 40.00 10.02 16.31
TOTAL RAINFALL (mm) = 42.00 42.00 42.00
RUNOFF COEFFICIENT = 0.95 0.24 0.39

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5176)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.2640	1.3175
0.1650	0.6418	4.0090	1.4908
0.9450	0.7489	4.7720	1.6604
1.7610	0.9426	5.1720	1.6704
2.4420	1.1144	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
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INFLOW : ID= 2 (1761) 48.800 1.508 6.00 16.31
OUTFLOW: ID= 1 (5176) 48.800 0.139 8.75 16.29

PEAK FLOW REDUCTION [Qout/Qin] (%) = 9.20
TIME SHIFT OF PEAK FLOW (min) = 165.00
MAXIMUM STORAGE USED (ha.m.) = 0.5398

ADD HYD (8056)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1762): 162.00	2.311	6.00	11.30
+ ID2= 2 (5176): 48.80	0.139	8.75	16.29
ID = 3 (8056): 210.80	2.360	6.00	12.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8066)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8056): 210.80	2.360	6.00	12.46
+ ID2= 2 (8058): 1676.15	8.213	7.00	10.49
ID = 3 (8066): 1886.95	9.766	6.75	10.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8070)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8066): 1886.95	9.766	6.75	10.71
+ ID2= 2 (8068): 660.32	14.245	6.25	15.40
ID = 3 (8070): 2547.27	21.565	6.25	11.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6190)
IN= 2--> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (1901.0) ----->

Distance	Elevation	Manning
0.00	261.34	0.0600
19.76	261.64	0.0600
59.28	261.29	0.0600
79.04	261.09	0.0600
103.74	255.99	0.0600
153.13	253.42	0.0600
192.65	252.78	0.0600
212.29	249.80	0.0600
217.29	249.50	0.0600 / 0.0350
217.30	249.05	0.0350
222.29	249.05	0.0350
222.30	249.50	0.0350 / 0.0600
227.30	249.80	0.0600
232.17	252.31	0.0600
276.63	253.53	0.0600
326.03	256.97	0.0600
355.67	257.30	0.0600
400.12	260.08	0.0600
469.28	261.25	0.0600
489.04	262.53	0.0600

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.45	249.50	.436E+04	1.0	0.43	75.97
1.07	250.12	.205E+05	5.4	0.51	62.96
1.70	250.75	.451E+05	14.8	0.64	50.78
2.32	251.37	.762E+05	29.2	0.74	43.46
2.94	251.99	.114E+06	49.1	0.84	38.61
3.57	252.62	.161E+06	70.2	0.85	38.18
4.19	253.24	.248E+06	93.4	0.73	44.29

4.81	253.86	.398E+06	154.9	0.76	42.79
5.44	254.49	.576E+06	246.1	0.83	38.99
6.06	255.11	.779E+06	362.7	0.90	35.79
6.68	255.73	.101E+07	506.5	0.98	33.15
7.30	256.35	.126E+07	687.6	1.06	30.52
7.93	256.98	.153E+07	902.8	1.15	28.18
8.55	257.60	.183E+07	1114.3	1.18	27.39
9.17	258.22	.216E+07	1407.8	1.26	25.55
9.80	258.85	.250E+07	1735.4	1.35	24.03
10.42	259.47	.286E+07	2097.5	1.42	22.73
11.04	260.09	.324E+07	2492.3	1.49	21.63
11.67	260.72	.364E+07	2837.8	1.51	21.39

<---- hydrograph ---->					
AREA	OPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW: ID= 2 (8070)	2547.27	21.56	6.25	11.93	1.99
OUTFLOW: ID= 1 (6190)	2547.27	15.05	7.00	11.93	0.64

CALIB STANDHYD (1901)			
ID= 1 DT=15.0 min	Area (ha)=	13.49	Dir. Conn. (%)=
	Total Imp(%)=	58.00	31.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)=	7.82	5.67	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	299.89	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	55.44	22.54	
over (min)	15.00	30.00	
Storage Coeff. (min)=	7.70 (ii)	27.11 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.10	0.04	

TOTALS		
PEAK FLOW (cms)=	0.58	0.25
TIME TO PEAK (hrs)=	6.00	6.00
RUNOFF VOLUME (mm)=	40.00	14.64
TOTAL RAINFALL (mm)=	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.35

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5190)			
IN= 2----> OUT= 1	OUTFLOW	STORAGE	OUTFLOW
DT= 15.0 min	(cms)	(ha.m.)	(cms)
	0.0000	0.0000	0.9030
	0.0460	0.1975	1.1330
	0.2610	0.2331	1.3190
	0.4870	0.2836	1.7190
	0.6750	0.3308	0.0000

AREA	OPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (1901)	13.490	0.737	6.00
OUTFLOW: ID= 1 (5190)	13.490	0.081	7.75

PEAK FLOW REDUCTION [Qout/Qin] (%) = 11.03
TIME SHIFT OF PEAK FLOW (min) = 105.00
MAXIMUM STORAGE USED (ha.m.) = 0.2035

CALIB STANDHYD (1902)			
ID= 1 DT=15.0 min	Area (ha)=	274.50	Dir. Conn. (%)=
	Total Imp(%)=	52.00	28.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)=	142.74	131.76	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	

Length (m)=	1352.77	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	55.44	19.06
over (min)	15.00	45.00
Storage Coeff. (min)=	19.00 (ii)	39.77 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.06	0.03

TOTALS		
PEAK FLOW (cms)=	7.73	3.73
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	40.00	13.65
TOTAL RAINFALL (mm)=	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.33

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9190)					
1 + 2 = 3	AREA	OPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (1902):	274.50	9.256	6.00	21.03	
+ ID2= 2 (5190):	13.49	0.081	7.75	22.42	
ID = 3 (9190):	287.99	9.276	6.00	21.09	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8072)					
1 + 2 = 3	AREA	OPEAK	TPEAK	R.V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (6190):	2547.27	15.045	7.00	11.93	
+ ID2= 2 (9190):	287.99	9.276	6.00	21.09	
ID = 3 (8072):	2835.26	19.568	6.50	12.86	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6192)		Routing time step (min)' = 15.00
IN= 2----> OUT= 1		

<----- DATA FOR SECTION (1921.0) ----->					
Distance	Elevation	Manning			
0.00	264.40	0.0500			
3.64	263.94	0.0500			
14.54	261.05	0.0500			
29.08	256.78	0.0500			
36.36	254.96	0.0500			
50.90	252.20	0.0500			
65.44	249.78	0.0500			
76.35	246.86	0.0500			
77.24	245.90	0.0500			
82.24	245.45	0.0500 / 0.0350	Main Channel		
82.25	244.90	0.0350	Main Channel		
87.25	244.90	0.0350	Main Channel		
87.26	245.45	0.0350 / 0.0500	Main Channel		
94.53	246.89	0.0500			
170.87	247.16	0.0500			
218.13	248.69	0.0500			
265.40	249.22	0.0500			
301.75	250.78	0.0500			
338.11	253.47	0.0500			
359.92	264.00	0.0500			

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)
0.55	245.45	.345E+04	2.0	0.71	29.20
1.53	246.43	.174E+05	14.6	1.05	19.92
2.50	247.40	.767E+05	52.8	0.86	24.20
3.48	248.38	.224E+06	199.8	1.11	18.71
4.46	249.36	.429E+06	441.2	1.29	16.19

5.43	250.33	.691E+06	874.3	1.58	13.18
6.41	251.31	.987E+06	1468.3	1.86	11.20
7.38	252.28	.131E+07	2224.8	2.13	9.79
8.36	253.26	.165E+07	3132.4	2.37	8.78
9.34	254.24	.201E+07	4256.4	2.65	7.87
10.31	255.21	.238E+07	5551.0	2.92	7.15
11.29	256.19	.276E+07	6996.8	3.17	6.57
12.27	257.17	.314E+07	8585.9	3.42	6.10
13.24	258.14	.352E+07	10315.9	3.65	5.71
14.22	259.12	.393E+07	12180.7	3.87	5.38
15.19	260.09	.434E+07	14178.2	4.09	5.10
16.17	261.07	.475E+07	16306.5	4.29	4.85
17.15	262.05	.517E+07	18558.6	4.49	4.64
18.12	263.02	.559E+07	20939.7	4.68	4.45

		<---- hydrograph ---->				<- pi pe / channel ->	
	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL	
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW :	ID= 2 (8072)	2835.26	19.57	6.50	12.86	1.65	
OUTFLOW :	ID= 1 (6192)	2835.26	18.26	7.00	12.86	1.62	

CALIB STANDHYD (1921)		Area (ha)= 72.27	Dir. Conn. (%)= 19.00
ID= 1 DT=15.0 min	Total Imp(%)= 42.00		

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	30.35	41.92
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	694.12	40.00
Mannings n	=	0.013	0.250

Max. Eff. Inten. (mm/hr)=	55.44	10.53
over (min)	15.00	45.00
Storage Coeff. (min)=	12.73 (ii)	39.06 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03

		TOTALS	(iii)
PEAK FLOW	(cms)=	1.64	0.67
TIME TO PEAK	(hrs)=	6.00	6.00
RUNOFF VOLUME	(mm)=	40.00	14.43
TOTAL RAINFALL	(mm)=	42.00	42.00
RUNOFF COEFFICIENT	=	0.95	0.20

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 57.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5192)		IN= 2--> OUT= 1	
DT= 15.0 min			
OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	3.1390	1.4346
0.2060	0.7072	3.8730	1.6637
0.9550	0.8170	4.8310	1.9184
1.7940	1.0081	5.2310	1.9284
2.4250	1.1871	0.0000	0.0000

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW :	ID= 2 (1921)	72.270	1.909	6.00
OUTFLOW :	ID= 1 (5192)	72.270	0.199	8.75

PEAK FLOW REDUCTION [Oout/Oin] (%) = 10.42
 TIME SHIFT OF PEAK FLOW (min) = 165.00
 MAXIMUM STORAGE USED (ha.m.) = 0.6829

CALIB STANDHYD (1922)		Area (ha)= 174.34	Dir. Conn. (%)= 18.00
ID= 1 DT=15.0 min	Total Imp(%)= 39.00		

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	67.99	106.35
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	1078.08	40.00
Mannings n	=	0.013	0.250
Max. Eff. Inten. (mm/hr)=	55.44	9.75	
over (min)	15.00	45.00	
Storage Coeff. (min)=	16.58 (ii)	43.73 (ii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.07	0.03	
PEAK FLOW	(cms)=	3.37	1.47
TIME TO PEAK	(hrs)=	6.00	6.50
RUNOFF VOLUME	(mm)=	40.00	8.13
TOTAL RAINFALL	(mm)=	42.00	42.00
RUNOFF COEFFICIENT	=	0.95	0.19

		TOTALS	(iii)
PEAK FLOW	(cms)=	3.944	3.944
TIME TO PEAK	(hrs)=	6.00	6.00
RUNOFF VOLUME	(mm)=	13.87	13.87
TOTAL RAINFALL	(mm)=	42.00	42.00
RUNOFF COEFFICIENT	=	0.33	0.33

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 57.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8074)		AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1922):	174.34	3.944	6.00	13.87	
+ ID2= 2 (5192):	72.27	0.199	8.75	14.42	
ID= 3 (8074):	246.61	4.014	6.00	14.03	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8076)		AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6192):	2835.26	18.263	7.00	12.86	
+ ID2= 2 (8074):	246.61	4.014	6.00	14.03	
ID= 3 (8076):	3081.87	19.940	7.00	12.95	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0196)		Area (ha)= 41.47	Curve Number (CN)= 70.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50	
	U. H. Tp(hrs)= 0.24		

Unit Hyd Qpeak	(cms)=	2.950
PEAK FLOW	(cms)=	0.489 (i)
TIME TO PEAK	(hrs)=	6.000
RUNOFF VOLUME	(mm)=	8.615
TOTAL RAINFALL	(mm)=	42.000
RUNOFF COEFFICIENT	=	0.205

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1941)		Area (ha)= 0.76	Dir. Conn. (%)= 15.00
ID= 1 DT=15.0 min	Total Imp(%)= 45.00		

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	0.34	0.42
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	71.18	40.00
Mannings n	=	0.013	0.250

Max. Eff. Inten. (mm/hr)= 55.44 18.27
 over (min) 15.00 30.00
 Storage Coeff. (min)= 3.25 (ii) 24.36 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. Tpeak (cms)= 0.11 0.04

PEAK FLOW (cms)= 0.02 0.02 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.00 0.027 (iii)
 RUNOFF VOLUME (mm)= 40.00 12.78 6.00
 TOTAL RAINFALL (mm)= 42.00 42.00 16.85
 RUNOFF COEFFICIENT = 0.95 0.30 0.40

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 67.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5194)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0510	0.0197
0.0030	0.0093	0.0640	0.0226
0.0150	0.0107	0.0740	0.0253
0.0270	0.0138	0.0740	0.0353
0.0380	0.0165	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
0.760	0.027	6.00	16.85
0.760	0.003	8.25	15.84

INFLOW : ID= 2 (1941)
 OUTFLOW: ID= 1 (5194)

PEAK FLOW REDUCTION [Qout/Qin] (%) = 9.88
 TIME SHIFT OF PEAK FLOW (min) = 135.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0083

CALIB NASHYD (1942)
 ID= 1 DT=15.0 min

Area (ha) = 255.33 Curve Number (CN) = 69.5
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 0.68

Unit Hyd Qpeak (cms) = 6.448

PEAK FLOW (cms) = 1.430 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 9.053
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.216

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9194)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
255.33	1.430	6.75	9.05
0.76	0.003	8.25	15.84
256.09	1.433	6.75	9.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8078)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
41.47	0.489	6.00	8.62
256.09	1.433	6.75	9.07
297.56	1.823	6.50	9.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6198)
 IN= 2--> OUT= 1

Routing time step (min) = 15.00

----- DATA FOR SECTION (1981.0) -----

Distance	Elevation	Manning
0.00	267.15	0.0500
22.99	265.94	0.0500
51.73	261.39	0.0500
74.72	258.75	0.0500
94.83	257.79	0.0500
114.95	254.36	0.0500
126.44	254.06	0.0500
132.19	253.68	0.0500
135.06	253.35	0.0500
137.94	252.93	0.0500 / 0.0300
140.81	252.41	0.0300
143.69	251.89	0.0300
146.56	252.51	0.0300
149.43	253.36	0.0300 / 0.0500
158.05	255.89	0.0500
172.42	256.25	0.0500
204.03	259.15	0.0500
229.90	260.36	0.0500
252.89	260.46	0.0500
284.50	260.45	0.0500

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.35	252.24	.111E+04	0.6	0.97	31.07
0.69	252.58	.442E+04	3.8	1.55	19.42
1.04	252.93	.974E+04	11.1	2.06	14.65
1.51	253.40	.203E+05	31.2	2.77	10.87
1.98	253.87	.357E+05	65.0	3.30	9.14
2.45	254.34	.596E+05	112.9	3.43	8.80
2.92	254.81	.935E+05	186.8	3.62	8.34
3.39	255.28	.131E+06	282.1	3.89	7.75
3.86	255.75	.172E+06	398.2	4.18	7.22
4.33	256.22	.221E+06	527.0	4.31	6.99
4.80	256.69	.282E+06	697.3	4.47	6.75
5.27	257.16	.350E+06	898.5	4.64	6.50
5.74	257.63	.425E+06	1131.4	4.82	6.26
6.21	258.10	.508E+06	1367.4	4.88	6.19
6.68	258.57	.603E+06	1635.6	4.91	6.14
7.15	259.04	.709E+06	1977.2	5.04	5.98
7.62	259.51	.826E+06	2360.5	5.17	5.83
8.09	259.98	.954E+06	2790.7	5.29	5.70
8.56	260.45	.110E+07	3179.6	5.25	5.74

----- hydrograph -----

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
297.56	1.82	6.50	9.01	0.48	1.13
297.56	1.64	7.25	9.01	0.46	1.10

INFLOW : ID= 2 (8078)
 OUTFLOW: ID= 1 (6198)

CALIB STANDHYD (1981)
 ID= 1 DT=15.0 min

Area (ha) = 149.95
 Total Imp(%) = 44.00
 Di r. Conn. (%) = 19.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	65.98
Dep. Storage (mm)	2.00
Average Slope (%)	0.50
Length (m)	999.83
Mannings n	0.013

Max. Eff. Inten. (mm/hr)= 55.44 11.45
 over (min) 15.00 45.00
 Storage Coeff. (min)= 15.85 (ii) 41.30 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. Tpeak (cms)= 0.07 0.03

PEAK FLOW (cms)= 3.12 1.41 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.50 3.678 (iii)
 RUNOFF VOLUME (mm)= 40.00 8.82 6.00
 TOTAL RAINFALL (mm)= 42.00 42.00 14.75
 RUNOFF COEFFICIENT = 0.95 0.21 0.35

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 57.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5198)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	10.9910	3.4957
0.4390	1.7800	13.3530	3.9512
2.6180	2.0449	15.3540	4.3979
5.1680	2.5932	15.7540	4.4079
7.9230	3.0040	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1981)	149.950	3.678	6.00	14.75
OUTFLOW: ID= 1 (5198)	149.950	0.371	9.25	14.74

PEAK FLOW REDUCTION [Qout/Qin] (%) = 10.07
TIME SHIFT OF PEAK FLOW (min) = 195.00
MAXIMUM STORAGE USED (ha.m.) = 1.5028

CALIB STANDHYD (1982)
ID= 1 DT=15.0 min

Area (ha) = 240.61
Total Imp(%) = 20.00
Dir. Conn. (%) = 9.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	48.12	192.49
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1266.52	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) over	55.44	5.41
(min)	15.00	60.00
Storage Coeff. (min)	18.27 (ii)	52.61 (ii)
Unit Hyd. Tpeak (min)	15.00	60.00
Unit Hyd. peak (cms)	0.06	0.02

TOTALS
PEAK FLOW (cms) = 2.22
TIME TO PEAK (hrs) = 6.00
RUNOFF VOLUME (mm) = 40.00
TOTAL RAINFALL (mm) = 42.00
RUNOFF COEFFICIENT = 0.95

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 57.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8080)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1982):	240.61	2.685	6.00	9.95
+ ID2= 2 (5198):	149.95	0.371	9.25	14.74
ID= 3 (8080):	390.56	2.804	6.00	11.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8082)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID= 1 DT=15.0 min	42.48	46.00	19.00	

ID1= 1 (6198):	297.56	1.639	7.25	9.01
+ ID2= 2 (8080):	390.56	2.804	6.00	11.79
ID= 3 (8082):	688.12	3.904	6.75	10.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8084)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8076):	3081.87	19.940	7.00	12.95
+ ID2= 2 (8082):	688.12	3.904	6.75	10.59
ID= 3 (8084):	3769.99	23.654	7.00	12.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6200)
IN= 2--> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (2001.0) ----->

Distance	Elevation	Manning	
0.00	252.47	0.0500	
10.38	252.29	0.0500	
20.76	252.28	0.0500	
41.52	247.66	0.0500	
62.28	247.47	0.0500	
80.45	247.34	0.0500	
90.83	246.85	0.0500	
103.81	245.51	0.0500	
106.00	244.50	0.0500	
108.99	244.05	0.0500 / 0.0300	Main Channel
109.00	243.50	0.0300	Main Channel
114.00	243.50	0.0300	Main Channel
114.01	244.05	0.0300 / 0.0500	Main Channel
118.99	244.50	0.0500	
132.36	245.09	0.0500	
150.52	245.83	0.0500	
199.83	249.24	0.0500	
207.62	250.16	0.0500	
236.16	253.11	0.0500	
256.92	253.83	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.28	243.77	.365E+04	0.7	0.54	81.42
0.55	244.05	.730E+04	2.2	0.81	54.59
1.05	244.55	.197E+05	7.2	0.97	45.51
1.54	245.04	.463E+05	17.2	0.99	44.76
2.04	245.54	.895E+05	34.8	1.03	42.81
2.53	246.03	.152E+06	62.8	1.09	40.43
3.03	246.53	.232E+06	104.7	1.20	36.91
3.52	247.02	.328E+06	159.7	1.29	34.19
4.02	247.52	.450E+06	223.4	1.32	33.56
4.51	248.01	.627E+06	326.9	1.38	31.99
5.01	248.51	.820E+06	463.0	1.50	29.52
5.50	249.00	.103E+07	626.0	1.62	27.30
6.00	249.50	.124E+07	820.1	1.75	25.24
6.49	249.99	.147E+07	1044.4	1.89	23.42
6.99	250.49	.170E+07	1292.6	2.01	21.95
7.48	250.98	.195E+07	1565.6	2.13	20.71
7.98	251.48	.220E+07	1864.5	2.25	19.65
8.47	251.97	.246E+07	2189.4	2.36	18.73
8.97	252.47	.274E+07	2427.5	2.35	18.80

<---- hydrograph ---->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8084)	3769.99	23.65	7.00	12.52	1.72	1.00
OUTFLOW: ID= 1 (6200)	3769.99	20.37	7.75	12.52	1.63	0.99

CALIB STANDHYD (2001)
ID= 1 DT=15.0 min

Area (ha) = 42.48
Total Imp(%) = 46.00
Dir. Conn. (%) = 19.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 19.54 22.94
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 532.17 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 55.44 19.19
 over (min)= 15.00 45.00

Storage Coeff. (min)= 10.86 (ii) 31.56 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03

PEAK FLOW (cms)= 1.02 0.74 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.50 1.326 (iii)
 RUNOFF VOLUME (mm)= 40.00 13.74 6.00
 TOTAL RAINFALL (mm)= 42.00 42.00 18.73
 RUNOFF COEFFICIENT = 0.95 0.33 42.00
 0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5200)
 IN= 2----> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.7210	1.0525
0.1450	0.4892	3.4130	1.1947
0.7930	0.5722	4.0030	1.3346
1.4410	0.7540	4.4030	1.3446
2.0370	0.8927	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2001)	42.480	1.326	6.00	18.73
OUTFLOW: ID= 1 (5200)	42.480	0.245	8.00	18.71

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 18.46
 TIME SHIFT OF PEAK FLOW (min) = 120.00
 MAXIMUM STORAGE USED (ha.m.) = 0.5023

CALIB STANDHYD (2002)
 ID= 1 DT=15.0 min

Area (ha)= 72.79
 Total Imp(%)= 31.00
 Dir. Conn. (%) = 13.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 22.56 50.23
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 696.61 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 55.44 13.78
 over (min)= 15.00 45.00

Storage Coeff. (min)= 12.76 (ii) 36.40 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03

PEAK FLOW (cms)= 1.13 1.08 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.50 1.565 (iii)
 RUNOFF VOLUME (mm)= 40.00 11.90 6.00
 TOTAL RAINFALL (mm)= 42.00 42.00 15.56
 RUNOFF COEFFICIENT = 0.95 0.28 42.00
 0.37

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8086)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2002):	72.79	1.565	6.00	15.56
+ ID2= 2 (5200):	42.48	0.245	8.00	18.71
ID = 3 (8086):	115.27	1.613	6.00	16.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8088)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6200):	3769.99	20.368	7.75	12.52
+ ID2= 2 (8086):	115.27	1.613	6.00	16.72
ID = 3 (8088):	3885.26	21.042	7.75	12.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8038)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8034):	5220.76	7.128	12.75	7.52
+ ID2= 2 (8088):	3885.26	21.042	7.75	12.64
ID = 3 (8038):	9106.02	27.086	7.75	9.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (1365)
 IN= 2----> OUT= 1
 Routing time step (min) = 15.00

<----- DATA FOR SECTION (1362.0) ----->

Distance	Elevation	Manning		
0.00	252.00	0.0400		
2.35	244.60	0.0400		
39.94	243.14	0.0400		
58.74	242.65	0.0400		
91.63	242.03	0.0400		
101.03	241.69	0.0400		
103.38	241.66	0.0400		
105.73	240.76	0.0400		
105.93	240.59	0.0400		
109.18	240.28	0.0400	0.0300	Main Channel
110.43	239.81	0.0300	0.0300	Main Channel
111.68	239.81	0.0300	0.0300	Main Channel
113.43	240.09	0.0300	0.0400	Main Channel
118.43	240.50	0.0400		
138.62	241.68	0.0400		
152.72	241.60	0.0400		
166.82	242.68	0.0400		
185.62	249.09	0.0400		
197.36	251.00	0.0400		
232.61	252.28	0.0400		

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.28	240.09	.118E+04	0.1	0.20	142.09
0.91	240.72	.114E+05	2.4	0.35	79.92
1.53	241.34	.352E+05	9.4	0.45	62.17
2.16	241.97	.831E+05	22.8	0.46	60.69
2.79	242.60	.172E+06	54.2	0.53	52.74
3.41	243.22	.297E+06	111.4	0.63	44.49
4.04	243.85	.445E+06	194.1	0.74	38.22
4.67	244.48	.612E+06	301.1	0.83	33.88
5.29	245.10	.793E+06	443.0	0.94	29.82
5.92	245.73	.976E+06	611.3	1.06	26.60
6.55	246.36	.116E+07	802.0	1.17	24.12
7.18	246.99	.135E+07	1014.0	1.27	22.16
7.80	247.61	.154E+07	1246.2	1.37	20.56

8.43	248.24	.173E+07	1498.0	1.46	19.24
9.06	248.87	.192E+07	1768.8	1.55	18.12
9.68	249.49	.212E+07	2049.6	1.63	17.24
10.31	250.12	.232E+07	2344.7	1.71	16.49
10.94	250.75	.252E+07	2658.5	1.78	15.83
11.56	251.37	.274E+07	2931.2	1.81	15.56

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8038)	9106.02	27.09	7.75	9.71	2.25	0.47
OUTFLOW: ID= 1 (1365)	9106.02	22.21	8.75	9.71	2.14	0.46

CALIB STANDHYD (1321) ID= 1 DT=15.0 min	Area Total	(ha)= 208.90 Imp(%)= 67.00	Di r. Conn. (%)= 46.00
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	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	139.96	68.94
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1180.11	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	55.44	18.19
over (min)	15.00	45.00
Storage Coeff. (min)=	17.51 (ii)	38.66 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.07	0.03

PEAK FLOW (cms)=	10.06	1.89	10.829 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	40.00	12.08	24.93
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.29	0.59

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5132)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	7.9970	7.9437
0.9210	5.2645	12.4650	8.5839
3.2690	5.3433	16.9060	9.2236
6.3980	5.6925	17.3060	9.2336
7.1860	6.8155	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (1321)	208.900	10.829	6.00	24.93
OUTFLOW: ID= 1 (5132)	208.900	0.676	9.50	24.92

PEAK FLOW REDUCTION [Qout/Qin] (%) = 6.24
TIME SHIFT OF PEAK FLOW (min) = 210.00
MAXIMUM STORAGE USED (ha.m.) = 3.8639

CALIB STANDHYD (1322) ID= 1 DT=15.0 min	Area Total	(ha)= 81.94 Imp(%)= 37.00	Di r. Conn. (%)= 25.00
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	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	30.32	51.62
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	739.10	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	55.44	9.73
over (min)	15.00	45.00
Storage Coeff. (min)=	13.22 (ii)	40.39 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00

Unit Hyd. peak (cms)=	0.08	0.03	*TOTALS*
PEAK FLOW (cms)=	2.42	0.75	2.710 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	40.00	9.11	16.84
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.22	0.40

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8090) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1322):	81.94	2.710	6.00	16.84
+ ID2= 2 (5132):	208.90	0.676	9.50	24.92
=====	=====	=====	=====	=====
ID = 3 (8090):	290.84	2.974	6.00	22.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1342) ID= 1 DT=15.0 min	Area Total	(ha)= 97.16 Imp(%)= 41.00	Di r. Conn. (%)= 28.00
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	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	39.84	57.32
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	804.82	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	55.44	5.67
over (min)	15.00	60.00
Storage Coeff. (min)=	13.92 (ii)	47.63 (ii)
Unit Hyd. Tpeak (min)=	15.00	60.00
Unit Hyd. peak (cms)=	0.07	0.02

PEAK FLOW (cms)=	3.15	0.54	3.304 (iii)
TIME TO PEAK (hrs)=	6.00	6.75	6.00
RUNOFF VOLUME (mm)=	40.00	6.83	16.12
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.16	0.38

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 54.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1341) ID= 1 DT=15.0 min	Area Total	(ha)= 58.79 Imp(%)= 30.00	Di r. Conn. (%)= 20.00
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	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	17.64	41.15
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	626.05	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	55.44	4.97
over (min)	15.00	60.00
Storage Coeff. (min)=	11.97 (ii)	47.52 (ii)
Unit Hyd. Tpeak (min)=	15.00	60.00
Unit Hyd. peak (cms)=	0.08	0.02

PEAK FLOW (cms)=	1.44	0.34	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.75	1.537 (iii)
RUNOFF VOLUME (mm)=	40.00	6.39	6.00
			13.11

TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.15 0.31

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 54.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5134)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.1250	1.0755
0.1070	0.6119	1.7570	1.1641
0.4130	0.6225	2.3860	1.2526
0.8380	0.7018	2.7860	1.2626
0.9790	0.8884	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (1341)	58.790	1.537	6.00	13.11
OUTFLOW: ID= 1 (5134)	58.790	0.098	10.25	13.08

PEAK FLOW REDUCTION [Oout/Oi n] (%) = 6.41
 TIME SHIFT OF PEAK FLOW (min) = 255.00
 MAXIMUM STORAGE USED (ha.m.) = 0.5634

ADD HYD (9134)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1342):	97.16	3.304	6.00	16.12
+ ID2= 2 (5134):	58.79	0.098	10.25	13.08
ID = 3 (9134):	155.95	3.340	6.00	14.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8092)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8090):	290.84	2.974	6.00	22.64
+ ID2= 2 (9134):	155.95	3.340	6.00	14.97
ID = 3 (8092):	446.79	6.314	6.00	19.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (1366)
 IN= 2---> OUT= 1

Routing time step (min) = 15.00

----- DATA FOR SECTION (1361.0) -----

Distance	Elevation	Manning	
0.00	262.81	0.0400	
37.23	260.54	0.0400	
79.12	256.98	0.0400	
111.70	254.74	0.0400	
172.20	252.76	0.0400	
251.32	248.82	0.0400	
255.97	248.75	0.0400	
260.43	248.76	0.0400	
265.43	248.49	0.0400	
268.68	248.18	0.0400 / 0.0300	Main Channel
269.93	247.71	0.0300	Main Channel
271.18	247.71	0.0300	Main Channel
272.93	247.99	0.0300	Main Channel
277.93	248.40	0.0300 / 0.0400	Main Channel
283.90	248.88	0.0400	
307.17	249.60	0.0400	
321.13	251.59	0.0400	

363.01 257.08 0.0400
 400.25 257.58 0.0400
 460.75 261.91 0.0400

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.47	248.18	881E+04	1.3	0.76	114.66
1.19	248.90	742E+05	17.3	1.22	71.44
1.92	249.63	278E+06	78.3	1.48	59.26
2.64	250.35	589E+06	210.3	1.87	46.69
3.36	251.07	974E+06	414.3	2.23	39.19
4.08	251.79	143E+07	696.2	2.55	34.32
4.81	252.52	197E+07	1063.0	2.83	30.86
5.53	253.24	259E+07	1505.7	3.06	28.62
6.25	253.96	331E+07	2051.3	3.26	26.85
6.97	254.68	413E+07	2720.9	3.46	25.30
7.70	255.41	504E+07	3584.5	3.73	23.43
8.42	256.13	601E+07	4580.9	4.00	21.87
9.14	256.85	704E+07	5707.9	4.25	20.57
9.86	257.57	818E+07	6636.1	4.26	20.53
10.59	258.30	945E+07	8115.0	4.50	19.42
11.31	259.02	108E+08	9759.2	4.74	18.45
12.03	259.74	122E+08	11569.5	4.96	17.61
12.75	260.46	137E+08	13550.6	5.18	16.87
13.48	261.19	153E+08	15636.4	5.37	16.29

----- hydrograph -----

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8092)	446.79	6.31	6.00	19.97	0.70	0.86
OUTFLOW: ID= 1 (1366)	446.79	1.96	7.25	19.96	0.50	0.77

CALIB
 STANDHYD (1362)
 ID= 1 DT=15.0 min

Area (ha) = 371.79
 Total Imp (%) = 26.00
 Dir. Conn. (%) = 14.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	96.67	275.12
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1574.36	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 37.80 9.53
 over (min) = 30.00 60.00
 Storage Coeff. (min) = 24.26 (ii) 51.65 (ii)
 Unit Hyd. Tpeak (min) = 30.00 60.00
 Unit Hyd. peak (cms) = 0.04 0.02

PEAK FLOW (cms) = 3.95 3.20 *TOTALS*
 TIME TO PEAK (hrs) = 6.25 6.75 5.638 (iii)
 RUNOFF VOLUME (mm) = 40.00 9.15 13.47
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.22 0.32

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (1361)
 ID= 1 DT=15.0 min

Area (ha) = 140.62
 Total Imp (%) = 55.00
 Dir. Conn. (%) = 29.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	77.34	63.28
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	968.23	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 55.44 17.42
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 15.55 (ii) 37.07 (ii)

Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.07 0.03

TOTALS
 PEAK FLOW (cms)= 4.50 1.71 5.198 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 40.00 12.01 20.13
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.29 0.48

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5136)
 IN= 2---> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	4.7060	4.3486
	0.3100	2.4365	5.9200	5.0324
	1.7670	2.5993	7.0820	5.7151
	3.4020	2.8510	7.4820	5.7251
	4.0310	3.5962	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1361)	140.620	5.198	6.00	20.13
OUTFLOW: ID= 1 (5136)	140.620	0.283	10.50	20.12

PEAK FLOW	REDUCTION [Qout/Qin] (%)
TIME SHIFT OF PEAK FLOW	(min)=270.00
MAXIMUM STORAGE USED	(ha.m.)= 2.2247

ADD HYD (8096)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1362):	371.79	5.638	6.25	13.47
+ ID2= 2 (5136):	140.62	0.283	10.50	20.12
=====				
ID = 3 (8096):	512.41	5.776	6.25	15.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8094)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1366):	446.79	1.959	7.25	19.96
+ ID2= 2 (8096):	512.41	5.776	6.25	15.29
=====				
ID = 3 (8094):	959.20	7.314	6.25	17.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8098)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1365):	9106.02	22.208	8.75	9.71
+ ID2= 2 (8094):	959.20	7.314	6.25	17.47
=====				
ID = 3 (8098):	10065.22	25.661	8.50	10.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1381)
 ID= 1 DT=15.0 min

Area (ha)= 96.24
 Total Imp(%)= 59.00
 Dir. Conn.(%)= 34.00

Surface Area (ha)= IMPERVIOUS 56.78 PERVIOUS (i) 39.46

Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 801.00 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 55.44 3.68
 over (min) 15.00 60.00
 Storage Coeff. (min)= 13.88 (ii) 53.97 (ii)
 Unit Hyd. Tpeak (min)= 15.00 60.00
 Unit Hyd. peak (cms)= 0.07 0.02

TOTALS
 PEAK FLOW (cms)= 3.79 0.23 3.857 (iii)
 TIME TO PEAK (hrs)= 6.00 6.75 6.00
 RUNOFF VOLUME (mm)= 40.00 3.42 15.86
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.08 0.38

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 28.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5138)
 IN= 2---> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	6.4380	2.7648
	0.3260	1.4281	8.0830	3.0750
	1.8650	1.6876	9.4100	3.3790
	3.4730	2.0447	9.8100	3.3890
	4.8170	2.3809	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1381)	96.240	3.857	6.00	15.86
OUTFLOW: ID= 1 (5138)	96.240	0.245	8.50	15.84

PEAK FLOW REDUCTION [Qout/Qin] (%) = 6.36
 TIME SHIFT OF PEAK FLOW (min)=150.00
 MAXIMUM STORAGE USED (ha.m.)= 1.0746

CALIB NASHYD (1382)
 ID= 1 DT=15.0 min

Area (ha)= 318.55
 Ia (mm)= 5.00
 U.H. Tp(hrs)= 1.09
 Curve Number (CN)= 33.2
 # of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 4.976
 PEAK FLOW (cms)= 0.328 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 2.476
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.059

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9138)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1382):	318.55	0.328	8.00	2.48
+ ID2= 2 (5138):	96.24	0.245	8.50	15.84
=====				
ID = 3 (9138):	414.79	0.572	8.00	5.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1402)
 ID= 1 DT=15.0 min

Area (ha)= 131.22
 Total Imp(%)= 29.00
 Dir. Conn.(%)= 21.00

Surface Area (ha)= IMPERVIOUS 38.05 PERVIOUS (i) 93.17

Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 935.31 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 55.44 2.16
 over (min)= 15.00 75.00
 Storage Coeff. (min)= 15.23 (ii) 64.84 (ii)
 Unit Hyd. Tpeak (min)= 15.00 75.00
 Unit Hyd. peak (cms)= 0.07 0.02

PEAK FLOW (cms)= 3.07 0.31 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 7.00 3.139 (iii)
 RUNOFF VOLUME (mm)= 40.00 3.40 11.09
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.08 0.26

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 37.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1401)
 ID= 1 DT=15.0 min
 Area (ha)= 27.37
 Total Imp(%)= 45.00 Di r. Conn. (%)= 33.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	12.32	15.05
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	427.16	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	55.44	3.09
over (min)=	15.00	60.00
Storage Coeff. (min)=	9.52 (ii)	52.50 (ii)
Unit Hyd. Tpeak (min)=	15.00	60.00
Unit Hyd. peak (cms)=	0.09	0.02
PEAK FLOW (cms)=	1.19	0.07
TIME TO PEAK (hrs)=	6.00	6.75
RUNOFF VOLUME (mm)=	40.00	3.77
TOTAL RAINFALL (mm)=	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.09

TOTALS
 1.212 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 37.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5140) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	1.8310	0.7127
	0.0930	0.3344	2.2990	0.8148
	0.5300	0.3869	2.6760	0.9148
	0.9880	0.4994	3.0760	0.9248
	1.3700	0.5962	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1401)	27.370	1.212	6.00	15.72
OUTFLOW: ID= 1 (5140)	27.370	0.081	8.25	15.69

PEAK FLOW REDUCTION [Oout/Oi n] (%) = 6.65
 TIME SHIFT OF PEAK FLOW (min) = 135.00
 MAXIMUM STORAGE USED (ha.m.) = 0.2900

ADD HYD (9140)
 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)

ID1= 1 (1402):	131.22	3.139	6.00	11.09
+ ID2= 2 (5140):	27.37	0.081	8.25	15.69
ID= 3 (9140):	158.59	3.182	6.00	11.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8102) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9138):	414.79	0.572	8.00	5.58
+ ID2= 2 (9140):	158.59	3.182	6.00	11.88
ID= 3 (8102):	573.38	3.463	6.00	7.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6142)
 IN= 2--> OUT= 1
 Routing time step (min) = 15.00

DATA FOR SECTION (1421.0) ----->			
Distance	Elevation	Manning	
0.00	295.50	0.0400	
7.25	295.36	0.0400	
14.50	295.22	0.0400	
21.75	295.22	0.0400	
29.00	294.23	0.0400	
52.56	286.45	0.0400	
67.06	281.44	0.0400	
77.94	278.28	0.0400	
82.81	277.20	0.0400 / 0.0300	Main Channel
83.21	276.80	0.0300	Main Channel
88.81	276.80	0.0300	Main Channel
96.41	276.80	0.0300	Main Channel
96.81	277.20	0.0300 / 0.0400	Main Channel
103.31	278.96	0.0400	
108.75	280.16	0.0400	
117.81	282.73	0.0400	
135.94	290.30	0.0400	
159.50	292.14	0.0400	
174.00	293.07	0.0400	
179.44	293.37	0.0400	

TRAVEL TIME TABLE					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.40	277.20	.238E+05	6.9	1.27	57.28
1.25	278.05	.889E+05	50.9	2.50	29.13
2.10	278.90	.179E+06	133.4	3.26	22.36
2.95	279.75	.293E+06	256.7	3.83	19.03
3.80	280.60	.432E+06	426.0	4.31	16.90
4.66	281.46	.593E+06	644.3	4.75	15.35
5.51	282.31	.776E+06	915.3	5.16	14.12
6.36	283.16	.978E+06	1242.2	5.55	13.12
7.21	284.01	.120E+07	1626.2	5.94	12.27
8.06	284.86	.143E+07	2065.6	6.30	11.57
8.91	285.71	.169E+07	2562.4	6.64	10.97
9.76	286.56	.196E+07	3118.0	6.97	10.46
10.61	287.41	.224E+07	3733.3	7.28	10.02
11.46	288.26	.255E+07	4411.9	7.57	9.62
12.31	289.11	.287E+07	5155.6	7.86	9.27
13.17	289.97	.321E+07	5966.6	8.14	8.96
14.02	290.82	.357E+07	6705.0	8.22	8.87
14.87	291.67	.398E+07	7494.3	8.24	8.85
15.72	292.52	.444E+07	8405.3	8.28	8.80

hydrograph						
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8102)	573.38	3.46	6.00	7.32	0.20	1.27
OUTFLOW: ID= 1 (6142)	573.38	1.45	6.75	7.32	0.08	1.27

CALIB STANDHYD (1421)
 ID= 1 DT=15.0 min
 Area (ha)= 69.77
 Total Imp(%)= 45.00 Di r. Conn. (%)= 15.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	31.40	38.37	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	682.01	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	55.44	13.73	
over (min)=	15.00	45.00	
Storage Coeff. (min)=	12.60 (ii)	36.27 (ii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.08	0.03	
PEAK FLOW (cms)=	1.26	0.83	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.50	1.591 (iii)
RUNOFF VOLUME (mm)=	40.00	9.82	6.00
TOTAL RAINFALL (mm)=	42.00	42.00	14.35
RUNOFF COEFFICIENT =	0.95	0.23	42.00
			0.34

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5142)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.6670	1.8169
0.2360	0.8525	5.8600	2.0773
1.3520	0.9863	6.8220	2.3321
2.5180	1.2732	7.2220	2.3421
3.4920	1.5199	0.0000	0.0000

INFLOW : ID= 2 (1421)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5142)	69.770	1.591	6.00	14.35
	69.770	0.184	9.00	14.34

PEAK FLOW REDUCTION [Qout/Qin] (%) = 11.54
 TIME SHIFT OF PEAK FLOW (min) = 180.00
 MAXIMUM STORAGE USED (ha. m.) = 0.6633

CALIB NASHYD (1422)
 ID= 1 DT=15.0 min

Area (ha) = 326.77 Curve Number (CN) = 61.2
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp (hrs) = 1.21

Unit Hyd Opeak (cms) = 4.626
 PEAK FLOW (cms) = 0.884 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 6.860
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.163

- (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9142)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1422):	326.77	0.884	8.00	6.86
+ ID2= 2 (5142):	69.77	0.184	9.00	14.34
=====	=====	=====	=====	=====
ID = 3 (9142):	396.54	1.062	8.00	8.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8104)

1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6142):	573.38	1.451	6.75	7.32
+ ID2= 2 (9142):	396.54	1.062	8.00	8.18
=====	=====	=====	=====	=====
ID = 3 (8104):	969.92	2.391	7.00	7.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1441)
 ID= 1 DT=15.0 min

Area (ha) = 6.29
 Total Imp (%) = 45.00 Dir. Conn. (%) = 15.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	2.83	3.46
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	204.78	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 55.44 19.54
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 6.12 (ii) 26.68 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.10 0.04

PEAK FLOW (cms) = 0.14 0.13 *TOTALS*
 TIME TO PEAK (hrs) = 6.00 6.25 0.220 (iii)
 RUNOFF VOLUME (mm) = 40.00 13.59 6.00
 TOTAL RAINFALL (mm) = 42.00 42.00 17.55
 RUNOFF COEFFICIENT = 0.95 0.32 0.42

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5144)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.4210	0.1639
0.0210	0.0769	0.5290	0.1874
0.1220	0.0890	0.6150	0.2104
0.2270	0.1148	1.0150	0.2204
0.3150	0.1371	0.0000	0.0000

INFLOW : ID= 2 (1441)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5144)	6.290	0.220	6.00	17.55
	6.290	0.020	8.50	17.41

PEAK FLOW REDUCTION [Qout/Qin] (%) = 9.25
 TIME SHIFT OF PEAK FLOW (min) = 150.00
 MAXIMUM STORAGE USED (ha. m.) = 0.0744

CALIB NASHYD (1442)
 ID= 1 DT=15.0 min

Area (ha) = 129.40 Curve Number (CN) = 74.9
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp (hrs) = 0.70

Unit Hyd Opeak (cms) = 3.147
 PEAK FLOW (cms) = 0.865 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 11.017
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.262

- (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9144)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1442):	129.40	0.865	7.00	11.02
+ ID2= 2 (5144):	6.29	0.020	8.50	17.41

ID = 3 (9144):	135.69	0.883	7.00	11.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8106)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8104):	969.92	2.391	7.00	7.67
+ ID2= 2 (9144):	135.69	0.883	7.00	11.31

ID = 3 (8106):	1105.61	3.274	7.00	8.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6146)		Routing time step (min) = 15.00	
IN= 2--> OUT= 1			

DATA FOR SECTION (1461.0) ----->			
Distance	Elevation	Manning	
0.00	262.00	0.0350	
39.58	257.13	0.0350	
123.69	257.51	0.0350	
183.07	256.83	0.0350	
202.86	255.73	0.0350	
212.75	254.73	0.0350	
222.65	252.86	0.0350	
232.44	250.70	0.0350 / 0.0300	Main Channel
236.44	250.20	0.0300	Main Channel
236.84	249.80	0.0300	Main Channel
242.44	249.80	0.0300	Main Channel
250.04	249.80	0.0300	Main Channel
250.44	250.20	0.0300	Main Channel
252.33	250.57	0.0300 / 0.0350	Main Channel
286.97	253.52	0.0350	
331.50	255.80	0.0350	
380.98	255.55	0.0350	
445.30	258.40	0.0350	
455.19	259.82	0.0350	
489.83	262.22	0.0350	

TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.39	250.19	.169E+05	5.4	1.04	52.00
0.77	250.57	.373E+05	16.6	1.44	37.46
1.44	251.24	.911E+05	60.3	2.14	25.19
2.11	251.91	.169E+06	136.3	2.61	20.64
2.79	252.59	.270E+06	249.7	2.99	18.05
3.46	253.26	.396E+06	405.4	3.31	16.28
4.13	253.93	.549E+06	603.3	3.56	15.15
4.80	254.60	.737E+06	857.8	3.77	14.31
5.48	255.28	.963E+06	1174.6	3.95	13.67
6.15	255.95	.128E+07	1449.5	3.66	14.74
6.82	256.62	.173E+07	2046.4	3.83	14.07
7.49	257.29	.227E+07	2665.2	3.81	14.17
8.17	257.97	.310E+07	3705.2	3.87	13.93
8.84	258.64	.400E+07	5127.4	4.15	12.99
9.51	259.31	.492E+07	6858.6	4.51	11.95
10.18	259.98	.587E+07	8815.5	4.86	11.09
10.86	260.66	.684E+07	10954.6	5.18	10.41
11.53	261.33	.785E+07	13324.3	5.49	9.82
12.20	262.00	.890E+07	15923.2	5.79	9.31

<---- hydrograph ---->					<- pi pe / channel -->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8106)	1105.61	3.27	7.00	8.12	0.23	1.04
OUTFLOW: ID= 1 (6146)	1105.61	2.90	8.25	8.12	0.21	1.04

CALIB STANDHYD (1462)			
ID= 1 DT=15.0 mi n			
Area Total	(ha)	Imp(%)	Di r. Conn. (%)
	112.22	32.00	13.00

IMPERVIOUS			PERVIOUS (i)		
Surface Area (ha)	35.91		76.31		
Dep. Storage (mm)	2.00		5.00		
Average Slope (%)	0.50		0.50		
Length (m)	864.95		40.00		
Mannings n	0.013		0.250		
Max. Eff. Inten. (mm/hr)	55.44		15.36		
over (mi n)	15.00		45.00		
Storage Coeff. (mi n)	14.53 (ii)		37.16 (ii)		
Unit Hyd. Tpeak (mi n)	15.00		45.00		
Unit Hyd. peak (cms)	0.07		0.03		
PEAK FLOW (cms)	1.66		1.81		*TOTALS*
TIME TO PEAK (hrs)	6.00		6.50		2.387 (iii)
RUNOFF VOLUME (mm)	40.00		12.97		6.00
TOTAL RAI NFALL (mm)	42.00		42.00		16.48
RUNOFF COEFFICIENT	0.95		0.31		42.00
					0.39

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1461)			
ID= 1 DT=15.0 mi n			
Area Total	(ha)	Imp(%)	Di r. Conn. (%)
	83.29	38.00	16.00

IMPERVIOUS			PERVIOUS (i)		
Surface Area (ha)	31.65		51.64		
Dep. Storage (mm)	2.00		5.00		
Average Slope (%)	0.50		0.50		
Length (m)	745.16		40.00		
Mannings n	0.013		0.250		
Max. Eff. Inten. (mm/hr)	55.44		17.13		
over (mi n)	15.00		45.00		
Storage Coeff. (mi n)	13.29 (ii)		34.95 (ii)		
Unit Hyd. Tpeak (mi n)	15.00		45.00		
Unit Hyd. peak (cms)	0.08		0.03		
PEAK FLOW (cms)	1.57		1.41		*TOTALS*
TIME TO PEAK (hrs)	6.00		6.50		2.143 (iii)
RUNOFF VOLUME (mm)	40.00		13.59		6.00
TOTAL RAI NFALL (mm)	42.00		42.00		17.81
RUNOFF COEFFICIENT	0.95		0.32		42.00
					0.42

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5146)				
IN= 2--> OUT= 1				
DT= 15.0 mi n				
OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)	
0.0000	0.0000	8.9200	2.5813	
0.2820	0.8782	11.0570	3.0328	
2.5360	1.0372	12.9860	3.4993	
4.7880	1.7148	13.3860	3.5093	
6.7570	2.1134	0.0000	0.0000	

INFLOW : ID= 2 (1461)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5146)	83.290	2.143	6.00	17.81
	83.290	0.547	7.75	17.81

PEAK FLOW REDUCTION [Qout/Qin] (%) = 25.53
 TIME SHIFT OF PEAK FLOW (min) = 105.00
 MAXIMUM STORAGE USED (ha.m.) = 0.8971

ADD HYD	(8099)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (1462):	112.22	2.387	6.00	16.48	
+ ID2= 2 (5146):	83.29	0.547	7.75	17.81	
ID = 3 (8099):	195.51	2.470	6.00	17.05	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8108)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (6146):	1105.61	2.900	8.25	8.12	
+ ID2= 2 (8099):	195.51	2.470	6.00	17.05	
ID = 3 (8108):	1301.12	4.078	7.75	9.46	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8100)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (8098):	10065.22	25.661	8.50	10.44	
+ ID2= 2 (8108):	1301.12	4.078	7.75	9.46	
ID = 3 (8100):	11366.34	29.557	8.25	10.33	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6202)
 IN= 2--> OUT= 1
 Routing time step (min) = 15.00

Distance	Elevation	Manning	
0.00	245.48	0.0400	
29.68	242.80	0.0400	
74.20	240.39	0.0400	
155.81	237.12	0.0400	
200.33	234.48	0.0400	
244.85	234.67	0.0400	
259.69	233.96	0.0400	
274.53	233.84	0.0400	
281.68	233.40	0.0400	
285.78	232.40	0.0400 / 0.0300	Main Channel
286.78	231.40	0.0300	Main Channel
296.78	231.40	0.0300	Main Channel
299.78	232.40	0.0300 / 0.0400	Main Channel
311.62	233.74	0.0400	
333.88	234.34	0.0400	
497.11	235.32	0.0400	
571.31	237.32	0.0400	
675.18	239.10	0.0400	
704.86	242.71	0.0400	
734.54	242.62	0.0400	

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)
0.50	231.90	.221E+05	4.7	0.85	78.67
1.00	232.40	.482E+05	15.3	1.28	52.40
1.60	233.00	.913E+05	38.3	1.68	39.77
2.20	233.60	.154E+06	72.9	1.90	35.24
2.80	234.20	.278E+06	124.2	1.79	37.30
3.40	234.80	.599E+06	214.3	1.44	46.59
4.01	235.41	.125E+07	445.0	1.43	46.90
4.61	236.01	.205E+07	838.4	1.64	40.81
5.21	236.61	.293E+07	1361.6	1.86	35.89

5.81	237.21	.389E+07	2011.2	2.08	32.23
6.41	237.81	.494E+07	2753.5	2.24	29.91
7.01	238.41	.612E+07	3635.0	2.39	28.04
7.61	239.01	.741E+07	4672.5	2.53	26.43
8.21	239.61	.880E+07	5999.3	2.74	24.44
8.82	240.22	.102E+08	7510.0	2.94	22.72
9.42	240.82	.117E+08	9201.9	3.15	21.23
10.02	241.42	.132E+08	11059.7	3.35	19.95
10.62	242.02	.148E+08	13073.4	3.54	18.87
11.22	242.62	.164E+08	15242.1	3.73	17.93

INFLOW	ID= 2 (8100)	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
		(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
OUTFLOW:	ID= 1 (6202)	*****	29.56	8.25	10.33	1.37	1.50
		*****	28.31	9.00	10.33	1.33	1.47

ADD HYD	(8112)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (6202):	11366.34	28.311	9.00	10.33	
+ ID2= 2 (8110):	468.53	13.711	6.25	23.64	
ID = 3 (8112):	11834.87	30.320	8.75	10.86	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5555)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min				
	0.0000	0.0000	*****	19.2100
	2.8500	0.5200	*****	24.7300
	8.1800	1.2600	*****	30.8600
	15.0900	2.2100	*****	37.6100
	23.2700	3.3900	*****	41.2200
	32.5300	4.7800	*****	48.9000
	36.5100	5.3900	*****	66.4700
	42.7600	6.3700	*****	100.1200
	53.8500	8.1300	*****	140.4100
	91.7300	14.3200	*****	140.4200

INFLOW	ID= 2 (8112)	AREA	OPEAK	TPEAK	R. V.
		(ha)	(cms)	(hrs)	(mm)
OUTFLOW:	ID= 1 (5555)	*****	30.320	8.75	10.86
		*****	29.978	9.25	10.86

PEAK FLOW REDUCTION [Qout/Qin] (%) = 98.87
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 4.4019

ADD HYD	(8114)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (5555):	11834.87	29.978	9.25	10.86	
+ ID2= 2 (8136):	2288.44	9.668	6.00	10.45	
ID = 3 (8114):	14123.31	35.775	9.00	10.79	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	Di r. Conn. (%)
STANDHYD (2061)	Total	Imp (%) = 76.00	50.00
ID= 1 DT=15.0 min			

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	37.88	11.96
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	576.43	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	55.44	63.60

over (min) 15.00 30.00
 Storage Coeff. (min)= 11.39 (ii) 24.21 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

TOTALS
 PEAK FLOW (cms)= 3.11 0.98 3.735 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 40.00 19.70 29.85
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.47 0.71

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5206)
 IN= 2----> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.5640	2.4459
0.1690	0.8986	4.2460	2.7424
1.3810	1.2959	4.8070	3.0770
2.2440	1.6904	5.2070	3.0870
2.8510	2.0623	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2061)	49.840	3.735	6.00	29.85
OUTFLOW: ID= 1 (5206)	49.840	0.469	7.25	29.82

PEAK FLOW REDUCTION [Qout/Qin] (%) = 12.57
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha. m.) = 0.9980

CALIB STANDHYD (2062)
 ID= 1 DT=15.0 min

Area (ha)= 225.70
 Total Imp (%) = 64.00
 Dir. Conn. (%) = 42.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 144.45 81.25
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1226.65 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 55.44 25.58
 over (min) 15.00 45.00
 Storage Coeff. (min)= 17.92 (ii) 36.38 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.06 0.03

TOTALS
 PEAK FLOW (cms)= 9.81 3.22 11.164 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 40.00 16.70 26.48
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.40 0.63

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8144)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2062):	225.70	11.164	6.00	26.48
+ ID2= 2 (5206):	49.84	0.469	7.25	29.82
ID = 3 (8144):	275.54	11.254	6.00	27.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2041)
 ID= 1 DT=15.0 min

Area (ha)= 209.90
 Total Imp (%) = 37.00
 Dir. Conn. (%) = 21.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 77.66 132.24
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1182.93 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 55.44 9.67
 over (min) 15.00 45.00
 Storage Coeff. (min)= 17.53 (ii) 44.77 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.07 0.03

TOTALS
 PEAK FLOW (cms)= 4.61 1.79 5.308 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 40.00 8.63 15.21
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.21 0.36

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5204)
 IN= 2----> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	13.0280	5.1901
0.5340	2.0405	16.1840	5.6427
3.0020	2.3552	19.0840	6.7700
5.5440	3.8411	19.4840	6.7800
8.4130	4.5335	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2041)	209.900	5.308	6.00	15.21
OUTFLOW: ID= 1 (5204)	209.900	0.812	8.25	15.21

PEAK FLOW REDUCTION [Qout/Qin] (%) = 15.30
 TIME SHIFT OF PEAK FLOW (min) = 135.00
 MAXIMUM STORAGE USED (ha. m.) = 2.0766

CALIB STANDHYD (2042)
 ID= 1 DT=15.0 min

Area (ha)= 114.52
 Total Imp (%) = 61.00
 Dir. Conn. (%) = 34.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 69.86 44.66
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 873.77 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 55.44 17.48
 over (min) 15.00 45.00
 Storage Coeff. (min)= 14.62 (ii) 36.11 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.07 0.03

TOTALS
 PEAK FLOW (cms)= 4.42 1.23 4.914 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 40.00 11.29 21.05
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.27 0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	(8140)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2042):	114.52	4.914	6.00	21.05	
+ ID2= 2 (5204):	209.90	0.812	8.25	15.21	
ID = 3 (8140):	324.42	5.098	6.00	17.27	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6206)	ROUTING TIME STEP (min) = 15.00
IN= 2---> OUT= 1	

<----- DATA FOR SECTION (2061.0) ----->

Distance	Elevation	Manning	
0.00	253.07	0.0350	
13.98	252.89	0.0350	
27.95	251.99	0.0350	
34.94	252.18	0.0350	
41.93	252.02	0.0350	
73.38	250.35	0.0350	
94.34	247.09	0.0350	
111.81	244.79	0.0350	
115.31	244.24	0.0350	
117.55	244.20	0.0350 / 0.0300	Main Channel
118.80	243.60	0.0300	Main Channel
122.30	243.60	0.0300	Main Channel
122.55	244.60	0.0300 / 0.0350	Main Channel
146.76	244.74	0.0350	
164.23	247.26	0.0350	
213.15	248.92	0.0350	
241.10	249.51	0.0350	
272.55	250.87	0.0350	
300.50	252.26	0.0350	
345.93	253.67	0.0350	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.30	243.90	.310E+04	1.4	1.23	36.25
0.60	244.20	.676E+04	4.6	1.81	24.64
1.12	244.72	.221E+05	17.1	2.07	21.60
1.64	245.24	.754E+05	64.1	2.28	19.60
2.17	245.77	.139E+06	149.8	2.88	15.50
2.69	246.29	.214E+06	273.3	3.43	13.04
3.21	246.81	.299E+06	436.1	3.91	11.43
3.73	247.33	.395E+06	632.6	4.30	10.40
4.25	247.85	.511E+06	836.4	4.39	10.18
4.77	248.37	.653E+06	1114.9	4.58	9.76
5.30	248.90	.822E+06	1472.1	4.81	9.30
5.82	249.42	.102E+07	1872.2	4.91	9.10
6.34	249.94	.126E+07	2444.6	5.22	8.56
6.86	250.46	.151E+07	3113.1	5.52	8.09
7.38	250.98	.180E+07	3843.5	5.74	7.79
7.90	251.50	.211E+07	4717.8	6.00	7.45
8.43	252.03	.245E+07	5701.4	6.24	7.16
8.95	252.55	.284E+07	6727.5	6.36	7.03
9.47	253.07	.326E+07	7885.3	6.48	6.90

<----- hydrograph -----> <- pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8140)	324.42	5.10	6.00	17.27	0.62	1.82
OUTFLOW: ID= 1 (6206)	324.42	2.92	6.50	17.27	0.44	1.45

ADD HYD 1 + 2 = 3	(8142)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6206):	324.42	2.922	6.50	17.27	
+ ID2= 2 (8144):	275.54	11.254	6.00	27.09	
ID = 3 (8142):	599.96	13.079	6.00	21.78	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	(8138)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8114):	14123.31	35.775	9.00	10.79	
+ ID2= 2 (8142):	599.96	13.079	6.00	21.78	
ID = 3 (8138):	14723.27	38.256	8.75	11.24	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6208)	ROUTING TIME STEP (min) = 15.00
IN= 2---> OUT= 1	

<----- DATA FOR SECTION (2081.0) ----->

Distance	Elevation	Manning	
0.00	249.19	0.1000	
73.11	244.13	0.1000	
121.86	243.39	0.1000	
243.71	240.01	0.1000	
353.38	238.75	0.1000	
450.87	235.40	0.1000	
499.61	231.40	0.1000	
511.80	230.96	0.1000	
523.98	230.12	0.1000	
534.50	227.80	0.1000 / 0.0500	Main Channel
536.50	226.70	0.0500	Main Channel
546.50	226.70	0.0500	Main Channel
548.50	227.80	0.0500 / 0.1000	Main Channel
572.72	230.25	0.1000	
682.40	230.50	0.1000	
804.25	233.95	0.1000	
926.11	235.83	0.1000	
1047.96	240.70	0.1000	
1145.45	243.26	0.1000	
1206.38	247.03	0.1000	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.55	227.25	.111E+05	2.0	0.33	92.44
1.10	227.80	.242E+05	6.5	0.50	61.56
2.23	228.93	.700E+05	26.0	0.68	44.91
3.36	230.06	.150E+06	60.8	0.74	41.04
4.49	231.19	.446E+06	130.8	0.54	56.83
5.62	232.32	.925E+06	287.4	0.57	53.66
6.76	233.46	.152E+07	522.9	0.63	48.32
7.89	234.59	.223E+07	827.2	0.68	44.91
9.02	235.72	.312E+07	1219.5	0.72	42.61
10.15	236.85	.418E+07	1780.6	0.78	39.11
11.28	237.98	.537E+07	2479.2	0.85	36.07
12.41	239.11	.669E+07	3282.9	0.90	33.95
13.54	240.24	.824E+07	4228.6	0.94	32.47
14.67	241.37	.997E+07	5384.6	0.99	30.87
15.81	242.51	.119E+08	6698.4	1.03	29.56
16.94	243.64	.140E+08	8235.2	1.08	28.25
18.07	244.77	.162E+08	10104.2	1.14	26.73
19.20	245.90	.185E+08	12251.5	1.21	25.21
20.33	247.03	.209E+08	14597.0	1.28	23.89

<----- hydrograph -----> <- pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8138)	*****	38.26	8.75	11.24	2.63	0.70
OUTFLOW: ID= 1 (6208)	*****	36.80	9.50	11.24	2.58	0.70

CALIB STANDBYD (2082)	Area Total	(ha)	Imp (%) =	Dir. Conn. (%) =
ID= 1 DT=15.0 min	426.60	64.00		37.00
Surface Area	(ha) =	273.02	PERVIOUS (i)	153.58
Dep. Storage	(mm) =	2.00		5.00
Average Slope	(%) =	0.50		0.50

Length (m) = 1686.42 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 37.80 24.86
 over (min) = 30.00 45.00

Storage Coeff. (min) = 25.28 (ii) 43.95 (ii)
 Unit Hyd. Tpeak (min) = 30.00 45.00
 Unit Hyd. peak (cms) = 0.04 0.03

PEAK FLOW (cms) = 11.76 5.34 *TOTALS*
 TIME TO PEAK (hrs) = 6.25 6.50 15.690 (iii)
 RUNOFF VOLUME (mm) = 40.00 15.12 6.25
 TOTAL RAINFALL (mm) = 42.00 42.00 24.32
 RUNOFF COEFFICIENT = 0.95 0.36 42.00
 0.58

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2081)
 ID= 1 DT=15.0 min

Area (ha) = 209.70
 Total Imp(%) = 35.00 Dir. Conn.(%) = 20.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha) = 73.39 136.31
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 1182.37 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 55.44 12.81
 over (min) = 15.00 45.00

Storage Coeff. (min) = 17.53 (ii) 41.86 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.07 0.03

PEAK FLOW (cms) = 4.39 2.53 *TOTALS*
 TIME TO PEAK (hrs) = 6.00 6.50 5.388 (iii)
 RUNOFF VOLUME (mm) = 40.00 11.39 6.00
 TOTAL RAINFALL (mm) = 42.00 42.00 17.11
 RUNOFF COEFFICIENT = 0.95 0.27 42.00
 0.41

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2084)
 ID= 1 DT=15.0 min

Area (ha) = 56.00
 Total Imp(%) = 75.00 Dir. Conn.(%) = 70.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha) = 42.00 14.00
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 611.01 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 55.44 19.49
 over (min) = 15.00 45.00

Storage Coeff. (min) = 11.80 (ii) 32.37 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.08 0.03

PEAK FLOW (cms) = 4.83 0.45 *TOTALS*
 TIME TO PEAK (hrs) = 6.00 6.50 5.014 (iii)
 RUNOFF VOLUME (mm) = 40.00 16.98 6.00
 TOTAL RAINFALL (mm) = 42.00 42.00 33.09
 RUNOFF COEFFICIENT = 0.95 0.40 42.00
 0.79

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

- CN* = 82.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9289)
 IN= 2--> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	2.8000	1.5745
	0.1900	0.9984	3.7500	1.7809
	1.0900	1.1991	4.7100	1.9291
	2.0200	1.3813	5.4800	2.0745

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2084)	56.000	5.014	6.00	33.09
OUTFLOW: ID= 1 (9289)	56.000	0.896	6.75	33.07

PEAK FLOW REDUCTION [Qout/Qin] (%) = 17.86
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 1.1575

ADD HYD (9262)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2081):	209.70	5.388	6.00	17.11
+ ID2= 2 (9289):	56.00	0.896	6.75	33.07
=====				
ID = 3 (9262):	265.70	5.515	6.00	20.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5208)
 IN= 2--> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	16.8530	8.3371
	0.8360	4.7353	21.1630	9.3975
	4.7810	5.1070	25.1140	10.4406
	8.9050	6.2484	25.5140	10.4506
	12.6070	7.1796	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9262)	265.700	5.515	6.00	20.47
OUTFLOW: ID= 1 (5208)	265.700	0.580	10.75	20.47

PEAK FLOW REDUCTION [Qout/Qin] (%) = 10.51
 TIME SHIFT OF PEAK FLOW (min) = 285.00
 MAXIMUM STORAGE USED (ha. m.) = 3.2830

ADD HYD (8148)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2082):	426.60	15.690	6.25	24.32
+ ID2= 2 (5208):	265.70	0.580	10.75	20.47
=====				
ID = 3 (8148):	692.30	15.899	6.25	22.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8146)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6208):	14723.27	36.799	9.50	11.24
+ ID2= 2 (8148):	692.30	15.899	6.25	22.84
=====				
ID = 3 (8146):	15415.57	38.892	9.25	11.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6210)
 IN= 2--> OUT= 1
 Routing time step (mi n)' = 15.00

----- DATA FOR SECTION (2101.0) -----

Distance	Elevation	Manning	
0.00	241.81	0.0800	
28.43	241.78	0.0800	
85.30	237.04	0.0800	
149.27	231.32	0.0800	
170.60	230.10	0.0800	
255.89	229.37	0.0800	
277.22	228.68	0.0800	
284.33	228.42	0.0800	
291.43	228.16	0.0800	
297.76	227.00	0.0800 / 0.0350	Main Channel
298.76	225.00	0.0350	Main Channel
312.76	225.00	0.0350	Main Channel
313.76	227.00	0.0350 / 0.0800	Main Channel
326.97	229.50	0.0800	
348.30	230.29	0.0800	
376.73	229.76	0.0800	
490.46	235.75	0.0800	
568.65	236.24	0.0800	
661.06	241.85	0.0800	
703.71	246.04	0.0800	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.67	225.67	.141E+05	6.6	0.69	35.86
1.33	226.33	.289E+05	20.4	1.04	23.65
2.00	227.00	.443E+05	39.3	1.31	18.81
2.93	227.93	.730E+05	77.8	1.57	15.64
3.85	228.85	.123E+06	131.1	1.57	15.65
4.78	229.78	.228E+06	205.6	1.33	18.51
5.70	230.70	.503E+06	356.7	1.05	23.49
6.63	231.63	.846E+06	608.7	1.06	23.17
7.55	232.55	.123E+07	946.4	1.14	21.65
8.48	233.48	.165E+07	1367.0	1.22	20.13
9.41	234.41	.211E+07	1871.8	1.31	18.80
10.33	235.33	.261E+07	2463.2	1.40	17.65
11.26	236.26	.317E+07	2999.4	1.40	17.63
12.18	237.18	.384E+07	3848.3	1.48	16.63
13.11	238.11	.454E+07	4806.0	1.56	15.75
14.03	239.03	.528E+07	5876.9	1.64	14.97
14.96	239.96	.605E+07	7062.7	1.72	14.29
15.88	240.88	.687E+07	8365.0	1.80	13.68
16.81	241.81	.771E+07	9506.9	1.82	13.52

----- hydrograph -----

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8146)	*****	38.89	9.25	11.76	1.99	1.30
OUTFLOW: ID= 1 (6210)	*****	38.87	9.50	11.76	1.97	1.30

CALIB STANDHYD (2104)
 ID= 1 DT=15.0 mi n
 Area (ha)= 192.70
 Total Imp(%)= 73.00
 Di r. Conn.(%)= 68.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	140.67	52.03
Dep. Storage	1.00	5.00
Average Slope	1.00	2.00
Length	1133.43	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)= over (mi n)	55.44 / 15.00	29.98 / 30.00
Storage Coeff. (mi n)=	13.88 (ii)	25.31 (ii)
Uni t Hyd. Tpeak (mi n)=	15.00	30.00
Uni t Hyd. peak (cms)=	0.07	0.04
PEAK FLOW (cms)=	15.18	1.94
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	41.00	16.21
TOTAL RAINFALL (mm)=	42.00	42.00
RUNOFF COEFFICIENT =	0.98	0.39

TOTALS
 16.372 (iii)
 6.00
 33.07
 42.00
 0.79

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 81.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9283)
 IN= 2--> OUT= 1
 DT= 15.0 mi n

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	2.5000	8.0000
	0.3000	2.0000	3.5000	10.0000
	0.9000	5.0000	4.5000	13.0000
	2.0000	7.5000	5.5000	14.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2104)	192.700	16.372	6.00	33.07
OUTFLOW: ID= 1 (9283)	192.700	0.858	8.50	33.06

PEAK FLOW REDUCTION [Qout/Qi n](%)= 5.24
 TIME SHIF T OF PEAK FLOW (mi n)=150.00
 MAXIMUM STORAGE USED (ha. m.)= 4.7926

CALIB STANDHYD (2103)
 ID= 1 DT=15.0 mi n
 Area (ha)= 11.10
 Total Imp(%)= 30.00
 Di r. Conn.(%)= 10.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	3.33	7.77
Dep. Storage	1.00	5.00
Average Slope	1.00	2.00
Length	272.03	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)= over (mi n)	55.44 / 15.00	38.84 / 30.00
Storage Coeff. (mi n)=	5.90 (ii)	16.20 (ii)
Uni t Hyd. Tpeak (mi n)=	15.00	30.00
Uni t Hyd. peak (cms)=	0.10	0.05
PEAK FLOW (cms)=	0.16	0.46
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	41.00	19.18
TOTAL RAINFALL (mm)=	42.00	42.00
RUNOFF COEFFICIENT =	0.98	0.46

TOTALS
 0.495 (iii)
 6.25
 21.36
 42.00
 0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9284)
 IN= 2--> OUT= 1
 DT= 15.0 mi n

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	0.5600	0.2067
	0.0400	0.1045	0.7400	0.2572
	0.2200	0.1166	0.9300	0.3046
	0.4000	0.1670	1.0900	0.3510

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2103)	11.100	0.495	6.25	21.36
OUTFLOW: ID= 1 (9284)	11.100	0.221	6.75	21.30

PEAK FLOW REDUCTION [Qout/Qi n](%)= 44.59
 TIME SHIF T OF PEAK FLOW (mi n)= 30.00
 MAXIMUM STORAGE USED (ha. m.)= 0.1184

ADD HYD (9258)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9283):	192.70	0.858	8.50	33.06
+ ID2= 2 (9284):	11.10	0.221	6.75	21.30
=====				
ID = 3 (9258):	203.80	0.987	6.75	32.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5210)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0060	0.0024
0.0020	0.0013	0.0080	0.0027
0.0030	0.0017	0.0090	0.0031
0.0050	0.0020	0.4090	0.0131

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9258)	203.800	0.987	6.75	32.42
OUTFLOW: ID= 1 (5210)	203.800	0.983	7.00	32.42

PEAK FLOW REDUCTION [Qout/Oin] (%) = 99.57
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha.m.) = 0.0281

CALIB NASHYD (2102)
ID= 1 DT=15.0 min

Area (ha) = 14.38 Curve Number (CN) = 80.0
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U.H. Tp (hrs) = 1.08

Unit Hyd Qpeak (cms) =	0.228
PEAK FLOW (cms) =	0.085 (i)
TIME TO PEAK (hrs) =	7.500
RUNOFF VOLUME (mm) =	13.497
TOTAL RAINFALL (mm) =	42.000
RUNOFF COEFFICIENT =	0.321

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9210)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2102):	14.38	0.085	7.50	13.50
+ ID2= 2 (5210):	203.80	0.983	7.00	32.42
=====				
ID = 3 (9210):	218.18	1.066	7.00	31.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8150)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6210):	15415.57	38.867	9.50	11.76
+ ID2= 2 (9210):	218.18	1.066	7.00	31.17
=====				
ID = 3 (8150):	15633.75	39.826	9.50	12.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2121)
ID= 1 DT=15.0 min

Area (ha) = 56.89 Dir. Conn. (%) = 74.00
Total Imp (%) = 91.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) =	51.77	5.12
Dep. Storage (mm) =	2.00	5.00

Average Slope (%) = 0.50
Length (m) = 615.85
Mannings n = 0.013

Max. Eff. Inten. (mm/hr) = 55.44
over (min) = 15.00
Storage Coeff. (min) = 11.85 (ii)
Unit Hyd. Tpeak (min) = 15.00
Unit Hyd. peak (cms) = 0.08

PEAK FLOW (cms) = 5.18
TIME TO PEAK (hrs) = 6.00
RUNOFF VOLUME (mm) = 40.00
TOTAL RAINFALL (mm) = 42.00
RUNOFF COEFFICIENT = 0.95

TOTALS
5.575 (iii)
6.00
34.90
42.00
0.83

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2124)
ID= 1 DT=15.0 min

Area (ha) = 4.40
Total Imp (%) = 75.00 Dir. Conn. (%) = 70.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 3.30
Dep. Storage (mm) = 2.00
Average Slope (%) = 0.50
Length (m) = 171.27
Mannings n = 0.013

Max. Eff. Inten. (mm/hr) = 55.44
over (min) = 15.00
Storage Coeff. (min) = 5.50 (ii)
Unit Hyd. Tpeak (min) = 15.00
Unit Hyd. peak (cms) = 0.11

PEAK FLOW (cms) = 0.45
TIME TO PEAK (hrs) = 6.00
RUNOFF VOLUME (mm) = 40.00
TOTAL RAINFALL (mm) = 42.00
RUNOFF COEFFICIENT = 0.95

TOTALS
0.463 (iii)
6.00
31.23
42.00
0.74

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9290)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.2200	0.1226
0.0100	0.0778	0.2900	0.1387
0.0800	0.0934	0.3700	0.1503
0.1600	0.1076	0.4300	0.1616

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2124)	4.400	0.463	6.00	31.23
OUTFLOW: ID= 1 (9290)	4.400	0.060	6.75	30.78

PEAK FLOW REDUCTION [Qout/Oin] (%) = 13.00
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha.m.) = 0.0895

ADD HYD (9264)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2121):	56.89	5.575	6.00	34.90

+ ID2= 2 (9290): 4.40 0.060 6.75 30.78
 ID= 3 (9264): 61.29 5.583 6.00 34.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5212)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.1000	2.1402
0.2080	1.2793	5.1480	2.2661
1.1880	1.5546	5.9930	2.3900
2.2120	1.7255	6.3930	2.4000
3.0680	1.9336	0.0000	0.0000

INFLOW: ID= 2 (9264)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5212)	61.290	5.583	6.00	34.60
	61.290	0.631	7.00	34.58

PEAK FLOW REDUCTION [Oout/Oi n] (%) = 11.30
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 1.4019

CALIB STANDHYD (2122)
 ID= 1 DT=15.0 min

Area (ha) = 267.10
 Total Imp(%) = 29.00
 Dir. Conn. (%) = 24.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	77.46	189.64
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1334.42	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) over (min)	55.44	10.49
Storage Coeff. (min)	18.85 (ii)	45.21 (ii)
Unit Hyd. Tpeak (min)	15.00	60.00
Unit Hyd. peak (cms)	0.06	0.02

PEAK FLOW (cms)	6.48	2.61	*TOTALS* 7.231 (iii)
TIME TO PEAK (hrs)	6.00	6.75	6.00
RUNOFF VOLUME (mm)	40.00	10.79	17.80
TOTAL RAINFALL (mm)	42.00	42.00	42.00
RUNOFF COEFFICIENT	0.95	0.26	0.42

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9312)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	13.3600	4.9180
0.9100	2.4658	17.8700	6.1368
5.1800	2.7404	22.4400	7.2882
9.6400	3.9605	26.1200	8.4139

INFLOW: ID= 2 (2122)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (9312)	267.100	7.231	6.00	17.80
	267.100	2.291	7.50	17.80

PEAK FLOW REDUCTION [Oout/Oi n] (%) = 31.68
 TIME SHIFT OF PEAK FLOW (min) = 90.00
 MAXIMUM STORAGE USED (ha.m.) = 2.5615

ADD HYD. (8152)
 1 + 2 = 3

AREA OPEAK TPEAK R. V.

(ha) (cms) (hrs) (mm)
 ID1= 1 (5212): 61.29 0.631 7.00 34.58
 + ID2= 2 (9312): 267.10 2.291 7.50 17.80
 ID= 3 (8152): 328.39 2.884 7.50 20.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6214)
 IN= 2--> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (2141.0) ----->

Distance	Elevation	Manning
0.00	259.29	0.0500
15.33	258.91	0.0500
38.33	258.57	0.0500
65.16	257.93	0.0500
95.82	253.44	0.0500
111.15	251.21	0.0500
130.32	249.67	0.0500
149.48	248.51	0.0500
150.66	248.50	0.0500 / 0.0370
150.81	247.50	0.0370
154.66	247.50	0.0370
155.66	248.50	0.0370 / 0.0500
160.98	248.50	0.0500
187.81	249.78	0.0500
199.31	252.13	0.0500
245.30	254.04	0.0500
264.46	253.99	0.0500
298.96	255.23	0.0500
329.62	257.02	0.0500
379.45	258.82	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	248.00	.648E+04	2.1	1.00	52.30
1.00	248.50	.139E+05	6.4	1.44	36.35
1.61	249.11	.570E+05	22.6	1.24	42.01
2.21	249.71	.143E+06	63.6	1.39	37.61
2.82	250.32	.263E+06	142.9	1.70	30.69
3.43	250.93	.403E+06	257.2	2.00	26.11
4.04	251.54	.562E+06	410.4	2.29	22.82
4.64	252.14	.735E+06	601.3	2.56	20.37
5.25	252.75	.933E+06	791.0	2.66	19.67
5.86	253.36	.117E+07	1038.0	2.79	18.74
6.46	253.96	.144E+07	1344.4	2.93	17.82
7.07	254.57	.178E+07	1664.1	2.93	17.83
7.68	255.18	.217E+07	2140.2	3.10	16.87
8.28	255.78	.259E+07	2722.9	3.30	15.83
8.89	256.39	.304E+07	3387.1	3.50	14.94
9.50	257.00	.351E+07	4132.7	3.69	14.16
10.11	257.61	.402E+07	4913.0	3.83	13.64
10.71	258.21	.458E+07	5648.3	3.87	13.50
11.32	258.82	.521E+07	6343.1	3.82	13.68

<---- hydrograph ----> <-pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8152)	328.39	2.88	7.50	20.93	0.60	1.06
OUTFLOW: ID= 1 (6214)	328.39	2.00	8.25	20.93	0.48	1.00

CALIB STANDHYD (2144)
 ID= 1 DT=15.0 min

Area (ha) = 93.10
 Total Imp(%) = 75.00
 Dir. Conn. (%) = 70.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	69.82	23.27
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	787.82	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) over (min)	55.44	12.19
Storage Coeff. (min)	13.74 (ii)	38.57 (ii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.08	0.03

PEAK FLOW (cms)= 7.58 0.43 *TOTALS* 7.750 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 40.00 11.13 31.34
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.27 0.75

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9285)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.6500	2.6150
0.3200	1.6583	6.2300	2.9579
1.8100	1.9916	7.8200	3.2041
3.3600	2.2943	9.1000	3.4456

INFLOW : ID= 2 (2144)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
93.100	93.100	7.750	6.00	31.34
OUTFLOW: ID= 1 (9285)	93.100	1.243	7.00	31.33

PEAK FLOW REDUCTION [Qout/Qin] (%) = 16.04
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 1.8726

CALIB STANDHYD (2143)
 ID= 1 DT=15.0 min

Area (ha) = 158.30
 Total Imp (%) = 26.00
 Dir. Conn. (%) = 9.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	41.16	117.14
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1027.29	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	55.44	13.77
over (min)	15.00	45.00
Storage Coeff. (min)=	16.11 (ii)	39.76 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.07	0.03

PEAK FLOW (cms)= 1.55 2.40 *TOTALS* 2.894 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 40.00 12.17 14.67
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.29 0.35

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9286)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	7.9200	2.8224
0.5400	1.3792	10.5900	3.5534
3.0700	1.5175	13.3000	4.2519
5.7200	2.2534	15.4900	4.9349

INFLOW : ID= 2 (2143)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
158.300	158.300	2.894	6.50	14.67
OUTFLOW: ID= 1 (9286)	158.300	0.682	8.50	14.67

PEAK FLOW REDUCTION [Qout/Qin] (%) = 23.56
 TIME SHIFT OF PEAK FLOW (min) = 120.00
 MAXIMUM STORAGE USED (ha. m.) = 1.3887

ADD HYD (9260)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9285):	93.10	1.243	7.00	31.33
+ ID2= 2 (9286):	158.30	0.682	8.50	14.67
ID = 3 (9260):	251.40	1.666	7.00	20.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5214)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.5370	0.9877
0.1280	0.4635	3.1860	1.1293
0.5900	0.5362	3.7090	1.2678
1.3690	0.6921	3.9980	1.2778
1.8980	0.8262	0.0000	0.0000

INFLOW : ID= 2 (9260)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
251.400	251.400	1.666	7.00	20.84
OUTFLOW: ID= 1 (5214)	251.400	1.267	8.50	20.83

PEAK FLOW REDUCTION [Qout/Qin] (%) = 76.08
 TIME SHIFT OF PEAK FLOW (min) = 90.00
 MAXIMUM STORAGE USED (ha. m.) = 0.6728

CALIB STANDHYD (2142)
 ID= 1 DT=15.0 min

Area (ha) = 65.54
 Total Imp (%) = 27.00
 Dir. Conn. (%) = 10.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	17.70	47.84
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	661.01	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	55.44	5.11
over (min)	15.00	60.00
Storage Coeff. (min)=	12.37 (ii)	47.53 (ii)
Unit Hyd. Tpeak (min)=	15.00	60.00
Unit Hyd. peak (cms)=	0.08	0.02

PEAK FLOW (cms)= 0.79 0.41 *TOTALS* 0.910 (iii)
 TIME TO PEAK (hrs)= 6.00 6.75 6.00
 RUNOFF VOLUME (mm)= 40.00 6.10 9.49
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.15 0.23

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 51.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9214)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2142):	65.54	0.910	6.00	9.49
+ ID2= 2 (5214):	251.40	1.267	8.50	20.83
ID = 3 (9214):	316.94	1.473	8.00	18.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8154) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6214):	328.39	2.005	8.25	20.93
+ ID2= 2 (9214):	316.94	1.473	8.00	18.49

ID = 3 (8154):	645.33	3.462	8.25	19.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2163) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	69.70 27.00	Dir. Conn. (%)=	9.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	18.82	50.88
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	681.66	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	55.44	17.75
over (min)	15.00	45.00
Storage Coeff. (min)=	12.60 (ii)	33.95 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03

TOTALS

PEAK FLOW (cms)=	0.75	1.46	1.645 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.50
RUNOFF VOLUME (mm)=	40.00	15.12	17.36
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.36	0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 78.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9271) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	3.4900	1.2605
	0.2400	0.6230	4.6600	1.5808
	1.3500	0.6885	5.8600	1.8854
	2.5200	1.0102	6.8200	2.1832

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2163)	69.700	1.645	6.50	17.36
OUTFLOW: ID= 1 (9271)	69.700	0.655	7.50	17.35

PEAK FLOW REDUCTION [Qout/Qin] (%) = 39.79
TIME SHIFT OF PEAK FLOW (min) = 60.00
MAXIMUM STORAGE USED (ha. m.) = 0.6497

CALIB STANDHYD (2169) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	3.50 30.00	Dir. Conn. (%)=	10.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	1.05	2.45
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	152.75	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	55.44	13.47
over (min)	15.00	30.00
Storage Coeff. (min)=	5.13 (ii)	28.99 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.11	0.04

TOTALS

PEAK FLOW (cms)=	0.05	0.06	0.090 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	40.00	11.45	14.30
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.27	0.34

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9272) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	0.1800	0.0659
	0.0100	0.0333	0.2400	0.0820
	0.0700	0.0372	0.3000	0.0971
	0.1300	0.0532	0.3500	0.1119

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2169)	3.500	0.090	6.00	14.30
OUTFLOW: ID= 1 (9272)	3.500	0.010	8.75	14.06

PEAK FLOW REDUCTION [Qout/Qin] (%) = 10.92
TIME SHIFT OF PEAK FLOW (min) = 165.00
MAXIMUM STORAGE USED (ha. m.) = 0.0327

ADD HYD (9251) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9271):	69.70	0.655	7.50	17.35
+ ID2= 2 (9272):	3.50	0.010	8.75	14.06

ID = 3 (9251):	73.20	0.664	7.50	17.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5216) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	3.7400	2.4559
	0.1890	1.6831	4.6960	2.6646
	1.0830	1.7903	5.4670	2.8688
	2.0170	2.0202	5.8670	2.8788
	2.7980	2.2179	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9251)	73.200	0.664	7.50	17.19
OUTFLOW: ID= 1 (5216)	73.200	0.084	19.50	17.16

PEAK FLOW REDUCTION [Qout/Qin] (%) = 12.69
TIME SHIFT OF PEAK FLOW (min) = 720.00
MAXIMUM STORAGE USED (ha. m.) = 0.7503

CALIB STANDHYD (2162) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	72.57 20.00	Dir. Conn. (%)=	7.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	14.51	58.06

Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 695.56 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 55.44 11.07
 over (min)= 15.00 45.00
 Storage Coeff. (mi n)= 12.75 (ii) 38.55 (ii)
 Unit Hyd. Tpeak (mi n)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03

PEAK FLOW (cms)= 0.61 0.98
 TIME TO PEAK (hrs)= 6.00 6.50
 RUNOFF VOLUME (mm)= 40.00 10.50
 TOTAL RAINFALL (mm)= 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.25

TOTALS
 1.131 (iii)
 6.50
 12.56
 42.00
 0.30

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9316)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.0000	0.3000
2.0000	0.1000	4.5000	0.4000
3.0000	0.2000	5.0000	0.5000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2162)	72.570	1.131	6.50	12.56
OUTFLOW: ID= 1 (9316)	72.570	1.043	6.50	12.56

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 92.19
 TIME SHIFT OF PEAK FLOW (mi n) = 0.00
 MAXIMUM STORAGE USED (ha. m.) = 0.0561

ADD HYD (9216)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (5216):	73.20	0.084	19.50	17.16
+ ID2= 2 (9316):	72.57	1.043	6.50	12.56
ID = 3 (9216):	145.77	1.046	6.50	14.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8156)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8154):	645.33	3.462	8.25	19.73
+ ID2= 2 (9216):	145.77	1.046	6.50	14.87
ID = 3 (8156):	791.10	3.861	8.00	18.83

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6218)
 IN= 2--> OUT= 1

Routing time step (mi n)' = 15.00

<----- DATA FOR SECTION (2181.0) ----->

Di stance	Elevation	Manning
0.00	256.38	0.0450
7.70	254.85	0.0450
15.40	253.20	0.0450
19.25	252.29	0.0450
23.10	251.17	0.0450

78.94	236.16	0.0450
80.87	235.80	0.0450
82.79	235.45	0.0450
84.57	234.60	0.0450 / 0.0300
85.07	233.60	0.0300
88.57	233.60	0.0300
93.57	233.60	0.0300
94.07	234.60	0.0300
96.27	235.13	0.0300 / 0.0450
98.19	235.20	0.0450
100.12	235.32	0.0450
138.63	239.95	0.0450
180.98	244.80	0.0450
188.69	245.00	0.0450
190.61	256.00	0.0450

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
1.00	234.60	.181E+05	6.0	0.66	50.64
2.13	235.73	.518E+05	23.8	0.93	36.27
3.25	236.85	.119E+06	60.0	1.02	33.03
4.38	237.98	.217E+06	120.6	1.12	29.99
5.51	239.11	.346E+06	210.5	1.23	27.39
6.63	240.23	.506E+06	334.1	1.33	25.22
7.76	241.36	.697E+06	495.6	1.43	23.44
8.88	242.48	.920E+06	699.8	1.53	21.91
10.01	243.61	.117E+07	950.8	1.63	20.59
11.14	244.74	.146E+07	1252.5	1.73	19.45
12.26	245.86	.178E+07	1619.6	1.83	18.37
13.39	246.99	.212E+07	2078.5	1.97	17.01
14.52	248.12	.247E+07	2589.0	2.11	15.88
15.64	249.24	.282E+07	3149.7	2.25	14.94
16.77	250.37	.319E+07	3759.8	2.37	14.14
17.89	251.49	.356E+07	4419.5	2.50	13.44
19.02	252.62	.395E+07	5126.4	2.61	12.84
20.15	253.75	.435E+07	5871.6	2.72	12.34
21.27	254.87	.476E+07	6663.2	2.82	11.89

<---- hydrograph ----> <- pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8156)	791.10	3.86	8.00	18.83	0.65	0.66
OUTFLOW: ID= 1 (6218)	791.10	3.31	8.75	18.83	0.56	0.66

CALIB
 STANDHYD (2183)
 ID= 1 DT=15.0 min

Area (ha)= 69.30
 Total Imp(%)= 22.00
 Di r. Conn. (%) = 7.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	15.25	54.05
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	679.71	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 55.44 17.72
 over (mi n)= 15.00 45.00
 Storage Coeff. (mi n)= 12.58 (ii) 33.95 (ii)
 Unit Hyd. Tpeak (mi n)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03

PEAK FLOW (cms)= 0.58 1.54
 TIME TO PEAK (hrs)= 6.00 6.50
 RUNOFF VOLUME (mm)= 40.00 15.70
 TOTAL RAINFALL (mm)= 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.37

TOTALS
 1.688 (iii)
 6.50
 17.40
 42.00
 0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 80.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9282)

IN= 2----> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.5000	3.5000
0.8000	0.1000	6.0000	4.0000
1.1000	2.0000	6.5000	4.5000
2.5000	3.0000	7.0000	5.5000

INFLOW : ID= 2 (2183)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
69.300	69.300	1.688	6.50	17.40
OUTFLOW: ID= 1 (9282)	69.300	0.829	7.25	17.40

PEAK FLOW REDUCTION [Qout/Qin] (%) = 49.12
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.2873

RESERVOIR (5218)
IN= 2----> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.5090	1.7553
0.2280	0.8236	5.6620	2.0069
1.3060	0.9528	6.5910	2.2530
2.4320	1.2300	6.9910	2.2630
3.3740	1.4683	0.0000	0.0000

INFLOW : ID= 2 (9282)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
69.300	69.300	0.829	7.25	17.40
OUTFLOW: ID= 1 (5218)	69.300	0.227	10.00	17.38

PEAK FLOW REDUCTION [Qout/Qin] (%) = 27.42
TIME SHIFT OF PEAK FLOW (min) = 165.00
MAXIMUM STORAGE USED (ha. m.) = 0.8217

CALIB NASHYD (2182)
ID= 1 DT=15.0 min

Area (ha) = 82.95
Curve Number (CN) = 67.7
U. H. Tp (hrs) = 5.00
of Linear Res. (N) = 1.50

Unit Hyd Opeak (cms) = 1.786
PEAK FLOW (cms) = 0.386 (i)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 8.530
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.203

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9218)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (2182):	82.95	0.386	7.00	8.53
+ ID2= 2 (5218):	69.30	0.227	10.00	17.38
ID = 3 (9218):	152.25	0.514	8.00	12.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8158)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (6218):	791.10	3.314	8.75	18.83
+ ID2= 2 (9218):	152.25	0.514	8.00	12.56
ID = 3 (8158):	943.35	3.821	8.75	17.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8151)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (8150):	15633.75	39.826	9.50	12.03
+ ID2= 2 (8158):	943.35	3.821	8.75	17.82
ID = 3 (8151):	16577.10	43.454	9.50	12.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5504)
IN= 2----> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	33.0100	0.6700
2.6400	0.0500	37.4800	0.7800
4.8300	0.0900	47.5400	1.0100
7.4100	0.1300	60.2300	1.2800
10.3300	0.1900	71.9530	1.5258
13.5300	0.2500	*****	3.0529
16.9900	0.3200	*****	5.3299
20.6800	0.3900	*****	8.4148
24.6000	0.4800	*****	11.7637
28.7100	0.5700	*****	11.7737

INFLOW : ID= 2 (8151)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
*****	*****	43.454	9.50	12.36
OUTFLOW: ID= 1 (5504)	*****	43.463	9.50	12.36

PEAK FLOW REDUCTION [Qout/Qin] (%) = 100.02
TIME SHIFT OF PEAK FLOW (min) = 0.00
MAXIMUM STORAGE USED (ha. m.) = 0.9172

**** WARNING : HYDROGRAPH PEAK WAS NOT REDUCED.
CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.

ROUTE CHN (6220)
IN= 2----> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (2201.0) ----->

Distance	Elevation	Manning	
0.00	245.59	0.0600	
5.76	245.45	0.0600	
23.03	242.27	0.0600	
57.58	232.35	0.0600	
86.37	226.18	0.0600	
109.41	224.31	0.0600	
115.17	223.86	0.0600	
120.92	222.78	0.0600	
126.68	221.93	0.0600	
128.06	221.50	0.0600 / 0.0300	Main Channel
129.56	220.00	0.0300	Main Channel
141.56	220.00	0.0300	Main Channel
143.06	221.50	0.0300 / 0.0600	Main Channel
149.72	223.77	0.0600	
155.47	225.09	0.0600	
253.36	229.21	0.0600	
333.98	227.50	0.0600	
454.90	229.25	0.0600	
500.97	233.11	0.0600	
570.07	237.42	0.0600	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.75	220.75	.262E+05	5.5	0.57	79.44
1.50	221.50	.555E+05	17.5	0.86	52.89
2.44	222.44	.103E+06	42.7	1.14	40.03
3.37	223.37	.171E+06	79.4	1.27	35.98
4.31	224.31	.263E+06	128.1	1.33	34.22
5.25	225.25	.393E+06	191.7	1.34	34.19
6.18	226.18	.594E+06	276.7	1.28	35.77
7.12	227.12	.872E+06	399.9	1.26	36.33
8.06	228.06	.127E+07	542.7	1.17	38.92
8.99	228.99	.199E+07	770.0	1.06	43.01
9.93	229.93	.297E+07	1151.8	1.06	42.95
10.86	230.86	.400E+07	1652.9	1.13	40.33
11.80	231.80	.507E+07	2251.8	1.22	37.53
12.74	232.74	.618E+07	2946.5	1.31	34.97
13.67	233.67	.733E+07	3726.8	1.39	32.79

14.61	234.61	.853E+07	4591.0	1.48	30.96
15.55	235.55	.977E+07	5544.1	1.55	29.37
16.48	236.48	.111E+08	6586.7	1.63	27.99
17.42	237.42	.124E+08	7719.2	1.71	26.77

		<---- hydrograph ---->				<- pi pe / channel ->	
	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL	
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (5504)	*****	43.46	9.50	12.36	2.46	1.14	
OUTFLOW: ID= 1 (6220)	*****	42.69	10.00	12.36	2.42	1.14	

CALIB STANDHYD (2207)			
ID= 1 DT=15.0 min	Area (ha)=	57.80	Dir. Conn. (%)= 8.00
	Total Imp(%)=	21.00	

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	12.14	45.66
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	620.75	40.00
Mannings n	=	0.013	0.250

Max. Eff. Inten. (mm/hr)=	55.44	18.46
over (min)	15.00	45.00
Storage Coeff. (min)=	11.91 (ii)	32.94 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03

TOTALS

PEAK FLOW (cms)=	0.57	1.38	1.511 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.50
RUNOFF VOLUME (mm)=	40.00	16.61	18.48
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.40	0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 82.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9278)			
IN= 2---> OUT= 1			
DT= 15.0 min			

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.5000	2.5000
0.2000	0.3000	0.6000	3.5000
0.3000	0.9000	0.7000	4.5000
0.4000	2.0000	0.8000	5.5000

INFLOW : ID= 2 (2207)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (9278)	57.800	1.511	6.50	18.48
	57.800	0.255	8.75	18.48

PEAK FLOW REDUCTION [Qout/Qin] (%) = 16.85
 TIME SHIFT OF PEAK FLOW (min) = 135.00
 MAXIMUM STORAGE USED (ha. m.) = 0.6284

CALIB STANDHYD (2204)			
ID= 1 DT=15.0 min	Area (ha)=	13.30	Dir. Conn. (%)= 70.00
	Total Imp(%)=	75.00	

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	9.98	3.33
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	297.77	40.00
Mannings n	=	0.013	0.250

Max. Eff. Inten. (mm/hr)=	55.44	17.21
over (min)	15.00	30.00
Storage Coeff. (min)=	7.66 (ii)	29.29 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00

Unit Hyd. peak (cms)=	0.10	0.04	*TOTALS*
PEAK FLOW (cms)=	1.30	0.11	1.363 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	40.00	15.21	32.56
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.36	0.78

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9279)			
IN= 2---> OUT= 1			
DT= 15.0 min			

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.6700	0.3747
0.0500	0.2376	0.8900	0.4238
0.2600	0.2854	1.1200	0.4591
0.4800	0.3287	1.3000	0.4937

INFLOW : ID= 2 (2204)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (9279)	13.300	1.363	6.00	32.56
	13.300	0.206	6.75	32.47

PEAK FLOW REDUCTION [Qout/Qin] (%) = 15.10
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 0.2747

ADD HYD (9255)			
1 + 2 = 3			

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (9278):	57.80	0.255	8.75	18.48
+ ID2= 2 (9279):	13.30	0.206	6.75	32.47
=====				
ID = 3 (9255):	71.10	0.426	6.75	21.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5220)			
IN= 2---> OUT= 1			
DT= 15.0 min			

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.6050	1.0142
0.1320	0.4759	3.2710	1.1595
0.7550	0.5505	3.8080	1.3018
1.4050	0.7107	4.2080	1.3118
1.9490	0.8484	0.0000	0.0000

INFLOW : ID= 2 (9255)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5220)	71.100	0.426	6.75	21.10
	71.100	0.282	12.00	21.08

PEAK FLOW REDUCTION [Qout/Qin] (%) = 66.04
 TIME SHIFT OF PEAK FLOW (min) = 315.00
 MAXIMUM STORAGE USED (ha. m.) = 0.4939

CALIB NASHYD (2202)			
ID= 1 DT=15.0 min	Area (ha)=	98.00	Curve Number (CN)= 74.3
	Ia (mm)=	5.00	# of Linear Res. (N)= 1.50
	U. H. Tp (hrs)=	1.50	

Unit Hyd. Qpeak (cms)=	1.115
PEAK FLOW (cms)=	0.361 (i)
TIME TO PEAK (hrs)=	8.250
RUNOFF VOLUME (mm)=	10.903
TOTAL RAINFALL (mm)=	42.000
RUNOFF COEFFICIENT =	0.260

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2202):	98.00	0.361	8.25	10.90
+ ID2= 2 (5220):	71.10	0.282	12.00	21.08

ID = 3 (9220):	169.10	0.547	11.75	15.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6220):	16577.10	42.685	10.00	12.36
+ ID2= 2 (9220):	169.10	0.547	11.75	15.18

ID = 3 (8160):	16746.20	43.134	10.00	12.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	110.64 37.00	Di r. Conn. (%)=	20.00
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	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	40.94	69.70
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	858.84	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	55.44	16.56
over (min)	15.00	45.00
Storage Coeff. (min)=	14.47 (ii)	36.43 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.07	0.03

TOTALS

PEAK FLOW (cms)=	2.52	1.80	3.248 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	40.00	13.97	19.18
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.33	0.46

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.3 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	43.20 72.00	Di r. Conn. (%)=	66.00
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	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	31.10	12.10
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	536.66	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	55.44	17.60
over (min)	15.00	45.00
Storage Coeff. (min)=	10.91 (ii)	32.35 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03

TOTALS

PEAK FLOW (cms)=	3.61	0.35	3.750 (iii)
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TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	40.00	15.35	31.62
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.37	0.75

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 79.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9288) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	2.1600	1.1836
	0.1500	0.7428	2.8900	1.3455
	0.8400	0.8895	3.6300	1.4651
	1.5600	1.0342	4.2300	1.5823

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2224)	43.200	3.750	6.00	31.62
OUTFLOW: ID= 1 (9288)	43.200	0.647	6.75	31.59

PEAK FLOW REDUCTION [Qout/Qin] (%) = 17.24
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.8496

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2221):	110.64	3.248	6.00	19.18
+ ID2= 2 (9288):	43.20	0.647	6.75	31.59

ID = 3 (9261):	153.84	3.347	6.00	22.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5222) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	7.2210	4.0213
	0.4910	2.1348	9.0080	4.5380
	2.1830	2.4433	15.0420	4.9977
	3.9370	2.9883	15.4420	5.0077
	5.4550	3.4692	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9261)	153.840	3.347	6.00	22.66
OUTFLOW: ID= 1 (5222)	153.840	0.439	10.25	22.66

PEAK FLOW REDUCTION [Qout/Qin] (%) = 13.12
TIME SHIFT OF PEAK FLOW (min) = 255.00
MAXIMUM STORAGE USED (ha. m.) = 1.9103

CALIB STANDHYD ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	65.69 84.00	Di r. Conn. (%)=	45.00
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	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	55.18	10.51
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	661.77	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	55.44	134.59
over (min)	15.00	30.00
Storage Coeff. (min)=	12.37 (ii)	21.87 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00

Unit Hyd. peak (cms)= 0.08 0.05
 PEAK FLOW (cms)= 3.58 1.94
 TIME TO PEAK (hrs)= 6.00 6.25
 RUNOFF VOLUME (mm)= 40.00 25.38
 TOTAL RAINFALL (mm)= 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.60

TOTALS
 4.871 (iii)
 0.76

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8164) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2222):	65.69	4.871	6.00	31.96
+ ID2= 2 (5222):	153.84	0.439	10.25	22.66
=====				
ID = 3 (8164):	219.53	4.972	6.00	25.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2242) ID= 1 DT=15.0 min	Area (ha)	Imp(%)	Di r. Conn. (%)
	52.75	36.00	27.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	18.99	33.76
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	593.01	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 55.44 15.65
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 11.59 (ii) 34.05 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03

TOTALS
 2.109 (iii)
 6.00
 21.47
 42.00
 0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2244) ID= 1 DT=15.0 min	Area (ha)	Imp(%)	Di r. Conn. (%)
	87.70	70.00	64.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	61.39	26.31
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	764.63	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 55.44 17.21
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 13.50 (ii) 35.12 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03

TOTALS
 6.58
 6.867 (iii)
 6.00
 31.08

TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.36 0.74

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9287) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	4.3900	2.3776
	0.3000	1.4858	5.8700	2.7087
	1.7000	1.7770	7.3700	2.9557
	3.1700	2.0741	8.5800	3.1980

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2244)	87.700	6.867	6.00	31.06
OUTFLOW: ID= 1 (9287)	87.700	1.253	7.00	31.06

PEAK FLOW REDUCTION [Qout/Oi n](%)= 18.25
 TIME SHIFT OF PEAK FLOW (min)= 60.00
 MAXIMUM STORAGE USED (ha. m.)= 1.6899

RESERVOIR (5224) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	0.4470	0.1810
	0.0230	0.0884	0.5620	0.2047
	0.1300	0.1032	0.6540	0.2278
	0.2410	0.1297	1.0540	0.2378
	0.3350	0.1532	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9287)	87.700	1.253	7.00	31.06
OUTFLOW: ID= 1 (5224)	87.700	1.322	7.25	31.05

PEAK FLOW REDUCTION [Qout/Oi n](%)=105.49
 TIME SHIFT OF PEAK FLOW (min)= 15.00
 MAXIMUM STORAGE USED (ha. m.)= 0.2520

**** WARNING : HYDROGRAPH PEAK WAS NOT REDUCED.
 CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.

ADD HYD (9224) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2242):	52.75	2.109	6.00	21.47
+ ID2= 2 (5224):	87.70	1.322	7.25	31.05
=====				
ID = 3 (9224):	140.45	2.122	6.00	27.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8166) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8164):	219.53	4.972	6.00	25.44
+ ID2= 2 (9224):	140.45	2.122	6.00	27.45
=====				
ID = 3 (8166):	359.98	7.094	6.00	26.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6226) IN= 2---> OUT= 1	Routing time step (min)
	= 15.00

----->
 <----- DATA FOR SECTION (2261.0) ----->

Distance	Elevation	Manning	
0.00	245.35	0.0600	
8.27	242.72	0.0600	
16.54	240.01	0.0600	
31.02	235.31	0.0600	
41.36	233.14	0.0600	
90.99	227.73	0.0600	
93.06	227.50	0.0600	
95.13	227.27	0.0600	
95.83	227.00	0.0600	
100.83	226.50	0.0600 / 0.0350	Main Channel
101.33	225.50	0.0350	Main Channel
101.83	226.50	0.0350 / 0.0600	Main Channel
105.83	227.00	0.0600	
107.54	227.39	0.0600	
109.60	227.62	0.0600	
111.67	227.86	0.0600	
146.83	231.76	0.0600	
173.71	234.77	0.0600	
186.12	238.23	0.0600	
202.67	238.35	0.0600	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	226.00	.403E+03	0.0	0.16	330.59
1.00	226.50	.161E+04	0.1	0.26	208.26
1.70	227.20	.173E+05	1.5	0.27	195.72
2.39	227.89	.541E+05	5.9	0.35	152.27
3.09	228.59	.119E+06	16.0	0.43	124.08
3.79	229.29	.212E+06	33.8	0.51	104.65
4.49	229.99	.334E+06	61.2	0.59	90.97
5.18	230.68	.484E+06	99.7	0.66	80.88
5.88	231.38	.663E+06	151.0	0.73	73.13
6.58	232.08	.870E+06	216.5	0.80	66.95
7.27	232.77	.111E+07	297.5	0.87	61.91
7.97	233.47	.137E+07	398.1	0.94	57.29
8.67	234.17	.165E+07	518.9	1.01	53.12
9.36	234.86	.196E+07	658.9	1.08	49.59
10.06	235.56	.228E+07	827.0	1.17	46.02
10.76	236.26	.262E+07	1015.4	1.25	42.96
11.46	236.96	.296E+07	1221.2	1.33	40.41
12.15	237.65	.332E+07	1444.6	1.40	38.26
12.85	238.35	.368E+07	1598.2	1.40	38.42

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8166)	359.98	7.09	6.00	26.22	2.48	0.36
OUTFLOW: ID= 1 (6226)	359.98	1.77	8.00	26.22	1.74	0.28

CALIB NASHYD (2267)	Area (ha)	Curve Number (CN) = 82.0
ID= 1 DT=15.0 min	10.50	# of Linear Res. (N) = 1.50
	U. H. Tp (hrs) = 0.47	

Unit Hyd Qpeak (cms) = 0.383
PEAK FLOW (cms) = 0.130 (i)
TIME TO PEAK (hrs) = 6.500
RUNOFF VOLUME (mm) = 14.297
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.340

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9276)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2--> OUT= 1	0.0000	0.0000	0.5300	0.1297
DT= 15.0 min	0.0400	0.0402	0.7100	0.1837
	0.2000	0.0341	0.8900	0.2402
	0.3800	0.0910	1.0300	0.2954
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)

INFLOW: ID= 2 (2267)	10.500	0.130	6.50	14.30
OUTFLOW: ID= 1 (9276)	10.500	0.209	7.75	14.28

PEAK FLOW REDUCTION [Qout/Qin] (%) = 160.75
TIME SHIFT OF PEAK FLOW (min) = 75.00
MAXIMUM STORAGE USED (ha. m.) = 0.0437

**** WARNING : HYDROGRAPH PEAK WAS NOT REDUCED.
 CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.
 **** ERROR : CHECK THE STORAGE-DISCHARGE TABLE.

CALIB STANDHYD (2264)	Area (ha)	Dir. Conn. (%) = 70.00
ID= 1 DT=15.0 min	117.80	
	Total Imp (%) = 75.00	

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	88.35	29.45
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	886.19	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) =	55.44	17.21
Storage Coeff. (min) =	15.00	45.00
Unit Hyd. Tpeak (min) =	14.74 (ii)	36.37 (ii)
Unit Hyd. peak (cms) =	15.00	45.00
	0.07	0.03

TOTALS
 9.637 (iii)

PEAK FLOW (cms) =	9.32	0.79	9.637 (iii)
TIME TO PEAK (hrs) =	6.00	6.50	6.00
RUNOFF VOLUME (mm) =	40.00	15.21	32.56
TOTAL RAINFALL (mm) =	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.36	0.78

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9277)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2--> OUT= 1	0.0000	0.0000	2.5000	8.0000
DT= 15.0 min	0.3000	2.0000	3.5000	10.0000
	0.9000	5.0000	4.5000	13.0000
	2.0000	7.5000	5.5000	14.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (2264)	117.800	9.637	6.00	32.56
OUTFLOW: ID= 1 (9277)	117.800	0.483	8.75	32.55

PEAK FLOW REDUCTION [Qout/Qin] (%) = 5.01
TIME SHIFT OF PEAK FLOW (min) = 165.00
MAXIMUM STORAGE USED (ha. m.) = 2.9130

ADD HYD (9253)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (9276):	10.50	0.209	7.75	14.28
+ ID2= 2 (9277):	117.80	0.483	8.75	32.55
ID= 3 (9253):	128.30	0.678	7.75	31.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5226)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2--> OUT= 1	0.0000	0.0000	1.7530	0.6823
DT= 15.0 min	0.0890	0.3202	2.2010	0.7801

0.5080 0.3704 | 2.5620 0.8758
 0.9450 0.4782 | 2.9620 0.8858
 1.3110 0.5708 | 0.0000 0.0000

AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (9253) 128.300 0.678 7.75 31.05
 OUTFLOW: ID= 1 (5226) 128.300 0.537 9.00 31.05

PEAK FLOW REDUCTION [Qout/Qin] (%) = 79.19
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha.m.) = 0.3786

CALIB (2262) | Area (ha) = 119.28 Curve Number (CN) = 77.1
 NASHYD (2262) | Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs) = 1.05

Unit Hyd Opeak (cms) = 1.934
 PEAK FLOW (cms) = 0.638 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 12.060
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.287

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9226) | AREA OPEAK TPEAK R.V.
 1 + 2 = 3 (ha) (cms) (hrs) (mm)
 ID1= 1 (2262): 119.28 0.638 7.50 12.06
 + ID2= 2 (5226): 128.30 0.537 9.00 31.05
 ID= 3 (9226): 247.58 1.097 8.75 21.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8168) | AREA OPEAK TPEAK R.V.
 1 + 2 = 3 (ha) (cms) (hrs) (mm)
 ID1= 1 (6226): 359.98 1.773 8.00 26.22
 + ID2= 2 (9226): 247.58 1.097 8.75 21.90
 ID= 3 (8168): 607.56 2.807 8.50 24.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8162) | AREA OPEAK TPEAK R.V.
 1 + 2 = 3 (ha) (cms) (hrs) (mm)
 ID1= 1 (8160): 16746.20 43.134 10.00 12.39
 + ID2= 2 (8168): 607.56 2.807 8.50 24.46
 ID= 3 (8162): 17353.76 45.679 10.00 12.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6228) | Routing time step (min) = 15.00
 IN= 2----> OUT= 1

<----- DATA FOR SECTION (2281.0) ----->
 Distance Elevation Manning
 0.00 245.00 0.0600
 18.60 244.74 0.0600
 37.20 242.47 0.0600
 93.01 233.71 0.0600
 125.57 226.95 0.0600
 213.93 221.63 0.0600
 218.58 221.15 0.0600
 223.23 220.45 0.0600

225.03 220.55 0.0600 / 0.0300 Mai n Channel
 225.53 218.05 0.0300 Mai n Channel
 232.53 218.05 0.0300 Mai n Channel
 240.03 218.05 0.0300 Mai n Channel
 241.03 220.55 0.0300 / 0.0600 Mai n Channel
 246.48 221.00 0.0600
 251.13 221.27 0.0600
 255.78 221.22 0.0600
 325.54 225.25 0.0600
 367.40 228.91 0.0600
 404.60 231.75 0.0600
 460.41 245.00 0.0600

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
1.25	219.30	.338E+05	16.0	0.86	35.15
2.50	220.55	.696E+05	48.3	1.26	24.02
3.94	221.99	.168E+06	116.0	1.26	24.14
5.38	223.43	.392E+06	247.5	1.15	26.37
6.81	224.86	.743E+06	472.6	1.16	26.20
8.25	226.30	.122E+07	821.6	1.23	24.67
9.69	227.74	.179E+07	1320.1	1.34	22.59
11.13	229.18	.243E+07	1965.7	1.47	20.58
12.57	230.62	.313E+07	2744.2	1.59	19.02
14.01	232.06	.390E+07	3683.4	1.72	17.66
15.44	233.49	.472E+07	4824.1	1.86	16.30
16.88	234.93	.557E+07	6092.4	1.99	15.23
18.32	236.37	.646E+07	7503.3	2.11	14.35
19.76	237.81	.739E+07	9061.2	2.23	13.59
21.20	239.25	.836E+07	10767.6	2.34	12.94
22.64	240.69	.937E+07	12624.8	2.45	12.37
24.07	242.12	.104E+08	14634.9	2.56	11.87
25.51	243.56	.115E+08	16762.8	2.65	11.45
26.95	245.00	.127E+08	18699.9	2.69	11.28

<---- hydrograph ----> <-pi pe / channel ->
 AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8162) ***** 45.68 10.00 12.81 2.40 1.22
 OUTFLOW: ID= 1 (6228) ***** 45.38 10.00 12.81 2.39 1.21

CALIB STANDHYD (2287) | Area (ha) = 130.50
 ID= 1 DT=15.0 min | Total Imp (%) = 23.00 Di r. Conn. (%) = 10.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 30.01 100.48
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 932.74 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr) = 55.44 8.37
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 15.20 (ii) 44.06 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.07 0.03

TOTALS
 PEAK FLOW (cms) = 1.46 1.19 1.914 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 40.00 8.05 11.25
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.19 0.27

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 61.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9275) |
 IN= 2----> OUT= 1
 DT= 15.0 min |
 OUTFLOW STORAGE OUTFLOW STORAGE
 (cms) (ha.m.) (cms) (ha.m.)
 0.0000 0.0000 | 4.5000 3.5000

0.8000	0.1000	6.0000	4.0000
1.5000	2.0000	6.5000	4.5000
2.5000	3.0000	7.0000	5.5000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2287)	130.500	1.914	6.00	11.25
OUTFLOW: ID= 1 (9275)	130.500	0.887	7.25	11.25

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 46.33
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha. m.) = 0.3381

RESERVOIR (5228)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	6.7260	2.6184
0.3410	1.2286	8.4460	2.9937
1.9480	1.4214	9.8320	3.3609
3.6280	1.8348	10.2320	3.3709
5.0330	2.1904	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9275)	130.500	0.887	7.25	11.25
OUTFLOW: ID= 1 (5228)	130.500	0.274	10.25	11.24

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 30.94
 TIME SHIFT OF PEAK FLOW (min) = 180.00
 MAXIMUM STORAGE USED (ha. m.) = 0.9890

CALIB
 NASHYD (2282)
 ID= 1 DT=15.0 min

Area (ha) = 180.03 Curve Number (CN) = 61.9
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp (hrs) = 1.12

Unit Hyd Qpeak (cms) = 2.754
 PEAK FLOW (cms) = 0.526 (i)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 7.019
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.167

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9228)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2282):	180.03	0.526	7.75	7.02
+ ID2= 2 (5228):	130.50	0.274	10.25	11.24

ID = 3 (9228):	310.53	0.727	8.75	8.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8170)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6228):	17353.76	45.384	10.00	12.81
+ ID2= 2 (9228):	310.53	0.727	8.75	8.79

ID = 3 (8170):	17664.29	46.077	10.00	12.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (2302)
 ID= 1 DT=15.0 min

Area (ha) = 30.47
 Total Imp (%) = 29.00 Di r. Conn. (%) = 11.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)	=	8.84	21.63
Dep. Storage (mm)	=	2.00	5.00
Average Slope (%)	=	0.50	0.50
Length (m)	=	450.70	40.00
Mannings n	=	0.013	0.250

Max. Eff. Inten. (mm/hr)	=	55.44	11.76
over (min)	=	15.00	45.00
Storage Coeff. (min)	=	9.83 (ii)	35.01 (ii)
Unit Hyd. Tpeak (min)	=	15.00	45.00
Unit Hyd. peak (cms)	=	0.09	0.03

PEAK FLOW (cms)	=	0.44	0.41	*TOTALS*
TIME TO PEAK (hrs)	=	6.00	6.50	0.599 (iii)
RUNOFF VOLUME (mm)	=	40.00	10.35	6.00
TOTAL RAINFALL (mm)	=	42.00	42.00	13.61
RUNOFF COEFFICIENT	=	0.95	0.25	42.00
				0.32

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (2307)
 ID= 1 DT=15.0 min

Area (ha) = 175.00
 Total Imp (%) = 25.00 Di r. Conn. (%) = 9.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 43.75	131.25
Dep. Storage (mm)	= 2.00	5.00
Average Slope (%)	= 0.50	0.50
Length (m)	= 1080.12	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr)	=	55.44	15.02
over (min)	=	15.00	45.00
Storage Coeff. (min)	=	16.60 (ii)	39.44 (ii)
Unit Hyd. Tpeak (min)	=	15.00	45.00
Unit Hyd. peak (cms)	=	0.07	0.03

PEAK FLOW (cms)	=	1.69	2.94	*TOTALS*
TIME TO PEAK (hrs)	=	6.00	6.50	3.496 (iii)
RUNOFF VOLUME (mm)	=	40.00	13.33	6.50
TOTAL RAINFALL (mm)	=	42.00	42.00	15.73
RUNOFF COEFFICIENT	=	0.95	0.32	42.00
				0.37

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9280)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	8.7500	3.0604
0.5900	1.4722	11.7100	3.8736
3.3900	1.6096	14.7000	4.6556
6.3200	2.4308	17.1100	5.4201

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2307)	175.000	3.496	6.50	15.73
OUTFLOW: ID= 1 (9280)	175.000	1.300	7.75	15.72

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 37.18
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha. m.) = 1.5115

RESERVOIR (5230)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	8.6830	2.7782
0.4700	1.3193	10.7480	3.1570
2.5870	1.5374	12.3570	3.5304
5.1590	1.9534	12.7570	3.5404
6.7830	2.3333	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9280)	175.000	1.300	7.75	15.72
OUTFLOW: ID= 1 (5230)	175.000	0.369	14.75	15.72

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 28.43
TIME SHIFT OF PEAK FLOW (min) = 420.00
MAXIMUM STORAGE USED (ha. m.) = 1.0372

ADD HYD (8172)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2302):	30.47	0.599	6.00	13.61
+ ID2= 2 (5230):	175.00	0.369	14.75	15.72

ID = 3 (8172):	205.47	0.610	6.00	15.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (2329)
ID= 1 DT=15.0 mi n

Area (ha) = 1.80 Curve Number (CN) = 69.0
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp (hrs) = 0.22

Unit Hyd Opeak (cms) = 0.143

PEAK FLOW (cms) = 0.022 (i)
TIME TO PEAK (hrs) = 6.000
RUNOFF VOLUME (mm) = 8.189
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.195

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9269)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.0900	0.0300
0.0100	0.0136	0.1200	0.0386
0.0400	0.0146	0.1500	0.0471
0.0700	0.0234	0.1800	0.0554

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2329)	1.800	0.022	6.00	8.19
OUTFLOW: ID= 1 (9269)	1.800	0.005	8.00	8.01

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 24.20
TIME SHIFT OF PEAK FLOW (min) = 120.00
MAXIMUM STORAGE USED (ha. m.) = 0.0074

CALIB STANDHYD (2327)
ID= 1 DT=15.0 mi n

Area (ha) = 184.00
Total Imp (%) = 24.00 Di r. Conn. (%) = 8.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 44.16 139.84
Dep. Storage (mm) = 2.00 5.00
Average Slope (%) = 0.50 0.50
Length (m) = 1107.55 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 55.44 10.73
over (min) = 15.00 45.00

Storage Coeff. (min) = 16.86 (ii) 42.98 (ii)
Unit Hyd. Tpeak (min) = 15.00 45.00
Unit Hyd. peak (cms) = 0.07 0.03

PEAK FLOW (cms) = 1.57 2.15 *TOTALS* (iii) 2.668
TIME TO PEAK (hrs) = 6.00 6.50 6.50
RUNOFF VOLUME (mm) = 40.00 9.83 12.24
TOTAL RAINFALL (mm) = 42.00 42.00 42.00
RUNOFF COEFFICIENT = 0.95 0.23 0.29

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 66.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9270)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.3000	3.5000
1.1000	0.2000	3.5000	4.5000
1.8000	1.5000	4.5000	6.0000
2.0000	2.0000	5.5000	8.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2327)	184.000	2.668	6.50	12.24
OUTFLOW: ID= 1 (9270)	184.000	1.309	7.25	12.24

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 49.08
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.5935

ADD HYD (9249)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9269):	1.80	0.005	8.00	8.01
+ ID2= 2 (9270):	184.00	1.309	7.25	12.24

ID = 3 (9249):	185.80	1.314	7.25	12.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5232)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	11.1130	4.3262
0.5630	2.0299	13.9540	4.9462
3.2190	2.3484	16.2450	5.5530
5.9950	3.0315	16.6450	5.5630
8.3150	3.6189	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9249)	185.800	1.314	7.25	12.20
OUTFLOW: ID= 1 (5232)	185.800	0.422	10.75	12.19

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 32.07
TIME SHIFT OF PEAK FLOW (min) = 210.00
MAXIMUM STORAGE USED (ha. m.) = 1.5202

CALIB NASHYD (2322)
ID= 1 DT=15.0 mi n

Area (ha) = 129.00 Curve Number (CN) = 58.1
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp (hrs) = 1.00

Unit Hyd Opeak (cms) = 2.209

PEAK FLOW (cms) = 0.358 (i)
TIME TO PEAK (hrs) = 7.500

RUNOFF VOLUME (mm) = 6.154
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.147

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2322):	129.00	0.358	7.50	6.15
+ ID2= 2 (5232):	185.80	0.422	10.75	12.19
ID = 3 (9232):	314.80	0.679	9.75	9.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8172):	205.47	0.610	6.00	15.41
+ ID2= 2 (9232):	314.80	0.679	9.75	9.72
ID = 3 (8173):	520.27	1.022	10.00	11.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8170):	17664.29	46.077	10.00	12.74
+ ID2= 2 (8173):	520.27	1.022	10.00	11.97
ID = 3 (8174):	18184.56	47.099	10.00	12.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6234)
 IN= 2--> OUT= 1
 Routing time step (min) = 15.00

<----- DATA FOR SECTION (2341.0) ----->

Distance	Elevation	Manning	
0.00	228.00	0.0550	
42.86	223.21	0.0550	
85.72	219.56	0.0550	
107.15	219.42	0.0550	
128.58	219.39	0.0550	
214.30	219.30	0.0550	
225.02	219.26	0.0550	
235.73	219.24	0.0550	
255.16	219.20	0.0550 / 0.0350	Main Channel
257.16	217.20	0.0350	Main Channel
303.16	217.20	0.0350	Main Channel
305.16	219.20	0.0350 / 0.0550	Main Channel
310.74	219.28	0.0550	
321.45	219.27	0.0550	
332.17	219.26	0.0550	
396.46	219.29	0.0550	
782.20	222.91	0.0550	
878.64	227.43	0.0550	
964.36	227.80	0.0550	
1060.79	228.00	0.0550	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.50	217.70	.887E+05	7.2	0.31	206.54
1.00	218.20	.179E+06	22.7	0.48	131.74
1.50	218.70	.272E+06	44.5	0.62	101.75
2.00	219.20	.365E+06	71.8	0.75	84.97
2.59	219.79	.993E+06	136.1	0.52	121.59
3.17	220.37	.189E+07	258.6	0.52	121.80
3.76	220.96	.294E+07	434.7	0.56	112.82
4.35	221.55	.415E+07	665.9	0.61	103.88

4.93	222.13	.551E+07	955.0	0.66	96.22
5.52	222.72	.703E+07	1305.1	0.71	89.80
6.11	223.31	.868E+07	1744.9	0.77	82.91
6.69	223.89	.104E+08	2261.4	0.83	76.46
7.28	224.48	.121E+08	2838.0	0.89	71.12
7.87	225.07	.139E+08	3472.9	0.95	66.63
8.45	225.65	.157E+08	4165.0	1.01	62.82
9.04	226.24	.176E+08	4913.5	1.07	59.54
9.63	226.83	.194E+08	5717.7	1.12	56.68
10.21	227.41	.214E+08	6577.1	1.17	54.18
10.80	228.00	.235E+08	6919.5	1.12	56.61

INFLOW : ID= 2 (8174)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW: ID= 1 (6234)	266.10	47.10	10.00	12.72	1.55	0.63
	266.10	41.69	11.50	12.72	1.44	0.60

CALIB NASHYD (2347)
 ID= 1 DT=15.0 mi n
 Area (ha) = 266.10
 I a (mm) = 5.00
 U. H. Tp (hrs) = 2.00
 Curve Number (CN) = 56.0
 # of Li near Res. (N) = 1.50

Unit Hyd Opeak (cms) = 2.276
 PEAK FLOW (cms) = 0.411 (i)
 TIME TO PEAK (hrs) = 9.500
 RUNOFF VOLUME (mm) = 5.766
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.137

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9281) IN= 2--> OUT= 1 DT= 15.0 mi n	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	13.3100	4.0635
	0.9000	1.7152	17.8000	5.3537
	5.1600	1.7695	22.3500	6.6446
	9.6100	3.0976	26.0300	7.9060

INFLOW : ID= 2 (2347)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (9281)	266.100	0.411	9.50	5.77
	266.100	0.276	14.00	5.76

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 67.20
 TIME SHIF T OF PEAK FLOW (mi n) = 270.00
 MAXIMUM STORAGE USED (ha. m.) = 0.5265

RESERVOIR (5234) IN= 2--> OUT= 1 DT= 15.0 mi n	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	0.5630	0.1745
	0.1300	0.0819	0.6550	0.1995
	0.2420	0.0947	0.7760	0.2240
	0.3350	0.1223	1.0550	0.2340
	0.4480	0.1460	0.0000	0.0000

INFLOW : ID= 2 (9281)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5234)	266.100	0.276	14.00	5.76
	266.100	0.273	14.75	5.76

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 98.99
 TIME SHIF T OF PEAK FLOW (mi n) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 0.1041

CALIB STANDHYD (2342)
 ID= 1 DT=15.0 mi n
 Area (ha) = 1.06
 Total Imp (%) = 22.00
 Di r. Conn. (%) = 8.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 0.23 0.83
 Dep. Storage (mm)= 1.00 5.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 84.06 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 55.44 3.94
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 2.91 (ii) 28.65 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.11 0.04

TOTALS
 PEAK FLOW (cms)= 0.01 0.01 0.017 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 41.00 3.86 6.82
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.98 0.09 0.16

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 39.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD. (9234)					
1 + 2 = 3					
	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (2342):	1.06	0.017	6.00	6.82	
+ ID2= 2 (5234):	266.10	0.273	14.75	5.76	
=====					
ID = 3 (9234):	267.16	0.273	14.75	5.77	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD. (8176)					
1 + 2 = 3					
	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (6234):	18184.56	41.686	11.50	12.72	
+ ID2= 2 (9234):	267.16	0.273	14.75	5.77	
=====					
ID = 3 (8176):	18451.72	41.922	11.50	12.62	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2369)				
ID= 1 DT=15.0 min				
Area	(ha)=	PERVIOUS (i)	Di r. Conn. (%)=	
Total	Imp(%)=			
441.90	23.00	8.00		

Surface Area (ha)= 101.64 340.26
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1716.39 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 37.80 11.65
 over (min)= 30.00 60.00
 Storage Coeff. (min)= 25.55 (ii) 50.83 (ii)
 Unit Hyd. Tpeak (min)= 30.00 60.00
 Unit Hyd. peak (cms)= 0.04 0.02

TOTALS
 PEAK FLOW (cms)= 2.62 4.87 6.116 (iii)
 TIME TO PEAK (hrs)= 6.25 6.75 6.75
 RUNOFF VOLUME (mm)= 40.00 10.73 13.07
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.26 0.31

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.0 Ia = Dep. Storage (Above)

- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9266)				
IN= 2---> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha. m.)	(cms)	(ha. m.)
	0.0000	0.0000	2.3000	8.0000
	0.2000	2.0000	3.5000	10.0000
	1.9000	7.0000	4.5000	13.0000
	2.0000	7.5000	5.5000	14.0000

INFLOW : ID= 2 (2369) 441.900 6.116 6.75
 OUTFLOW: ID= 1 (9266) 441.900 0.912 10.50 13.07

PEAK FLOW REDUCTION [Qout/Qin] (%) = 14.91
 TIME SHIFT OF PEAK FLOW (min) = 225.00
 MAXIMUM STORAGE USED (ha. m.) = 4.0951

RESERVOIR (5236)				
IN= 2---> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha. m.)	(cms)	(ha. m.)
	0.0000	0.0000	3.1380	1.5897
	1.0220	0.7542	3.7950	2.0204
	1.7530	0.8200	4.4970	2.4373
	2.3290	1.2567	0.0000	0.0000

INFLOW : ID= 2 (9266) 441.900 0.912 10.50 13.06
 OUTFLOW: ID= 1 (5236) 441.900 0.822 13.25 13.06

PEAK FLOW REDUCTION [Qout/Qin] (%) = 90.14
 TIME SHIFT OF PEAK FLOW (min) = 165.00
 MAXIMUM STORAGE USED (ha. m.) = 0.6070

CALIB NASHYD (2362)			
ID= 1 DT=15.0 min			
Area	(ha)=	Curve Number	(CN)=
Ia	(mm)=	# of Linear Res.	(N)=
U. H. Tp	(hrs)=		
52.59	5.00	58.7	1.50
1.24			

Unit Hyd Opeak (cms)= 0.726

PEAK FLOW (cms)= 0.128 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 6.299
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.150

- (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD. (9236)					
1 + 2 = 3					
	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (2362):	52.59	0.128	8.00	6.30	
+ ID2= 2 (5236):	441.90	0.822	13.25	13.06	
=====					
ID = 3 (9236):	494.49	0.886	12.75	12.34	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6238)		
IN= 2---> OUT= 1		
Routing time step (min) = 15.00		

<---- DATA FOR SECTION (2381.0) ---->		
Distance	Elevation	Manning
0.00	241.54	0.0380
602.55	227.00	0.0380
1702.00	224.50	0.0380

1721.25	223.00	0.0380
1725.10	222.60	0.0380
1780.94	222.50	0.0380
1782.87	222.45	0.0380
1784.79	222.40	0.0380 / 0.0300
1786.57	221.75	0.0300
1787.07	220.75	0.0300
1787.57	220.75	0.0300
1791.57	220.75	0.0300
1794.07	221.75	0.0300
1798.27	222.00	0.0300
1800.19	222.35	0.0300 / 0.0380
1802.12	222.47	0.0380
1840.63	223.00	0.0380
1900.00	225.00	0.0380
1950.00	226.00	0.0380
2242.61	240.00	0.0380

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.80	221.55	.101E+05	5.2	1.14	32.36
1.60	222.35	.296E+05	18.7	1.40	26.36
2.64	223.39	.249E+06	151.8	1.35	27.33
3.68	224.43	.613E+06	512.2	1.85	19.94
4.71	225.46	.152E+07	1066.8	1.55	23.80
5.75	226.50	.359E+07	2824.9	1.75	21.16
6.79	227.54	.663E+07	6528.0	2.18	16.93
7.83	228.58	.994E+07	12158.6	2.71	13.62
8.87	229.62	.134E+08	19268.8	3.19	11.58
9.91	230.66	.170E+08	27785.3	3.63	10.19
10.94	231.69	.207E+08	37667.1	4.03	9.18
11.98	232.73	.246E+08	48891.7	4.40	8.40
13.02	233.77	.287E+08	61448.0	4.75	7.78
14.06	234.81	.329E+08	75332.7	5.08	7.28
15.10	235.85	.372E+08	90547.8	5.39	6.85
16.14	236.89	.417E+08	*****	5.69	6.49
17.17	237.92	.464E+08	*****	5.98	6.18
18.21	238.96	.512E+08	*****	6.25	5.91
19.25	240.00	.561E+08	*****	6.52	5.67

		<----- hydrograph ----->				<----- pi pe / channel ----->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (9236)	494.49	0.89	12.75	12.34	0.14	1.14	
OUTFLOW: ID= 1 (6238)	494.49	0.88	13.25	12.34	0.13	1.14	

CALIB STANDHYD (2389)	Area (ha)= 4.70	Imp(%)= 21.00	Di r. Conn.(%)= 7.00
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	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	0.99	3.71
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	177.01	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	55.44	11.31
over (mi n)	15.00	45.00
Storage Coeff. (mi n)=	5.61 (ii)	31.19 (ii)
Unit Hyd. Tpeak (mi n)=	15.00	45.00
Unit Hyd. peak (cms)=	0.11	0.03

PEAK FLOW (cms)=	0.05	0.07	0.078 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.50
RUNOFF VOLUME (mm)=	40.00	10.58	12.64
TOTAL RAI NFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFI CI ENT =	0.95	0.25	0.30

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 69.0 Ia = Dep Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9267)
 IN= 2----> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.2300	0.0785
0.0200	0.0364	0.3100	0.1005
0.0900	0.0393	0.3900	0.1220
0.1700	0.0616	0.4600	0.1429

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2389)	4.700	0.078	6.50	12.64
OUTFLOW: ID= 1 (9267)	4.700	0.018	8.25	12.55

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 22.92
 TIME SHI FT OF PEAK FLOW (mi n)=105.00
 MAXI MUM STORAGE USED (ha. m.) = 0.0328

CALIB NASHYD (2387)
 ID= 1 DT=15.0 mi n

Area (ha)= 90.20	Curve Number (CN)= 54.0
Ia (mm)= 5.00	# of Li near Res. (N)= 1.50
U. H. Tp(hrs)= 0.86	

Unit Hyd Opeak (cms)= 1.791

PEAK FLOW (cms)= 0.242 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 5.334
TOTAL RAI NFALL (mm)= 42.000
RUNOFF COEFFI CI ENT = 0.127

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9268)
 IN= 2----> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.5100	1.3564
0.3100	0.5624	6.0400	1.7959
1.7500	0.5748	7.5800	2.2374
3.2600	1.0285	8.8300	2.6688

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2387)	90.200	0.242	7.25	5.33
OUTFLOW: ID= 1 (9268)	90.200	0.119	11.25	5.33

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 49.17
 TIME SHI FT OF PEAK FLOW (mi n)=240.00
 MAXI MUM STORAGE USED (ha. m.) = 0.2160

ADD HYD (9247)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9267):	4.70	0.018	8.25	12.55
+ ID2= 2 (9268):	90.20	0.119	11.25	5.33
ID = 3 (9247):	94.90	0.134	11.00	5.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5238)
 IN= 2----> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.8780	1.5095
0.1960	0.7083	4.8600	1.7258
1.1230	0.8194	5.6680	1.9375
2.0920	1.0578	6.0680	1.9475
2.9010	1.2627	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9247)	94.900	0.134	11.00	5.69

OUTFLOW: ID= 1 (5238) 94.900 0.069 17.25 5.68

PEAK FLOW REDUCTION [Qout/Qin] (%) = 51.41
 TIME SHIFT OF PEAK FLOW (min) = 375.00
 MAXIMUM STORAGE USED (ha.m.) = 0.2484

CALIB NASHYD (2382) Area (ha) = 216.80 Curve Number (CN) = 56.8
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.56

Unit Hyd Qpeak (cms) = 2.368

PEAK FLOW (cms) = 0.413 (i)
 TIME TO PEAK (hrs) = 8.500
 RUNOFF VOLUME (mm) = 5.916
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.141

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9238) 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (2382): 216.80 0.413 8.50 5.92
 + ID2= 2 (5238): 94.90 0.069 17.25 5.68
 ID = 3 (9238): 311.70 0.429 8.75 5.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8180) 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (6238): 494.49 0.880 13.25 12.34
 + ID2= 2 (9238): 311.70 0.429 8.75 5.84
 ID = 3 (8180): 806.19 1.217 12.25 9.83

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8178) 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (8176): 18451.72 41.922 11.50 12.62
 + ID2= 2 (8180): 806.19 1.217 12.25 9.83
 ID = 3 (8178): 19257.91 43.123 11.50 12.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6240) IN= 2--> OUT= 1 Routing time step (min) = 15.00

<----- DATA FOR SECTION (2401.0) ----->

Distance	Elevation	Manning	
0.00	222.00	0.0450	
11.46	221.00	0.0450	
208.98	221.00	0.0450	
404.04	220.97	0.0450	
808.08	220.83	0.0450	
905.60	220.17	0.0450	
919.53	219.43	0.0450	
933.47	219.22	0.0450	
945.26	219.21	0.0450 / 0.0300	Main Channel
946.26	217.81	0.0300	Main Channel
975.26	217.81	0.0300	Main Channel
1003.26	217.81	0.0300	Main Channel
1005.26	219.21	0.0300 / 0.0450	Main Channel
1017.06	219.28	0.0450	
1030.99	219.26	0.0450	

1044.92 219.23 0.0450
 1058.86 219.23 0.0450
 1253.91 219.22 0.0450
 1323.57 221.05 0.0450
 1379.30 222.00 0.0450

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.20	218.01	307E+05	2.2	0.20	227.52
0.40	218.21	617E+05	7.1	0.31	143.95
0.60	218.41	928E+05	14.0	0.41	110.33
0.80	218.61	124E+06	22.6	0.49	91.46
1.00	218.81	156E+06	32.8	0.57	79.15
1.20	219.01	188E+06	44.5	0.64	70.38
1.40	219.21	220E+06	57.5	0.70	63.77
1.63	219.44	413E+06	82.5	0.54	83.52
1.87	219.68	632E+06	120.9	0.51	87.03
2.10	219.91	858E+06	170.0	0.53	84.11
2.33	220.14	109E+07	228.7	0.56	79.62
2.56	220.37	134E+07	294.4	0.59	76.03
2.80	220.61	162E+07	371.1	0.61	72.75
3.03	220.84	192E+07	456.8	0.64	70.18
3.26	221.07	251E+07	552.1	0.59	75.74
3.49	221.30	333E+07	711.8	0.57	78.07
3.73	221.54	417E+07	907.0	0.58	76.62
3.96	221.77	502E+07	1134.1	0.61	73.70
4.19	222.00	587E+07	1390.9	0.64	70.34

<---- hydrograph ----> <- pipe / channel ->
 AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW: ID= 2 (8178) ***** 43.12 11.50 12.50 1.18 0.63
 OUTFLOW: ID= 1 (6240) ***** 40.62 12.50 12.50 1.13 0.61

CALIB NASHYD (0240) Area (ha) = 345.47 Curve Number (CN) = 46.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 3.60

Unit Hyd Qpeak (cms) = 1.637
 PEAK FLOW (cms) = 0.242 (i)
 TIME TO PEAK (hrs) = 12.000
 RUNOFF VOLUME (mm) = 4.078
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.097

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2407) Area (ha) = 88.90
 ID= 1 DT=15.0 min Total Imp(%) = 20.00 Dir. Conn. (%) = 7.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 17.78 71.12
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 769.85 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr) = 55.44 9.54
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 13.55 (ii) 40.93 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.08 0.03

TOTALS
 PEAK FLOW (cms) = 0.73 1.00 1.195 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.50
 RUNOFF VOLUME (mm) = 40.00 9.15 11.31
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.22 0.27

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.0 Ia = Dep. Storage (Above)

- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9273)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.4500	5.4723
0.3000	4.6746	5.9500	5.8930
1.7300	4.7225	7.4700	6.3047
3.2100	5.1511	8.7000	6.7071

INFLOW : ID= 2 (2407)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
88.900	88.900	1.195	6.50	11.31
OUTFLOW: ID= 1 (9273)	88.900	0.056	12.50	11.26

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 4.73
TIME SHIFT OF PEAK FLOW (mi n) = 360.00
MAXIMUM STORAGE USED (ha. m.) = 0.8806

ADD HYD (9252)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0240):	345.47	0.242	12.00	4.08
+ ID2= 2 (9273):	88.90	0.056	12.50	11.26
=====				
ID = 3 (9252):	434.37	0.298	12.00	5.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8182)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6240):	19257.91	40.621	12.50	12.50
+ ID2= 2 (9252):	434.37	0.298	12.00	5.55
=====				
ID = 3 (8182):	19692.28	40.918	12.50	12.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (2427)
ID= 1 DT=15.0 mi n

Area (ha) = 5.40 Curve Number (CN) = 74.0
Ia (mm) = 5.00 # of Li near Res. (N) = 1.50
U. H. Tp (hrs) = 0.37

Unit Hyd Opeak (cms) = 0.249

PEAK FLOW (cms) = 0.056 (i)
TIME TO PEAK (hrs) = 6.500
RUNOFF VOLUME (mm) = 10.366
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.247

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9274)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.2700	0.0857
0.0200	0.0379	0.3600	0.1115
0.1000	0.0399	0.4500	0.1369
0.1900	0.0663	0.5300	0.1618

INFLOW : ID= 2 (2427)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
5.400	5.400	0.056	6.50	10.37
OUTFLOW: ID= 1 (9274)	5.400	0.016	9.00	10.28

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 28.21

TIME SHIFT OF PEAK FLOW (mi n) = 150.00
MAXIMUM STORAGE USED (ha. m.) = 0.0298

RESERVOIR (5242)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.0480	1.0185
0.0020	1.0087	0.0600	1.0212
0.0140	1.0101	0.0700	1.0238
0.0260	1.0130	0.4700	1.0338
0.0360	1.0155	0.0000	0.0000

INFLOW : ID= 2 (9274)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
5.400	5.400	0.016	9.00	10.28
OUTFLOW: ID= 1 (5242)	5.400	0.000	33.75	0.15

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 0.69
TIME SHIFT OF PEAK FLOW (mi n) = *****
MAXIMUM STORAGE USED (ha. m.) = 0.0547

CALIB NASHYD (2422)
ID= 1 DT=15.0 mi n

Area (ha) = 652.48 Curve Number (CN) = 51.6
Ia (mm) = 5.00 # of Li near Res. (N) = 1.50
U. H. Tp (hrs) = 5.37

Unit Hyd Opeak (cms) = 2.073

PEAK FLOW (cms) = 0.393 (i)
TIME TO PEAK (hrs) = 13.000
RUNOFF VOLUME (mm) = 4.970
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.118

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9242)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2422):	652.48	0.393	13.00	4.97
+ ID2= 2 (5242):	5.40	0.000	33.75	0.15
=====				
ID = 3 (9242):	657.88	0.393	13.00	4.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8184)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8182):	19692.28	40.918	12.50	12.35
+ ID2= 2 (9242):	657.88	0.393	13.00	4.93
=====				
ID = 3 (8184):	20350.16	41.310	12.50	12.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6244)
IN= 2--> OUT= 1

Routing time step (mi n)' = 15.00

Distance	Elevation	Manning
0.00	225.00	0.0450
2.50	220.10	0.0450
3.00	220.06	0.0450
3.64	220.07	0.0450
7.28	220.08	0.0450
10.91	220.09	0.0450
14.55	219.81	0.0450
18.19	219.29	0.0450
24.13	219.21	0.0450 / 0.0300
24.63	217.81	0.0300

Main Channel
Main Channel

69.13	217.81	0.0300	Main Channel
114.13	217.81	0.0300	Main Channel
115.13	219.21	0.0300 / 0.0450	Main Channel
120.06	219.25	0.0450	
123.70	219.32	0.0450	
127.34	219.77	0.0450	
130.98	219.88	0.0450	
134.61	219.99	0.0450	
350.00	220.18	0.0450	
360.18	225.00	0.0450	

TRAVEL TIME TABLE					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.35	218.16	.108E+06	9.0	0.29	200.52
0.70	218.51	.216E+06	28.4	0.45	126.89
1.05	218.86	.325E+06	55.7	0.59	97.27
1.40	219.21	.434E+06	89.7	0.71	80.65
1.79	219.60	.574E+06	135.9	0.81	70.36
2.17	219.98	.726E+06	190.8	0.90	63.38
2.56	220.37	.111E+07	264.3	0.82	69.82
2.94	220.75	.157E+07	371.7	0.81	70.36
3.33	221.14	.203E+07	503.4	0.85	67.30
3.72	221.53	.250E+07	656.6	0.90	63.40
4.10	221.91	.296E+07	829.6	0.96	59.54
4.49	222.30	.343E+07	1021.1	1.02	56.00
4.87	222.68	.390E+07	1230.2	1.08	52.83
5.26	223.07	.437E+07	1456.2	1.14	50.01
5.65	223.46	.484E+07	1698.4	1.20	47.51
6.03	223.84	.531E+07	1956.3	1.26	45.28
6.42	224.23	.579E+07	2229.3	1.32	43.28
6.80	224.61	.626E+07	2517.0	1.38	41.48
7.19	225.00	.674E+07	2819.2	1.44	39.85

hydrograph				pi pe / channel ->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8184)	41.31	12.50	12.11	0.87	0.51
OUTFLOW: ID= 1 (6244)	37.13	14.50	12.11	0.81	0.49

CALIB STANDHYD (2441)	Area (ha)= 91.52	Dir. Conn. (%)= 15.00
ID= 1 DT=15.0 mi n	Total Imp (%)= 45.00	

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)=	41.18	50.34	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	781.11	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	55.44	14.62	
over (mi n)	15.00	45.00	
Storage Coeff. (mi n)=	13.67 (ii)	36.75 (ii)	
Uni t Hyd. Tpeak (mi n)=	15.00	45.00	
Uni t Hyd. peak (cms)=	0.08	0.03	

TOTALS	1.60	1.15	2.062 (iii)
PEAK FLOW (cms)=	6.00	6.50	
TIME TO PEAK (hrs)=	40.00	10.41	
RUNOFF VOLUME (mm)=	42.00	42.00	
TOTAL RAINFALL (mm)=	0.95	0.25	0.35
RUNOFF COEFFICIENT =			

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 60.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2449)	Area (ha)= 143.20	Dir. Conn. (%)= 17.00
ID= 1 DT=15.0 mi n	Total Imp (%)= 34.00	

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)=	48.69	94.51	

Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	977.07	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	55.44	12.89
over (mi n)	15.00	45.00
Storage Coeff. (mi n)=	15.63 (ii)	39.91 (ii)
Uni t Hyd. Tpeak (mi n)=	15.00	45.00
Uni t Hyd. peak (cms)=	0.07	0.03

TOTALS	2.68	1.81	3.402 (iii)
PEAK FLOW (cms)=	6.00	6.50	
TIME TO PEAK (hrs)=	40.00	11.23	16.12
RUNOFF VOLUME (mm)=	42.00	42.00	42.00
TOTAL RAINFALL (mm)=	0.95	0.27	0.38
RUNOFF COEFFICIENT =			

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9265)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2---> OUT= 1	0.0000	0.0000	7.1600	2.7857
DT= 15.0 mi n	0.4900	1.4537	9.5800	3.4259
	2.7800	1.6401	12.0300	4.0179
	5.1700	2.2743	14.0100	4.5968

		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2449)	143.200	3.402	6.00	16.12	
OUTFLOW: ID= 1 (9265)	143.200	0.495	8.75	16.11	

PEAK FLOW REDUCTION [Qout/Qi n] (%)= 14.56
TIME SHIF T OF PEAK FLOW (mi n)=165.00
MAXIMUM STORAGE USED (ha. m.)= 1.4548

ADD HYD (9245)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (2441):	91.52	2.062	6.00	14.85
+ ID2= 2 (9265):	143.20	0.495	8.75	16.11
ID= 3 (9245):	234.72	2.204	6.00	15.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5244)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2---> OUT= 1	0.0000	0.0000	15.7000	11.1120
DT= 15.0 mi n	0.7950	7.8678	19.7140	11.9879
	4.5470	8.3178	22.9500	12.8451
	8.4690	9.2829	23.3500	12.8551
	11.7470	10.1128	0.0000	0.0000

		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9245)	234.720	2.204	6.00	15.62	
OUTFLOW: ID= 1 (5244)	234.720	0.226	17.75	15.61	

PEAK FLOW REDUCTION [Qout/Qi n] (%)= 10.25
TIME SHIF T OF PEAK FLOW (mi n)=705.00
MAXIMUM STORAGE USED (ha. m.)= 2.2365

CALIB NASHYD (2442)	Area (ha)=1674.00	Curve Number (CN)= 62.1
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YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 60.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5446)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	10.1800	6.2621
0.8000	2.9957	12.5050	7.1218
3.0350	3.4813	14.4090	7.9633
5.6510	4.4353	14.8090	7.9733
7.7470	5.2683	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (4461)	236.120	5.236	6.00	15.54
OUTFLOW: ID= 1 (5446)	236.120	0.654	9.25	15.54

PEAK FLOW REDUCTION [Qout/Qin] (%) = 12.48
TIME SHIFT OF PEAK FLOW (min) = 195.00
MAXIMUM STORAGE USED (ha.m.) = 2.4486

ADD HYD (8192)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4462):	1238.89	1.329	12.00	6.83
+ ID2= 2 (5446):	236.12	0.654	9.25	15.54
-----	-----	-----	-----	-----
ID = 3 (8192):	1475.01	1.935	11.75	8.23

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8194)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8188):	42609.04	76.482	14.50	11.93
+ ID2= 2 (8192):	1475.01	1.935	11.75	8.23
-----	-----	-----	-----	-----
ID = 3 (8194):	44084.05	78.205	14.25	11.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0442)
ID= 1 DT=15.0 min

Area (ha) = 117.26 Curve Number (CN) = 43.0
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 1.17

Unit Hyd Opeak (cms) = 1.705
PEAK FLOW (cms) = 0.170 (i)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 3.634
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.087

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0440)
ID= 1 DT=15.0 min

Area (ha) = 226.35 Curve Number (CN) = 60.0
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 1.11

Unit Hyd Opeak (cms) = 3.481
PEAK FLOW (cms) = 0.621 (i)
TIME TO PEAK (hrs) = 7.750
RUNOFF VOLUME (mm) = 6.577

TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.157

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0438)
ID= 1 DT=15.0 min

Area (ha) = 130.70 Curve Number (CN) = 51.0
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 0.60

Unit Hyd Opeak (cms) = 3.738
PEAK FLOW (cms) = 0.416 (i)
TIME TO PEAK (hrs) = 6.750
RUNOFF VOLUME (mm) = 4.765
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.113

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0436)
ID= 1 DT=15.0 min

Area (ha) = 187.51 Curve Number (CN) = 55.0
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 0.73

Unit Hyd Opeak (cms) = 4.391
PEAK FLOW (cms) = 0.592 (i)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 5.501
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.131

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0434)
ID= 1 DT=15.0 min

Area (ha) = 56.64 Curve Number (CN) = 46.0
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 0.52

Unit Hyd Opeak (cms) = 1.867
PEAK FLOW (cms) = 0.167 (i)
TIME TO PEAK (hrs) = 6.500
RUNOFF VOLUME (mm) = 3.974
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.095

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8206)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0434):	56.64	0.167	6.50	3.97
+ ID2= 2 (0436):	187.51	0.592	7.00	5.50
-----	-----	-----	-----	-----
ID = 3 (8206):	244.15	0.751	7.00	5.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6438)
IN= 2--> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (4381.0) ----->

Distance	Elevation	Manning
0.00	256.00	0.0600
9.23	255.50	0.0600
27.69	255.00	0.0600
50.77	254.90	0.0600
64.62	254.85	0.0600
78.47	252.26	0.0600
96.93	249.44	0.0600
129.24	245.94	0.0600

145.40	245.76	0.0600	
146.20	245.80	0.0600 / 0.0350	Main Channel
146.70	245.30	0.0350	Main Channel
147.70	245.30	0.0350	Main Channel
148.20	245.80	0.0350 / 0.0600	Main Channel
150.01	245.78	0.0600	
152.32	245.79	0.0600	
163.86	246.72	0.0600	
	175.40	249.07	0.0600
186.94	253.15	0.0600	
196.17	255.38	0.0600	
228.48	255.55	0.0600	

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.50	245.80	.221E+04	0.9	1.02	40.44
1.01	246.31	.352E+05	14.0	0.99	41.77
1.53	246.83	.841E+05	47.3	1.39	29.61
2.04	247.34	.144E+06	102.0	1.76	23.49
2.55	247.85	.213E+06	177.3	2.07	19.99
3.07	248.37	.291E+06	274.4	2.34	17.66
3.58	248.88	.378E+06	394.6	2.58	15.97
4.09	249.39	.474E+06	541.4	2.83	14.60
4.61	249.91	.578E+06	719.9	3.08	13.38
5.12	250.42	.688E+06	923.8	3.33	12.40
5.63	250.93	.803E+06	1153.3	3.56	11.61
6.14	251.44	.928E+06	1408.6	3.77	10.95
6.66	251.96	.105E+07	1690.6	3.98	10.38
7.17	252.47	.119E+07	2002.3	4.18	9.88
7.68	252.98	.133E+07	2345.4	4.38	9.43
8.20	253.50	.147E+07	2710.1	4.56	9.05
8.71	254.01	.162E+07	3101.0	4.73	8.72
9.22	254.52	.178E+07	3521.8	4.90	8.43
9.74	255.04	.195E+07	3467.6	4.39	9.39

		hydrograph			-> pi pe / channel ->	
INFLOW	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
ID= 2 (8208)	244.15	0.75	7.00	5.15	0.41	1.02
OUTFLOW: ID= 1 (6438)	244.15	0.67	7.75	5.15	0.37	1.02

ADD HYD (8208)		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3					
ID1= 1 (0438):	130.70	0.416	6.75	4.76	
+ ID2= 2 (6438):	244.15	0.666	7.75	5.15	
ID = 3 (8208):	374.85	1.025	7.50	5.01	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8210)		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3					
ID1= 1 (0440):	226.35	0.621	7.75	6.58	
+ ID2= 2 (8208):	374.85	1.025	7.50	5.01	
ID = 3 (8210):	601.20	1.644	7.50	5.60	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0432)		Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 mi n		114.21	53.0
		5.00	# of Li near Res. (N)= 1.50
		1.21	

Uni t Hyd Qpeak (cms)= 1.617
 PEAK FLOW (cms)= 0.232 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 5.180
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.123

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0430)		Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 mi n		111.64	55.0
		5.00	# of Li near Res. (N)= 1.50
		0.52	

Uni t Hyd Qpeak (cms)= 3.687
 PEAK FLOW (cms)= 0.457 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 5.441
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.130

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0428)		Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 mi n		50.53	51.0
		5.00	# of Li near Res. (N)= 1.50
		0.50	

Uni t Hyd Qpeak (cms)= 1.725
 PEAK FLOW (cms)= 0.184 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 4.733
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.113

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0426)		Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 mi n		247.17	61.0
		5.00	# of Li near Res. (N)= 1.50
		0.98	

Uni t Hyd Qpeak (cms)= 4.315
 PEAK FLOW (cms)= 0.770 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 6.793
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.162

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0424)		Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 mi n		49.57	59.0
		5.00	# of Li near Res. (N)= 1.50
		0.37	

Uni t Hyd Qpeak (cms)= 2.293
 PEAK FLOW (cms)= 0.296 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 6.128
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.146

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8198)		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3					
ID1= 1 (0424):	49.57	0.296	6.50	6.13	
+ ID2= 2 (0426):	247.17	0.770	7.50	6.79	
ID = 3 (8198):	296.74	0.999	7.00	6.68	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6428)
IN= 2--> OUT= 1

Routing time step (mi.n)'= 15.00

DATA FOR SECTION (4281.0) -----

Distance	Elevation	Manning	
0.00	246.54	0.0900	
8.80	246.29	0.0900	
30.78	246.03	0.0900	
41.78	243.97	0.0900	
63.77	237.16	0.0900	
85.76	232.53	0.0900	
96.75	230.16	0.0900	
109.94	228.40	0.0900	
112.14	228.28	0.0900 / 0.0450	Main Channel
112.84	228.00	0.0450	Main Channel
113.34	227.50	0.0450	Main Channel
114.34	227.50	0.0450	Main Channel
114.84	228.00	0.0450	Main Channel
116.54	228.29	0.0450 / 0.0900	Main Channel
118.74	228.40	0.0900	
120.94	228.52	0.0900	
149.52	229.51	0.0900	
169.31	230.69	0.0900	
186.00	234.27	0.0900	
217.69	243.56	0.0900	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi.n)
0.78	228.28	.205E+04	1.6	0.97	21.60
1.58	229.08	.202E+05	16.6	1.02	20.37
2.39	229.89	.665E+05	64.8	1.22	17.11
3.19	230.69	.133E+06	158.1	1.49	14.05
4.00	231.50	.213E+06	308.6	1.82	11.48
4.80	232.30	.299E+06	504.9	2.11	9.89
5.61	233.11	.394E+06	746.6	2.37	8.79
6.41	233.91	.496E+06	1034.5	2.61	7.99
7.21	234.71	.605E+06	1373.3	2.84	7.35
8.02	235.52	.722E+06	1763.3	3.06	6.82
8.82	236.32	.844E+06	2202.0	3.27	6.39
9.63	237.13	.974E+06	2690.5	3.46	6.03
10.43	237.93	.111E+07	3239.5	3.66	5.71
11.23	238.73	.125E+07	3839.0	3.84	5.43
12.04	239.54	.140E+07	4489.1	4.03	5.18
12.84	240.34	.155E+07	5190.5	4.20	4.97
13.65	241.15	.170E+07	5943.8	4.36	4.78
14.45	241.95	.187E+07	6749.8	4.53	4.61
15.26	242.76	.204E+07	7609.4	4.68	4.46

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8198)	296.74	1.00	7.00	6.68	0.49	0.97
OUTFLOW: ID= 1 (6428)	296.74	0.96	7.50	6.68	0.47	0.97

ADD HYD (8200)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0428):	50.53	0.184	6.50	4.73
+ ID2= 2 (6428):	296.74	0.963	7.50	6.68
ID = 3 (8200):	347.27	1.118	7.25	6.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8202)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0430):	111.64	0.457	6.50	5.44
+ ID2= 2 (8200):	347.27	1.118	7.25	6.40
ID = 3 (8202):	458.91	1.531	7.00	6.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6432)
IN= 2--> OUT= 1

Routing time step (mi.n)'= 15.00

----- DATA FOR SECTION (4321.0) -----

Distance	Elevation	Manning	
0.00	221.00	0.0600	
15.29	219.85	0.0600	
45.86	219.86	0.0600	
76.44	219.96	0.0600	
110.84	220.23	0.0600	
112.00	220.15	0.0600	
114.66	220.09	0.0600	
118.48	219.73	0.0600	
120.80	219.70	0.0600 / 0.0350	Main Channel
121.30	219.20	0.0350	Main Channel
122.30	219.20	0.0350	Main Channel
122.80	219.70	0.0350 / 0.0600	Main Channel
129.95	220.11	0.0600	
175.81	220.24	0.0600	
214.03	220.55	0.0600	
252.25	220.61	0.0600	
290.47	220.70	0.0600	
328.69	221.04	0.0600	
347.80	221.08	0.0600	
378.37	221.04	0.0600	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi.n)
0.08	219.28	.185E+03	0.0	0.23	149.38
0.17	219.37	.399E+03	0.1	0.34	100.62
0.25	219.45	.641E+03	0.1	0.42	80.98
0.33	219.53	.911E+03	0.2	0.49	69.79
0.42	219.62	.121E+04	0.3	0.55	62.35
0.50	219.70	.154E+04	0.5	0.60	56.91
0.61	219.81	.267E+04	0.7	0.56	61.28
0.71	219.91	.901E+04	1.3	0.30	112.96
0.82	220.02	.249E+05	3.4	0.28	122.71
0.92	220.12	.452E+05	6.8	0.31	109.98
1.03	220.23	.747E+05	12.2	0.33	102.05
1.14	220.34	.112E+06	21.3	0.39	87.59
1.24	220.44	.153E+06	33.0	0.44	77.01
1.35	220.55	.196E+06	47.2	0.49	69.22
1.46	220.66	.248E+06	62.3	0.51	66.48
1.56	220.76	.310E+06	83.4	0.55	62.06
1.67	220.87	.376E+06	108.8	0.59	57.56
1.77	220.97	.444E+06	137.7	0.64	53.77
1.88	221.08	.517E+06	163.6	0.65	52.70

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8202)	458.91	1.53	7.00	6.17	0.72	0.30
OUTFLOW: ID= 1 (6432)	458.91	1.14	9.50	6.17	0.68	0.35

ADD HYD (8204)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0432):	114.21	0.232	8.00	5.18
+ ID2= 2 (6432):	458.91	1.138	9.50	6.17
ID = 3 (8204):	573.12	1.344	9.25	5.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8212)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8204):	573.12	1.344	9.25	5.97
+ ID2= 2 (8210):	601.20	1.644	7.50	5.60
ID = 3 (8212):	1174.32	2.836	8.00	5.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6442)
IN= 2--> OUT= 1

Routing time step (mi.n)' = 15.00

<----- DATA FOR SECTION (4421.0) ----->

Distance	Elevation	Manning	
0.00	221.00	0.0350	
26.73	220.80	0.0350	
53.47	220.60	0.0350	
66.83	220.40	0.0350	
80.20	220.20	0.0350	
347.52	220.00	0.0350	
354.21	219.59	0.0350	
360.89	219.50	0.0350	
367.57	219.08	0.0350	
368.76	219.00	0.0350 / 0.0300	Main Channel
369.26	218.50	0.0300	Main Channel
374.26	218.50	0.0300	Main Channel
374.76	219.00	0.0300 / 0.0350	Main Channel
380.94	220.33	0.0350	
387.62	219.62	0.0350	
394.31	219.23	0.0350	
454.45	219.07	0.0350	
514.60	219.09	0.0350	
660.00	219.19	0.0350	
661.63	221.00	0.0350	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.12	218.62	.100E+04	0.2	0.28	92.89
0.25	218.75	.205E+04	0.6	0.43	60.09
0.37	218.87	.315E+04	1.1	0.55	46.99
0.50	219.00	.430E+04	1.8	0.66	39.67
0.63	219.13	.141E+05	3.4	0.37	69.99
0.77	219.27	.652E+05	13.7	0.33	79.14
0.90	219.40	.124E+06	35.5	0.45	58.02
1.03	219.53	.183E+06	65.7	0.56	46.45
1.17	219.67	.245E+06	103.8	0.66	39.30
1.30	219.80	.307E+06	149.3	0.76	34.30
1.43	219.93	.371E+06	201.6	0.85	30.65
1.57	220.07	.440E+06	256.5	0.91	28.56
1.70	220.20	.541E+06	325.7	0.94	27.70
1.83	220.33	.663E+06	415.2	0.98	26.62
1.97	220.47	.787E+06	520.3	1.03	25.21
2.10	220.60	.913E+06	638.5	1.09	23.82
2.23	220.73	.104E+07	767.2	1.15	22.62
2.37	220.87	.117E+07	908.0	1.21	21.54
2.50	221.00	.131E+07	1060.7	1.27	20.57

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8212)	1174.32	2.84	8.00	5.78	0.59	0.44
OUTFLOW: ID= 1 (6442)	1174.32	2.60	9.75	5.78	0.57	0.47

ADD HYD (8214)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0442):	117.26	0.170	8.00	3.63
+ ID2= 2 (6442):	1174.32	2.599	9.75	5.78
ID = 3 (8214):	1291.58	2.744	9.75	5.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8196)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8194):	44084.05	78.205	14.25	11.81
+ ID2= 2 (8214):	1291.58	2.744	9.75	5.59
ID = 3 (8196):	45375.62	79.234	14.25	11.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHYD (0444)
ID= 1 DT=15.0 mi n

Area (ha)= 221.65
Curve Number (CN)= 56.0
U. H. Tp(hrs)= 1.03
of Li near Res. (N)= 1.50

Unit Hyd Opeak (cms)= 3.656

PEAK FLOW (cms)= 0.556 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 5.731
TOTAL RAINFALL (mm)= 42.000
RUNOFF COEFFICIENT = 0.136

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8216)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0444):	221.65	0.556	7.50	5.73
+ ID2= 2 (8196):	45375.62	79.234	14.25	11.63
ID = 3 (8216):	45597.28	79.368	14.25	11.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

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V V I SSSS U U A L
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
W I SSSS UUUU A A LLLLL

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***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\Visual Otthymo 2.4\VO2\vo1.nat
 Output filename: C:\Users\amanu\AppData\Local\Temp\14a82fd7-d8b8-4b9f-aaa2-7731d664f7ae\Scenario.out
 Summary filename: C:\Users\amanu\AppData\Local\Temp\14a82fd7-d8b8-4b9f-aaa2-7731d664f7ae\Scenario.sum

DATE: 08/29/2012 TIME: 01:56:05

USER:

COMMENTS: _____

 ** SIMULATION NUMBER: 1 **

READ STORM
 Ptotal = 54.40 mm
 Filename: C:\Users\amanu\AppData\Local\Temp\14a82fd7-d8b8-4b9f-aaa2-7731d664f7ae\31710974
 Comments: 5-Year 12-Hour SCS II Design Storm

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.36	3.25	2.18	6.25	9.79	9.25	1.90
0.50	1.36	3.50	2.18	6.50	9.79	9.50	1.90
0.75	1.36	3.75	2.18	6.75	4.35	9.75	1.90
1.00	1.36	4.00	2.18	7.00	4.35	10.00	1.90
1.25	1.36	4.25	3.26	7.25	3.26	10.25	1.09
1.50	1.36	4.50	3.26	7.50	3.26	10.50	1.09
1.75	1.36	4.75	4.35	7.75	3.26	10.75	1.09
2.00	1.36	5.00	4.35	8.00	3.26	11.00	1.09
2.25	1.63	5.25	6.53	8.25	1.90	11.25	1.09
2.50	1.63	5.50	6.53	8.50	1.90	11.50	1.09
2.75	1.63	5.75	26.11	8.75	1.90	11.75	1.09
3.00	1.63	6.00	71.81	9.00	1.90	12.00	1.09

CALIB STANDHYD (1662)
 ID= 1 DT=15.0 min
 Area (ha)= 141.06
 Total Imp(%)= 70.00 Di r. Conn.(%)= 38.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	98.74	42.32
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	969.74	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	71.81	88.34
over (min)=	15.00	30.00
Storage Coeff. (min)=	14.03 (ii)	25.28 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.07	0.04
PEAK FLOW (cms)=	8.01	4.74
TIME TO PEAK (hrs)=	6.00	6.25

TOTALS
 11.073 (iii)
 6.00

RUNOFF VOLUME (mm)= 52.40 27.66 37.06
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.51 0.68

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1661)
 ID= 1 DT=15.0 min
 Area (ha)= 293.64
 Total Imp(%)= 42.00 Di r. Conn.(%)= 23.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	123.33	170.31
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1399.14	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	71.81	43.05
over (min)=	15.00	45.00
Storage Coeff. (min)=	17.49 (ii)	32.47 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.07	0.03

TOTALS
 12.281 (iii)
 6.00
 28.27
 54.40
 0.52

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5166)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	7.2300	6.6504
0.4310	3.5677	9.1580	7.5825
1.6680	3.8178	10.9790	8.5125
3.5970	4.8509	11.3790	8.5225
5.3070	5.7381	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1661)	293.640	12.281	6.00	28.27
OUTFLOW: ID= 1 (5166)	293.640	3.520	7.50	28.26

PEAK FLOW REDUCTION [Qout/Qin] (%) = 28.66
 TIME SHIFT OF PEAK FLOW (min) = 90.00
 MAXIMUM STORAGE USED (ha.m.) = 4.8148

ADD HYD (8134)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1662):	141.06	11.073	6.00	37.06
+ ID2= 2 (5166):	293.64	3.520	7.50	28.26
ID= 3 (8134):	434.70	11.277	6.00	31.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1642)
 ID= 1 DT=15.0 min
 Area (ha)= 120.35
 Total Imp(%)= 22.00 Di r. Conn.(%)= 17.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	26.48	93.87
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	895.73	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	71.81	17.32
Storage Coeff. (min)=	15.00	45.00
Unit Hyd. Tpeak (min)=	13.38 (ii)	34.95 (ii)
Unit Hyd. Tpeak (cms)=	15.00	45.00
	0.08	0.03
PEAK FLOW (cms)=	3.11	2.59
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	52.40	17.50
TOTAL RAINFALL (mm)=	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.32

TOTALS
4.168 (iii)
6.00
23.43
54.40
0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.3 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1641)	Area (ha)=	Dir. Conn. (%)=
ID= 1 DT=15.0 min	113.89	53.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	80.86	33.03
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	871.36	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	71.81	59.11
Storage Coeff. (min)=	15.00	30.00
Unit Hyd. Tpeak (min)=	13.16 (ii)	26.36 (ii)
Unit Hyd. Tpeak (cms)=	15.00	30.00
	0.08	0.04
PEAK FLOW (cms)=	9.25	2.41
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	52.40	23.65
TOTAL RAINFALL (mm)=	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.43

TOTALS
10.772 (iii)
6.00
38.89
54.40
0.71

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.3 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5164)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1 DT= 15.0 min	0.0000	0.0000	8.4260	4.5513
	0.3860	1.9228	9.2330	4.9251
	3.3700	2.2339	9.9990	5.2982
	4.5710	3.3613	10.3990	5.3082
	5.7170	4.0568	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1641)	113.890	10.772	6.00	38.89
OUTFLOW: ID= 1 (5164)	113.890	3.473	6.50	38.87

PEAK FLOW REDUCTION [Qout/Qin] (%) = 32.24
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha.m.) = 2.3622

ADD HYD (8132)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (1642):	120.35	4.168	6.00	23.43
+ ID2= 2 (5164):	113.89	3.473	6.50	38.87
ID= 3 (8132):	234.24	6.889	6.50	30.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0152)	Area (ha)=	Curve Number (CN)=
ID= 1 DT=15.0 min	124.37	59.0
	U. H. Tp (hrs)= 5.00	# of Linear Res. (N)= 1.50
		0.89

Unit Hyd Opeak (cms)= 2.372

PEAK FLOW (cms)=	0.660 (i)
TIME TO PEAK (hrs)=	7.250
RUNOFF VOLUME (mm)=	10.672
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.196

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1481)	Area (ha)=	Dir. Conn. (%)=
ID= 1 DT=15.0 min	61.75	17.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	29.02	32.73
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	641.61	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	71.81	4.73
Storage Coeff. (min)=	15.00	60.00
Unit Hyd. Tpeak (min)=	10.95 (ii)	47.21 (ii)
Unit Hyd. Tpeak (cms)=	15.00	60.00
	0.08	0.02

PEAK FLOW (cms)=	1.72	0.26
TIME TO PEAK (hrs)=	6.00	6.75
RUNOFF VOLUME (mm)=	52.40	4.53
TOTAL RAINFALL (mm)=	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.08

TOTALS
1.796 (iii)
6.00
12.67
54.40
0.23

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 23.5 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5148)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1 DT= 15.0 min	0.0000	0.0000	4.1310	1.6343
	0.2090	0.7802	5.1870	1.8598
	1.1960	0.9062	6.0380	2.0805
	2.2280	1.1562	6.4380	2.0905
	3.0910	1.3741	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1481)	61.750	1.796	6.00	12.67
OUTFLOW: ID= 1 (5148)	61.750	0.139	8.75	12.65

PEAK FLOW REDUCTION [Qout/Qin] (%) = 7.76
TIME SHIFT OF PEAK FLOW (min) = 165.00
MAXIMUM STORAGE USED (ha.m.) = 0.5204

CALIB NASHYD (1482) Area (ha) = 356.14 Curve Number (CN) = 31.3
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 1.12

Unit Hyd Qpeak (cms) = 5.438

PEAK FLOW (cms) = 0.585 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 3.986
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.073

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9148) AREA OPEAK TPEAK R. V.
 1 + 2 = 3 (ha) (cms) (hrs) (mm)
 ID1= 1 (1482): 356.14 0.585 8.00 3.99
 + ID2= 2 (5148): 61.75 0.139 8.75 12.65
 ID = 3 (9148): 417.89 0.723 8.00 5.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1501) Area (ha) = 0.16 Di r. Conn. (%) = 15.00
 ID= 1 DT=15.0 min Total Imp(%) = 45.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 0.07 0.09
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 32.66 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 71.81 3.02
 over (min) = 15.00 60.00
 Storage Coeff. (min) = 1.83 (ii) 45.21 (ii)
 Unit Hyd. Tpeak (min) = 15.00 60.00
 Unit Hyd. peak (cms) = 0.11 0.02

TOTALS
 PEAK FLOW (cms) = 0.00 0.00 0.005 (iii)
 TIME TO PEAK (hrs) = 6.00 6.75 6.00
 RUNOFF VOLUME (mm) = 52.40 2.94 10.25
 TOTAL RAINFALL (mm) = 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.05 0.19

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 16.4 Ia = Dep Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5150) IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.0110	0.0042
0.0010	0.0020	0.0140	0.0049
0.0030	0.0023	0.0160	0.0054
0.0060	0.0030	0.0160	0.0154
0.0080	0.0036	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
0.160	0.005	6.00	10.25
0.160	0.000	7.75	7.19

PEAK FLOW REDUCTION [Qout/Qin] (%) = 9.58

TIME SHIFT OF PEAK FLOW (min) = 105.00
 MAXIMUM STORAGE USED (ha. m.) = 0.0009

CALIB NASHYD (1502) Area (ha) = 105.72 Curve Number (CN) = 17.2
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 0.58

Unit Hyd Qpeak (cms) = 3.134

PEAK FLOW (cms) = 0.135 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 1.874
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.034

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9150) AREA OPEAK TPEAK R. V.
 1 + 2 = 3 (ha) (cms) (hrs) (mm)
 ID1= 1 (1502): 105.72 0.135 6.75 1.87
 + ID2= 2 (5150): 0.16 0.000 7.75 7.19
 ID = 3 (9150): 105.88 0.135 6.75 1.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8116) AREA OPEAK TPEAK R. V.
 1 + 2 = 3 (ha) (cms) (hrs) (mm)
 ID1= 1 (9148): 417.89 0.723 8.00 5.27
 + ID2= 2 (9150): 105.88 0.135 6.75 1.88
 ID = 3 (8116): 523.77 0.833 7.50 4.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6152) Routing time step (min) = 15.00
 IN= 2--> OUT= 1

<----- DATA FOR SECTION (1521.0) ----->

Distance	Elevation	Manning
0.00	290.00	0.0500
15.34	286.87	0.0500
46.03	284.84	0.0500
88.22	283.07	0.0500
126.57	282.11	0.0500
153.42	280.34	0.0500
157.26	280.07	0.0500
161.09	279.72	0.0500
167.76	278.94	0.0500 / 0.0300
168.01	278.60	0.0300
168.76	278.60	0.0300
169.51	278.60	0.0300
169.76	278.90	0.0300 / 0.0500
173.76	279.40	0.0500
180.27	279.69	0.0500
184.11	279.93	0.0500
218.62	281.86	0.0500
260.82	283.20	0.0500
360.54	289.25	0.0500
379.72	289.80	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.30	278.90	.114E+04	0.4	0.80	45.70
0.87	279.47	.932E+04	4.2	0.98	37.22
1.45	280.05	.353E+05	17.5	1.09	33.68
2.02	280.62	.831E+05	47.8	1.26	29.01
2.59	281.19	.155E+06	102.9	1.46	25.06
3.17	281.77	.250E+06	189.3	1.66	22.03
3.74	282.34	.375E+06	298.5	1.75	20.91

4.32	282.92	.547E+06	456.5	1.83	19.98
4.89	283.49	.767E+06	705.7	2.02	18.12
5.46	284.06	.102E+07	1038.0	2.24	16.34
6.04	284.64	.130E+07	1444.1	2.45	14.98
6.61	285.21	.161E+07	1940.1	2.66	13.79
7.18	285.78	.194E+07	2522.6	2.86	12.80
7.76	286.36	.229E+07	3187.4	3.06	11.98
8.33	286.93	.267E+07	3942.3	3.25	11.28
8.91	287.51	.306E+07	4824.9	3.46	10.58
9.48	288.08	.347E+07	5795.7	3.67	9.99
10.05	288.65	.390E+07	6855.4	3.86	9.48
10.63	289.23	.434E+07	8005.0	4.05	9.04

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8116)	523.77	0.83	7.50	4.58	0.36	0.82
OUTFLOW: ID= 1 (6152)	523.77	0.78	8.50	4.58	0.36	0.82

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
(8122)				
1 + 2 = 3				
ID1= 1 (0152):	124.37	0.660	7.25	10.67
+ ID2= 2 (6152):	523.77	0.784	8.50	4.58
ID = 3 (8122):	648.14	1.390	8.00	5.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD	Area (ha)	Curve Number (CN)
(0158)	178.59	61.0
ID= 1 DT=15.0 min	U.H. Tp(hrs)= 5.00	# of Linear Res. (N)= 1.50
	U.H. Tp(hrs)= 0.94	

Unit Hyd Opeak (cms) = 3.230

PEAK FLOW (cms) =	0.973 (i)
TIME TO PEAK (hrs) =	7.250
RUNOFF VOLUME (mm) =	11.394
TOTAL RAINFALL (mm) =	54.400
RUNOFF COEFFICIENT =	0.209

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD	Area (ha)	Curve Number (CN)
(0156)	83.49	45.0
ID= 1 DT=15.0 min	U.H. Tp(hrs)= 5.00	# of Linear Res. (N)= 1.50
	U.H. Tp(hrs)= 0.62	

Unit Hyd Opeak (cms) = 2.306

PEAK FLOW (cms) =	0.364 (i)
TIME TO PEAK (hrs) =	6.750
RUNOFF VOLUME (mm) =	6.641
TOTAL RAINFALL (mm) =	54.400
RUNOFF COEFFICIENT =	0.122

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD	Area Total	IMPERVIOUS	PERVIOUS (i)
(1541)	11.15		
ID= 1 DT=15.0 min	Imp(%)= 75.00	Dir. Conn.(%)= 70.00	

Surface Area (ha) =	8.36	2.79
Dep. Storage (mm) =	2.00	5.00
Average Slope (%) =	0.50	0.50
Length (m) =	272.64	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr) =	71.81	3.94
over (min) =	15.00	60.00
Storage Coeff. (min) =	6.55 (ii)	45.56 (ii)
Unit Hyd. Tpeak (min) =	15.00	60.00

Unit Hyd. peak (cms) =	0.10	0.02	*TOTALS*
PEAK FLOW (cms) =	1.45	0.02	1.457 (iii)
TIME TO PEAK (hrs) =	6.00	6.75	6.00
RUNOFF VOLUME (mm) =	52.40	4.90	38.15
TOTAL RAINFALL (mm) =	54.40	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.09	0.70

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 31.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5154)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1				
DT= 15.0 min				
	0.0000	0.0000	0.7460	0.3544
	0.0380	0.1987	0.9370	0.3839
	0.2160	0.2386	1.0900	0.4129
	0.4020	0.2749	1.4900	0.4229
	0.5580	0.3133	0.0000	0.0000

INFLOW : ID= 2 (1541)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
OUTFLOW: ID= 1 (5154)	11.150	1.457	6.00	38.15
	11.150	0.307	6.25	38.03

PEAK FLOW REDUCTION [Qout/Qin] (%) = 21.08
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha.m.) = 0.2582

CALIB NASHYD	Area (ha)	Curve Number (CN)
(1542)	189.17	39.1
ID= 1 DT=15.0 min	U.H. Tp(hrs)= 5.00	# of Linear Res. (N)= 1.50
	U.H. Tp(hrs)= 0.95	

Unit Hyd Opeak (cms) = 3.396

PEAK FLOW (cms) =	0.478 (i)
TIME TO PEAK (hrs) =	7.250
RUNOFF VOLUME (mm) =	5.423
TOTAL RAINFALL (mm) =	54.400
RUNOFF COEFFICIENT =	0.100

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
(9154)				
1 + 2 = 3				
ID1= 1 (1542):	189.17	0.478	7.25	5.42
+ ID2= 2 (5154):	11.15	0.307	6.25	38.03
ID = 3 (9154):	200.32	0.723	6.50	7.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
(8118)				
1 + 2 = 3				
ID1= 1 (0156):	83.49	0.364	6.75	6.64
+ ID2= 2 (9154):	200.32	0.723	6.50	7.24
ID = 3 (8118):	283.81	1.081	6.50	7.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6158)	Routing time step (min)
IN= 2--> OUT= 1	= 15.00

<----- DATA FOR SECTION (1581.0) ----->

Distance	Elevation	Manning	
0.00	290.00	0.0400	
19.20	288.88	0.0400	
48.00	288.26	0.0400	
119.99	283.68	0.0400	
167.99	282.07	0.0400	
196.79	281.32	0.0400	
210.79	280.90	0.0400	
219.79	280.38	0.0400 / 0.0300	Main Channel
220.04	279.65	0.0300	Main Channel
220.79	279.65	0.0300	Main Channel
221.54	279.65	0.0300	Main Channel
221.79	280.39	0.0300 / 0.0400	Main Channel
225.79	280.59	0.0400	
235.19	280.88	0.0400	
254.39	281.50	0.0400	
273.58	282.56	0.0400	
302.38	283.42	0.0400	
359.98	284.03	0.0400	
436.77	289.37	0.0400	
475.17	290.37	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.37	280.01	.973E+03	0.5	0.86	31.78
0.73	280.38	.210E+04	1.5	1.18	23.16
1.30	280.95	.149E+05	8.6	0.95	28.91
1.86	281.51	.579E+05	40.0	1.13	24.12
2.43	282.08	.133E+06	118.5	1.46	18.75
2.99	282.64	.236E+06	254.6	1.77	15.46
3.56	283.21	.369E+06	450.3	2.00	13.67
4.13	283.78	.542E+06	702.7	2.13	12.86
4.69	284.34	.767E+06	1125.2	2.41	11.36
5.26	284.91	.101E+07	1697.1	2.75	9.93
5.82	285.47	.127E+07	2379.1	3.07	8.91
6.39	286.04	.155E+07	3171.4	3.36	8.14
6.95	286.60	.184E+07	4075.5	3.64	7.53
7.52	287.17	.215E+07	5093.1	3.89	7.03
8.09	287.74	.247E+07	6226.5	4.13	6.62
8.65	288.30	.281E+07	7461.2	4.36	6.28
9.22	288.87	.318E+07	8635.1	4.46	6.13
9.78	289.43	.357E+07	10143.6	4.67	5.86
10.35	290.00	.398E+07	11608.8	4.79	5.71

		<---- hydrograph ---->			<- pi pe / channel ->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW :	ID= 2 (8118)	283.81	1.08	6.50	7.06	0.57
OUTFLOW:	ID= 1 (6158)	283.81	0.98	7.25	7.06	0.54

ADD HYD (8120)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0158):	178.59	0.973	7.25	11.39
+ ID2= 2 (6158):	283.81	0.982	7.25	7.06
ID = 3 (8120):	462.40	1.955	7.25	8.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8124)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (8120):	462.40	1.955	7.25	8.74
+ ID2= 2 (8122):	648.14	1.390	8.00	5.75
ID = 3 (8124):	1110.54	3.287	7.50	6.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6160)
IN= 2----> OUT= 1 Routing time step (mi n)' = 15.00

<----- DATA FOR SECTION (1601.0) ----->

Distance	Elevation	Manning	
0.00	261.02	0.0400	
29.89	254.30	0.0400	
40.75	252.58	0.0400	
62.49	251.30	0.0400	
114.11	250.94	0.0400	
130.41	248.58	0.0400	
141.28	247.50	0.0400	
154.86	246.87	0.0400	
155.20	246.70	0.0400	
160.20	246.30	0.0400 / 0.0300	Main Channel
160.30	245.30	0.0300	Main Channel
165.20	245.30	0.0300	Main Channel
165.30	246.30	0.0300 / 0.0400	Main Channel
168.45	246.96	0.0400	
195.62	249.20	0.0400	
203.77	250.82	0.0400	
225.50	256.78	0.0400	
244.52	261.14	0.0400	
255.39	261.95	0.0400	
268.97	261.92	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.50	245.80	.120E+05	2.9	1.18	68.31
1.00	246.30	.242E+05	8.5	1.70	47.47
1.92	247.22	.825E+05	31.7	1.86	43.34
2.84	248.14	.248E+06	102.5	2.00	40.39
3.76	249.06	.506E+06	247.7	2.37	34.03
4.68	249.98	.832E+06	486.8	2.83	28.49
5.60	250.90	.121E+07	814.9	3.27	24.71
6.52	251.82	.180E+07	1157.1	3.11	25.97
7.44	252.74	.253E+07	1840.2	3.52	22.92
8.36	253.66	.331E+07	2727.1	3.98	20.25
9.29	254.59	.414E+07	3782.6	4.43	18.22
10.21	255.51	.499E+07	5010.7	4.86	16.61
11.13	256.43	.589E+07	6397.4	5.26	15.33
12.05	257.35	.681E+07	7932.8	5.64	14.31
12.97	258.27	.777E+07	9621.2	5.99	13.46
13.89	259.19	.877E+07	11469.4	6.33	12.74
14.81	260.11	.980E+07	13478.4	6.66	12.12
15.73	261.03	.109E+08	15651.7	6.97	11.58
16.65	261.95	.120E+08	17492.5	7.07	11.42

		<---- hydrograph ---->			<- pi pe / channel ->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW :	ID= 2 (8124)	1110.54	3.29	7.50	6.99	0.53
OUTFLOW:	ID= 1 (6160)	1110.54	2.82	9.00	6.99	0.48

CALIB STANDBYD (1601)	Area Total (ha)	Imp (%)	Dir. Conn. (%)
ID= 1 DT=15.0 mi n	19.03	75.00	69.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	14.27	4.76
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	356.18	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	71.81	13.24
over (mi n)	15.00	45.00
Storage Coeff. (mi n)=	7.69 (ii)	31.71 (ii)
Unit Hyd. Tpeak (mi n)=	15.00	45.00
Unit Hyd. peak (cms)=	0.10	0.03

	TOTALS		
PEAK FLOW (cms)=	2.37	0.11	2.410 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	52.40	11.86	39.83
TOTAL RAI NFALL (mm)=	54.40	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.22	0.73

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 55.6 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5160)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	1.2730	0.6034
	0.0640	0.3377	1.5980	0.6540
	0.3690	0.4055	1.8600	0.7037
	0.6870	0.4676	2.2600	0.7137
	0.9520	0.5332	0.0000	0.0000
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1601)	19.030	2.410	6.00	39.83
OUTFLOW: ID= 1 (5160)	19.030	0.547	6.50	39.76

PEAK FLOW REDUCTION [Qout/Qin] (%) = 22.68
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha.m.) = 0.4421

CALIB NASHYD (1602)			
ID= 1 DT=15.0 min			
Area (ha)	= 299.79	Curve Number (CN)	= 60.3
Ia (mm)	= 5.00	# of Linear Res. (N)	= 1.50
U.H. Tp (hrs)	= 1.42		

Unit Hyd Opeak (cms) = 3.592

PEAK FLOW (cms) = 1.176 (i)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 11.197
TOTAL RAINFALL (mm) = 54.400
RUNOFF COEFFICIENT = 0.206

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9160)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1602):	299.79	1.176	8.00	11.20
+ ID2= 2 (5160):	19.03	0.547	6.50	39.76
=====				
ID = 3 (9160):	318.82	1.467	6.75	12.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8126)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6160):	1110.54	2.817	9.00	6.99
+ ID2= 2 (9160):	318.82	1.467	6.75	12.90
=====				
ID = 3 (8126):	1429.36	4.089	8.75	8.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1621)			
ID= 1 DT=15.0 min			
Area Total	(ha) = 46.07	Dir. Conn. (%) =	20.00
Imp (%) =	45.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 20.73	25.34
Dep. Storage (mm)	= 2.00	5.00
Average Slope (%)	= 0.50	0.50
Length (m)	= 554.20	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr) = 71.81 over (min) = 15.00
Storage Coeff. (min) = 10.03 (ii) 32.52 (ii)
Unit Hyd. Tpeak (min) = 15.00 45.00

Unit Hyd. peak (cms) =	0.09	0.03	*TOTALS*
PEAK FLOW (cms) =	1.55	0.66	1.817 (iii)
TIME TO PEAK (hrs) =	6.00	6.50	6.00
RUNOFF VOLUME (mm) =	52.40	11.91	20.01
TOTAL RAINFALL (mm) =	54.40	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.22	0.37

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 51.1 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5162)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	3.0820	1.1997
	0.1560	0.5629	3.8700	1.3716
	0.8930	0.6512	4.5050	1.5398
	1.6620	0.8407	4.9050	1.5498
	2.3060	1.0035	0.0000	0.0000
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1621)	46.070	1.817	6.00	20.01
OUTFLOW: ID= 1 (5162)	46.070	0.297	7.75	19.99

PEAK FLOW REDUCTION [Qout/Qin] (%) = 16.33
TIME SHIFT OF PEAK FLOW (min) = 105.00
MAXIMUM STORAGE USED (ha.m.) = 0.5802

CALIB STANDHYD (1622)			
ID= 1 DT=15.0 min			
Area Total	(ha) = 144.07	Dir. Conn. (%) =	11.00
Imp (%) =	26.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 37.46	106.61
Dep. Storage (mm)	= 2.00	5.00
Average Slope (%)	= 0.50	0.50
Length (m)	= 980.03	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr) = 71.81 over (min) = 15.00
Storage Coeff. (min) = 14.12 (ii) 40.26 (ii)
Unit Hyd. Tpeak (min) = 15.00 45.00
Unit Hyd. peak (cms) = 0.07 0.03

PEAK FLOW (cms) =	2.36	1.70	*TOTALS*
TIME TO PEAK (hrs) =	6.00	6.50	3.035 (iii)
RUNOFF VOLUME (mm) =	52.40	10.00	6.00
TOTAL RAINFALL (mm) =	54.40	54.40	14.67
RUNOFF COEFFICIENT =	0.96	0.18	54.40
			0.27

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 51.1 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9162)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1622):	144.07	3.035	6.00	14.67
+ ID2= 2 (5162):	46.07	0.297	7.75	19.99
=====				
ID = 3 (9162):	190.14	3.099	6.00	15.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8126):	1429.36	4.089	8.75	8.31
+ ID2= 2 (9162):	190.14	3.099	6.00	15.96

ID = 3 (8128):	1619.50	4.948	8.00	9.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8128):	1619.50	4.948	8.00	9.21
+ ID2= 2 (8132):	234.24	6.889	6.50	30.94

ID = 3 (8130):	1853.74	11.631	6.50	11.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6166)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1661.0) ----->

Distance	Elevation	Manning	
0.00	248.36	0.0500	
41.67	246.98	0.0500	
95.82	244.93	0.0500	
149.98	242.42	0.0500	
191.64	241.88	0.0500	
224.97	239.90	0.0500	
229.13	239.35	0.0500	
233.30	237.77	0.0500	
237.47	236.57	0.0500	
240.63	235.60	0.0500 / 0.0300	Main Channel
241.63	234.10	0.0300	Main Channel
247.13	234.10	0.0300	Main Channel
247.53	235.60	0.0300 / 0.0500	Main Channel
254.13	236.25	0.0500	
266.63	236.77	0.0500	
283.29	237.84	0.0500	
291.63	238.89	0.0500	
329.12	244.66	0.0500	
370.78	249.55	0.0500	
412.44	253.17	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.75	234.85	.130E+05	6.0	1.36	36.45
1.50	235.60	.276E+05	18.1	1.95	25.41
2.25	236.35	.544E+05	40.0	2.18	22.69
3.00	237.10	.117E+06	80.1	2.04	24.32
3.75	237.85	.213E+06	148.9	2.07	23.89
4.50	238.60	.334E+06	255.1	2.27	21.85
5.25	239.35	.473E+06	395.8	2.49	19.90
6.00	240.10	.631E+06	563.8	2.65	18.66
6.75	240.85	.824E+06	773.2	2.79	17.77
7.50	241.60	.106E+07	1037.9	2.92	16.97
8.26	242.36	.135E+07	1326.7	2.92	16.94
9.01	243.11	.173E+07	1758.5	3.01	16.44
9.76	243.86	.217E+07	2285.9	3.13	15.81
10.51	244.61	.265E+07	2909.5	3.26	15.17
11.26	245.36	.318E+07	3610.1	3.37	14.67
12.01	246.11	.377E+07	4417.4	3.48	14.21
12.76	246.86	.441E+07	5348.0	3.60	13.75
13.51	247.61	.512E+07	6394.0	3.71	13.34
14.26	248.36	.589E+07	7580.3	3.82	12.95

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8130)	1853.74	11.63	6.50	11.96	1.10

OUTFLOW: ID= 1 (6166) 1853.74 9.26 7.00 11.95 0.95 1.48

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6166):	1853.74	9.257	7.00	11.95
+ ID2= 2 (8134):	434.70	11.277	6.00	31.12

ID = 3 (8136):	2288.44	14.955	7.00	15.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2021)
ID= 1 DT=15.0 min Area Total (ha)= 107.32 Imp(%)= 45.00 Di r. Conn. (%)= 27.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	48.29	59.03
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	845.85	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	71.81	43.99
over (min)	15.00	30.00
Storage Coeff. (min)=	12.93 (ii)	27.79 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04
PEAK FLOW (cms)=	4.47	3.10
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	52.40	21.49
TOTAL RAINFALL (mm)=	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.40

TOTALS
6.403 (iii)
6.00
29.84
54.40
0.55

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5202)
IN= 2--> OUT= 1
DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.0000	5.1480	2.8069
0.3630	1.2717		6.4200	3.2412
1.5670	1.4796		7.4640	3.6910
2.7880	1.9296		7.8640	3.7010
3.8580	2.3404		0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2021)	107.320	6.403	6.00	29.84
OUTFLOW: ID= 1 (5202)	107.320	2.023	6.75	29.83

PEAK FLOW REDUCTION [Out/Oi n] (%) = 31.59
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 1.6625

CALIB STANDHYD (2022)
ID= 1 DT=15.0 min Area Total (ha)= 361.21 Imp(%)= 60.00 Di r. Conn. (%)= 36.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	216.73	144.48
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1551.79	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	71.81	60.07

over (min) 15.00 45.00
 Storage Coeff. (min)= 18.61 (ii) 31.72 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.06 0.03

TOTALS
 PEAK FLOW (cms)= 17.12 8.91 20.978 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 52.40 24.29 34.41
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.45 0.63

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 73.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8110)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2022):	361.21	20.978	6.00	34.41
+ ID2= 2 (5202):	107.32	2.023	6.75	29.83
ID = 3 (8110):	468.53	21.201	6.00	33.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (0102)
 ID= 1 DT=15.0 min

Area	(ha)	Imp(%)	Dir. Conn. (%)
Total	466.00	23.00	9.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	107.18	358.82
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1762.57	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 48.96 9.63
 over (min) 30.00 60.00
 Storage Coeff. (min)= 23.41 (ii) 50.69 (ii)
 Unit Hyd. Tpeak (min)= 30.00 60.00
 Unit Hyd. peak (cms)= 0.04 0.02

TOTALS
 PEAK FLOW (cms)= 4.18 4.27 6.461 (iii)
 TIME TO PEAK (hrs)= 6.25 6.75 6.25
 RUNOFF VOLUME (mm)= 52.40 9.19 13.08
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.17 0.24

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 49.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0100)
 ID= 1 DT=15.0 min

Area	(ha)	Curve Number (CN)
Ia	693.84	40.0
U.H. Tp(hrs)=	5.00	# of Linear Res. (N) = 1.50
	1.95	

Unit Hyd Opeak (cms)= 6.087

PEAK FLOW (cms)= 1.069 (i)
 TIME TO PEAK (hrs)= 9.250
 RUNOFF VOLUME (mm)= 5.648
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.104

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8000)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0100):	693.84	1.069	9.25	5.65
+ ID2= 2 (0102):	466.00	6.461	6.25	13.08
ID = 3 (8000):	1159.84	6.998	6.25	8.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6104)
 IN= 2----> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1041.0) ----->

Distance	Elevation	Manning
0.00	330.16	0.0450
50.03	328.36	0.0450
89.34	326.33	0.0450
117.94	324.30	0.0450
160.82	322.75	0.0450
178.34	319.96	0.0450
183.34	319.81	0.0450
184.34	319.38	0.0450 / 0.0300
185.09	318.78	0.0300
185.84	318.72	0.0300
186.84	319.32	0.0300 / 0.0450
193.84	319.70	0.0450
198.84	320.38	0.0450
200.13	320.81	0.0450
218.00	322.49	0.0450
239.44	323.49	0.0450
250.17	323.96	0.0450
275.18	325.20	0.0450
310.92	326.47	0.0450
353.81	330.00	0.0450

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.30	319.02	.624E+03	0.1	0.45	73.00
0.60	319.32	.181E+04	0.6	0.67	49.46
1.23	319.95	.128E+05	4.9	0.77	43.38
1.86	320.58	.396E+05	19.1	0.96	34.50
2.48	321.20	.757E+05	43.5	1.15	28.98
3.11	321.83	.125E+06	81.2	1.30	25.58
3.74	322.46	.187E+06	135.9	1.45	22.92
4.37	323.09	.269E+06	195.6	1.45	22.90
5.00	323.72	.387E+06	287.8	1.48	22.44
5.63	324.35	.545E+06	428.4	1.56	21.22
6.25	324.97	.735E+06	630.5	1.71	19.42
6.88	325.60	.952E+06	876.1	1.83	18.12
7.51	326.23	.120E+07	1177.0	1.95	17.03
8.14	326.86	.149E+07	1554.4	2.08	15.93
8.77	327.49	.179E+07	2009.7	2.23	14.88
9.40	328.12	.213E+07	2533.9	2.37	13.99
10.02	328.74	.249E+07	3112.6	2.49	13.31
10.65	329.37	.288E+07	3760.9	2.60	12.75
11.28	330.00	.330E+07	4494.8	2.71	12.23

<---- hydrograph ----> <- pipe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8000)	1159.84	7.00	6.25	8.63	1.32	0.79
OUTFLOW: ID= 1 (6104)	1159.84	5.33	7.00	8.63	1.25	0.77

CALIB
 STANDHYD (1041)
 ID= 1 DT=15.0 min

Area	(ha)	Imp(%)	Dir. Conn. (%)
Total	0.67	75.00	70.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.50	0.17
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	66.83	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 71.81 4.86
 over (min) 15.00 45.00

Storage Coeff. (min)= 2.82 (ii) 38.68 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.11 0.03

PEAK FLOW (cms)= 0.09 0.00 *TOTALS* 0.094 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 52.40 6.03 38.46
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.11 0.71

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 36.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5104)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.0450	0.0213
0.0020	0.0120	0.0560	0.0231
0.0130	0.0144	0.0660	0.0248
0.0240	0.0165	0.4660	0.0348
0.0340	0.0189	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
0.670	0.094	6.00	38.46
0.670	0.021	6.25	36.20

INFLOW: ID= 2 (1041)
 OUTFLOW: ID= 1 (5104)
 PEAK FLOW REDUCTION [Qout/Qin] (%) = 22.31
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 0.0162

CALIB NASHYD (1042)
 ID= 1 DT=15.0 min

Area (ha)= 527.14 Curve Number (CN)= 42.6
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.34

Unit Hyd Qpeak (cms)= 6.731

PEAK FLOW (cms)= 1.188 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 6.190
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.114

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9104)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
527.14	1.188	8.00	6.19
+ ID2= 2 (5104): 0.67	0.021	6.25	36.20
-----	-----	-----	-----
ID = 3 (9104): 527.81	1.194	8.00	6.23

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8002)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1159.84	5.330	7.00	8.63
+ ID2= 2 (9104): 527.81	1.194	8.00	6.23
-----	-----	-----	-----
ID = 3 (8002): 1687.65	6.441	7.25	7.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDBYD (1061)
 ID= 1 DT=15.0 min

Area (ha)= 32.26
 Total Imp(%)= 67.00 Dir. Conn. (%)= 51.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)	PERVIOUS (i)
21.61	10.65
2.00	5.00
0.50	0.50
463.75	40.00
0.013	0.250

Max. Eff. Inten. (mm/hr)= 71.81 19.25
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 9.01 (ii) 29.69 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.04

PEAK FLOW (cms)= 2.85 0.39 *TOTALS* 3.089 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 52.40 14.21 33.69
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.26 0.62

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 56.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5106)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.1580	0.9744
0.1090	0.5252	2.7090	1.0694
0.6250	0.6260	3.1540	1.1625
1.1640	0.7386	3.5540	1.1725
1.6140	0.8505	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
32.260	3.089	6.00	33.69
32.260	0.644	6.75	33.65

INFLOW: ID= 2 (1061)
 OUTFLOW: ID= 1 (5106)
 PEAK FLOW REDUCTION [Qout/Qin] (%) = 20.84
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 0.6366

CALIB NASHYD (1062)
 ID= 1 DT=15.0 min

Area (ha)= 227.27 Curve Number (CN)= 57.5
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.09

Unit Hyd Qpeak (cms)= 3.556

PEAK FLOW (cms)= 0.988 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 10.199
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.187

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9106)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
227.27	0.988	7.75	10.20
+ ID2= 2 (5106): 32.26	0.644	6.75	33.65
-----	-----	-----	-----
ID = 3 (9106): 259.53	1.551	6.75	13.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8002):	1687.65	6.441	7.25	7.88
+ ID2= 2 (9106):	259.53	1.551	6.75	13.11

ID = 3 (8004):	1947.18	7.938	7.00	8.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6108)
IN= 2---> OUT= 1 Routing time step (min)'= 15.00

Distance	Elevation	Manning	
0.00	326.06	0.0400	
12.06	325.74	0.0400	
30.15	324.28	0.0400	
59.27	320.35	0.0400	
72.37	317.60	0.0400	
87.45	314.52	0.0400	
93.48	313.91	0.0400	
105.54	313.55	0.0400	
106.17	313.40	0.0400	
110.62	313.04	0.0400 / 0.0300	Main Channel
110.82	312.97	0.0300	Main Channel
111.57	313.08	0.0300	Main Channel
111.67	313.48	0.0300 / 0.0400	Main Channel
129.66	316.62	0.0400	
150.77	318.95	0.0400	
180.92	319.61	0.0400	
205.04	321.23	0.0400	
232.18	322.09	0.0400	
268.37	322.31	0.0400	
298.52	326.00	0.0400	

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.43	313.40	.244E+04	0.7	0.59	58.83
1.09	314.06	.214E+05	10.4	1.01	34.24
1.76	314.73	.600E+05	40.2	1.39	24.89
2.42	315.39	.109E+06	92.1	1.75	19.80
3.08	316.05	.169E+06	166.5	2.05	16.88
3.75	316.72	.237E+06	264.4	2.31	14.97
4.41	317.38	.318E+06	386.3	2.52	13.72
5.07	318.04	.411E+06	539.3	2.72	12.71
5.74	318.71	.517E+06	725.5	2.91	11.88
6.40	319.37	.642E+06	909.2	2.94	11.77
7.06	320.03	.806E+06	1179.3	3.04	11.38
7.72	320.69	.989E+06	1515.9	3.18	10.87
8.39	321.36	.119E+07	1895.5	3.30	10.49
9.05	322.02	.143E+07	2310.2	3.36	10.30
9.71	322.68	.172E+07	2763.2	3.33	10.39
10.38	323.35	.205E+07	3477.6	3.53	9.81
11.04	324.01	.238E+07	4284.3	3.73	9.27
11.70	324.67	.274E+07	5154.0	3.91	8.85
12.37	325.34	.311E+07	6104.0	4.08	8.48

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8004)	1947.18	7.94	7.00	8.58	0.92
OUTFLOW: ID= 1 (6108)	1947.18	6.93	7.75	8.58	0.85

CALIB STANDHYD (1081)
ID= 1 DT=15.0 min
Area (ha)= 40.81
Total Imp(%)= 75.00
Dir. Conn.(%)= 70.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	30.61	10.20
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	521.60	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (min)	71.81 / 15.00	3.76 / 60.00

Storage Coeff. (min)=	9.67 (ii)	49.42 (ii)
Unit Hyd. Tpeak (min)=	15.00	60.00
Unit Hyd. peak (cms)=	0.09	0.02

PEAK FLOW (cms)=	4.86	0.06	*TOTALS* 4.877 (iii)
TIME TO PEAK (hrs)=	6.00	6.75	6.00
RUNOFF VOLUME (mm)=	52.40	4.68	38.08
TOTAL RAINFALL (mm)=	54.40	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.09	0.70

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 30.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5108)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.7300	1.2970
0.1380	0.7271	3.4270	1.4050
0.7910	0.8733	3.9900	1.5110
1.4720	1.0061	4.3900	1.5210
2.0420	1.1467	0.0000	0.0000

INFLOW : ID= 2 (1081)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
40.810	40.810	4.877	6.00	38.08
OUTFLOW: ID= 1 (5108)	40.810	1.092	6.50	38.05

PEAK FLOW REDUCTION [Qout/Qin] (%) = 22.40
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 0.9358

CALIB NASHYD (1082)
ID= 1 DT=15.0 min

Area (ha)= 166.24
Ia (mm)= 5.00
U. H. Tp(hrs)= 0.89
Curve Number (CN)= 32.2
of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 3.171

PEAK FLOW (cms)=	0.334 (i)
TIME TO PEAK (hrs)=	7.250
RUNOFF VOLUME (mm)=	4.127
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.076

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9108)
1 + 2 = 3

ID1= 1 (1082):	166.24	0.334	7.25	4.13
+ ID2= 2 (5108):	40.81	1.092	6.50	38.05

ID = 3 (9108):	207.05	1.392	6.50	10.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8006)
1 + 2 = 3

ID1= 1 (6108):	1947.18	6.928	7.75	8.58
+ ID2= 2 (9108):	207.05	1.392	6.50	10.81

ID = 3 (8006):	2154.23	7.715	7.75	8.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (1101)
ID= 1 DT=15.0 min

Area (ha)= 60.97
Total Imp(%)= 69.00 Dir. Conn.(%)= 55.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 42.07 18.90
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 637.55 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 71.81 16.38
over (min)= 15.00 45.00
Storage Coeff. (min)= 10.91 (ii) 32.97 (ii)

Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.08 0.03

TOTALS
PEAK FLOW (cms)= 5.49 0.51 5.702 (iii)
TIME TO PEAK (hrs)= 6.00 6.50 6.00
RUNOFF VOLUME (mm)= 52.40 12.48 34.44
TOTAL RAINFALL (mm)= 54.40 54.40 54.40
RUNOFF COEFFICIENT = 0.96 0.23 0.63

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 52.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5110)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.0780	1.8615
0.2060	1.0120	5.1210	2.0372
1.1810	1.2083	5.9610	2.2096
2.2000	1.4181	6.3610	2.2196
3.0510	1.6293	0.0000	0.0000

AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW: ID= 2 (1101) 60.970 5.702 6.00 34.44
OUTFLOW: ID= 1 (5110) 60.970 1.235 6.75 34.42

PEAK FLOW REDUCTION [Oout/Oin] (%) = 21.65
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha.m.) = 1.2293

CALIB
NASHYD (1102)
ID= 1 DT=15.0 min

Area (ha)= 262.95 Curve Number (CN)= 55.4
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U.H. Tp(hrs)= 1.29

Unit Hyd Opeak (cms)= 3.483

PEAK FLOW (cms)= 0.947 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 9.545
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.175

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9110)
1 + 2 = 3

AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (1102): 262.95 0.947 8.00 9.54
+ ID2= 2 (5110): 60.97 1.235 6.75 34.42

ID = 3 (9110): 323.92 2.049 6.75 14.23

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8008)
1 + 2 = 3

AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 (8006): 2154.23 7.715 7.75 8.79
+ ID2= 2 (9110): 323.92 2.049 6.75 14.23

ID = 3 (8008): 2478.15 9.354 7.50 9.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6112)
IN= 2---> OUT= 1

Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1121.0) ----->

Distance	Elevation	Manning
0.00	320.59	0.0400
13.48	317.93	0.0400
40.45	311.52	0.0400
64.04	307.68	0.0400
74.15	307.01	0.0400
91.00	307.22	0.0400
107.86	305.21	0.0400
111.23	304.86	0.0400
113.32	304.60	0.0400
117.77	304.24	0.0400 / 0.0300
117.97	304.17	0.0300
118.72	304.28	0.0300
118.82	304.68	0.0300 / 0.0400
124.71	304.76	0.0400
171.90	306.30	0.0400
205.60	307.24	0.0400
239.31	308.24	0.0400
262.90	310.64	0.0400
289.86	314.34	0.0400
333.68	317.66	0.0400

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.43	304.60	.372E+04	0.6	0.54	97.65
1.12	305.29	.463E+05	12.5	0.85	61.89
1.80	305.97	.149E+06	53.0	1.13	46.85
2.49	306.66	.311E+06	134.2	1.37	38.62
3.18	307.35	.554E+06	261.6	1.50	35.27
3.87	308.04	.894E+06	490.7	1.74	30.37
4.55	308.72	.129E+07	833.0	2.05	25.75
5.24	309.41	.171E+07	1269.8	2.36	22.39
5.93	310.10	.215E+07	1789.0	2.64	20.01
6.62	310.79	.261E+07	2392.7	2.90	18.21
7.30	311.47	.310E+07	3087.8	3.16	16.74
7.99	312.16	.361E+07	3873.6	3.40	15.52
8.68	312.85	.413E+07	4741.0	3.64	14.52
9.37	313.54	.467E+07	5689.3	3.86	13.69
10.05	314.22	.523E+07	6718.4	4.07	12.97
10.74	314.91	.581E+07	7770.7	4.24	12.46
11.43	315.60	.641E+07	8897.5	4.40	12.01
12.12	316.29	.704E+07	10113.2	4.55	11.61
12.80	316.97	.770E+07	11420.1	4.70	11.24

<----- hydrograph -----> <- pipe / channel ->
AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW: ID= 2 (8008) 2478.15 9.35 7.50 9.50 0.94 0.74
OUTFLOW: ID= 1 (6112) 2478.15 7.56 8.75 9.50 0.83 0.69

CALIB
STANDHYD (1121)
ID= 1 DT=15.0 min

Area (ha)= 9.96
Total Imp(%)= 55.00 Dir. Conn.(%)= 25.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 5.48 4.48
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 257.68 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 71.81 8.21
over (min)= 15.00 45.00

Storage Coeff. (min)= 6.34 (ii) 35.41 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.10 0.03

PEAK FLOW (cms)= 0.47 0.06 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.50 0.489 (iii)
 RUNOFF VOLUME (mm)= 52.40 17.33
 TOTAL RAINFALL (mm)= 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.10 0.32

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 26.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5112)
 IN= 2----> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.6660	0.2787
0.0340	0.1406	0.8360	0.3122
0.1930	0.1653	0.9740	0.3450
0.3590	0.2034	1.3740	0.3550
0.4980	0.2383	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
9.960	0.489	6.00	17.33
9.960	0.029	8.25	17.23

INFLOW : ID= 2 (1121)
 OUTFLOW: ID= 1 (5112)

PEAK FLOW REDUCTION [Qout/Qin](%)= 5.93
 TIME SHIFT OF PEAK FLOW (min)=135.00
 MAXIMUM STORAGE USED (ha.m.)= 0.1200

CALIB NASHYD (1122)
 ID= 1 DT=15.0 min

Area (ha)= 404.93 Curve Number (CN)= 31.1
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.90

Unit Hyd Qpeak (cms)= 3.635

PEAK FLOW (cms)= 0.445 (i)
 TIME TO PEAK (hrs)= 9.250
 RUNOFF VOLUME (mm)= 3.971
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.073

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9112)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
404.93	0.445	9.25	3.97
9.96	0.029	8.25	17.23
414.89	0.474	9.25	4.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8010)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
2478.15	7.562	8.75	9.50
414.89	0.474	9.25	4.29
2893.04	8.034	8.75	8.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6114)
 IN= 2----> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1141.0) ----->

Distance	Elevation	Manning
0.00	312.27	0.0500
14.07	309.70	0.0500
28.14	306.51	0.0500
35.17	305.81	0.0500
42.20	304.95	0.0500
52.75	304.24	0.0500
77.37	303.61	0.0500
98.47	301.33	0.0500
131.76	300.40	0.0500 / 0.0300
136.21	300.04	0.0300
136.41	299.97	0.0300
137.16	300.08	0.0300
137.26	300.48	0.0300 / 0.0500
140.68	300.53	0.0500
179.36	302.20	0.0500
214.53	303.82	0.0500
242.66	305.27	0.0500
288.38	307.66	0.0500
316.52	311.76	0.0500
348.17	313.48	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.43	300.40	366E+04	0.7	0.59	88.31
1.05	301.02	505E+05	12.1	0.75	69.49
1.68	301.65	165E+06	52.7	0.99	52.28
2.30	302.27	324E+06	130.3	1.26	41.40
2.93	302.90	520E+06	247.1	1.48	35.08
3.55	303.52	754E+06	406.8	1.68	30.91
4.18	304.15	104E+07	586.1	1.76	29.55
4.80	304.77	138E+07	856.0	1.93	26.96
5.43	305.40	177E+07	1201.1	2.12	24.56
6.05	306.02	219E+07	1612.2	2.30	22.63
6.68	306.65	264E+07	2090.6	2.47	21.07
7.30	307.27	313E+07	2653.0	2.64	19.65
7.93	307.90	364E+07	3303.9	2.83	18.36
8.55	308.52	417E+07	4053.9	3.03	17.14
9.18	309.15	471E+07	4871.9	3.22	16.12
9.80	309.77	527E+07	5756.2	3.41	15.25
10.43	310.40	584E+07	6699.8	3.58	14.52
11.05	311.02	642E+07	7710.5	3.74	13.89
11.68	311.65	702E+07	8788.5	3.90	13.32

<---- hydrograph ----> <-pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8010)	2893.04	8.03	8.75	8.76	0.83	0.68
OUTFLOW: ID= 1 (6114)	2893.04	7.07	10.50	8.76	0.78	0.67

CALIB STANDHYD (1141)
 ID= 1 DT=15.0 min

Area (ha)= 19.45
 Total Imp(%)= 45.00
 Dir. Conn. (%)= 15.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 8.75 10.70
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 360.09 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 71.81 13.58
 over (min) 15.00 45.00
 Storage Coeff. (min)= 7.75 (ii) 31.52 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.10 0.03

TOTALS
 PEAK FLOW (cms)= 0.53 0.25 0.625 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 52.40 9.86 16.24
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.18 0.30

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 43.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5114)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1	(cms)	(ha. m.)	(cms)	(ha. m.)
DT= 15.0 min	0.0000	0.0000	1.3010	0.5065
	0.0660	0.2376	1.6340	0.5791
	0.3770	0.2749	1.9020	0.6501
	0.7020	0.3549	2.3020	0.6601
	0.9730	0.4237	0.0000	0.0000

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (1141)	19.450	0.625	6.00	16.24
OUTFLOW: ID= 1 (5114)	19.450	0.058	8.75	16.19

PEAK FLOW REDUCTION [Qout/Qin] (%) = 9.33
 TIME SHIFT OF PEAK FLOW (min) = 165.00
 MAXIMUM STORAGE USED (ha. m.) = 0.2099

CALIB	Area	(ha)	Curve Number	(CN) = 51.3
NASHYD (1142)	Ia	(mm) = 5.00	# of Linear Res. (N) = 1.50	
ID= 1 DT=15.0 min	U.H. Tp	(hrs) = 1.09		

Unit Hyd Qpeak (cms) = 4.197

PEAK FLOW (cms) = 0.947 (i)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 8.324
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.153

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9114)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1142):	268.19	0.947	7.75	8.32
+ ID2= 2 (5114):	19.45	0.058	8.75	16.19
ID = 3 (9114):	287.64	1.004	7.75	8.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8012)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6114):	2893.04	7.067	10.50	8.76
+ ID2= 2 (9114):	287.64	1.004	7.75	8.86
ID = 3 (8012):	3180.68	7.817	10.25	8.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	Curve Number	(CN) = 64.0
NASHYD (0116)	Ia	(mm) = 5.00	# of Linear Res. (N) = 1.50	
ID= 1 DT=15.0 min	U.H. Tp	(hrs) = 0.80		

Unit Hyd Qpeak (cms) = 4.394

PEAK FLOW (cms) = 1.414 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 12.512
 TOTAL RAINFALL (mm) = 54.400

RUNOFF COEFFICIENT = 0.230

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8014)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0116):	206.43	1.414	7.00	12.51
+ ID2= 2 (8012):	3180.68	7.817	10.25	8.76
ID = 3 (8014):	3387.11	8.601	10.00	8.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6118)	Routing time step (min) = 15.00
IN= 2--> OUT= 1	

Distance	Elevation	Manning	
0.00	307.18	0.0450	
24.89	299.75	0.0450	
37.34	296.39	0.0450	
71.57	292.75	0.0450	
96.46	291.12	0.0450	
108.90	289.43	0.0450	
112.02	288.86	0.0450	
115.13	288.46	0.0450	
117.24	288.32	0.0450 / 0.0350	Main Channel
117.49	288.15	0.0350	Main Channel
118.24	288.00	0.0350	Main Channel
118.99	288.20	0.0350	Main Channel
119.24	288.36	0.0350 / 0.0450	Main Channel
121.35	288.40	0.0450	
149.35	289.36	0.0450	
171.14	290.47	0.0450	
208.47	293.14	0.0450	
236.48	295.17	0.0450	
280.04	299.48	0.0450	
308.04	307.00	0.0450	

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)
0.32	288.32	.116E+04	0.2	0.62	81.92
1.30	289.30	.631E+05	22.2	1.07	47.43
2.29	290.29	.218E+06	117.5	1.64	30.95
3.27	291.27	.448E+06	314.6	2.15	23.72
4.25	292.25	.756E+06	625.6	2.53	20.13
5.24	293.24	.115E+07	1093.2	2.91	17.50
6.22	294.22	.161E+07	1731.6	3.28	15.50
7.20	295.20	.214E+07	2547.4	3.63	14.01
8.19	296.19	.274E+07	3584.9	4.00	12.72
9.17	297.17	.338E+07	4859.1	4.39	11.60
10.15	298.15	.407E+07	6341.5	4.76	10.70
11.13	299.13	.480E+07	8024.6	5.11	9.97
12.12	300.12	.556E+07	10006.6	5.49	9.27
13.10	301.10	.635E+07	12244.6	5.89	8.64
14.08	302.08	.716E+07	14681.7	6.26	8.13
15.07	303.07	.799E+07	17316.5	6.62	7.69
16.05	304.05	.884E+07	20148.1	6.96	7.31
17.03	305.03	.971E+07	23176.2	7.29	6.98
18.02	306.02	.106E+08	26400.8	7.61	6.69

INFLOW : ID= 2 (8014)	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
OUTFLOW: ID= 1 (6118)	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
	3387.11	8.60	10.00	8.99	0.70	0.74
	3387.11	8.04	11.25	8.99	0.67	0.73

CALIB	Area	(ha)	Curve Number	(CN) = 45.0
NASHYD (0118)	Ia	(mm) = 5.00	# of Linear Res. (N) = 1.50	
ID= 1 DT=15.0 min	U.H. Tp	(hrs) = 1.12		

Unit Hyd Qpeak (cms) = 5.200

PEAK FLOW (cms)= 0.950 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 6.723
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.124

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8016)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0118):	340.53	0.950	7.75	6.72
+ ID2= 2 (6118):	3387.11	8.042	11.25	8.99

ID = 3 (8016):	3727.64	8.654	11.00	8.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6122)
 IN= 2----> OUT= 1
 Routing time step (min)' = 15.00

DATA FOR SECTION (1221.0) ----->				
Distance	Elevation	Manning		
0.00	277.00	0.0500		
52.18	272.82	0.0500		
86.97	269.38	0.0500		
134.80	267.35	0.0500		
160.89	263.85	0.0500		
169.58	262.88	0.0500		
178.28	262.45	0.0500		
195.67	262.07	0.0500		
213.07	260.99	0.0500		
226.01	259.80	0.0500 / 0.0300	Main	Channel
226.11	259.30	0.0300	Main	Channel
230.11	259.30	0.0300	Main	Channel
230.21	259.80	0.0300 / 0.0500	Main	Channel
235.01	260.20	0.0500		
239.15	260.57	0.0500		
243.50	260.93	0.0500		
300.03	264.43	0.0500		
339.16	268.34	0.0500		
373.95	272.36	0.0500		
430.48	277.46	0.0500		

TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	259.80	.626E+04	2.5	1.23	41.35
1.41	260.71	.462E+05	21.8	1.44	35.27
2.31	261.61	.149E+06	79.6	1.63	31.19
3.22	262.52	.340E+06	200.1	1.80	28.34
4.12	263.42	.636E+06	446.8	2.15	23.71
5.03	264.33	.995E+06	819.9	2.52	20.23
5.93	265.23	.141E+07	1337.0	2.90	17.56
6.84	266.14	.187E+07	1984.8	3.25	15.67
7.74	267.04	.237E+07	2766.0	3.57	14.26
8.65	267.95	.292E+07	3596.9	3.76	13.53
9.55	268.85	.355E+07	4575.4	3.93	12.95
10.46	269.76	.427E+07	5814.9	4.16	12.23
11.36	270.66	.503E+07	7319.0	4.44	11.46
12.27	271.57	.584E+07	9009.7	4.71	10.81
13.17	272.47	.670E+07	10886.3	4.96	10.26
14.08	273.38	.761E+07	12893.2	5.17	9.84
14.98	274.28	.858E+07	15098.1	5.37	9.48
15.89	275.19	.961E+07	17527.6	5.57	9.14
16.79	276.09	.107E+08	20190.3	5.76	8.83

<---- hydrograph ----->					<-pi pe / channel -->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW: ID= 2 (8016)	3727.64	8.65	11.00	8.79	0.79	
OUTFLOW: ID= 1 (6122)	3727.64	8.49	11.75	8.79	0.78	

CALIB STANDHYD (1221)
 ID= 1 DT=15.0 min
 Area (ha)= 117.91
 Total Imp(%)= 44.00
 Dir. Conn.(%)= 16.00

IMPERVIOUS PERVIOUS (i)			
Surface Area (ha)=	51.88	66.03	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	886.60	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	71.81	9.92	
over (min)	15.00	45.00	
Storage Coeff. (min)=	13.30 (ii)	40.25 (ii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.08	0.03	
TOTALS			
PEAK FLOW (cms)=	2.88	0.98	3.268 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	52.40	7.52	14.70
TOTAL RAINFALL (mm)=	54.40	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.14	0.27

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 36.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5122)				
IN= 2----> OUT= 1				
DT= 15.0 min				
OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)	
0.0000	0.0000	7.8880	3.0390	
0.3990	1.4100	9.9040	3.4850	
2.2850	1.6269	11.5290	3.9214	
4.2550	2.1164	11.9290	3.9314	
5.9010	2.5338	0.0000	0.0000	

INFLOW: ID= 2 (1221)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
117.910	117.910	3.268	6.00	14.70
OUTFLOW: ID= 1 (5122)	117.910	0.323	8.75	14.69

PEAK FLOW REDUCTION [Out/Oi n](%)= 9.88				
TIME SHIFT OF PEAK FLOW (min)=165.00				
MAXIMUM STORAGE USED (ha. m.)= 1.1414				

CALIB NASHYD (1222)
 ID= 1 DT=15.0 min
 Area (ha)= 246.95
 Curve Number (CN)= 43.8
 # of Linear Res. (N)= 1.50
 U. H. Tp (hrs)= 1.13

Unit Hyd Opeak (cms)= 3.728
 PEAK FLOW (cms)= 0.655 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 6.447
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.119

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8018)				
1 + 2 = 3				
ID1= 1 (1222):	246.95	0.655	7.75	6.45
+ ID2= 2 (5122):	117.91	0.323	8.75	14.69

ID = 3 (8018):	364.86	0.972	8.00	9.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8020)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (6122):	3727.64	8.492	11.75	8.79	
+ ID2= 2 (8018):	364.86	0.972	8.00	9.11	

ID = 3 (8020):	4092.50	9.171	11.75	8.81	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	STANDHYD (1261)	Area (ha)	Imp (%)	Di r. Conn. (%)
ID= 1 DT=15.0 min		50.19	36.00	26.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	18.07	32.12
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	578.45	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	71.81	8.92
over (min)	15.00	45.00
Storage Coeff. (min)=	10.29 (ii)	38.42 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.09	0.03

TOTALS
 PEAK FLOW (cms)= 2.18 0.44 2.351 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 52.40 8.73 20.09
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.16 0.37

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 48.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5126)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min	0.0000	0.0000	3.3570	1.2179
	0.1700	0.5264	4.2160	1.4221
	0.9720	0.5968	4.9080	1.6217
	1.8110	0.8165	5.3080	1.6317
	2.5120	0.9954	0.0000	0.0000

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (1261)	50.190	2.351	6.00	20.09
OUTFLOW: ID= 1 (5126)	50.190	0.526	7.00	20.07

PEAK FLOW REDUCTION [Qout/Qin] (%) = 22.39
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 0.5598

CALIB	NASHYD (1262)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min		83.33	57.7
		5.00	# of Linear Res. (N)= 1.50
		U. H. Tp(hrs)= 0.57	

Unit Hyd Opeak (cms)=	2.509
PEAK FLOW (cms)=	0.599 (i)
TIME TO PEAK (hrs)=	6.500
RUNOFF VOLUME (mm)=	10.113
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.186

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	(8024)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (1262):	83.33	0.599	6.50	10.11	
+ ID2= 2 (5126):	50.19	0.526	7.00	20.07	

ID = 3 (8024):	133.52	1.106	7.00	13.86	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	NASHYD (0124)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min		121.43	43.0
		Ia (mm)= 5.00	# of Linear Res. (N)= 1.50
		U. H. Tp(hrs)= 0.51	

Unit Hyd Opeak (cms)= 4.049

PEAK FLOW (cms)=	0.569 (i)
TIME TO PEAK (hrs)=	6.500
RUNOFF VOLUME (mm)=	6.147
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.113

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	(8026)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0124):	121.43	0.569	6.50	6.15	
+ ID2= 2 (8024):	133.52	1.106	7.00	13.86	

ID = 3 (8026):	254.95	1.663	6.75	10.18	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6128)	Routing time step (min)
IN= 2--> OUT= 1	= 15.00

<----- DATA FOR SECTION (1281.0) ----->			
Distance	Elevation	Manning	
0.00	270.58	0.0400	
7.71	270.36	0.0400	
15.43	269.48	0.0400	
23.14	265.88	0.0400	
42.43	261.83	0.0400	
61.71	258.31	0.0400	
75.21	256.62	0.0400	
90.64	255.35	0.0400	
92.49	255.00	0.0400 / 0.0300	Main Channel
93.49	254.00	0.0300	Main Channel
94.49	253.90	0.0300	Main Channel
94.99	253.80	0.0300	Main Channel
95.49	253.70	0.0300	Main Channel
95.99	253.80	0.0300	Main Channel
98.49	255.00	0.0300 / 0.0400	Main Channel
100.28	255.44	0.0400	
134.99	257.45	0.0400	
148.49	259.67	0.0400	
167.77	263.42	0.0400	
190.92	270.00	0.0400	

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.65	254.35	.439E+04	2.0	1.24	36.10
1.30	255.00	.131E+05	9.4	1.92	23.39
2.18	255.88	.433E+05	37.0	2.30	19.54
3.06	256.76	.131E+06	113.3	2.34	19.20
3.95	257.65	.273E+06	273.8	2.70	16.64
4.83	258.53	.453E+06	538.5	3.21	14.01
5.71	259.41	.658E+06	902.9	3.70	12.14
6.59	260.29	.886E+06	1368.4	4.16	10.79
7.48	261.18	.114E+07	1936.3	4.59	9.79
8.36	262.06	.141E+07	2610.9	4.99	9.01
9.24	262.94	.171E+07	3398.7	5.37	8.36
10.12	263.82	.202E+07	4309.6	5.75	7.81
11.01	264.71	.235E+07	5347.5	6.13	7.33

11.89	265.59	.270E+07	6501.0	6.48	6.93
12.77	266.47	.307E+07	7808.6	6.86	6.55
13.65	267.35	.345E+07	9250.5	7.23	6.21
14.54	268.24	.384E+07	10808.3	7.59	5.92
15.42	269.12	.424E+07	12482.5	7.94	5.66
16.30	270.00	.466E+07	14127.4	8.18	5.49

<---- hydrograph ----> <-pi pe / channel ->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8026)	254.95	1.66	6.75	10.18	0.53	1.24
OUTFLOW: ID= 1 (6128)	254.95	1.38	7.50	10.18	0.44	1.24

CALIB STANDHYD (1281) ID= 1 DT=15.0 min	Area (ha)= 55.87	Imp(%)= 50.00	Di r. Conn.(%)= 29.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	27.93	27.93
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	610.30	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	71.81	16.37
over (min)	15.00	45.00
Storage Coeff. (min)=	10.63 (ii)	32.69 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.09	0.03

TOTALS

PEAK FLOW (cms)=	2.68	0.76	2.986 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	52.40	12.74	24.24
TOTAL RAINFALL (mm)=	54.40	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.23	0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 54.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5128) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	3.7380	1.5139
	0.1890	0.7401	4.6930	1.7113
	1.0830	0.8643	5.4630	1.9045
	2.0160	1.0853	5.8630	1.9045
	2.7960	1.2819	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1281)	55.870	2.986	6.00	24.24
OUTFLOW: ID= 1 (5128)	55.870	0.601	7.25	24.22

PEAK FLOW REDUCTION [Qout/Oin] (%) = 20.14
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha.m.) = 0.8006

CALIB NASHYD (1282) ID= 1 DT=15.0 min	Area (ha)= 184.36	Curve Number (CN)= 62.0
	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50
	U.H. Tp(hrs)= 0.62	

Unit Hyd Qpeak (cms)=	5.093
PEAK FLOW (cms)=	1.438 (i)
TIME TO PEAK (hrs)=	6.750
RUNOFF VOLUME (mm)=	11.652
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.214

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9128) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1282):	184.36	1.438	6.75	11.65
+ ID2= 2 (5128):	55.87	0.601	7.25	24.22
ID = 3 (9128):	240.23	2.002	7.00	14.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8028) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6128):	254.95	1.381	7.50	10.18
+ ID2= 2 (9128):	240.23	2.002	7.00	14.58
ID = 3 (8028):	495.18	3.311	7.25	12.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8022) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8020):	4092.50	9.171	11.75	8.81
+ ID2= 2 (8028):	495.18	3.311	7.25	12.31
ID = 3 (8022):	4587.68	10.204	11.25	9.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1201) ID= 1 DT=15.0 min	Area (ha)= 176.35	Imp(%)= 45.00	Di r. Conn.(%)= 22.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	79.36	96.99
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1084.28	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	71.81	13.09
over (min)	15.00	45.00
Storage Coeff. (min)=	15.01 (ii)	39.13 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.07	0.03

TOTALS

PEAK FLOW (cms)=	5.64	1.92	6.406 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	52.40	10.34	19.59
TOTAL RAINFALL (mm)=	54.40	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.19	0.36

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 47.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5120) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	11.7960	4.5929
	0.5970	2.1554	14.8120	5.2509
	3.4170	2.4937	17.2430	5.8948
	6.3630	3.2187	17.6430	5.9048
	8.8260	3.8422	0.0000	0.0000

INFLOW : ID= 2 (1201) 176.350 6.406 6.00 19.58
 OUTFLOW: ID= 1 (5120) 176.350 1.003 8.00 19.58

PEAK FLOW REDUCTION [Qout/Qin] (%) = 15.66
 TIME SHIFT OF PEAK FLOW (min) = 120.00
 MAXIMUM STORAGE USED (ha.m.) = 2.2053

CALIB STANDHYD (1202) | Area (ha) = 70.67
 ID= 1 DT=15.0 min | Total Imp(%) = 43.00 | Dir. Conn. (%) = 20.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 30.39 40.28
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 686.39 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 71.81 12.83
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 11.41 (ii) 35.73 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.08 0.03

TOTALS
 PEAK FLOW (cms) = 2.28 0.82 2.611 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 52.40 10.24 18.67
 TOTAL RAINFALL (mm) = 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.19 0.34

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 47.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8030) | Area (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 1 + 2 = 3
 ID1= 1 (1202): 70.67 2.611 6.00 18.67
 + ID2= 2 (5120): 176.35 1.003 8.00 19.58
 ID = 3 (8030): 247.02 2.848 6.00 19.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8032) | Area (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 1 + 2 = 3
 ID1= 1 (8022): 4587.68 10.204 11.25 9.19
 + ID2= 2 (8030): 247.02 2.848 6.00 19.32
 ID = 3 (8032): 4834.70 10.880 11.25 9.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6130) | Routing time step (min) = 15.00
 IN= 2---> OUT= 1

<----- DATA FOR SECTION (1301.0) ----->
 Distance Elevation Mannng
 0.00 257.18 0.0350
 62.45 254.70 0.0350
 109.29 252.74 0.0350
 150.92 251.55 0.0350
 192.56 248.96 0.0350
 239.39 247.16 0.0350
 291.84 246.00 0.0350

296.84 245.00 0.0350
 298.84 245.00 0.0350 / 0.0300
 299.24 244.60 0.0300
 301.84 244.60 0.0300
 303.44 244.60 0.0300
 303.84 245.00 0.0300 / 0.0350
 306.84 245.00 0.0350
 309.84 246.00 0.0350
 317.46 247.22 0.0350
 348.68 254.83 0.0350
 359.09 256.70 0.0350
 452.76 258.23 0.0350
 515.22 258.33 0.0350

TRAVEL TIME TABLE
 DEPTH (m) ELEV (m) VOLUME (cu. m.) FLOW RATE (cms) VELOCITY (m/s) TRAV. TIME (min)
 0.40 245.00 .870E+04 1.2 0.64 124.12
 1.04 245.64 .468E+05 9.5 0.96 81.81
 1.68 246.28 .109E+06 25.6 1.11 70.86
 2.32 246.92 .257E+06 63.5 1.17 67.51
 2.96 247.56 .498E+06 142.7 1.36 58.18
 3.61 248.21 .800E+06 266.7 1.58 50.01
 4.25 248.85 .116E+07 437.4 1.78 44.24
 4.89 249.49 .157E+07 669.1 2.01 39.20
 5.53 250.13 .205E+07 954.9 2.23 35.36
 6.17 250.77 .252E+07 1294.4 2.43 32.42
 6.81 251.41 .305E+07 1689.6 2.62 30.07
 7.45 252.05 .363E+07 2090.7 2.73 28.93
 8.09 252.69 .429E+07 2553.1 2.82 27.98
 8.73 253.33 .501E+07 3138.4 2.96 26.60
 9.37 253.97 .579E+07 3803.8 3.11 25.36
 10.02 254.62 .662E+07 4549.3 3.25 24.25
 10.66 255.26 .751E+07 5364.9 3.38 23.32
 11.30 255.90 .845E+07 6265.7 3.51 22.49
 11.94 256.54 .946E+07 7260.1 3.63 21.72

<---- hydrograph ----> <- pi pe / channel ->
 AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8032) 4834.70 10.88 11.25 9.71 1.09 0.97
 OUTFLOW: ID= 1 (6130) 4834.70 10.45 12.25 9.71 1.08 0.97

CALIB STANDHYD (1301) | Area (ha) = 320.20
 ID= 1 DT=15.0 min | Total Imp(%) = 64.00 | Dir. Conn. (%) = 47.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 204.93 115.27
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 1461.05 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 71.81 55.13
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 17.95 (ii) 31.52 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.06 0.03

TOTALS
 PEAK FLOW (cms) = 20.16 6.54 22.982 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 52.40 24.18 37.44
 TOTAL RAINFALL (mm) = 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.44 0.69

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5130) |
 IN= 2---> OUT= 1
 DT= 15.0 min
 OUTFLOW (cms) STORAGE (ha.m.) OUTFLOW (cms) STORAGE (ha.m.)
 0.0000 0.0000 18.6040 9.7083
 1.1320 5.3237 22.6740 10.8355

6.6360	6.1006	30.4650	12.2155
10.3780	7.2341	31.8650	12.2255
14.1960	8.4233	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1301)	320.200	22.982	6.00	37.44
OUTFLOW: ID= 1 (5130)	320.200	7.659	7.00	37.44

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 33.33
 TIME SHIFT OF PEAK FLOW (mi n) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 6.4530

CALIB NASHYD (1302)
 ID= 1 DT=15.0 mi n
 Area (ha)= 65.86 Curve Number (CN)= 80.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.69

Unit Hyd Opeak (cms)= 1.620
 PEAK FLOW (cms)= 0.887 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 21.236
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.390

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8036)
 1 + 2 = 3
 ID1= 1 (1302): 65.86 0.887 6.75 21.24
 + ID2= 2 (5130): 320.20 7.659 7.00 37.44
 ID = 3 (8036): 386.06 8.539 7.00 34.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8034)
 1 + 2 = 3
 ID1= 1 (6130): 4834.70 10.448 12.25 9.71
 + ID2= 2 (8036): 386.06 8.539 7.00 34.67
 ID = 3 (8034): 5220.76 11.749 12.25 11.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1861)
 ID= 1 DT=15.0 mi n
 Area (ha)= 6.85
 Total Imp(%)= 51.00 Dir. Conn.(%)= 27.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	3.49	3.36
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	213.70	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	71.81	17.11
over (mi n)	15.00	30.00
Storage Coeff. (mi n)=	5.66 (ii)	27.34 (ii)
Unit Hyd. Tpeak (mi n)=	15.00	30.00
Unit Hyd. peak (cms)=	0.11	0.04

TOTALS
 PEAK FLOW (cms)= 0.35 0.11 0.421 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 52.40 12.69 23.41
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.23 0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

CN* = 52.5 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5186)
 IN= 2---> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.4580	0.1865
0.0230	0.0917	0.5750	0.2106
0.1330	0.1071	0.6700	0.2341
0.2470	0.1341	1.0700	0.2441
0.3430	0.1582	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1861)	6.850	0.421	6.00	23.41
OUTFLOW: ID= 1 (5186)	6.850	0.067	7.00	23.27

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 16.03
 TIME SHIFT OF PEAK FLOW (mi n) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 0.0980

CALIB STANDHYD (1862)
 ID= 1 DT=15.0 mi n
 Area (ha)= 344.94
 Total Imp(%)= 43.00 Dir. Conn.(%)= 23.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	148.32	196.62
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1516.44	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 71.81 14.14
 over (mi n) 15.00 45.00
 Storage Coeff. (mi n)= 18.35 (ii) 41.74 (ii)
 Unit Hyd. Tpeak (mi n)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.06 0.03

TOTALS
 PEAK FLOW (cms)= 10.52 4.04 12.141 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 52.40 11.64 21.01
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.21 0.39

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 52.5 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9186)
 1 + 2 = 3
 ID1= 1 (1862): 344.94 12.141 6.00 21.01
 + ID2= 2 (5186): 6.85 0.067 7.00 23.27
 ID = 3 (9186): 351.79 12.154 6.00 21.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1881)
 ID= 1 DT=15.0 mi n
 Area (ha)= 1.12
 Total Imp(%)= 75.00 Dir. Conn.(%)= 46.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.84	0.28
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	86.41	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 71.81 67.24
 over (min) 15.00 30.00
 Storage Coeff. (min)= 3.29 (ii) 15.83 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.11 0.05

PEAK FLOW (cms)= 0.10 0.03 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.25 0.121 (iii)
 RUNOFF VOLUME (mm)= 52.40 20.48 6.00
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.38 0.65

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5188)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0750	0.0357
0.0040	0.0200	0.0940	0.0386
0.0220	0.0240	0.1100	0.0416
0.0410	0.0277	0.5100	0.0516
0.0560	0.0315	0.0000	0.0000

INFLOW : ID= 2 (1881)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5188)	1.120	0.121	6.00	35.16
	1.120	0.021	6.75	34.03

PEAK FLOW REDUCTION [Out/Oi n] (%) = 17.74
 TIME SHIF T OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0241

CALIB STANDHYD (1882)
 ID= 1 DT=15.0 min

Area (ha)= 307.41
 Total Imp(%)= 41.00 Di r. Conn. (%) = 25.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)= 126.04	181.37
Dep. Storage (mm)= 2.00	5.00
Average Slope (%)= 0.50	0.50
Length (m)= 1431.57	40.00
Mannings n = 0.013	0.250

Max. Eff. Inten. (mm/hr)= 71.81 15.74
 over (min) 15.00 45.00
 Storage Coeff. (min)= 17.73 (ii) 40.14 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.06 0.03

PEAK FLOW (cms)= 10.36 4.24 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.50 12.067 (iii)
 RUNOFF VOLUME (mm)= 52.40 13.62 6.00
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.25 0.43

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9188)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (1882):	307.41	12.067	6.00	23.31
+ ID2= 2 (5188):	1.12	0.021	6.75	34.03
ID = 3 (9188):	308.53	12.070	6.00	23.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8068)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (9186):	351.79	12.154	6.00	21.06
+ ID2= 2 (9188):	308.53	12.070	6.00	23.35
ID = 3 (8068):	660.32	24.224	6.00	22.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1821)
 ID= 1 DT=15.0 min

Area (ha)= 111.80
 Total Imp(%)= 40.00 Di r. Conn. (%) = 17.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)= 44.72	67.08
Dep. Storage (mm)= 2.00	5.00
Average Slope (%)= 0.50	0.50
Length (m)= 863.33	40.00
Mannings n = 0.013	0.250

Max. Eff. Inten. (mm/hr)= 71.81 14.58
 over (min) 15.00 45.00
 Storage Coeff. (min)= 13.09 (ii) 36.20 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03

PEAK FLOW (cms)= 2.92 1.54 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.50 3.539 (iii)
 RUNOFF VOLUME (mm)= 52.40 11.71 18.63
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.22 0.34

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 52.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5182)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	5.7800	2.5656
0.3790	1.0207	7.3720	3.0829
1.6240	1.1939	8.7710	3.5956
3.0930	1.6631	9.1710	3.6056
4.3530	2.1032	0.0000	0.0000

INFLOW : ID= 2 (1821)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5182)	111.800	3.539	6.00	18.63
	111.800	1.044	7.25	18.62

PEAK FLOW REDUCTION [Out/Oi n] (%) = 29.49
 TIME SHIF T OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha.m.) = 1.1147

CALIB NASHYD (1822)
 ID= 1 DT=15.0 min

Area (ha)= 44.40 Curve Number (CN)= 57.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.79

Unit Hyd Opeak (cms)= 0.966

PEAK FLOW (cms)= 0.244 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 9.977
 TOTAL RAINFALL (mm)= 54.400

RUNOFF COEFFICIENT = 0.183

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8062) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1822):	44.40	0.244	7.00	9.98
+ ID2= 2 (5182):	111.80	1.044	7.25	18.62

ID = 3 (8062):	156.20	1.285	7.25	16.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1841) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	145.07 48.00	Dir. Conn. (%)=	19.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	69.63	75.44
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	983.43	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)=	71.81	20.75
over (min)	15.00	45.00
Storage Coeff. (min)=	14.15 (ii)	34.22 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.07	0.03
PEAK FLOW (cms)=	4.10	2.52
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	52.40	14.57
TOTAL RAINFALL (mm)=	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.27

TOTALS
5.142 (iii)
0.40

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 55.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5184) IN= 2---- OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	5.9270	2.3472
	0.2980	1.0088	7.4630	2.7585
	1.6660	1.1775	8.7490	3.1642
	3.1100	1.4809	9.1490	3.1742
	4.4100	1.8897	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1841)	145.070	5.142	6.00	21.76
OUTFLOW: ID= 1 (5184)	145.070	2.433	6.75	21.75

PEAK FLOW REDUCTION [Qout/Qin] (%) = 47.32
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 1.3480

CALIB STANDHYD (1842) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	117.45 42.00	Dir. Conn. (%)=	17.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	49.33	68.12
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	884.87	40.00

Mannings n	=	0.013	0.250
Max. Eff. Inten. (mm/hr)=		71.81	17.66
over (min)		15.00	45.00
Storage Coeff. (min)=		13.28 (ii)	34.69 (ii)
Unit Hyd. Tpeak (min)=		15.00	45.00
Unit Hyd. peak (cms)=		0.08	0.03
PEAK FLOW (cms)=		3.05	1.93
TIME TO PEAK (hrs)=		6.00	6.50
RUNOFF VOLUME (mm)=		52.40	13.57
TOTAL RAINFALL (mm)=		54.40	54.40
RUNOFF COEFFICIENT =		0.96	0.25

TOTALS
3.836 (iii)
6.00
20.17
54.40
0.37

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 55.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8060) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1842):	117.45	3.836	6.00	20.17
+ ID2= 2 (5184):	145.07	2.433	6.75	21.75

ID = 3 (8060):	262.52	4.774	6.50	21.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8064) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8060):	262.52	4.774	6.50	21.05
+ ID2= 2 (8062):	156.20	1.285	7.25	16.16

ID = 3 (8064):	418.72	5.329	6.50	19.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1781) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	55.50 39.00	Dir. Conn. (%)=	13.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	21.65	33.86
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	608.28	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)=	71.81	50.91
over (min)	15.00	30.00
Storage Coeff. (min)=	10.61 (ii)	24.62 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.04
PEAK FLOW (cms)=	1.19	2.20
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	52.40	23.09
TOTAL RAINFALL (mm)=	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.42

TOTALS
2.634 (iii)
6.25
26.90
54.40
0.49

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 74.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5178)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	6.1870	0.7248
0.1880	0.3394	9.3670	0.8313
1.7260	0.4003	10.7440	0.9285
3.2000	0.4933	11.1440	0.9385
4.6710	0.6039	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1781)	55.500	2.634	6.25	26.90
OUTFLOW: ID= 1 (5178)	55.500	2.174	6.50	26.89

PEAK FLOW REDUCTION [Qout/Qin] (%) = 82.53
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha. m.) = 0.4447

CALIB NASHYD (1782)
ID= 1 DT=15.0 min

Area (ha)	Curve Number (CN)
219.91	77.6
5.00	# of Linear Res. (N) = 1.50
0.76	

Unit Hyd Qpeak (cms) = 4.921

PEAK FLOW (cms) = 2.515 (i)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 19.583
TOTAL RAINFALL (mm) = 54.400
RUNOFF COEFFICIENT = 0.360

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8046)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1782):	219.91	2.515	7.00	19.58
+ ID2= 2 (5178):	55.50	2.174	6.50	26.89
ID = 3 (8046):	275.41	4.584	6.50	21.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0180)
ID= 1 DT=15.0 min

Area (ha)	Curve Number (CN)
226.65	65.0
5.00	# of Linear Res. (N) = 1.50
1.17	

Unit Hyd Qpeak (cms) = 3.299

PEAK FLOW (cms) = 1.200 (i)
TIME TO PEAK (hrs) = 7.750
RUNOFF VOLUME (mm) = 13.003
TOTAL RAINFALL (mm) = 54.400
RUNOFF COEFFICIENT = 0.239

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8050)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0180):	226.65	1.200	7.75	13.00
+ ID2= 2 (8046):	275.41	4.584	6.50	21.06
ID = 3 (8050):	502.06	5.572	6.50	17.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1741)
ID= 1 DT=15.0 min

Area Total (ha)	Imp (%)	Dir. Conn. (%)
56.30	35.00	18.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)	Dep. Storage (mm)	Average Slope (%)	Length (m)	Mannings n
19.70	2.00	0.50	612.64	0.013
36.60	5.00	0.50	40.00	0.250
0.50	0.50			
0.013	0.250			

Max. Eff. Inten. (mm/hr) over (min)	Storage Coeff. (min)	Unit Hyd. Tpeak (min)	Unit Hyd. peak (cms)
71.81	15.00	15.00	0.09
18.02	10.65 (ii)	45.00	
45.00	31.88 (ii)	45.00	
31.88 (ii)		45.00	
45.00		0.03	

PEAK FLOW (cms)	TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
1.67	6.00	52.40	54.40	0.96
1.10	6.50	15.54	54.40	0.29
2.125 (iii)	6.00	22.17	54.40	0.41
6.00	22.17	54.40	54.40	0.41
15.54	22.17	54.40	54.40	0.41
54.40	54.40	54.40	54.40	0.41
0.96	0.29			

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 64.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5174)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	1.1370	1.3070
0.0580	0.5427	1.4280	1.5354
0.3290	0.6196	1.6620	1.7593
0.6130	0.8627	2.0620	1.7693
0.8510	1.0621	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1741)	56.300	2.125	6.00	22.17
OUTFLOW: ID= 1 (5174)	56.300	0.461	7.50	22.13

PEAK FLOW REDUCTION [Qout/Qin] (%) = 21.71
TIME SHIFT OF PEAK FLOW (min) = 90.00
MAXIMUM STORAGE USED (ha. m.) = 0.7346

CALIB NASHYD (1742)
ID= 1 DT=15.0 min

Area (ha)	Curve Number (CN)
82.80	71.0
5.00	# of Linear Res. (N) = 1.50
0.69	

Unit Hyd Qpeak (cms) = 2.055

PEAK FLOW (cms) = 0.809 (i)
TIME TO PEAK (hrs) = 6.750
RUNOFF VOLUME (mm) = 15.652
TOTAL RAINFALL (mm) = 54.400
RUNOFF COEFFICIENT = 0.288

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8044)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1742):	82.80	0.809	6.75	15.65
+ ID2= 2 (5174):	56.30	0.461	7.50	22.13
ID = 3 (8044):	139.10	1.229	7.25	18.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB (0172) Area (ha)= 202.23 Curve Number (CN)= 78.0
 NASHYD (0172) I a (mm)= 5.00 # of Linear Res. (N)= 1.50
 ID= 1 DT=15.0 min U. H. Tp(hrs)= 0.89

Unit Hyd Qpeak (cms)= 3.875
 PEAK FLOW (cms)= 2.077 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 19.917
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.366

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB (0168) Area (ha)= 247.13 Curve Number (CN)= 52.0
 NASHYD (0168) I a (mm)= 5.00 # of Linear Res. (N)= 1.50
 ID= 1 DT=15.0 min U. H. Tp(hrs)= 0.75

Unit Hyd Qpeak (cms)= 5.611
 PEAK FLOW (cms)= 1.186 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 8.463
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.156

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB (0170) Area (ha)= 166.91 Curve Number (CN)= 65.0
 NASHYD (0170) I a (mm)= 5.00 # of Linear Res. (N)= 1.50
 ID= 1 DT=15.0 min U. H. Tp(hrs)= 0.80

Unit Hyd Qpeak (cms)= 3.553
 PEAK FLOW (cms)= 1.182 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 12.923
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.238

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8040)
 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 ID1= 1 (0168): 247.13 1.186 7.00 8.46
 + ID2= 2 (0170): 166.91 1.182 7.00 12.92

 ID = 3 (8040): 414.04 2.368 7.00 10.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8042)
 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 ID1= 1 (0172): 202.23 2.077 7.00 19.92
 + ID2= 2 (8040): 414.04 2.368 7.00 10.26

 ID = 3 (8042): 616.27 4.446 7.00 13.43

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6174)
 IN= 2----> OUT= 1
 Routing time step (min)'= 15.00

 <----- DATA FOR SECTION (1741.0) ----->
 Distance Elevation Manning

0.00	296.00	0.0450
13.36	288.94	0.0450
26.71	288.64	0.0450
51.19	287.96	0.0450
73.45	287.32	0.0450
86.81	286.48	0.0450
102.39	285.08	0.0450
110.73	282.76	0.0450
115.73	281.06	0.0450
115.74	280.35	0.0450 / 0.0350
120.73	280.35	0.0350
120.74	281.10	0.0350 / 0.0450
125.74	282.64	0.0450
135.78	286.87	0.0450
149.13	289.58	0.0450
162.49	292.29	0.0450
175.84	293.95	0.0450
195.88	295.13	0.0450
213.68	296.02	0.0450
220.36	296.09	0.0450

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.71	281.06	.136E+05	6.0	1.69	37.89
1.50	281.85	.358E+05	22.5	2.42	26.43
2.28	282.63	.726E+05	53.8	2.85	22.47
3.07	283.42	.124E+06	104.1	3.24	19.79
3.86	284.21	.189E+06	176.7	3.60	17.81
4.64	284.99	.268E+06	274.9	3.94	16.26
5.43	285.78	.369E+06	390.8	4.07	15.73
6.21	286.56	.501E+06	551.6	4.23	15.15
7.00	287.35	.674E+06	750.5	4.28	14.97
7.79	288.14	.920E+06	993.3	4.15	15.44
8.57	288.92	.126E+07	1361.5	4.14	15.47
9.36	289.71	.167E+07	1986.0	4.56	14.04
10.15	290.50	.210E+07	2739.7	5.02	12.76
10.93	291.28	.254E+07	3615.0	5.47	11.70
11.72	292.07	.300E+07	4608.8	5.91	10.83
12.50	292.85	.347E+07	5693.3	6.30	10.16
13.29	293.64	.397E+07	6892.9	6.67	9.60
14.08	294.43	.450E+07	8150.4	6.97	9.19
14.86	295.21	.506E+07	9525.0	7.23	8.86

<---- hydrograph ----> <-pi pe / channel-->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8042)	616.27	4.45	7.00	13.43	0.53	1.69
OUTFLOW: ID= 1 (6174)	616.27	4.08	8.00	13.43	0.48	1.69

 ADD HYD (8048)
 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 ID1= 1 (6174): 616.27 4.080 8.00 13.43
 + ID2= 2 (8044): 139.10 1.229 7.25 18.28

 ID = 3 (8048): 755.37 5.226 7.75 14.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8052)
 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 ID1= 1 (8048): 755.37 5.226 7.75 14.32
 + ID2= 2 (8050): 502.06 5.572 6.50 17.42

 ID = 3 (8052): 1257.43 9.382 7.25 15.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8058)
 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 ID1= 1 (8052): 1257.43 9.382 7.25 15.56

+ ID2= 2 (8064): 418.72 5.329 6.50 19.22

 ID = 3 (8058): 1676.15 14.490 7.00 16.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1762) | Area (ha)= 162.00
 ID= 1 DT=15.0 min | Total Imp(%)= 26.00 Dir. Conn.(%)= 11.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 42.12 119.88
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1039.23 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 71.81 13.95
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 14.63 (ii) 38.15 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.07 0.03

PEAK FLOW (cms)= 2.62 2.55 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.50 3.642 (iii)
 RUNOFF VOLUME (mm)= 52.40 12.81 17.17
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.24 0.32

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (1761) | Area (ha)= 48.80
 ID= 1 DT=15.0 min | Total Imp(%)= 50.00 Dir. Conn.(%)= 21.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 24.40 24.40
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 570.38 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 71.81 23.46
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 10.21 (ii) 29.31 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.04

PEAK FLOW (cms)= 1.72 1.08 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.25 2.381 (iii)
 RUNOFF VOLUME (mm)= 52.40 16.11 23.73
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.30 0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5176)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.2640	1.3175
0.1650	0.6418	4.0090	1.4908
0.9450	0.7489	4.7720	1.6604
1.7610	0.9426	5.1720	1.6704

2.4420 1.1144 | 0.0000 0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1761)	48.800	2.381	6.00	23.73
OUTFLOW: ID= 1 (5176)	48.800	0.508	7.25	23.71

PEAK FLOW REDUCTION [Qout/Qin] (%) = 21.34
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha.m.) = 0.6909

ADD HYD (8056)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1762):	162.00	3.642	6.00	17.17
+ ID2= 2 (5176):	48.80	0.508	7.25	23.71

ID = 3 (8056):	210.80	3.717	6.00	18.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8066)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8056):	210.80	3.717	6.00	18.68
+ ID2= 2 (8058):	1676.15	14.490	7.00	16.47

ID = 3 (8066):	1886.95	17.581	6.50	16.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8070)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8066):	1886.95	17.581	6.50	16.72
+ ID2= 2 (8068):	660.32	24.224	6.00	22.13

ID = 3 (8070):	2547.27	36.045	6.00	18.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6190)
 IN= 2--> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1901.0) ----->

Distance	Elevation	Manning
0.00	261.34	0.0600
19.76	261.64	0.0600
59.28	261.29	0.0600
79.04	261.09	0.0600
103.74	255.99	0.0600
153.13	253.42	0.0600
192.65	252.78	0.0600
212.29	249.80	0.0600
217.29	249.50	0.0600 / 0.0350
217.30	249.05	0.0350
222.29	249.05	0.0350
222.30	249.50	0.0350 / 0.0600
227.30	249.80	0.0600
232.17	252.31	0.0600
276.63	253.53	0.0600
326.03	256.97	0.0600
355.67	257.30	0.0600
400.12	260.08	0.0600
469.28	261.25	0.0600
489.04	262.53	0.0600

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.45	249.50	.436E+04	1.0	0.43	75.97
1.07	250.12	.205E+05	5.4	0.51	62.96
1.70	250.75	.451E+05	14.8	0.64	50.78

2.32	251.37	.762E+05	29.2	0.74	43.46
2.94	251.99	.114E+06	49.1	0.84	38.61
3.57	252.62	.161E+06	70.2	0.85	38.18
4.19	253.24	.248E+06	93.4	0.73	44.29
4.81	253.86	.398E+06	154.9	0.76	42.79
5.44	254.49	.576E+06	246.1	0.83	38.99
6.06	255.11	.779E+06	362.7	0.90	35.79
6.68	255.73	.101E+07	506.5	0.98	33.15
7.30	256.35	.126E+07	687.6	1.06	30.52
7.93	256.98	.153E+07	902.8	1.15	28.18
8.55	257.60	.183E+07	1114.3	1.18	27.39
9.17	258.22	.216E+07	1407.8	1.26	25.55
9.80	258.85	.250E+07	1735.4	1.35	24.03
10.42	259.47	.286E+07	2097.5	1.42	22.73
11.04	260.09	.324E+07	2492.3	1.49	21.63
11.67	260.72	.364E+07	2837.8	1.51	21.39

		<---- hydrograph ---->			<-pi pe / channel -->		
	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL	
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW :	ID= 2 (8070)	2547.27	36.05	6.00	18.12	2.53	0.77
OUTFLOW :	ID= 1 (6190)	2547.27	24.96	6.75	18.12	2.14	0.71

CALIB		Area (ha)= 13.49		Di r. Conn. (%)= 31.00	
STANDHYD (1901)		Total Imp(%)= 58.00			
ID= 1 DT=15.0 min					

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	7.82	5.67
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	299.89	40.00
Mannings n	=	0.013	0.250

Max. Eff. Inten. (mm/hr)=	71.81	57.43
over (min)	15.00	30.00
Storage Coeff. (min)=	6.94 (ii)	20.29 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.10	0.05

TOTALS		
PEAK FLOW (cms)=	0.77	0.45
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	52.40	22.73
TOTAL RAINFALL (mm)=	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.42
		1.060 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5190)		OUTFLOW		STORAGE	
IN= 2--> OUT= 1		(cms)		(ha.m.)	
DT= 15.0 min					
	OUTFLOW	STORAGE	OUTFLOW	STORAGE	
	(cms)	(ha.m.)	(cms)	(ha.m.)	
	0.0000	0.0000	0.9030	0.3849	
	0.0460	0.1975	1.1330	0.4289	
	0.2610	0.2331	1.3190	0.4720	
	0.4870	0.2836	1.7190	0.4820	
	0.6750	0.3308	0.0000	0.0000	

	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
INFLOW :	ID= 2 (1901)	13.490	1.060	6.00	31.92
OUTFLOW :	ID= 1 (5190)	13.490	0.288	6.75	31.85

PEAK FLOW REDUCTION [Oout/Oi n] (%) = 27.20
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha.m.) = 0.2403

CALIB		Area (ha)= 274.50		Di r. Conn. (%)= 28.00	
STANDHYD (1902)		Total Imp(%)= 52.00			
ID= 1 DT=15.0 min					

		IMPERVIOUS	PERVIOUS (i)	
Surface Area	(ha)=	142.74	131.76	
Dep. Storage	(mm)=	2.00	5.00	
Average Slope	(%)=	0.50	0.50	
Length	(m)=	1352.77	40.00	
Mannings n	=	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	71.81	49.29		
over (min)	15.00	45.00		
Storage Coeff. (min)=	17.14 (ii)	31.33 (ii)		
Unit Hyd. Tpeak (min)=	15.00	45.00		
Unit Hyd. peak (cms)=	0.07	0.03		
PEAK FLOW (cms)=	10.52	6.69		*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.50		13.367 (iii)
RUNOFF VOLUME (mm)=	52.40	21.39		6.00
TOTAL RAINFALL (mm)=	54.40	54.40		30.07
RUNOFF COEFFICIENT =	0.96	0.39		54.40
				0.55

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD. (9190)		AREA		OPEAK		TPEAK		R. V.	
1 + 2 = 3		(ha)		(cms)		(hrs)		(mm)	
ID1= 1 (1902):	274.50	13.367	6.00	30.07					
+ ID2= 2 (5190):	13.49	0.288	6.75	31.85					
ID = 3 (9190):	287.99	13.396	6.00	30.16					

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD. (8072)		AREA		OPEAK		TPEAK		R. V.	
1 + 2 = 3		(ha)		(cms)		(hrs)		(mm)	
ID1= 1 (6190):	2547.27	24.957	6.75	18.12					
+ ID2= 2 (9190):	287.99	13.396	6.00	30.16					
ID = 3 (8072):	2835.26	33.820	6.50	19.35					

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6192)		Routing time step (min)'= 15.00	
IN= 2--> OUT= 1			

<----- DATA FOR SECTION (1921.0) ----->			
Distance	Elevation	Manning	
0.00	264.40	0.0500	
3.64	263.94	0.0500	
14.54	261.05	0.0500	
29.08	256.78	0.0500	
36.36	254.96	0.0500	
50.90	252.20	0.0500	
65.44	249.78	0.0500	
76.35	246.86	0.0500	
77.24	245.90	0.0500	
82.24	245.45	0.0500 / 0.0350	Main Channel
82.25	244.90	0.0350	Main Channel
87.25	244.90	0.0350	Main Channel
87.26	245.45	0.0350 / 0.0500	Main Channel
94.53	246.89	0.0500	
170.87	247.16	0.0500	
218.13	248.69	0.0500	
265.40	249.22	0.0500	
301.75	250.78	0.0500	
338.11	253.47	0.0500	
359.92	264.00	0.0500	

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)
0.55	245.45	345E+04	2.0	0.71	29.20

1.53	246.43	.174E+05	14.6	1.05	19.92
2.50	247.40	.767E+05	52.8	0.86	24.20
3.48	248.38	.224E+06	199.8	1.11	18.71
4.46	249.36	.429E+06	441.2	1.29	16.19
5.43	250.33	.691E+06	874.3	1.58	13.18
6.41	251.31	.987E+06	1468.3	1.86	11.20
7.38	252.28	.131E+07	2224.8	2.13	9.79
8.36	253.26	.165E+07	3132.4	2.37	8.78
9.34	254.24	.201E+07	4256.4	2.65	7.87
10.31	255.21	.238E+07	5551.0	2.92	7.15
11.29	256.19	.276E+07	6996.8	3.17	6.57
12.27	257.17	.314E+07	8585.9	3.42	6.10
13.24	258.14	.353E+07	10315.9	3.65	5.71
14.22	259.12	.393E+07	12180.7	3.87	5.38
15.19	260.09	.434E+07	14178.2	4.09	5.10
16.17	261.07	.475E+07	16306.5	4.29	4.85
17.15	262.05	.517E+07	18558.6	4.49	4.64
18.12	263.02	.559E+07	20939.7	4.68	4.45

<--- hydrograph --->						<--- pi pe / channel --->	
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL		
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW : ID= 2 (8072)	2835.26	33.82	6.50	19.35	2.02	0.94	
OUTFLOW : ID= 1 (6192)	2835.26	29.96	7.00	19.35	1.92	0.96	

CALIB STANDHYD (1921)	Area (ha)= 72.27	Total Imp(%)= 42.00	Di r. Conn.(%)= 19.00
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Surface Area	(ha)= 30.35	PERVIOUS (i) 41.92
Dep. Storage	(mm)= 2.00	5.00
Average Slope	(%)= 0.50	0.50
Length	(m)= 694.12	40.00
Mannings n	= 0.013	0.250
Max. Eff. Inten. (mm/hr)=	71.81	17.51
over (min)	15.00	45.00
Storage Coeff. (min)=	11.48 (ii)	32.96 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03
PEAK FLOW (cms)=	2.21	1.21
TIME TO PEAK (hrs)=	6.00	6.00
RUNOFF VOLUME (mm)=	52.40	13.78
TOTAL RAINFALL (mm)=	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.25

TOTALS
2.705 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 57.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5192)	IN= 2--> OUT= 1	DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
			0.0000	0.0000	3.1390	1.4346
			0.2060	0.7072	3.8730	1.6637
			0.9550	0.8170	4.8310	1.9184
			1.7940	1.0081	5.2310	1.9284
			2.4250	1.1871	0.0000	0.0000
INFLOW : ID= 2 (1921)	72.270	2.705	6.00	21.11		
OUTFLOW : ID= 1 (5192)	72.270	0.842	7.00	21.10		

PEAK FLOW REDUCTION [Qout/Qin] (%) = 31.12
TIME SHIFT OF PEAK FLOW (min) = 60.00
MAXIMUM STORAGE USED (ha.m.) = 0.8055

CALIB STANDHYD (1922)	Area (ha)= 174.34	Total Imp(%)= 39.00	Di r. Conn.(%)= 18.00
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Surface Area	(ha)= 67.99	PERVIOUS (i) 106.35
Dep. Storage	(mm)= 2.00	5.00
Average Slope	(%)= 0.50	0.50
Length	(m)= 1078.08	40.00
Mannings n	= 0.013	0.250
Max. Eff. Inten. (mm/hr)=	71.81	16.27
over (min)	15.00	45.00
Storage Coeff. (min)=	14.95 (ii)	37.07 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.07	0.03
PEAK FLOW (cms)=	4.57	2.68
TIME TO PEAK (hrs)=	6.00	6.00
RUNOFF VOLUME (mm)=	52.40	13.33
TOTAL RAINFALL (mm)=	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.25

TOTALS
5.653 (iii)
6.00
20.37
54.40
0.37

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 57.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8074)	1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1922):		174.34	5.653	6.00	20.37
+ ID2= 2 (5192):		72.27	0.842	7.00	21.10
ID = 3 (8074):		246.61	5.753	6.00	20.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8076)	1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6192):		2835.26	29.957	7.00	19.35
+ ID2= 2 (8074):		246.61	5.753	6.00	20.58
ID = 3 (8076):		3081.87	33.114	7.00	19.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0196)	Area (ha)= 41.47	Curve Number (CN)= 70.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50
	U.H. Tp(hrs)= 0.24	

Unit Hyd Qpeak (cms)=	2.950
PEAK FLOW (cms)=	0.828 (i)
TIME TO PEAK (hrs)=	6.000
RUNOFF VOLUME (mm)=	14.154
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.260

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1941)	Area (ha)= 0.76	Total Imp(%)= 45.00	Di r. Conn.(%)= 15.00
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Surface Area	(ha)= 0.34	PERVIOUS (i) 0.42
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Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 71.18 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 71.81 47.62
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 2.93 (ii) 17.32 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.11 0.05

TOTALS
 PEAK FLOW (cms)= 0.02 0.03 0.041 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 52.40 20.15 24.97
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.37 0.46

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 67.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5194)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0510	0.0197
0.0030	0.0093	0.0640	0.0226
0.0150	0.0107	0.0740	0.0253
0.0270	0.0138	0.0740	0.0353
0.0380	0.0165	0.0000	0.0000

INFLOW : ID= 2 (1941) AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 OUTFLOW: ID= 1 (5194) 0.760 0.041 6.00 24.97
 0.760 0.012 6.75 23.96

PEAK FLOW REDUCTION [Qout/Qin] (%) = 29.90
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0104

CALIB NASHYD (1942)
 ID= 1 DT=15.0 min

Area (ha) = 255.33 Curve Number (CN) = 69.5
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp (hrs) = 0.68

Unit Hyd Opeak (cms) = 6.448

PEAK FLOW (cms) = 2.401 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 14.894
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.274

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9194)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1 = 1 (1942):	255.33	2.401	6.75	14.89
+ ID2 = 2 (5194):	0.76	0.012	6.75	23.96
ID = 3 (9194):	256.09	2.414	6.75	14.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8078)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1 = 1 (1942):	255.33	2.401	6.75	14.89
+ ID2 = 2 (5194):	0.76	0.012	6.75	23.96
ID = 3 (9194):	256.09	2.414	6.75	14.92

ID1 = 1 (0196): 41.47 0.828 6.00 14.15
 + ID2 = 2 (9194): 256.09 2.414 6.75 14.92

 ID = 3 (8078): 297.56 3.066 6.50 14.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6198)
 IN= 2--> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (1981.0) ----->

Distance	Elevation	Manning
0.00	267.15	0.0500
22.99	265.94	0.0500
51.73	261.39	0.0500
74.72	258.75	0.0500
94.83	257.79	0.0500
114.95	254.36	0.0500
126.44	254.06	0.0500
132.19	253.68	0.0500
135.06	253.35	0.0500
137.94	252.93	0.0500 / 0.0300
140.81	252.41	0.0300
143.69	251.89	0.0300
146.56	252.51	0.0300
149.43	253.36	0.0300 / 0.0500
158.05	255.89	0.0500
172.42	256.25	0.0500
204.03	259.15	0.0500
229.90	260.36	0.0500
252.89	260.46	0.0500
284.50	260.45	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.35	252.24	.111E+04	0.6	0.97	31.07
0.69	252.58	.442E+04	3.8	1.55	19.42
1.04	252.93	.974E+04	11.1	2.06	14.65
1.51	253.40	.203E+05	31.2	2.77	10.87
1.98	253.87	.357E+05	65.0	3.30	9.14
2.45	254.34	.596E+05	112.9	3.43	8.80
2.92	254.81	.935E+05	186.8	3.62	8.34
3.39	255.28	.131E+06	282.1	3.89	7.75
3.86	255.75	.172E+06	398.2	4.18	7.22
4.33	256.22	.221E+06	527.0	4.31	6.99
4.80	256.69	.282E+06	697.3	4.47	6.75
5.27	257.16	.350E+06	898.5	4.64	6.50
5.74	257.63	.425E+06	1131.4	4.82	6.26
6.21	258.10	.508E+06	1367.4	4.88	6.19
6.68	258.57	.603E+06	1635.6	4.91	6.14
7.15	259.04	.709E+06	1977.2	5.04	5.98
7.62	259.51	.826E+06	2360.5	5.17	5.83
8.09	259.98	.954E+06	2790.7	5.29	5.70
8.56	260.45	.110E+07	3179.6	5.25	5.74

<---- hydrograph ----> <- pipe / channel ->
 INFLOW : ID= 2 (8078) AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm) MAX DEPTH (m) MAX VEL (m/s)
 OUTFLOW: ID= 1 (6198) 297.56 3.07 6.50 14.81 0.61 1.37
 297.56 2.87 7.00 14.81 0.59 1.32

CALIB STANDHYD (1981)
 ID= 1 DT=15.0 min

Area (ha) = 149.95
 Total Imp (%) = 44.00 Di r. Conn. (%) = 19.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	65.98	83.97
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	999.83	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 71.81 18.96
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 14.29 (ii) 35.10 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.07 0.03

TOTALS

PEAK FLOW (cms) = 4.22 2.54 5.263 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 52.40 14.36 21.59
 TOTAL RAINFALL (mm) = 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.26 0.40

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PEROUS LOSSES:
 CN* = 57.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5198)				
IN= 2--> OUT= 1				
DT= 15.0 min				
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)	R. V. (mm)
0.0000	0.0000	10.9910	3.4957	
0.4390	1.7800	13.3530	3.9512	
2.6180	2.0449	15.3540	4.3979	
5.1680	2.5932	15.7540	4.4079	
7.9230	3.0040	0.0000	0.0000	

INFLOW : ID= 2 (1981)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
149.950	149.950	5.263	6.00	21.59
OUTFLOW: ID= 1 (5198)	149.950	1.401	7.50	21.58

PEAK FLOW REDUCTION [Qout/Qin] (%) = 26.61
 TIME SHIFT OF PEAK FLOW (min) = 90.00
 MAXIMUM STORAGE USED (ha.m.) = 1.9012

CALIB STANDHYD (1982)			
ID= 1 DT=15.0 min			
Area (ha)	Imp (%)	Dir. Conn. (%)	PERVIOUS (i)
240.61	20.00	9.00	
Surface Area (ha) = 48.12			192.49
Dep. Storage (mm) = 2.00			5.00
Average Slope (%) = 0.50			0.50
Length (m) = 1266.52			40.00
Mannings n = 0.013			0.250
Max. Eff. Inten. (mm/hr) = 71.81			11.88
over (min) = 15.00			45.00
Storage Coeff. (min) = 16.47 (ii)			41.55 (ii)
Unit Hyd. Tpeak (min) = 15.00			45.00
Unit Hyd. peak (cms) = 0.07			0.03
TOTALS			
PEAK FLOW (cms) = 3.02			3.34
TIME TO PEAK (hrs) = 6.00			6.50
RUNOFF VOLUME (mm) = 52.40			11.63
TOTAL RAINFALL (mm) = 54.40			54.40
RUNOFF COEFFICIENT = 0.96			0.28

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PEROUS LOSSES:
 CN* = 57.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8080)				
1 + 2 = 3				
ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1982):	240.61	4.345	6.00	15.30
+ ID2= 2 (5198):	149.95	1.401	7.50	21.58
ID = 3 (8080):	390.56	4.663	6.50	17.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8082)				
1 + 2 = 3				
ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6198):	297.56	2.874	7.00	14.81
+ ID2= 2 (8080):	390.56	4.663	6.50	17.71
ID = 3 (8082):	688.12	7.074	6.50	16.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8084)				
1 + 2 = 3				
ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8076):	3081.87	33.114	7.00	19.44
+ ID2= 2 (8082):	688.12	7.074	6.50	16.46
ID = 3 (8084):	3769.99	39.558	6.75	18.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6200)		Routing time step (min) = 15.00	
IN= 2--> OUT= 1			

<----- DATA FOR SECTION (2001.0) ----->		
Distance	Elevation	Manning
0.00	252.47	0.0500
10.38	252.29	0.0500
20.76	252.28	0.0500
41.52	247.66	0.0500
62.28	247.47	0.0500
80.45	247.34	0.0500
90.83	246.85	0.0500
103.81	245.51	0.0500
106.00	244.50	0.0500
108.99	244.05	0.0500 / 0.0300
109.00	243.50	0.0300
114.00	243.50	0.0300
114.01	244.05	0.0300 / 0.0500
118.99	244.50	0.0500
132.36	245.09	0.0500
150.52	245.83	0.0500
199.83	249.24	0.0500
207.62	250.16	0.0500
236.16	253.11	0.0500
256.92	253.83	0.0500

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.28	243.77	.365E+04	0.7	0.54	81.42
0.55	244.05	.730E+04	2.2	0.81	54.59
1.05	244.55	.197E+05	7.2	0.97	45.51
1.54	245.04	.463E+05	17.2	0.99	44.76
2.04	245.54	.895E+05	34.8	1.03	42.81
2.53	246.03	.152E+06	62.8	1.09	40.43
3.03	246.53	.232E+06	104.7	1.20	36.91
3.52	247.02	.328E+06	159.7	1.29	34.19
4.02	247.52	.450E+06	223.4	1.32	33.56
4.51	248.01	.627E+06	326.9	1.38	31.99
5.01	248.51	.820E+06	463.0	1.50	29.52
5.50	249.00	.103E+07	626.0	1.62	27.30
6.00	249.50	.124E+07	820.1	1.75	25.24
6.49	249.99	.147E+07	1044.4	1.89	23.42
6.99	250.49	.170E+07	1292.6	2.01	21.95
7.48	250.98	.195E+07	1565.6	2.13	20.71
7.98	251.48	.220E+07	1864.5	2.25	19.65
8.47	251.97	.246E+07	2189.4	2.36	18.73
8.97	252.47	.274E+07	2427.5	2.35	18.80

		<---- hydrograph ---->		<-pi pe / channel ->	
INFLOW : ID= 2 (8084)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)
3769.99	3769.99	39.56	6.75	18.90	2.12
OUTFLOW: ID= 1 (6200)	3769.99	33.78	7.75	18.90	1.03

 CALIB
 STANDHYD (2001)
 ID= 1 DT=15.0 min

Area (ha)= 42.48
 Total Imp(%)= 46.00 Dir. Conn.(%)= 19.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	19.54	22.94
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	532.17	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	71.81	49.58
over (min)	15.00	30.00
Storage Coeff. (min)=	9.79 (ii)	23.95 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.04

PEAK FLOW (cms)=	1.37	1.47	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	2.291 (iii)
RUNOFF VOLUME (mm)=	52.40	21.51	6.00
TOTAL RAINFALL (mm)=	54.40	54.40	27.38
RUNOFF COEFFICIENT =	0.96	0.40	54.40
			0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.7 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR (5200)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.7210	1.0525
0.1450	0.4892	3.4130	1.1947
0.7930	0.5722	4.0030	1.3346
1.4410	0.7540	4.4030	1.3446
2.0370	0.8927	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2001)	42.480	2.291	6.00	27.38
OUTFLOW: ID= 1 (5200)	42.480	0.843	6.75	27.36

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 36.81
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 0.5882

 CALIB
 STANDHYD (2002)
 ID= 1 DT=15.0 min

Area (ha)= 72.79
 Total Imp(%)= 31.00 Dir. Conn.(%)= 13.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	22.56	50.23
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	696.61	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	71.81	22.44
over (min)	15.00	45.00
Storage Coeff. (min)=	11.51 (ii)	30.95 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03

PEAK FLOW (cms)=	1.52	1.90	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.50	2.315 (iii)
RUNOFF VOLUME (mm)=	52.40	18.99	6.00
TOTAL RAINFALL (mm)=	54.40	54.40	23.34
RUNOFF COEFFICIENT =	0.96	0.35	54.40
			0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.7 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8086)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2002):	72.79	2.315	6.00	23.34
+ ID2= 2 (5200):	42.48	0.843	6.75	27.36

ID = 3 (8086):	115.27	2.914	6.50	24.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8088)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6200):	3769.99	33.784	7.75	18.90
+ ID2= 2 (8086):	115.27	2.914	6.50	24.82

ID = 3 (8088):	3885.26	34.988	7.50	19.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8038)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8034):	5220.76	11.749	12.25	11.56
+ ID2= 2 (8088):	3885.26	34.988	7.50	19.07

ID = 3 (8038):	9106.02	46.380	7.50	14.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (1365)
 IN= 2---> OUT= 1

<----- DATA FOR SECTION (1362.0) ----->

Distance	Elevation	Manning
0.00	252.00	0.0400
2.35	244.60	0.0400
39.94	243.14	0.0400
58.74	242.65	0.0400
91.63	242.03	0.0400
101.03	241.69	0.0400
103.38	241.66	0.0400
105.73	240.76	0.0400
105.93	240.59	0.0400
109.18	240.28	0.0400 / 0.0300
110.43	239.81	0.0300
111.68	239.81	0.0300
113.43	240.09	0.0300 / 0.0400
118.43	240.50	0.0400
138.62	241.68	0.0400
152.72	241.60	0.0400
166.82	242.68	0.0400
185.62	249.09	0.0400
197.36	251.00	0.0400
232.61	252.28	0.0400

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.28	240.09	.118E+04	0.1	0.20	142.09
0.91	240.72	.114E+05	2.4	0.35	79.92
1.53	241.34	.352E+05	9.4	0.45	62.17
2.16	241.97	.831E+05	22.8	0.46	60.69
2.79	242.60	.172E+06	54.2	0.53	52.74
3.41	243.22	.297E+06	111.4	0.63	44.49
4.04	243.85	.445E+06	194.1	0.74	38.22

4.67	244.48	.612E+06	301.1	0.83	33.88
5.29	245.10	.793E+06	443.0	0.94	29.82
5.92	245.73	.976E+06	611.3	1.06	26.60
6.55	246.36	.116E+07	802.0	1.17	24.12
7.18	246.99	.135E+07	1014.0	1.27	22.16
7.80	247.61	.154E+07	1246.2	1.37	20.56
8.43	248.24	.173E+07	1498.0	1.46	19.24
9.06	248.87	.192E+07	1768.8	1.55	18.12
9.69	249.49	.212E+07	2049.6	1.63	17.24
10.31	250.12	.232E+07	2344.7	1.71	16.49
10.94	250.75	.252E+07	2658.5	1.78	15.83
11.56	251.37	.274E+07	2931.2	1.81	15.56

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8038)	9106.02	46.38	7.50	14.76	2.63	0.51
OUTFLOW: ID= 1 (1365)	9106.02	38.34	8.50	14.76	2.46	0.50

CALIB STANDHYD (1321) ID= 1 DT=15.0 min	Area Total	(ha)= 208.90 Imp(%)= 67.00	Dir. Conn. (%)= 46.00
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	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	139.96	68.94
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1180.11	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	71.81	47.63
over (min)	15.00	45.00
Storage Coeff. (min)=	15.79 (ii)	30.18 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.07	0.03

PEAK FLOW (cms)=	13.65	3.44	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.50	15.108 (iii)
RUNOFF VOLUME (mm)=	52.40	19.12	34.43
TOTAL RAINFALL (mm)=	54.40	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.35	0.63

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5132) IN= 2----> OUT= 1 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	7.9970	7.9437
0.9210	5.2645	12.4650	8.5839
3.2690	5.3433	16.9060	9.2236
6.3980	5.6925	17.3060	9.2336
7.1860	6.8155	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1321)	208.900	15.108	6.00	34.43
OUTFLOW: ID= 1 (5132)	208.900	1.371	8.25	34.42

PEAK FLOW REDUCTION [Qout/Qin] (%)=	9.07
TIME SHIFT OF PEAK FLOW (min)=	135.00
MAXIMUM STORAGE USED (ha.m.)=	5.2809

CALIB STANDHYD (1322) ID= 1 DT=15.0 min	Area Total	(ha)= 81.94 Imp(%)= 37.00	Dir. Conn. (%)= 25.00
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	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	30.32	51.62
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	739.10	40.00

Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)=	71.81	16.24
over (min)	15.00	45.00
Storage Coeff. (min)=	11.92 (ii)	34.06 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03

PEAK FLOW (cms)=	3.26	1.36	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.50	3.807 (iii)
RUNOFF VOLUME (mm)=	52.40	14.90	6.00
TOTAL RAINFALL (mm)=	54.40	54.40	24.27
RUNOFF COEFFICIENT =	0.96	0.27	54.40
			0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	(8090)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1322):		81.94	3.807	6.00	24.27
+ ID2= 2 (5132):		208.90	1.371	8.25	34.42
ID = 3 (8090):		290.84	4.178	6.00	31.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1342) ID= 1 DT=15.0 min	Area Total	(ha)= 97.16 Imp(%)= 41.00	Dir. Conn. (%)= 28.00
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	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	39.84	57.32
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	804.82	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	71.81	12.45
over (min)	15.00	45.00
Storage Coeff. (min)=	12.55 (ii)	37.17 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03

PEAK FLOW (cms)=	4.24	1.11	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.50	4.687 (iii)
RUNOFF VOLUME (mm)=	52.40	11.36	6.00
TOTAL RAINFALL (mm)=	54.40	54.40	22.85
RUNOFF COEFFICIENT =	0.96	0.21	54.40
			0.42

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 54.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1341) ID= 1 DT=15.0 min	Area Total	(ha)= 58.79 Imp(%)= 30.00	Dir. Conn. (%)= 20.00
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	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	17.64	41.15
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	626.05	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	71.81	10.92
over (min)	15.00	45.00
Storage Coeff. (min)=	10.79 (ii)	36.74 (ii)

Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.09 0.03

TOTALS
 PEAK FLOW (cms)= 1.93 0.70 2.213 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 52.40 10.69 19.03
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.20 0.35

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 54.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5134)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.1250	1.0755
0.1070	0.6119	1.7570	1.1641
0.4130	0.6225	2.3860	1.2526
0.8380	0.7018	2.7860	1.2626
0.9790	0.8884	0.0000	0.0000

INFLOW: ID= 2 (1341)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
58.790	58.790	2.213	6.00	19.03
OUTFLOW: ID= 1 (5134)	58.790	0.551	7.25	19.01

PEAK FLOW REDUCTION [Qout/Qin] (%) = 24.89
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha.m.) = 0.6507

ADD HYD (9134)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1342):	97.16	4.687	6.00	22.85
+ ID2= 2 (5134):	58.79	0.551	7.25	19.01
ID = 3 (9134):	155.95	4.738	6.00	21.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8092)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8090):	290.84	4.178	6.00	31.56
+ ID2= 2 (9134):	155.95	4.738	6.00	21.40
ID = 3 (8092):	446.79	8.916	6.00	28.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (1366)
 IN= 2--> OUT= 1
 Routing time step (min)' = 15.00

----- DATA FOR SECTION (1361.0) ----->

Distance	Elevation	Manning
0.00	262.81	0.0400
37.23	260.54	0.0400
79.12	256.98	0.0400
111.70	254.74	0.0400
172.20	252.76	0.0400
251.32	248.82	0.0400
255.97	248.75	0.0400
260.43	248.76	0.0400
265.43	248.49	0.0400
268.68	248.18	0.0400 / 0.0300
269.93	247.71	0.0300

Main Channel
 Main Channel

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.47	248.18	881E+04	1.3	0.76	114.66
1.19	248.90	.742E+05	17.3	1.22	71.44
1.92	249.63	.278E+06	78.3	1.48	59.26
2.64	250.35	.589E+06	210.3	1.87	46.69
3.36	251.07	.974E+06	414.3	2.23	39.19
4.08	251.79	.143E+07	696.2	2.55	34.32
4.81	252.52	.197E+07	1063.0	2.83	30.86
5.53	253.24	.259E+07	1505.7	3.06	28.62
6.25	253.96	.331E+07	2051.3	3.26	26.85
6.97	254.68	.413E+07	2720.9	3.46	25.30
7.70	255.41	.504E+07	3584.5	3.73	23.43
8.42	256.13	.601E+07	4580.9	4.00	21.87
9.14	256.85	.704E+07	5707.9	4.25	20.57
9.86	257.57	.818E+07	6636.1	4.26	20.53
10.59	258.30	.945E+07	8115.0	4.50	19.42
11.31	259.02	1.08E+08	9759.2	4.74	18.45
12.03	259.74	.122E+08	11569.5	4.96	17.61
12.75	260.46	.137E+08	13550.6	5.18	16.87
13.48	261.19	.153E+08	15636.4	5.37	16.29

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.47	248.18	881E+04	1.3	0.76	114.66
1.19	248.90	.742E+05	17.3	1.22	71.44
1.92	249.63	.278E+06	78.3	1.48	59.26
2.64	250.35	.589E+06	210.3	1.87	46.69
3.36	251.07	.974E+06	414.3	2.23	39.19
4.08	251.79	.143E+07	696.2	2.55	34.32
4.81	252.52	.197E+07	1063.0	2.83	30.86
5.53	253.24	.259E+07	1505.7	3.06	28.62
6.25	253.96	.331E+07	2051.3	3.26	26.85
6.97	254.68	.413E+07	2720.9	3.46	25.30
7.70	255.41	.504E+07	3584.5	3.73	23.43
8.42	256.13	.601E+07	4580.9	4.00	21.87
9.14	256.85	.704E+07	5707.9	4.25	20.57
9.86	257.57	.818E+07	6636.1	4.26	20.53
10.59	258.30	.945E+07	8115.0	4.50	19.42
11.31	259.02	1.08E+08	9759.2	4.74	18.45
12.03	259.74	.122E+08	11569.5	4.96	17.61
12.75	260.46	.137E+08	13550.6	5.18	16.87
13.48	261.19	.153E+08	15636.4	5.37	16.29

----- hydrograph ----->

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8092)	446.79	8.92	6.00	28.02	0.81
OUTFLOW: ID= 1 (1366)	446.79	2.98	7.25	28.02	0.55

CALIB STANDHYD (1362)
 ID= 1 DT=15.0 min

Area Total (ha) = 371.79
 Imp (%) = 26.00
 Dir. Conn. (%) = 14.00

IMPERVIOUS (ha)	PERVIOUS (i)
96.67	275.12
Dep. Storage (mm) = 2.00	5.00
Average Slope (%) = 0.50	0.50
Length (m) = 1574.36	40.00
Mannings n = 0.013	0.250

Max. Eff. Inten. (mm/hr) = 71.81 15.93
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 18.77 (ii) 41.08 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.06 0.03

TOTALS
 PEAK FLOW (cms) = 6.82 6.41 9.410 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 52.40 14.96 20.20
 TOTAL RAINFALL (mm) = 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.27 0.37

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1361)
 ID= 1 DT=15.0 min

Area Total (ha) = 140.62
 Imp (%) = 55.00
 Dir. Conn. (%) = 29.00

IMPERVIOUS (ha)	PERVIOUS (i)
77.34	63.28
Dep. Storage (mm) = 2.00	5.00
Average Slope (%) = 0.50	0.50

Length (m) = 968.23 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr) = 71.81 45.74
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 14.02 (ii) 28.65 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.07 0.04

TOTALS
 PEAK FLOW (cms) = 6.09 3.40 8.202 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 52.40 19.03 28.71
 TOTAL RAINFALL (mm) = 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.35 0.53

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5136)
 IN= 2---> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.7060	4.3486
0.3100	2.4365	5.9200	5.0324
1.7670	2.5993	7.0820	5.7151
3.4020	2.8510	7.4820	5.7251
4.0310	3.5962	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (1361)	140.620	8.202	6.00	28.71
OUTFLOW: ID= 1 (5136)	140.620	1.719	7.25	28.69

PEAK FLOW REDUCTION [Oout/Oin] (%) = 20.96
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha.m.) = 2.6016

ADD HYD (8096)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1362):	371.79	9.410	6.00	20.20
+ ID2= 2 (5136):	140.62	1.719	7.25	28.69
ID = 3 (8096):	512.41	9.547	6.00	22.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8094)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1366):	446.79	2.982	7.25	28.02
+ ID2= 2 (8096):	512.41	9.547	6.00	22.53
ID = 3 (8094):	959.20	11.796	6.50	25.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8098)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1365):	9106.02	38.337	8.50	14.76
+ ID2= 2 (8094):	959.20	11.796	6.50	25.09
ID = 3 (8098):	10065.22	44.373	8.00	15.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1381)
 ID= 1 DT=15.0 mi n

Area (ha) = 96.24
 Total Imp (%) = 59.00
 Dir. Conn. (%) = 34.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	56.78	39.46
Dep. Storage	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	801.00	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 71.81 8.12
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 12.51 (ii) 41.72 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.08 0.03

TOTALS
 PEAK FLOW (cms) = 5.11 0.47 5.297 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 52.40 5.78 21.63
 TOTAL RAINFALL (mm) = 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.11 0.40

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 28.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5138)
 IN= 2---> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	6.4380	2.7648
0.3260	1.4281	8.0830	3.0750
1.8650	1.6876	9.4100	3.3790
3.4730	2.0447	9.8100	3.3890
4.8170	2.3809	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (1381)	96.240	5.297	6.00	21.63
OUTFLOW: ID= 1 (5138)	96.240	0.447	8.00	21.62

PEAK FLOW REDUCTION [Oout/Oin] (%) = 8.44
 TIME SHIFT OF PEAK FLOW (min) = 120.00
 MAXIMUM STORAGE USED (ha.m.) = 1.4492

CALIB
 NASHYD (1382)
 ID= 1 DT=15.0 mi n

Area (ha) = 318.55
 Ia (mm) = 5.00
 U.H. Tp (hrs) = 1.09
 Curve Number (CN) = 33.2
 # of Linear Res. (N) = 1.50

Unit Hyd Opeak (cms) = 4.976
 PEAK FLOW (cms) = 0.577 (i)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 4.315
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.079

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9138)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1382):	318.55	0.577	7.75	4.32
+ ID2= 2 (5138):	96.24	0.447	8.00	21.62
ID = 3 (9138):	414.79	1.022	8.00	8.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (1402)
ID= 1 DT=15.0 min

Area (ha)= 131.22
Total Imp(%)= 29.00 Dir. Conn.(%)= 21.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	38.05	93.17
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	935.31	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	71.81	4.38
over (min)	15.00	60.00
Storage Coeff. (min)	13.73 (ii)	51.12 (ii)
Unit Hyd. Tpeak (min)	15.00	60.00
Unit Hyd. peak (cms)	0.08	0.02

TOTALS

PEAK FLOW (cms)	4.15	0.65	4.342 (iii)
TIME TO PEAK (hrs)	6.00	6.75	6.00
RUNOFF VOLUME (mm)	52.40	5.85	15.63
TOTAL RAINFALL (mm)	54.40	54.40	54.40
RUNOFF COEFFICIENT	0.96	0.11	0.29

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 37.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (1401)
ID= 1 DT=15.0 min

Area (ha)= 27.37
Total Imp(%)= 45.00 Dir. Conn.(%)= 33.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	12.32	15.05
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	427.16	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	71.81	5.26
over (min)	15.00	45.00
Storage Coeff. (min)	8.58 (ii)	43.32 (ii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.09	0.03

TOTALS

PEAK FLOW (cms)	1.59	0.15	1.645 (iii)
TIME TO PEAK (hrs)	6.00	6.50	6.00
RUNOFF VOLUME (mm)	52.40	6.43	21.60
TOTAL RAINFALL (mm)	54.40	54.40	54.40
RUNOFF COEFFICIENT	0.96	0.12	0.40

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 37.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5140)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.8310	0.7127
0.0930	0.3344	2.2990	0.8148
0.5300	0.3869	2.6760	0.9148
0.9880	0.4994	3.0760	0.9248
1.3700	0.5962	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1401)	27.370	1.645	6.00	21.60
OUTFLOW: ID= 1 (5140)	27.370	0.257	6.75	21.57

PEAK FLOW REDUCTION [Qout/Qin] (%) = 15.66
TIME SHIFT OF PEAK FLOW (min) = 45.00

MAXIMUM STORAGE USED (ha. m.) = 0.3546

ADD HYD (9140)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1402):	131.22	4.342	6.00	15.63
+ ID2= 2 (5140):	27.37	0.257	6.75	21.57
ID = 3 (9140):	158.59	4.401	6.00	16.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8102)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9138):	414.79	1.022	8.00	8.33
+ ID2= 2 (9140):	158.59	4.401	6.00	16.65
ID = 3 (8102):	573.38	4.859	6.00	10.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6142)
IN= 2---> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1421.0) ----->

Distance	Elevation	Manning
0.00	295.50	0.0400
7.25	295.36	0.0400
14.50	295.22	0.0400
21.75	295.22	0.0400
29.00	294.23	0.0400
52.56	286.45	0.0400
67.06	281.44	0.0400
77.94	278.28	0.0400
82.81	277.20	0.0400 / 0.0300
83.21	276.80	0.0300
88.81	276.80	0.0300
96.41	276.80	0.0300
96.81	277.20	0.0300 / 0.0400
103.31	278.96	0.0400
108.75	280.16	0.0400
117.81	282.73	0.0400
135.94	290.30	0.0400
159.50	292.14	0.0400
174.00	293.07	0.0400
179.44	293.37	0.0400

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.40	277.20	.238E+05	6.9	1.27	57.28
1.25	278.05	.889E+05	50.9	2.50	29.13
2.10	278.90	.179E+06	133.4	3.26	22.36
2.95	279.75	.293E+06	256.7	3.83	19.03
3.80	280.60	.432E+06	426.0	4.31	16.90
4.66	281.46	.593E+06	644.3	4.75	15.35
5.51	282.31	.776E+06	915.3	5.16	14.12
6.36	283.16	.978E+06	1242.2	5.55	13.12
7.21	284.01	.120E+07	1626.2	5.94	12.27
8.06	284.86	.143E+07	2065.6	6.30	11.57
8.91	285.71	.169E+07	2562.4	6.64	10.97
9.76	286.56	.196E+07	3118.0	6.97	10.46
10.61	287.41	.224E+07	3733.3	7.28	10.02
11.46	288.26	.255E+07	4411.9	7.57	9.62
12.31	289.11	.287E+07	5155.6	7.86	9.27
13.17	289.97	.321E+07	5966.6	8.14	8.96
14.02	290.82	.357E+07	6705.0	8.22	8.87
14.87	291.67	.398E+07	7494.3	8.24	8.85
15.72	292.52	.444E+07	8405.3	8.28	8.80

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8102)	573.38	4.86	6.00	10.63	0.28	1.27
OUTFLOW: ID= 1 (6142)	573.38	2.15	7.00	10.63	0.12	1.27

CALIB
STANDHYD (1421)
ID= 1 DT=15.0 min

Area (ha)= 69.77
Total Imp(%)= 45.00 Dir. Conn.(%)= 15.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	31.40	38.37
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	682.01	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	71.81	22.51
over (min)	15.00	45.00
Storage Coeff. (min)=	11.36 (ii)	30.79 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03

PEAK FLOW (cms)=	1.69	1.47	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.50	2.300 (iii)
RUNOFF VOLUME (mm)=	52.40	15.83	6.00
TOTAL RAINFALL (mm)=	54.40	54.40	21.31
RUNOFF COEFFICIENT =	0.96	0.29	54.40
			0.39

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5142)
IN= 2----> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.6670	1.8169
0.2360	0.8525	5.8600	2.0773
1.3520	0.9863	6.8220	2.3321
2.5180	1.2732	7.2220	2.3421
3.4920	1.5199	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (1421)	69.770	2.300	6.00	21.31
OUTFLOW: ID= 1 (5142)	69.770	0.572	7.50	21.30

PEAK FLOW REDUCTION [Qout/Oin] (%) = 24.87
TIME SHIFT OF PEAK FLOW (min) = 90.00
MAXIMUM STORAGE USED (ha.m.) = 0.8948

CALIB
NASHYD (1422)
ID= 1 DT=15.0 min

Area (ha)= 326.77 Curve Number (CN)= 61.2
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 1.21

Unit Hyd Opeak (cms)=	4.626
PEAK FLOW (cms)=	1.496 (i)
TIME TO PEAK (hrs)=	8.000
RUNOFF VOLUME (mm)=	11.507
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.212

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9142)
1 + 2 = 3

ID1= 1 (1422):	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
+ ID2= 2 (5142):	326.77	1.496	8.00	11.51
	69.77	0.572	7.50	21.30

ID = 3 (9142): 396.54 2.061 7.75 13.23

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8104)
1 + 2 = 3

ID1= 1 (6142):	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
+ ID2= 2 (9142):	396.54	2.061	7.75	13.23
ID = 3 (8104):	969.92	4.118	7.50	11.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (1441)
ID= 1 DT=15.0 min

Area (ha)= 6.29
Total Imp(%)= 45.00 Dir. Conn.(%)= 15.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	2.83	3.46
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	204.78	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	71.81	50.50
over (min)	15.00	30.00
Storage Coeff. (min)=	5.52 (ii)	19.58 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.11	0.05

PEAK FLOW (cms)=	0.18	0.25	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	0.337 (iii)
RUNOFF VOLUME (mm)=	52.40	21.29	6.00
TOTAL RAINFALL (mm)=	54.40	54.40	25.96
RUNOFF COEFFICIENT =	0.96	0.39	54.40
			0.48

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5144)
IN= 2----> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.4210	0.1639
0.0210	0.0769	0.5290	0.1874
0.1220	0.0890	0.6150	0.2104
0.2270	0.1148	1.0150	0.2204
0.3150	0.1371	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (1441)	6.290	0.337	6.00	25.96
OUTFLOW: ID= 1 (5144)	6.290	0.111	6.75	25.81

PEAK FLOW REDUCTION [Qout/Oin] (%) = 32.96
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha.m.) = 0.0882

CALIB
NASHYD (1442)
ID= 1 DT=15.0 min

Area (ha)= 129.40 Curve Number (CN)= 74.9
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 0.70

Unit Hyd Opeak (cms)=	3.147
PEAK FLOW (cms)=	1.429 (i)
TIME TO PEAK (hrs)=	6.750

RUNOFF VOLUME (mm) = 17.829
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.328

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9144) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1442):	129.40	1.429	6.75	17.83
+ ID2= 2 (5144):	6.29	0.111	6.75	25.81
ID = 3 (9144):	135.69	1.540	6.75	18.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8106) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8104):	969.92	4.118	7.50	11.69
+ ID2= 2 (9144):	135.69	1.540	6.75	18.20
ID = 3 (8106):	1105.61	5.510	7.50	12.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6146)
IN= 2---> OUT= 1
Routing time step (min)' = 15.00

DATA FOR SECTION (1461.0) ----->			
Distance	Elevation	Manning	
0.00	262.00	0.0350	
39.58	257.13	0.0350	
123.69	257.51	0.0350	
183.07	256.83	0.0350	
202.86	255.73	0.0350	
212.75	254.73	0.0350	
222.65	252.86	0.0350	
232.44	250.70	0.0350 / 0.0300	Main Channel
236.44	250.20	0.0300	Main Channel
236.84	249.80	0.0300	Main Channel
242.44	249.80	0.0300	Main Channel
250.04	249.80	0.0300	Main Channel
250.44	250.20	0.0300	Main Channel
252.33	250.57	0.0300 / 0.0350	Main Channel
286.97	253.52	0.0350	
331.50	255.80	0.0350	
380.98	255.55	0.0350	
445.30	258.40	0.0350	
455.19	259.82	0.0350	
489.83	262.22	0.0350	

TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.39	250.19	.169E+05	5.4	1.04	52.00
0.77	250.57	.373E+05	16.6	1.44	37.46
1.44	251.24	.911E+05	60.3	2.14	25.19
2.11	251.91	.169E+06	136.3	2.61	20.64
2.79	252.59	.270E+06	249.7	2.99	18.05
3.46	253.26	.396E+06	405.4	3.31	16.28
4.13	253.93	.549E+06	603.3	3.56	15.15
4.80	254.60	.737E+06	857.8	3.77	14.31
5.48	255.28	.963E+06	1174.6	3.95	13.67
6.15	255.95	.128E+07	1449.5	3.66	14.74
6.82	256.62	.173E+07	2046.4	3.83	14.07
7.49	257.29	.227E+07	2665.2	3.81	14.17
8.17	257.97	.310E+07	3705.2	3.87	13.93
8.84	258.64	.400E+07	5127.4	4.15	12.99
9.51	259.31	.492E+07	6858.6	4.51	11.95
10.18	259.98	.587E+07	8815.5	4.86	11.09
10.86	260.66	.684E+07	10954.6	5.18	10.41
11.53	261.33	.785E+07	13324.3	5.49	9.82
12.20	262.00	.890E+07	15923.2	5.79	9.31

<---- hydrograph -----> <-pi pe / channel -->

AREA (ha) = 1105.61
 OPEAK (cms) = 5.51
 TPEAK (hrs) = 7.50
 R. V. (mm) = 12.49
 MAX DEPTH (m) = 0.39
 MAX VEL (m/s) = 1.04
 INFLOW : ID= 2 (8106) 1105.61
 OUTFLOW: ID= 1 (6146) 1105.61
 4.88 8.25 12.49 0.35 1.04

CALIB STANDHYD (1462)
ID= 1 DT=15.0 min
Area (ha) = 112.22
Total Imp(%) = 32.00
Di r. Conn. (%) = 13.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	35.91	76.31
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	864.95	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	71.81	24.79
over (min)	15.00	45.00
Storage Coeff. (min)	13.10 (ii)	31.79 (ii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.08	0.03
PEAK FLOW (cms)	2.24	3.14
TIME TO PEAK (hrs)	6.00	6.50
RUNOFF VOLUME (mm)	52.40	20.51
TOTAL RAINFALL (mm)	54.40	54.40
RUNOFF COEFFICIENT	0.96	0.38

TOTALS
3.727 (iii)
6.50
24.66
54.40
0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1461)
ID= 1 DT=15.0 min
Area (ha) = 83.29
Total Imp(%) = 38.00
Di r. Conn. (%) = 16.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	31.65	51.64
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	745.16	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	71.81	44.58
over (min)	15.00	30.00
Storage Coeff. (min)	11.98 (ii)	26.76 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04
PEAK FLOW (cms)	2.11	2.80
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	52.40	21.36
TOTAL RAINFALL (mm)	54.40	54.40
RUNOFF COEFFICIENT	0.96	0.39

TOTALS
3.865 (iii)
6.00
26.32
54.40
0.48

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5146)
IN= 2---> OUT= 1
DT= 15.0 min
OUTFLOW (cms) | STORAGE (ha. m.) | OUTFLOW (cms) | STORAGE (ha. m.)
0.0000 | 0.0000 | 8.9200 | 2.5813
0.2820 | 0.8782 | 11.0570 | 3.0328

2. 5360 1. 0372 | 12. 9860 3. 4993
 4. 7880 1. 7148 | 13. 3860 3. 5093
 6. 7570 2. 1134 | 0. 0000 0. 0000

AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (1461) 83. 290 3. 865 6. 00 26. 32
 OUTFLOW: ID= 1 (5146) 83. 290 2. 037 6. 75 26. 31

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 52. 70
 TIME SHIF T OF PEAK FLOW (mi n) = 45. 00
 MAXI MUM STORAGE USED (ha. m.) = 1. 0123

ADD HYD (8099)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (1462): 112. 22 3. 727 6. 50 24. 66
 + ID2= 2 (5146): 83. 29 2. 037 6. 75 26. 31
 ID = 3 (8099): 195. 51 5. 226 6. 50 25. 36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8108)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (6146): 1105. 61 4. 882 8. 25 12. 49
 + ID2= 2 (8099): 195. 51 5. 226 6. 50 25. 36
 ID = 3 (8108): 1301. 12 7. 784 6. 75 14. 43

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8100)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8098): 10065. 22 44. 373 8. 00 15. 75
 + ID2= 2 (8108): 1301. 12 7. 784 6. 75 14. 43
 ID = 3 (8100): 11366. 34 50. 734 8. 00 15. 60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6202)
 IN= 2----> OUT= 1
 Routing time step (mi n)' = 15. 00

<----- DATA FOR SECTI ON (2021. 0) ----->
 Di stance El evati on Manni ng
 0. 00 245. 48 0. 0400
 29. 68 242. 80 0. 0400
 74. 20 240. 39 0. 0400
 155. 81 237. 12 0. 0400
 200. 33 234. 48 0. 0400
 244. 85 234. 67 0. 0400
 259. 69 233. 96 0. 0400
 274. 53 233. 84 0. 0400
 281. 68 233. 40 0. 0400
 285. 78 232. 40 0. 0400 / 0. 0300 Mai n Channel
 286. 78 231. 40 0. 0300 Mai n Channel
 296. 78 231. 40 0. 0300 Mai n Channel
 299. 78 232. 40 0. 0300 / 0. 0400 Mai n Channel
 311. 62 233. 74 0. 0400
 333. 88 234. 34 0. 0400
 497. 11 235. 32 0. 0400
 571. 31 237. 32 0. 0400
 675. 18 239. 10 0. 0400
 704. 86 242. 71 0. 0400
 734. 54 242. 62 0. 0400

<----- TRAVEL TIME TABLE ----->
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV. TIME
 (m) (m) (cu. m.) (cms) (m/s) (mi n)
 0. 50 231. 90 . 221E+05 4. 7 0. 85 78. 67

1. 00 232. 40 . 482E+05 15. 3 1. 28 52. 40
 1. 60 233. 00 . 913E+05 38. 3 1. 68 39. 77
 2. 20 233. 60 . 154E+06 72. 9 1. 90 35. 24
 2. 80 234. 20 . 278E+06 124. 2 1. 79 37. 30
 3. 40 234. 80 . 599E+06 214. 3 1. 44 46. 59
 4. 01 235. 41 . 125E+07 445. 0 1. 43 46. 90
 4. 61 236. 01 . 205E+07 838. 4 1. 64 40. 81
 5. 21 236. 61 . 293E+07 1361. 6 1. 86 35. 89
 5. 81 237. 21 . 389E+07 2011. 2 2. 08 32. 23
 6. 41 237. 81 . 494E+07 2753. 5 2. 24 29. 91
 7. 01 238. 41 . 612E+07 3635. 0 2. 39 28. 04
 7. 61 239. 01 . 741E+07 4672. 5 2. 53 26. 43
 8. 21 239. 61 . 880E+07 5999. 3 2. 74 24. 44
 8. 82 240. 22 . 102E+08 7510. 0 2. 94 22. 72
 9. 42 240. 82 . 117E+08 9201. 9 3. 15 21. 23
 10. 02 241. 42 . 132E+08 11059. 7 3. 35 19. 95
 10. 62 242. 02 . 148E+08 13073. 4 3. 54 18. 87
 11. 22 242. 62 . 164E+08 15242. 1 3. 73 17. 93

<---- hydrograph ----> <-pi pe / channel ->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8100) ***** 50. 73 8. 00 15. 60 1. 82 1. 75
 OUTFLOW: ID= 1 (6202) ***** 48. 71 8. 75 15. 60 1. 78 1. 74

ADD HYD (8112)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (6202): 11366. 34 48. 714 8. 75 15. 60
 + ID2= 2 (8110): 468. 53 21. 201 6. 00 33. 36
 ID = 3 (8112): 11834. 87 51. 778 8. 50 16. 30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5555)
 IN= 2----> OUT= 1
 DT= 15. 0 mi n
 OUTFLOW STORAGE OUTFLOW STORAGE
 (cms) (ha. m.) (cms) (ha. m.)
 0. 0000 0. 0000 ***** 19. 2100
 2. 8500 0. 5200 ***** 24. 7300
 8. 1800 1. 2600 ***** 30. 8600
 15. 0900 2. 2100 ***** 37. 6100
 23. 2700 3. 3900 ***** 41. 2200
 32. 5300 4. 7800 ***** 48. 9000
 36. 5100 5. 3900 ***** 66. 4700
 42. 7600 6. 3700 ***** 100. 1200
 53. 8500 8. 1300 ***** 140. 4100
 91. 7300 14. 3200 ***** 140. 4200

AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (8112) ***** 51. 778 8. 50 16. 30
 OUTFLOW: ID= 1 (5555) ***** 50. 918 9. 00 16. 30

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 98. 34
 TIME SHIF T OF PEAK FLOW (mi n) = 30. 00
 MAXI MUM STORAGE USED (ha. m.) = 7. 6703

ADD HYD (8114)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (5555): 11834. 87 50. 918 9. 00 16. 30
 + ID2= 2 (8136): 2288. 44 14. 955 7. 00 15. 59
 ID = 3 (8114): 14123. 31 60. 838 8. 75 16. 19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2061)
 ID= 1 DT=15. 0 mi n
 Area (ha) = 49. 84
 Total Imp (%) = 76. 00
 Di r. Conn. (%) = 50. 00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	37.88	11.96	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	576.43	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	71.81	94.82	
over (min)	15.00	30.00	
Storage Coeff. (min)=	10.27 (ii)	21.20 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.09	0.05	
PEAK FLOW (cms)=	4.16	1.57	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	5.190 (iii)
RUNOFF VOLUME (mm)=	52.40	29.39	40.90
TOTAL RAINFALL (mm)=	54.40	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.54	0.75

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5206)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.5640	2.4459
0.1690	0.8986	4.2460	2.7624
1.3810	1.2959	4.8070	3.0770
2.2440	1.6904	5.2070	3.0870
2.8510	2.0623	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2061)	49.840	5.190	6.00	40.90
OUTFLOW: ID= 1 (5206)	49.840	1.119	6.75	40.87

PEAK FLOW REDUCTION [Qout/Qin] (%) = 21.56
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha.m.) = 1.2109

CALIB
STANDHYD (2062)
ID= 1 DT=15.0 min

Area (ha)= 225.70
Total Imp(%)= 64.00
Di r. Conn. (%) = 42.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	144.45	81.25	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	1226.65	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	71.81	63.77	
over (min)	15.00	30.00	
Storage Coeff. (min)=	16.16 (ii)	28.97 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.07	0.04	
PEAK FLOW (cms)=	13.33	6.10	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	17.202 (iii)
RUNOFF VOLUME (mm)=	52.40	25.54	36.82
TOTAL RAINFALL (mm)=	54.40	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.47	0.68

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8144)

1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2062):	225.70	17.202	6.00	36.82
+ ID2= 2 (5206):	49.84	1.119	6.75	40.87
-----	-----	-----	-----	-----
ID = 3 (8144):	275.54	17.329	6.00	37.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (2041)
ID= 1 DT=15.0 min

Area (ha)= 209.90
Total Imp(%)= 37.00
Di r. Conn. (%) = 21.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	77.66	132.24	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	1182.93	40.00	
Mannings n =	0.013	0.250	

Max. Eff. Inten. (mm/hr)=	71.81	16.15	
over (min)	15.00	45.00	
Storage Coeff. (min)=	15.81 (ii)	37.99 (ii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.07	0.03	

PEAK FLOW (cms)=	6.26	3.26	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.50	7.582 (iii)
RUNOFF VOLUME (mm)=	52.40	14.13	22.16
TOTAL RAINFALL (mm)=	54.40	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.26	0.41

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5204)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	13.0280	5.1901
0.5340	2.0405	16.1840	5.6427
3.0020	2.3552	19.0840	6.7700
5.5440	3.8411	19.4840	6.7800
8.4130	4.5335	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2041)	209.900	7.582	6.00	22.16
OUTFLOW: ID= 1 (5204)	209.900	2.916	7.00	22.16

PEAK FLOW REDUCTION [Qout/Qin] (%) = 38.46
TIME SHIFT OF PEAK FLOW (min) = 60.00
MAXIMUM STORAGE USED (ha.m.) = 2.3466

CALIB
STANDHYD (2042)
ID= 1 DT=15.0 min

Area (ha)= 114.52
Total Imp(%)= 61.00
Di r. Conn. (%) = 34.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	69.86	44.66	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	873.77	40.00	
Mannings n =	0.013	0.250	

Max. Eff. Inten. (mm/hr)=	71.81	46.07	
over (min)	15.00	30.00	
Storage Coeff. (min)=	13.18 (ii)	27.77 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.08	0.04	

PEAK FLOW (cms)=	5.96	2.45	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	7.485 (iii)
RUNOFF VOLUME (mm)=	52.40	17.95	29.67

TOTAL RAINFALL (mm) = 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.33 0.55

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 61.0 Ia = Dep Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8140) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2042):	114.52	7.485	6.00	29.67
+ ID2= 2 (5204):	209.90	2.916	7.00	22.16

ID = 3 (8140):	324.42	7.748	6.00	24.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6206)
 IN= 2--> OUT= 1 Routing time step (min)' = 15.00

----- DATA FOR SECTION (2061.0) -----			
Distance	Elevation	Manning	
0.00	253.07	0.0350	
13.98	252.89	0.0350	
27.95	251.99	0.0350	
34.94	252.18	0.0350	
41.93	252.02	0.0350	
73.38	250.35	0.0350	
94.34	247.09	0.0350	
111.81	244.79	0.0350	
115.31	244.24	0.0350	
117.55	244.20	0.0350 / 0.0300	Main Channel
118.80	243.60	0.0300	Main Channel
122.30	243.60	0.0300	Main Channel
122.55	244.60	0.0300 / 0.0350	Main Channel
146.76	244.74	0.0350	
164.23	247.26	0.0350	
213.15	248.92	0.0350	
241.10	249.51	0.0350	
272.55	250.87	0.0350	
300.50	252.26	0.0350	
345.93	253.67	0.0350	

----- TRAVEL TIME TABLE -----					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.30	243.90	.310E+04	1.4	1.23	36.25
0.60	244.20	.676E+04	4.6	1.81	24.64
1.12	244.72	.221E+05	17.1	2.07	21.60
1.64	245.24	.754E+05	64.1	2.28	19.60
2.17	245.77	.139E+06	149.8	2.88	15.50
2.69	246.29	.214E+06	273.3	3.43	13.04
3.21	246.81	.299E+06	436.1	3.91	11.43
3.73	247.33	.395E+06	632.6	4.30	10.40
4.25	247.85	.511E+06	836.4	4.39	10.18
4.77	248.37	.653E+06	1114.9	4.58	9.76
5.30	248.90	.822E+06	1472.1	4.81	9.30
5.82	249.42	.102E+07	1872.2	4.91	9.10
6.34	249.94	.126E+07	2444.6	5.22	8.56
6.86	250.46	.151E+07	3113.1	5.52	8.09
7.38	250.98	.180E+07	3843.5	5.74	7.79
7.90	251.50	.211E+07	4717.8	6.00	7.45
8.43	252.03	.245E+07	5701.4	6.24	7.16
8.95	252.55	.284E+07	6727.5	6.36	7.03
9.47	253.07	.326E+07	7885.3	6.48	6.90

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8140)	324.42	7.75	6.00	24.81	0.73	1.87
OUTFLOW: ID= 1 (6206)	324.42	4.94	6.50	24.81	0.61	1.82

ADD HYD (8142) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6206):	324.42	4.943	6.50	24.81
+ ID2= 2 (8144):	275.54	17.329	6.00	37.56

ID = 3 (8142):	599.96	20.163	6.00	30.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8138) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8114):	14123.31	60.838	8.75	16.19
+ ID2= 2 (8142):	599.96	20.163	6.00	30.66

ID = 3 (8138):	14723.27	65.054	8.25	16.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6208)
 IN= 2--> OUT= 1 Routing time step (min)' = 15.00

----- DATA FOR SECTION (2081.0) -----			
Distance	Elevation	Manning	
0.00	249.19	0.1000	
73.11	244.13	0.1000	
121.86	243.39	0.1000	
243.71	240.01	0.1000	
353.38	238.75	0.1000	
450.87	235.40	0.1000	
499.61	231.40	0.1000	
511.80	230.96	0.1000	
523.98	230.12	0.1000	
534.50	227.80	0.1000 / 0.0500	Main Channel
536.50	226.70	0.0500	Main Channel
546.50	226.70	0.0500	Main Channel
548.50	227.80	0.0500 / 0.1000	Main Channel
572.72	230.25	0.1000	
682.40	230.50	0.1000	
804.25	233.95	0.1000	
926.11	235.83	0.1000	
1047.96	240.70	0.1000	
1145.45	243.26	0.1000	
1206.38	247.03	0.1000	

----- TRAVEL TIME TABLE -----					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.55	227.25	.111E+05	2.0	0.33	92.44
1.10	227.80	.242E+05	6.5	0.50	61.56
2.23	228.93	.700E+05	26.0	0.68	44.91
3.36	230.06	.150E+06	60.8	0.74	41.04
4.49	231.19	.446E+06	130.8	0.54	56.83
5.62	232.32	.925E+06	287.4	0.57	53.66
6.76	233.46	.152E+07	522.9	0.63	48.32
7.89	234.59	.223E+07	827.2	0.68	44.91
9.02	235.72	.312E+07	1219.5	0.72	42.61
10.15	236.85	.418E+07	1780.6	0.78	39.11
11.28	237.98	.537E+07	2479.2	0.85	36.07
12.41	239.11	.669E+07	3282.9	0.90	33.95
13.54	240.24	.824E+07	4228.6	0.94	32.47
14.67	241.37	.997E+07	5384.6	0.99	30.87
15.81	242.51	.119E+08	6698.4	1.03	29.56
16.94	243.64	.140E+08	8235.2	1.08	28.25
18.07	244.77	.162E+08	10104.2	1.14	26.73
19.20	245.90	.185E+08	12251.5	1.21	25.21
20.33	247.03	.209E+08	14597.0	1.28	23.89

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8138)	*****	65.05	8.25	16.78	3.43	0.73
OUTFLOW: ID= 1 (6208)	*****	61.96	9.25	16.78	3.38	0.74

CALIB
STANDHYD (2082)
ID= 1 DT=15.0 min

Area (ha)= 426.60
Total Imp(%)= 64.00 Dir. Conn.(%)= 37.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 273.02 153.58
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1686.42 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 48.96 62.87
over (min)= 30.00 45.00
Storage Coeff. (min)= 22.80 (ii) 35.68 (ii)
Unit Hyd. Tpeak (min)= 30.00 45.00
Unit Hyd. peak (cms)= 0.04 0.03

TOTALS
22.942 (iii)
6.25
34.10
54.40
0.63

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (2081)
ID= 1 DT=15.0 min

Area (ha)= 209.70
Total Imp(%)= 35.00 Dir. Conn.(%)= 20.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 73.39 136.31
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1182.37 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 71.81 20.97
over (min)= 15.00 45.00
Storage Coeff. (min)= 15.81 (ii) 35.79 (ii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.07 0.03

TOTALS
7.808 (iii)
6.00
25.09
54.40
0.46

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (2084)
ID= 1 DT=15.0 min

Area (ha)= 56.00
Total Imp(%)= 75.00 Dir. Conn.(%)= 70.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 42.00 14.00
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 611.01 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 71.81 49.09
over (min)= 15.00 30.00
Storage Coeff. (min)= 10.64 (ii) 24.86 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.09 0.04

TOTALS
7.031 (iii)
6.48
0.87

TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 52.40 26.10 44.51
TOTAL RAINFALL (mm)= 54.40 54.40 54.40
RUNOFF COEFFICIENT = 0.96 0.48 0.82

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 82.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9289)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.8000	1.5745
0.1900	0.9984	3.7500	1.7809
1.0900	1.1991	4.7100	1.9291
2.0200	1.3813	5.4800	2.0745

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
56.000	7.031	6.00	44.51
56.000	2.026	6.50	44.48

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 28.82
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 1.3837

ADD HYD (9262)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
209.70	7.808	6.00	25.09
56.00	2.026	6.50	44.48

265.70	8.376	6.50	29.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5208)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	16.8530	8.3371
0.8360	4.7353	21.1630	9.3975
4.7810	5.1070	25.1140	10.4406
8.9050	6.2484	25.5140	10.4506
12.6070	7.1796	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
265.700	8.376	6.50	29.17
265.700	1.426	8.75	29.17

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 17.02
TIME SHIFT OF PEAK FLOW (min) = 135.00
MAXIMUM STORAGE USED (ha. m.) = 4.7926

ADD HYD (8148)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
426.60	22.942	6.25	34.10
265.70	1.426	8.75	29.17

692.30	23.258	6.25	32.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8146)

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (6208)	14723.27	61.963	9.25	16.78
+ ID2= 2 (8148)	692.30	23.258	6.25	32.21

ID = 3 (8146)	15415.57	65.450	9.00	17.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6210)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2101.0) ----->

Distance	Elevation	Manning	
0.00	241.81	0.0800	
28.43	241.78	0.0800	
85.30	237.04	0.0800	
149.27	231.32	0.0800	
170.60	230.10	0.0800	
255.89	229.37	0.0800	
277.22	228.68	0.0800	
284.33	228.42	0.0800	
291.43	228.16	0.0800	
297.76	227.00	0.0800 / 0.0350	Main Channel
298.76	225.00	0.0350	Main Channel
312.76	225.00	0.0350	Main Channel
313.76	227.00	0.0350 / 0.0800	Main Channel
326.97	229.50	0.0800	
348.30	230.29	0.0800	
376.73	229.76	0.0800	
490.46	235.75	0.0800	
568.65	236.24	0.0800	
661.06	241.85	0.0800	
703.71	246.04	0.0800	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.67	225.67	.141E+05	6.6	0.69	35.86
1.33	226.33	.289E+05	20.4	1.04	23.65
2.00	227.00	.443E+05	39.3	1.31	18.81
2.93	227.93	.730E+05	77.8	1.57	15.64
3.85	228.85	.123E+06	131.1	1.57	15.65
4.78	229.78	.228E+06	205.6	1.33	18.51
5.70	230.70	.503E+06	356.7	1.05	23.49
6.63	231.63	.846E+06	608.7	1.06	23.17
7.55	232.55	.123E+07	946.4	1.14	21.65
8.48	233.48	.165E+07	1367.0	1.22	20.13
9.41	234.41	.211E+07	1871.8	1.31	18.80
10.33	235.33	.261E+07	2463.2	1.40	17.65
11.26	236.26	.317E+07	2999.4	1.40	17.63
12.18	237.18	.384E+07	3848.3	1.48	16.63
13.11	238.11	.454E+07	4806.0	1.56	15.75
14.03	239.03	.528E+07	5876.9	1.64	14.97
14.96	239.96	.605E+07	7062.7	1.72	14.29
15.88	240.88	.687E+07	8365.0	1.80	13.68
16.81	241.81	.771E+07	9506.9	1.82	13.52

<----- hydrograph -----> <--pi pe / channel -->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8146)	*****	65.45	9.00	17.47	2.63	1.48
OUTFLOW: ID= 1 (6210)	*****	65.26	9.25	17.47	2.63	1.48

CALIB STANDHYD (2104)
ID= 1 DT=15.0 mi n Area (ha)= 192.70 Total Imp(%)= 73.00 Di r. Conn.(%)= 68.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	140.67	52.03
Dep. Storage	1.00	5.00
Average Slope	1.00	2.00
Length	1133.43	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)= over (mi n)	71.81 / 15.00	46.48 / 30.00
Storage Coeff. (mi n)	12.52 (ii)	22.11 (ii)
Unit Hyd. Tpeak (mi n)	15.00	30.00

Unit Hyd. peak (cms)=	0.08	0.04	*TOTALS*
PEAK FLOW (cms)=	20.46	3.24	22.526 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	53.40	25.07	44.33
TOTAL RAINFALL (mm)=	54.40	54.40	54.40
RUNOFF COEFFICIENT =	0.98	0.46	0.81

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 81.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9283)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.5000	8.0000
0.3000	2.0000	3.5000	10.0000
0.9000	5.0000	4.5000	13.0000
2.0000	7.5000	5.5000	14.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2104)	192.700	22.526	6.00	44.33
OUTFLOW: ID= 1 (9283)	192.700	1.448	8.00	44.33

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 6.43
TIME SHIFT OF PEAK FLOW (mi n) = 120.00
MAXIMUM STORAGE USED (ha. m.) = 6.2530

CALIB STANDHYD (2103)
ID= 1 DT=15.0 mi n

Area (ha)= 11.10 Total Imp(%)= 30.00 Di r. Conn.(%)= 10.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	3.33	7.77
Dep. Storage	1.00	5.00
Average Slope	1.00	2.00
Length	272.03	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)= over (mi n)	71.81 / 15.00	58.53 / 15.00
Storage Coeff. (mi n)	5.32 (ii)	14.06 (ii)
Unit Hyd. Tpeak (mi n)	15.00	15.00
Unit Hyd. peak (cms)=	0.11	0.07

PEAK FLOW (cms)=	0.21	0.91	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.00	1.121 (iii)
RUNOFF VOLUME (mm)=	53.40	28.95	31.39
TOTAL RAINFALL (mm)=	54.40	54.40	54.40
RUNOFF COEFFICIENT =	0.98	0.53	0.58

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9284)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.5600	0.2067
0.0400	0.1045	0.7400	0.2572
0.2200	0.1166	0.9300	0.3046
0.4000	0.1670	1.0900	0.3510

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
--	-----------	-------------	-------------	------------

INFLOW : ID= 2 (2103) 11.100 1.121 6.00 31.39
 OUTFLOW: ID= 1 (9284) 11.100 0.373 6.50 31.33

PEAK FLOW REDUCTION [Qout/Qin] (%) = 33.24
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 0.1618

ADD HYD	(9258)	AREA	OPEAK	TPEAK	R.V.
1 + 2 =	3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (9283):		192.70	1.448	8.00	44.33
+ ID2= 2 (9284):		11.10	0.373	6.50	31.33

ID = 3 (9258):		203.80	1.642	7.00	43.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5210)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2---> OUT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min				
	0.0000	0.0000	0.0000	0.0024
	0.0020	0.0013	0.0080	0.0027
	0.0030	0.0017	0.0090	0.0031
	0.0050	0.0020	0.0490	0.0131

INFLOW : ID= 2 (9258)	AREA	OPEAK	TPEAK	R.V.
OUTFLOW: ID= 1 (5210)	(ha)	(cms)	(hrs)	(mm)
	203.800	1.642	7.00	43.62
	203.800	1.642	7.25	43.62

PEAK FLOW REDUCTION [Qout/Qin] (%) = 99.97
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0440

CALIB NASHYD (2102)	Area	(ha)	Curve Number (CN) =	80.0
ID= 1 DT=15.0 min	Ia	(mm)	# of Linear Res. (N) =	1.50
	U.H.	Tp(hrs)		

Unit Hyd Opeak	(cms)	=	0.228
PEAK FLOW	(cms)	=	0.137 (i)
TIME TO PEAK	(hrs)	=	7.500
RUNOFF VOLUME	(mm)	=	21.418
TOTAL RAINFALL	(mm)	=	54.400
RUNOFF COEFFICIENT	=		0.394

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	(9210)	AREA	OPEAK	TPEAK	R.V.
1 + 2 =	3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (2102):		14.38	0.137	7.50	21.42
+ ID2= 2 (5210):		203.80	1.642	7.25	43.62

ID = 3 (9210):		218.18	1.778	7.25	42.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8150)	AREA	OPEAK	TPEAK	R.V.
1 + 2 =	3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6210):		15415.57	65.263	9.25	17.47
+ ID2= 2 (9210):		218.18	1.778	7.25	42.15

ID = 3 (8150):		15633.75	66.814	9.25	17.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2121)	Area	(ha)	Imp(%) =	56.89	Di r. Conn. (%) =	74.00
ID= 1 DT=15.0 min	Total					

Surface Area	(ha)	=	51.77	PERVIOUS (i)
Dep. Storage	(mm)	=	2.00	5.00
Average Slope	(%)	=	0.50	0.50
Length	(m)	=	615.85	40.00
Mannings n	=		0.013	0.250
Max. Eff. Inten. (mm/hr)=			71.81	134.01
over (min)			15.00	30.00
Storage Coeff. (min)=			10.69 (ii)	20.20 (ii)
Unit Hyd. Tpeak (min)=			15.00	30.00
Unit Hyd. peak (cms)=			0.09	0.05

TOTALS
 PEAK FLOW (cms) = 6.95 0.97 7.587 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 52.40 30.16 46.62
 TOTAL RAINFALL (mm) = 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.55 0.86

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2124)	Area	(ha)	Imp(%) =	4.40	Di r. Conn. (%) =	70.00
ID= 1 DT=15.0 min	Total					

Surface Area	(ha)	=	3.30	PERVIOUS (i)
Dep. Storage	(mm)	=	2.00	5.00
Average Slope	(%)	=	0.50	0.50
Length	(m)	=	171.27	40.00
Mannings n	=		0.013	0.250
Max. Eff. Inten. (mm/hr)=			71.81	19.35
over (min)			15.00	30.00
Storage Coeff. (min)=			4.96 (ii)	25.59 (ii)
Unit Hyd. Tpeak (min)=			15.00	30.00
Unit Hyd. peak (cms)=			0.11	0.04

TOTALS
 PEAK FLOW (cms) = 0.59 0.04 0.622 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 52.40 17.36 41.88
 TOTAL RAINFALL (mm) = 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.32 0.77

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9290)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2---> OUT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min				
	0.0000	0.0000	0.2200	0.1226
	0.0100	0.0778	0.2900	0.1387
	0.0800	0.0934	0.3700	0.1503
	0.1600	0.1076	0.4300	0.1616

INFLOW : ID= 2 (2124)	AREA	OPEAK	TPEAK	R.V.
OUTFLOW: ID= 1 (9290)	(ha)	(cms)	(hrs)	(mm)
	4.400	0.622	6.00	41.88
	4.400	0.157	6.25	41.44

PEAK FLOW REDUCTION [Qout/Qin] (%) = 25.29
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha.m.) = 0.1073

ADD HYD	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (2121):	56.89	7.587	6.00	46.62
+ ID2= 2 (9290):	4.40	0.157	6.25	41.44
===== ID = 3 (9264):	61.29	7.608	6.00	46.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5212)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1	(cms)	(ha. m.)	(cms)	(ha. m.)
DT= 15.0 min	0.0000	0.0000	4.1000	2.1402
	0.2080	1.2793	5.1480	2.2661
	1.1880	1.5546	5.9930	2.3900
	2.2120	1.7255	6.3930	2.4000
	3.0680	1.9336	0.0000	0.0000

INFLOW	AREA	OPEAK	TPEAK	R. V.
ID= 2 (9264)	(ha)	(cms)	(hrs)	(mm)
61.290	61.290	7.608	6.00	46.24

OUTFLOW	AREA	OPEAK	TPEAK	R. V.
ID= 1 (5212)	(ha)	(cms)	(hrs)	(mm)
61.290	61.290	1.718	6.75	46.22

PEAK FLOW REDUCTION [Qout/Qin] (%) = 22.59
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 1.6622

CALIB STANDHYD (2122)	Area	IMPERVIOUS	PERVIOUS (i)
ID= 1 DT=15.0 min	(ha)	(%)	(%)
	267.10	29.00	24.00

Surface Area	Dep. Storage	Average Slope	Length	Mannings n
(ha)	(mm)	(%)	(m)	
77.46	2.00	0.50	1334.42	0.013

Max. Eff. Inten.	over	Storage Coeff.	Unit Hyd. Tpeak	Unit Hyd. peak
(mm/hr)	(min)	(min)	(min)	(cms)
71.81	15.00	17.00 (ii)	15.00	0.07

PEAK FLOW	TIME TO PEAK	RUNOFF VOLUME	TOTAL RAINFALL	RUNOFF COEFFICIENT
(cms)	(hrs)	(mm)	(mm)	
8.81	6.00	52.40	54.40	0.96

PERVIOUS (i)	*TOTALS*
(%)	(iii)
0.250	10.834

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9312)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1	(cms)	(ha. m.)	(cms)	(ha. m.)
DT= 15.0 min	0.0000	0.0000	13.3600	4.9180
	0.9100	2.4658	17.8700	6.1368
	5.1800	2.7404	22.4400	7.2882
	9.6400	3.9605	26.1200	8.4139

INFLOW	AREA	OPEAK	TPEAK	R. V.
ID= 2 (2122)	(ha)	(cms)	(hrs)	(mm)
267.100	267.100	10.834	6.00	25.85

OUTFLOW	AREA	OPEAK	TPEAK	R. V.
ID= 1 (9312)	(ha)	(cms)	(hrs)	(mm)
267.100	267.100	5.610	6.75	25.85

PEAK FLOW REDUCTION [Qout/Qin] (%) = 51.78

TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 2.8664

ADD HYD	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (5212):	61.29	1.718	6.75	46.22
+ ID2= 2 (9312):	267.10	5.610	6.75	25.85
===== ID = 3 (8152):	328.39	7.329	6.75	29.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6214)	Routing time step (min) = 15.00
IN= 2--> OUT= 1	

<----- DATA FOR SECTION (2141.0) ----->		
Distance	Elevation	Manning
0.00	259.29	0.0500
15.33	258.91	0.0500
38.33	258.57	0.0500
65.16	257.93	0.0500
95.82	253.44	0.0500
111.15	251.21	0.0500
130.32	249.67	0.0500
149.48	248.51	0.0500
150.66	248.50	0.0500 / 0.0370
150.81	247.50	0.0370
154.66	247.50	0.0370
155.66	248.50	0.0370 / 0.0500
160.98	248.50	0.0500
187.81	249.78	0.0500
199.31	252.13	0.0500
245.30	254.04	0.0500
264.46	253.99	0.0500
298.96	255.23	0.0500
329.62	257.02	0.0500
379.45	258.82	0.0500

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)
0.50	248.00	648E+04	2.1	1.00	52.30
1.00	248.50	139E+05	6.4	1.44	36.35
1.61	249.11	570E+05	22.6	1.24	42.01
2.21	249.71	143E+06	63.6	1.39	37.61
2.82	250.32	263E+06	142.9	1.70	30.69
3.43	250.93	403E+06	257.2	2.00	26.11
4.04	251.54	562E+06	410.4	2.29	22.82
4.64	252.14	735E+06	601.3	2.56	20.37
5.25	252.75	933E+06	791.0	2.66	19.67
5.86	253.36	117E+07	1038.0	2.79	18.74
6.46	253.96	144E+07	1344.4	2.93	17.82
7.07	254.57	178E+07	1664.1	2.93	17.83
7.68	255.18	217E+07	2140.2	3.10	16.87
8.28	255.78	259E+07	2722.9	3.30	15.83
8.89	256.39	304E+07	3387.1	3.50	14.94
9.50	257.00	351E+07	4132.7	3.69	14.16
10.11	257.61	402E+07	4913.0	3.83	13.64
10.71	258.21	458E+07	5648.3	3.87	13.50
11.32	258.82	521E+07	6343.1	3.82	13.68

<----- hydrograph ----->					
INFLOW	AREA	OPEAK	TPEAK	R. V.	<- pi pe / channel ->
ID= 2 (8152)	(ha)	(cms)	(hrs)	(mm)	(m) MAX DEPTH
328.39	328.39	7.33	6.75	29.65	1.04
OUTFLOW	AREA	OPEAK	TPEAK	R. V.	(m/s) MAX VEL
ID= 1 (6214)	(ha)	(cms)	(hrs)	(mm)	
328.39	328.39	5.34	7.25	29.65	0.88

CALIB STANDHYD (2144)	Area	IMPERVIOUS	PERVIOUS (i)
ID= 1 DT=15.0 min	(ha)	(%)	(%)
	93.10	75.00	70.00

Surface Area	Dep. Storage	Average Slope
(ha)	(mm)	(%)
69.82	2.00	0.50

Length (m) = 787.82 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 71.81 20.02
 over (min) = 15.00 45.00

Storage Coeff. (min) = 12.39 (ii) 32.75 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.08 0.03

PEAK FLOW (cms) = 10.22 0.77
 TIME TO PEAK (hrs) = 6.00 6.50
 RUNOFF VOLUME (mm) = 52.40 17.90
 TOTAL RAINFALL (mm) = 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.33

TOTALS
 10.532 (iii)
 6.00
 42.05
 54.40
 0.77

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9285)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.6500	2.6150
0.3200	1.6583	6.2300	2.9579
1.8100	1.9916	7.8200	3.2041
3.3600	2.2943	9.1000	3.4456

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2144)	93.100	10.532	6.00	42.05
OUTFLOW: ID= 1 (9285)	93.100	2.784	6.50	42.04

PEAK FLOW REDUCTION [Qout/Qin] (%) = 26.44
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 2.2046

CALIB STANDHYD (2143)
 ID= 1 DT=15.0 min

Area (ha) = 158.30
 Total Imp(%) = 26.00
 Dir. Conn. (%) = 9.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	41.16	117.14
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1027.29	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 71.81 22.40
 over (min) = 15.00 45.00

Storage Coeff. (min) = 14.53 (ii) 33.99 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.07 0.03

TOTALS
 4.828 (iii)
 6.50
 22.36
 54.40
 0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9286)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.6500	2.6150
0.3200	1.6583	6.2300	2.9579
1.8100	1.9916	7.8200	3.2041
3.3600	2.2943	9.1000	3.4456

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
0.0000	0.0000	7.9200	2.8224	
0.5400	1.3792	10.5900	3.5534	
3.0700	1.5175	13.3000	4.2519	
5.7200	2.2534	15.4900	4.9349	

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2143)	158.300	4.828	6.50	22.36
OUTFLOW: ID= 1 (9286)	158.300	3.091	7.00	22.36

PEAK FLOW REDUCTION [Qout/Qin] (%) = 64.02
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 1.5319

ADD HYD (9260)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9285):	93.10	2.784	6.50	42.04
+ ID2= 2 (9286):	158.30	3.091	7.00	22.36
ID = 3 (9260):	251.40	5.412	7.00	29.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5214)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.5370	0.9877
0.1280	0.4635	3.1860	1.1293
0.5900	0.5362	3.7090	1.2678
1.3690	0.6921	3.9980	1.2778
1.8980	0.8262	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9260)	251.400	5.412	7.00	29.64
OUTFLOW: ID= 1 (5214)	251.400	3.589	7.50	29.64

PEAK FLOW REDUCTION [Qout/Qin] (%) = 66.31
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 1.2382

CALIB STANDHYD (2142)
 ID= 1 DT=15.0 min

Area (ha) = 65.54
 Total Imp(%) = 27.00
 Dir. Conn. (%) = 10.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	17.70	47.84
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	661.01	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 71.81 11.22
 over (min) = 15.00 45.00

Storage Coeff. (min) = 11.15 (ii) 36.81 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.08 0.03

TOTALS
 1.401 (iii)
 6.00
 14.43
 54.40
 0.27

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 51.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9214)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2142):	65.54	1.401	6.00	14.43
+ ID2= 2 (5214):	251.40	3.589	7.50	29.64
=====				
ID = 3 (9214):	316.94	4.012	7.50	26.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8154)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6214):	328.39	5.341	7.25	29.65
+ ID2= 2 (9214):	316.94	4.012	7.50	26.50
=====				
ID = 3 (8154):	645.33	9.235	7.25	28.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2163)				
ID= 1 DT=15.0 min				
	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
Total	69.70	27.00	9.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	18.82	50.88
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	681.66	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	71.81	45.62
over (min)	15.00	30.00
Storage Coeff. (min)	11.36 (ii)	26.00 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04

PEAK FLOW (cms)	1.01	2.87	*TOTALS*
TIME TO PEAK (hrs)	6.00	6.25	3.269 (iii)
RUNOFF VOLUME (mm)	52.40	23.54	26.14
TOTAL RAINFALL (mm)	54.40	54.40	54.40
RUNOFF COEFFICIENT	0.96	0.43	0.48

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 78.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9271)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	3.4900	1.2605
	0.2400	0.6230	4.6600	1.5808
	1.3500	0.6885	5.8600	1.8854
	2.5200	1.0102	6.8200	2.1832

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2163)	69.700	3.269	6.25	26.14
OUTFLOW: ID= 1 (9271)	69.700	1.634	6.75	26.13

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 49.98
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 0.7677

CALIB STANDHYD (2169)	
Area	(ha) = 3.50

|| ID= 1 DT=15.0 min | Total Imp(%)= 30.00 Di r. Conn. (%)= 10.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	1.05	2.45
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	152.75	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	71.81	21.99
over (min)	15.00	30.00
Storage Coeff. (min)	4.63 (ii)	24.24 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.11	0.04

PEAK FLOW (cms)	0.07	0.11	*TOTALS*
TIME TO PEAK (hrs)	6.00	6.25	0.138 (iii)
RUNOFF VOLUME (mm)	52.40	18.32	6.00
TOTAL RAINFALL (mm)	54.40	54.40	21.72
RUNOFF COEFFICIENT	0.96	0.34	0.40

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9272)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	0.1800	0.0659
	0.0100	0.0333	0.2400	0.0820
	0.0700	0.0372	0.3000	0.0971
	0.1300	0.0532	0.3500	0.1119

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2169)	3.500	0.138	6.00	21.72
OUTFLOW: ID= 1 (9272)	3.500	0.067	6.75	21.49

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 48.57
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.0372

ADD HYD (9251)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9271):	69.70	1.634	6.75	26.13
+ ID2= 2 (9272):	3.50	0.067	6.75	21.49
=====				
ID = 3 (9251):	73.20	1.701	6.75	25.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5216)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	3.7400	2.4559
	0.1890	1.6831	4.6960	2.6646
	1.0830	1.7903	5.4670	2.8688
	2.0170	2.0202	5.8670	2.8788
	2.7980	2.2179	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9251)	73.200	1.701	6.75	25.91
OUTFLOW: ID= 1 (5216)	73.200	0.130	16.75	25.88

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 7.63
TIME SHIFT OF PEAK FLOW (min) = 600.00
MAXIMUM STORAGE USED (ha. m.) = 1.1550

CALIB
STANDHYD (2162)
ID= 1 DT=15.0 min

Area (ha)= 72.57
Total Imp(%)= 20.00 Dir. Conn.(%)= 7.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 14.51 58.06
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 695.56 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 71.81 18.30
over (min)= 15.00 45.00
Storage Coeff. (min)= 11.50 (ii) 32.60 (ii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.08 0.03

PEAK FLOW (cms)= 0.82 1.75 *TOTALS* 1.941 (iii)
TIME TO PEAK (hrs)= 6.00 6.50 6.50
RUNOFF VOLUME (mm)= 52.40 16.98 19.46
TOTAL RAINFALL (mm)= 54.40 54.40 54.40
RUNOFF COEFFICIENT = 0.96 0.31 0.36

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9316)
IN= 2 -> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.0000	0.3000
2.0000	0.1000	4.5000	0.4000
3.0000	0.2000	5.0000	0.5000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2162)	72.570	1.941	6.50	19.46
OUTFLOW: ID= 1 (9316)	72.570	1.764	6.50	19.46

PEAK FLOW REDUCTION [Qout/ Qin] (%) = 90.87
TIME SHIFT OF PEAK FLOW (min) = 0.00
MAXIMUM STORAGE USED (ha.m.) = 0.0962

ADD HYD (9216)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (5216):	73.20	0.130	16.75	25.88
+ ID2= 2 (9316):	72.57	1.764	6.50	19.46
-----	-----	-----	-----	-----
ID = 3 (9216):	145.77	1.777	6.50	22.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8156)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8154):	645.33	9.235	7.25	28.10
+ ID2= 2 (9216):	145.77	1.777	6.50	22.68
-----	-----	-----	-----	-----
ID = 3 (8156):	791.10	10.337	7.25	27.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6218)

IN= 2 -> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2181.0) ----->

Distance	Elevation	Manning
0.00	256.38	0.0450
7.70	254.85	0.0450
15.40	253.20	0.0450
19.25	252.29	0.0450
23.10	251.17	0.0450
27.94	236.16	0.0450
32.79	235.80	0.0450
37.64	235.45	0.0450
42.49	234.60	0.0450 / 0.0300
47.34	233.60	0.0300
52.19	233.60	0.0300
57.04	233.60	0.0300
61.89	234.60	0.0300 / 0.0450
66.74	235.13	0.0450
71.59	235.20	0.0450
76.44	235.32	0.0450
81.29	239.95	0.0450
86.14	180.98	0.0450
90.99	188.69	0.0450
95.84	190.61	0.0450

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
1.00	234.60	181E+05	6.0	0.66	50.64
2.13	235.73	518E+05	23.8	0.93	36.27
3.25	236.85	119E+06	60.0	1.02	33.03
4.38	237.98	217E+06	120.6	1.12	29.99
5.51	239.11	346E+06	210.5	1.23	27.39
6.63	240.23	506E+06	334.1	1.33	25.22
7.76	241.36	697E+06	495.6	1.43	23.44
8.88	242.48	920E+06	699.8	1.53	21.91
10.01	243.61	117E+07	950.8	1.63	20.59
11.14	244.74	146E+07	1252.5	1.73	19.45
12.26	245.86	178E+07	1619.6	1.83	18.37
13.39	246.99	212E+07	2078.5	1.97	17.01
14.52	248.12	247E+07	2589.0	2.11	15.88
15.64	249.24	282E+07	3149.7	2.25	14.94
16.77	250.37	319E+07	3759.8	2.37	14.14
17.89	251.49	356E+07	4419.5	2.50	13.44
19.02	252.62	395E+07	5126.4	2.61	12.84
20.15	253.75	435E+07	5871.6	2.72	12.34
21.27	254.87	476E+07	6663.2	2.82	11.89

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8156)	791.10	10.34	7.25	27.10	1.28	0.71
OUTFLOW: ID= 1 (6218)	791.10	7.73	8.00	27.10	1.11	0.68

CALIB
STANDHYD (2183)
ID= 1 DT=15.0 min

Area (ha)= 69.30
Total Imp(%)= 22.00 Dir. Conn.(%)= 7.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 15.25 54.05
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 679.71 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 71.81 45.34
over (min)= 15.00 30.00
Storage Coeff. (min)= 11.34 (ii) 26.02 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04

PEAK FLOW (cms)= 0.78 3.03 *TOTALS* 3.339 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.25
RUNOFF VOLUME (mm)= 52.40 24.36 26.33
TOTAL RAINFALL (mm)= 54.40 54.40 54.40
RUNOFF COEFFICIENT = 0.96 0.45 0.48

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 80.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9282)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.5000	3.5000
0.8000	0.1000	6.0000	4.0000
1.1000	2.0000	6.5000	4.5000
2.5000	3.0000	7.0000	5.5000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2183)	69.300	3.339	6.25	26.33
OUTFLOW: ID= 1 (9282)	69.300	0.894	7.25	26.33

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 26.79
TIME SHI FT OF PEAK FLOW (mi n) = 60.00
MAXI MUM STORAGE USED (ha. m.) = 0.6987

RESERVOIR (5218)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.5090	1.7553
0.2280	0.8236	5.6620	2.0069
1.3060	0.9528	6.5910	2.2530
2.4320	1.2300	6.9910	2.2630
3.3740	1.4683	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9282)	69.300	0.894	7.25	26.33
OUTFLOW: ID= 1 (5218)	69.300	0.828	10.00	26.31

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 92.60
TIME SHI FT OF PEAK FLOW (mi n) = 165.00
MAXI MUM STORAGE USED (ha. m.) = 0.8957

CALIB NASHDY (2182)
ID= 1 DT=15.0 mi n

Area (ha) = 82.95 Curve Number (CN) = 67.7
Ia (mm) = 5.00 # of Li near Res. (N) = 1.50
U. H. Tp (hrs) = 0.79

Unit Hyd Qpeak (cms) = 1.786
PEAK FLOW (cms) = 0.650 (i)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 14.100
TOTAL RAI NFALL (mm) = 54.400
RUNOFF COEFFI CI ENT = 0.259

- (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9218)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2182):	82.95	0.650	7.00	14.10
+ ID2= 2 (5218):	69.30	0.828	10.00	26.31

ID = 3 (9218):	152.25	1.223	9.50	19.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8158)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6218):	791.10	7.728	8.00	27.10

+ ID2= 2 (9218):	152.25	1.223	9.50	19.66

ID = 3 (8158):	943.35	8.502	8.00	25.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8151)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8150):	15633.75	66.814	9.25	17.81
+ ID2= 2 (8158):	943.35	8.502	8.00	25.90

ID = 3 (8151):	16577.10	74.177	9.00	18.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5504)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	33.0100	0.6700
2.6400	0.0500	37.4800	0.7800
4.8300	0.0900	47.5400	1.0100
7.4100	0.1300	60.2300	1.2800
10.3300	0.1900	71.9530	1.5258
13.5300	0.2500	*****	3.0529
16.9900	0.3200	*****	5.3299
20.6800	0.3900	*****	8.4148
24.6000	0.4800	*****	11.7637
28.7100	0.5700	*****	11.7737

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (8151)	*****	74.177	9.00	18.27
OUTFLOW: ID= 1 (5504)	*****	74.146	9.00	18.27

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 99.96
TIME SHI FT OF PEAK FLOW (mi n) = 0.00
MAXI MUM STORAGE USED (ha. m.) = 1.5737

ROUTE CHN (6220)
IN= 2--> OUT= 1

Routing time step (mi n)' = 15.00

<----- DATA FOR SECTION (2201.0) ----->

Di stance	Elevation	Manning	
0.00	245.59	0.0600	
5.76	245.45	0.0600	
23.03	242.27	0.0600	
57.58	232.35	0.0600	
86.37	226.18	0.0600	
109.41	224.31	0.0600	
115.17	223.86	0.0600	
120.92	222.78	0.0600	
126.68	221.93	0.0600	
128.06	221.50	0.0600 / 0.0300	Main Channel
129.56	220.00	0.0300	Main Channel
141.56	220.00	0.0300	Main Channel
143.06	221.50	0.0300 / 0.0600	Main Channel
149.72	223.77	0.0600	
155.47	225.09	0.0600	
253.36	229.21	0.0600	
333.98	227.50	0.0600	
454.90	229.25	0.0600	
500.97	233.11	0.0600	
570.07	237.42	0.0600	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.75	220.75	.262E+05	5.5	0.57	79.44
1.50	221.50	.555E+05	17.5	0.86	52.89
2.44	222.44	.103E+06	42.7	1.14	40.03
3.37	223.37	.171E+06	79.4	1.27	35.98
4.31	224.31	.263E+06	128.1	1.33	34.22
5.25	225.25	.393E+06	191.7	1.34	34.19
6.18	226.18	.594E+06	276.7	1.28	35.77
7.12	227.12	.872E+06	399.9	1.26	36.33

8.06	228.06	.127E+07	542.7	1.17	38.92
8.99	228.99	.199E+07	770.0	1.06	43.01
9.93	229.93	.297E+07	1151.8	1.06	42.95
10.86	230.86	.400E+07	1652.9	1.13	40.33
11.80	231.80	.507E+07	2251.8	1.22	37.53
12.74	232.74	.618E+07	2946.5	1.31	34.97
13.67	233.67	.733E+07	3726.8	1.39	32.79
14.61	234.61	.853E+07	4591.0	1.48	30.96
15.55	235.55	.977E+07	5544.1	1.55	29.37
16.48	236.48	.111E+08	6586.7	1.63	27.99
17.42	237.42	.124E+08	7719.2	1.71	26.77

		<---- hydrograph ---->			<-pi pe / channel -->	
	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW: ID= 2 (5504)	*****	74.15	9.00	18.27	3.24	1.25
OUTFLOW: ID= 1 (6220)	*****	72.08	9.50	18.27	3.19	1.24

CALIB STANDHYD (2207)		Area (ha)=	57.80	Dir. Conn. (%)=	8.00
ID= 1 DT=15.0 min		Total Imp(%)=	21.00		

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	12.14	45.66
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	620.75	40.00
Mannings n	=	0.013	0.250
Max. Eff. Inten. (mm/hr)=		71.81	46.78
over (min)		15.00	30.00
Storage Coeff. (min)=	10.74 (ii)		25.23 (ii)
Unit Hyd. Tpeak (min)=	15.00		30.00
Unit Hyd. peak (cms)=	0.09		0.04

		TOTALS		(iii)
PEAK FLOW	(cms)=	0.76	2.69	2.972
TIME TO PEAK	(hrs)=	6.00	6.25	6.25
RUNOFF VOLUME	(mm)=	52.40	25.62	27.77
TOTAL RAINFALL	(mm)=	54.40	54.40	54.40
RUNOFF COEFFICIENT	=	0.96	0.47	0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 82.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9278)		OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2---> OUT= 1		(cms)	(ha. m.)	(cms)	(ha. m.)
DT= 15.0 min		0.0000	0.0000	0.5000	2.5000
		0.2000	0.3000	0.6000	3.5000
		0.3000	0.9000	0.7000	4.5000
		0.4000	2.0000	0.8000	5.5000

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (2207)	57.800	2.972	6.25	27.77
OUTFLOW: ID= 1 (9278)	57.800	0.312	8.50	27.76

PEAK FLOW REDUCTION [Qout/Qin] (%) = 10.50
 TIME SHIFT OF PEAK FLOW (min) = 135.00
 MAXIMUM STORAGE USED (ha. m.) = 1.0340

CALIB STANDHYD (2204)		Area (ha)=	13.30	Dir. Conn. (%)=	70.00
ID= 1 DT=15.0 min		Total Imp(%)=	75.00		

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	9.98	3.33
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50

Length	(m)=	297.77	40.00
Mannings n	=	0.013	0.250
Max. Eff. Inten. (mm/hr)=		71.81	44.29
over (min)		15.00	30.00
Storage Coeff. (min)=	6.91 (ii)		21.73 (ii)
Unit Hyd. Tpeak (min)=	15.00		30.00
Unit Hyd. peak (cms)=	0.10		0.05

		TOTALS		(iii)
PEAK FLOW	(cms)=	1.72	0.20	1.841
TIME TO PEAK	(hrs)=	6.00	6.25	6.00
RUNOFF VOLUME	(mm)=	52.40	23.69	43.79
TOTAL RAINFALL	(mm)=	54.40	54.40	54.40
RUNOFF COEFFICIENT	=	0.96	0.44	0.80

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9279)		OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2---> OUT= 1		(cms)	(ha. m.)	(cms)	(ha. m.)
DT= 15.0 min		0.0000	0.0000	0.6700	0.3747
		0.0500	0.2376	0.8900	0.4238
		0.2600	0.2854	1.1200	0.4591
		0.4800	0.3287	1.3000	0.4937

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (2204)	13.300	1.841	6.00	43.79
OUTFLOW: ID= 1 (9279)	13.300	0.475	6.50	43.70

PEAK FLOW REDUCTION [Qout/Qin] (%) = 25.82
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 0.3309

ADD HYD (9255)		AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (9278):		57.80	0.312	8.50	27.76
+ ID2= 2 (9279):		13.30	0.475	6.50	43.70
=====					
ID = 3 (9255):		71.10	0.737	6.50	30.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5220)		OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2---> OUT= 1		(cms)	(ha. m.)	(cms)	(ha. m.)
DT= 15.0 min		0.0000	0.0000	2.6050	1.0142
		0.1320	0.4759	3.2710	1.1595
		0.7550	0.5505	3.8080	1.3018
		1.4050	0.7107	4.2080	1.3118
		1.9490	0.8484	0.0000	0.0000

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (9255)	71.100	0.737	6.50	30.74
OUTFLOW: ID= 1 (5220)	71.100	0.384	9.75	30.73

PEAK FLOW REDUCTION [Qout/Qin] (%) = 52.09
 TIME SHIFT OF PEAK FLOW (min) = 195.00
 MAXIMUM STORAGE USED (ha. m.) = 0.5062

CALIB NASHYD (2202)		Area (ha)=	98.00	Curve Number (CN)=	74.3
ID= 1 DT=15.0 min		Ia (mm)=	5.00	# of Linear Res. (N)=	1.50
		U. H. Tp(hrs)=	1.50		

Unit Hyd Qpeak (cms)= 1.115

PEAK FLOW (cms)= 0.591 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 17.680
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.325

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9220) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2202):	98.00	0.591	8.25	17.68
+ ID2= 2 (5220):	71.10	0.384	9.75	30.73

ID = 3 (9220):	169.10	0.943	9.25	23.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8160) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6220):	16577.10	72.082	9.50	18.27
+ ID2= 2 (9220):	169.10	0.943	9.25	23.17

ID = 3 (8160):	16746.20	73.021	9.50	18.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2221) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	110.64 37.00	Dir. Conn.(%)=	20.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	40.94	69.70
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	858.84	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	71.81	43.08
over (min)	15.00	30.00
Storage Coeff. (min)=	13.05 (ii)	28.03 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04

PEAK FLOW (cms)=	3.40	3.57	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	5.625 (iii)
RUNOFF VOLUME (mm)=	52.40	21.94	6.00
TOTAL RAINFALL (mm)=	54.40	54.40	28.03
RUNOFF COEFFICIENT =	0.96	0.40	54.40
			0.52

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.3 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2224) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	43.20 72.00	Dir. Conn.(%)=	66.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	31.10	12.10
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	536.66	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	71.81	45.17
over (min)	15.00	30.00
Storage Coeff. (min)=	9.84 (ii)	24.54 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.04

PEAK FLOW (cms)=	4.83	0.70	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	5.264 (iii)
RUNOFF VOLUME (mm)=	52.40	23.88	6.00
TOTAL RAINFALL (mm)=	54.40	54.40	42.70
RUNOFF COEFFICIENT =	0.96	0.44	54.40
			0.78

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 79.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9288) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	2.1600	1.1836
	0.1500	0.7428	2.8900	1.3455
	0.8400	0.8895	3.6300	1.4651
	1.5600	1.0342	4.2300	1.5823

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2224)	43.200	5.264	6.00	42.70
OUTFLOW: ID= 1 (9288)	43.200	1.483	6.50	42.67

PEAK FLOW REDUCTION [Qout/Qin] (%) =	28.17
TIME SHIFT OF PEAK FLOW (min) =	30.00
MAXIMUM STORAGE USED (ha.m.) =	1.0196

ADD HYD (9261) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2221):	110.64	5.625	6.00	28.03
+ ID2= 2 (9288):	43.20	1.483	6.50	42.67

ID = 3 (9261):	153.84	6.292	6.25	32.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5222) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	7.2210	4.0213
	0.4910	2.1348	9.0080	4.5380
	2.1830	2.4433	15.0420	4.9977
	3.9370	2.9883	15.4420	5.0077
	5.4550	3.4692	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9261)	153.840	6.292	6.25	32.14
OUTFLOW: ID= 1 (5222)	153.840	1.927	7.50	32.14

PEAK FLOW REDUCTION [Qout/Qin] (%) =	30.63
TIME SHIFT OF PEAK FLOW (min) =	75.00
MAXIMUM STORAGE USED (ha.m.) =	2.4018

CALIB STANDHYD (2222) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	65.69 84.00	Dir. Conn.(%)=	45.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	55.18	10.51
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50

Length (m) = 661.77 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr) = 71.81 191.32
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 11.16 (ii) 19.41 (ii)
 Uni t Hyd. Tpeak (min) = 15.00 30.00
 Uni t Hyd. peak (cms) = 0.08 0.05

TOTALS
 PEAK FLOW (cms) = 4.81 2.92 6.797 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 52.40 36.32 43.56
 TOTAL RAINFALL (mm) = 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.67 0.80

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8164) 1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2222):	65.69	6.797	6.00	43.56
+ ID2= 2 (5222):	153.84	1.927	7.50	32.14

ID = 3 (8164):	219.53	6.960	6.00	35.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2242) ID= 1 DT=15.0 min			
Area Total	(ha)	Imp(%)	Di r. Conn.(%)
	52.75	36.00	27.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 18.99 33.76
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 593.01 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 71.81 25.05
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 10.45 (ii) 29.06 (ii)
 Uni t Hyd. Tpeak (min) = 15.00 30.00
 Uni t Hyd. peak (cms) = 0.09 0.04

TOTALS
 PEAK FLOW (cms) = 2.37 1.60 3.362 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 52.40 22.91 30.87
 TOTAL RAINFALL (mm) = 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.42 0.57

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2244) ID= 1 DT=15.0 min			
Area Total	(ha)	Imp(%)	Di r. Conn.(%)
	87.70	70.00	64.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 61.39 26.31
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 764.63 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 71.81 44.29
 over (min) = 15.00 30.00

Storage Coeff. (min) = 12.17 (ii) 26.99 (ii)
 Uni t Hyd. Tpeak (min) = 15.00 30.00
 Uni t Hyd. peak (cms) = 0.08 0.04

TOTALS
 PEAK FLOW (cms) = 8.86 1.41 9.744 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 52.40 23.69 42.07
 TOTAL RAINFALL (mm) = 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.44 0.77

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9287) IN= 2--> OUT= 1 DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	4.3900	2.3776
	0.3000	1.4858	5.8700	2.7087
	1.7000	1.7770	7.3700	2.9557
	3.1700	2.0741	8.5800	3.1980
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2244)	87.700	9.744	6.00	42.07
OUTFLOW: ID= 1 (9287)	87.700	2.845	6.50	42.05
PEAK FLOW REDUCTION [Qout/Qin](%) = 29.20 TIME SHIFT OF PEAK FLOW (min) = 30.00 MAXIMUM STORAGE USED (ha. m.) = 2.0240				

RESERVOIR (5224) IN= 2--> OUT= 1 DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	0.4470	0.1810
	0.0230	0.0884	0.5620	0.2047
	0.1300	0.1032	0.6540	0.2278
	0.2410	0.1297	1.0540	0.2378
	0.3350	0.1532	0.0000	0.0000
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9287)	87.700	2.845	6.50	42.05
OUTFLOW: ID= 1 (5224)	87.700	2.938	6.75	42.04
PEAK FLOW REDUCTION [Qout/Qin](%) = 103.29 TIME SHIFT OF PEAK FLOW (min) = 15.00 MAXIMUM STORAGE USED (ha. m.) = 0.2941				

**** WARNING : HYDROGRAPH PEAK WAS NOT REDUCED.
 CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.

ADD HYD (9224) 1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2242):	52.75	3.362	6.00	30.87
+ ID2= 2 (5224):	87.70	2.938	6.75	42.04

ID = 3 (9224):	140.45	4.116	6.75	37.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8166) 1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8164):	219.53	6.960	6.00	35.55
+ ID2= 2 (9224):	140.45	4.116	6.75	37.84

ID = 3 (8166):	359.98	10.342	6.00	36.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6226)
IN= 2--> OUT= 1
Routing time step (min) = 15.00

DATA FOR SECTION (2261.0) ----->

Distance	Elevation	Manning	Main Channel
0.00	245.35	0.0600	
8.27	242.72	0.0600	
16.54	240.01	0.0600	
31.02	235.31	0.0600	
41.36	233.14	0.0600	
90.99	227.73	0.0600	
93.06	227.50	0.0600	
95.13	227.27	0.0600	
95.83	227.00	0.0600	
100.83	226.50	0.0600 / 0.0350	Main Channel
101.33	225.50	0.0350	Main Channel
101.83	226.50	0.0350 / 0.0600	Main Channel
105.83	227.00	0.0600	
107.54	227.39	0.0600	
109.60	227.62	0.0600	
111.67	227.86	0.0600	
146.83	231.76	0.0600	
173.71	234.77	0.0600	
186.12	238.23	0.0600	
202.67	238.35	0.0600	

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	226.00	.403E+03	0.0	0.16	330.59
1.00	226.50	.161E+04	0.1	0.26	208.26
1.70	227.20	.173E+05	1.5	0.27	195.72
2.39	227.89	.541E+05	5.9	0.35	152.27
3.09	228.59	.119E+06	16.0	0.43	124.08
3.79	229.29	.212E+06	33.8	0.51	104.65
4.49	229.99	.334E+06	61.2	0.59	90.97
5.18	230.68	.484E+06	99.7	0.66	80.88
5.88	231.38	.663E+06	151.0	0.73	73.13
6.58	232.08	.870E+06	216.5	0.80	66.95
7.27	232.77	.111E+07	297.5	0.87	61.91
7.97	233.47	.137E+07	398.1	0.94	57.29
8.67	234.17	.165E+07	518.9	1.01	53.12
9.36	234.86	.196E+07	658.9	1.08	49.59
10.06	235.56	.228E+07	827.0	1.17	46.02
10.76	236.26	.262E+07	1015.4	1.25	42.96
11.46	236.96	.296E+07	1221.2	1.33	40.41
12.15	237.65	.332E+07	1444.6	1.40	38.26
12.85	238.35	.368E+07	1598.2	1.40	38.42

hydrograph ----->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8166)	359.98	10.34	6.00	36.45	2.70
OUTFLOW: ID= 1 (6226)	359.98	3.42	7.50	36.44	2.00

CALIB NASHYD (2267)
ID= 1 DT=15.0 min
Area (ha) = 10.50
Curve Number (CN) = 82.0
U. H. Tp (hrs) = 0.47
of Linear Res. (N) = 1.50

Unit Hyd Qpeak (cms) = 0.383
PEAK FLOW (cms) = 0.208 (i)
TIME TO PEAK (hrs) = 6.500
RUNOFF VOLUME (mm) = 22.481
TOTAL RAINFALL (mm) = 54.400
RUNOFF COEFFICIENT = 0.413

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9276)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.5000	8.0000
0.0400	0.0402	3.5000	10.0000
0.2000	0.0341	4.5000	13.0000
0.3800	0.0910	5.5000	14.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
0.0000	0.0000	0.5300	0.1297
0.0400	0.0402	0.7100	0.1837
0.2000	0.0341	0.8900	0.2402
0.3800	0.0910	1.0300	0.2954

INFLOW: ID= 2 (2267) 10.500 0.208 6.50
OUTFLOW: ID= 1 (9276) 10.500 0.225 7.50

PEAK FLOW REDUCTION [Out/Oi n] (%) = 107.92
TIME SHIFT OF PEAK FLOW (min) = 60.00
MAXIMUM STORAGE USED (ha.m.) = 0.0363

**** WARNING : HYDROGRAPH PEAK WAS NOT REDUCED.
CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.
**** ERROR : CHECK THE STORAGE-DISCHARGE TABLE.

CALIB STANDHYD (2264)
ID= 1 DT=15.0 min
Area (ha) = 117.80
Total Imp (%) = 75.00
Dir. Conn. (%) = 70.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) = 88.35	29.45
Dep. Storage (mm) = 2.00	5.00
Average Slope (%) = 0.50	0.50
Length (m) = 886.19	40.00
Mannings n = 0.013	0.250
Max. Eff. Inten. (mm/hr) = 71.81	44.29
over (min) = 15.00	30.00
Storage Coeff. (min) = 13.30 (ii)	28.11 (ii)
Unit Hyd. Tpeak (min) = 15.00	30.00
Unit Hyd. peak (cms) = 0.08	0.04
PEAK FLOW (cms) = 12.59	1.55
TIME TO PEAK (hrs) = 6.00	6.25
RUNOFF VOLUME (mm) = 52.40	23.69
TOTAL RAINFALL (mm) = 54.40	54.40
RUNOFF COEFFICIENT = 0.96	0.44

TOTALS
13.555 (iii)
6.00
43.79
54.40
0.80

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9277)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.5000	8.0000
0.3000	2.0000	3.5000	10.0000
0.9000	5.0000	4.5000	13.0000
2.0000	7.5000	5.5000	14.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (2264)	117.800	13.555	6.00
OUTFLOW: ID= 1 (9277)	117.800	0.680	8.50

PEAK FLOW REDUCTION [Out/Oi n] (%) = 5.02
TIME SHIFT OF PEAK FLOW (min) = 150.00
MAXIMUM STORAGE USED (ha.m.) = 3.9015

ADD HYD (9253)
1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (9276):	10.50	0.225	7.50
+ ID2= 2 (9277):	117.80	0.680	8.50
ID = 3 (9253):	128.30	0.885	7.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5226)
IN= 2----> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.7530	0.6823
0.0890	0.3202	2.2010	0.7801
0.5080	0.3704	2.5620	0.8758
0.9450	0.4782	2.9620	0.8858
1.3110	0.5708	0.0000	0.0000

INFLOW : ID= 2 (9253) 128.300 0.885 7.50 42.03
OUTFLOW: ID= 1 (5226) 128.300 0.769 9.25 42.02

PEAK FLOW REDUCTION [Qout/Qin] (%) = 86.81
TIME SHIFT OF PEAK FLOW (min) = 105.00
MAXIMUM STORAGE USED (ha.m.) = 0.4425

CALIB NASHYD (2262)
ID= 1 DT=15.0 min

Area (ha) = 119.28 Curve Number (CN) = 77.1
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U.H. Tp(hrs) = 1.05

Unit Hyd Opeak (cms) = 1.934

PEAK FLOW (cms) = 1.039 (i)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 19.363
TOTAL RAINFALL (mm) = 54.400
RUNOFF COEFFICIENT = 0.356

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9226)
1 + 2 = 3

ID	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (2262):	119.28	1.039	7.50	19.36
+ ID2= 2 (5226):	128.30	0.769	9.25	42.02
ID = 3 (9226):	247.58	1.719	8.25	31.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8168)
1 + 2 = 3

ID	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6226):	359.98	3.422	7.50	36.44
+ ID2= 2 (9226):	247.58	1.719	8.25	31.11
ID = 3 (8168):	607.56	5.085	7.75	34.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8162)
1 + 2 = 3

ID	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8160):	16746.20	73.021	9.50	18.32
+ ID2= 2 (8168):	607.56	5.085	7.75	34.27
ID = 3 (8162):	17353.76	77.376	9.25	18.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6228)
IN= 2----> OUT= 1

Routing time step (min) = 15.00

----- DATA FOR SECTION (2281.0) -----
Distance 0.00 Elevation 245.00 Manning 0.0600

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
18.60	244.74	0.0600			
37.20	242.47	0.0600			
93.01	233.71	0.0600			
125.57	226.95	0.0600			
213.93	221.63	0.0600			
218.58	221.15	0.0600			
223.23	220.45	0.0600			
225.03	220.55	0.0600 / 0.0300	0.0300	0.0300	Main Channel
225.53	218.05	0.0300			Main Channel
232.53	218.05	0.0300			Main Channel
240.03	218.05	0.0300			Main Channel
241.03	220.55	0.0300 / 0.0600	0.0600		Main Channel
246.48	221.00	0.0600			
251.13	221.27	0.0600			
255.78	221.22	0.0600			
325.54	225.25	0.0600			
367.40	228.91	0.0600			
404.60	231.75	0.0600			
460.41	245.00	0.0600			

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
1.25	219.30	.338E+05	16.0	0.86	35.15
2.50	220.55	.696E+05	48.3	1.26	24.02
3.94	221.99	.168E+06	116.0	1.26	24.14
5.38	223.43	.392E+06	247.5	1.15	26.37
6.81	224.86	.743E+06	472.6	1.16	26.20
8.25	226.30	.122E+07	821.6	1.23	24.67
9.69	227.74	.179E+07	1320.1	1.34	22.59
11.13	229.18	.243E+07	1965.7	1.47	20.58
12.57	230.62	.313E+07	2744.2	1.59	19.02
14.01	232.06	.390E+07	3683.4	1.72	17.66
15.44	233.49	.472E+07	4824.1	1.86	16.30
16.88	234.93	.557E+07	6092.4	1.99	15.23
18.32	236.37	.646E+07	7503.3	2.11	14.35
19.76	237.81	.739E+07	9061.2	2.23	13.59
21.20	239.25	.836E+07	10767.6	2.34	12.94
22.64	240.69	.937E+07	12624.8	2.45	12.37
24.07	242.12	.104E+08	14634.9	2.56	11.87
25.51	243.56	.115E+08	16762.8	2.65	11.45
26.95	245.00	.127E+08	18699.9	2.69	11.28

----- hydrograph -----
AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm) MAX DEPTH (m) MAX VEL (m/s)
INFLOW : ID= 2 (8162) ***** 77.38 9.25 18.88 3.12 1.26
OUTFLOW: ID= 1 (6228) ***** 76.03 9.75 18.88 3.10 1.26

CALIB STANDHYD (2287)
ID= 1 DT=15.0 min

Area (ha) = 130.50
Total Imp (%) = 23.00 Di r. Conn. (%) = 10.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 30.01 100.48
Dep. Storage (mm) = 2.00 5.00
Average Slope (%) = 0.50 0.50
Length (m) = 932.74 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 71.81 14.09
over (min) = 15.00 45.00
Storage Coeff. (min) = 13.71 (ii) 37.14 (ii)
Unit Hyd. Tpeak (min) = 15.00 45.00
Unit Hyd. peak (cms) = 0.08 0.03

TOTALS
PEAK FLOW (cms) = 1.97 2.19 2.849 (iii)
TIME TO PEAK (hrs) = 6.00 6.50 6.00
RUNOFF VOLUME (mm) = 52.40 13.29 17.20
TOTAL RAINFALL (mm) = 54.40 54.40 54.40
RUNOFF COEFFICIENT = 0.96 0.24 0.32

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 61.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9275)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.5000	3.5000
0.8000	0.1000	6.0000	4.0000
1.5000	2.0000	6.5000	4.5000
2.5000	3.0000	7.0000	5.5000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2287)	130.500	2.849	6.00	17.20
OUTFLOW: ID= 1 (9275)	130.500	1.040	7.50	17.20

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 36.50
TIME SHIFT OF PEAK FLOW (min) = 90.00
MAXIMUM STORAGE USED (ha. m.) = 0.7525

RESERVOIR (5228)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	6.7260	2.6184
0.3410	1.2286	8.4460	2.9937
1.9480	1.4214	9.8320	3.3609
3.6280	1.8348	10.2320	3.3709
5.0330	2.1904	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9275)	130.500	1.040	7.50	17.20
OUTFLOW: ID= 1 (5228)	130.500	0.849	10.75	17.19

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 81.60
TIME SHIFT OF PEAK FLOW (min) = 195.00
MAXIMUM STORAGE USED (ha. m.) = 1.2896

CALIB NASHYD (2282)
ID= 1 DT=15.0 mi n

Area (ha) = 180.03
Curve Number (CN) = 61.9
U. H. Tp(hrs) = 1.12
of Linear Res. (N) = 1.50

Unit Hyd Opeak (cms) = 2.754

PEAK FLOW (cms) = 0.892 (i)
TIME TO PEAK (hrs) = 7.750
RUNOFF VOLUME (mm) = 11.759
TOTAL RAINFALL (mm) = 54.400
RUNOFF COEFFICIENT = 0.216

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9228)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2282):	180.03	0.892	7.75	11.76
+ ID2= 2 (5228):	130.50	0.849	10.75	17.19
-----	-----	-----	-----	-----
ID = 3 (9228):	310.53	1.476	10.50	14.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8170)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6228):	17353.76	76.034	9.75	18.88
+ ID2= 2 (9228):	310.53	1.476	10.50	14.04
-----	-----	-----	-----	-----
ID = 3 (8170):	17664.29	77.297	10.25	18.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2302)
ID= 1 DT=15.0 mi n

Area (ha) = 30.47
Total Imp(%) = 29.00
Dir. Conn. (%) = 11.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 8.84
Dep. Storage (mm) = 2.00
Average Slope (%) = 0.50
Length (m) = 450.70
Mannings n = 0.013

Max. Eff. Inten. (mm/hr) = 71.81
over (min) = 15.00
Storage Coeff. (min) = 8.86 (ii)
Unit Hyd. Tpeak (min) = 15.00
Unit Hyd. peak (cms) = 0.09

PEAK FLOW (cms) = 0.58
TIME TO PEAK (hrs) = 6.00
RUNOFF VOLUME (mm) = 52.40
TOTAL RAINFALL (mm) = 54.40
RUNOFF COEFFICIENT = 0.96

PERVIOUS (i)
21.63
5.00
0.50
40.00
0.250
19.39
30.00
29.48 (ii)
30.00
0.04
TOTALS
1.068 (iii)
6.00
20.64
54.40
0.38

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 66.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2307)
ID= 1 DT=15.0 mi n

Area (ha) = 175.00
Total Imp(%) = 25.00
Dir. Conn. (%) = 9.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 43.75
Dep. Storage (mm) = 2.00
Average Slope (%) = 0.50
Length (m) = 1080.12
Mannings n = 0.013

Max. Eff. Inten. (mm/hr) = 71.81
over (min) = 15.00
Storage Coeff. (min) = 14.97 (ii)
Unit Hyd. Tpeak (min) = 15.00
Unit Hyd. peak (cms) = 0.07

PEAK FLOW (cms) = 2.29
TIME TO PEAK (hrs) = 6.00
RUNOFF VOLUME (mm) = 52.40
TOTAL RAINFALL (mm) = 54.40
RUNOFF COEFFICIENT = 0.96

PERVIOUS (i)
131.25
5.00
0.50
40.00
0.250
24.23
45.00
33.83 (ii)
45.00
0.03
TOTALS
5.797 (iii)
6.50
23.88
54.40
0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9280)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	8.7500	3.0604
0.5900	1.4722	11.7100	3.8736
3.3900	1.6096	14.7000	4.6556
6.3200	2.4308	17.1100	5.4201

AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)

INFLOW : ID= 2 (2307) 175.000 5.797 6.50 23.88
 OUTFLOW: ID= 1 (9280) 175.000 3.605 7.00 23.87

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 62.18
 TIME SHIF T OF PEAK FLOW (mi n) = 30.00
 MAXI MUM STORAGE USED (ha. m.) = 1.6758

RESERVOIR (5230)
 IN= 2--> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	8.6830	2.7782
0.4700	1.3193	10.7480	3.1570
2.5870	1.5374	12.3570	3.5304
5.1590	1.9534	12.7570	3.5404
6.7830	2.3333	0.0000	0.0000

INFLOW : ID= 2 (9280) 175.000 3.605 7.00 23.87
 OUTFLOW: ID= 1 (5230) 175.000 1.299 8.25 23.87

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 36.04
 TIME SHIF T OF PEAK FLOW (mi n) = 75.00
 MAXI MUM STORAGE USED (ha. m.) = 1.4075

ADD HYD (8172)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2302):	30.47	1.068	6.00	20.64
+ ID2= 2 (5230):	175.00	1.299	8.25	23.87
=====				
ID = 3 (8172):	205.47	1.454	8.25	23.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (2329)
 ID= 1 DT=15.0 mi n

Area (ha)= 1.80 Curve Number (CN)= 69.0
 la (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.22

Unit Hyd Qpeak (cms)= 0.143

PEAK FLOW (cms)= 0.038 (i)
 TIME TO PEAK (hrs)= 6.000
 RUNOFF VOLUME (mm)= 13.492
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.248

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9269)
 IN= 2--> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.0900	0.0300
0.0100	0.0136	0.1200	0.0386
0.0400	0.0146	0.1500	0.0471
0.0700	0.0234	0.1800	0.0554

INFLOW : ID= 2 (2329) 1.800 0.038 6.00 13.49
 OUTFLOW: ID= 1 (9269) 1.800 0.009 7.75 13.31

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 23.83
 TIME SHIF T OF PEAK FLOW (mi n)=105.00
 MAXI MUM STORAGE USED (ha. m.) = 0.0123

CALIB STANDHYD (2327)
 ID= 1 DT=15.0 mi n

Area (ha)= 184.00
 Total Imp(%)= 24.00 Dir. Conn. (%) = 8.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 44.16 139.84
 Dep. Storage over (mi n)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1107.55 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 71.81 17.80
 Storage Coeff. (mi n)= 15.00 45.00
 Unit Hyd. Tpeak (mi n)= 15.20 (ii) 36.54 (ii)
 Unit Hyd. peak (cms)= 0.07 0.03

PEAK FLOW (cms)= 2.13 3.88 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.50 4.517 (iii)
 RUNOFF VOLUME (mm)= 52.40 15.96 6.50
 TOTAL RAINFALL (mm)= 54.40 54.40 18.87
 RUNOFF COEFFICIENT = 0.96 0.29 0.35

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 66.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9270)
 IN= 2--> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.0000	4.0000
1.1000	0.3000	3.5000	4.5000
1.8000	1.8000	4.5000	6.0000
2.0000	2.5000	5.5000	8.0000

INFLOW : ID= 2 (2327) 184.000 4.517 6.50 18.87
 OUTFLOW: ID= 1 (9270) 184.000 1.566 7.75 18.87

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 34.66
 TIME SHIF T OF PEAK FLOW (mi n) = 75.00
 MAXI MUM STORAGE USED (ha. m.) = 1.3029

ADD HYD (9249)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9269):	1.80	0.009	7.75	13.31
+ ID2= 2 (9270):	184.00	1.566	7.75	18.87
=====				
ID = 3 (9249):	185.80	1.575	7.75	18.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5232)
 IN= 2--> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	11.1130	4.3262
0.5630	2.0299	13.9540	4.9462
3.2190	2.3484	16.2450	5.5530
5.9950	3.0315	16.6450	5.5630
8.3150	3.6189	0.0000	0.0000

INFLOW : ID= 2 (9249) 185.800 1.575 7.75 18.82
 OUTFLOW: ID= 1 (5232) 185.800 1.128 11.50 18.81

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 71.62
 TIME SHIF T OF PEAK FLOW (mi n)=225.00
 MAXI MUM STORAGE USED (ha. m.) = 2.0980

CALIB NASHYD (2322) Area (ha)= 129.00 Curve Number (CN)= 58.1
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.00

Unit Hyd Opeak (cms)= 2.209
 PEAK FLOW (cms)= 0.612 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 10.385
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.191

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9232)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2322):	129.00	0.612	7.50	10.39
+ ID2= 2 (5232):	185.80	1.128	11.50	18.81
-----	-----	-----	-----	-----
ID = 3 (9232):	314.80	1.449	11.50	15.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8173)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8172):	205.47	1.454	8.25	23.39
+ ID2= 2 (9232):	314.80	1.449	11.50	15.36
-----	-----	-----	-----	-----
ID = 3 (8173):	520.27	2.373	8.25	18.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8174)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8170):	17664.29	77.297	10.25	18.80
+ ID2= 2 (8173):	520.27	2.373	8.25	18.53
-----	-----	-----	-----	-----
ID = 3 (8174):	18184.56	79.043	10.00	18.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6234)
 IN= 2--> OUT= 1
 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2341.0) ----->

Distance	Elevation	Manning	
0.00	228.00	0.0550	
42.86	223.21	0.0550	
85.72	219.56	0.0550	
107.15	219.42	0.0550	
128.58	219.39	0.0550	
214.30	219.30	0.0550	
225.02	219.26	0.0550	
235.73	219.24	0.0550	
255.16	219.20	0.0550 / 0.0350	Main Channel
257.16	217.20	0.0350	Main Channel
303.16	217.20	0.0350	Main Channel
305.16	219.20	0.0350 / 0.0550	Main Channel
310.74	219.28	0.0550	
321.45	219.27	0.0550	
332.17	219.26	0.0550	
396.46	219.29	0.0550	
782.20	222.91	0.0550	
878.64	227.43	0.0550	
964.36	227.80	0.0550	
1060.79	228.00	0.0550	

<----- TRAVEL TIME TABLE ----->
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME

(m)	(m)	(cu. m.)	(cms)	(m/s)	(mi n)
0.50	217.70	.887E+05	7.2	0.31	206.54
1.00	218.20	.179E+06	22.7	0.48	131.74
1.50	218.70	.272E+06	44.5	0.62	101.75
2.00	219.20	.366E+06	71.8	0.75	84.97
2.59	219.79	.993E+06	136.1	0.52	121.59
3.17	220.37	.189E+07	258.6	0.52	121.80
3.76	220.96	.294E+07	434.7	0.56	112.82
4.35	221.55	.415E+07	665.9	0.61	103.88
4.93	222.13	.551E+07	955.0	0.66	96.22
5.52	222.72	.703E+07	1305.1	0.71	89.80
6.11	223.31	.868E+07	1744.9	0.77	82.91
6.69	223.89	.104E+08	2261.4	0.83	76.46
7.28	224.48	.121E+08	2838.0	0.89	71.12
7.87	225.07	.139E+08	3472.9	0.95	66.63
8.45	225.65	.157E+08	4165.0	1.01	62.82
9.04	226.24	.176E+08	4913.5	1.07	59.54
9.63	226.83	.194E+08	5717.7	1.12	56.68
10.21	227.41	.214E+08	6577.1	1.17	54.18
10.80	228.00	.235E+08	6919.5	1.12	56.61

<---- hydrograph ----> <- pipe / channel ->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW: ID= 2 (8174) ***** 79.04 10.00 18.79 2.07 0.71
 OUTFLOW: ID= 1 (6234) ***** 72.43 11.50 18.79 2.01 0.74

CALIB NASHYD (2347) Area (ha)= 266.10 Curve Number (CN)= 56.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 2.00

Unit Hyd Opeak (cms)= 2.276
 PEAK FLOW (cms)= 0.699 (i)
 TIME TO PEAK (hrs)= 9.250
 RUNOFF VOLUME (mm)= 9.766
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.180

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9281)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	13.3100	4.0635
0.9000	1.7152	17.8000	5.3537
5.1600	1.7685	22.3500	6.6446
9.6100	3.0976	26.0300	7.9060

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2347)	266.100	0.699	9.25	9.77
OUTFLOW: ID= 1 (9281)	266.100	0.468	14.00	9.76

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 66.94
 TIME SHI FT OF PEAK FLOW (mi n)=285.00
 MAXI MUM STORAGE USED (ha. m.) = 0.8924

RESERVOIR (5234)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.5630	0.1745
0.1300	0.0819	0.6550	0.1995
0.2420	0.0947	0.7760	0.2240
0.3350	0.1223	1.0550	0.2340
0.4480	0.1460	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9281)	266.100	0.468	14.00	9.76
OUTFLOW: ID= 1 (5234)	266.100	0.465	14.50	9.76

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 99.28
 TIME SHI FT OF PEAK FLOW (mi n) = 30.00

MAXIMUM STORAGE USED (ha. m.) = 0.1502

CALIB STANDHYD (2342)
ID= 1 DT=15.0 min

Area Total	(ha)= 1.06	Dir. Conn. (%)= 8.00
Imp(%)= 22.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.23	0.83
Dep. Storage (mm)=	1.00	5.00
Average Slope (%)=	1.00	2.00
Length (m)=	84.06	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	71.81	6.79
over (min)	15.00	30.00
Storage Coeff. (min)=	2.63 (ii)	23.32 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.11	0.04

		TOTALS
PEAK FLOW (cms)=	0.02	0.024 (iii)
TIME TO PEAK (hrs)=	6.00	6.00
RUNOFF VOLUME (mm)=	53.40	10.33
TOTAL RAINFALL (mm)=	54.40	54.40
RUNOFF COEFFICIENT =	0.98	0.19

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 39.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9234)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2342):	1.06	0.024	6.00	10.33
+ ID2= 2 (5234):	266.10	0.465	14.50	9.76
ID = 3 (9234):	267.16	0.465	14.50	9.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8176)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6234):	18184.56	72.428	11.50	18.79
+ ID2= 2 (9234):	267.16	0.465	14.50	9.77
ID = 3 (8176):	18451.72	72.814	11.50	18.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2369)
ID= 1 DT=15.0 min

Area Total	(ha)= 441.90	Dir. Conn. (%)= 8.00
Imp(%)= 23.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	101.64	340.26
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1716.39	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	48.96	19.20
over (min)	30.00	45.00
Storage Coeff. (min)=	23.04 (ii)	43.74 (ii)
Unit Hyd. Tpeak (min)=	30.00	45.00
Unit Hyd. peak (cms)=	0.04	0.03

		TOTALS
PEAK FLOW (cms)=	3.55	11.499 (iii)
TIME TO PEAK (hrs)=	6.25	6.50

RUNOFF VOLUME (mm)=	52.40	17.30	20.11
TOTAL RAINFALL (mm)=	54.40	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.32	0.37

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9266)
IN= 2----> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.3000	8.0000
0.2000	2.0000	3.5000	10.0000
1.9000	7.0000	4.5000	13.0000
2.0000	7.5000	5.5000	14.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2369)	441.900	11.499	6.50	20.11
OUTFLOW: ID= 1 (9266)	441.900	1.582	9.75	20.11

PEAK FLOW REDUCTION [Out/Oi n] (%) = 13.76
TIME SHIFT OF PEAK FLOW (min) = 195.00
MAXIMUM STORAGE USED (ha. m.) = 6.0638

RESERVOIR (5236)
IN= 2----> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.1380	1.5897
1.0220	0.7542	3.7950	2.0204
1.7530	0.8200	4.4970	2.4373
2.3290	1.2567	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9266)	441.900	1.582	9.75	20.11
OUTFLOW: ID= 1 (5236)	441.900	1.574	10.50	20.11

PEAK FLOW REDUCTION [Out/Oi n] (%) = 99.50
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.8040

CALIB NASHYD (2362)
ID= 1 DT=15.0 min

Area Total	(ha)= 52.59	Curve Number (CN)= 58.7
Ia (mm)= 5.00		# of Linear Res. (N)= 1.50
U. H. Tp (hrs)= 1.24		

Unit Hyd Opeak (cms)=	0.726
PEAK FLOW (cms)=	0.218 (i)
TIME TO PEAK (hrs)=	8.000
RUNOFF VOLUME (mm)=	10.618
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.195

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9236)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2362):	52.59	0.218	8.00	10.62
+ ID2= 2 (5236):	441.90	1.574	10.50	20.11
ID = 3 (9236):	494.49	1.748	10.25	19.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6238)
IN= 2--> OUT= 1

Routing time step (mi.n)' = 15.00

----- DATA FOR SECTION (2381.0) -----

Distance	Elevation	Manning	
0.00	241.54	0.0380	
602.55	227.00	0.0380	
1702.00	224.50	0.0380	
1721.25	223.00	0.0380	
1725.10	222.60	0.0380	
1780.94	222.50	0.0380	
1782.87	222.45	0.0380	
1784.79	222.40	0.0380 / 0.0300	Main Channel
1786.57	221.75	0.0300	Main Channel
1787.07	220.75	0.0300	Main Channel
1787.57	220.75	0.0300	Main Channel
1791.57	220.75	0.0300	Main Channel
1794.07	221.75	0.0300	Main Channel
1798.27	222.00	0.0300	Main Channel
1800.19	222.35	0.0300 / 0.0380	Main Channel
1802.12	222.47	0.0380	
1840.63	223.00	0.0380	
1900.00	225.00	0.0380	
1950.00	226.00	0.0380	
2242.61	240.00	0.0380	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi.n)
0.80	221.55	.101E+05	5.2	1.14	32.36
1.60	222.35	.296E+05	18.7	1.40	26.36
2.64	223.39	.249E+06	151.8	1.35	27.33
3.68	224.43	.613E+06	512.2	1.85	19.94
4.71	225.46	.152E+07	1066.8	1.55	23.80
5.75	226.50	.359E+07	2824.9	1.75	21.16
6.79	227.54	.663E+07	6528.0	2.18	16.93
7.83	228.58	.994E+07	12158.6	2.71	13.62
8.87	229.62	.134E+08	19268.8	3.19	11.58
9.91	230.66	.170E+08	27785.3	3.63	10.19
10.94	231.69	.207E+08	37667.1	4.03	9.18
11.98	232.73	.246E+08	48891.7	4.40	8.40
13.02	233.77	.287E+08	61448.0	4.75	7.78
14.06	234.81	.329E+08	75332.7	5.08	7.28
15.10	235.85	.372E+08	90547.8	5.39	6.85
16.14	236.89	.417E+08	*****	5.69	6.49
17.17	237.92	.464E+08	*****	5.98	6.18
18.21	238.96	.512E+08	*****	6.25	5.91
19.25	240.00	.561E+08	*****	6.52	5.67

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (9236)	494.49	1.75	10.25	19.10	0.27	1.14
OUTFLOW: ID= 1 (6238)	494.49	1.70	11.25	19.10	0.26	1.14

CALIB STANDHYD (2389)
ID= 1 DT=15.0 mi.n

Area (ha)= 4.70
Total Imp(%)= 21.00
Dir. Conn.(%)= 7.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.99	3.71
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	177.01	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	71.81	18.67
over (mi.n)	15.00	30.00
Storage Coeff. (mi.n)=	5.06 (ii)	25.99 (ii)
Unit Hyd. Tpeak (mi.n)=	15.00	30.00
Unit Hyd. peak (cms)=	0.11	0.04

	PEAK FLOW (cms)	TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
	0.06	0.14	52.40	54.40	0.96
	0.152	6.25	19.57	54.40	0.31
	0.152	6.25	19.57	54.40	0.36

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%

YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9267)
IN= 2--> OUT= 1
DT= 15.0 mi.n

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.2300	0.0785
0.0200	0.0364	0.3100	0.1005
0.0900	0.0393	0.3900	0.1220
0.1700	0.0616	0.4600	0.1429

INFLOW : ID= 2 (2389)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	4.700	0.152	6.25	19.57
OUTFLOW: ID= 1 (9267)	4.700	0.093	6.75	19.47

PEAK FLOW REDUCTION [Qout/Qin] (%) = 61.00
TIME SHIFT OF PEAK FLOW (mi.n) = 30.00
MAXIMUM STORAGE USED (ha.m.) = 0.0405

CALIB NASHYD (2387)
ID= 1 DT=15.0 mi.n

Area (ha)= 90.20
Ia (mm)= 5.00
U.H. Tp(hrs)= 0.86
Curve Number (CN)= 54.0
of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 1.791

PEAK FLOW (cms)=	0.417 (i)
TIME TO PEAK (hrs)=	7.000
RUNOFF VOLUME (mm)=	9.065
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.167

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9268)
IN= 2--> OUT= 1
DT= 15.0 mi.n

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.5100	1.3564
0.3100	0.5624	6.0400	1.7959
1.7500	0.5748	7.5800	2.2374
3.2600	1.0285	8.8300	2.6688

INFLOW : ID= 2 (2387)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	90.200	0.417	7.00	9.06
OUTFLOW: ID= 1 (9268)	90.200	0.203	11.00	9.06

PEAK FLOW REDUCTION [Qout/Qin] (%) = 48.63
TIME SHIFT OF PEAK FLOW (mi.n)=240.00
MAXIMUM STORAGE USED (ha.m.) = 0.3680

ADD HYD (9247)
1 + 2 = 3

ID= 1 (9267):	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	4.70	0.093	6.75	19.47
+ ID2= 2 (9268):	90.20	0.203	11.00	9.06
-----	-----	-----	-----	-----
ID = 3 (9247):	94.90	0.220	11.00	9.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5238)
IN= 2--> OUT= 1
DT= 15.0 mi.n

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
---------------	-----------------	---------------	-----------------

0.0000	0.0000	3.8780	1.5095
0.1960	0.7083	4.8600	1.7258
1.1230	0.8194	5.6680	1.9375
2.0920	1.0578	6.0680	1.9475
2.9010	1.2627	0.0000	0.0000

INFLOW : ID= 2 (9247) 94.900 0.220 11.00 9.58
 OUTFLOW: ID= 1 (5238) 94.900 0.115 17.00 9.57

PEAK FLOW REDUCTION [Qout/Qin] (%) = 52.28
 TIME SHIFT OF PEAK FLOW (min) = 360.00
 MAXIMUM STORAGE USED (ha.m.) = 0.4162

CALIB NASHYD (2382) Area (ha) = 216.80 Curve Number (CN) = 56.8
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.56

Unit Hyd Opeak (cms) = 2.368
 PEAK FLOW (cms) = 0.705 (i)
 TIME TO PEAK (hrs) = 8.250
 RUNOFF VOLUME (mm) = 10.007
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.184

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9238) 1 + 2 = 3
 ID1= 1 (2382): 216.80 0.705 8.25 10.01
 + ID2= 2 (5238): 94.90 0.115 17.00 9.57
 ID = 3 (9238): 311.70 0.734 8.75 9.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8180) 1 + 2 = 3
 ID1= 1 (6238): 494.49 1.698 11.25 19.10
 + ID2= 2 (9238): 311.70 0.734 8.75 9.87
 ID = 3 (8180): 806.19 2.359 11.00 15.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8178) 1 + 2 = 3
 ID1= 1 (8176): 18451.72 72.814 11.50 18.66
 + ID2= 2 (8180): 806.19 2.359 11.00 15.53
 ID = 3 (8178): 19257.91 75.143 11.50 18.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6240) IN= 2---> OUT= 1 Routing time step (min) = 15.00

<----- DATA FOR SECTION (2401.0) ----->

Distance	Elevation	Manning
0.00	222.00	0.0450
11.46	221.00	0.0450
208.98	221.00	0.0450
404.04	220.97	0.0450
808.08	220.83	0.0450
905.60	220.17	0.0450

919.53	219.43	0.0450		
933.47	219.22	0.0450		
945.26	219.21	0.0450 / 0.0300	Main	Channel
946.26	217.81	0.0300	Main	Channel
975.26	217.81	0.0300	Main	Channel
1003.26	217.81	0.0300	Main	Channel
1005.26	219.21	0.0300 / 0.0450	Main	Channel
1017.06	219.28	0.0450		
1030.99	219.26	0.0450		
1044.92	219.23	0.0450		
1058.86	219.23	0.0450		
1253.91	219.22	0.0450		
1323.57	221.05	0.0450		
1379.30	222.00	0.0450		

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.20	218.01	.307E+05	2.2	0.20	227.52
0.40	218.21	.617E+05	7.1	0.31	143.95
0.60	218.41	.928E+05	14.0	0.41	110.33
0.80	218.61	.124E+06	22.6	0.49	91.46
1.00	218.81	.156E+06	32.8	0.57	79.15
1.20	219.01	.188E+06	44.5	0.64	70.38
1.40	219.21	.220E+06	57.5	0.70	63.77
1.63	219.44	.413E+06	82.5	0.54	83.52
1.87	219.68	.632E+06	120.9	0.51	87.03
2.10	219.91	.858E+06	170.0	0.53	84.11
2.33	220.14	.109E+07	228.7	0.56	79.62
2.56	220.37	.134E+07	294.4	0.59	76.03
2.80	220.61	.162E+07	371.1	0.61	72.75
3.03	220.84	.192E+07	456.8	0.64	70.18
3.26	221.07	.251E+07	552.1	0.59	75.74
3.49	221.30	.333E+07	711.8	0.57	78.07
3.73	221.54	.417E+07	907.0	0.58	76.62
3.96	221.77	.502E+07	1134.1	0.61	73.70
4.19	222.00	.587E+07	1390.9	0.64	70.34

<---- hydrograph ----> <- pipe / channel ->
 INFLOW : ID= 2 (8178) AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm) MAX DEPTH (m) MAX VEL (m/s)
 OUTFLOW: ID= 1 (6240) ***** 75.14 11.50 18.53 1.56 0.58
 69.00 12.75 18.53 1.50 0.62

CALIB NASHYD (0240) Area (ha) = 345.47 Curve Number (CN) = 46.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 3.60

Unit Hyd Opeak (cms) = 1.637
 PEAK FLOW (cms) = 0.416 (i)
 TIME TO PEAK (hrs) = 12.000
 RUNOFF VOLUME (mm) = 7.010
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.129

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2407) Area (ha) = 88.90
 ID= 1 DT=15.0 min Total Imp(%) = 20.00 Dir. Conn. (%) = 7.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 17.78 71.12
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 769.85 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 71.81 15.94
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 12.22 (ii) 34.52 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.08 0.03

PEAK FLOW (cms) = 0.98 1.82 *TOTALS*
 TIME TO PEAK (hrs) = 6.00 6.50 2.060 (iii)
 RUNOFF VOLUME (mm) = 52.40 14.96 6.50 17.58

TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.28 0.32

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9273)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.4500	5.4723
0.3000	4.6746	5.9500	5.8930
1.7300	4.7225	7.4700	6.3047
3.2100	5.1511	8.7000	6.7071

INFLOW : ID= 2 (2407) 88.900 2.060 6.50 17.58
 OUTFLOW: ID= 1 (9273) 88.900 0.088 12.50 17.54

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 4.27
 TIME SHIFT OF PEAK FLOW (min) = 360.00
 MAXIMUM STORAGE USED (ha.m.) = 1.3713

ADD HYD (9252)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0240):	345.47	0.416	12.00	7.01
+ ID2= 2 (9273):	88.90	0.088	12.50	17.54
ID = 3 (9252):	434.37	0.503	12.00	9.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8182)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6240):	19257.91	68.998	12.75	18.53
+ ID2= 2 (9252):	434.37	0.503	12.00	9.17
ID = 3 (8182):	19692.28	69.493	12.75	18.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (2427)
 ID= 1 DT=15.0 min

Area (ha)= 5.40 Curve Number (CN)= 74.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.37

Unit Hyd Opeak (cms)= 0.249
 PEAK FLOW (cms)= 0.092 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 16.826
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.309

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9274)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.2700	0.0857
0.0200	0.0379	0.3600	0.1115

0.1000 0.0399 0.4500 0.1369
 0.1900 0.0663 0.5300 0.1618

INFLOW : ID= 2 (2427) 5.400 0.092 6.50 16.83
 OUTFLOW: ID= 1 (9274) 5.400 0.070 7.50 16.74

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 75.91
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0399

RESERVOIR (5242)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0480	1.0185
0.0020	1.0087	0.0600	1.0212
0.0140	1.0101	0.0700	1.0238
0.0260	1.0130	0.4700	1.0338
0.0360	1.0155	0.0000	0.0000

INFLOW : ID= 2 (9274) 5.400 0.070 7.50 16.74
 OUTFLOW: ID= 1 (5242) 5.400 0.000 35.00 0.27

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 0.25
 TIME SHIFT OF PEAK FLOW (min) = *****
 MAXIMUM STORAGE USED (ha.m.) = 0.0889

CALIB NASHYD (2422)
 ID= 1 DT=15.0 min

Area (ha)= 652.48 Curve Number (CN)= 51.6
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 5.37

Unit Hyd Opeak (cms)= 2.073
 PEAK FLOW (cms)= 0.671 (i)
 TIME TO PEAK (hrs)= 13.000
 RUNOFF VOLUME (mm)= 8.477
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.156

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9242)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2422):	652.48	0.671	13.00	8.48
+ ID2= 2 (5242):	5.40	0.000	35.00	0.27
ID = 3 (9242):	657.88	0.671	13.00	8.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8184)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8182):	19692.28	69.493	12.75	18.32
+ ID2= 2 (9242):	657.88	0.671	13.00	8.41
ID = 3 (8184):	20350.16	70.164	12.75	18.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6244)
 IN= 2---> OUT= 1

Routing time step (min)' = 15.00
 <----- DATA FOR SECTION (2441.0) ----->
 Distance El evation Manning
 0.00 225.00 0.0450

2.50	220.10	0.0450
3.00	220.06	0.0450
3.64	220.07	0.0450
7.28	220.08	0.0450
10.91	220.09	0.0450
14.55	219.81	0.0450
18.19	219.29	0.0450
24.13	219.21	0.0450 / 0.0300
24.63	217.81	0.0300
69.13	217.81	0.0300
114.13	217.81	0.0300
115.13	219.21	0.0300 / 0.0450
120.06	219.25	0.0450
123.70	219.32	0.0450
127.34	219.77	0.0450
130.98	219.88	0.0450
134.61	219.99	0.0450
350.00	220.18	0.0450
360.18	225.00	0.0450

TRAVEL TIME TABLE						
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)	
0.35	218.16	.108E+06	9.0	0.29	200.52	
0.70	218.51	.216E+06	28.4	0.45	126.89	
1.05	218.86	.325E+06	55.7	0.59	97.27	
1.40	219.21	.434E+06	89.7	0.71	80.65	
1.79	219.60	.574E+06	135.9	0.81	70.36	
2.17	219.98	.726E+06	190.8	0.90	63.38	
2.56	220.37	.111E+07	264.3	0.82	69.82	
2.94	220.75	.157E+07	371.7	0.81	70.36	
3.33	221.14	.203E+07	503.4	0.85	67.30	
3.72	221.53	.250E+07	656.6	0.90	63.40	
4.10	221.91	.296E+07	829.6	0.96	59.54	
4.49	222.30	.343E+07	1021.1	1.02	56.00	
4.87	222.68	.390E+07	1230.2	1.08	52.83	
5.26	223.07	.437E+07	1456.2	1.14	50.01	
5.65	223.46	.484E+07	1698.4	1.20	47.51	
6.03	223.84	.531E+07	1956.3	1.26	45.28	
6.42	224.23	.579E+07	2229.3	1.32	43.28	
6.80	224.61	.626E+07	2517.0	1.38	41.48	
7.19	225.00	.674E+07	2819.2	1.44	39.85	

<--- hydrograph --->						
		AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	<--- pipe / channel --->
						MAX. DEPTH (m) MAX VEL (m/s)
INFLOW: ID= 2 (8184)		*****	70.16	12.75	18.00	1.20 0.63
OUTFLOW: ID= 1 (6244)		*****	65.79	14.00	18.00	1.15 0.62

CALIB STANDHYD (2441)	Area (ha)= 91.52	Dir. Conn. (%)= 15.00
ID= 1 DT=15.0 mi n	Total Imp(%)= 45.00	

Surface Area (ha)= 41.18	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)= 2.00		
Average Slope (%)= 0.50		
Length (m)= 781.11		
Mannings n = 0.013		0.250

Max. Eff. Inten. (mm/hr)= 71.81	23.87
over (mi n)= 15.00	45.00
Storage Coeff. (mi n)= 12.33 (ii)	31.30 (ii)
Unit Hyd. Tpeak (mi n)= 15.00	45.00
Unit Hyd. peak (cms)= 0.08	0.03

PEAK FLOW (cms)= 2.16	2.02	*TOTALS*	2.997 (iii)
TIME TO PEAK (hrs)= 6.00	6.50		6.00
RUNOFF VOLUME (mm)= 52.40	16.71		22.06
TOTAL RAINFALL (mm)= 54.40	54.40		54.40
RUNOFF COEFFICIENT = 0.96	0.31		0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 60.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2449)	Area (ha)= 143.20	Dir. Conn. (%)= 17.00
ID= 1 DT=15.0 mi n	Total Imp(%)= 34.00	

Surface Area (ha)= 48.69	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)= 2.00		94.51
Average Slope (%)= 0.50		5.00
Length (m)= 977.07		40.00
Mannings n = 0.013		0.250

Max. Eff. Inten. (mm/hr)= 71.81	21.11
over (mi n)= 15.00	45.00
Storage Coeff. (mi n)= 14.10 (ii)	34.03 (ii)
Unit Hyd. Tpeak (mi n)= 15.00	45.00
Unit Hyd. peak (cms)= 0.07	0.03

PEAK FLOW (cms)= 3.63	3.22	*TOTALS*	4.958 (iii)
TIME TO PEAK (hrs)= 6.00	6.50		6.00
RUNOFF VOLUME (mm)= 52.40	18.01		23.86
TOTAL RAINFALL (mm)= 54.40	54.40		54.40
RUNOFF COEFFICIENT = 0.96	0.33		0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9265)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2---> OUT= 1	0.0000	0.0000	7.1600	2.7857
DT= 15.0 mi n	0.4900	1.4537	9.5800	3.4259
	2.7800	1.6401	12.0300	4.0179
	5.1700	2.2743	14.0100	4.5968

INFLOW: ID= 2 (2449)	143.200	4.958	6.00	23.86
OUTFLOW: ID= 1 (9265)	143.200	2.504	7.00	23.85

PEAK FLOW REDUCTION [Qout/Qi n] (%)= 50.49
TIME SHIFT OF PEAK FLOW (mi n)= 60.00
MAXIMUM STORAGE USED (ha. m.)= 1.6212

ADD HYD (9245)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (2441):	91.52	2.997	6.00	22.06
+ ID2= 2 (9265):	143.20	2.504	7.00	23.85
ID = 3 (9245):	234.72	3.931	7.00	23.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5244)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2---> OUT= 1	0.0000	0.0000	15.7000	11.1120
DT= 15.0 mi n	0.7950	7.8678	19.7140	11.9879
	4.5470	8.3178	22.9500	12.8451
	8.4690	9.2829	23.3500	12.8551
	11.7470	10.1128	0.0000	0.0000

INFLOW: ID= 2 (9245)	234.720	3.931	7.00	23.15
----------------------	---------	-------	------	-------

OUTFLOW: ID= 1 (5244) 234.720 0.352 14.75 23.14

PEAK FLOW REDUCTION [Qout/Qin] (%) = 8.95
TIME SHIFT OF PEAK FLOW (min) = 465.00
MAXIMUM STORAGE USED (ha.m.) = 3.4801

CALIB NASHVD (2442) Area (ha) = 1674.00 Curve Number (CN) = 62.1
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U.H. Tp(hrs) = 2.20

Unit Hyd Qpeak (cms) = 12.991

PEAK FLOW (cms) = 5.004 (i)
TIME TO PEAK (hrs) = 9.750
RUNOFF VOLUME (mm) = 11.900
TOTAL RAINFALL (mm) = 54.400
RUNOFF COEFFICIENT = 0.219

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9244) AREA OPEAK TPEAK R.V.
1 + 2 = 3 (ha) (cms) (hrs) (mm)
ID1= 1 (2442): 1674.00 5.004 9.75 11.90
+ ID2= 2 (5244): 234.72 0.352 14.75 23.14

ID = 3 (9244): 1908.72 5.319 10.00 13.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8186) AREA OPEAK TPEAK R.V.
1 + 2 = 3 (ha) (cms) (hrs) (mm)
ID1= 1 (6244): 20350.16 65.786 14.00 18.00
+ ID2= 2 (9244): 1908.72 5.319 10.00 13.28

ID = 3 (8186): 22258.88 69.701 14.00 17.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ HYD (8190) AREA (ha) = *****
DT=15.0 min TPEAK (hrs) = 0.00
VOLUME (mm) = 0.00

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HOWEVER THE TABLE BELOW AND HYDROGRAPH FILES DO.

ACTUAL PEAK FLOW: PEAK (0.00) + BASE (*****) = ***** (cms).

TIME hrs	FLOW cms	TIME hrs	FLOW cms	TIME hrs	FLOW cms	TIME hrs	FLOW cms	TIME hrs	FLOW cms
0.00	*****	60.50	*****	121.00	*****	181.50	*****	242.00	*****
0.25	*****	60.75	*****	121.25	*****	181.75	*****	242.25	*****
0.50	*****	61.00	*****	121.50	*****	182.00	*****	242.50	*****
0.75	*****	61.25	*****	121.75	*****	182.25	*****	242.75	*****
1.00	*****	61.50	*****	122.00	*****	182.50	*****	243.00	*****
1.25	*****	61.75	*****	122.25	*****	182.75	*****	243.25	*****
1.50	*****	62.00	*****	122.50	*****	183.00	*****	243.50	*****
1.75	*****	62.25	*****	122.75	*****	183.25	*****	243.75	*****
2.00	*****	62.50	*****	123.00	*****	183.50	*****	244.00	*****
2.25	*****	62.75	*****	123.25	*****	183.75	*****	244.25	*****
2.50	*****	63.00	*****	123.50	*****	184.00	*****	244.50	*****
2.75	*****	63.25	*****	123.75	*****	184.25	*****	244.75	*****
3.00	*****	63.50	*****	124.00	*****	184.50	*****	245.00	*****
3.25	*****	63.75	*****	124.25	*****	184.75	*****	245.25	*****
3.50	*****	64.00	*****	124.50	*****	185.00	*****	245.50	*****
3.75	*****	64.25	*****	124.75	*****	185.25	*****	245.75	*****
4.00	*****	64.50	*****	125.00	*****	185.50	*****	246.00	*****
4.25	*****	64.75	*****	125.25	*****	185.75	*****	246.25	*****
4.50	*****	65.00	*****	125.50	*****	186.00	*****	246.50	*****
4.75	*****	65.25	*****	125.75	*****	186.25	*****	246.75	*****
5.00	*****	65.50	*****	126.00	*****	186.50	*****	247.00	*****
5.25	*****	65.75	*****	126.25	*****	186.75	*****	247.25	*****
5.50	*****	66.00	*****	126.50	*****	187.00	*****	247.50	*****

5.75	*****	66.25	*****	126.75	*****	187.25	*****	247.75	*****
6.00	*****	66.50	*****	127.00	*****	187.50	*****	248.00	*****
6.25	*****	66.75	*****	127.25	*****	187.75	*****	248.25	*****
6.50	*****	67.00	*****	127.50	*****	188.00	*****	248.50	*****
6.75	*****	67.25	*****	127.75	*****	188.25	*****	248.75	*****
7.00	*****	67.50	*****	128.00	*****	188.50	*****	249.00	*****
7.25	*****	67.75	*****	128.25	*****	188.75	*****	249.25	*****
7.50	*****	68.00	*****	128.50	*****	189.00	*****	249.50	*****
7.75	*****	68.25	*****	128.75	*****	189.25	*****	249.75	*****
8.00	*****	68.50	*****	129.00	*****	189.50	*****	250.00	*****
8.25	*****	68.75	*****	129.25	*****	189.75	*****	250.25	*****
8.50	*****	69.00	*****	129.50	*****	190.00	*****	250.50	*****
8.75	*****	69.25	*****	129.75	*****	190.25	*****	250.75	*****
9.00	*****	69.50	*****	130.00	*****	190.50	*****	251.00	*****
9.25	*****	69.75	*****	130.25	*****	190.75	*****	251.25	*****
9.50	*****	70.00	*****	130.50	*****	191.00	*****	251.50	*****
9.75	*****	70.25	*****	130.75	*****	191.25	*****	251.75	*****
10.00	*****	70.50	*****	131.00	*****	191.50	*****	252.00	*****
10.25	*****	70.75	*****	131.25	*****	191.75	*****	252.25	*****
10.50	*****	71.00	*****	131.50	*****	192.00	*****	252.50	*****
10.75	*****	71.25	*****	131.75	*****	192.25	*****	252.75	*****
11.00	*****	71.50	*****	132.00	*****	192.50	*****	253.00	*****
11.25	*****	71.75	*****	132.25	*****	192.75	*****	253.25	*****
11.50	*****	72.00	*****	132.50	*****	193.00	*****	253.50	*****
11.75	*****	72.25	*****	132.75	*****	193.25	*****	253.75	*****
12.00	*****	72.50	*****	133.00	*****	193.50	*****	254.00	*****
12.25	*****	72.75	*****	133.25	*****	193.75	*****	254.25	*****
12.50	*****	73.00	*****	133.50	*****	194.00	*****	254.50	*****
12.75	*****	73.25	*****	133.75	*****	194.25	*****	254.75	*****
13.00	*****	73.50	*****	134.00	*****	194.50	*****	255.00	*****
13.25	*****	73.75	*****	134.25	*****	194.75	*****	255.25	*****
13.50	*****	74.00	*****	134.50	*****	195.00	*****	255.50	*****
13.75	*****	74.25	*****	134.75	*****	195.25	*****	255.75	*****
14.00	*****	74.50	*****	135.00	*****	195.50	*****	256.00	*****
14.25	*****	74.75	*****	135.25	*****	195.75	*****	256.25	*****
14.50	*****	75.00	*****	135.50	*****	196.00	*****	256.50	*****
14.75	*****	75.25	*****	135.75	*****	196.25	*****	256.75	*****
15.00	*****	75.50	*****	136.00	*****	196.50	*****	257.00	*****
15.25	*****	75.75	*****	136.25	*****	196.75	*****	257.25	*****
15.50	*****	76.00	*****	136.50	*****	197.00	*****	257.50	*****
15.75	*****	76.25	*****	136.75	*****	197.25	*****	257.75	*****
16.00	*****	76.50	*****	137.00	*****	197.50	*****	258.00	*****
16.25	*****	76.75	*****	137.25	*****	197.75	*****	258.25	*****
16.50	*****	77.00	*****	137.50	*****	198.00	*****	258.50	*****
16.75	*****	77.25	*****	137.75	*****	198.25	*****	258.75	*****
17.00	*****	77.50	*****	138.00	*****	198.50	*****	259.00	*****
17.25	*****	77.75	*****	138.25	*****	198.75	*****	259.25	*****
17.50	*****	78.00	*****	138.50	*****	199.00	*****	259.50	*****
17.75	*****	78.25	*****	138.75	*****	199.25	*****	259.75	*****
18.00	*****	78.50	*****	139.00	*****	199.50	*****	260.00	*****
18.25	*****	78.75	*****	139.25	*****	199.75	*****	260.25	*****
18.50	*****	79.00	*****	139.50	*****	200.00	*****	260.50	*****
18.75	*****	79.25	*****	139.75	*****	200.25	*****	260.75	*****
19.00	*****	79.50	*****	140.00	*****	200.50	*****	261.00	*****
19.25	*****	79.75	*****	140.25	*****	200.75	*****	261.25	*****
19.50	*****	80.00	*****	140.50	*****	201.00	*****	261.50	*****
19.75	*****	80.25	*****	140.75	*****	201.25	*****	261.75	*****
20.00	*****	80.50	*****	141.00	*****	201.50	*****	262.00	*****
20.25	*****	80.75	*****	141.25	*****	201.75	*****	262.25	*****
20.50	*****	81.00	*****	141.50	*****	202.00	*****	262.50	*****
20.75	*****	81.25	*****	141.75	*****	202.25	*****	262.75	*****
21.00	*****	81.50	*****	142.00	*****	202.50	*****	263.00	*****
21.25	*****	81.75	*****	142.25	*****	202.75	*****	263.25	*****
21.50	*****	82.00	*****	142.50	*****	203.00	*****	263.50	*****
21.75	*****	82.25	*****	142.75	*****	203.25	*****	263.75	*****
22.00	*****	82.50	*****	143.00	*****	203.50	*****	264.00	*****
22.25	*****	82.75	*****	143.25	*****	203.75	*****	264.25	*****
22.50	*****	83.00	*****	143.50	*****	204.00	*****	264.50	*****
22.75	*****	83.25	*****	143.75	*****	204.25	*****	264.75	*****
23.00	*****	83.50	*****	144.00	*****	204.50	*****	265.00	*****
23.25	*****	83.75	*****	144.25	*****	204.75	*****	265.25	*****
23.50	*****	84.00	*****	144.50	*****	205.00	*****	265.50	*****
23.75	*****	84.25	*****	144.75	*****	205.25	*****	265.75	*****
24.00	*****	84.50	*****	145.00	*****	205.50	*****	266.00	*****
24.25	*****	84.75	*****	145.25	*****	205.75	*****	266.25	*****
24.50	*****	85.00	*****	145.50	*****	206.00	*****	266.50	*****
24.75	*****	85.25	*****	145.75	*****	206.25	*****	266.75	*****
25.00	*****	85.50	*****	146.00	*****	206.50	*****	267.00	*****
25.25	*****	85.75	*****	146.25	*****	206.75	*****	267.25	*****
25.50	*****	86.00	*****	146.50	*****	207.00	*****	267.50	*****
25.75	*****	86.25	*****	146.75	*****	207.25	*****	267.75	*****
26.00	*****	86.50	*****	147.00	*****	207.50	*****	268.00	*****
26.25	*****	86.75	*****	147.25	*****	207.75	*****	268.25	*****
26.50	*****	87.00	*****	147.50	*****	208.00	*****	268.50	*****

STANDHYD (4461) Area (ha)= 236.12
 ID= 1 DT=15.0 min Total Imp(%)= 47.00 Dir. Conn.(%)= 17.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	110.98	125.14
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1254.64	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	71.81	24.46
over (min)	15.00	45.00
Storage Coeff. (min)	16.38 (ii)	35.17 (ii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.07	0.03

TOTALS

PEAK FLOW (cms)	5.61	4.85	7.628 (iii)
TIME TO PEAK (hrs)	6.00	6.50	6.00
RUNOFF VOLUME (mm)	52.40	16.88	22.92
TOTAL RAINFALL (mm)	54.40	54.40	54.40
RUNOFF COEFFICIENT	0.96	0.31	0.42

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 60.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5446)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	10.1800	6.2621
0.8000	2.9957	12.5050	7.1218
3.0350	3.4813	14.4090	7.9633
5.6510	4.4353	14.8090	7.9733
7.7470	5.2683	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (4461)	236.120	7.628	6.00	22.92
OUTFLOW: ID= 1 (5446)	236.120	1.975	7.75	22.91

PEAK FLOW REDUCTION [Oout/Oin](%)= 25.89
 TIME SHIFT OF PEAK FLOW (min)=105.00
 MAXIMUM STORAGE USED (ha.m.)= 3.2520

ADD HYD (8192)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (4462):	1238.89	2.230	12.00	11.47
+ ID2= 2 (5446):	236.12	1.975	7.75	22.91
ID = 3 (8192):	1475.01	3.574	8.00	13.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8194)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8188):	42609.04	135.487	14.00	17.79
+ ID2= 2 (8192):	1475.01	3.574	8.00	13.30
ID = 3 (8194):	44084.05	138.247	14.00	17.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0442)
 ID= 1 DT=15.0 min

Area (ha)= 117.26 Curve Number (CN)= 43.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 1.17

Unit Hyd Opeak (cms)= 1.705

PEAK FLOW (cms)= 0.295 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 6.270
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.115

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0440)
 ID= 1 DT=15.0 min

Area (ha)= 226.35 Curve Number (CN)= 60.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 1.11

Unit Hyd Opeak (cms)= 3.481

PEAK FLOW (cms)= 1.056 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 11.059
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.203

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0438)
 ID= 1 DT=15.0 min

Area (ha)= 130.70 Curve Number (CN)= 51.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.60

Unit Hyd Opeak (cms)= 3.738

PEAK FLOW (cms)= 0.721 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 8.134
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.150

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0436)
 ID= 1 DT=15.0 min

Area (ha)= 187.51 Curve Number (CN)= 55.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.73

Unit Hyd Opeak (cms)= 4.391

PEAK FLOW (cms)= 1.019 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 9.333
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.172

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0434)
 ID= 1 DT=15.0 min

Area (ha)= 56.64 Curve Number (CN)= 46.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.52

Unit Hyd Opeak (cms)= 1.867

PEAK FLOW (cms)= 0.293 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 6.832
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.126

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8206)
 1 + 2 = 3

AREA OPEAK TPEAK R.V.

	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0434):	56.64	0.293	6.50	6.83
+ ID2= 2 (0436):	187.51	1.019	7.00	9.33
ID = 3 (8206):	244.15	1.301	6.75	8.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6438)
IN= 2--> OUT= 1

Routing time step (min)'= 15.00

<----- DATA FOR SECTION (4381.0) ----->

Distance	Elevation	Manning	
0.00	256.00	0.0600	
9.23	255.50	0.0600	
27.69	255.00	0.0600	
50.77	254.90	0.0600	
64.62	254.85	0.0600	
78.47	252.26	0.0600	
96.93	249.44	0.0600	
129.24	245.94	0.0600	
145.40	245.76	0.0600	
146.20	245.80	0.0600 /0.0350	Main Channel
146.70	245.30	0.0350	Main Channel
147.70	245.30	0.0350	Main Channel
148.20	245.80	0.0350 /0.0600	Main Channel
150.01	245.78	0.0600	
152.32	245.79	0.0600	
163.86	246.72	0.0600	
175.40	249.07	0.0600	
186.94	253.15	0.0600	
196.17	255.38	0.0600	
228.48	255.55	0.0600	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.50	245.80	.221E+04	0.9	1.02	40.44
1.01	246.31	.352E+05	14.0	0.99	41.77
1.53	246.83	.841E+05	47.3	1.39	29.61
2.04	247.34	.144E+06	102.0	1.76	23.49
2.55	247.85	.213E+06	177.3	2.07	19.99
3.07	248.37	.291E+06	274.4	2.34	17.66
3.58	248.88	.378E+06	394.6	2.58	15.97
4.09	249.39	.474E+06	541.4	2.83	14.60
4.61	249.91	.578E+06	719.9	3.08	13.38
5.12	250.42	.688E+06	923.8	3.33	12.40
5.63	250.93	.803E+06	1153.3	3.56	11.61
6.14	251.44	.925E+06	1408.6	3.77	10.95
6.66	251.96	.105E+07	1690.6	3.98	10.38
7.17	252.47	.119E+07	2002.3	4.18	9.88
7.68	252.98	.133E+07	2345.4	4.38	9.43
8.20	253.50	.147E+07	2710.1	4.56	9.05
8.71	254.01	.162E+07	3101.0	4.73	8.72
9.22	254.52	.178E+07	3521.8	4.90	8.43
9.74	255.04	.195E+07	3467.6	4.39	9.39

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8206)	244.15	1.30	6.75	8.75	0.52	1.02
OUTFLOW: ID= 1 (6438)	244.15	1.15	7.75	8.75	0.51	1.02

ADD HYD (8208)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0438):	130.70	0.721	6.75	8.13
+ ID2= 2 (6438):	244.15	1.147	7.75	8.75
ID = 3 (8208):	374.85	1.768	7.50	8.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8210)
1 + 2 = 3

	AREA	OPEAK	TPEAK	R. V.
--	------	-------	-------	-------

	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0440):	226.35	1.056	7.75	11.06
+ ID2= 2 (8208):	374.85	1.768	7.50	8.54
ID = 3 (8210):	601.20	2.822	7.50	9.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0432)
ID= 1 DT=15.0 mi n

Area (ha)=	114.21	Curve Number (CN)=	53.0
Ia (mm)=	5.00	# of Li near Res. (N)=	1.50
U. H. Tp(hrs)=	1.21		

Unit Hyd Opeak (cms)= 1.617

PEAK FLOW (cms)= 0.398 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 8.817
TOTAL RAI NFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.162

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0430)
ID= 1 DT=15.0 mi n

Area (ha)=	111.64	Curve Number (CN)=	55.0
Ia (mm)=	5.00	# of Li near Res. (N)=	1.50
U. H. Tp(hrs)=	0.52		

Unit Hyd Opeak (cms)= 3.687

PEAK FLOW (cms)= 0.790 (i)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 9.231
TOTAL RAI NFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.170

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0428)
ID= 1 DT=15.0 mi n

Area (ha)=	50.53	Curve Number (CN)=	51.0
Ia (mm)=	5.00	# of Li near Res. (N)=	1.50
U. H. Tp(hrs)=	0.50		

Unit Hyd Opeak (cms)= 1.725

PEAK FLOW (cms)= 0.320 (i)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 8.080
TOTAL RAI NFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.149

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0426)
ID= 1 DT=15.0 mi n

Area (ha)=	247.17	Curve Number (CN)=	61.0
Ia (mm)=	5.00	# of Li near Res. (N)=	1.50
U. H. Tp(hrs)=	0.98		

Unit Hyd Opeak (cms)= 4.315

PEAK FLOW (cms)= 1.310 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 11.401
TOTAL RAI NFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.210

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0424)
ID= 1 DT=15.0 mi n

Area (ha)=	49.57	Curve Number (CN)=	59.0
Ia (mm)=	5.00	# of Li near Res. (N)=	1.50
U. H. Tp(hrs)=	0.37		

Unit Hyd Opeak (cms)= 2.293

PEAK FLOW (cms) = 0.506 (i)
 TIME TO PEAK (hrs) = 6.500
 RUNOFF VOLUME (mm) = 10.325
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.190

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8198)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0424):	49.57	0.506	6.50	10.32
+ ID2= 2 (0426):	247.17	1.310	7.25	11.40

ID = 3 (8198):	296.74	1.705	7.00	11.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6428)		Routing time step (min) = 15.00	
IN= 2	OUT= 1		

<----- DATA FOR SECTION (4281.0) ----->			
Distance	Elevation	Manning	
0.00	246.54	0.0900	
8.80	246.29	0.0900	
30.78	246.03	0.0900	
41.78	243.97	0.0900	
63.77	237.16	0.0900	
85.76	232.53	0.0900	
96.75	230.16	0.0900	
109.94	228.40	0.0900	
112.14	228.28	0.0900 / 0.0450	Main Channel
112.84	228.00	0.0450	Main Channel
113.34	227.50	0.0450	Main Channel
114.34	227.50	0.0450	Main Channel
114.84	228.00	0.0450	Main Channel
116.54	228.29	0.0450 / 0.0900	Main Channel
118.74	228.40	0.0900	
120.94	228.52	0.0900	
149.52	229.51	0.0900	
169.31	230.69	0.0900	
186.00	234.27	0.0900	
217.69	243.56	0.0900	

<----- TRAVEL TIME TABLE ----->						
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)	
0.78	228.28	.205E+04	1.6	0.97	21.60	
1.58	229.08	.202E+05	16.6	1.02	20.37	
2.39	229.89	.665E+05	64.8	1.22	17.11	
3.19	230.69	.133E+06	158.1	1.49	14.05	
4.00	231.50	.213E+06	308.6	1.82	11.48	
4.80	232.30	.299E+06	504.9	2.11	9.89	
5.61	233.11	.394E+06	746.6	2.37	8.79	
6.41	233.91	.496E+06	1034.5	2.61	7.99	
7.21	234.71	.605E+06	1373.3	2.84	7.35	
8.02	235.52	.722E+06	1763.3	3.06	6.82	
8.82	236.32	.844E+06	2202.0	3.27	6.39	
9.63	237.13	.974E+06	2690.5	3.46	6.03	
10.43	237.93	.111E+07	3239.5	3.66	5.71	
11.23	238.73	.125E+07	3839.0	3.84	5.43	
12.04	239.54	.140E+07	4489.1	4.03	5.18	
12.84	240.34	.155E+07	5190.5	4.20	4.97	
13.65	241.15	.170E+07	5943.8	4.36	4.78	
14.45	241.95	.187E+07	6749.8	4.53	4.61	
15.26	242.76	.204E+07	7609.4	4.68	4.46	

<----- hydrograph ----->				<-pi pe / channel ->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8198)	296.74	1.70	7.00	11.22	0.79
OUTFLOW: ID= 1 (6428)	296.74	1.64	7.50	11.22	0.78

| ADD HYD (8200) |

1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0428):	50.53	0.320	6.50	8.08
+ ID2= 2 (6428):	296.74	1.640	7.50	11.22

ID = 3 (8200):	347.27	1.911	7.25	10.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8202)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0430):	111.64	0.790	6.50	9.23
+ ID2= 2 (8200):	347.27	1.911	7.25	10.76

ID = 3 (8202):	458.91	2.625	7.00	10.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6432)		Routing time step (min) = 15.00	
IN= 2	OUT= 1		

<----- DATA FOR SECTION (4321.0) ----->			
Distance	Elevation	Manning	
0.00	221.00	0.0600	
15.29	219.85	0.0600	
45.86	219.86	0.0600	
76.44	219.96	0.0600	
110.84	220.23	0.0600	
112.00	220.15	0.0600	
114.66	220.09	0.0600	
118.48	219.73	0.0600	
120.80	219.70	0.0600 / 0.0350	Main Channel
121.30	219.20	0.0350	Main Channel
122.30	219.20	0.0350	Main Channel
122.80	219.70	0.0350 / 0.0600	Main Channel
129.95	220.11	0.0600	
175.81	220.24	0.0600	
214.03	220.55	0.0600	
252.25	220.61	0.0600	
290.47	220.70	0.0600	
328.69	221.04	0.0600	
347.80	221.08	0.0600	
378.37	221.04	0.0600	

<----- TRAVEL TIME TABLE ----->						
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)	
0.08	219.28	.185E+03	0.0	0.23	149.38	
0.17	219.37	.399E+03	0.1	0.34	100.62	
0.25	219.45	.641E+03	0.1	0.42	80.98	
0.33	219.53	.911E+03	0.2	0.49	69.79	
0.42	219.62	.121E+04	0.3	0.55	62.35	
0.50	219.70	.154E+04	0.5	0.60	56.91	
0.61	219.81	.267E+04	0.7	0.56	61.28	
0.71	219.91	.901E+04	1.3	0.30	112.96	
0.82	220.02	.249E+05	3.4	0.28	122.71	
0.92	220.12	.452E+05	6.8	0.31	109.98	
1.03	220.23	.747E+05	12.2	0.33	102.05	
1.14	220.34	.112E+06	21.3	0.39	87.59	
1.24	220.44	.153E+06	33.0	0.44	77.01	
1.35	220.55	.196E+06	47.2	0.49	69.22	
1.46	220.66	.248E+06	62.3	0.51	66.48	
1.56	220.76	.310E+06	83.4	0.55	62.06	
1.67	220.87	.376E+06	108.8	0.59	57.56	
1.77	220.97	.444E+06	137.7	0.64	53.77	
1.88	221.08	.517E+06	163.6	0.65	52.70	

<----- hydrograph ----->				<-pi pe / channel ->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8202)	458.91	2.62	7.00	10.39	0.78
OUTFLOW: ID= 1 (6432)	458.91	1.77	9.25	10.39	0.73

| ADD HYD (8204) |

1 + 2 = 3		AREA	OPEAK	TPEAK	R. V.
		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0432):		114.21	0.398	8.00	8.82
+ ID2= 2 (6432):		458.91	1.768	9.25	10.39

ID = 3 (8204):		573.12	2.132	9.00	10.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8212)		AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8204):		573.12	2.132	9.00	10.08
+ ID2= 2 (8210):		601.20	2.822	7.50	9.49

ID = 3 (8212):		1174.32	4.704	8.00	9.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6442)
IN= 2----> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (4421.0) ----->

Distance	Elevation	Manning		
0.00	221.00	0.0350		
26.73	220.80	0.0350		
53.47	220.60	0.0350		
66.83	220.40	0.0350		
80.20	220.20	0.0350		
347.52	220.00	0.0350		
354.21	219.59	0.0350		
360.89	219.50	0.0350		
367.57	219.08	0.0350		
368.76	219.00	0.0350 / 0.0300	Main	Channel
369.26	218.50	0.0300	Main	Channel
374.26	218.50	0.0300	Main	Channel
374.76	219.00	0.0300 / 0.0350	Main	Channel
380.94	220.33	0.0350		
387.62	219.62	0.0350		
394.31	219.23	0.0350		
454.45	219.07	0.0350		
514.60	219.09	0.0350		
660.00	219.19	0.0350		
661.63	221.00	0.0350		

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(mi n)
0.12	218.62	.100E+04	0.2	0.28	92.89
0.25	218.75	.205E+04	0.6	0.43	60.09
0.37	218.87	.315E+04	1.1	0.55	46.99
0.50	219.00	.430E+04	1.8	0.66	39.67
0.63	219.13	.141E+05	3.4	0.37	69.99
0.77	219.27	.652E+05	13.7	0.33	79.14
0.90	219.40	.124E+06	35.5	0.45	58.02
1.03	219.53	.183E+06	65.7	0.56	46.45
1.17	219.67	.245E+06	103.8	0.66	39.30
1.30	219.80	.307E+06	149.3	0.76	34.30
1.43	219.93	.371E+06	201.6	0.85	30.65
1.57	220.07	.440E+06	256.5	0.91	28.56
1.70	220.20	.514E+06	325.7	0.94	27.70
1.83	220.33	.663E+06	415.2	0.98	26.62
1.97	220.47	.787E+06	520.3	1.03	25.21
2.10	220.60	.913E+06	638.5	1.09	23.82
2.23	220.73	.104E+07	767.2	1.15	22.62
2.37	220.87	.117E+07	908.0	1.21	21.54
2.50	221.00	.131E+07	1060.7	1.27	20.57

<---- hydrograph ---->

	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8212)	1174.32	4.70	8.00	9.77	0.65	0.37
OUTFLOW: ID= 1 (6442)	1174.32	4.12	9.75	9.77	0.64	0.37

| ADD HYD (8214) |

1 + 2 = 3		AREA	OPEAK	TPEAK	R. V.
		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0442):		117.26	0.295	8.00	6.27
+ ID2= 2 (6442):		1174.32	4.116	9.75	9.77

ID = 3 (8214):		1291.58	4.367	9.50	9.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8196)		AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8194):		44084.05	138.247	14.00	17.64
+ ID2= 2 (8214):		1291.58	4.367	9.50	9.46

ID = 3 (8196):		45375.62	141.048	13.75	17.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0444)
ID= 1 DT=15.0 mi n

Area (ha)= 221.65
Ia (mm)= 5.00
U. H. Tp(hrs)= 1.03

Curve Number (CN)= 56.0
of Linear Res. (N)= 1.50

Unit Hyd Qpeak (cms)= 3.656

PEAK FLOW (cms)= 0.953 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 9.707
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.178

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8216)		AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0444):		221.65	0.953	7.50	9.71
+ ID2= 2 (8196):		45375.62	141.048	13.75	17.41

ID = 3 (8216):		45597.28	141.314	13.75	17.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

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V V I SSSS U U A L
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
W I SSSS UUUU A A LLLLL

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***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\Visual Otthymo 2.4\VO2\voi.n.dat
 Output filename: C:\Users\amanluc\AppData\Local\Temp\1c422446-0837-4e2d-8849-fb8c271f1fda\Scenario.out
 Summary filename: C:\Users\amanluc\AppData\Local\Temp\1c422446-0837-4e2d-8849-fb8c271f1fda\Scenario.sum

DATE: 08/29/2012 TIME: 01:56:49

USER:

COMMENTS: _____

 ** SIMULATION NUMBER: 1 **

READ STORM
 Ptotal = 62.70 mm
 Filename: C:\Users\amanluc\AppData\Local\Temp\1c422446-0837-4e2d-8849-fb8c271f1fda\330170b9
 Comments: 10-Year 12-Hour SCS II Design Storm

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.57	3.25	2.51	6.25	11.29
0.50	1.57	3.50	2.51	6.50	11.29
0.75	1.57	3.75	2.51	6.75	5.02
1.00	1.57	4.00	2.51	7.00	5.02
1.25	1.57	4.25	3.76	7.25	3.76
1.50	1.57	4.50	3.76	7.50	3.76
1.75	1.57	4.75	5.02	7.75	3.76
2.00	1.57	5.00	5.02	8.00	3.76
2.25	1.88	5.25	7.52	8.25	2.19
2.50	1.88	5.50	7.52	8.50	2.19
2.75	1.88	5.75	30.10	8.75	2.19
3.00	1.88	6.00	82.76	9.00	2.19

CALIB
 STANDHYD (1662)
 ID= 1 DT=15.0 min
 Area (ha)= 141.06
 Total Imp(%)= 70.00 Di r. Conn.(%)= 38.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	98.74	42.32
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	969.74	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	82.76	109.41
over (min)	15.00	30.00
Storage Coeff. (min)=	13.26 (ii)	23.58 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04
PEAK FLOW (cms)=	9.44	6.10
TIME TO PEAK (hrs)=	6.00	6.25

TOTALS
 13.433 (iii)
 6.00

RUNOFF VOLUME (mm)= 60.70 34.29 44.33
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.55 0.71

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (1661)
 ID= 1 DT=15.0 min
 Area (ha)= 293.64
 Total Imp(%)= 42.00 Di r. Conn.(%)= 23.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	123.33	170.31
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1399.14	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	82.76	54.70
over (min)	15.00	45.00
Storage Coeff. (min)=	16.52 (ii)	30.14 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.07	0.03
PEAK FLOW (cms)=	10.84	9.79
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	60.70	26.71
TOTAL RAINFALL (mm)=	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.43

TOTALS
 15.047 (iii)
 6.00
 34.52
 62.70
 0.55

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5166)
 IN= 2---> OUT= 1
 DT= 15.0 min

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1661)	293.640	15.047	6.00	34.52
OUTFLOW: ID= 1 (5166)	293.640	4.944	7.25	34.52

PEAK FLOW REDUCTION [Qout/Qi] (%) = 32.86
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha.m.) = 5.5605

ADD HYD (8134)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1662):	141.06	13.433	6.00	44.33
+ ID2= 2 (5166):	293.64	4.944	7.25	34.52
ID= 3 (8134):	434.70	13.685	6.00	37.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1642)
 ID= 1 DT=15.0 min
 Area (ha)= 120.35
 Total Imp(%)= 22.00 Di r. Conn.(%)= 17.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	26.48	93.87
Dep. Storage over (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	895.73	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	82.76	22.52
Storage Coeff. (min)=	15.00	45.00
Unit Hyd. Tpeak (min)=	12.64 (ii)	32.06 (ii)
Unit Hyd. peak (cms)=	15.00	45.00
	0.08	0.03
PEAK FLOW (cms)=	3.67	3.50
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	60.70	22.51
TOTAL RAINFALL (mm)=	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.36

TOTALS
5.127 (iii)
6.00
29.00
62.70
0.46

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1641)	Area (ha)=	Imp(%)=	Dir. Conn.(%)=
ID= 1 DT=15.0 min	113.89	71.00	53.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	80.86	33.03
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	871.36	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	82.76	74.31
Storage Coeff. over (min)=	15.00	30.00
Unit Hyd. Tpeak (min)=	12.43 (ii)	24.48 (ii)
Unit Hyd. peak (cms)=	15.00	30.00
	0.08	0.04
PEAK FLOW (cms)=	10.89	3.15
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	60.70	29.70
TOTAL RAINFALL (mm)=	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.47

TOTALS
12.914 (iii)
6.00
46.13
62.70
0.74

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5164)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1 DT= 15.0 min	0.0000	0.0000	8.4260	4.5513
	0.3860	1.9228	9.2330	4.9251
	3.3700	2.2339	9.9990	5.2982
	4.5710	3.3613	10.3990	5.3082
	5.7170	4.0568	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1641)	113.890	12.914	6.00	46.13
OUTFLOW: ID= 1 (5164)	113.890	3.879	6.75	46.12

PEAK FLOW REDUCTION [Qout/Qin] (%) = 30.03
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha.m.) = 2.7508

ADD HYD (8132)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (1642):	120.35	5.127	6.00	29.00
+ ID2= 2 (5164):	113.89	3.879	6.75	46.12
ID = 3 (8132):	234.24	8.297	6.50	37.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0152)	Area (ha)=	Curve Number (CN)=
ID= 1 DT=15.0 min	124.37	59.0
	U. H. Tp(hrs)= 5.00	# of Linear Res. (N)= 1.50
		0.89

Unit Hyd Opeak (cms)= 2.372

PEAK FLOW (cms)=	0.875 (i)
TIME TO PEAK (hrs)=	7.250
RUNOFF VOLUME (mm)=	14.044
TOTAL RAINFALL (mm)=	62.700
RUNOFF COEFFICIENT =	0.224

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1481)	Area (ha)=	Imp(%)=	Dir. Conn.(%)=
ID= 1 DT=15.0 min	61.75	47.00	17.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	29.02	32.73
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	641.61	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	82.76	8.22
Storage Coeff. over (min)=	15.00	45.00
Unit Hyd. Tpeak (min)=	10.35 (ii)	39.41 (ii)
Unit Hyd. peak (cms)=	15.00	45.00
	0.09	0.03
PEAK FLOW (cms)=	2.02	0.41
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	60.70	6.03
TOTAL RAINFALL (mm)=	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.10

TOTALS
2.179 (iii)
6.00
15.32
62.70
0.24

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 23.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5148)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1 DT= 15.0 min	0.0000	0.0000	4.1310	1.6343
	0.2090	0.7802	5.1870	1.8598
	1.1960	0.9062	6.0380	2.0805
	2.2280	1.1562	6.4380	2.0905
	3.0910	1.3741	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1481)	61.750	2.179	6.00	15.32
OUTFLOW: ID= 1 (5148)	61.750	0.170	8.50	15.31

PEAK FLOW REDUCTION [Qout/Qin] (%) = 7.81
TIME SHIFT OF PEAK FLOW (min) = 150.00
MAXIMUM STORAGE USED (ha.m.) = 0.6354

 CALIB NASHYD (1482) | Area (ha)= 356.14 Curve Number (CN)= 31.3
 ID= 1 DT=15.0 min | I a (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.12

Unit Hyd Qpeak (cms)= 5.438

PEAK FLOW (cms)= 0.791 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 5.365
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.086

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (9148) | AREA OPEAK TPEAK R. V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (1482): 356.14 0.791 7.75 5.36
 + ID2= 2 (5148): 61.75 0.170 8.50 15.31

 ID = 3 (9148): 417.89 0.959 8.00 6.83

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB STANDHYD (1501) | Area (ha)= 0.16
 ID= 1 DT=15.0 min | Total Imp(%)= 45.00 Di r. Conn.(%)= 15.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.07 0.09
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 32.66 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 82.76 4.04
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 1.73 (ii) 40.33 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.11 0.03

TOTALS
 PEAK FLOW (cms)= 0.01 0.00 0.006 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 60.70 3.94 12.34
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.06 0.20

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 16.4 I a = Dep Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR (5150) |
 IN= 2----> OUT= 1
 DT= 15.0 min |

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.0110	0.0042
0.0010	0.0020	0.0140	0.0049
0.0030	0.0023	0.0160	0.0054
0.0060	0.0030	0.0160	0.0154
0.0080	0.0036	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
0.160	0.006	6.00	12.34
0.160	0.001	7.50	9.27

PEAK FLOW REDUCTION [Qout/Qin] (%) = 9.89

TIME SHIFT OF PEAK FLOW (min)= 90.00
 MAXIMUM STORAGE USED (ha. m.) = 0.0011

 CALIB NASHYD (1502) | Area (ha)= 105.72 Curve Number (CN)= 17.2
 ID= 1 DT=15.0 min | I a (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.58

Unit Hyd Qpeak (cms)= 3.134

PEAK FLOW (cms)= 0.183 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 2.540
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.041

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (9150) | AREA OPEAK TPEAK R. V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (1502): 105.72 0.183 6.75 2.54
 + ID2= 2 (5150): 0.16 0.001 7.50 9.27

 ID = 3 (9150): 105.88 0.184 6.75 2.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8116) | AREA OPEAK TPEAK R. V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (9148): 417.89 0.959 8.00 6.83
 + ID2= 2 (9150): 105.88 0.184 6.75 2.55

 ID = 3 (8116): 523.77 1.111 7.50 5.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6152) | Routing time step (min)' = 15.00
 IN= 2----> OUT= 1

<----- DATA FOR SECTION (1521.0) ----->

Distance	Elevation	Manning
0.00	290.00	0.0500
15.34	286.87	0.0500
46.03	284.84	0.0500
88.22	283.07	0.0500
126.57	282.11	0.0500
153.42	280.34	0.0500
157.26	280.07	0.0500
161.09	279.72	0.0500
167.76	278.94	0.0500 / 0.0300
168.01	278.60	0.0300
168.76	278.60	0.0300
169.51	278.60	0.0300
169.76	278.90	0.0300 / 0.0500
173.76	279.40	0.0500
180.27	279.69	0.0500
184.11	279.93	0.0500
218.62	281.86	0.0500
260.82	283.20	0.0500
360.54	289.25	0.0500
379.72	289.80	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.30	278.90	.114E+04	0.4	0.80	45.70
0.87	279.47	.932E+04	4.2	0.98	37.22
1.45	280.05	.353E+05	17.5	1.09	33.68
2.02	280.62	.831E+05	47.8	1.26	29.01
2.59	281.19	.155E+06	102.9	1.46	25.06
3.17	281.77	.250E+06	189.3	1.66	22.03
3.74	282.34	.375E+06	298.5	1.75	20.91

4.32	282.92	.547E+06	456.5	1.83	19.98
4.89	283.49	.767E+06	705.7	2.02	18.12
5.46	284.06	.102E+07	1038.0	2.24	16.34
6.04	284.64	.130E+07	1444.1	2.45	14.98
6.61	285.21	.161E+07	1940.1	2.66	13.79
7.18	285.78	.194E+07	2522.6	2.86	12.80
7.76	286.36	.229E+07	3187.4	3.06	11.98
8.33	286.93	.267E+07	3942.3	3.25	11.28
8.91	287.51	.306E+07	4824.9	3.46	10.58
9.48	288.08	.347E+07	5795.7	3.67	9.99
10.05	288.65	.390E+07	6855.4	3.86	9.48
10.63	289.23	.434E+07	8005.0	4.05	9.04

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8116)	523.77	1.11	7.50	5.97	0.41	0.83
OUTFLOW: ID= 1 (6152)	523.77	1.05	8.50	5.97	0.40	0.83

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
(8122)				
1 + 2 = 3				
ID1= 1 (0152):	124.37	0.875	7.25	14.04
+ ID2= 2 (6152):	523.77	1.046	8.50	5.97
ID = 3 (8122):	648.14	1.849	8.00	7.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD	Area (ha)	Curve Number (CN)
(0158)	178.59	61.0
ID= 1 DT=15.0 min	U.H. Tp(hrs)= 5.00	# of Linear Res. (N)= 1.50
		0.94

Unit Hyd Opeak (cms) = 3.230

PEAK FLOW	(cms) = 1.287 (i)
TIME TO PEAK	(hrs) = 7.250
RUNOFF VOLUME	(mm) = 14.959
TOTAL RAINFALL	(mm) = 62.700
RUNOFF COEFFICIENT	= 0.239

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD	Area (ha)	Curve Number (CN)
(0156)	83.49	45.0
ID= 1 DT=15.0 min	U.H. Tp(hrs)= 5.00	# of Linear Res. (N)= 1.50
		0.62

Unit Hyd Opeak (cms) = 2.306

PEAK FLOW	(cms) = 0.488 (i)
TIME TO PEAK	(hrs) = 6.750
RUNOFF VOLUME	(mm) = 8.855
TOTAL RAINFALL	(mm) = 62.700
RUNOFF COEFFICIENT	= 0.141

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD	Area (ha)	Dir. Conn. (%)
(1541)	11.15	70.00
ID= 1 DT=15.0 min	Total Imp(%)= 75.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha) = 8.36	2.79
Dep. Storage	(mm) = 2.00	5.00
Average Slope	(%) = 0.50	0.50
Length	(m) = 272.64	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr) over	(min) = 82.76	5.27
Storage Coeff. (min)	= 6.19 (ii)	40.92 (ii)
Unit Hyd. Tpeak (min)	= 15.00	45.00

Unit Hyd. peak (cms) =	0.10	0.03	*TOTALS*
PEAK FLOW (cms) =	1.69	0.03	1.700 (iii)
TIME TO PEAK (hrs) =	6.00	6.50	6.00
RUNOFF VOLUME (mm) =	60.70	6.55	44.45
TOTAL RAINFALL (mm) =	62.70	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.10	0.71

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 31.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5154)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1				
DT= 15.0 min				
	0.0000	0.0000	0.7460	0.3544
	0.0380	0.1987	0.9370	0.3839
	0.2160	0.2386	1.0900	0.4129
	0.4020	0.2749	1.4900	0.4229
	0.5580	0.3133	0.0000	0.0000

INFLOW : ID= 2 (1541)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
OUTFLOW: ID= 1 (5154)	11.150	1.700	6.00	44.45
	11.150	0.453	6.25	44.34

PEAK FLOW REDUCTION [Qout/Qin] (%) = 26.62
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha.m.) = 0.2901

CALIB NASHYD	Area (ha)	Curve Number (CN)
(1542)	189.17	39.1
ID= 1 DT=15.0 min	U.H. Tp(hrs)= 5.00	# of Linear Res. (N)= 1.50
		0.95

Unit Hyd Opeak (cms) = 3.396

PEAK FLOW	(cms) = 0.644 (i)
TIME TO PEAK	(hrs) = 7.250
RUNOFF VOLUME	(mm) = 7.264
TOTAL RAINFALL	(mm) = 62.700
RUNOFF COEFFICIENT	= 0.116

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
(9154)				
1 + 2 = 3				
ID1= 1 (1542):	189.17	0.644	7.25	7.26
+ ID2= 2 (5154):	11.15	0.453	6.25	44.34
ID = 3 (9154):	200.32	0.988	6.50	9.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
(8118)				
1 + 2 = 3				
ID1= 1 (0156):	83.49	0.488	6.75	8.86
+ ID2= 2 (9154):	200.32	0.988	6.50	9.33
ID = 3 (8118):	283.81	1.469	6.50	9.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN	Routing time step (min)
(6158)	15.00
IN= 2--> OUT= 1	

<----- DATA FOR SECTION (1581.0) ----->

Distance	Elevation	Manning	
0.00	290.00	0.0400	
19.20	288.88	0.0400	
48.00	288.26	0.0400	
119.99	283.68	0.0400	
167.99	282.07	0.0400	
196.79	281.32	0.0400	
210.79	280.90	0.0400	
219.79	280.38	0.0400 / 0.0300	Main Channel
220.04	279.65	0.0300	Main Channel
220.79	279.65	0.0300	Main Channel
221.54	279.65	0.0300	Main Channel
221.79	280.39	0.0300 / 0.0400	Main Channel
225.79	280.59	0.0400	
235.19	280.88	0.0400	
254.39	281.50	0.0400	
273.58	282.56	0.0400	
302.38	283.42	0.0400	
359.98	284.03	0.0400	
436.77	289.37	0.0400	
475.17	290.37	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.37	280.01	.973E+03	0.5	0.86	31.78
0.73	280.38	.210E+04	1.5	1.18	23.16
1.30	280.95	.149E+05	8.6	0.95	28.91
1.86	281.51	.579E+05	40.0	1.13	24.12
2.43	282.08	.133E+06	118.5	1.46	18.75
2.99	282.64	.236E+06	254.6	1.77	15.46
3.56	283.21	.369E+06	450.3	2.00	13.67
4.13	283.78	.542E+06	702.7	2.13	12.86
4.69	284.34	.767E+06	1125.2	2.41	11.36
5.26	284.91	.101E+07	1697.1	2.75	9.93
5.82	285.47	.127E+07	2379.1	3.07	8.91
6.39	286.04	.155E+07	3171.4	3.36	8.14
6.95	286.60	.184E+07	4075.5	3.64	7.53
7.52	287.17	.215E+07	5093.1	3.89	7.03
8.09	287.74	.247E+07	6226.5	4.13	6.62
8.65	288.30	.281E+07	7461.2	4.36	6.28
9.22	288.87	.318E+07	8635.1	4.46	6.13
9.78	289.43	.357E+07	10143.6	4.67	5.86
10.35	290.00	.398E+07	11608.8	4.79	5.71

		<---- hydrograph ---->			<- pi pe / channel ->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW :	ID= 2 (8118)	283.81	1.47	6.50	9.19	0.72
OUTFLOW :	ID= 1 (6158)	283.81	1.35	7.00	9.19	0.67

ADD HYD (8120)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0158):	178.59	1.287	7.25	14.96
+ ID2= 2 (6158):	283.81	1.352	7.00	9.19
ID = 3 (8120):	462.40	2.630	7.25	11.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8124)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (8120):	462.40	2.630	7.25	11.42
+ ID2= 2 (8122):	648.14	1.849	8.00	7.52
ID = 3 (8124):	1110.54	4.376	7.50	9.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6160)
IN= 2--> OUT= 1 Routing time step (mi n)' = 15.00

<----- DATA FOR SECTION (1601.0) ----->

Distance	Elevation	Manning	
0.00	261.02	0.0400	
29.89	254.30	0.0400	
40.75	252.58	0.0400	
62.49	251.30	0.0400	
114.11	250.94	0.0400	
130.41	248.58	0.0400	
141.28	247.50	0.0400	
154.86	246.87	0.0400	
155.20	246.70	0.0400	
160.20	246.30	0.0400 / 0.0300	Main Channel
160.30	245.30	0.0300	Main Channel
165.20	245.30	0.0300	Main Channel
165.30	246.30	0.0300 / 0.0400	Main Channel
168.45	246.96	0.0400	
195.62	249.20	0.0400	
203.77	250.82	0.0400	
225.50	256.78	0.0400	
244.52	261.14	0.0400	
255.39	261.95	0.0400	
268.97	261.92	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.50	245.80	.120E+05	2.9	1.18	68.31
1.00	246.30	.242E+05	8.5	1.70	47.47
1.92	247.22	.825E+05	31.7	1.86	43.34
2.84	248.14	.248E+06	102.5	2.00	40.39
3.76	249.06	.506E+06	247.7	2.37	34.03
4.68	249.98	.832E+06	486.8	2.83	28.49
5.60	250.90	.121E+07	814.9	3.27	24.71
6.52	251.82	.180E+07	1157.1	3.11	25.97
7.44	252.74	.253E+07	1840.2	3.52	22.92
8.36	253.66	.331E+07	2727.1	3.98	20.25
9.29	254.59	.414E+07	3782.6	4.43	18.22
10.21	255.51	.499E+07	5010.7	4.86	16.61
11.13	256.43	.589E+07	6397.4	5.26	15.33
12.05	257.35	.681E+07	7932.8	5.64	14.31
12.97	258.27	.777E+07	9621.2	5.99	13.46
13.89	259.19	.877E+07	11469.4	6.33	12.74
14.81	260.11	.980E+07	13478.4	6.66	12.12
15.73	261.03	.109E+08	15651.7	6.97	11.58
16.65	261.95	.120E+08	17492.5	7.07	11.42

		<---- hydrograph ---->			<- pi pe / channel ->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW :	ID= 2 (8124)	1110.54	4.38	7.50	9.14	0.63
OUTFLOW :	ID= 1 (6160)	1110.54	3.79	8.75	9.14	0.58

CALIB STANDHYD (1601)	Area Total (ha)	Imp (%)	Dir. Conn. (%)
ID= 1 DT=15.0 mi n	19.03	75.00	69.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	14.27	4.76
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	356.18	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	82.76	17.45
over (mi n)	15.00	30.00
Storage Coeff. (mi n)	7.27 (ii)	28.78 (ii)
Unit Hyd. Tpeak (mi n)	15.00	30.00
Unit Hyd. peak (cms)	0.10	0.04

		TOTALS
PEAK FLOW (cms)	2.76	0.16
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	60.70	15.49
TOTAL RAI NFALL (mm)	62.70	62.70
RUNOFF COEFFICIENT	0.97	0.74

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 55.6 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5160)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	1.2730	0.6034
	0.0640	0.3377	1.5980	0.6540
	0.3690	0.4055	1.8600	0.7037
	0.6870	0.4676	2.2600	0.7137
	0.9520	0.5332	0.0000	0.0000
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1601)	19.030	2.858	6.00	46.68
OUTFLOW: ID= 1 (5160)	19.030	0.787	6.25	46.61

PEAK FLOW REDUCTION [Qout/Qin] (%) = 27.54
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 0.4961

CALIB NASHYD (1602)			
ID= 1 DT=15.0 min			
Area (ha)	= 299.79	Curve Number (CN)	= 60.3
Ia (mm)	= 5.00	# of Linear Res. (N)	= 1.50
U. H. Tp (hrs)	= 1.42		

Unit Hyd Opeak (cms) = 3.592

PEAK FLOW (cms) = 1.553 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 14.712
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.235

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9160)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1602):	299.79	1.553	8.00	14.71
+ ID2= 2 (5160):	19.03	0.787	6.25	46.61
=====				
ID = 3 (9160):	318.82	1.966	6.75	16.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8126)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6160):	1110.54	3.791	8.75	9.14
+ ID2= 2 (9160):	318.82	1.966	6.75	16.62
=====				
ID = 3 (8126):	1429.36	5.477	8.50	10.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1621)			
ID= 1 DT=15.0 min			
Area Total	(ha) = 46.07	Dir. Conn. (%) =	20.00
Imp (%) =	45.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 20.73	25.34
Dep. Storage (mm)	= 2.00	5.00
Average Slope (%)	= 0.50	0.50
Length (m)	= 554.20	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr) = 82.76 over (min) = 20.51
 Storage Coeff. (min) = 9.48 (ii) 29.64 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00

Unit Hyd. peak (cms)	= 0.09	0.04	*TOTALS*
PEAK FLOW (cms)	= 1.82	0.98	2.415 (iii)
TIME TO PEAK (hrs)	= 6.00	6.25	6.00
RUNOFF VOLUME (mm)	= 60.70	15.51	24.55
TOTAL RAINFALL (mm)	= 62.70	62.70	62.70
RUNOFF COEFFICIENT	= 0.97	0.25	0.39

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 51.1 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5162)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	3.0820	1.1997
	0.1560	0.5629	3.8700	1.3716
	0.8930	0.6512	4.5050	1.5398
	1.6620	0.8407	4.9050	1.5498
	2.3060	1.0035	0.0000	0.0000
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1621)	46.070	2.415	6.00	24.55
OUTFLOW: ID= 1 (5162)	46.070	0.644	7.00	24.53

PEAK FLOW REDUCTION [Qout/Qin] (%) = 26.68
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 0.6245

CALIB STANDHYD (1622)			
ID= 1 DT=15.0 min			
Area Total	(ha) = 144.07	Dir. Conn. (%) =	11.00
Imp (%) =	26.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 37.46	106.61
Dep. Storage (mm)	= 2.00	5.00
Average Slope (%)	= 0.50	0.50
Length (m)	= 980.03	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr) = 82.76 over (min) = 14.21
 Storage Coeff. (min) = 13.34 (ii) 36.69 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.08 0.03

PEAK FLOW (cms)	= 2.78	2.37	3.738 (iii)
TIME TO PEAK (hrs)	= 6.00	6.50	6.00
RUNOFF VOLUME (mm)	= 60.70	13.15	18.38
TOTAL RAINFALL (mm)	= 62.70	62.70	62.70
RUNOFF COEFFICIENT	= 0.97	0.21	0.29

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 51.1 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9162)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1622):	144.07	3.738	6.00	18.38
+ ID2= 2 (5162):	46.07	0.644	7.00	24.53
=====				
ID = 3 (9162):	190.14	3.820	6.00	19.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	(8128)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8126):		1429.36	5.477	8.50	10.81
+ ID2= 2 (9162):		190.14	3.820	6.00	19.87

ID = 3 (8128):		1619.50	6.644	7.00	11.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	(8130)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8128):		1619.50	6.644	7.00	11.87
+ ID2= 2 (8132):		234.24	8.297	6.50	37.32

ID = 3 (8130):		1853.74	14.728	6.50	15.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6166)
IN= 2---> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1661.0) ----->

Distance	Elevation	Manning	
0.00	248.36	0.0500	
41.67	246.98	0.0500	
95.82	244.93	0.0500	
149.98	242.42	0.0500	
191.64	241.88	0.0500	
224.97	239.90	0.0500	
229.13	239.35	0.0500	
233.30	237.77	0.0500	
237.47	236.57	0.0500	
240.63	235.60	0.0500 / 0.0300	Main Channel
241.63	234.10	0.0300	Main Channel
247.13	234.10	0.0300	Main Channel
247.53	235.60	0.0300 / 0.0500	Main Channel
254.13	236.25	0.0500	
266.63	236.77	0.0500	
283.29	237.84	0.0500	
291.63	238.89	0.0500	
329.12	244.66	0.0500	
370.78	249.55	0.0500	
412.44	253.17	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.75	234.85	.130E+05	6.0	1.36	36.45
1.50	235.60	.276E+05	18.1	1.95	25.41
2.25	236.35	.544E+05	40.0	2.18	22.69
3.00	237.10	.117E+06	80.1	2.04	24.32
3.75	237.85	.213E+06	148.9	2.07	23.89
4.50	238.60	.334E+06	255.1	2.27	21.85
5.25	239.35	.473E+06	395.8	2.49	19.90
6.00	240.10	.631E+06	563.8	2.65	18.66
6.75	240.85	.824E+06	773.2	2.79	17.77
7.50	241.60	.106E+07	1037.9	2.92	16.97
8.26	242.36	.135E+07	1326.7	2.92	16.94
9.01	243.11	.173E+07	1758.5	3.01	16.44
9.76	243.86	.217E+07	2285.9	3.13	15.81
10.51	244.61	.265E+07	2909.5	3.26	15.17
11.26	245.36	.318E+07	3610.1	3.37	14.67
12.01	246.11	.377E+07	4417.4	3.48	14.21
12.76	246.86	.441E+07	5348.0	3.60	13.75
13.51	247.61	.512E+07	6394.0	3.71	13.34
14.26	248.36	.589E+07	7580.3	3.82	12.95

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8130)	1853.74	14.73	6.50	15.09	1.29
					1.74

OUTFLOW: ID= 1 (6166) 1853.74 12.42 7.00 15.09 1.15 1.62

ADD HYD 1 + 2 = 3	(8136)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6166):		1853.74	12.417	7.00	15.09
+ ID2= 2 (8134):		434.70	13.685	6.00	37.70

ID = 3 (8136):		2288.44	20.196	6.75	19.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2021)
ID= 1 DT=15.0 min Area Total (ha)= 107.32 Imp(%)= 45.00 Di r. Conn. (%)= 27.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	48.29	59.03
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	845.85	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	82.76	55.82
over (min)	15.00	30.00
Storage Coeff. (min)=	12.21 (ii)	25.72 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04
PEAK FLOW (cms)=	5.26	4.11
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	60.70	27.22
TOTAL RAINFALL (mm)=	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.43
TOTALS		7.863 (iii)
		6.00
		36.26
		62.70
		0.58

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5202)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	5.1480	2.8069
0.3630	1.2717	6.4200	3.2412
1.5670	1.4796	7.4640	3.6910
2.7880	1.9296	7.8640	3.7010
3.8580	2.3404	0.0000	0.0000

INFLOW : ID= 2 (2021)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5202)	107.320	7.863	6.00	36.26
	107.320	2.681	6.75	36.25

PEAK FLOW REDUCTION [Out/Oi n] (%) = 34.10
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 1.8977

CALIB STANDHYD (2022)
ID= 1 DT=15.0 min Area Total (ha)= 361.21 Imp(%)= 60.00 Di r. Conn. (%)= 36.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	216.73	144.48
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1551.79	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	82.76	75.36

over (min) 15.00 30.00
 Storage Coeff. (min)= 17.58 (ii) 29.56 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.07 0.04

TOTALS
 PEAK FLOW (cms)= 20.28 12.71 28.387 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 60.70 30.46 41.35
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.49 0.66

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 73.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8110)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2022):	361.21	28.387	6.00	41.35
+ ID2= 2 (5202):	107.32	2.681	6.75	36.25
ID = 3 (8110):	468.53	28.661	6.00	40.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (0102)
 ID= 1 DT=15.0 min

Area	(ha)	Imp(%)	Dir. Conn. (%)
Total	466.00	23.00	9.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	107.18	358.82
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1762.57	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 82.76 12.80
 over (min) 15.00 45.00
 Storage Coeff. (min)= 18.98 (ii) 43.32 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.06 0.03

TOTALS
 PEAK FLOW (cms)= 6.30 6.55 8.922 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 60.70 12.12 16.49
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.19 0.26

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 49.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0100)
 ID= 1 DT=15.0 min

Area	(ha)	Curve Number (CN)
Ia	693.84	40.0
U.H. Tp(hrs)=	5.00	# of Linear Res. (N) = 1.50
	1.95	

Unit Hyd Opeak (cms)= 6.087

PEAK FLOW (cms)= 1.434 (i)
 TIME TO PEAK (hrs)= 9.250
 RUNOFF VOLUME (mm)= 7.560
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.121

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8000)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0100):	693.84	1.434	9.25	7.56
+ ID2= 2 (0102):	466.00	8.922	6.00	16.49
ID = 3 (8000):	1159.84	9.798	6.50	11.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6104)
 IN= 2----> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1041.0) ----->

Distance	Elevation	Manning
0.00	330.16	0.0450
50.03	328.36	0.0450
89.34	326.33	0.0450
117.94	324.30	0.0450
160.82	322.75	0.0450
178.34	319.96	0.0450
183.34	319.81	0.0450
184.34	319.38	0.0450 / 0.0300
185.09	318.78	0.0300
185.84	318.72	0.0300
186.84	319.32	0.0300 / 0.0450
193.84	319.70	0.0450
198.84	320.38	0.0450
200.13	320.81	0.0450
218.00	322.49	0.0450
239.44	323.49	0.0450
250.17	323.96	0.0450
275.18	325.20	0.0450
310.92	326.47	0.0450
353.81	330.00	0.0450

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.30	319.02	.624E+03	0.1	0.45	73.00
0.60	319.32	.181E+04	0.6	0.67	49.46
1.23	319.95	.128E+05	4.9	0.77	43.38
1.86	320.58	.396E+05	19.1	0.96	34.50
2.48	321.20	.757E+05	43.5	1.15	28.98
3.11	321.83	.125E+06	81.2	1.30	25.58
3.74	322.46	.187E+06	135.9	1.45	22.92
4.37	323.09	.269E+06	195.6	1.45	22.90
5.00	323.72	.387E+06	287.8	1.48	22.44
5.63	324.35	.545E+06	428.4	1.56	21.22
6.25	324.97	.735E+06	630.5	1.71	19.42
6.88	325.60	.952E+06	876.1	1.83	18.12
7.51	326.23	.120E+07	1177.0	1.95	17.03
8.14	326.86	.149E+07	1554.4	2.08	15.93
8.77	327.49	.179E+07	2009.7	2.23	14.88
9.40	328.12	.213E+07	2533.9	2.37	13.99
10.02	328.74	.249E+07	3112.6	2.49	13.31
10.65	329.37	.288E+07	3760.9	2.60	12.75
11.28	330.00	.330E+07	4494.8	2.71	12.23

<---- hydrograph ----> <- pipe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8000)	1159.84	9.80	6.50	11.15	1.44	0.82
OUTFLOW: ID= 1 (6104)	1159.84	7.09	6.75	11.15	1.32	0.79

CALIB
 STANDHYD (1041)
 ID= 1 DT=15.0 min

Area	(ha)	Imp(%)	Dir. Conn. (%)
Total	0.67	75.00	70.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.50	0.17
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	66.83	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 82.76 8.45
 over (min) 15.00 45.00

Storage Coeff. (min)= 2.66 (ii) 31.41 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.11 0.03

PEAK FLOW (cms)= 0.11 0.00 *TOTALS* 0.109 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 60.70 8.03 44.87
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.13 0.72

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 36.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5104)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.0450	0.0213
0.0020	0.0120	0.0560	0.0231
0.0130	0.0144	0.0660	0.0248
0.0240	0.0165	0.4660	0.0348
0.0340	0.0189	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1041)	0.670	0.109	6.00	44.87
OUTFLOW: ID= 1 (5104)	0.670	0.029	6.25	42.61

PEAK FLOW REDUCTION [Qout/Qin](%) = 26.55
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 0.0182

CALIB NASHYD (1042)
 ID= 1 DT=15.0 min

Area (ha)= 527.14 Curve Number (CN)= 42.6
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.34

Unit Hyd Qpeak (cms)= 6.731

PEAK FLOW (cms)= 1.594 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 8.269
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.132

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9104)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1042):	527.14	1.594	8.00	8.27
+ ID2= 2 (5104):	0.67	0.029	6.25	42.61
ID = 3 (9104):	527.81	1.601	8.00	8.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8002)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6104):	1159.84	7.088	6.75	11.15
+ ID2= 2 (9104):	527.81	1.601	8.00	8.31
ID = 3 (8002):	1687.65	8.543	7.00	10.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANHYD (1061)
 ID= 1 DT=15.0 min

Area (ha)= 32.26
 Total Imp(%)= 67.00
 Dir. Conn. (%)= 51.00

IMPERVIOUS PERVIOUS (i)

	(ha)	(mm)	(%)	(m)	
Surface Area	21.61	10.65			
Dep. Storage	2.00	5.00			
Average Slope	0.50	0.50			
Length	463.75	40.00			
Mannings n	0.013	0.250			

Max. Eff. Inten. (mm/hr)= 82.76 25.11
 over (min) = 15.00 30.00
 Storage Coeff. (min)= 8.52 (ii) 27.11 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.04

PEAK FLOW (cms)= 3.34 0.53 *TOTALS* 3.664 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 60.70 18.38 39.96
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.29 0.64

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 56.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5106)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.1580	0.9744
0.1090	0.5252	2.7090	1.0694
0.6250	0.6260	3.1540	1.1625
1.1640	0.7386	3.5540	1.1725
1.6140	0.8505	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1061)	32.260	3.664	6.00	39.96
OUTFLOW: ID= 1 (5106)	32.260	0.981	6.50	39.92

PEAK FLOW REDUCTION [Qout/Qin](%) = 26.78
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 0.7010

CALIB NASHYD (1062)
 ID= 1 DT=15.0 min

Area (ha)= 227.27 Curve Number (CN)= 57.5
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.09

Unit Hyd Qpeak (cms)= 3.556

PEAK FLOW (cms)= 1.310 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 13.443
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.214

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9106)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1062):	227.27	1.310	7.50	13.44
+ ID2= 2 (5106):	32.26	0.981	6.50	39.92
ID = 3 (9106):	259.53	2.124	6.75	16.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8004)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8002):	1687.65	8.543	7.00	10.26
+ ID2= 2 (9106):	259.53	2.124	6.75	16.73

ID = 3 (8004):	1947.18	10.589	7.00	11.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6108)	
IN= 2---> OUT= 1	
Routing time step (min)'= 15.00	

<----- DATA FOR SECTION (1081.0) ----->				
Distance	Elevation	Manning		
0.00	326.06	0.0400		
12.06	325.74	0.0400		
30.15	324.28	0.0400		
59.27	320.35	0.0400		
72.37	317.60	0.0400		
87.45	314.52	0.0400		
93.48	313.91	0.0400		
105.54	313.55	0.0400		
106.17	313.40	0.0400		
110.62	313.04	0.0400 / 0.0300	Main Channel	
110.82	312.97	0.0300	Main Channel	
111.57	313.08	0.0300	Main Channel	
111.67	313.48	0.0300 / 0.0400	Main Channel	
129.66	316.62	0.0400		
150.77	318.95	0.0400		
180.92	319.61	0.0400		
205.04	321.23	0.0400		
232.18	322.09	0.0400		
268.37	322.31	0.0400		
298.52	326.00	0.0400		

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.43	313.40	.244E+04	0.7	0.59	58.83
1.09	314.06	.214E+05	10.4	1.01	34.24
1.76	314.73	.600E+05	40.2	1.39	24.89
2.42	315.39	.109E+06	92.1	1.75	19.80
3.08	316.05	.169E+06	166.5	2.05	16.88
3.75	316.72	.237E+06	264.4	2.31	14.97
4.41	317.38	.318E+06	386.3	2.52	13.72
5.07	318.04	.411E+06	539.3	2.72	12.71
5.74	318.71	.517E+06	725.5	2.91	11.88
6.40	319.37	.642E+06	909.2	2.94	11.77
7.06	320.03	.806E+06	1179.3	3.04	11.38
7.72	320.69	.989E+06	1515.9	3.18	10.87
8.39	321.36	.119E+07	1895.5	3.30	10.49
9.05	322.02	.143E+07	2310.2	3.36	10.30
9.71	322.68	.172E+07	2763.2	3.33	10.39
10.38	323.35	.205E+07	3477.6	3.53	9.81
11.04	324.01	.238E+07	4284.3	3.73	9.27
11.70	324.67	.274E+07	5154.0	3.91	8.85
12.37	325.34	.311E+07	6104.0	4.08	8.48

<----- hydrograph ----->					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)
INFLOW : ID= 2 (8004)	1947.18	10.59	7.00	11.12	1.10
OUTFLOW: ID= 1 (6108)	1947.18	9.55	7.50	11.12	1.03

CALIB STANDHYD (1081)			
ID= 1 DT=15.0 mi n			
Area Total	(ha)= 40.81	Imp(%)= 75.00	Dir. Conn.(%)= 70.00
IMPERVIOUS PERVIOUS (i)			
Surface Area	(ha)= 30.61	10.20	
Dep. Storage	(mm)= 2.00	5.00	
Average Slope	(%)= 0.50	0.50	
Length	(m)= 521.60	40.00	
Mannings n	= 0.013	0.250	
Max. Eff. Inten. (mm/hr)= over (mi n)	82.76	5.02	45.00

Storage Coeff. (mi n)=	9.14 (ii)	44.53 (ii)	
Unit Hyd. Tpeak (mi n)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.09	0.03	
			TOTALS
PEAK FLOW (cms)=	5.69	0.09	5.727 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	60.70	6.25	44.37
TOTAL RAINFALL (mm)=	62.70	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.10	0.71

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 30.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5108)				
IN= 2---> OUT= 1				
DT= 15.0 mi n				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	2.7300	1.2970
	0.1380	0.7271	3.4270	1.4050
	0.7910	0.8733	3.9900	1.5110
	1.4720	1.0061	4.3900	1.5210
	2.0420	1.1467	0.0000	0.0000
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1081)	40.810	5.727	6.00	44.37
OUTFLOW: ID= 1 (5108)	40.810	1.546	6.25	44.33
PEAK FLOW REDUCTION [Qout/Qi n] (%) = 27.00				
TIME SHIFT OF PEAK FLOW (mi n) = 15.00				
MAXIMUM STORAGE USED (ha. m.) = 1.0400				

CALIB NASHYD (1082)			
ID= 1 DT=15.0 mi n			
Area (ha)=	166.24	Curve Number (CN)=	32.2
Ia (mm)=	5.00	# of Linear Res. (N)=	1.50
U. H. Tp(hrs)=	0.89		

Unit Hyd Opeak (cms)=	3.171
PEAK FLOW (cms)=	0.452 (i)
TIME TO PEAK (hrs)=	7.250
RUNOFF VOLUME (mm)=	5.551
TOTAL RAINFALL (mm)=	62.700
RUNOFF COEFFICIENT =	0.089

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9108)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1082):	166.24	0.452	7.25	5.55
+ ID2= 2 (5108):	40.81	1.546	6.25	44.33

ID = 3 (9108):	207.05	1.948	6.50	13.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8006)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6108):	1947.18	9.554	7.50	11.12
+ ID2= 2 (9108):	207.05	1.948	6.50	13.20

ID = 3 (8006):	2154.23	10.659	7.50	11.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (1101)
ID= 1 DT=15.0 min

Area (ha)= 60.97
Total Imp(%)= 69.00 Dir. Conn.(%)= 55.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 42.07 18.90
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 637.55 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 82.76 21.48
over (min)= 15.00 45.00
Storage Coeff. (min)= 10.31 (ii) 30.10 (ii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.09 0.03

TOTALS
PEAK FLOW (cms)= 6.45 0.70 6.738 (iii)
TIME TO PEAK (hrs)= 6.00 6.50 6.00
RUNOFF VOLUME (mm)= 60.70 16.23 40.69
TOTAL RAINFALL (mm)= 62.70 62.70 62.70
RUNOFF COEFFICIENT = 0.97 0.26 0.65

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 52.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5110)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.0780	1.8615
0.2060	1.0120	5.1210	2.0372
1.1810	1.2083	5.9610	2.2096
2.2000	1.4181	6.3610	2.2196
3.0510	1.6293	0.0000	0.0000

INFLOW : ID= 2 (1101) 60.970 6.738 6.00 40.69
OUTFLOW: ID= 1 (5110) 60.970 1.806 6.50 40.67

PEAK FLOW REDUCTION [Oout/Oin](%)= 26.80
TIME SHIFT OF PEAK FLOW (min)= 30.00
MAXIMUM STORAGE USED (ha.m.)= 1.3477

CALIB
NASHYD (1102)
ID= 1 DT=15.0 min

Area (ha)= 262.95 Curve Number (CN)= 55.4
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U.H. Tp(hrs)= 1.29

Unit Hyd Qpeak (cms)= 3.483
PEAK FLOW (cms)= 1.257 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 12.609
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.201

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9110)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1102): 262.95	1.257	8.00	12.61
+ ID2= 2 (5110): 60.97	1.806	6.50	40.67
ID = 3 (9110): 323.92	2.831	6.75	17.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8008)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8006): 2154.23	10.659	7.50	11.32	
+ ID2= 2 (9110): 323.92	2.831	6.75	17.89	
ID = 3 (8008): 2478.15	13.072	7.25	12.18	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6112)
IN= 2--> OUT= 1

Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1121.0) ----->

Distance	Elevation	Manning
0.00	320.59	0.0400
13.48	317.93	0.0400
40.45	311.52	0.0400
64.04	307.68	0.0400
74.15	307.01	0.0400
91.00	307.22	0.0400
107.86	305.21	0.0400
111.23	304.86	0.0400
113.32	304.60	0.0400
117.77	304.24	0.0400 / 0.0300
117.97	304.17	0.0300
118.72	304.28	0.0300
118.82	304.68	0.0300 / 0.0400
124.71	304.76	0.0400
171.90	306.30	0.0400
205.60	307.24	0.0400
239.31	308.24	0.0400
262.90	310.64	0.0400
289.86	314.34	0.0400
333.68	317.66	0.0400

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.43	304.60	.372E+04	0.6	0.54	97.65
1.12	305.29	.463E+05	12.5	0.85	61.89
1.80	305.97	.149E+06	53.0	1.13	46.85
2.49	306.66	.311E+06	134.2	1.37	38.62
3.18	307.35	.554E+06	261.6	1.50	35.27
3.87	308.04	.894E+06	490.7	1.74	30.37
4.55	308.72	.129E+07	833.0	2.05	25.75
5.24	309.41	.171E+07	1269.8	2.36	22.39
5.93	310.10	.215E+07	1789.0	2.64	20.01
6.62	310.79	.261E+07	2392.7	2.90	18.21
7.30	311.47	.310E+07	3087.8	3.16	16.74
7.99	312.16	.361E+07	3873.6	3.40	15.52
8.68	312.85	.413E+07	4741.0	3.64	14.52
9.37	313.54	.467E+07	5689.3	3.86	13.69
10.05	314.22	.523E+07	6718.4	4.07	12.97
10.74	314.91	.581E+07	7770.7	4.24	12.46
11.43	315.60	.641E+07	8897.5	4.40	12.01
12.12	316.29	.704E+07	10113.2	4.55	11.61
12.80	316.97	.770E+07	11420.1	4.70	11.24

<---- hydrograph ----> <- pipe / channel ->
INFLOW : ID= 2 (8008) 2478.15 13.07 7.25 12.18 1.13 0.86
OUTFLOW: ID= 1 (6112) 2478.15 10.41 8.25 12.18 1.00 0.78

CALIB
STANDHYD (1121)
ID= 1 DT=15.0 min

Area (ha)= 9.96
Total Imp(%)= 55.00 Dir. Conn.(%)= 25.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 5.48 4.48
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 257.68 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 82.76 10.94
over (min)= 15.00 45.00

Storage Coeff. (min)= 5.99 (ii) 31.91 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.10 0.03

PEAK FLOW (cms)= 0.54 0.08 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.50 0.575 (iii)
 RUNOFF VOLUME (mm)= 60.70 7.49 6.00
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.12 0.33

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 26.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5112)
 IN= 2- -> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.6660	0.2787
0.0340	0.1406	0.8360	0.3122
0.1930	0.1653	0.9740	0.3450
0.3590	0.2034	1.3740	0.3550
0.4980	0.2383	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
9.960	0.575	6.00	20.79
9.960	0.045	8.00	20.69

INFLOW: ID= 2 (1121)
 OUTFLOW: ID= 1 (5112)

PEAK FLOW REDUCTION [Qout/Qin](%)= 7.77
 TIME SHIFT OF PEAK FLOW (min)=120.00
 MAXIMUM STORAGE USED (ha.m.)= 0.1424

CALIB NASHYD (1122)
 ID= 1 DT=15.0 min

Area (ha)= 404.93 Curve Number (CN)= 31.1
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 1.90

Unit Hyd Qpeak (cms)= 3.635

PEAK FLOW (cms)= 0.600 (i)
 TIME TO PEAK (hrs)= 9.250
 RUNOFF VOLUME (mm)= 5.345
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.085

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9112)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
404.93	0.600	9.25	5.35
9.96	0.045	8.00	20.69
414.89	0.634	9.25	5.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8010)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
2478.15	10.412	8.25	12.18
414.89	0.634	9.25	5.71
2893.04	11.042	8.25	11.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6114) | Routing time step (min)'= 15.00
 IN= 2- -> OUT= 1

<----- DATA FOR SECTION (1141.0) ----->

Distance	Elevation	Manning
0.00	312.27	0.0500
14.07	309.70	0.0500
28.14	306.51	0.0500
35.17	305.81	0.0500
42.20	304.95	0.0500
52.75	304.24	0.0500
77.37	303.61	0.0500
98.47	301.33	0.0500
131.76	300.40	0.0500 / 0.0300
136.21	300.04	0.0300
136.41	299.97	0.0300
137.16	300.08	0.0300
137.26	300.48	0.0300 / 0.0500
140.68	300.53	0.0500
179.36	302.20	0.0500
214.53	303.82	0.0500
242.66	305.27	0.0500
288.38	307.66	0.0500
316.52	311.76	0.0500
348.17	313.48	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.43	300.40	366E+04	0.7	0.59	88.31
1.05	301.02	505E+05	12.1	0.75	69.49
1.68	301.65	165E+06	52.7	0.99	52.28
2.30	302.27	324E+06	130.3	1.26	41.40
2.93	302.90	520E+06	247.1	1.48	35.08
3.55	303.52	754E+06	406.8	1.68	30.91
4.18	304.15	104E+07	586.1	1.76	29.55
4.80	304.77	138E+07	856.0	1.93	26.96
5.43	305.40	177E+07	1201.1	2.12	24.56
6.05	306.02	219E+07	1612.2	2.30	22.63
6.68	306.65	264E+07	2090.6	2.47	21.07
7.30	307.27	313E+07	2653.0	2.64	19.65
7.93	307.90	364E+07	3303.9	2.83	18.36
8.55	308.52	417E+07	4053.9	3.03	17.14
9.18	309.15	471E+07	4871.9	3.22	16.12
9.80	309.77	527E+07	5756.2	3.41	15.25
10.43	310.40	584E+07	6699.8	3.58	14.52
11.05	311.02	642E+07	7710.5	3.74	13.89
11.68	311.65	702E+07	8788.5	3.90	13.32

<---- hydrograph ----> <- pi pe / channel ->
 AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW: ID= 2 (8010) 2893.04 11.04 8.25 11.25 1.00 0.73
 OUTFLOW: ID= 1 (6114) 2893.04 9.40 9.75 11.25 0.90 0.70

CALIB STANDHYD (1141)
 ID= 1 DT=15.0 min

Area (ha)= 19.45 Dir. Conn. (%)= 15.00
 Total Imp(%)= 45.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 8.75 10.70
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 360.09 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 82.76 17.93
 over (min) 15.00 30.00
 Storage Coeff. (min)= 7.32 (ii) 28.59 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.10 0.04

TOTALS
 PEAK FLOW (cms)= 0.61 0.37 0.837 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 60.70 12.91 20.08
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.21 0.32

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 43.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5114) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	1.3010	0.5065
	0.0660	0.2376	1.6340	0.5791
	0.3770	0.2749	1.9020	0.6501
	0.7020	0.3549	2.3020	0.6601
	0.9730	0.4237	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1141)	19.450	0.837	6.00	20.08
OUTFLOW: ID= 1 (5114)	19.450	0.126	7.50	20.04

PEAK FLOW REDUCTION [Qout/Oin] (%) = 15.10
 TIME SHIFT OF PEAK FLOW (min) = 90.00
 MAXIMUM STORAGE USED (ha. m.) = 0.2449

CALIB NASHYD (1142) ID= 1 DT=15.0 min	Area (ha) = 268.19 Ia (mm) = 5.00 U.H. Tp(hrs) = 1.09	Curve Number (CN) = 51.3 # of Linear Res. (N) = 1.50
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Unit Hyd Qpeak (cms) = 4.197
 PEAK FLOW (cms) = 1.262 (i)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 11.041
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.176

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9114) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1142):	268.19	1.262	7.75	11.04
+ ID2= 2 (5114):	19.45	0.126	7.50	20.04
=====				
ID = 3 (9114):	287.64	1.388	7.50	11.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8012) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6114):	2893.04	9.402	9.75	11.25
+ ID2= 2 (9114):	287.64	1.388	7.50	11.65
=====				
ID = 3 (8012):	3180.68	10.477	9.75	11.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0116) ID= 1 DT=15.0 min	Area (ha) = 206.43 Ia (mm) = 5.00 U.H. Tp(hrs) = 0.80	Curve Number (CN) = 64.0 # of Linear Res. (N) = 1.50
--	---	---

Unit Hyd Qpeak (cms) = 4.394
 PEAK FLOW (cms) = 1.864 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 16.364
 TOTAL RAINFALL (mm) = 62.700

RUNOFF COEFFICIENT = 0.261

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8014) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0116):	206.43	1.864	7.00	16.36
+ ID2= 2 (8012):	3180.68	10.477	9.75	11.29
=====				
ID = 3 (8014):	3387.11	11.633	9.50	11.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6118) IN= 2--> OUT= 1	Routing time step (min) = 15.00
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<----- DATA FOR SECTION (1181.0) ----->

Distance	Elevation	Manning
0.00	307.18	0.0450
24.89	299.75	0.0450
37.34	296.39	0.0450
71.57	292.75	0.0450
96.46	291.12	0.0450
108.90	289.43	0.0450
112.02	288.86	0.0450
115.13	288.46	0.0450
117.24	288.32	0.0450 / 0.0350
117.49	288.15	0.0350
118.24	288.00	0.0350
118.99	288.20	0.0350
119.24	288.36	0.0350 / 0.0450
121.35	288.40	0.0450
149.35	289.36	0.0450
171.14	290.47	0.0450
208.47	293.14	0.0450
236.48	295.17	0.0450
280.04	299.48	0.0450
308.04	307.00	0.0450

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.32	288.32	.116E+04	0.2	0.62	81.92
1.30	289.30	.631E+05	22.2	1.07	47.43
2.29	290.29	.218E+06	117.5	1.64	30.95
3.27	291.27	.448E+06	314.6	2.15	23.72
4.25	292.25	.756E+06	625.6	2.53	20.13
5.24	293.24	.115E+07	1093.2	2.91	17.50
6.22	294.22	.161E+07	1731.6	3.28	15.50
7.20	295.20	.214E+07	2547.4	3.63	14.01
8.19	296.19	.274E+07	3584.9	4.00	12.72
9.17	297.17	.338E+07	4859.1	4.39	11.60
10.15	298.15	.407E+07	6341.5	4.76	10.70
11.13	299.13	.480E+07	8024.6	5.11	9.97
12.12	300.12	.556E+07	10006.6	5.49	9.27
13.10	301.10	.635E+07	12244.6	5.89	8.64
14.08	302.08	.716E+07	14681.7	6.26	8.13
15.07	303.07	.799E+07	17316.5	6.62	7.69
16.05	304.05	.884E+07	20148.1	6.96	7.31
17.03	305.03	.971E+07	23176.2	7.29	6.98
18.02	306.02	.106E+08	26400.8	7.61	6.69

<----- hydrograph -----> <- pipe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8014)	3387.11	11.63	9.50	11.60	0.83	0.80
OUTFLOW: ID= 1 (6118)	3387.11	10.85	10.50	11.60	0.79	0.78

CALIB NASHYD (0118) ID= 1 DT=15.0 min	Area (ha) = 340.53 Ia (mm) = 5.00 U.H. Tp(hrs) = 1.12	Curve Number (CN) = 45.0 # of Linear Res. (N) = 1.50
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Unit Hyd Qpeak (cms) = 5.200

PEAK FLOW (cms)= 1.273 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 8.965
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.143

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8016)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0118):	340.53	1.273	7.75	8.97
+ ID2= 2 (6118):	3387.11	10.853	10.50	11.60

ID = 3 (8016):	3727.64	11.774	10.50	11.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6122)
 IN= 2----> OUT= 1
 Routing time step (min)' = 15.00

DATA FOR SECTION (1221.0) ----->				
Distance	Elevation	Manning		
0.00	277.00	0.0500		
52.18	272.82	0.0500		
86.97	269.38	0.0500		
134.80	267.35	0.0500		
160.89	263.85	0.0500		
169.58	262.88	0.0500		
178.28	262.45	0.0500		
195.67	262.07	0.0500		
213.07	260.99	0.0500		
226.01	259.80	0.0500 / 0.0300	Main Channel	
226.11	259.30	0.0300	Main Channel	
230.11	259.30	0.0300	Main Channel	
230.21	259.80	0.0300 / 0.0500	Main Channel	
235.01	260.20	0.0500		
239.15	260.57	0.0500		
243.50	260.93	0.0500		
300.03	264.43	0.0500		
339.16	268.34	0.0500		
373.95	272.36	0.0500		
430.48	277.46	0.0500		

TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.50	259.80	.626E+04	2.5	1.23	41.35
1.41	260.71	.462E+05	21.8	1.44	35.27
2.31	261.61	.149E+06	79.6	1.63	31.19
3.22	262.52	.340E+06	200.1	1.80	28.34
4.12	263.42	.636E+06	446.8	2.15	23.71
5.03	264.33	.995E+06	819.9	2.52	20.23
5.93	265.23	.141E+07	1337.0	2.90	17.56
6.84	266.14	.187E+07	1984.8	3.25	15.67
7.74	267.04	.237E+07	2766.0	3.57	14.26
8.65	267.95	.292E+07	3596.9	3.76	13.53
9.55	268.85	.355E+07	4575.4	3.93	12.95
10.46	269.76	.427E+07	5814.9	4.16	12.23
11.36	270.66	.503E+07	7319.0	4.44	11.46
12.27	271.57	.584E+07	9009.7	4.71	10.81
13.17	272.47	.670E+07	10886.3	4.96	10.26
14.08	273.38	.761E+07	12893.2	5.17	9.84
14.98	274.28	.858E+07	15098.1	5.37	9.48
15.89	275.19	.961E+07	17527.6	5.57	9.14
16.79	276.09	.107E+08	20190.3	5.76	8.83

<---- hydrograph ----->					<-pi pe / channel -->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW: ID= 2 (8016)	3727.64	11.77	10.50	11.36	0.93	
OUTFLOW: ID= 1 (6122)	3727.64	11.50	11.25	11.36	0.92	

CALIB STANDHYD (1221)
 ID= 1 DT=15.0 mi n
 Area (ha)= 117.91
 Total Imp(%)= 44.00
 Di r. Conn.(%)= 16.00

IMPERVIOUS PERVIOUS (i)			
Surface Area (ha)=	51.88	66.03	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	886.60	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	82.76	13.18	
over (mi n)	15.00	45.00	
Storage Coeff. (mi n)=	12.56 (ii)	36.62 (ii)	
Unit Hyd. Tpeak (mi n)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.08	0.03	
TOTALS			
PEAK FLOW (cms)=	3.39	1.36	3.941 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	60.70	9.92	18.05
TOTAL RAINFALL (mm)=	62.70	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.16	0.29

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 36.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5122)				
IN= 2----> OUT= 1				
DT= 15.0 mi n				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	7.8880	3.0390
	0.3990	1.4100	9.9040	3.4850
	2.2850	1.6269	11.5290	3.9214
	4.2550	2.1164	11.9290	3.9314
	5.9010	2.5338	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1221)	117.910	3.941	6.00	18.05
OUTFLOW: ID= 1 (5122)	117.910	0.397	8.75	18.04

PEAK FLOW REDUCTION [Out/Oi n](%)= 10.08				
TIME SHIFT OF PEAK FLOW (mi n)=165.00				
MAXIMUM STORAGE USED (ha. m.)= 1.4048				

CALIB NASHYD (1222)
 ID= 1 DT=15.0 mi n
 Area (ha)= 246.95
 Curve Number (CN)= 43.8
 U. H. Tp(hrs)= 1.13
 # of Li near Res. (N)= 1.50

Unit Hyd Opeak (cms)= 3.728
 PEAK FLOW (cms)= 0.878 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 8.605
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.137
 (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8018)				
1 + 2 = 3				
ID1= 1 (1222):	246.95	0.878	7.75	8.61
+ ID2= 2 (5122):	117.91	0.397	8.75	18.04

ID = 3 (8018):	364.86	1.268	8.00	11.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6122):	3727.64	11.502	11.25	11.36
+ ID2= 2 (8018):	364.86	1.268	8.00	11.65

ID = 3 (8020):	4092.50	12.444	11.00	11.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1261) ID= 1 DT=15.0 min	Area (ha)=	Imp (%)=	Di r. Conn. (%)=
	50.19	36.00	26.00

	IMPERVIOUS (ha)=	PERVIOUS (i) (mm)=
Surface Area	18.07	32.12
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	578.45	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	82.76	11.89
over (min)	15.00	45.00
Storage Coeff. (min)=	9.72 (ii)	34.80 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.09	0.03

TOTALS			
PEAK FLOW (cms)=	2.55	0.61	2.800 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	60.70	11.54	24.32
TOTAL RAINFALL (mm)=	62.70	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.18	0.39

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 48.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5126) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	3.3570	1.2179
	0.1700	0.5264	4.2160	1.4221
	0.9720	0.5968	4.9080	1.6217
	1.8110	0.8165	5.3080	1.6317
	2.5120	0.9954	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1261)	50.190	2.800	6.00	24.32
OUTFLOW: ID= 1 (5126)	50.190	0.920	6.75	24.30

PEAK FLOW REDUCTION [Qout/Qin] (%) = 32.87
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 0.5999

CALIB NASHYD (1262) ID= 1 DT=15.0 min	Area (ha)=	Curve Number (CN)=
	83.33	57.7
	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50
	U. H. Tp(hrs)= 0.57	

Unit Hyd Opeak (cms)=	2.509
PEAK FLOW (cms)=	0.797 (i)
TIME TO PEAK (hrs)=	6.500
RUNOFF VOLUME (mm)=	13.328
TOTAL RAINFALL (mm)=	62.700
RUNOFF COEFFICIENT =	0.213

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1262):	83.33	0.797	6.50	13.33
+ ID2= 2 (5126):	50.19	0.920	6.75	24.30

ID = 3 (8024):	133.52	1.714	6.75	17.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0124) ID= 1 DT=15.0 min	Area (ha)=	Curve Number (CN)=
	121.43	43.0
	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50
	U. H. Tp(hrs)= 0.51	

Unit Hyd Opeak (cms)= 4.049

PEAK FLOW (cms)=	0.766 (i)
TIME TO PEAK (hrs)=	6.500
RUNOFF VOLUME (mm)=	8.210
TOTAL RAINFALL (mm)=	62.700
RUNOFF COEFFICIENT =	0.131

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0124):	121.43	0.766	6.50	8.21
+ ID2= 2 (8024):	133.52	1.714	6.75	17.45

ID = 3 (8026):	254.95	2.466	6.75	13.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6128) IN= 2--> OUT= 1	Routing time step (min)=
	15.00

<----- DATA FOR SECTION (1281.0) ----->			
Di stance	El evation	Manning	
0.00	270.58	0.0400	
7.71	270.36	0.0400	
15.43	269.48	0.0400	
23.14	265.88	0.0400	
42.43	261.83	0.0400	
61.71	258.31	0.0400	
75.21	256.62	0.0400	
90.64	255.35	0.0400	
92.49	255.00	0.0400 / 0.0300	Mai n Channel
93.49	254.00	0.0300	Mai n Channel
94.49	253.90	0.0300	Mai n Channel
94.99	253.80	0.0300	Mai n Channel
95.49	253.70	0.0300	Mai n Channel
95.99	253.80	0.0300	Mai n Channel
98.49	255.00	0.0300 / 0.0400	Mai n Channel
100.28	255.44	0.0400	
134.99	257.45	0.0400	
148.49	259.67	0.0400	
167.77	263.42	0.0400	
190.92	270.00	0.0400	

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.65	254.35	.439E+04	2.0	1.24	36.10
1.30	255.00	.131E+05	9.4	1.92	23.39
2.18	255.88	.433E+05	37.0	2.30	19.54
3.06	256.76	.131E+06	113.3	2.34	19.20
3.95	257.65	.273E+06	273.8	2.70	16.64
4.83	258.53	.453E+06	538.5	3.21	14.01
5.71	259.41	.658E+06	902.9	3.70	12.14
6.59	260.29	.886E+06	1368.4	4.16	10.79
7.48	261.18	.114E+07	1936.3	4.59	9.79
8.36	262.06	.141E+07	2610.9	4.99	9.01
9.24	262.94	.171E+07	3398.7	5.37	8.36
10.12	263.82	.202E+07	4309.6	5.75	7.81
11.01	264.71	.235E+07	5347.5	6.13	7.33

11.89	265.59	.270E+07	6501.0	6.48	6.93
12.77	266.47	.307E+07	7808.6	6.86	6.55
13.65	267.35	.345E+07	9250.5	7.23	6.21
14.54	268.24	.384E+07	10808.3	7.59	5.92
15.42	269.12	.424E+07	12482.5	7.94	5.66
16.30	270.00	.466E+07	14127.4	8.18	5.49

<---- hydrograph ----> <-pi / channel ->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8026)	254.95	2.47	6.75	13.05	0.69	1.27
OUTFLOW: ID= 1 (6128)	254.95	1.95	7.25	13.05	0.63	1.24

CALIB STANDHYD (1281) ID= 1 DT=15.0 min	Area (ha)= 55.87	Imp(%)= 50.00	Di r. Conn.(%)= 29.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	27.93	27.93
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	610.30	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)=	82.76	21.46
over (min)	15.00	30.00
Storage Coeff. (min)=	10.04 (ii)	29.84 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.04
PEAK FLOW (cms)=	3.14	1.12
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	60.70	16.55
TOTAL RAINFALL (mm)=	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.26

TOTALS
3.829 (iii)
6.00
29.36
62.70
0.47

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 54.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5128)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.7380	1.5139
0.1890	0.7401	4.6930	1.7113
1.0830	0.8643	5.4630	1.9045
2.0160	1.0853	5.8630	1.9045
2.7960	1.2819	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1281)	55.870	3.829	6.00	29.36
OUTFLOW: ID= 1 (5128)	55.870	1.115	6.75	29.34

PEAK FLOW REDUCTION [Qout/Oi n](%)= 29.13
TIME SHIFT OF PEAK FLOW (min)= 45.00
MAXIMUM STORAGE USED (ha.m.)= 0.8772

CALIB NASHYD (1282) ID= 1 DT=15.0 min	Area (ha)= 184.36	Curve Number (CN)= 62.0
	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50
	U.H. Tp(hrs)= 0.62	

Unit Hyd Qpeak (cms)=	5.093
PEAK FLOW (cms)=	1.901 (i)
TIME TO PEAK (hrs)=	6.750
RUNOFF VOLUME (mm)=	15.278
TOTAL RAINFALL (mm)=	62.700
RUNOFF COEFFICIENT =	0.244

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9128) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1282):	184.36	1.901	6.75	15.28
+ ID2= 2 (5128):	55.87	1.115	6.75	29.34
ID = 3 (9128):	240.23	3.017	6.75	18.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8028) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6128):	254.95	1.949	7.25	13.05
+ ID2= 2 (9128):	240.23	3.017	6.75	18.55
ID = 3 (8028):	495.18	4.758	7.00	15.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8022) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8020):	4092.50	12.444	11.00	11.38
+ ID2= 2 (8028):	495.18	4.758	7.00	15.72
ID = 3 (8022):	4587.68	13.890	10.75	11.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1201) ID= 1 DT=15.0 min	Area (ha)= 176.35	Imp(%)= 45.00	Di r. Conn.(%)= 22.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	79.36	96.99
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1084.28	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)=	82.76	17.28
over (min)	15.00	45.00
Storage Coeff. (min)=	14.18 (ii)	35.77 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.07	0.03

TOTALS
6.65 2.65 7.732 (iii)
6.00 6.50 6.00
60.70 13.53 23.91
62.70 62.70 62.70
0.97 0.22 0.38

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 47.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5120)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	11.7960	4.5929
0.5970	2.1554	14.8120	5.2509
3.4170	2.4937	17.2430	5.8948
6.3630	3.2187	17.6430	5.9048
8.8260	3.8422	0.0000	0.0000

INFLOW : ID= 2 (1201) AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 176.350 7.732 6.00 23.91
 OUTFLOW: ID= 1 (5120) 176.350 2.126 7.25 23.91

PEAK FLOW REDUCTION [Qout/Qin] (%) = 27.49
 TIME SHIF T OF PEAK FLOW (mi n) = 75.00
 MAXIMUM STORAGE USED (ha. m.) = 2.3500

CALIB STANDHYD (1202) | Area (ha) = 70.67
 ID= 1 DT=15.0 mi n | Total Imp(%) = 43.00 Di r. Conn. (%) = 20.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 30.39 40.28
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 686.39 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 82.76 16.94
 over (mi n) = 15.00 45.00
 Storage Coeff. (mi n) = 10.78 (ii) 32.54 (ii)
 Unit Hyd. Tpeak (mi n) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.09 0.03

TOTALS
 PEAK FLOW (cms) = 2.68 1.13 3.142 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 60.70 13.41 22.87
 TOTAL RAINFALL (mm) = 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.21 0.36

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 47.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8030) | Area (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 1 + 2 = 3 |
 ID1= 1 (1202): 70.67 3.142 6.00 22.87
 + ID2= 2 (5120): 176.35 2.126 7.25 23.91
 ID = 3 (8030): 247.02 3.428 6.00 23.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8032) | Area (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 1 + 2 = 3 |
 ID1= 1 (8022): 4587.68 13.890 10.75 11.85
 + ID2= 2 (8030): 247.02 3.428 6.00 23.61
 ID = 3 (8032): 4834.70 14.633 10.75 12.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6130) | Routing time step (mi n)' = 15.00
 IN= 2----> OUT= 1 |

<----- DATA FOR SECTION (1301.0) ----->
 Di stance El evati on Manni ng
 0.00 257.18 0.0350
 62.45 254.70 0.0350
 109.29 252.74 0.0350
 150.92 251.55 0.0350
 192.56 248.96 0.0350
 239.39 247.16 0.0350

291.84	246.00	0.0350		
296.84	245.00	0.0350		
298.84	245.00	0.0350 / 0.0300	Mai n	Channel
299.24	244.60	0.0300	Mai n	Channel
301.84	244.60	0.0300	Mai n	Channel
303.44	244.60	0.0300	Mai n	Channel
303.84	245.00	0.0300 / 0.0350	Mai n	Channel
306.84	245.00	0.0350		
309.84	246.00	0.0350		
317.46	247.22	0.0350		
348.68	254.83	0.0350		
359.09	256.70	0.0350		
452.76	258.23	0.0350		
515.22	258.33	0.0350		

<----- TRAVEL TIME TABLE ----->
 DEPTH (m) ELEV (m) VOLUME (cu. m.) FLOW RATE (cms) VELOCITY (m/s) TRAV. TIME (mi n)
 0.40 245.00 .870E+04 1.2 0.64 124.12
 1.04 245.64 .468E+05 9.5 0.96 81.81
 1.68 246.28 .109E+06 25.6 1.11 70.86
 2.32 246.92 .257E+06 63.5 1.17 67.51
 2.96 247.56 .498E+06 142.7 1.36 58.18
 3.61 248.21 .800E+06 266.7 1.58 50.01
 4.25 248.85 .116E+07 437.4 1.78 44.24
 4.89 249.49 .157E+07 669.1 2.01 39.20
 5.53 250.13 .203E+07 954.9 2.23 35.36
 6.17 250.77 .252E+07 1294.4 2.43 32.42
 6.81 251.41 .305E+07 1689.6 2.62 30.07
 7.45 252.05 .363E+07 2090.7 2.73 28.93
 8.09 252.69 .429E+07 2553.1 2.82 27.98
 8.73 253.33 .501E+07 3138.4 2.96 26.60
 9.37 253.97 .579E+07 3803.8 3.11 25.36
 10.02 254.62 .662E+07 4549.3 3.25 24.25
 10.66 255.26 .751E+07 5364.9 3.38 23.32
 11.30 255.90 .845E+07 6265.7 3.51 22.49
 11.94 256.54 .946E+07 7260.1 3.63 21.72

<---- hydrograph ----> <-pi pe / channel ->
 AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm) MAX DEPTH (m) MAX VEL (m/s)
 INFLOW : ID= 2 (8032) 4834.70 14.63 10.75 12.45 1.24 1.01
 OUTFLOW: ID= 1 (6130) 4834.70 13.90 11.75 12.45 1.22 1.00

CALIB STANDHYD (1301) | Area (ha) = 320.20
 ID= 1 DT=15.0 mi n | Total Imp(%) = 64.00 Di r. Conn. (%) = 47.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 204.93 115.27
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 1461.05 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 82.76 69.23
 over (mi n) = 15.00 30.00
 Storage Coeff. (mi n) = 16.96 (ii) 29.35 (ii)
 Unit Hyd. Tpeak (mi n) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.07 0.04

TOTALS
 PEAK FLOW (cms) = 23.87 9.34 29.818 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 60.70 30.35 44.61
 TOTAL RAINFALL (mm) = 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.48 0.71

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5130) |
 IN= 2----> OUT= 1 |
 DT= 15.0 mi n |
 OUTFLOW (cms) STORAGE (ha. m.) | OUTFLOW (cms) STORAGE (ha. m.)
 0.0000 0.0000 | 18.6040 9.7083

1. 1320	5. 3237	22. 6740	10. 8355
6. 6360	6. 1006	30. 4650	12. 2155
10. 3780	7. 2341	31. 8650	12. 2255
14. 1960	8. 4233	0. 0000	0. 0000

INFLOW : ID= 2 (1301)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5130)	320. 200	29. 818	6. 00	44. 61
	320. 200	10. 368	6. 75	44. 61

PEAK FLOW REDUCTION [Qout/Qin] (%) = 34. 77
 TIME SHIFT OF PEAK FLOW (min) = 45. 00
 MAXIMUM STORAGE USED (ha. m.) = 7. 2529

CALIB NASHYD (1302)	Area (ha)	Imp (%)	Dir. Conn. (%)
ID= 1 DT=15. 0 min	65. 86	5. 00	27. 00
	U. H. Tp (hrs) = 0. 69		

Unit Hyd Qpeak (cms) = 1. 620

PEAK FLOW (cms)	1. 139 (i)
TIME TO PEAK (hrs)	6. 750
RUNOFF VOLUME (mm)	26. 988
TOTAL RAINFALL (mm)	62. 700
RUNOFF COEFFICIENT	0. 430

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8036)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (1302):	65. 86	1. 139	6. 75	26. 99
+ ID2= 2 (5130):	320. 20	10. 368	6. 75	44. 61
ID = 3 (8036):	386. 06	11. 507	6. 75	41. 60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8034)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (6130):	4834. 70	13. 900	11. 75	12. 45
+ ID2= 2 (8036):	386. 06	11. 507	6. 75	41. 60
ID = 3 (8034):	5220. 76	15. 884	7. 25	14. 61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1861)	Area (ha)	Imp (%)	Dir. Conn. (%)
ID= 1 DT=15. 0 min	6. 85	51. 00	27. 00

Surface Area (ha)	3. 49	PERVIOUS (i)	3. 36
Dep. Storage (mm)	2. 00		5. 00
Average Slope (%)	0. 50		0. 50
Length (m)	213. 70		40. 00
Mannings n	0. 013		0. 250

Max. Eff. Inten. (mm/hr) over (min)	82. 76	22. 42
Storage Coeff. (min)	15. 00	30. 00
Unit Hyd. Tpeak (min)	5. 35 (ii)	24. 81 (ii)
Unit Hyd. peak (cms)	15. 00	30. 00
	0. 11	0. 04

PEAK FLOW (cms)	0. 41	0. 16	*TOTALS* (iii)
TIME TO PEAK (hrs)	6. 00	6. 25	6. 00
RUNOFF VOLUME (mm)	60. 70	16. 49	28. 42
TOTAL RAINFALL (mm)	62. 70	62. 70	62. 70
RUNOFF COEFFICIENT	0. 97	0. 26	0. 45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 52. 5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5186)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2--> OUT= 1				
DT= 15. 0 min	0. 0000	0. 0000	0. 4580	0. 1865
	0. 0230	0. 0917	0. 5750	0. 2106
	0. 1330	0. 1071	0. 6700	0. 2341
	0. 2470	0. 1341	1. 0700	0. 2441
	0. 3430	0. 1582	0. 0000	0. 0000

INFLOW : ID= 2 (1861)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5186)	6. 850	0. 504	6. 00	28. 42
	6. 850	0. 127	6. 75	28. 28

PEAK FLOW REDUCTION [Qout/Qin] (%) = 25. 28
 TIME SHIFT OF PEAK FLOW (min) = 45. 00
 MAXIMUM STORAGE USED (ha. m.) = 0. 1070

CALIB STANDHYD (1862)	Area (ha)	Imp (%)	Dir. Conn. (%)
ID= 1 DT=15. 0 min	344. 94	43. 00	23. 00

Surface Area (ha)	148. 32	PERVIOUS (i)	196. 62
Dep. Storage (mm)	2. 00		5. 00
Average Slope (%)	0. 50		0. 50
Length (m)	1516. 44		40. 00
Mannings n	0. 013		0. 250

Max. Eff. Inten. (mm/hr) over (min)	82. 76	18. 62
Storage Coeff. (min)	15. 00	45. 00
Unit Hyd. Tpeak (min)	17. 34 (ii)	38. 29 (ii)
Unit Hyd. peak (cms)	15. 00	45. 00
	0. 07	0. 03

PEAK FLOW (cms)	12. 45	5. 57	*TOTALS* (iii)
TIME TO PEAK (hrs)	6. 00	6. 50	6. 00
RUNOFF VOLUME (mm)	60. 70	15. 19	25. 66
TOTAL RAINFALL (mm)	62. 70	62. 70	62. 70
RUNOFF COEFFICIENT	0. 97	0. 24	0. 41

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 52. 5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9186)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (1862):	344. 94	14. 728	6. 00	25. 66
+ ID2= 2 (5186):	6. 85	0. 127	6. 75	28. 28
ID = 3 (9186):	351. 79	14. 743	6. 00	25. 71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1881)	Area (ha)	Imp (%)	Dir. Conn. (%)
ID= 1 DT=15. 0 min	1. 12	75. 00	46. 00

Surface Area (ha)	0. 84	PERVIOUS (i)	0. 28
Dep. Storage (mm)	2. 00		5. 00
Average Slope (%)	0. 50		0. 50
Length (m)	86. 41		40. 00
Mannings n	0. 013		0. 250

Max. Eff. Inten. (mm/hr)= 82.76 85.25
 over (min)= 15.00 15.00
 Storage Coeff. (min)= 3.11 (ii) 14.51 (ii)
 Unit Hyd. Tpeak (min)= 15.00 15.00
 Unit Hyd. peak (cms)= 0.11 0.07

PEAK FLOW (cms)= 0.12 0.05
 TIME TO PEAK (hrs)= 6.00 6.00
 RUNOFF VOLUME (mm)= 60.70 25.90
 TOTAL RAINFALL (mm)= 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.41

TOTALS
 (iii)
 0.165
 41.90
 62.70
 0.67

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5188)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0750	0.0357
0.0040	0.0200	0.0940	0.0386
0.0220	0.0240	0.1100	0.0416
0.0410	0.0277	0.5100	0.0516
0.0560	0.0315	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1881)	1.120	0.165	6.00	41.90
OUTFLOW: ID= 1 (5188)	1.120	0.036	6.50	40.78

PEAK FLOW REDUCTION [Qout/Qin] (%) = 21.86
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0269

CALIB STANDHYD (1882)
 ID= 1 DT=15.0 min

Area (ha)= 307.41
 Total Imp(%)= 41.00
 Dir. Conn. (%) = 25.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	126.04	181.37
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1431.57	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 82.76 20.63
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 16.75 (ii) 36.86 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.07 0.03

PEAK FLOW (cms)= 12.26 5.80
 TIME TO PEAK (hrs)= 6.00 6.50
 RUNOFF VOLUME (mm)= 60.70 17.68
 TOTAL RAINFALL (mm)= 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.28

TOTALS
 (iii)
 14.641
 28.43
 62.70
 0.45

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9188)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1882):	307.41	14.641	6.00	28.43
+ ID2= 2 (5188):	1.12	0.036	6.50	40.78
ID = 3 (9188):	308.53	14.645	6.00	28.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8068)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9186):	351.79	14.743	6.00	25.71
+ ID2= 2 (9188):	308.53	14.645	6.00	28.48
ID = 3 (8068):	660.32	29.388	6.00	27.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1821)
 ID= 1 DT=15.0 min

Area (ha)= 111.80
 Total Imp(%)= 40.00
 Dir. Conn. (%) = 17.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	44.72	67.08
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	863.33	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 82.76 19.18
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 12.37 (ii) 33.07 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03

PEAK FLOW (cms)= 3.44 2.11
 TIME TO PEAK (hrs)= 6.00 6.50
 RUNOFF VOLUME (mm)= 60.70 15.27
 TOTAL RAINFALL (mm)= 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.24

TOTALS
 (iii)
 4.304
 6.00
 23.00
 62.70
 0.37

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 52.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5182)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	5.7800	2.5656
0.3790	1.0207	7.3720	3.0829
1.6240	1.1939	8.7710	3.5956
3.0930	1.6631	9.1710	3.6056
4.3530	2.1032	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1821)	111.800	4.304	6.00	23.00
OUTFLOW: ID= 1 (5182)	111.800	1.685	7.00	22.99

PEAK FLOW REDUCTION [Qout/Qin] (%) = 39.16
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 1.2149

CALIB NASHYD (1822)
 ID= 1 DT=15.0 min

Area (ha)= 44.40
 Ia (mm)= 5.00
 U. H. Tp(hrs)= 0.79
 Curve Number (CN)= 57.0
 # of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 0.966

PEAK FLOW (cms)= 0.325 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 13.159

TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.210

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8062) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1822):	44.40	0.325	7.00	13.16
+ ID2= 2 (5182):	111.80	1.685	7.00	22.99

ID = 3 (8062):	156.20	2.010	7.00	20.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1841) ID= 1 DT=15.0 min	Area (ha)	Total Imp(%)	Dir. Conn. (%)
	145.07	48.00	19.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	69.63	75.44
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	983.43	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)= over (min)	82.76 15.00	44.15 30.00
Storage Coeff. (min)=	13.37 (ii)	28.21 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04

TOTALS
 PEAK FLOW (cms)= 4.84 3.93 7.269 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 60.70 18.81 26.77
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.30 0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 55.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5184) IN= 2--- OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	5.9270	2.3472
	0.2980	1.0088	7.4630	2.7585
	1.6660	1.1775	8.7490	3.1642
	3.1100	1.4809	9.1490	3.1742
	4.4100	1.8897	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1841)	145.070	7.269	6.00	26.77
OUTFLOW: ID= 1 (5184)	145.070	3.431	6.75	26.77

PEAK FLOW REDUCTION [Qout/Qin] (%) = 47.20
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 1.6030

CALIB STANDHYD (1842) ID= 1 DT=15.0 min	Area (ha)	Total Imp(%)	Dir. Conn. (%)
	117.45	42.00	17.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	49.33	68.12
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50

Length (m)=	884.87	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)= over (min)	82.76 15.00	23.10 45.00
Storage Coeff. (min)=	12.55 (ii)	31.77 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03

TOTALS
 PEAK FLOW (cms)= 3.59 2.63 4.681 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 60.70 17.59 24.92
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.28 0.40

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 55.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8060) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1842):	117.45	4.681	6.00	24.92
+ ID2= 2 (5184):	145.07	3.431	6.75	26.76

ID = 3 (8060):	262.52	6.899	6.50	25.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8064) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8060):	262.52	6.899	6.50	25.94
+ ID2= 2 (8062):	156.20	2.010	7.00	20.20

ID = 3 (8064):	418.72	7.927	6.50	23.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1781) ID= 1 DT=15.0 min	Area (ha)	Total Imp(%)	Dir. Conn. (%)
	55.50	39.00	13.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	21.65	33.86
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	608.28	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)= over (min)	82.76 15.00	64.19 30.00
Storage Coeff. (min)=	10.02 (ii)	22.80 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.04

TOTALS
 PEAK FLOW (cms)= 1.40 2.88 3.374 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.25
 RUNOFF VOLUME (mm)= 60.70 29.08 33.19
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.46 0.53

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 74.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5178)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	6.1870	0.7248
0.1880	0.3394	9.3670	0.8313
1.7260	0.4003	10.7440	0.9285
3.2000	0.4933	11.1440	0.9385
4.6710	0.6039	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1781)	55.500	3.374	6.25	33.19
OUTFLOW: ID= 1 (5178)	55.500	3.119	6.25	33.19

PEAK FLOW REDUCTION [Qout/Oin] (%) = 92.45
TIME SHIFT OF PEAK FLOW (min) = 0.00
MAXIMUM STORAGE USED (ha. m.) = 0.4997

CALIB NASHYD (1782)
ID= 1 DT=15.0 min

Area (ha) = 219.91 Curve Number (CN) = 77.6
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 0.76

Unit Hyd Opeak (cms) = 4.921
PEAK FLOW (cms) = 3.239 (i)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 25.024
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.399

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8046)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1782):	219.91	3.239	7.00	25.02
+ ID2= 2 (5178):	55.50	3.119	6.25	33.19
-----	-----	-----	-----	-----
ID = 3 (8046):	275.41	5.988	6.50	26.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0180)
ID= 1 DT=15.0 min

Area (ha) = 226.65 Curve Number (CN) = 65.0
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 1.17

Unit Hyd Opeak (cms) = 3.299
PEAK FLOW (cms) = 1.576 (i)
TIME TO PEAK (hrs) = 7.750
RUNOFF VOLUME (mm) = 16.982
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.271

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8050)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0180):	226.65	1.576	7.75	16.98
+ ID2= 2 (8046):	275.41	5.988	6.50	26.67
-----	-----	-----	-----	-----
ID = 3 (8050):	502.06	7.298	6.50	22.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1741)
ID= 1 DT=15.0 min

Area (ha) = 56.30
Total Imp(%) = 35.00 Dir. Conn. (%) = 18.00

	IMPERVIOUS (%)	PERVIOUS (i) (%)
Surface Area (ha)	19.70	36.60
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	612.64	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	82.76	23.49
over (min)	15.00	30.00
Storage Coeff. (min)	10.07 (ii)	29.16 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.04

PEAK FLOW (cms) = 1.96 1.63 2.967 (iii)
TIME TO PEAK (hrs) = 6.00 6.25 6.00
RUNOFF VOLUME (mm) = 60.70 20.06 27.37
TOTAL RAINFALL (mm) = 62.70 62.70 62.70
RUNOFF COEFFICIENT = 0.97 0.32 0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5174)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	1.1370	1.3070
0.0580	0.5427	1.4280	1.5354
0.3290	0.6196	1.6620	1.7593
0.6130	0.8627	2.0620	1.7693
0.8510	1.0621	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1741)	56.300	2.967	6.00	27.37
OUTFLOW: ID= 1 (5174)	56.300	0.624	7.25	27.33

PEAK FLOW REDUCTION [Qout/Oin] (%) = 21.03
TIME SHIFT OF PEAK FLOW (min) = 75.00
MAXIMUM STORAGE USED (ha. m.) = 0.8732

CALIB NASHYD (1742)
ID= 1 DT=15.0 min

Area (ha) = 82.80 Curve Number (CN) = 71.0
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 0.69

Unit Hyd Opeak (cms) = 2.055
PEAK FLOW (cms) = 1.057 (i)
TIME TO PEAK (hrs) = 6.750
RUNOFF VOLUME (mm) = 20.256
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.323

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8044)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1742):	82.80	1.057	6.75	20.26
+ ID2= 2 (5174):	56.30	0.624	7.25	27.33
-----	-----	-----	-----	-----
ID = 3 (8044):	139.10	1.658	7.00	23.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB NASHYD (0172) Area (ha)= 202.23 Curve Number (CN)= 78.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.89

Unit Hyd Qpeak (cms)= 3.875

PEAK FLOW (cms)= 2.677 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 25.429
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.406

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB NASHYD (0168) Area (ha)= 247.13 Curve Number (CN)= 52.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.75

Unit Hyd Qpeak (cms)= 5.611

PEAK FLOW (cms)= 1.583 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 11.218
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.179

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB NASHYD (0170) Area (ha)= 166.91 Curve Number (CN)= 65.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.80

Unit Hyd Qpeak (cms)= 3.553

PEAK FLOW (cms)= 1.557 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 16.878
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.269

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8040)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0168): 247.13 1.583 7.00 11.22
 + ID2= 2 (0170): 166.91 1.557 7.00 16.88
 =====
 ID = 3 (8040): 414.04 3.140 7.00 13.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8042)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0172): 202.23 2.677 7.00 25.43
 + ID2= 2 (8040): 414.04 3.140 7.00 13.50
 =====
 ID = 3 (8042): 616.27 5.816 7.00 17.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6174)
 IN= 2--> OUT= 1
 Routing time step (min)= 15.00
 <----- DATA FOR SECTION (1741.0) ----->

Distance	Elevation	Manning
0.00	296.00	0.0450
13.36	288.94	0.0450
26.71	288.64	0.0450
51.19	287.96	0.0450
73.45	287.32	0.0450
86.81	286.48	0.0450
102.39	285.08	0.0450
110.73	282.76	0.0450
115.73	281.06	0.0450 / 0.0350
115.74	280.35	0.0350
120.73	280.35	0.0350
120.74	281.10	0.0350 / 0.0450
125.74	282.64	0.0450
135.78	286.87	0.0450
149.13	289.58	0.0450
162.49	292.29	0.0450
175.84	293.95	0.0450
195.88	295.13	0.0450
213.68	296.02	0.0450
220.36	296.09	0.0450

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.71	281.06	.136E+05	6.0	1.69	37.89
1.50	281.85	.358E+05	22.5	2.42	26.43
2.28	282.63	.726E+05	53.8	2.85	22.47
3.07	283.42	.124E+06	104.1	3.24	19.79
3.86	284.21	.189E+06	176.7	3.60	17.81
4.64	284.99	.268E+06	274.9	3.94	16.26
5.43	285.78	.369E+06	390.8	4.07	15.73
6.21	286.56	.501E+06	551.6	4.23	15.15
7.00	287.35	.674E+06	750.5	4.28	14.97
7.79	288.14	.920E+06	993.3	4.15	15.44
8.57	288.92	.126E+07	1361.5	4.14	15.47
9.36	289.71	.167E+07	1986.0	4.56	14.04
10.15	290.50	.210E+07	2739.7	5.02	12.76
10.93	291.28	.254E+07	3615.0	5.47	11.70
11.72	292.07	.300E+07	4608.8	5.91	10.83
12.50	292.85	.347E+07	5693.3	6.30	10.16
13.29	293.64	.397E+07	6892.9	6.67	9.60
14.08	294.43	.450E+07	8150.4	6.97	9.19
14.86	295.21	.506E+07	9525.0	7.23	8.86

<---- hydrograph ----> <- pipe / channel -->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8042)	616.27	5.82	7.00	17.41	0.69	1.69
OUTFLOW: ID= 1 (6174)	616.27	5.32	8.00	17.41	0.63	1.69

 ADD HYD (8048)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (6174): 616.27 5.322 8.00 17.41
 + ID2= 2 (8044): 139.10 1.658 7.00 23.12
 =====
 ID = 3 (8048): 755.37 6.828 7.75 18.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8052)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8048): 755.37 6.828 7.75 18.46
 + ID2= 2 (8050): 502.06 7.298 6.50 22.30
 =====
 ID = 3 (8052): 1257.43 12.269 7.00 19.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8058)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)

ID1= 1 (8052): 1257.43 12.269 7.00 19.99
 + ID2= 2 (8064): 418.72 7.927 6.50 23.80

 ID = 3 (8058): 1676.15 20.133 6.75 20.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1762)
 ID= 1 DT=15.0 min

Area (ha)	= 162.00	Di r. Conn. (%)	= 11.00
Total Imp (%)	= 26.00		

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 42.12 119.88
Dep. Storage (mm)	= 2.00 5.00
Average Slope (%)	= 0.50 0.50
Length (m)	= 1039.23 40.00
Mannings n	= 0.013 0.250

Max. Eff. Inten. (mm/hr)	= 82.76 18.35
over (min)	= 15.00 45.00
Storage Coeff. (min)	= 13.82 (ii) 34.90 (ii)
Unit Hyd. Tpeak (min)	= 15.00 45.00
Unit Hyd. peak (cms)	= 0.08 0.03

TOTALS
 PEAK FLOW (cms) = 3.09 3.51 4.523 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 60.70 16.69 21.53
 TOTAL RAINFALL (mm) = 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.27 0.34

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (1761)
 ID= 1 DT=15.0 min

Area (ha)	= 48.80	Di r. Conn. (%)	= 21.00
Total Imp (%)	= 50.00		

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 24.40 24.40
Dep. Storage (mm)	= 2.00 5.00
Average Slope (%)	= 0.50 0.50
Length (m)	= 570.38 40.00
Mannings n	= 0.013 0.250

Max. Eff. Inten. (mm/hr)	= 82.76 49.51
over (min)	= 15.00 30.00
Storage Coeff. (min)	= 9.64 (ii) 23.81 (ii)
Unit Hyd. Tpeak (min)	= 15.00 30.00
Unit Hyd. peak (cms)	= 0.09 0.04

TOTALS
 PEAK FLOW (cms) = 2.01 1.56 2.986 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 60.70 20.70 29.10
 TOTAL RAINFALL (mm) = 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.33 0.46

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5176)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.2640	1.3175
0.1650	0.6418	4.0090	1.4908
0.9450	0.7489	4.7720	1.6604

1.7610	0.9426	5.1720	1.6704
2.4420	1.1144	0.0000	0.0000

INFLOW : ID= 2 (1761)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5176)	48.800	2.986	6.00	29.10
	48.800	0.959	6.75	29.08

PEAK FLOW REDUCTION [Qout/Qin] (%) = 32.11
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 0.7540

ADD HYD (8056)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1762):	162.00	4.523	6.00 21.53
+ ID2= 2 (5176):	48.80	0.959	6.75 29.08
-----	-----	-----	-----
ID = 3 (8056):	210.80	5.052	6.50 23.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8066)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8056):	210.80	5.052	6.50 23.28
+ ID2= 2 (8058):	1676.15	20.133	6.75 20.94
-----	-----	-----	-----
ID = 3 (8066):	1886.95	24.860	6.50 21.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8070)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8066):	1886.95	24.860	6.50 21.21
+ ID2= 2 (8068):	660.32	29.388	6.00 27.00
-----	-----	-----	-----
ID = 3 (8070):	2547.27	44.691	6.50 22.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6190)
 IN= 2---> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1901.0) ----->

Distance	Elevation	Manning	
0.00	261.34	0.0600	
19.76	261.64	0.0600	
59.28	261.29	0.0600	
79.04	261.09	0.0600	
103.74	255.99	0.0600	
153.13	253.42	0.0600	
192.65	252.78	0.0600	
212.29	249.80	0.0600	
217.29	249.50	0.0600 / 0.0350	Main Channel
217.30	249.05	0.0350	Main Channel
222.29	249.05	0.0350	Main Channel
222.30	249.50	0.0350 / 0.0600	Main Channel
227.30	249.80	0.0600	
232.17	252.31	0.0600	
276.63	253.53	0.0600	
326.03	256.97	0.0600	
355.67	257.30	0.0600	
400.12	260.08	0.0600	
469.28	261.25	0.0600	
489.04	262.53	0.0600	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (Cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.45	249.50	436E+04	1.0	0.43	75.97
1.07	250.12	205E+05	5.4	0.51	62.96

1.70	250.75	.451E+05	14.8	0.64	50.78
2.32	251.37	.762E+05	29.2	0.74	43.46
2.94	251.99	.114E+06	49.1	0.84	38.61
3.57	252.62	.161E+06	70.2	0.85	38.18
4.19	253.24	.248E+06	93.4	0.73	44.29
4.81	253.86	.398E+06	154.9	0.76	42.79
5.44	254.49	.576E+06	246.1	0.83	38.99
6.06	255.11	.779E+06	362.7	0.90	35.79
6.68	255.73	.101E+07	506.5	0.98	33.15
7.30	256.35	.126E+07	687.6	1.06	30.52
7.93	256.98	.153E+07	902.8	1.15	28.18
8.55	257.60	.183E+07	1114.3	1.18	27.39
9.17	258.22	.216E+07	1407.8	1.26	25.55
9.80	258.85	.250E+07	1735.4	1.35	24.03
10.42	259.47	.286E+07	2097.5	1.42	22.73
11.04	260.09	.324E+07	2492.3	1.49	21.63
11.67	260.72	.364E+07	2837.8	1.51	21.39

<---- hydrograph ---->					
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8070)	2547.27	44.69	6.50	22.71	2.80
OUTFLOW: ID= 1 (6190)	2547.27	33.74	6.75	22.71	2.47

CALIB STANDHYD (1901)			
ID= 1 DT=15.0 min	Area (ha)= 13.49	Di r. Conn.(%)= 31.00	
	Total Imp(%)= 58.00		

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)= 7.82		5.67	
Dep. Storage (mm)= 2.00		5.00	
Average Slope (%)= 0.50		0.50	
Length (m)= 299.89		40.00	
Mannings n = 0.013		0.250	
Max. Eff. Inten. (mm/hr)= 82.76		72.42	
over (min)= 15.00		30.00	
Storage Coeff. (min)= 6.56 (ii)		18.73 (ii)	
Unit Hyd. Tpeak (min)= 15.00		30.00	
Unit Hyd. peak (cms)= 0.10		0.05	
PEAK FLOW (cms)= 0.90		0.59	*TOTALS*
TIME TO PEAK (hrs)= 6.00		6.25	1.281 (iii)
RUNOFF VOLUME (mm)= 60.70		28.62	38.56
TOTAL RAINFALL (mm)= 62.70		62.70	62.70
RUNOFF COEFFICIENT = 0.97		0.46	0.62

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5190)			
IN= 2--> OUT= 1			
DT= 15.0 min			
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.9030	0.3849
0.0460	0.1975	1.1330	0.4289
0.2610	0.2331	1.3190	0.4720
0.4870	0.2836	1.7190	0.4820
0.6750	0.3308	0.0000	0.0000
AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1901)	13.490	1.281	6.00
OUTFLOW: ID= 1 (5190)	13.490	0.403	6.75

PEAK FLOW REDUCTION [Oout/Oi n] (%) = 31.45
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 0.2688

CALIB STANDHYD (1902)			
ID= 1 DT=15.0 min	Area (ha)= 274.50	Di r. Conn.(%)= 28.00	
	Total Imp(%)= 52.00		

STAGE			
Surface Area (ha)= 142.74	IMPERVIOUS	PERVIOUS (i)	
Dep. Storage (mm)= 2.00		131.76	
Average Slope (%)= 0.50		5.00	
Length (m)= 1352.77		0.50	
Mannings n = 0.013		40.00	
		0.250	
Max. Eff. Inten. (mm/hr)= 82.76		62.48	
over (min)= 15.00		30.00	
Storage Coeff. (min)= 16.19 (ii)		29.10 (ii)	
Unit Hyd. Tpeak (min)= 15.00		30.00	
Unit Hyd. peak (cms)= 0.07		0.04	
PEAK FLOW (cms)= 12.45		9.64	*TOTALS*
TIME TO PEAK (hrs)= 6.00		6.25	18.530 (iii)
RUNOFF VOLUME (mm)= 60.70		27.06	6.00
TOTAL RAINFALL (mm)= 62.70		62.70	36.48
RUNOFF COEFFICIENT = 0.97		0.43	62.70
			0.58

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9190)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1902):	274.50	18.530	6.00	36.48
+ ID2= 2 (5190):	13.49	0.403	6.75	38.48
ID = 3 (9190):	287.99	18.566	6.00	36.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8072)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6190):	2547.27	33.743	6.75	22.71
+ ID2= 2 (9190):	287.99	18.566	6.00	36.58
ID = 3 (8072):	2835.26	43.221	6.50	24.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6192)		Routing time step (min)' = 15.00
IN= 2--> OUT= 1		

<----- DATA FOR SECTION (1921.0) ----->			
Distance	Elevation	Manning	
0.00	264.40	0.0500	
3.64	263.94	0.0500	
14.54	261.05	0.0500	
29.08	256.78	0.0500	
36.36	254.96	0.0500	
50.90	252.20	0.0500	
65.44	249.78	0.0500	
76.35	246.86	0.0500	
77.24	245.90	0.0500	
82.24	245.45	0.0500 / 0.0350	Main Channel
82.25	244.90	0.0350	Main Channel
87.25	244.90	0.0350	Main Channel
87.26	245.45	0.0350 / 0.0500	Main Channel
94.53	246.89	0.0500	
170.87	247.16	0.0500	
218.13	248.69	0.0500	
265.40	249.22	0.0500	
301.75	250.78	0.0500	
338.11	253.47	0.0500	
359.92	264.00	0.0500	

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)

0.55	245.45	.345E+04	2.0	0.71	29.20
1.53	246.43	.174E+05	14.6	1.05	19.92
2.50	247.40	.767E+05	52.8	0.86	24.20
3.48	248.38	.224E+06	199.8	1.11	18.71
4.46	249.36	.429E+06	441.2	1.29	16.19
5.43	250.33	.691E+06	874.3	1.58	13.18
6.41	251.31	.987E+06	1468.3	1.86	11.20
7.38	252.28	1.31E+07	2224.8	2.13	9.79
8.36	253.26	1.65E+07	3132.4	2.37	8.78
9.34	254.24	2.01E+07	4256.4	2.65	7.87
10.31	255.21	2.38E+07	5551.0	2.92	7.15
11.29	256.19	2.76E+07	6996.8	3.17	6.57
12.27	257.17	3.14E+07	8585.9	3.42	6.10
13.24	258.14	3.53E+07	10315.9	3.65	5.71
14.22	259.12	3.93E+07	12180.7	3.87	5.38
15.19	260.09	4.34E+07	14178.2	4.09	5.10
16.17	261.07	4.75E+07	16306.5	4.29	4.85
17.15	262.05	5.17E+07	18558.6	4.49	4.64
18.12	263.02	5.59E+07	20939.7	4.68	4.45

<---- hydrograph ---->						
	AREA	OPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8072)	2835.26	43.22	6.50	24.12	2.26	0.90
OUTFLOW: ID= 1 (6192)	2835.26	38.42	7.00	24.12	2.14	0.92

CALIB STANDHYD (1921)			
ID= 1 DT=15.0 min	Area (ha)= 72.27	Imp(%)= 42.00	Dir. Conn. (%)= 19.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)=	30.35	41.92	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	694.12	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	82.76	22.90	
over (min)	15.00	45.00	
Storage Coeff. (min)=	10.85 (ii)	30.14 (ii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.08	0.03	
PEAK FLOW (cms)=	2.60	1.65	*TOTALS* 3.282 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	60.70	17.85	25.99
TOTAL RAINFALL (mm)=	62.70	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.28	0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 57.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5192)			
IN= 2--> OUT= 1			
DT= 15.0 min			
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.1390	1.4346
0.2060	0.7072	3.8730	1.6637
0.9550	0.8170	4.8310	1.9184
1.7940	1.0081	5.2310	1.9284
2.4250	1.1871	0.0000	0.0000

	AREA	OPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (1921)	72.270	3.282	6.00	25.99
OUTFLOW: ID= 1 (5192)	72.270	1.272	7.00	25.98

PEAK FLOW REDUCTION [Qout/Qin] (%) = 38.76
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 0.8925

CALIB STANDHYD (1922)			
ID= 1 DT=15.0 min	Area (ha)= 174.34	Imp(%)= 39.00	Dir. Conn. (%)= 18.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)=	67.99	106.35	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	1078.08	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	82.76	21.32	
over (min)	15.00	45.00	
Storage Coeff. (min)=	14.13 (ii)	33.98 (ii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.07	0.03	

TOTALS			
PEAK FLOW (cms)=	5.39	3.67	6.901 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	60.70	17.31	25.12
TOTAL RAINFALL (mm)=	62.70	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.28	0.40

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 57.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8074)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1922):	174.34	6.901	6.00	25.12
+ ID2= 2 (5192):	72.27	1.272	7.00	25.98
ID = 3 (8074):	246.61	7.022	6.00	25.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8076)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6192):	2835.26	38.423	7.00	24.12
+ ID2= 2 (8074):	246.61	7.022	6.00	25.37
ID = 3 (8076):	3081.87	42.737	6.75	24.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0196)			
ID= 1 DT=15.0 min	Area (ha)= 41.47	Curve Number (CN)= 70.0	Ia (mm)= 5.00
	U.H. Tp(hrs)= 0.24	# of Linear Res. (N)= 1.50	

Unit Hyd Opeak (cms)=	2.950
PEAK FLOW (cms)=	1.088 (i)
TIME TO PEAK (hrs)=	6.000
RUNOFF VOLUME (mm)=	18.348
TOTAL RAINFALL (mm)=	62.700
RUNOFF COEFFICIENT =	0.293

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1941)			
ID= 1 DT=15.0 min	Area (ha)= 0.76	Imp(%)= 45.00	Dir. Conn. (%)= 15.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 0.34 0.42
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 71.18 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 82.76 60.60
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 2.77 (ii) 15.84 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.11 0.05

TOTALS
 PEAK FLOW (cms)= 0.03 0.04 0.051 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 60.70 25.58 30.83
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.41 0.49

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 67.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5194)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0510	0.0197
0.0030	0.0093	0.0640	0.0226
0.0150	0.0107	0.0740	0.0253
0.0270	0.0138	0.0740	0.0353
0.0380	0.0165	0.0000	0.0000

INFLOW : ID= 2 (1941)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5194)	0.760	0.051	6.00	30.83
	0.760	0.019	6.75	29.82

PEAK FLOW REDUCTION [Qout/Qin] (%) = 37.31
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0119

CALIB NASHYD (1942)
 ID= 1 DT=15.0 min

Area (ha)= 255.33 Curve Number (CN)= 69.5
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.68

Unit Hyd Opeak (cms)= 6.448
 PEAK FLOW (cms)= 3.145 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 19.322
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.308

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9194)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1942):	255.33	3.145	6.75	19.32
+ ID2= 2 (5194):	0.76	0.019	6.75	29.82
=====				
ID = 3 (9194):	256.09	3.164	6.75	19.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8078)
 1 + 2 = 3

AREA OPEAK TPEAK R. V.

(ha) (cms) (hrs) (mm)
 ID1= 1 (0196): 41.47 1.088 6.00 18.35
 + ID2= 2 (9194): 256.09 3.164 6.75 19.35
 ID = 3 (8078): 297.56 4.027 6.50 19.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6198)
 IN= 2---> OUT= 1

Routing time step (min)= 15.00

<----- DATA FOR SECTION (1981.0) ----->

Distance	Elevation	Manning
0.00	267.15	0.0500
22.99	265.94	0.0500
51.73	261.39	0.0500
74.72	258.75	0.0500
94.83	257.79	0.0500
114.95	254.36	0.0500
126.44	254.06	0.0500
132.19	253.68	0.0500
135.06	253.35	0.0500
137.94	252.93	0.0500 / 0.0300
140.81	252.41	0.0300
143.69	251.89	0.0300
146.56	252.51	0.0300
149.43	253.36	0.0300 / 0.0500
158.05	255.89	0.0500
172.42	256.25	0.0500
204.03	259.15	0.0500
229.90	260.36	0.0500
252.89	260.46	0.0500
284.50	260.45	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.35	252.24	.111E+04	0.6	0.97	31.07
0.69	252.58	.442E+04	3.8	1.55	19.42
1.04	252.93	.974E+04	11.1	2.06	14.65
1.51	253.40	.203E+05	31.2	2.77	10.87
1.98	253.87	.357E+05	65.0	3.30	9.14
2.45	254.34	.595E+05	112.9	3.43	8.80
2.92	254.81	.935E+05	186.8	3.62	8.34
3.39	255.28	.131E+06	282.1	3.89	7.75
3.86	255.75	.172E+06	398.2	4.18	7.22
4.33	256.22	.221E+06	527.0	4.31	6.99
4.80	256.69	.282E+06	697.3	4.47	6.75
5.27	257.16	.350E+06	898.5	4.64	6.50
5.74	257.63	.425E+06	1131.4	4.82	6.26
6.21	258.10	.508E+06	1367.4	4.88	6.19
6.68	258.57	.603E+06	1635.6	4.91	6.14
7.15	259.04	.709E+06	1977.2	5.04	5.98
7.62	259.51	.826E+06	2360.5	5.17	5.83
8.09	259.98	.954E+06	2790.7	5.29	5.70
8.56	260.45	.110E+07	3179.6	5.25	5.74

<---- hydrograph ----> <-pi pe / channel ->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8078) 297.56 4.03 6.50 19.21 0.70 1.57
 OUTFLOW: ID= 1 (6198) 297.56 3.88 6.75 19.21 0.70 1.56

CALIB STANDBYD (1981)
 ID= 1 DT=15.0 min

Area (ha)= 149.95
 Total Imp (%) = 44.00 Di r. Conn. (%) = 19.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 65.98 83.97
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 999.83 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 82.76 24.74
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 13.50 (ii) 32.21 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03

PEAK FLOW (cms)=	4.98	3.45	6.416	(iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00	
RUNOFF VOLUME (mm)=	60.70	18.57	26.57	
TOTAL RAINFALL (mm)=	62.70	62.70	62.70	
RUNOFF COEFFICIENT =	0.97	0.30	0.42	

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 57.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5198) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	10.9910	3.4957
	0.4390	1.7800	13.3530	3.9512
	2.6180	2.0449	15.3540	4.3979
	5.1680	2.5932	15.7540	4.4079
	7.9230	3.0040	0.0000	0.0000

INFLOW : ID= 2 (1981)	149.950	6.416	6.00	26.57
OUTFLOW: ID= 1 (5198)	149.950	2.581	7.00	26.56

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 40.24
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 2.0427

CALIB STANDHYD (1982) ID= 1 DT=15.0 min	Area (ha) = 240.61 Total Imp (%) = 20.00	Dir. Conn. (%) = 9.00
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Surface Area (ha)=	48.12	IMPERVIOUS	PERVIOUS (i)	192.49
Dep. Storage (mm)=	2.00			5.00
Average Slope (%)=	0.50			0.50
Length (m)=	1266.52			40.00
Mannings n =	0.013			0.250
Max. Eff. Inten. (mm/hr)=	82.76			15.71
over (min)	15.00			45.00
Storage Coeff. (min)=	15.56	(ii)		37.99 (ii)
Unit Hyd. Tpeak (min)=	15.00			45.00
Unit Hyd. peak (cms)=	0.07			0.03

PEAK FLOW (cms)=	3.57	4.63	5.725	(iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.50	
RUNOFF VOLUME (mm)=	60.70	15.22	19.31	
TOTAL RAINFALL (mm)=	62.70	62.70	62.70	
RUNOFF COEFFICIENT =	0.97	0.24	0.31	

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 57.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8080) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1982):	240.61	5.725	6.50	19.31
+ ID2= 2 (5198):	149.95	2.581	7.00	26.56
ID = 3 (8080):	390.56	6.499	6.75	22.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8082) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6198):	297.56	3.878	6.75	19.21
+ ID2= 2 (8080):	390.56	6.499	6.75	22.10
ID = 3 (8082):	688.12	10.377	6.75	20.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8084) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8076):	3081.87	42.737	6.75	24.22
+ ID2= 2 (8082):	688.12	10.377	6.75	20.85
ID = 3 (8084):	3769.99	53.114	6.75	23.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6200) IN= 2--> OUT= 1	Routing time step (min)' = 15.00
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<----- DATA FOR SECTION (2001.0) ----->

Distance	Elevation	Manning		
0.00	252.47	0.0500		
10.38	252.29	0.0500		
20.76	252.28	0.0500		
41.52	247.66	0.0500		
62.28	247.47	0.0500		
80.45	247.34	0.0500		
90.83	246.85	0.0500		
103.81	245.51	0.0500		
106.00	244.50	0.0500		
108.99	244.05	0.0500 / 0.0300	0.300	Main Channel
114.00	243.50	0.0300	0.300	Main Channel
114.01	244.05	0.0300 / 0.0500	0.0500	Main Channel
118.99	244.50	0.0500		
132.36	245.09	0.0500		
150.52	245.83	0.0500		
199.83	249.24	0.0500		
207.62	250.16	0.0500		
236.16	253.11	0.0500		
256.92	253.83	0.0500		

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.28	243.77	.365E+04	0.7	0.54	81.42
0.55	244.05	.730E+04	2.2	0.81	54.59
1.05	244.55	.197E+05	7.2	0.97	45.51
1.54	245.04	.463E+05	17.2	0.99	44.76
2.04	245.54	.895E+05	34.8	1.03	42.81
2.53	246.03	.152E+06	62.8	1.09	40.43
3.03	246.53	.232E+06	104.7	1.20	36.91
3.52	247.02	.328E+06	159.7	1.29	34.19
4.02	247.52	.450E+06	223.4	1.32	33.56
4.51	248.01	.627E+06	326.9	1.38	31.99
5.01	248.51	.820E+06	463.0	1.50	29.52
5.50	249.00	.103E+07	626.0	1.62	27.30
6.00	249.50	.124E+07	820.1	1.75	25.24
6.49	249.99	.147E+07	1044.4	1.89	23.42
6.99	250.49	.170E+07	1292.6	2.01	21.95
7.48	250.98	.195E+07	1565.6	2.13	20.71
7.98	251.48	.220E+07	1864.5	2.25	19.65
8.47	251.97	.246E+07	2189.4	2.36	18.73
8.97	252.47	.274E+07	2427.5	2.35	18.80

INFLOW : ID= 2 (8084)	3769.99	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW: ID= 1 (6200)	3769.99	3769.99	44.46	7.50	23.60	2.21	1.05

CALIB
STANDHYD (2001)
ID= 1 DT=15.0 min

Area (ha)	= 42.48	Dir. Conn. (%)	= 19.00
Total Imp (%)	= 46.00		

Surface Area (ha)	= 19.54	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)	= 2.00		
Average Slope (%)	= 0.50		
Length (m)	= 532.17		
Mannings n	= 0.013		

Max. Eff. Inten. (mm/hr)	= 82.76	62.83
over (min)	15.00	30.00
Storage Coeff. (min)	= 9.25 (ii)	22.13 (ii)
Unit Hyd. Tpeak (min)	= 15.00	30.00
Unit Hyd. peak (cms)	= 0.09	0.04

PEAK FLOW (cms)	= 1.60	1.94	2.838 (iii)
TIME TO PEAK (hrs)	= 6.00	6.25	6.00
RUNOFF VOLUME (mm)	= 60.70	27.21	33.57
TOTAL RAINFALL (mm)	= 62.70	62.70	62.70
RUNOFF COEFFICIENT	= 0.97	0.43	0.54

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5200)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.7210	1.0525
0.1450	0.4892	3.4130	1.1947
0.7930	0.5722	4.0030	1.3346
1.4410	0.7540	4.4030	1.3446
2.0370	0.8927	0.0000	0.0000

INFLOW: ID= 2 (2001)	42.480	2.838	6.00	33.57
OUTFLOW: ID= 1 (5200)	42.480	1.147	6.75	33.55

PEAK FLOW REDUCTION [Qout/Qin] (%) = 40.40
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha.m.) = 0.6746

CALIB
STANDHYD (2002)
ID= 1 DT=15.0 min

Area (ha)	= 72.79	Dir. Conn. (%)	= 13.00
Total Imp (%)	= 31.00		

Surface Area (ha)	= 22.56	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)	= 2.00		
Average Slope (%)	= 0.50		
Length (m)	= 696.61		
Mannings n	= 0.013		

Max. Eff. Inten. (mm/hr)	= 82.76	46.98
over (min)	15.00	30.00
Storage Coeff. (min)	= 10.87 (ii)	25.34 (ii)
Unit Hyd. Tpeak (min)	= 15.00	30.00
Unit Hyd. peak (cms)	= 0.08	0.04

PEAK FLOW (cms)	= 1.79	2.96	3.641 (iii)
TIME TO PEAK (hrs)	= 6.00	6.25	6.00
RUNOFF VOLUME (mm)	= 60.70	24.26	28.99
TOTAL RAINFALL (mm)	= 62.70	62.70	62.70
RUNOFF COEFFICIENT	= 0.97	0.39	0.46

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%

YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8086)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2002):	72.79	3.641	6.00	28.99
+ ID2= 2 (5200):	42.48	1.147	6.75	33.55
ID= 3 (8086):	115.27	4.161	6.25	30.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8088)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6200):	3769.99	44.464	7.50	23.60
+ ID2= 2 (8086):	115.27	4.161	6.25	30.67
ID= 3 (8088):	3885.26	46.028	7.50	23.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8038)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8034):	5220.76	15.884	7.25	14.61
+ ID2= 2 (8088):	3885.26	46.028	7.50	23.81
ID= 3 (8038):	9106.02	61.724	7.50	18.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (1365)
IN= 2---> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (1362.0) ----->

Distance	Elevation	Manning
0.00	252.00	0.0400
2.35	244.60	0.0400
39.94	243.14	0.0400
58.74	242.65	0.0400
91.63	242.03	0.0400
101.03	241.69	0.0400
103.38	241.66	0.0400
105.73	240.76	0.0400
105.93	240.59	0.0400
109.18	240.28	0.0400 / 0.0300
110.43	239.81	0.0300
111.68	239.81	0.0300
113.43	240.09	0.0300 / 0.0400
118.43	240.50	0.0400
138.62	241.68	0.0400
152.72	241.60	0.0400
166.82	242.68	0.0400
185.62	249.09	0.0400
197.36	251.00	0.0400
232.61	252.28	0.0400

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.28	240.09	.118E+04	0.1	0.20	142.09
0.91	240.72	.114E+05	2.4	0.35	79.92
1.53	241.34	.352E+05	9.4	0.45	62.17
2.16	241.97	.831E+05	22.8	0.46	60.69
2.79	242.60	.172E+06	54.2	0.53	52.74
3.41	243.22	.297E+06	111.4	0.63	44.49

4.04	243.85	.445E+06	194.1	0.74	38.22
4.67	244.48	.612E+06	301.1	0.83	33.88
5.29	245.10	.793E+06	443.0	0.94	29.82
5.92	245.73	.976E+06	611.3	1.06	26.60
6.55	246.36	.116E+07	802.0	1.17	24.12
7.18	246.99	.135E+07	1014.0	1.27	22.16
7.80	247.61	.154E+07	1246.2	1.37	20.56
8.43	248.24	.173E+07	1498.0	1.46	19.24
9.06	248.87	.192E+07	1768.8	1.55	18.12
9.68	249.49	.212E+07	2049.6	1.63	17.24
10.31	250.12	.232E+07	2344.7	1.71	16.49
10.94	250.75	.252E+07	2658.5	1.78	15.83
11.56	251.37	.274E+07	2931.2	1.81	15.56

		<---- hydrograph ---->		<- pipe / channel ->			
	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL	
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW :	ID= 2 (8038)	9106.02	61.72	7.50	18.54	2.87	0.55
OUTFLOW :	ID= 1 (1365)	9106.02	51.32	8.25	18.54	2.73	0.53

CALIB STANDHYD (1321)		Area (ha)= 208.90	Dir. Conn. (%)= 46.00
ID= 1 DT=15.0 min		Total Imp (%)= 67.00	

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	139.96	68.94
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	1180.11	40.00
Mannings n	=	0.013	0.250

Max. Eff. Inten. (mm/hr)=	82.76	60.78
over (min)	15.00	30.00
Storage Coeff. (min)=	14.92 (ii)	27.97 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.07	0.04

PEAK FLOW	(cms)=	16.13	5.00	*TOTALS*
TIME TO PEAK	(hrs)=	6.00	6.25	19.270 (iii)
RUNOFF VOLUME	(mm)=	60.70	24.33	41.06
TOTAL RAINFALL	(mm)=	62.70	62.70	62.70
RUNOFF COEFFICIENT	=	0.97	0.39	0.65

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5132)		AREA	OPEAK	TPEAK	R. V.
IN= 2----> OUT= 1		(ha)	(cms)	(hrs)	(mm)
DT= 15.0 min					
	OUTFLOW	STORAGE	OUTFLOW	STORAGE	
	(cms)	(ha.m.)	(cms)	(ha.m.)	
	0.0000	0.0000	7.9970	7.9437	
	0.9210	5.2645	12.4650	8.5839	
	3.2690	5.3433	16.9060	9.2236	
	6.3980	5.6925	17.3060	9.2336	
	7.1860	6.8155	0.0000	0.0000	
INFLOW :	ID= 2 (1321)	208.900	19.270	6.00	41.06
OUTFLOW :	ID= 1 (5132)	208.900	4.081	7.00	41.05

PEAK FLOW REDUCTION [Qout/Qin] (%) = 21.18
TIME SHIFT OF PEAK FLOW (min) = 60.00
MAXIMUM STORAGE USED (ha.m.) = 5.4449

CALIB STANDHYD (1322)		Area (ha)= 81.94	Dir. Conn. (%)= 25.00
ID= 1 DT=15.0 min		Total Imp (%)= 37.00	

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	30.32	51.62
Dep. Storage	(mm)=	2.00	5.00

Average Slope	(%)=	0.50	0.50
Length	(m)=	739.10	40.00
Mannings n	=	0.013	0.250

Max. Eff. Inten. (mm/hr)=	82.76	21.24
over (min)	15.00	45.00
Storage Coeff. (min)=	11.27 (ii)	31.15 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03

PEAK FLOW	(cms)=	3.83	1.85	*TOTALS*	
TIME TO PEAK	(hrs)=	6.00	6.50	4.593 (iii)	
RUNOFF VOLUME	(mm)=	60.70	19.28	6.00	
TOTAL RAINFALL	(mm)=	62.70	62.70	29.64	
RUNOFF COEFFICIENT	=	0.97	0.31	62.70	0.47

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8090)		AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1322):		81.94	4.593	6.00	29.64
+ ID2= 2 (5132):		208.90	4.081	7.00	41.05
ID 3 (8090):		290.84	5.530	7.00	37.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1342)		Area (ha)= 97.16	Dir. Conn. (%)= 28.00
ID= 1 DT=15.0 min		Total Imp (%)= 41.00	

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	39.84	57.32
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	804.82	40.00
Mannings n	=	0.013	0.250

Max. Eff. Inten. (mm/hr)=	82.76	16.44
over (min)	15.00	45.00
Storage Coeff. (min)=	11.86 (ii)	33.88 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03

PEAK FLOW	(cms)=	4.99	1.53	*TOTALS*	
TIME TO PEAK	(hrs)=	6.00	6.50	5.617 (iii)	
RUNOFF VOLUME	(mm)=	60.70	14.87	6.00	
TOTAL RAINFALL	(mm)=	62.70	62.70	27.70	
RUNOFF COEFFICIENT	=	0.97	0.24	62.70	0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 54.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1341)		Area (ha)= 58.79	Dir. Conn. (%)= 20.00
ID= 1 DT=15.0 min		Total Imp (%)= 30.00	

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	17.64	41.15
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	626.05	40.00
Mannings n	=	0.013	0.250

Max. Eff. Inten. (mm/hr)=	82.76	14.47
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over (min) 15.00 45.00
 Storage Coeff. (min)= 10.20 (ii) 33.38 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.09 0.03

TOTALS
 PEAK FLOW (cms)= 2.27 0.98 2.664 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 60.70 14.03 23.37
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.22 0.37

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 54.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5134)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.1250	1.0755
0.1070	0.6119	1.7570	1.1641
0.4130	0.6225	2.3860	1.2526
0.8380	0.7018	2.7860	1.2626
0.9790	0.8884	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1341)	58.790	2.664	6.00	23.37
OUTFLOW: ID= 1 (5134)	58.790	0.832	7.00	23.34

PEAK FLOW REDUCTION [Qout/Qin] (%) = 31.23
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 0.7019

ADD HYD (9134)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1342):	97.16	5.617	6.00	27.70
+ ID2= 2 (5134):	58.79	0.832	7.00	23.34
ID = 3 (9134):	155.95	5.678	6.00	26.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8092)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8090):	290.84	5.530	7.00	37.84
+ ID2= 2 (9134):	155.95	5.678	6.00	26.06
ID = 3 (8092):	446.79	10.732	6.00	33.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (1366)
 IN= 2--> OUT= 1

Routing time step (min) = 15.00

----- DATA FOR SECTION (1361.0) ----->

Distance	Elevation	Manning
0.00	262.81	0.0400
37.23	260.54	0.0400
79.12	256.98	0.0400
111.70	254.74	0.0400
172.20	252.76	0.0400
251.32	248.82	0.0400
255.97	248.75	0.0400
260.43	248.76	0.0400
265.43	248.49	0.0400

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.47	248.18	.881E+04	1.3	0.76	114.66
1.19	248.90	.742E+05	17.3	1.22	71.44
1.92	249.63	.278E+06	78.3	1.48	59.26
2.64	250.35	.589E+06	210.3	1.87	46.69
3.36	251.07	.974E+06	414.3	2.23	39.19
4.08	251.79	.143E+07	696.2	2.55	34.32
4.81	252.52	.197E+07	1063.0	2.83	30.86
5.53	253.24	.259E+07	1505.7	3.06	28.62
6.25	253.96	.331E+07	2051.3	3.26	26.85
6.97	254.68	.413E+07	2720.9	3.46	25.30
7.70	255.41	.504E+07	3584.5	3.73	23.43
8.42	256.13	.601E+07	4580.9	4.00	21.87
9.14	256.85	.704E+07	5707.9	4.25	20.57
9.86	257.57	.818E+07	6636.1	4.26	20.53
10.59	258.30	.945E+07	8115.0	4.50	19.42
11.31	259.02	.108E+08	9759.2	4.74	18.45
12.03	259.74	.122E+08	11569.5	4.96	17.61
12.75	260.46	.137E+08	13550.6	5.18	16.87
13.48	261.19	.153E+08	15636.4	5.37	16.29

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.47	248.18	.881E+04	1.3	0.76	114.66
1.19	248.90	.742E+05	17.3	1.22	71.44
1.92	249.63	.278E+06	78.3	1.48	59.26
2.64	250.35	.589E+06	210.3	1.87	46.69
3.36	251.07	.974E+06	414.3	2.23	39.19
4.08	251.79	.143E+07	696.2	2.55	34.32
4.81	252.52	.197E+07	1063.0	2.83	30.86
5.53	253.24	.259E+07	1505.7	3.06	28.62
6.25	253.96	.331E+07	2051.3	3.26	26.85
6.97	254.68	.413E+07	2720.9	3.46	25.30
7.70	255.41	.504E+07	3584.5	3.73	23.43
8.42	256.13	.601E+07	4580.9	4.00	21.87
9.14	256.85	.704E+07	5707.9	4.25	20.57
9.86	257.57	.818E+07	6636.1	4.26	20.53
10.59	258.30	.945E+07	8115.0	4.50	19.42
11.31	259.02	.108E+08	9759.2	4.74	18.45
12.03	259.74	.122E+08	11569.5	4.96	17.61
12.75	260.46	.137E+08	13550.6	5.18	16.87
13.48	261.19	.153E+08	15636.4	5.37	16.29

----- hydrograph ----->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8092)	446.79	10.73	6.00	33.73	0.90	0.98
OUTFLOW: ID= 1 (1366)	446.79	4.89	7.50	33.73	0.63	0.83

CALIB STANDHYD (1362)
 ID= 1 DT=15.0 min

Area (ha) = 371.79
 Total Imp (%) = 26.00
 Dir. Conn. (%) = 14.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 96.67 275.12
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 1574.36 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 82.76 20.83
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 17.73 (ii) 37.77 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.06 0.03

TOTALS
 PEAK FLOW (cms) = 8.08 8.76 11.686 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 60.70 19.37 25.15
 TOTAL RAINFALL (mm) = 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.31 0.40

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1361)
 ID= 1 DT=15.0 min

Area (ha) = 140.62
 Total Imp (%) = 55.00
 Dir. Conn. (%) = 29.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 77.34 63.28

Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 968.23 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 82.76 58.41
 over (min) 15.00 30.00
 Storage Coeff. (min)= 13.25 (ii) 26.51 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

TOTALS
 PEAK FLOW (cms)= 7.18 4.54 10.034 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 60.70 24.24 34.81
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.39 0.56

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5136)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.7060	4.3486
0.3100	2.4365	5.9200	5.0324
1.7670	2.5993	7.0820	5.7151
3.4020	2.8510	7.4820	5.7251
4.0310	3.5962	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1361)	140.620	10.034	6.00	34.81
OUTFLOW: ID= 1 (5136)	140.620	2.935	7.00	34.80

PEAK FLOW REDUCTION [Qout/Qin] (%) = 29.25
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 2.8002

ADD HYD (8096)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1362):	371.79	11.686	6.00	25.15
+ ID2= 2 (5136):	140.62	2.935	7.00	34.80
-----	-----	-----	-----	-----
ID = 3 (8096):	512.41	13.253	6.50	27.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8094)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1366):	446.79	4.895	7.50	33.73
+ ID2= 2 (8096):	512.41	13.253	6.50	27.80
-----	-----	-----	-----	-----
ID = 3 (8094):	959.20	16.512	6.50	30.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8098)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1365):	9106.02	51.320	8.25	18.54
+ ID2= 2 (8094):	959.20	16.512	6.50	30.56
-----	-----	-----	-----	-----
ID = 3 (8098):	10065.22	60.285	8.00	19.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANHYD (1381)
 ID= 1 DT=15.0 min

Area (ha)= 96.24
 Total Imp(%)= 59.00 Dir. Conn.(%)= 34.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	56.78	39.46
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	801.00	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 82.76 10.83
 over (min) 15.00 45.00
 Storage Coeff. (min)= 11.82 (ii) 37.85 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03

TOTALS
 PEAK FLOW (cms)= 6.01 0.66 6.278 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 60.70 7.67 25.70
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.12 0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 28.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5138)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	6.4380	2.7648
0.3260	1.4281	8.0830	3.0750
1.8650	1.6876	9.4100	3.3790
3.4730	2.0447	9.8100	3.3890
4.8170	2.3809	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1381)	96.240	6.278	6.00	25.70
OUTFLOW: ID= 1 (5138)	96.240	1.013	7.00	25.69

PEAK FLOW REDUCTION [Qout/Qin] (%) = 16.14
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 1.5452

CALIB NASHYD (1382)
 ID= 1 DT=15.0 min

Area (ha)= 318.55 Curve Number (CN)= 33.2
 Ia (mm)= 5.00 # of Li near Res. (N)= 1.50
 U. H. Tp(hrs)= 1.09

Unit Hyd Opeak (cms)= 4.976
 PEAK FLOW (cms)= 0.778 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 5.801
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.093

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9138)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1382):	318.55	0.778	7.75	5.80
+ ID2= 2 (5138):	96.24	1.013	7.00	25.69
-----	-----	-----	-----	-----
ID = 3 (9138):	414.79	1.761	7.00	10.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (1402)
ID= 1 DT=15.0 min

Area (ha)=	131.22	Dir. Conn. (%)=	21.00
Total Imp (%)=	29.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	38.05	93.17
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	935.31	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	82.76	7.60
over (min)	15.00	45.00
Storage Coeff. (min)=	12.97 (ii)	42.96 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03

TOTALS
PEAK FLOW (cms)= 4.89 1.02 5.294 (iii)
TIME TO PEAK (hrs)= 6.00 6.50 6.00
RUNOFF VOLUME (mm)= 60.70 7.81 18.92
TOTAL RAINFALL (mm)= 62.70 62.70 62.70
RUNOFF COEFFICIENT = 0.97 0.12 0.30

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 37.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (1401)
ID= 1 DT=15.0 min

Area (ha)=	27.37	Dir. Conn. (%)=	33.00
Total Imp (%)=	45.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	12.32	15.05
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	427.16	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	82.76	9.15
over (min)	15.00	45.00
Storage Coeff. (min)=	8.11 (ii)	35.95 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.10	0.03

TOTALS
PEAK FLOW (cms)= 1.85 0.22 1.942 (iii)
TIME TO PEAK (hrs)= 6.00 6.50 6.00
RUNOFF VOLUME (mm)= 60.70 8.55 25.76
TOTAL RAINFALL (mm)= 62.70 62.70 62.70
RUNOFF COEFFICIENT = 0.97 0.14 0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 37.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5140)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.8310	0.7127
0.0930	0.3344	2.2990	0.8148
0.5300	0.3869	2.6760	0.9148
0.9880	0.4994	3.0760	0.9248
1.3700	0.5962	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1401)	27.370	1.942	6.00	25.76
OUTFLOW: ID= 1 (5140)	27.370	0.451	6.50	25.72

PEAK FLOW REDUCTION [Qout/Qin] (%) = 23.22
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha.m.) = 0.3814

ADD HYD (9140)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1402):	131.22	5.294	6.00	18.92
+ ID2= 2 (5140):	27.37	0.451	6.50	25.72
ID = 3 (9140):	158.59	5.364	6.00	20.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8102)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9138):	414.79	1.761	7.00	10.41
+ ID2= 2 (9140):	158.59	5.364	6.00	20.09
ID = 3 (8102):	573.38	5.963	6.00	13.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6142)
IN= 2--> OUT= 1
Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1421.0) ----->

Distance	Elevation	Manning
0.00	295.50	0.0400
7.25	295.36	0.0400
14.50	295.22	0.0400
21.75	295.22	0.0400
29.00	294.23	0.0400
52.56	286.45	0.0400
67.06	281.44	0.0400
77.94	278.28	0.0400
82.81	277.20	0.0400 / 0.0300
83.21	276.80	0.0300
88.81	276.80	0.0300
96.41	276.80	0.0300
96.81	277.20	0.0300 / 0.0400
103.31	278.96	0.0400
108.75	280.16	0.0400
117.81	282.73	0.0400
135.94	290.30	0.0400
159.50	292.14	0.0400
174.00	293.07	0.0400
179.44	293.37	0.0400

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.40	277.20	.238E+05	6.9	1.27	57.28
1.25	278.05	.889E+05	50.9	2.50	29.13
2.10	278.90	.179E+06	133.4	3.26	22.36
2.95	279.75	.293E+06	256.7	3.83	19.03
3.80	280.60	.432E+06	426.0	4.31	16.90
4.66	281.46	.593E+06	644.3	4.75	15.35
5.51	282.31	.776E+06	915.3	5.16	14.12
6.36	283.16	.978E+06	1242.2	5.55	13.12
7.21	284.01	.120E+07	1626.2	5.94	12.27
8.06	284.86	.143E+07	2065.6	6.30	11.57
8.91	285.71	.169E+07	2562.4	6.64	10.97
9.76	286.56	.196E+07	3118.0	6.97	10.46
10.61	287.41	.224E+07	3733.3	7.28	10.02
11.46	288.26	.255E+07	4411.9	7.57	9.62
12.31	289.11	.287E+07	5155.6	7.86	9.27
13.17	289.97	.321E+07	5966.6	8.14	8.96
14.02	290.82	.357E+07	6705.0	8.22	8.87
14.87	291.67	.398E+07	7494.3	8.24	8.85
15.72	292.52	.444E+07	8405.3	8.28	8.80

<----- hydrograph -----> <-pipe / channel ->

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
-----------	-------------	-------------	------------	---------------	---------------

INFLOW : ID= 2 (8102) 573.38 5.96 6.00 13.09 0.34 1.27
 OUTFLOW: ID= 1 (6142) 573.38 3.05 7.25 13.09 0.18 1.27

CALIB STANHYD (1421)
 ID= 1 DT=15.0 min Area (ha)= 69.77
 Total Imp(%)= 45.00 Dir. Conn.(%)= 15.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	31.40	38.37
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	682.01	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	82.76	47.61
over (min)	15.00	30.00
Storage Coeff. (min)=	10.73 (ii)	25.13 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.04

TOTALS
 PEAK FLOW (cms)= 1.99 2.30 3.417 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 60.70 20.36 26.41
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.32 0.42

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.6670	1.8169
0.2360	0.8525	5.8600	2.0773
1.3520	0.9863	6.8220	2.3321
2.5180	1.2732	7.2220	2.3421
3.4920	1.5199	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1421)	69.770	3.417	6.00	26.41
OUTFLOW: ID= 1 (5142)	69.770	1.206	7.00	26.40

PEAK FLOW REDUCTION [Qout/Qin] (%) = 35.29
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 0.9769

CALIB NASHYD (1422)
 ID= 1 DT=15.0 min Area (ha)= 326.77 Curve Number (CN)= 61.2
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.21

Unit Hyd Opeak (cms)=	4.626
PEAK FLOW (cms)=	1.973 (i)
TIME TO PEAK (hrs)=	8.000
RUNOFF VOLUME (mm)=	15.103
TOTAL RAINFALL (mm)=	62.700
RUNOFF COEFFICIENT =	0.241

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9142) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1422):	326.77	1.973	8.00	15.10

+ ID2= 2 (5142):	69.77	1.206	7.00	26.40
ID = 3 (9142):	396.54	3.074	7.00	17.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8104) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6142):	573.38	3.050	7.25	13.09
+ ID2= 2 (9142):	396.54	3.074	7.00	17.09
ID = 3 (8104):	969.92	6.092	7.00	14.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANHYD (1441)
 ID= 1 DT=15.0 min Area (ha)= 6.29
 Total Imp(%)= 45.00 Dir. Conn.(%)= 15.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	2.83	3.46
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	204.78	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	82.76	64.02
over (min)	15.00	30.00
Storage Coeff. (min)=	5.22 (ii)	18.00 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.11	0.05

TOTALS
 PEAK FLOW (cms)= 0.21 0.33 0.419 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 60.70 26.94 32.00
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.43 0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.4210	0.1639
0.0210	0.0769	0.5290	0.1874
0.1220	0.0890	0.6150	0.2104
0.2270	0.1148	1.0150	0.2204
0.3150	0.1371	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1441)	6.290	0.419	6.00	32.00
OUTFLOW: ID= 1 (5144)	6.290	0.166	6.75	31.86

PEAK FLOW REDUCTION [Qout/Qin] (%) = 39.62
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 0.1006

CALIB NASHYD (1442)
 ID= 1 DT=15.0 min Area (ha)= 129.40 Curve Number (CN)= 74.9
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.70

Unit Hyd Opeak (cms)= 3.147

PEAK FLOW (cms) = 1.855 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 22.910
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.365

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9144) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1442):	129.40	1.855	6.75	22.91
+ ID2= 2 (5144):	6.29	0.166	6.75	31.86
ID = 3 (9144):	135.69	2.021	6.75	23.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8106) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8104):	969.92	6.092	7.00	14.73
+ ID2= 2 (9144):	135.69	2.021	6.75	23.32
ID = 3 (8106):	1105.61	8.086	7.00	15.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6146)
 IN= 2----> OUT= 1
 Routing time step (min) = 15.00

<----- DATA FOR SECTION (1461.0) ----->

Distance	Elevation	Manning	
0.00	262.00	0.0350	
39.58	257.13	0.0350	
123.69	257.51	0.0350	
183.07	256.83	0.0350	
202.86	255.73	0.0350	
212.75	254.73	0.0350	
222.65	252.86	0.0350	
232.44	250.70	0.0350 / 0.0300	Main Channel
236.44	250.20	0.0300	Main Channel
236.84	249.80	0.0300	Main Channel
242.44	249.80	0.0300	Main Channel
250.04	249.80	0.0300	Main Channel
250.44	250.20	0.0300	Main Channel
252.33	250.57	0.0300 / 0.0350	Main Channel
286.97	253.52	0.0350	
331.50	255.80	0.0350	
380.98	255.55	0.0350	
445.30	258.40	0.0350	
455.19	259.82	0.0350	
489.83	262.22	0.0350	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	TRAVEL TIME (min)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.39	250.19	.169E+05	5.4	5.4	1.04	52.00
0.77	250.57	.373E+05	16.6	16.6	1.44	37.46
1.44	251.24	.911E+05	60.3	60.3	2.14	25.19
2.11	251.91	.169E+06	136.3	136.3	2.61	20.64
2.79	252.59	.270E+06	249.7	249.7	2.99	18.05
3.46	253.26	.396E+06	405.4	405.4	3.31	16.28
4.13	253.93	.549E+06	603.3	603.3	3.56	15.15
4.80	254.60	.737E+06	857.8	857.8	3.77	14.31
5.48	255.28	.963E+06	1174.6	1174.6	3.95	13.67
6.15	255.95	.128E+07	1449.5	1449.5	3.66	14.74
6.82	256.62	.173E+07	2046.4	2046.4	3.83	14.07
7.49	257.29	.227E+07	2665.2	2665.2	3.81	14.17
8.17	257.97	.310E+07	3705.2	3705.2	3.87	13.93
8.84	258.64	.400E+07	5127.4	5127.4	4.15	12.99
9.51	259.31	.492E+07	6858.6	6858.6	4.51	11.95
10.18	259.98	.587E+07	8815.5	8815.5	4.86	11.09
10.86	260.66	.684E+07	10954.6	10954.6	5.18	10.41
11.53	261.33	.785E+07	13324.3	13324.3	5.49	9.82
12.20	262.00	.890E+07	15923.2	15923.2	5.79	9.31

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8106)	1105.61	8.09	7.00	15.78	0.48	1.11
OUTFLOW: ID= 1 (6146)	1105.61	6.78	8.00	15.78	0.43	1.07

CALIB STANDHYD (1462)
 ID= 1 DT=15.0 min
 Area (ha) = 112.22
 Total Imp(%) = 32.00
 Di r. Conn. (%) = 13.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	35.91	76.31
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	864.95	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (min)	82.76	51.44
Storage Coeff. (min)	15.00	30.00
Unit Hyd. Tpeak (min)	12.38 (ii)	26.34 (ii)
Unit Hyd. peak (cms)	15.00	30.00
	0.08	0.04

PEAK FLOW (cms) = 2.64
 TIME TO PEAK (hrs) = 6.00
 RUNOFF VOLUME (mm) = 60.70
 TOTAL RAINFALL (mm) = 62.70
 RUNOFF COEFFICIENT = 0.97

5.937 (iii)
 6.25
 30.57
 62.70
 0.49

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1461)
 ID= 1 DT=15.0 min
 Area (ha) = 83.29
 Total Imp(%) = 38.00
 Di r. Conn. (%) = 16.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	31.65	51.64
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	745.16	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (min)	82.76	56.57
Storage Coeff. (min)	15.00	30.00
Unit Hyd. Tpeak (min)	11.32 (ii)	24.76 (ii)
Unit Hyd. peak (cms)	15.00	30.00
	0.08	0.04

PEAK FLOW (cms) = 2.49
 TIME TO PEAK (hrs) = 6.00
 RUNOFF VOLUME (mm) = 60.70
 TOTAL RAINFALL (mm) = 62.70
 RUNOFF COEFFICIENT = 0.97

4.840 (iii)
 6.00
 32.44
 62.70
 0.52

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5146)
 IN= 2----> OUT= 1
 DT= 15.0 min
 OUTFLOW (cms) | STORAGE (ha. m.) | OUTFLOW (cms) | STORAGE (ha. m.)

0.0000	0.0000	8.9200	2.5813
0.2820	0.8782	11.0570	3.0328
2.5360	1.0372	12.9860	3.4993
4.7880	1.7148	13.3860	3.5093
6.7570	2.1134	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (1461)	83.290	4.840	6.00	32.44
OUTFLOW: ID= 1 (5146)	83.290	2.789	6.50	32.43

PEAK FLOW REDUCTION [Qout/Qin] (%) = 57.63
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 1.1321

ADD HYD (8099)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1462):	112.22	5.937	6.25	30.57
+ ID2= 2 (5146):	83.29	2.789	6.50	32.43
=====				
ID = 3 (8099):	195.51	6.967	6.50	31.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8108)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6146):	1105.61	6.779	8.00	15.78
+ ID2= 2 (8099):	195.51	6.967	6.50	31.36
=====				
ID = 3 (8108):	1301.12	10.027	6.50	18.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8100)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8098):	10065.22	60.285	8.00	19.68
+ ID2= 2 (8108):	1301.12	10.027	6.50	18.12
=====				
ID = 3 (8100):	11366.34	68.784	8.00	19.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6202)
 IN= 2----> OUT= 1
 Routing time step (min) = 15.00

DATA FOR SECTION (2021.0) ----->				
Distance	Elevation	Manning		
0.00	245.48	0.0400		
29.68	242.80	0.0400		
74.20	240.39	0.0400		
155.81	237.12	0.0400		
200.33	234.48	0.0400		
244.85	234.67	0.0400		
259.69	233.96	0.0400		
274.53	233.84	0.0400		
281.68	233.40	0.0400		
285.78	232.40	0.0400 / 0.0300	Main Channel	
286.78	231.40	0.0300	Main Channel	
296.78	231.40	0.0300	Main Channel	
299.78	232.40	0.0300 / 0.0400	Main Channel	
311.62	233.74	0.0400		
333.88	234.34	0.0400		
497.11	235.32	0.0400		
571.31	237.32	0.0400		
675.18	239.10	0.0400		
704.86	242.71	0.0400		
734.54	242.62	0.0400		

<----- TRAVEL TIME TABLE ----->
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME

(m)	(m)	(cu. m.)	(cms)	(m/s)	(mi n)
0.50	231.90	.221E+05	4.7	0.85	78.67
1.00	232.40	.482E+05	15.3	1.28	52.40
1.60	233.00	.913E+05	38.3	1.68	39.77
2.20	233.60	.154E+06	72.9	1.90	35.24
2.80	234.20	.278E+06	124.2	1.79	37.30
3.40	234.80	.599E+06	214.3	1.44	46.59
4.01	235.41	.128E+07	445.0	1.43	46.90
4.61	236.01	.205E+07	838.4	1.64	40.81
5.21	236.61	.293E+07	1361.6	1.86	35.89
5.81	237.21	.389E+07	2011.2	2.08	32.23
6.41	237.81	.494E+07	2753.5	2.24	29.91
7.01	238.41	.612E+07	3635.0	2.39	28.04
7.61	239.01	.741E+07	4672.5	2.53	26.43
8.21	239.61	.880E+07	5999.3	2.74	24.44
8.82	240.22	.102E+08	7510.0	2.94	22.72
9.42	240.82	.117E+08	9201.9	3.15	21.23
10.02	241.42	.132E+08	11059.7	3.35	19.95
10.62	242.02	.148E+08	13073.4	3.54	18.87
11.22	242.62	.164E+08	15242.1	3.73	17.93

<---- hydrograph ---->					<- pi pe / channel ->	
INFLOW: ID= 2 (8100)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW: ID= 1 (6202)	*****	68.78	8.00	19.50	2.13	1.87
	*****	66.05	8.50	19.50	2.08	1.85

ADD HYD (8112)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6202):	11366.34	66.051	8.50	19.50
+ ID2= 2 (8110):	468.53	28.661	6.00	40.18
=====				
ID = 3 (8112):	11834.87	70.195	8.25	20.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5555)			
IN= 2----> OUT= 1	DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)
		0.0000	0.0000
		2.8500	0.5200
		8.1800	1.2600
		15.0900	2.2100
		23.2700	3.3900
		32.5300	4.7800
		36.5100	5.3900
		42.7600	6.3700
		53.8500	8.1300
		91.7300	14.3200

INFLOW: ID= 2 (8112)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
OUTFLOW: ID= 1 (5555)	*****	70.195	8.25	20.32
	*****	68.507	8.75	20.32

PEAK FLOW REDUCTION [Qout/Qin] (%) = 97.60
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 10.5349

ADD HYD (8114)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (5555):	11834.87	68.507	8.75	20.32
+ ID2= 2 (8136):	2288.44	20.196	6.75	19.38
=====				
ID = 3 (8114):	14123.31	81.779	8.50	20.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2061) | Area (ha) = 49.84

| ID= 1 DT=15.0 min | Total Imp(%)= 76.00 Dir. Conn.(%)= 50.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	37.88	11.96
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	576.43	40.00
Mannings n	=	0.013	0.250

Max. Eff. Inten.	(mm/hr)=	82.76	116.75
over	(min)	15.00	30.00
Storage Coeff.	(min)=	9.70 (ii)	19.76 (ii)
Unit Hyd. Tpeak	(min)=	15.00	30.00
Unit Hyd. peak	(cms)=	0.09	0.05

TOTALS
6.207 (iii)
6.00
48.48
62.70
0.77

PEAK FLOW	(cms)=	4.88	2.00
TIME TO PEAK	(hrs)=	6.00	6.25
RUNOFF VOLUME	(mm)=	60.70	36.25
TOTAL RAINFALL	(mm)=	62.70	62.70
RUNOFF COEFFICIENT	=	0.97	0.58

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5206)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha. m.)	(cms)	(ha. m.)
0.0000	0.0000	3.5640	2.4459
0.1690	0.8986	4.2460	2.7624
1.3810	1.2959	4.8070	3.0770
2.2440	1.6904	5.2070	3.0870
2.8510	2.0623	0.0000	0.0000

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (2061)	49.840	6.207	6.00	48.48
OUTFLOW: ID= 1 (5206)	49.840	1.540	6.75	48.45

PEAK FLOW REDUCTION [Qout/Qin] (%) = 24.80
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 1.3782

CALIB
STANDHYD (2062)
ID= 1 DT=15.0 min

Area (ha)= 225.70
Total Imp(%)= 64.00 Dir. Conn.(%)= 42.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	144.45	81.25
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	1226.65	40.00
Mannings n	=	0.013	0.250

Max. Eff. Inten.	(mm/hr)=	82.76	79.66
over	(min)	15.00	30.00
Storage Coeff.	(min)=	15.27 (ii)	26.98 (ii)
Unit Hyd. Tpeak	(min)=	15.00	30.00
Unit Hyd. peak	(cms)=	0.07	0.04

TOTALS
20.865 (iii)
6.00
44.00
62.70
0.70

PEAK FLOW	(cms)=	15.75	7.94
TIME TO PEAK	(hrs)=	6.00	6.25
RUNOFF VOLUME	(mm)=	60.70	31.91
TOTAL RAINFALL	(mm)=	62.70	62.70
RUNOFF COEFFICIENT	=	0.97	0.51

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8144)
1 + 2 = 3

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (2062):	225.70	20.865	6.00	44.00
+ ID2= 2 (5206):	49.84	1.540	6.75	48.45
ID = 3 (8144):	275.54	21.017	6.00	44.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (2041)
ID= 1 DT=15.0 min

Area (ha)= 209.90
Total Imp(%)= 37.00 Dir. Conn.(%)= 21.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	77.66	132.24
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	1182.93	40.00
Mannings n	=	0.013	0.250

Max. Eff. Inten.	(mm/hr)=	82.76	21.15
over	(min)	15.00	45.00
Storage Coeff.	(min)=	14.94 (ii)	34.85 (ii)
Unit Hyd. Tpeak	(min)=	15.00	45.00
Unit Hyd. peak	(cms)=	0.07	0.03

TOTALS
9.234 (iii)
6.00
27.22
62.70
0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5204)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha. m.)	(cms)	(ha. m.)
0.0000	0.0000	13.0280	5.1901
0.5340	2.0405	16.1840	5.6427
3.0020	2.3552	19.0840	6.7700
5.5440	3.8411	19.4840	6.7800
8.4130	4.5335	0.0000	0.0000

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (2041)	209.900	9.234	6.00	27.22
OUTFLOW: ID= 1 (5204)	209.900	3.521	7.00	27.21

PEAK FLOW REDUCTION [Qout/Qin] (%) = 38.13
TIME SHIFT OF PEAK FLOW (min) = 60.00
MAXIMUM STORAGE USED (ha. m.) = 2.6669

CALIB
STANDHYD (2042)
ID= 1 DT=15.0 min

Area (ha)= 114.52
Total Imp(%)= 61.00 Dir. Conn.(%)= 34.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	69.86	44.66
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	873.77	40.00
Mannings n	=	0.013	0.250

Max. Eff. Inten.	(mm/hr)=	82.76	58.99
over	(min)	15.00	30.00
Storage Coeff.	(min)=	12.46 (ii)	25.67 (ii)
Unit Hyd. Tpeak	(min)=	15.00	30.00
Unit Hyd. peak	(cms)=	0.08	0.04

TOTALS

PEAK FLOW (cms)= 7.02 3.28 9.083 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 60.70 22.92 35.77
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.37 0.57

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2042):	114.52	9.083	6.00	35.77
+ ID2= 2 (5204):	209.90	3.521	7.00	27.21
ID = 3 (8140):	324.42	9.404	6.00	30.23

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6206)
 IN= 2--> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2061.0) ----->

Distance	Elevation	Manning	
0.00	253.07	0.0350	
13.98	252.89	0.0350	
27.95	251.99	0.0350	
34.94	252.18	0.0350	
41.93	252.02	0.0350	
73.38	250.35	0.0350	
94.34	247.09	0.0350	
111.81	244.79	0.0350	
115.31	244.24	0.0350	
117.55	244.20	0.0350 /0.0300	Main Channel
118.80	243.60	0.0300	Main Channel
122.30	243.60	0.0300	Main Channel
122.55	244.60	0.0300 /0.0350	Main Channel
146.76	244.74	0.0350	
164.23	247.26	0.0350	
213.15	248.92	0.0350	
241.10	249.51	0.0350	
272.55	250.87	0.0350	
300.50	252.26	0.0350	
345.93	253.67	0.0350	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.30	243.90	.310E+04	1.4	1.23	36.25
0.60	244.20	.676E+04	4.6	1.81	24.64
1.12	244.72	.221E+05	17.1	2.07	21.60
1.64	245.24	.754E+05	64.1	2.28	19.60
2.17	245.77	.139E+06	149.8	2.88	15.50
2.99	246.29	.214E+06	273.3	3.43	13.04
3.21	246.81	.299E+06	436.1	3.91	11.43
3.73	247.33	.395E+06	632.6	4.30	10.40
4.25	247.85	.511E+06	836.4	4.39	10.18
4.77	248.37	.653E+06	1114.9	4.58	9.76
5.30	248.90	.822E+06	1472.1	4.81	9.30
5.82	249.42	.102E+07	1872.2	4.91	9.10
6.34	249.94	.126E+07	2444.6	5.22	8.56
6.86	250.46	.151E+07	3113.1	5.52	8.09
7.38	250.98	.180E+07	3843.5	5.74	7.79
7.90	251.50	.211E+07	4717.8	6.00	7.45
8.43	252.03	.245E+07	5701.4	6.24	7.16
8.95	252.55	.284E+07	6727.5	6.36	7.03
9.47	253.07	.326E+07	7885.3	6.48	6.90

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8140)	324.42	9.40	6.00	30.23	0.80	1.91
OUTFLOW: ID= 1 (6206)	324.42	6.67	6.50	30.23	0.69	1.85

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6206):	324.42	6.670	6.50	30.23
+ ID2= 2 (8144):	275.54	21.017	6.00	44.81
ID = 3 (8142):	599.96	24.557	6.00	36.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8114):	14123.31	81.779	8.50	20.17
+ ID2= 2 (8142):	599.96	24.557	6.00	36.92
ID = 3 (8138):	14723.27	87.616	8.25	20.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6208)
 IN= 2--> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2081.0) ----->

Distance	Elevation	Manning	
0.00	249.19	0.1000	
73.11	244.13	0.1000	
121.86	243.39	0.1000	
243.71	240.01	0.1000	
353.38	238.75	0.1000	
450.87	235.40	0.1000	
499.61	231.40	0.1000	
511.80	230.96	0.1000	
523.98	230.12	0.1000	
534.50	227.80	0.1000 /0.0500	Main Channel
536.50	226.70	0.0500	Main Channel
546.50	226.70	0.0500	Main Channel
548.50	227.80	0.0500 /0.1000	Main Channel
572.72	230.25	0.1000	
682.40	230.50	0.1000	
804.25	233.95	0.1000	
926.11	235.83	0.1000	
1047.96	240.70	0.1000	
1145.45	243.26	0.1000	
1206.38	247.03	0.1000	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.55	227.25	.111E+05	2.0	0.33	92.44
1.10	227.80	.242E+05	6.5	0.50	61.56
2.23	228.93	.700E+05	26.0	0.68	44.91
3.36	230.06	.150E+06	60.8	0.74	41.04
4.49	231.19	.446E+06	130.8	0.54	56.83
5.62	232.32	.925E+06	287.4	0.57	53.66
6.76	233.46	.152E+07	522.9	0.63	48.32
7.89	234.59	.223E+07	827.2	0.68	44.91
9.02	235.72	.312E+07	1219.5	0.72	42.61
10.15	236.85	.418E+07	1780.6	0.78	39.11
11.28	237.98	.537E+07	2479.2	0.85	36.07
12.41	239.11	.669E+07	3282.9	0.90	33.95
13.54	240.24	.824E+07	4228.6	0.94	32.47
14.67	241.37	.997E+07	5384.6	0.99	30.87
15.81	242.51	.119E+08	6698.4	1.03	29.56
16.94	243.64	.140E+08	8235.2	1.08	28.25
18.07	244.77	.162E+08	10104.2	1.14	26.73
19.20	245.90	.185E+08	12251.5	1.21	25.21
20.33	247.03	.209E+08	14597.0	1.28	23.89

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8138)	*****	87.62	8.25	20.85	3.80	0.65
OUTFLOW: ID= 1 (6208)	*****	81.12	9.00	20.85	3.70	0.67

CALIB
STANDHYD (2082)
ID= 1 DT=15.0 min

Area (ha)	= 426.60	Dir. Conn. (%)	= 37.00
Total Imp (%)	= 64.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 273.02	153.58
Dep. Storage (mm)	= 2.00	5.00
Average Slope (%)	= 0.50	0.50
Length (m)	= 1686.42	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr)	= 82.76	79.07
over (min)	= 15.00	45.00
Storage Coeff. (min)	= 18.48 (ii)	30.23 (ii)
Unit Hyd. Tpeak (min)	= 15.00	45.00
Unit Hyd. peak (cms)	= 0.06	0.03

TOTALS
29.657 (iii)
6.00
40.94
62.70
0.65

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (2081)
ID= 1 DT=15.0 min

Area (ha)	= 209.70	Dir. Conn. (%)	= 20.00
Total Imp (%)	= 35.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 73.39	136.31
Dep. Storage (mm)	= 2.00	5.00
Average Slope (%)	= 0.50	0.50
Length (m)	= 1182.37	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr)	= 82.76	44.12
over (min)	= 15.00	30.00
Storage Coeff. (min)	= 14.93 (ii)	29.77 (ii)
Unit Hyd. Tpeak (min)	= 15.00	30.00
Unit Hyd. peak (cms)	= 0.07	0.04

TOTALS
11.324 (iii)
6.00
30.84
62.70
0.49

- ***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
- ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (2084)
ID= 1 DT=15.0 min

Area (ha)	= 56.00	Dir. Conn. (%)	= 70.00
Total Imp (%)	= 75.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 42.00	14.00
Dep. Storage (mm)	= 2.00	5.00
Average Slope (%)	= 0.50	0.50
Length (m)	= 611.01	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr)	= 82.76	61.33
over (min)	= 15.00	30.00
Storage Coeff. (min)	= 10.05 (ii)	23.06 (ii)

Unit Hyd. Tpeak (min)	= 15.00	30.00
Unit Hyd. peak (cms)	= 0.09	0.04

TOTALS
8.332 (iii)
6.00
52.28
62.70
0.83

PEAK FLOW (cms)	= 7.60	1.14
TIME TO PEAK (hrs)	= 6.00	6.25
RUNOFF VOLUME (mm)	= 60.70	32.63
TOTAL RAINFALL (mm)	= 62.70	62.70
RUNOFF COEFFICIENT	= 0.97	0.52

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 82.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9289)
IN= 2---> OUT= 1
DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	2.8000	1.5745
	0.1900	0.9984	3.7500	1.7809
	1.0900	1.1991	4.7100	1.9291
	2.0200	1.3813	5.4800	2.0745

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2084)	56.000	8.332	6.00	52.28
OUTFLOW: ID= 1 (9289)	56.000	2.663	6.50	52.26

PEAK FLOW REDUCTION [Out/Oi n] (%) = 31.96
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 1.5558

ADD HYD (9262)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2081):	209.70	11.324	6.00	30.84
+ ID2= 2 (9289):	56.00	2.663	6.50	52.26
ID = 3 (9262):	265.70	12.804	6.25	35.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5208)
IN= 2---> OUT= 1
DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	16.8530	8.3371
	0.8360	4.7353	21.1630	9.3975
	4.7810	5.1070	25.1140	10.4406
	8.9050	6.2484	25.5140	10.4506
	12.6070	7.1796	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9262)	265.700	12.804	6.25	35.35
OUTFLOW: ID= 1 (5208)	265.700	3.927	7.50	35.35

PEAK FLOW REDUCTION [Out/Oi n] (%) = 30.67
TIME SHIFT OF PEAK FLOW (min) = 75.00
MAXIMUM STORAGE USED (ha. m.) = 5.0449

ADD HYD (8148)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2082):	426.60	29.657	6.00	40.94
+ ID2= 2 (5208):	265.70	3.927	7.50	35.35
ID = 3 (8148):	692.30	29.922	6.00	38.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8146)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (6208):	14723.27	81.117	9.00	20.85
+ ID2= 2 (8148):	692.30	29.922	6.00	38.80
=====				
ID = 3 (8146):	15415.57	85.264	9.00	21.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6210)
IN= 2--> OUT= 1 Routing time step (min)' = 15.00

Distance	Elevation	Manning	
0.00	241.81	0.0800	
28.43	241.78	0.0800	
85.30	237.04	0.0800	
149.27	231.32	0.0800	
170.60	230.10	0.0800	
255.89	229.37	0.0800	
277.22	228.68	0.0800	
284.33	228.42	0.0800	
291.43	228.16	0.0800	
297.76	227.00	0.0800 / 0.0350	Main Channel
298.76	225.00	0.0350	Main Channel
312.76	225.00	0.0350	Main Channel
313.76	227.00	0.0350 / 0.0800	Main Channel
326.97	229.50	0.0800	
348.30	230.29	0.0800	
376.73	229.76	0.0800	
490.46	235.75	0.0800	
568.65	236.24	0.0800	
661.06	241.85	0.0800	
703.71	246.04	0.0800	

DEPTH (m)	ELEV (m)	VOLUME (cu. m)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.67	225.67	.141E+05	6.6	0.69	35.86
1.33	226.33	.289E+05	20.4	1.04	23.65
2.00	227.00	.443E+05	39.3	1.31	18.81
2.93	227.93	.730E+05	77.8	1.57	15.64
3.85	228.85	.123E+06	131.1	1.57	15.65
4.78	229.78	.228E+06	205.6	1.33	18.51
5.70	230.70	.503E+06	356.7	1.05	23.49
6.63	231.63	.846E+06	608.7	1.06	23.17
7.55	232.55	.123E+07	946.4	1.14	21.65
8.48	233.48	.165E+07	1367.0	1.22	20.13
9.41	234.41	.211E+07	1871.8	1.31	18.80
10.33	235.33	.261E+07	2463.2	1.40	17.65
11.26	236.26	.317E+07	2999.4	1.40	17.63
12.18	237.18	.384E+07	3848.3	1.48	16.63
13.11	238.11	.454E+07	4806.0	1.56	15.75
14.03	239.03	.528E+07	5876.9	1.64	14.97
14.96	239.96	.605E+07	7062.7	1.72	14.29
15.88	240.88	.687E+07	8365.0	1.80	13.68
16.81	241.81	.771E+07	9506.9	1.82	13.52

INFLOW : ID= 2 (8146)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW: ID= 1 (6210)	85.26	85.26	9.00	21.66	3.06	1.57
	84.88	84.88	9.00	21.66	3.05	1.57

CALIB STANDHYD (2104)
ID= 1 DT=15.0 min Area (ha)= 192.70 Total Imp(%)= 73.00 Di r. Conn.(%)= 68.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	140.67	52.03
Dep. Storage	1.00	5.00
Average Slope	1.00	2.00
Length	1133.43	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	82.76	58.29
over (mi n)	15.00	30.00
Storage Coeff. (mi n)=	11.83 (ii)	20.58 (ii)
Unit Hyd. Tpeak (mi n)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.05

PEAK FLOW (cms)=	24.08	4.22	*TOTALS* 26.804 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	61.70	31.45	52.02
TOTAL RAINFALL (mm)=	62.70	62.70	62.70
RUNOFF COEFFICIENT =	0.98	0.50	0.83

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 81.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9283)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.5000	8.0000
0.3000	2.0000	3.5000	10.0000
0.9000	5.0000	4.5000	13.0000
2.0000	7.5000	5.5000	14.0000

INFLOW : ID= 2 (2104)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (9283)	192.700	26.804	6.00	52.02
	192.700	1.870	8.00	52.01

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 6.98
TIME SHIFT OF PEAK FLOW (mi n) = 120.00
MAXIMUM STORAGE USED (ha. m.) = 7.2057

CALIB STANDHYD (2103)
ID= 1 DT=15.0 min

Area (ha)= 11.10 Total Imp(%)= 30.00 Di r. Conn.(%)= 10.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	3.33	7.77
Dep. Storage	1.00	5.00
Average Slope	1.00	2.00
Length	272.03	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	82.76	72.32
over (mi n)	15.00	15.00
Storage Coeff. (mi n)=	5.02 (ii)	13.06 (ii)
Unit Hyd. Tpeak (mi n)=	15.00	15.00
Unit Hyd. peak (cms)=	0.11	0.08

PEAK FLOW (cms)=	0.25	1.16	*TOTALS* 1.410 (iii)
TIME TO PEAK (hrs)=	6.00	6.00	6.00
RUNOFF VOLUME (mm)=	61.70	35.86	38.45
TOTAL RAINFALL (mm)=	62.70	62.70	62.70
RUNOFF COEFFICIENT =	0.98	0.57	0.61

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9284)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.5600	0.2067
0.0400	0.1045	0.7400	0.2572
0.2200	0.1166	0.9300	0.3046

0.4000 0.1670 | 1.0900 0.3510

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2103)	11.100	1.410	6.00	38.45
OUTFLOW: ID= 1 (9284)	11.100	0.461	6.50	38.39

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 32.72
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 0.1862

ADD HYD (9258)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9283):	192.70	1.870	8.00	52.01
+ ID2= 2 (9284):	11.10	0.461	6.50	38.39

ID = 3 (9258):	203.80	2.142	7.00	51.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5210)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.0060	0.0024
0.0020	0.0013	0.0080	0.0027
0.0030	0.0017	0.0090	0.0031
0.0050	0.0020	0.0090	0.0131

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9258)	203.800	2.142	7.00	51.27
OUTFLOW: ID= 1 (5210)	203.800	2.140	7.00	51.27

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 99.91
 TIME SHIFT OF PEAK FLOW (min) = 0.00
 MAXIMUM STORAGE USED (ha. m.) = 0.0565

CALIB NASHYD (2102)
 ID= 1 DT=15.0 min

Area (ha) = 14.38 Curve Number (CN) = 80.0
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.08

Unit Hyd Qpeak (cms) = 0.228

PEAK FLOW (cms) = 0.175 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 27.218
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.434

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9210)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2102):	14.38	0.175	7.50	27.22
+ ID2= 2 (5210):	203.80	2.140	7.00	51.27

ID = 3 (9210):	218.18	2.313	7.00	49.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8150)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6210):	15415.57	84.876	9.00	21.66
+ ID2= 2 (9210):	218.18	2.313	7.00	49.68

ID = 3 (8150):	15633.75	86.880	9.00	22.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2121)
 ID= 1 DT=15.0 min

Area (ha) = 56.89
 Total Imp(%) = 91.00 Dir. Conn.(%) = 74.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	51.77	5.12
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	615.85	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	82.76	164.47
over (min)	15.00	30.00
Storage Coeff. (min)	10.10 (ii)	18.86 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.05

TOTALS
 PEAK FLOW (cms) = 8.15 1.23 8.971 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 60.70 37.07 54.56
 TOTAL RAINFALL (mm) = 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.59 0.87

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2124)
 ID= 1 DT=15.0 min

Area (ha) = 4.40
 Total Imp(%) = 75.00 Dir. Conn.(%) = 70.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	3.30	1.10
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	171.27	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	82.76	25.11
over (min)	15.00	30.00
Storage Coeff. (min)	4.69 (ii)	23.28 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.11	0.04

TOTALS
 PEAK FLOW (cms) = 0.69 0.06 0.726 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 60.70 22.30 49.18
 TOTAL RAINFALL (mm) = 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.36 0.78

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9290)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.2200	0.1226
0.0100	0.0778	0.2900	0.1387
0.0800	0.0934	0.3700	0.1503
0.1600	0.1076	0.4300	0.1616

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2124)	4.400	0.726	6.00	49.18
OUTFLOW: ID= 1 (9290)	4.400	0.210	6.25	48.73

PEAK FLOW REDUCTION [Qout/Qin] (%) = 28.92
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 0.1215

ADD HYD (9264) 1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2121):	56.89	8.971	6.00	54.56
+ ID2= 2 (9290):	4.40	0.210	6.25	48.73
ID = 3 (9264):	61.29	9.046	6.00	54.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5212)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.1000	2.1402
0.2080	1.2793	5.1480	2.2661
1.1880	1.5546	5.9930	2.3900
2.2120	1.7255	6.3930	2.4000
3.0680	1.9336	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9264)	61.290	9.046	6.00	54.14
OUTFLOW: ID= 1 (5212)	61.290	2.576	6.50	54.11

PEAK FLOW REDUCTION [Qout/Qin] (%) = 28.48
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 1.8158

CALIB STANDHYD (2122)
 ID= 1 DT=15.0 min

Area (ha) = 267.10
 Total Imp (%) = 29.00
 Dir. Conn. (%) = 24.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	77.46	189.64
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1334.42	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	82.76	22.61
over (min)	15.00	45.00
Storage Coeff. (min)	16.06 (ii)	35.45 (ii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.07	0.03

TOTALS

PEAK FLOW (cms)	10.42	6.75	13.224 (iii)
TIME TO PEAK (hrs)	6.00	6.50	6.00
RUNOFF VOLUME (mm)	60.70	22.47	31.65
TOTAL RAINFALL (mm)	62.70	62.70	62.70
RUNOFF COEFFICIENT	0.97	0.36	0.50

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9312)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	13.3600	4.9180
0.9100	2.4658	17.8700	6.1368
5.1800	2.7404	22.4400	7.2882
9.6400	3.9605	26.1200	8.4139

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9312)	61.290	9.046	6.00	54.14
OUTFLOW: ID= 1 (9312)	61.290	2.576	6.50	54.11

INFLOW: ID= 2 (2122) 267.100 13.224 6.00 31.65
 OUTFLOW: ID= 1 (9312) 267.100 7.083 6.75 31.64

PEAK FLOW REDUCTION [Qout/Qin] (%) = 53.56
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 3.2695

ADD HYD (8152) 1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (5212):	61.29	2.576	6.50	54.11
+ ID2= 2 (9312):	267.10	7.083	6.75	31.64
ID = 3 (8152):	328.39	9.497	6.75	35.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6214)
 IN= 2--> OUT= 1

Routing time step (min) = 15.00

<--- DATA FOR SECTION (2141.0) --->

Distance	Elevation	Manning
0.00	259.29	0.0500
15.33	258.91	0.0500
38.33	258.57	0.0500
65.16	257.93	0.0500
95.82	253.44	0.0500
111.15	251.21	0.0500
130.32	249.67	0.0500
149.48	248.51	0.0500
150.66	248.50	0.0500 / 0.0370
150.81	247.50	0.0370
154.66	247.50	0.0370
155.66	248.50	0.0370 / 0.0500
160.98	248.50	0.0500
187.81	249.78	0.0500
199.31	252.13	0.0500
245.30	254.04	0.0500
264.46	253.99	0.0500
298.96	255.23	0.0500
329.62	257.02	0.0500
379.45	258.82	0.0500

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	248.00	.648E+04	2.1	1.00	52.30
1.00	248.50	.139E+05	6.4	1.44	36.35
1.61	249.11	.570E+05	22.6	1.24	42.01
2.21	249.71	.143E+06	63.6	1.39	37.61
2.82	250.32	.263E+06	142.9	1.70	30.69
3.43	250.93	.403E+06	257.2	2.00	26.11
4.04	251.54	.562E+06	410.4	2.29	22.82
4.64	252.14	.735E+06	601.3	2.56	20.37
5.25	252.75	.933E+06	791.0	2.66	19.67
5.86	253.36	.117E+07	1038.0	2.79	18.74
6.46	253.96	.144E+07	1344.4	2.93	17.82
7.07	254.57	.178E+07	1664.1	2.93	17.83
7.68	255.18	.217E+07	2140.2	3.10	16.87
8.28	255.78	.259E+07	2722.9	3.30	15.83
8.89	256.39	.304E+07	3387.1	3.50	14.94
9.50	257.00	.351E+07	4132.7	3.69	14.16
10.11	257.61	.402E+07	4913.0	3.83	13.64
10.71	258.21	.458E+07	5648.3	3.87	13.50
11.32	258.82	.521E+07	6343.1	3.82	13.68

hydrograph

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8152)	328.39	9.50	6.75	35.84	1.12	1.39
OUTFLOW: ID= 1 (6214)	328.39	7.50	7.25	35.84	1.04	1.42

CALIB STANDHYD (2144)
 ID= 1 DT=15.0 min

Area (ha) = 93.10
 Total Imp (%) = 75.00
 Dir. Conn. (%) = 70.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	69.82	23.27	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	787.82	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	82.76	25.92	
over (min)	15.00	45.00	
Storage Coeff. (min)=	11.70 (ii)	30.06 (ii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.08	0.03	
			TOTALS
PEAK FLOW (cms)=	12.02	1.03	12.451 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	60.70	22.96	49.38
TOTAL RAINFALL (mm)=	62.70	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.37	0.79

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9285)
IN= 2----> OUT= 1
DT= 15.0 min

	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	4.6500	2.6150
	0.3200	1.6583	6.2300	2.9579
	1.8100	1.9916	7.8200	3.2041
	3.3600	2.2943	9.1000	3.4456

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (2144)	93.100	12.451	6.00	49.38
OUTFLOW: ID= 1 (9285)	93.100	3.878	6.50	49.36

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 31.14
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha.m.) = 2.4246

CALIB
STANDHYD (2143)
ID= 1 DT=15.0 min

Area (ha) = 158.30
Total Imp(%) = 26.00
Di r. Conn. (%) = 9.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	41.16	117.14	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	1027.29	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	82.76	46.82	
over (min)	15.00	30.00	
Storage Coeff. (min)=	13.73 (ii)	28.22 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.08	0.04	
			TOTALS
PEAK FLOW (cms)=	2.47	6.50	7.623 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.25
RUNOFF VOLUME (mm)=	60.70	24.74	27.98
TOTAL RAINFALL (mm)=	62.70	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.39	0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9286)
IN= 2----> OUT= 1
DT= 15.0 min

	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	7.9200	2.8224
	0.5400	1.3792	10.5900	3.5534
	3.0700	1.5175	13.3000	4.2519
	5.7200	2.2534	15.4900	4.9349

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (2143)	158.300	7.623	6.25	27.98
OUTFLOW: ID= 1 (9286)	158.300	4.044	6.75	27.97

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 53.05
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha.m.) = 1.7891

ADD HYD (9260)
1 + 2 = 3

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (9285):	93.10	3.878	6.50	49.36
+ ID2= 2 (9286):	158.30	4.044	6.75	27.97
=====				
ID = 3 (9260):	251.40	7.663	6.75	35.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5214)
IN= 2----> OUT= 1
DT= 15.0 min

	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	2.5370	0.9877
	0.1280	0.4635	3.1860	1.1293
	0.5900	0.5362	3.7090	1.2678
	1.3690	0.6921	3.9980	1.2778
	1.8980	0.8262	0.0000	0.0000

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (9260)	251.400	7.663	6.75	35.89
OUTFLOW: ID= 1 (5214)	251.400	7.627	7.00	35.89

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 99.54
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha.m.) = 1.4343

CALIB
STANDHYD (2142)
ID= 1 DT=15.0 min

Area (ha) = 65.54
Total Imp(%) = 27.00
Di r. Conn. (%) = 10.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	17.70	47.84	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	661.01	40.00	
Mannings n =	0.013	0.250	

Max. Eff. Inten. (mm/hr)=	82.76	14.87
over (min)	15.00	45.00
Storage Coeff. (min)=	10.54 (ii)	33.46 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.09	0.03

			TOTALS
PEAK FLOW (cms)=	1.25	1.17	1.724 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	60.70	13.40	18.13
TOTAL RAINFALL (mm)=	62.70	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.21	0.29

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 51.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2142):	65.54	1.724	6.00	18.13
+ ID2= 2 (5214):	251.40	7.627	7.00	35.89
=====				
ID = 3 (9214):	316.94	8.476	7.00	32.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6214):	328.39	7.497	7.25	35.84
+ ID2= 2 (9214):	316.94	8.476	7.00	32.22
=====				
ID = 3 (8154):	645.33	15.262	7.00	34.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2163) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	69.70 27.00	Di r. Conn. (%)=	9.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	18.82	50.88
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	681.66	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	82.76	57.51
over (min)	15.00	30.00
Storage Coeff. (min)=	10.73 (ii)	24.08 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.04

TOTALS

PEAK FLOW (cms)=	1.19	3.78	4.223 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.25
RUNOFF VOLUME (mm)=	60.70	29.65	32.45
TOTAL RAINFALL (mm)=	62.70	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.47	0.52

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 78.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9271) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	3.4900	1.2605
	0.2400	0.6230	4.6600	1.5808
	1.3500	0.6885	5.8600	1.8854
	2.5200	1.0102	6.8200	2.1832

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2163)	69.700	4.223	6.25	32.45
OUTFLOW: ID= 1 (9271)	69.700	2.105	6.75	32.44

PEAK FLOW REDUCTION [Qout/Qin] (%) = 49.85
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 0.8994

CALIB STANDHYD (2169) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	3.50 30.00	Di r. Conn. (%)=	10.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	1.05	2.45
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	152.75	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	82.76	46.17
over (min)	15.00	30.00
Storage Coeff. (min)=	4.37 (ii)	18.95 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.11	0.05

PEAK FLOW (cms)=	0.08	0.16	*TOTALS* 0.181 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	60.70	23.44	27.16
TOTAL RAINFALL (mm)=	62.70	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.37	0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9272) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	0.1800	0.0659
	0.0100	0.0333	0.2400	0.0820
	0.0700	0.0372	0.3000	0.0971
	0.1300	0.0532	0.3500	0.1119

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2169)	3.500	0.181	6.00	27.16
OUTFLOW: ID= 1 (9272)	3.500	0.090	6.75	26.93

PEAK FLOW REDUCTION [Qout/Qin] (%) = 49.72
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 0.0433

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9271):	69.70	2.105	6.75	32.44
+ ID2= 2 (9272):	3.50	0.090	6.75	26.93
=====				
ID = 3 (9251):	73.20	2.196	6.75	32.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5216) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	3.7400	2.4559
	0.1890	1.6831	4.6960	2.6646
	1.0830	1.7903	5.4670	2.8688
	2.0170	2.0202	5.8670	2.8788
	2.7980	2.2179	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9251)	73.200	2.196	6.75	32.17
OUTFLOW: ID= 1 (5216)	73.200	0.165	15.25	32.14

PEAK FLOW REDUCTION [Oout/Oin] (%) = 7.52
 TIME SHIFT OF PEAK FLOW (min) = 510.00
 MAXIMUM STORAGE USED (ha.m.) = 1.4703

CALIB STANDHYD (2162)
 ID= 1 DT=15.0 min

Area (ha) =	72.57
Total Imp (%) =	20.00
Dir. Conn. (%) =	7.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) =	14.51	58.06
Dep. Storage (mm) =	2.00	5.00
Average Slope (%) =	0.50	0.50
Length (m) =	695.56	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr) =	82.76	23.78
over (min) =	15.00	30.00
Storage Coeff. (min) =	10.86 (ii)	29.86 (ii)
Unit Hyd. Tpeak (min) =	15.00	30.00
Unit Hyd. peak (cms) =	0.08	0.04

TOTALS
 PEAK FLOW (cms) = 0.96 2.58 2.94 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.25
 RUNOFF VOLUME (mm) = 60.70 21.85 24.57
 TOTAL RAINFALL (mm) = 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.35 0.39

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9316)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.0000	0.3000
2.0000	0.1000	4.5000	0.4000
3.0000	0.2000	5.0000	0.5000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2162)	72.570	2.940	6.25	24.57
OUTFLOW: ID= 1 (9316)	72.570	2.523	6.50	24.57

PEAK FLOW REDUCTION [Oout/Oin] (%) = 85.83
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha.m.) = 0.1670

ADD HYD (9216)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (5216):	73.20	0.165	15.25	32.14
+ ID2= 2 (9316):	72.57	2.523	6.50	24.57
ID= 3 (9216):	145.77	2.550	6.50	28.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8156)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8154):	645.33	15.262	7.00	34.06
+ ID2= 2 (9216):	145.77	2.550	6.50	28.37
ID= 3 (8156):	791.10	16.877	7.00	33.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6218)
 IN= 2--> OUT= 1

Routing time step (min) = 15.00

DATA FOR SECTION (2181.0)

Distance	Elevation	Manning
0.00	256.38	0.0450
7.70	254.85	0.0450
15.40	253.20	0.0450
19.25	252.29	0.0450
23.10	251.17	0.0450
27.94	236.16	0.0450
32.79	235.80	0.0450
37.64	235.45	0.0450
42.49	234.60	0.0450 / 0.0300
47.34	233.60	0.0300
52.19	233.60	0.0300
57.04	233.60	0.0300
61.89	234.60	0.0300 / 0.0450
66.74	235.13	0.0450
71.59	235.20	0.0450
76.44	235.32	0.0450
81.29	239.95	0.0450
86.14	180.98	0.0450
90.99	188.69	0.0450
95.84	190.61	0.0450

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
1.00	234.60	.181E+05	6.0	0.66	50.64
2.13	235.73	.518E+05	23.8	0.93	36.27
3.25	236.85	.119E+06	60.0	1.02	33.03
4.38	237.98	.217E+06	120.6	1.12	29.99
5.51	239.11	.346E+06	210.5	1.23	27.39
6.63	240.23	.506E+06	334.1	1.33	25.22
7.76	241.36	.697E+06	495.6	1.43	23.44
8.88	242.48	.920E+06	699.8	1.53	21.91
10.01	243.61	.117E+07	950.8	1.63	20.59
11.14	244.74	.146E+07	1252.5	1.73	19.45
12.26	245.86	.178E+07	1619.6	1.83	18.37
13.39	246.99	.212E+07	2078.5	1.97	17.01
14.52	248.12	.247E+07	2589.0	2.11	15.88
15.64	249.24	.282E+07	3149.7	2.25	14.94
16.77	250.37	.319E+07	3759.8	2.37	14.14
17.89	251.49	.356E+07	4419.5	2.50	13.44
19.02	252.62	.395E+07	5126.4	2.61	12.84
20.15	253.75	.435E+07	5871.6	2.72	12.34
21.27	254.87	.476E+07	6663.2	2.82	11.89

hydrograph

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8156)	791.10	16.88	7.00	33.01	1.69	0.80
OUTFLOW: ID= 1 (6218)	791.10	11.90	7.50	33.01	1.38	0.73

CALIB STANDHYD (2183)
 ID= 1 DT=15.0 min

Area (ha) =	69.30
Total Imp (%) =	22.00
Dir. Conn. (%) =	7.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) =	15.25	54.05
Dep. Storage (mm) =	2.00	5.00
Average Slope (%) =	0.50	0.50
Length (m) =	679.71	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr) =	82.76	57.01
over (min) =	15.00	30.00
Storage Coeff. (min) =	10.71 (ii)	24.11 (ii)
Unit Hyd. Tpeak (min) =	15.00	30.00
Unit Hyd. peak (cms) =	0.09	0.04

TOTALS
 PEAK FLOW (cms) = 0.92 3.98 4.324 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.25
 RUNOFF VOLUME (mm) = 60.70 30.63 32.73
 TOTAL RAINFALL (mm) = 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.49 0.52

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 80.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9282)
 IN= 2--> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.5000	3.5000
0.8000	0.1000	6.0000	4.0000
1.1000	2.0000	6.5000	4.5000
2.5000	3.0000	7.0000	5.5000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2183)	69.300	4.324	6.25	32.73
OUTFLOW: ID= 1 (9282)	69.300	0.939	7.50	32.73

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 21.71
 TIME SHIF T OF PEAK FLOW (mi n) = 75.00
 MAXI MUM STORAGE USED (ha. m.) = 0.9852

RESERVOIR (5218)
 IN= 2--> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.5090	1.7553
0.2280	0.8236	5.6620	2.0069
1.3060	0.9528	6.5910	2.2530
2.4320	1.2300	6.9910	2.2630

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9282)	69.300	0.939	7.50	32.73
OUTFLOW: ID= 1 (5218)	69.300	0.884	9.75	32.72

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 94.18
 TIME SHIF T OF PEAK FLOW (mi n) = 135.00
 MAXI MUM STORAGE USED (ha. m.) = 0.9023

CALIB
 NASHYD (2182)
 ID= 1 DT=15.0 mi n

Area (ha) = 82.95 Curve Number (CN) = 67.7
 Ia (mm) = 5.00 # of Li near Res. (N) = 1.50
 U.H. Tp(hrs) = 0.79

Unit Hyd Opeak (cms) = 1.786

PEAK FLOW (cms) = 0.852 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 18.344
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.293

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9218)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2182):	82.95	0.852	7.00	18.34
+ ID2= 2 (5218):	69.30	0.884	9.75	32.72
ID = 3 (9218):	152.25	1.429	9.25	24.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8158)

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (6218):	791.10	11.904	7.50	33.01
+ ID2= 2 (9218):	152.25	1.429	9.25	24.89
ID = 3 (8158):	943.35	12.889	7.50	31.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8151)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8150):	15633.75	86.880	9.00	22.05
+ ID2= 2 (8158):	943.35	12.889	7.50	31.70
ID = 3 (8151):	16577.10	96.597	8.75	22.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5504)
 IN= 2--> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	33.0100	0.6700
2.6400	0.0500	37.4800	0.7800
4.8300	0.0900	47.5400	1.0100
7.4100	0.1300	60.2300	1.2800
10.3300	0.1900	71.9530	1.5258
13.5300	0.2500	*****	3.0529
16.9900	0.3200	*****	5.3299
20.6800	0.3900	*****	8.4148
24.6000	0.4800	*****	11.7637
28.7100	0.5700	*****	11.7737

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (8151)	*****	96.597	8.75	22.60
OUTFLOW: ID= 1 (5504)	*****	96.554	8.75	22.60

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 99.96
 TIME SHIF T OF PEAK FLOW (mi n) = 0.00
 MAXI MUM STORAGE USED (ha. m.) = 2.0497

ROUTE CHN (6220)
 IN= 2--> OUT= 1

Routing time step (mi n)' = 15.00

<----- DATA FOR SECTION (2201.0) ----->			
Di stance	El evati on	Manni ng	
0.00	245.59	0.0600	
5.76	245.45	0.0600	
23.03	242.27	0.0600	
57.58	232.35	0.0600	
86.37	226.18	0.0600	
109.41	224.31	0.0600	
115.17	223.86	0.0600	
120.92	222.78	0.0600	
126.68	221.93	0.0600	
128.06	221.50	0.0600 /0.0300	Main Channel
129.56	220.00	0.0300	Main Channel
141.56	220.00	0.0300	Main Channel
143.06	221.50	0.0300 /0.0600	Main Channel
149.72	223.77	0.0600	
155.47	225.09	0.0600	
253.36	229.21	0.0600	
333.98	227.50	0.0600	
454.90	229.25	0.0600	
500.97	233.11	0.0600	
570.07	237.42	0.0600	

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.75	220.75	.262E+05	5.5	0.57	79.44
1.50	221.50	.535E+05	17.5	0.86	52.89
2.44	222.44	.103E+06	42.7	1.14	40.03
3.37	223.37	.171E+06	79.4	1.27	35.98
4.31	224.31	.263E+06	128.1	1.33	34.22

5.25	225.25	.393E+06	191.7	1.34	34.19
6.18	226.18	.594E+06	276.7	1.28	35.77
7.12	227.12	.872E+06	399.9	1.26	36.33
8.06	228.06	.127E+07	542.7	1.17	38.92
8.99	228.99	.199E+07	770.0	1.06	43.01
9.93	229.93	.297E+07	1151.8	1.06	42.95
10.86	230.86	.400E+07	1652.9	1.13	40.33
11.80	231.80	.507E+07	2251.8	1.22	37.53
12.74	232.74	.618E+07	2946.5	1.31	34.97
13.67	233.67	.733E+07	3726.8	1.39	32.79
14.61	234.61	.853E+07	4591.0	1.48	30.96
15.55	235.55	.977E+07	5544.1	1.55	29.37
16.48	236.48	.111E+08	6586.7	1.63	27.99
17.42	237.42	.124E+08	7719.2	1.71	26.77

		<---- hydrograph ---->			<- pi pe / channel ->	
	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (5504)	*****	96.55	8.75	22.60	3.70	1.29
OUTFLOW: ID= 1 (6220)	*****	93.55	9.25	22.60	3.65	1.29

CALIB STANDHYD (2207)	Area (ha)= 57.80	Dir. Conn. (%)= 8.00
ID= 1 DT=15.0 min	Total Imp(%)= 21.00	

Surface Area (ha)=	12.14	IMPERVIOUS	PERVIOUS (i)	45.66
Dep. Storage (mm)=	2.00			5.00
Average Slope (%)=	0.50			0.50
Length (m)=	620.75			40.00
Mannings n =	0.013			0.250
Max. Eff. Inten. (mm/hr)=	82.76			58.56
over (min)=	15.00			30.00
Storage Coeff. (min)=	10.15 (ii)			23.40 (ii)
Unit Hyd. Tpeak (min)=	15.00			30.00
Unit Hyd. peak (cms)=	0.09			0.04
TOTALS				
PEAK FLOW (cms)=	0.89			3.827 (iii)
TIME TO PEAK (hrs)=	6.00			6.25
RUNOFF VOLUME (mm)=	60.70			34.38
TOTAL RAINFALL (mm)=	62.70			62.70
RUNOFF COEFFICIENT =	0.97			0.55

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 82.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9278)	IN= 2--> OUT= 1	DT= 15.0 min	
OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.5000	2.5000
0.2000	0.3000	0.6000	3.5000
0.3000	0.9000	0.7000	4.5000
0.4000	2.0000	0.8000	5.5000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2207)	57.800	3.827	6.25
OUTFLOW: ID= 1 (9278)	57.800	0.339	8.75
PEAK FLOW REDUCTION [Qout/Qin] (%)=	8.86		
TIME SHIFT OF PEAK FLOW (min)=	150.00		
MAXIMUM STORAGE USED (ha. m.)=	1.3300		

CALIB STANDHYD (2204)	Area (ha)= 13.30	Dir. Conn. (%)= 70.00
ID= 1 DT=15.0 min	Total Imp(%)= 75.00	

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)=	9.98	3.33
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	297.77	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	82.76	55.82
over (min)=	15.00	30.00
Storage Coeff. (min)=	6.53 (ii)	20.04 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.10	0.05

TOTALS
 PEAK FLOW (cms)= 2.00 0.26 2.165 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 60.70 29.84 51.44
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.48 0.82

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9279)	IN= 2--> OUT= 1	DT= 15.0 min	
OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.6700	0.3747
0.0500	0.2376	0.8900	0.4238
0.2600	0.2854	1.1200	0.4591
0.4800	0.3287	1.3000	0.4937

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2204)	13.300	2.165	6.00
OUTFLOW: ID= 1 (9279)	13.300	0.628	6.25
PEAK FLOW REDUCTION [Qout/Qin] (%)=	28.98		
TIME SHIFT OF PEAK FLOW (min)=	15.00		
MAXIMUM STORAGE USED (ha. m.)=	0.3688		

ADD HYD (9255)	1 + 2 = 3
ID1= 1 (9278):	57.80 0.339 8.75 34.38
+ ID2= 2 (9279):	13.30 0.628 6.25 51.35
ID= 3 (9255):	71.10 0.914 6.50 37.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5220)	IN= 2--> OUT= 1	DT= 15.0 min	
OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.6050	1.0142
0.1320	0.4759	3.2710	1.1595
0.7550	0.5505	3.8080	1.3018
1.4050	0.7107	4.2080	1.3118
1.9490	0.8484	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9255)	71.100	0.914	6.50
OUTFLOW: ID= 1 (5220)	71.100	0.471	8.50
PEAK FLOW REDUCTION [Qout/Qin] (%)=	51.51		
TIME SHIFT OF PEAK FLOW (min)=	120.00		
MAXIMUM STORAGE USED (ha. m.)=	0.5168		

CALIB NASHYD (2202)	Area (ha)= 98.00	Curve Number (CN)= 74.3
---------------------	------------------	-------------------------

| ID= 1 DT=15.0 min | I a (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 1.50

Unit Hyd Qpeak (cms)= 1.115

PEAK FLOW (cms)= 0.764 (i)
 TIME TO PEAK (hrs)= 8.00
 RUNOFF VOLUME (mm)= 22.745
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.363

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2202):	98.00	0.764	8.00	22.74
+ ID2= 2 (5220):	71.10	0.471	8.50	37.54
ID = 3 (9220):	169.10	1.230	8.50	28.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6220):	16577.10	93.553	9.25	22.60
+ ID2= 2 (9220):	169.10	1.230	8.50	28.97
ID = 3 (8160):	16746.20	94.729	9.25	22.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2221)
 ID= 1 DT=15.0 min
 Area Total (ha)= 110.64
 Imp(%)= 37.00 Dir. Conn.(%)= 20.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 40.94 69.70
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 858.84 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 82.76 54.60
 over (min) 15.00 30.00
 Storage Coeff. (min)= 12.33 (ii) 25.95 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

TOTALS
 PEAK FLOW (cms)= 4.01 4.73 6.997 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 60.70 27.76 34.35
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.44 0.55

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.3 I a = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2224)
 ID= 1 DT=15.0 min
 Area Total (ha)= 43.20
 Imp(%)= 72.00 Dir. Conn.(%)= 66.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 31.10 12.10
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50

Length (m)= 536.66 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 82.76 56.88
 over (min) 15.00 30.00
 Storage Coeff. (min)= 9.30 (ii) 22.70 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.04

TOTALS
 PEAK FLOW (cms)= 5.65 0.91 6.239 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 60.70 30.06 50.28
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.48 0.80

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.0 I a = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9288) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	2.1600	1.1836
	0.1500	0.7428	2.8900	1.3455
	0.8400	0.8895	3.6300	1.4651
	1.5600	1.0342	4.2300	1.5823
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2224)	43.200	6.239	6.00	50.28
OUTFLOW: ID= 1 (9288)	43.200	1.968	6.50	50.25

PEAK FLOW REDUCTION [Qout/Qin] (%) = 31.55
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 1.1476

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2221):	110.64	6.997	6.00	34.35
+ ID2= 2 (9288):	43.20	1.968	6.50	50.25
ID = 3 (9261):	153.84	8.254	6.25	38.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5222) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	7.2210	4.0213
	0.4910	2.1348	9.0080	4.5380
	2.1830	2.4433	15.0420	4.9977
	3.9370	2.9883	15.4420	5.0077
	5.4550	3.4692	0.0000	0.0000
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9261)	153.840	8.254	6.25	38.82
OUTFLOW: ID= 1 (5222)	153.840	3.002	7.25	38.81

PEAK FLOW REDUCTION [Qout/Qin] (%) = 36.36
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 2.7086

CALIB STANDHYD (2222)
 ID= 1 DT=15.0 min
 Area Total (ha)= 65.69
 Imp(%)= 84.00 Dir. Conn.(%)= 45.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 55.18 10.51
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 661.77 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 82.76 229.98
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 10.54 (ii) 18.21 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.05

PEAK FLOW (cms)= 5.65 3.61
 TIME TO PEAK (hrs)= 6.00 6.25
 RUNOFF VOLUME (mm)= 60.70 43.88
 TOTAL RAINFALL (mm)= 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.70

TOTALS
 8.134 (iii)
 6.00
 51.45
 62.70
 0.82

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8164)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2222):	65.69	8.134	6.00	51.45
+ ID2= 2 (5222):	153.84	3.002	7.25	38.81
=====				
ID = 3 (8164):	219.53	8.341	6.00	42.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2242)
 ID= 1 DT=15.0 min

Area Total (ha)	Imp (%)	Dir. Conn. (%)
52.75	36.00	27.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	18.99	33.76
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	593.01	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 82.76 51.44
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 9.87 (ii) 23.83 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.04

TOTALS
 4.207 (iii)
 6.00
 37.52
 62.70
 0.60

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2244)
 ID= 1 DT=15.0 min

Area Total (ha)	Imp (%)	Dir. Conn. (%)
87.70	70.00	64.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	61.39	26.31
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	764.63	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 82.76 55.82
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 11.50 (ii) 25.00 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

TOTALS
 11.602 (iii)
 6.00
 49.59
 62.70
 0.79

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9287)
 IN= 2---> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	4.3900	2.3776
	0.3000	1.4858	5.8700	2.7087
	1.7000	1.7770	7.3700	2.9557
	3.1700	2.0741	8.5800	3.1980

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2244)	87.700	11.602	6.00	49.59
OUTFLOW: ID= 1 (9287)	87.700	3.889	6.50	49.58

PEAK FLOW REDUCTION [Qout/Qin] (%) = 33.52
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 2.2606

RESERVOIR (5224)
 IN= 2---> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	0.4470	0.1810
	0.0230	0.0884	0.5620	0.2047
	0.1300	0.1032	0.6540	0.2278
	0.2410	0.1297	1.0540	0.2378
	0.3350	0.1532	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9287)	87.700	3.889	6.50	49.58
OUTFLOW: ID= 1 (5224)	87.700	4.466	6.50	49.57

PEAK FLOW REDUCTION [Qout/Qin] (%) = 114.83
 TIME SHIFT OF PEAK FLOW (min) = 0.00
 MAXIMUM STORAGE USED (ha. m.) = 0.3547

**** WARNING : HYDROGRAPH PEAK WAS NOT REDUCED.
 CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.

ADD HYD (9224)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2242):	52.75	4.207	6.00	37.52
+ ID2= 2 (5224):	87.70	4.466	6.50	49.57
=====				
ID = 3 (9224):	140.45	6.595	6.50	45.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8166)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8164):	219.53	8.341	6.00	42.59

+ ID2= 2 (9224): 140.45 6.595 6.50 45.04
 ID= 3 (8166): 359.98 12.653 6.00 43.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6226)
 IN= 2---> OUT= 1 Routing time step (min)= 15.00

<----- DATA FOR SECTION (2261.0) ----->

Distance	Elevation	Manning		
0.00	245.35	0.0600		
8.27	242.72	0.0600		
16.54	240.01	0.0600		
31.02	235.31	0.0600		
41.36	233.14	0.0600		
90.99	227.73	0.0600		
93.06	227.50	0.0600		
95.13	227.27	0.0600		
95.83	227.00	0.0600		
100.83	226.50	0.0600 / 0.0350	Main Channel	
101.33	225.50	0.0350	Main Channel	
101.83	226.50	0.0350 / 0.0600	Main Channel	
105.83	227.00	0.0600		
107.54	227.39	0.0600		
109.60	227.62	0.0600		
111.67	227.86	0.0600		
146.83	231.76	0.0600		
173.71	234.77	0.0600		
186.12	238.23	0.0600		
202.67	238.35	0.0600		

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	226.00	.403E+03	0.0	0.16	330.59
1.00	226.50	.161E+04	0.1	0.26	208.26
1.70	227.20	.173E+05	1.5	0.27	195.72
2.39	227.89	.541E+05	5.9	0.35	152.27
3.09	228.59	.119E+06	16.0	0.43	124.08
3.79	229.29	.212E+06	33.8	0.51	104.65
4.49	229.99	.334E+06	61.2	0.59	90.97
5.18	230.68	.484E+06	99.7	0.66	80.88
5.88	231.38	.663E+06	151.0	0.73	73.13
6.58	232.08	.870E+06	216.5	0.80	66.95
7.27	232.77	.111E+07	297.5	0.87	61.91
7.97	233.47	.137E+07	398.1	0.94	57.29
8.67	234.17	.165E+07	518.9	1.01	53.12
9.36	234.86	.196E+07	658.9	1.08	49.59
10.06	235.56	.228E+07	827.0	1.17	46.02
10.76	236.26	.262E+07	1015.4	1.25	42.96
11.46	236.96	.296E+07	1221.2	1.33	40.41
12.15	237.65	.332E+07	1444.6	1.40	38.26
12.85	238.35	.368E+07	1598.2	1.40	38.42

<---- hydrograph ----> <-pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8166)	359.98	12.65	6.00	43.55	2.86	0.40
OUTFLOW: ID= 1 (6226)	359.98	4.85	7.50	43.54	2.23	0.33

CALIB NASHYD (2267)
 ID= 1 DT=15.0 min
 Area (ha)= 10.50 Curve Number (CN)= 82.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.47

Unit Hyd Qpeak (cms)= 0.383
 PEAK FLOW (cms)= 0.266 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 28.426
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.453

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9276)

IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.5300	0.1297
0.0400	0.0402	0.7100	0.1837
0.2000	0.0341	0.8900	0.2402
0.3800	0.0910	1.0300	0.2954

AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 INFLOW: ID= 2 (2267) 10.500 0.266 6.50 28.43
 OUTFLOW: ID= 1 (9276) 10.500 0.248 6.75 28.40

PEAK FLOW REDUCTION [Qout/Qin] (%) = 93.55
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 0.0496

**** ERROR : CHECK THE STORAGE-DISCHARGE TABLE.

CALIB STANDHYD (2264)
 ID= 1 DT=15.0 min

Area (ha)= 117.80
 Total Imp(%)= 75.00 Dir. Conn.(%)= 70.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 88.35 29.45
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 886.19 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 82.76 55.82
 over (min) 15.00 30.00
 Storage Coeff. (min)= 12.56 (ii) 26.07 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

TOTALS
 PEAK FLOW (cms)= 14.82 2.04 16.122 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 60.70 29.84 51.44
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.48 0.82

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9277)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.5000	8.0000
0.3000	2.0000	3.5000	10.0000
0.9000	5.0000	4.5000	13.0000
2.0000	7.5000	5.5000	14.0000

AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 INFLOW: ID= 2 (2264) 117.800 16.122 6.00 51.44
 OUTFLOW: ID= 1 (9277) 117.800 0.814 8.50 51.43

PEAK FLOW REDUCTION [Qout/Qin] (%) = 5.05
 TIME SHIFT OF PEAK FLOW (min)=150.00
 MAXIMUM STORAGE USED (ha. m.) = 4.5720

ADD HYD (9253)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9276):	10.50	0.248	6.75	28.40
+ ID2= 2 (9277):	117.80	0.814	8.50	51.43
ID= 3 (9253):	128.30	1.024	8.50	49.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5226)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	1.7530	0.6823
0.0890	0.3202	2.2010	0.7801
0.5080	0.3704	2.5620	0.8758
0.9450	0.4782	2.9620	0.8858
1.3110	0.5708	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9253)	128.300	1.024	8.50	49.54
OUTFLOW: ID= 1 (5226)	128.300	0.924	9.25	49.54

PEAK FLOW REDUCTION [Qout/Qin](%) = 90.25
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.4834

CALIB NASHYD (2262)
ID= 1 DT=15.0 min

Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
119.28	1.339	7.500	24.77
5.00	0.924	9.25	49.54
1.05			

Unit Hyd Opeak (cms) = 1.934

PEAK FLOW (cms) = 1.339 (i)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 24.769
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.395

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9226)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2262)	119.28	1.339	7.50	24.77
+ ID2= 2 (5226)	128.30	0.924	9.25	49.54
ID = 3 (9226)	247.58	2.200	8.00	37.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8168)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6226)	359.98	4.854	7.50	43.54
+ ID2= 2 (9226)	247.58	2.200	8.00	37.60
ID = 3 (8168)	607.56	6.977	7.75	41.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8162)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8160)	16746.20	94.729	9.25	22.66
+ ID2= 2 (8168)	607.56	6.977	7.75	41.12
ID = 3 (8162)	17353.76	100.392	9.25	23.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6228)
IN= 2--> OUT= 1

Routing time step (min) = 15.00

----- DATA FOR SECTION (2281.0) -----
Distance Elevation Manning

0.00	245.00	0.0600
18.60	244.74	0.0600
37.20	242.47	0.0600
93.01	233.71	0.0600
125.57	226.95	0.0600
213.93	221.63	0.0600
218.58	221.15	0.0600
223.23	220.45	0.0600
225.03	220.55	0.0600 / 0.0300
225.53	218.05	0.0300
232.53	218.05	0.0300
240.03	218.05	0.0300
241.03	220.55	0.0300 / 0.0600
246.48	221.00	0.0600
251.13	221.27	0.0600
255.78	221.22	0.0600
325.54	225.25	0.0600
367.40	228.91	0.0600
404.60	231.75	0.0600
460.41	245.00	0.0600

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
1.25	219.30	.338E+05	16.0	0.86	35.15
2.50	220.55	.696E+05	48.3	1.26	24.02
3.94	221.99	1.168E+06	116.0	1.26	24.14
5.38	223.43	.392E+06	247.5	1.15	26.37
6.81	224.86	.743E+06	472.6	1.16	26.20
8.25	226.30	.122E+07	821.6	1.23	24.67
9.69	227.74	.179E+07	1320.1	1.34	22.59
11.13	229.18	.243E+07	1965.7	1.47	20.58
12.57	230.62	.313E+07	2744.2	1.59	19.02
14.01	232.06	.390E+07	3683.4	1.72	17.66
15.44	233.49	.472E+07	4824.1	1.86	16.30
16.88	234.93	.557E+07	6092.4	1.99	15.23
18.32	236.37	.646E+07	7503.3	2.11	14.35
19.76	237.81	.739E+07	9061.2	2.23	13.59
21.20	239.25	.836E+07	10767.6	2.34	12.94
22.64	240.69	.937E+07	12624.8	2.45	12.37
24.07	242.12	.104E+08	14634.9	2.56	11.87
25.51	243.56	.115E+08	16762.8	2.65	11.45
26.95	245.00	.127E+08	18699.9	2.69	11.28

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VLS (m/s)
INFLOW: ID= 2 (8162)	100.39	9.25	23.31	3.61	1.26	
OUTFLOW: ID= 1 (6228)	98.38	10.00	23.31	3.57	1.26	

CALIB STANDHYD (2287)
ID= 1 DT=15.0 min

Area Total	(ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
130.50	23.00			

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	30.01	100.48
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	932.74	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	82.76	18.52
over (min)	15.00	45.00
Storage Coeff. (min)	12.95 (ii)	33.95 (ii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.08	0.03

	PEAK FLOW (cms)	TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
	2.32	6.00	60.70	62.70	0.97
	3.02	6.50	17.29	62.70	0.28
	3.613 (iii)	6.50	21.63	62.70	0.35

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9275)				
IN= 2--> OUT= 1				
DT= 15.0 mi n				
OUTFLOW	STORAGE	OUTFLOW	STORAGE	
(cms)	(ha. m.)	(cms)	(ha. m.)	
0.0000	0.0000	4.5000	3.5000	
0.8000	0.1000	6.0000	4.0000	
1.5000	2.0000	6.5000	4.5000	
2.5000	3.0000	7.0000	5.5000	
AREA	OPEAK	TPEAK	R. V.	
(ha)	(cms)	(hrs)	(mm)	
INFLOW: ID= 2 (2287)	130.500	3.613	6.50	21.63
OUTFLOW: ID= 1 (9275)	130.500	1.161	7.75	21.63
PEAK FLOW	REDUCTION	[Qout/Qi n] (%) = 32.14		
TIME SHI FT OF PEAK FLOW	USED	(mi n) = 75.00		
MAXIMUM STORAGE		(ha. m.) = 1.0832		

RESERVOIR (5228)				
IN= 2--> OUT= 1				
DT= 15.0 mi n				
OUTFLOW	STORAGE	OUTFLOW	STORAGE	
(cms)	(ha. m.)	(cms)	(ha. m.)	
0.0000	0.0000	6.7260	2.6184	
0.3410	1.2286	8.4460	2.9937	
1.9480	1.4214	9.8320	3.3609	
3.6280	1.8348	10.2320	3.3709	
5.0330	2.1904	0.0000	0.0000	
AREA	OPEAK	TPEAK	R. V.	
(ha)	(cms)	(hrs)	(mm)	
INFLOW: ID= 2 (9275)	130.500	1.161	7.75	21.63
OUTFLOW: ID= 1 (5228)	130.500	1.006	10.25	21.63
PEAK FLOW	REDUCTION	[Qout/Qi n] (%) = 86.60		
TIME SHI FT OF PEAK FLOW	USED	(mi n) = 150.00		
MAXIMUM STORAGE		(ha. m.) = 1.3084		

CALIB NASHYD (2282)				
ID= 1 DT=15.0 mi n				
Area	(ha)	Curve Number	(CN)	
180.03		61.9		
Ia	(mm)	# of Li near Res. (N)		
5.00		1.50		
U. H. Tp	(hrs)			
1.12				
Uni t Hyd Qpeak	(cms)	= 2.754		
PEAK FLOW	(cms)	= 1.175 (i)		
TIME TO PEAK	(hrs)	= 7.750		
RUNOFF VOLUME	(mm)	= 15.420		
TOTAL RAI NFALL	(mm)	= 62.700		
RUNOFF COEFFI CI ENT		= 0.246		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9228)				
1 + 2 = 3				
ID	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (2282):	180.03	1.175	7.75	15.42
+ ID2= 2 (5228):	130.50	1.006	10.25	21.63
ID = 3 (9228):	310.53	1.906	10.00	18.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8170)				
1 + 2 = 3				
ID	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6228):	17353.76	98.377	10.00	23.31
+ ID2= 2 (9228):	310.53	1.906	10.00	18.03
ID = 3 (8170):	17664.29	100.283	10.00	23.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2302)			
ID= 1 DT=15.0 mi n			
Area	(ha)	Dir. Conn. (%)	
Total	30.47	29.00	11.00
IMPERVIOUS PERVERIOUS (i)			
Surface Area	(ha)	8.84	21.63
Dep. Storage	(mm)	2.00	5.00
Average Slope	(%)	0.50	0.50
Length	(m)	450.70	40.00
Mannings n	=	0.013	0.250
Max. Eff. Inten. (mm/hr)	=	82.76	25.18
over (mi n)	=	15.00	30.00
Storage Coeff. (mi n)	=	8.37 (ii)	26.94 (ii)
Uni t Hyd. Tpeak (mi n)	=	15.00	30.00
Uni t Hyd. peak (cms)	=	0.09	0.04
PEAK FLOW	(cms)	0.68	1.08
TIME TO PEAK	(hrs)	6.00	6.25
RUNOFF VOLUME	(mm)	60.70	21.50
TOTAL RAI NFALL	(mm)	62.70	62.70
RUNOFF COEFFI CI ENT	=	0.97	0.34

TOTALS
1.350 (iii)
6.00
25.81
62.70
0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLI TTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVERIOUS LOSSES:
CN* = 66.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFI CI ENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2307)			
ID= 1 DT=15.0 mi n			
Area	(ha)	Dir. Conn. (%)	
Total	175.00	25.00	9.00
IMPERVIOUS PERVERIOUS (i)			
Surface Area	(ha)	43.75	131.25
Dep. Storage	(mm)	2.00	5.00
Average Slope	(%)	0.50	0.50
Length	(m)	1080.12	40.00
Mannings n	=	0.013	0.250
Max. Eff. Inten. (mm/hr)	=	82.76	50.19
over (mi n)	=	15.00	30.00
Storage Coeff. (mi n)	=	14.14 (ii)	28.24 (ii)
Uni t Hyd. Tpeak (mi n)	=	15.00	30.00
Uni t Hyd. peak (cms)	=	0.07	0.04
PEAK FLOW	(cms)	2.70	7.81
TIME TO PEAK	(hrs)	6.00	6.25
RUNOFF VOLUME	(mm)	60.70	26.73
TOTAL RAI NFALL	(mm)	62.70	62.70
RUNOFF COEFFI CI ENT	=	0.97	0.43

TOTALS
9.074 (iii)
6.25
29.79
62.70
0.48

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLI TTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVERIOUS LOSSES:
CN* = 75.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFI CI ENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9280)			
IN= 2--> OUT= 1			
DT= 15.0 mi n			
OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha. m.)	(cms)	(ha. m.)
0.0000	0.0000	8.7500	3.0604
0.5900	1.4722	11.7100	3.8736
3.3900	1.6096	14.7000	4.6556
6.3200	2.4308	17.1100	5.4201
AREA	OPEAK	TPEAK	R. V.

INFLOW : ID= 2 (2307) (ha) (cms) (hrs) (mm)
 OUTFLOW: ID= 1 (9280) 175.000 9.074 6.25 29.79
 175.000 4.908 6.75 29.79

PEAK FLOW REDUCTION [Qout/Qin] (%) = 54.09
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 2.0380

RESERVOIR (5230)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	8.6830	2.7782
0.4700	1.3193	10.7480	3.1570
2.5870	1.5374	12.3570	3.5304
5.1590	1.9534	12.7570	3.5404
6.7830	2.3333	0.0000	0.0000

INFLOW : ID= 2 (9280) AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 OUTFLOW: ID= 1 (5230) 175.000 4.908 6.75 29.79
 175.000 3.126 7.75 29.78

PEAK FLOW REDUCTION [Qout/Qin] (%) = 63.70
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 1.6414

ADD HYD (8172)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2302):	30.47	1.350	6.00	25.81
+ ID2= 2 (5230):	175.00	3.126	7.75	29.78
ID = 3 (8172):	205.47	3.372	7.75	29.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (2329)
 ID= 1 DT=15.0 min

Area (ha) = 1.80 Curve Number (CN) = 69.0
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 0.22

Unit Hyd Qpeak (cms) = 0.143

PEAK FLOW (cms) = 0.050 (i)
 TIME TO PEAK (hrs) = 6.000
 RUNOFF VOLUME (mm) = 17.517
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.279

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9269)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.0900	0.0300
0.0100	0.0136	0.1200	0.0386
0.0400	0.0146	0.1500	0.0471
0.0700	0.0234	0.1800	0.0554

INFLOW : ID= 2 (2329) AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 OUTFLOW: ID= 1 (9269) 1.800 0.050 6.00 17.52
 1.800 0.025 7.00 17.34

PEAK FLOW REDUCTION [Qout/Qin] (%) = 49.78
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 0.0142

CALIB STANDHYD (2327)
 ID= 1 DT=15.0 min

Area (ha) = 184.00 Dir. Conn. (%) = 8.00
 Total Imp(%) = 24.00

Storage Coeff. = 0.50
 IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 44.16 139.84
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 1107.55 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 82.76 23.19
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 14.36 (ii) 33.55 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.07 0.03

TOTALS

PEAK FLOW (cms) = 2.51 5.26 5.978 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.50
 RUNOFF VOLUME (mm) = 60.70 20.58 23.79
 TOTAL RAINFALL (mm) = 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.33 0.38

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9270)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.0000	4.0000
1.1000	0.3000	3.5000	4.5000
1.8000	1.8000	4.5000	6.0000
2.0000	2.5000	5.5000	8.0000

INFLOW : ID= 2 (2327) AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 OUTFLOW: ID= 1 (9270) 184.000 5.978 6.50 23.79
 184.000 1.804 7.75 23.79

PEAK FLOW REDUCTION [Qout/Qin] (%) = 30.18
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha. m.) = 1.8147

ADD HYD (9249)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9269):	1.80	0.025	7.00	17.34
+ ID2= 2 (9270):	184.00	1.804	7.75	23.79
ID = 3 (9249):	185.80	1.817	7.75	23.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5232)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	11.1130	4.3262
0.5630	2.0299	13.9540	4.9462
3.2190	2.3484	16.2450	5.5530
5.9950	3.0315	16.6450	5.5630
8.3150	3.6189	0.0000	0.0000

INFLOW : ID= 2 (9249) AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 OUTFLOW: ID= 1 (5232) 185.800 1.817 7.75 23.73
 185.800 1.455 10.75 23.72

PEAK FLOW REDUCTION [Qout/Qin] (%) = 80.12
 TIME SHIFT OF PEAK FLOW (min) = 180.00
 MAXIMUM STORAGE USED (ha. m.) = 2.1374

 CALIB (2322) Area (ha) = 129.00 Curve Number (CN) = 58.1
 NASHYD (2322) Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 ID= 1 DT=15.0 min U. H. Tp(hrs) = 1.00

Unit Hyd Qpeak (cms) = 2.209
 PEAK FLOW (cms) = 0.811 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 13.680
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.218

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (9232)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2322):	129.00	0.811	7.25	13.68
+ ID2= 2 (5232):	185.80	1.455	10.75	23.72

ID = 3 (9232):	314.80	1.965	10.50	19.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8173)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8172):	205.47	3.372	7.75	29.19
+ ID2= 2 (9232):	314.80	1.965	10.50	19.61

ID = 3 (8173):	520.27	4.489	7.75	23.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8174)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8170):	17664.29	100.283	10.00	23.22
+ ID2= 2 (8173):	520.27	4.489	7.75	23.39

ID = 3 (8174):	18184.56	102.956	10.00	23.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6234)
 IN= 2----> OUT= 1
 Routing time step (min) = 15.00

<----- DATA FOR SECTION (2341.0) ----->

Distance	Elevation	Manning	
0.00	228.00	0.0550	
42.86	223.21	0.0550	
85.72	219.56	0.0550	
107.15	219.42	0.0550	
128.58	219.39	0.0550	
214.30	219.30	0.0550	
225.02	219.26	0.0550	
235.73	219.24	0.0550	
255.16	219.20	0.0550 / 0.0350	Main Channel
257.16	217.20	0.0350	Main Channel
303.16	217.20	0.0350	Main Channel
305.16	219.20	0.0350 / 0.0550	Main Channel
310.74	219.28	0.0550	
321.45	219.27	0.0550	
332.17	219.26	0.0550	
396.46	219.29	0.0550	
782.20	222.91	0.0550	
878.64	227.43	0.0550	
964.36	227.80	0.0550	
1060.79	228.00	0.0550	

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	217.70	.887E+05	7.2	0.31	206.54
1.00	218.20	.179E+06	22.7	0.48	131.74
1.50	218.70	.272E+06	44.5	0.62	101.75
2.00	219.20	.366E+06	71.8	0.75	84.97
2.59	219.79	.993E+06	136.1	0.52	121.59
3.17	220.37	.189E+07	258.6	0.52	121.80
3.76	220.96	.294E+07	434.7	0.56	112.82
4.35	221.55	.415E+07	665.9	0.61	103.88
4.93	222.13	.551E+07	955.0	0.66	96.22
5.52	222.72	.703E+07	1305.1	0.71	89.80
6.11	223.31	.868E+07	1744.9	0.77	82.91
6.69	223.89	.104E+08	2261.4	0.83	76.46
7.28	224.48	.121E+08	2838.0	0.89	71.12
7.87	225.07	.139E+08	3472.9	0.95	66.63
8.45	225.65	.157E+08	4165.0	1.01	62.82
9.04	226.24	.176E+08	4913.5	1.07	59.54
9.63	226.83	.194E+08	5717.7	1.12	56.68
10.21	227.41	.214E+08	6577.1	1.17	54.18
10.80	228.00	.235E+08	6919.5	1.12	56.61

<---- hydrograph ----> <-pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8174)	*****	102.96	10.00	23.22	2.28	0.62
OUTFLOW: ID= 1 (6234)	*****	91.03	12.00	23.22	2.18	0.66

 CALIB (2347) Area (ha) = 266.10 Curve Number (CN) = 56.0
 NASHYD (2347) Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 ID= 1 DT=15.0 min U. H. Tp(hrs) = 2.00

Unit Hyd Qpeak (cms) = 2.276
 PEAK FLOW (cms) = 0.926 (i)
 TIME TO PEAK (hrs) = 9.250
 RUNOFF VOLUME (mm) = 12.894
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.206

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR (9281)
 IN= 2----> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	13.3100	4.0635
0.9000	1.7152	17.8000	5.3537
5.1600	1.7685	22.3500	6.6446
9.6100	3.0976	26.0300	7.9060

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2347)	266.100	0.926	9.25	12.89
OUTFLOW: ID= 1 (9281)	266.100	0.618	14.00	12.89

PEAK FLOW REDUCTION [Qout/Oin] (%) = 66.77
 TIME SHIFT OF PEAK FLOW (min) = 285.00
 MAXIMUM STORAGE USED (ha. m.) = 1.1784

 RESERVOIR (5234)
 IN= 2----> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.5630	0.1745
0.1300	0.0819	0.6550	0.1995
0.2420	0.0947	0.7760	0.2240
0.3350	0.1223	1.0550	0.2340
0.4480	0.1460	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9281)	266.100	0.618	14.00	12.89
OUTFLOW: ID= 1 (5234)	266.100	0.613	14.75	12.89

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 99.10
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 0.1880

CALIB STANDHYD (2342)
 ID= 1 DT=15.0 min

Area Total	(ha) = 1.06	Dir. Conn. (%) = 8.00
Imp (%)	= 22.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 0.23	0.83
Dep. Storage (mm)	= 1.00	5.00
Average Slope (%)	= 1.00	2.00
Length (m)	= 84.06	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr) over (min)	= 82.76 / 15.00	= 9.10 / 30.00
Storage Coeff. (min)	= 2.48 (ii)	= 20.89 (ii)
Unit Hyd. Tpeak (min)	= 15.00	= 30.00
Unit Hyd. peak (cms)	= 0.11	= 0.05

TOTALS (iii)

PEAK FLOW (cms)	= 0.02	0.02	0.030
TIME TO PEAK (hrs)	= 6.00	6.25	6.00
RUNOFF VOLUME (mm)	= 61.70	8.77	12.99
TOTAL RAINFALL (mm)	= 62.70	62.70	62.70
RUNOFF COEFFICIENT	= 0.98	0.14	0.21

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 39.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9234) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2342):	1.06	0.030	6.00	12.99
+ ID2= 2 (5234):	266.10	0.613	14.75	12.89
=====				
ID = 3 (9234):	267.16	0.613	14.75	12.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8176) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6234):	18184.56	91.030	12.00	23.22
+ ID2= 2 (9234):	267.16	0.613	14.75	12.89
=====				
ID = 3 (8176):	18451.72	91.572	12.00	23.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2369)
 ID= 1 DT=15.0 min

Area Total	(ha) = 441.90	Dir. Conn. (%) = 8.00
Imp (%)	= 23.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 101.64	340.26
Dep. Storage (mm)	= 2.00	5.00
Average Slope (%)	= 0.50	0.50
Length (m)	= 1716.39	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr) over (min)	= 82.76 / 15.00	= 24.91 / 45.00
Storage Coeff. (min)	= 18.68 (ii)	= 37.33 (ii)
Unit Hyd. Tpeak (min)	= 15.00	= 45.00
Unit Hyd. peak (cms)	= 0.06	= 0.03

TOTALS

PEAK FLOW (cms) = 5.36 12.99 14.951 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.50
 RUNOFF VOLUME (mm) = 60.70 22.23 25.31
 TOTAL RAINFALL (mm) = 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.35 0.40

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9266)
 IN= 2 ---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.3000	8.0000
0.2000	2.0000	3.5000	10.0000
1.9000	7.0000	4.5000	13.0000
2.0000	7.5000	5.5000	14.0000

INFLOW : ID= 2 (2369)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (9266)	441.900	14.951	6.50	25.31
	441.900	2.047	9.25	25.30

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 13.69
 TIME SHIFT OF PEAK FLOW (min) = 165.00
 MAXIMUM STORAGE USED (ha. m.) = 7.5793

RESERVOIR (5236)
 IN= 2 ---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.1380	1.5897
1.0220	0.7542	3.7950	2.0204
1.7530	0.8200	4.4970	2.4373
2.3290	1.2567	0.0000	0.0000

INFLOW : ID= 2 (9266)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5236)	441.900	2.047	9.25	25.30
	441.900	1.937	11.50	25.30

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 94.62
 TIME SHIFT OF PEAK FLOW (min) = 135.00
 MAXIMUM STORAGE USED (ha. m.) = 0.9597

CALIB NASHYD (2362)
 ID= 1 DT=15.0 min

Area Total	(ha) = 52.59	Curve Number (CN) = 58.7
Ia (mm)	= 5.00	# of Li near Res. (N) = 1.50
U. H. Tp (hrs)	= 1.24	

Unit Hyd Opeak (cms)	= 0.726
PEAK FLOW (cms)	= 0.288 (i)
TIME TO PEAK (hrs)	= 8.000
RUNOFF VOLUME (mm)	= 13.978
TOTAL RAINFALL (mm)	= 62.700
RUNOFF COEFFICIENT	= 0.223

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9236) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2362):	52.59	0.288	8.00	13.98
+ ID2= 2 (5236):	441.90	1.937	11.50	25.30
=====				
ID = 3 (9236):	494.49	2.137	10.50	24.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6238)
IN= 2--> OUT= 1
Routing time step (min) = 15.00

----- DATA FOR SECTION (2381.0) -----

Distance	Elevation	Manning	
0.00	241.54	0.0380	
602.55	227.00	0.0380	
1702.00	224.50	0.0380	
1721.25	223.00	0.0380	
1725.10	222.60	0.0380	
1780.94	222.50	0.0380	
1782.87	222.45	0.0380	
1784.79	222.40	0.0380 / 0.0300	Main Channel
1786.57	221.75	0.0300	Main Channel
1787.07	220.75	0.0300	Main Channel
1787.57	220.75	0.0300	Main Channel
1791.57	220.75	0.0300	Main Channel
1794.07	221.75	0.0300	Main Channel
1798.27	222.00	0.0300	Main Channel
1800.19	222.35	0.0300 / 0.0380	Main Channel
1802.12	222.47	0.0380	
1840.63	223.00	0.0380	
1900.00	225.00	0.0380	
1950.00	226.00	0.0380	
2242.61	240.00	0.0380	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.80	221.55	.101E+05	5.2	1.14	32.36
1.60	222.35	.296E+05	18.7	1.40	26.36
2.64	223.39	.249E+06	151.8	1.35	27.33
3.68	224.43	.613E+06	512.2	1.85	19.94
4.71	225.46	.152E+07	1066.8	1.55	23.80
5.75	226.50	.359E+07	2824.9	1.75	21.16
6.79	227.54	.663E+07	6528.0	2.18	16.93
7.83	228.58	.994E+07	12158.6	2.71	13.62
8.87	229.62	.134E+08	19268.8	3.19	11.58
9.91	230.66	.170E+08	27785.3	3.63	10.19
10.94	231.69	.207E+08	37667.1	4.03	9.18
11.98	232.73	.246E+08	48891.7	4.40	8.40
13.02	233.77	.287E+08	61448.0	4.75	7.78
14.06	234.81	.329E+08	75332.7	5.08	7.28
15.10	235.85	.372E+08	90547.8	5.39	6.85
16.14	236.89	.417E+08	*****	5.69	6.49
17.17	237.92	.464E+08	*****	5.98	6.18
18.21	238.96	.512E+08	*****	6.25	5.91
19.25	240.00	.561E+08	*****	6.52	5.67

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (9236)	494.49	2.14	10.50	24.10	0.33	1.14
OUTFLOW: ID= 1 (6238)	494.49	2.13	11.50	24.10	0.33	1.14

CALIB STANDHYD (2389)
ID= 1 DT=15.0 min
Area (ha)= 4.70
Total Imp(%)= 21.00
Dir. Conn.(%)= 7.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.99	3.71
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	177.01	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	82.76	24.25
over (min)	15.00	30.00
Storage Coeff. (min)=	4.78 (ii)	23.63 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.11	0.04
PEAK FLOW (cms)=	0.07	0.19
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	60.70	21.99
TOTAL RAINFALL (mm)=	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.35

TOTALS
0.203 (iii)
6.25
24.69
62.70
0.39

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9267)
IN= 2--> OUT= 1
DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	0.2300	0.0785
	0.0200	0.0364	0.3100	0.1005
	0.0900	0.0393	0.3900	0.1220
	0.1700	0.0616	0.4600	0.1429

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2389)	4.700	0.203	6.25	24.69
OUTFLOW: ID= 1 (9267)	4.700	0.114	6.75	24.60

PEAK FLOW REDUCTION [Qout/Qin] (%) = 56.33
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 0.0467

CALIB NASHYD (2387)
ID= 1 DT=15.0 min

Area (ha)= 90.20
Ia (mm)= 5.00
U. H. Tp(hrs)= 0.86
Curve Number (CN)= 54.0
of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 1.791
PEAK FLOW (cms)= 0.556 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 11.992
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.191

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9268)
IN= 2--> OUT= 1
DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	4.5100	1.3564
	0.3100	0.5624	6.0400	1.7959
	1.7500	0.5748	7.5800	2.2374
	3.2600	1.0285	8.8300	2.6688

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2387)	90.200	0.556	7.00	11.99
OUTFLOW: ID= 1 (9268)	90.200	0.269	11.00	11.99

PEAK FLOW REDUCTION [Qout/Qin] (%) = 48.32
TIME SHIFT OF PEAK FLOW (min)=240.00
MAXIMUM STORAGE USED (ha. m.) = 0.4874

ADD HYD (9247)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9267):	4.70	0.114	6.75	24.60
+ ID2= 2 (9268):	90.20	0.269	11.00	11.99
-----	-----	-----	-----	-----
ID = 3 (9247):	94.90	0.287	11.00	12.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5238)
IN= 2--> OUT= 1

DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	3.8780	1.5095
	0.1960	0.7083	4.8600	1.7258
	1.1230	0.8194	5.6680	1.9375
	2.0920	1.0578	6.0680	1.9475
	2.9010	1.2627	0.0000	0.0000
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (9247)	94.900	0.287	11.00	12.61
OUTFLOW: ID= 1 (5238)	94.900	0.151	17.00	12.60
PEAK FLOW REDUCTION [Qout/Qin](%)= 52.68				
TIME SHIFT OF PEAK FLOW (min)=360.00				
MAXIMUM STORAGE USED (ha.m.)= 0.5469				

CALIB NASHYD (2382)	Area (ha)	(ha)= 216.80	Curve Number (CN)= 56.8
ID= 1 DT=15.0 min	Ia (mm)	= 5.00	# of Linear Res. (N)= 1.50
	U.H. Tp(hrs)	= 1.56	

Unit Hyd Opeak (cms)= 2.368
 PEAK FLOW (cms)= 0.934 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 13.201
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.211

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9238)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (2382):	216.80	0.934	8.25	13.20
+ ID2= 2 (5238):	94.90	0.151	17.00	12.60
ID = 3 (9238):	311.70	0.974	8.50	13.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8180)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (6238):	494.49	2.125	11.50	24.10
+ ID2= 2 (9238):	311.70	0.974	8.50	13.02
ID = 3 (8180):	806.19	3.009	10.50	19.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8178)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8176):	18451.72	91.572	12.00	23.07
+ ID2= 2 (8180):	806.19	3.009	10.50	19.81
ID = 3 (8178):	19257.91	94.494	12.00	22.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6240)	Routing time step (min)'= 15.00	
IN= 2----> OUT= 1		
----- DATA FOR SECTION (2401.0) -----		
Distance	Elevation	Manning
0.00	222.00	0.0450
11.46	221.00	0.0450
208.98	221.00	0.0450
404.04	220.97	0.0450

808.08	220.83	0.0450
905.60	220.17	0.0450
919.53	219.43	0.0450
933.47	219.22	0.0450
945.26	219.21	0.0450 / 0.0300
946.26	217.81	0.0300
975.26	217.81	0.0300
1003.26	217.81	0.0300
1005.26	219.21	0.0300 / 0.0450
1017.06	219.28	0.0450
1030.99	219.26	0.0450
1044.92	219.23	0.0450
1058.86	219.23	0.0450
1253.91	219.22	0.0450
1323.57	221.05	0.0450
1379.30	222.00	0.0450

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.20	218.01	.307E+05	2.2	0.20	227.52
0.40	218.21	.617E+05	7.1	0.31	143.95
0.60	218.41	.928E+05	14.0	0.41	110.33
0.80	218.61	.124E+06	22.6	0.49	91.46
1.00	218.81	.156E+06	32.8	0.57	79.15
1.20	219.01	.188E+06	44.5	0.64	70.38
1.40	219.21	.220E+06	57.5	0.70	63.77
1.63	219.44	.413E+06	82.5	0.54	83.52
1.87	219.68	.632E+06	120.9	0.51	87.03
2.10	219.91	.858E+06	170.0	0.53	84.11
2.33	220.14	.109E+07	228.7	0.56	79.62
2.56	220.37	.134E+07	294.4	0.59	76.03
2.80	220.61	.162E+07	371.1	0.61	72.75
3.03	220.84	.192E+07	456.8	0.64	70.18
3.26	221.07	.251E+07	552.1	0.59	75.74
3.49	221.30	.333E+07	711.8	0.57	78.07
3.73	221.54	.417E+07	907.0	0.58	76.62
3.96	221.77	.502E+07	1134.1	0.61	73.70
4.19	222.00	.587E+07	1390.9	0.64	70.34

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
806.19	0.974	8.50	13.02	1.71	0.53
94.90	0.151	17.00	12.60	1.66	0.53
311.70	0.974	8.50	13.02		

CALIB NASHYD (0240)	Area (ha)	(ha)= 345.47	Curve Number (CN)= 46.0
ID= 1 DT=15.0 min	Ia (mm)	= 5.00	# of Linear Res. (N)= 1.50
	U.H. Tp(hrs)	= 3.60	

Unit Hyd Opeak (cms)= 1.637
 PEAK FLOW (cms)= 0.554 (i)
 TIME TO PEAK (hrs)= 12.000
 RUNOFF VOLUME (mm)= 9.341
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.149

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2407)	Area (ha)	(ha)= 88.90	Di r. Conn. (%)= 7.00
ID= 1 DT=15.0 min	Total Imp(%)= 20.00		

Surface Area (ha)	17.78	71.12
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	769.85	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	82.76	20.84
over (min)	15.00	45.00
Storage Coeff. (min)	11.54 (ii)	31.57 (ii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.08	0.03

PEAK FLOW (cms)= 1.15 2.48 *TOTALS* 2.750 (iii)

TIME TO PEAK (hrs)= 6.00 6.50 6.50
 RUNOFF VOLUME (mm)= 60.70 19.37 22.26
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.31 0.36

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9273)
 IN= 2----> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.4500	5.4723
0.3000	4.6746	5.9500	5.8930
1.7300	4.7225	7.4700	6.3047
3.2100	5.1511	8.7000	6.7071

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (2407)	88.900	2.750	6.50	22.26
OUTFLOW: ID= 1 (9273)	88.900	0.111	12.50	22.22

PEAK FLOW REDUCTION [Out/Oi n] (%) = 4.05
 TIME SHIFT OF PEAK FLOW (min) = 360.00
 MAXIMUM STORAGE USED (ha.m.) = 1.7372

ADD HYD (9252)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0240):	345.47	0.554	12.00	9.34
+ ID2= 2 (9273):	88.90	0.111	12.50	22.22
ID = 3 (9252):	434.37	0.665	12.00	11.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8182)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6240):	19257.91	86.928	13.50	22.94
+ ID2= 2 (9252):	434.37	0.665	12.00	11.98
ID = 3 (8182):	19692.28	87.562	13.50	22.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (2427)
 ID= 1 DT=15.0 min

Area (ha)	Curve Number (CN)
Ia (mm) = 5.00	(CN) = 74.0
U.H. Tp(hrs) = 0.37	# of Linear Res. (N) = 1.50

Unit Hyd Qpeak (cms) = 0.249

PEAK FLOW (cms) = 0.119 (i)
 TIME TO PEAK (hrs) = 6.500
 RUNOFF VOLUME (mm) = 21.658
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.345

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9274)
 IN= 2----> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
---------------	-----------------	---------------	-----------------

0.0000	0.0000	0.2700	0.0857
0.0200	0.0379	0.3600	0.1115
0.1000	0.0399	0.4500	0.1369
0.1900	0.0663	0.5300	0.1618

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (2427)	5.400	0.119	6.50	66
OUTFLOW: ID= 1 (9274)	5.400	0.102	7.00	21.57

PEAK FLOW REDUCTION [Out/Oi n] (%) = 85.82
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0409

RESERVOIR (5242)
 IN= 2----> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0480	1.0185
0.0020	1.0087	0.0600	1.0212
0.0140	1.0101	0.0700	1.0238
0.0260	1.0130	0.4700	1.0338
0.0360	1.0155	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (9274)	5.400	0.102	7.00	57
OUTFLOW: ID= 1 (5242)	5.400	0.000	35.50	0.37

PEAK FLOW REDUCTION [Out/Oi n] (%) = 0.22
 TIME SHIFT OF PEAK FLOW (min) = *****
 MAXIMUM STORAGE USED (ha.m.) = 0.1145

CALIB NASHYD (2422)
 ID= 1 DT=15.0 min

Area (ha)	Curve Number (CN)
Ia (mm) = 5.00	(CN) = 51.6
U.H. Tp(hrs) = 5.37	# of Linear Res. (N) = 1.50

Unit Hyd Qpeak (cms) = 2.073

PEAK FLOW (cms) = 0.889 (i)
 TIME TO PEAK (hrs) = 13.000
 RUNOFF VOLUME (mm) = 11.240
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.179

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9242)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (2422):	652.48	0.889	13.00	11.24
+ ID2= 2 (5242):	5.40	0.000	35.50	0.37
ID = 3 (9242):	657.88	0.889	13.00	11.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8184)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8182):	19692.28	87.562	13.50	22.69
+ ID2= 2 (9242):	657.88	0.889	13.00	11.15
ID = 3 (8184):	20350.16	88.449	13.50	22.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6244)
 IN= 2----> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (2441.0) ----->

Distance	Elevation	Manning
0.00	225.00	0.0450
2.50	220.10	0.0450
3.00	220.06	0.0450
3.64	220.07	0.0450
7.28	220.08	0.0450
10.91	220.09	0.0450
14.55	219.81	0.0450
18.19	219.29	0.0450
24.13	219.21	0.0450 / 0.0300
24.63	217.81	0.0300
69.13	217.81	0.0300
114.13	217.81	0.0300
115.13	219.21	0.0300 / 0.0450
120.06	219.25	0.0450
123.70	219.32	0.0450
127.34	219.77	0.0450
130.98	219.88	0.0450
134.61	219.99	0.0450
350.00	220.18	0.0450
360.18	225.00	0.0450

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.35	218.16	.108E+06	9.0	0.29	200.52
0.70	218.51	.216E+06	28.4	0.45	126.89
1.05	218.86	.325E+06	55.7	0.59	97.27
1.40	219.21	.434E+06	89.7	0.71	80.65
1.79	219.60	.574E+06	135.9	0.81	70.36
2.17	219.98	.726E+06	190.8	0.90	63.38
2.56	220.37	.111E+07	264.3	0.82	69.82
2.94	220.75	.157E+07	371.7	0.81	70.36
3.33	221.14	.203E+07	503.4	0.85	67.30
3.72	221.53	.250E+07	656.6	0.90	63.40
4.10	221.91	.296E+07	829.6	0.96	59.54
4.49	222.30	.343E+07	1021.1	1.02	56.00
4.87	222.68	.390E+07	1230.2	1.08	52.83
5.26	223.07	.437E+07	1456.2	1.14	50.01
5.65	223.46	.484E+07	1698.4	1.20	47.51
6.03	223.84	.531E+07	1956.3	1.26	45.28
6.42	224.23	.579E+07	2229.3	1.32	43.28
6.80	224.61	.626E+07	2517.0	1.38	41.48
7.19	225.00	.674E+07	2819.2	1.44	39.85

----- hydrograph ----- <- pi pe / channel ->

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8184)	88.45	13.50	22.32	1.39	0.70
OUTFLOW: ID= 1 (6244)	84.54	14.00	22.32	1.35	0.69

----- CALIB STANDHYD (2441) -----

Area Total	(ha)= 91.52	Dir. Conn. (%) = 15.00
Imp (%) = 45.00		

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	41.18	50.34
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	781.11	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (mi n)	82.76	50.26
Storage Coeff. (mi n)	11.65 (ii)	25.73 (ii)
Unit Hyd. Tpeak (mi n)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04

TOTALS

PEAK FLOW (cms)	2.54	3.14	4.498 (iii)
TIME TO PEAK (hrs)	6.00	6.25	6.00
RUNOFF VOLUME (mm)	60.70	21.43	27.32
TOTAL RAINFALL (mm)	62.70	62.70	62.70
RUNOFF COEFFICIENT	0.97	0.34	0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 60.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB STANDHYD (2449) -----

Area Total	(ha)= 143.20	Dir. Conn. (%) = 17.00
Imp (%) = 34.00		

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	48.69	94.51
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	977.07	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) over (mi n)	82.76	44.43
Storage Coeff. (mi n)	13.32 (ii)	28.12 (ii)
Unit Hyd. Tpeak (mi n)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04

TOTALS

PEAK FLOW (cms)	4.28	4.98	7.374 (iii)
TIME TO PEAK (hrs)	6.00	6.25	6.00
RUNOFF VOLUME (mm)	60.70	23.07	29.47
TOTAL RAINFALL (mm)	62.70	62.70	62.70
RUNOFF COEFFICIENT	0.97	0.37	0.47

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- RESERVOIR (9265) -----

IN= 2 -> OUT= 1	DT= 15.0 mi n	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
		0.0000	0.0000	7.1600	2.7857
		0.4900	1.4537	9.5800	3.4259
		2.7800	1.6401	12.0300	4.0179
		5.1700	2.2743	14.0100	4.5968

INFLOW : ID= 2 (2449)	143.200	7.374	6.00	29.47
OUTFLOW: ID= 1 (9265)	143.200	3.637	6.75	29.46

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 49.32
 TIME SHIFT OF PEAK FLOW (mi n) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 1.8761

----- ADD HYD (9245) -----

ID1= 1 (2441):	91.52	4.498	6.00	27.32
+ ID2= 2 (9265):	143.20	3.637	6.75	29.46
ID = 3 (9245):	234.72	6.303	6.50	28.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- RESERVOIR (5244) -----

IN= 2 -> OUT= 1	DT= 15.0 mi n	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
		0.0000	0.0000	15.7000	11.1120
		0.7950	7.8678	19.7140	11.9879
		4.5470	8.3178	22.9500	12.8451
		8.4690	9.2829	23.3500	12.8551
		11.7470	10.1128	0.0000	0.0000

AREA OPEAK TPEAK R.V.

RUNOFF VOLUME (mm) = 15.061
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.240

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (4461) | Area (ha) = 236.12
 ID= 1 DT=15.0 min | Total Imp(%) = 47.00 | Di r. Conn. (%) = 17.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	110.98	125.14
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1254.64	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) over (min)	82.76	51.44
Storage Coeff. (min)	15.47 (ii)	29.43 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.07	0.04

			TOTALS
PEAK FLOW (cms)	6.63	7.45	11.263 (iii)
TIME TO PEAK (hrs)	6.00	6.25	6.00
RUNOFF VOLUME (mm)	60.70	21.64	28.28
TOTAL RAINFALL (mm)	62.70	62.70	62.70
RUNOFF COEFFICIENT	0.97	0.35	0.45

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 60.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5446) IN= 2 --> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	10.1800	6.2621
	0.8000	2.9957	12.5050	7.1218
	3.0350	3.4813	14.4090	7.9633
	5.6510	4.4353	14.8090	7.9733
	7.7470	5.2683	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (4461)	236.120	11.263	6.00	28.28
OUTFLOW: ID= 1 (5446)	236.120	3.433	7.25	28.28

PEAK FLOW REDUCTION [Qout/Qin] (%) = 30.48
 TIME SHIF T OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha.m.) = 3.6454

ADD HYD (8192) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4462):	1238.89	2.928	12.00	15.06
+ ID2= 2 (5446):	236.12	3.433	7.25	28.28
ID = 3 (8192):	1475.01	5.226	7.25	17.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8194) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8188):	42609.04	174.186	14.00	22.09
+ ID2= 2 (8192):	1475.01	5.226	7.25	17.18
ID = 3 (8194):	44084.05	177.611	14.00	21.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0442) | Area (ha) = 117.26 | Curve Number (CN) = 43.0
 ID= 1 DT=15.0 min | Ia (mm) = 5.00 | # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 1.17

Unit Hyd Qpeak (cms) = 1.705

PEAK FLOW (cms) = 0.395 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 8.373
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.134

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0440) | Area (ha) = 226.35 | Curve Number (CN) = 60.0
 ID= 1 DT=15.0 min | Ia (mm) = 5.00 | # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 1.11

Unit Hyd Qpeak (cms) = 3.481

PEAK FLOW (cms) = 1.396 (i)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 14.536
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.232

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0438) | Area (ha) = 130.70 | Curve Number (CN) = 51.0
 ID= 1 DT=15.0 min | Ia (mm) = 5.00 | # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 0.60

Unit Hyd Qpeak (cms) = 3.738

PEAK FLOW (cms) = 0.963 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 10.792
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.172

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0436) | Area (ha) = 187.51 | Curve Number (CN) = 55.0
 ID= 1 DT=15.0 min | Ia (mm) = 5.00 | # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 0.73

Unit Hyd Qpeak (cms) = 4.391

PEAK FLOW (cms) = 1.356 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 12.334
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.197

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0434) | Area (ha) = 56.64 | Curve Number (CN) = 46.0
 ID= 1 DT=15.0 min | Ia (mm) = 5.00 | # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 0.52

Unit Hyd Qpeak (cms) = 1.867

PEAK FLOW (cms) = 0.394 (i)
 TIME TO PEAK (hrs) = 6.500
 RUNOFF VOLUME (mm) = 9.103
 TOTAL RAINFALL (mm) = 62.700

RUNOFF COEFFICIENT = 0.145

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0434):	56.64	0.394	6.50	9.10
+ ID2= 2 (0436):	187.51	1.356	7.00	12.33
ID = 3 (8206):	244.15	1.736	6.75	11.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6438)
IN= 2---> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (4381.0) ----->

Distance	Elevation	Manning	
0.00	256.00	0.0600	
9.23	255.50	0.0600	
27.69	255.00	0.0600	
50.77	254.90	0.0600	
64.62	254.85	0.0600	
78.47	252.26	0.0600	
96.93	249.44	0.0600	
129.24	245.94	0.0600	
145.40	245.76	0.0600	
146.20	245.80	0.0600 / 0.0350	Main Channel
146.70	245.30	0.0350	Main Channel
147.70	245.30	0.0350	Main Channel
148.20	245.80	0.0350 / 0.0600	Main Channel
150.01	245.78	0.0600	
152.32	245.79	0.0600	
163.86	246.72	0.0600	
175.40	249.07	0.0600	
186.94	253.15	0.0600	
196.17	255.38	0.0600	
228.48	255.55	0.0600	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	245.80	221E+04	0.9	1.02	40.44
1.01	246.31	.352E+05	14.0	0.99	41.77
1.53	246.83	.841E+05	47.3	1.39	29.61
2.04	247.34	.144E+06	102.0	1.76	23.49
2.55	247.85	.213E+06	177.3	2.07	19.99
3.07	248.37	.291E+06	274.4	2.34	17.66
3.58	248.88	.378E+06	394.6	2.58	15.97
4.09	249.39	.474E+06	541.4	2.83	14.60
4.61	249.91	.578E+06	719.9	3.08	13.38
5.12	250.42	.688E+06	923.8	3.33	12.40
5.63	250.93	.803E+06	1153.3	3.56	11.61
6.14	251.44	.925E+06	1408.6	3.77	10.95
6.66	251.96	.105E+07	1690.6	3.98	10.38
7.17	252.47	.119E+07	2002.3	4.18	9.88
7.68	252.98	.133E+07	2345.4	4.38	9.43
8.20	253.50	.147E+07	2710.1	4.56	9.05
8.71	254.01	.162E+07	3101.0	4.73	8.72
9.22	254.52	.178E+07	3521.8	4.90	8.43
9.74	255.04	.195E+07	3467.6	4.39	9.39

<---- hydrograph ---->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8206)	244.15	1.74	6.75	11.58	0.53	1.02
OUTFLOW: ID= 1 (6438)	244.15	1.53	7.75	11.58	0.52	1.02

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0438):	130.70	0.963	6.75	10.79
+ ID2= 2 (6438):	244.15	1.527	7.75	11.58

ID = 3 (8208): 374.85 2.355 7.50 11.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0440):	226.35	1.396	7.75	14.54
+ ID2= 2 (8208):	374.85	2.355	7.50	11.31
ID = 3 (8210):	601.20	3.750	7.50	12.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0432)
ID= 1 DT=15.0 min Area (ha)= 114.21 Curve Number (CN)= 53.0
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 1.21

Unit Hyd Qpeak (cms)= 1.617
PEAK FLOW (cms)= 0.530 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 11.676
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.186

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0430)
ID= 1 DT=15.0 min Area (ha)= 111.64 Curve Number (CN)= 55.0
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 0.52

Unit Hyd Qpeak (cms)= 3.687
PEAK FLOW (cms)= 1.054 (i)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 12.200
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.195

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0428)
ID= 1 DT=15.0 min Area (ha)= 50.53 Curve Number (CN)= 51.0
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 0.50

Unit Hyd Qpeak (cms)= 1.725
PEAK FLOW (cms)= 0.428 (i)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 10.720
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.171

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0426)
ID= 1 DT=15.0 min Area (ha)= 247.17 Curve Number (CN)= 61.0
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 0.98

Unit Hyd Qpeak (cms)= 4.315
PEAK FLOW (cms)= 1.733 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 14.967
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.239

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB
 NASHYD (0424) | Area (ha)= 49.57 Curve Number (CN)= 59.0
 ID= 1 DT=15.0 min | I a (mm)= 5.00 # of Li near Res. (N)= 1.50
 U. H. Tp(hrs)= 0.37

Unit Hyd Opeak (cms)= 2.293

PEAK FLOW (cms)= 0.670 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 13.586
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.217

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8198) | AREA OPEAK TPEAK R. V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0424): 49.57 0.670 6.50 13.59
 + ID2= 2 (0426): 247.17 1.733 7.25 14.97
 =====
 ID = 3 (8198): 296.74 2.260 6.75 14.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6428) | Routing time step (min)' = 15.00
 IN= 2--> OUT= 1

<----- DATA FOR SECTION (4281.0) ----->

Distance	Elevation	Manning	
0.00	246.54	0.0900	
8.80	246.29	0.0900	
30.78	246.03	0.0900	
41.78	243.97	0.0900	
63.77	237.16	0.0900	
85.76	232.53	0.0900	
96.75	230.16	0.0900	
109.94	228.40	0.0900	
112.14	228.28	0.0900 / 0.0450	Main Channel
112.84	228.00	0.0450	Main Channel
113.34	227.50	0.0450	Main Channel
114.34	227.50	0.0450	Main Channel
114.84	228.00	0.0450	Main Channel
116.54	228.29	0.0450 / 0.0900	Main Channel
118.74	228.40	0.0900	
120.94	228.52	0.0900	
149.52	229.51	0.0900	
169.31	230.69	0.0900	
186.00	234.27	0.0900	
217.69	243.56	0.0900	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.78	228.28	.205E+04	1.6	0.97	21.60
1.58	229.08	.202E+05	16.6	1.02	20.37
2.39	229.89	.665E+05	64.8	1.22	17.11
3.19	230.69	.133E+06	158.1	1.49	14.05
4.00	231.50	.213E+06	308.6	1.82	11.48
4.80	232.30	.299E+06	504.9	2.11	9.89
5.61	233.11	.394E+06	746.6	2.37	8.79
6.41	233.91	.496E+06	1034.5	2.61	7.99
7.21	234.71	.605E+06	1373.3	2.84	7.35
8.02	235.52	.722E+06	1763.3	3.06	6.82
8.82	236.32	.844E+06	2202.0	3.27	6.39
9.63	237.13	.974E+06	2690.5	3.46	6.03
10.43	237.93	.111E+07	3239.5	3.66	5.71
11.23	238.73	.125E+07	3839.0	3.84	5.43
12.04	239.54	.140E+07	4489.1	4.03	5.18
12.84	240.34	.155E+07	5190.5	4.20	4.97
13.65	241.15	.170E+07	5943.8	4.36	4.78
14.45	241.95	.187E+07	6749.8	4.53	4.61
15.26	242.76	.204E+07	7609.4	4.68	4.46

<---- hydrograph ----> <-pi pe / channel ->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL

INFLOW : ID= 2 (8198) 296.74 (ha) (cms) (hrs) (mm) (m) (m/s)
 2.26 6.75 14.74 0.82 0.97
 OUTFLOW: ID= 1 (6428) 296.74 2.17 7.25 14.74 0.81 0.97

 ADD HYD (8200) | AREA OPEAK TPEAK R. V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0428): 50.53 0.428 6.50 10.72
 + ID2= 2 (6428): 296.74 2.171 7.25 14.74
 =====
 ID = 3 (8200): 347.27 2.537 7.25 14.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8202) | AREA OPEAK TPEAK R. V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0430): 111.64 1.054 6.50 12.20
 + ID2= 2 (8200): 347.27 2.537 7.25 14.15
 =====
 ID = 3 (8202): 458.91 3.490 7.00 13.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6432) | Routing time step (min)' = 15.00
 IN= 2--> OUT= 1

<----- DATA FOR SECTION (4321.0) ----->

Distance	Elevation	Manning	
0.00	221.00	0.0600	
15.29	219.85	0.0600	
45.86	219.86	0.0600	
76.44	219.96	0.0600	
110.84	220.23	0.0600	
112.00	220.15	0.0600	
114.66	220.09	0.0600	
118.48	219.73	0.0600	
120.80	219.70	0.0600 / 0.0350	Main Channel
121.30	219.20	0.0350	Main Channel
122.30	219.20	0.0350	Main Channel
122.80	219.70	0.0350 / 0.0600	Main Channel
129.95	220.11	0.0600	
175.81	220.24	0.0600	
214.03	220.55	0.0600	
252.25	220.61	0.0600	
290.47	220.70	0.0600	
328.69	221.04	0.0600	
347.80	221.08	0.0600	
378.37	221.04	0.0600	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.08	219.28	.185E+03	0.0	0.23	149.38
0.17	219.37	.399E+03	0.1	0.34	100.62
0.25	219.45	.641E+03	0.1	0.42	80.98
0.33	219.53	.911E+03	0.2	0.49	69.79
0.42	219.62	.121E+04	0.3	0.55	62.35
0.50	219.70	.154E+04	0.5	0.60	56.91
0.61	219.81	.267E+04	0.7	0.56	61.28
0.71	219.91	.901E+04	1.3	0.30	112.96
0.82	220.02	.249E+05	3.4	0.28	122.71
0.92	220.12	.452E+05	6.8	0.31	109.98
1.03	220.23	.747E+05	12.2	0.33	102.05
1.14	220.34	.112E+06	21.3	0.39	87.59
1.24	220.44	.153E+06	33.0	0.44	77.01
1.35	220.55	.196E+06	47.2	0.49	69.22
1.46	220.66	.248E+06	62.3	0.51	66.48
1.56	220.76	.310E+06	83.4	0.55	62.06
1.67	220.87	.376E+06	108.8	0.59	57.56
1.77	220.97	.444E+06	137.7	0.64	53.77
1.88	221.08	.517E+06	163.6	0.65	52.70

<---- hydrograph ----> <-pi pe / channel ->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL

INFLOW : ID= 2 (8202) (ha) (cms) (hrs) (mm) (m) (m/s)
 458.91 3.49 7.00 13.68 0.82 0.28
 OUTFLOW: ID= 1 (6432) 458.91 2.33 9.25 13.68 0.76 0.29

 ADD HYD (8204)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0432): 114.21 0.530 8.00 11.68
 + ID2= 2 (6432): 458.91 2.333 9.25 13.68

 ID = 3 (8204): 573.12 2.816 9.00 13.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8212)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8204): 573.12 2.816 9.00 13.28
 + ID2= 2 (8210): 601.20 3.750 7.50 12.52

 ID = 3 (8212): 1174.32 6.240 8.00 12.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6442)
 IN= 2----> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (4421.0) ----->
 Di stance Elevati on Manni ng
 0.00 221.00 0.0350
 26.73 220.80 0.0350
 53.47 220.60 0.0350
 66.83 220.40 0.0350
 80.20 220.20 0.0350
 347.52 220.00 0.0350
 354.21 219.59 0.0350
 360.89 219.50 0.0350
 367.57 219.08 0.0350
 368.76 219.00 0.0350 /0.0300 Mai n Channel
 369.26 218.50 0.0300 Mai n Channel
 374.26 218.50 0.0300 Mai n Channel
 374.76 219.00 0.0300 /0.0350 Mai n Channel
 380.94 220.33 0.0350
 387.62 219.62 0.0350
 394.31 219.23 0.0350
 454.45 219.07 0.0350
 514.60 219.09 0.0350
 660.00 219.19 0.0350
 661.63 221.00 0.0350

<----- TRAVEL TIME TABLE ----->
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV. TIME
 (m) (m) (cu. m.) (cms) (m/s) (mi n)
 0.12 218.62 .100E+04 0.2 0.28 92.89
 0.25 218.75 .205E+04 0.6 0.43 60.09
 0.37 218.87 .315E+04 1.1 0.55 46.99
 0.50 219.00 .430E+04 1.8 0.66 39.67
 0.63 219.13 .141E+05 3.4 0.37 69.99
 0.77 219.27 .652E+05 13.7 0.33 79.14
 0.90 219.40 .124E+06 35.5 0.45 58.02
 1.03 219.53 .183E+06 65.7 0.56 46.45
 1.17 219.67 .245E+06 103.8 0.66 39.30
 1.30 219.80 .307E+06 149.3 0.76 34.30
 1.43 219.93 .371E+06 201.6 0.85 30.65
 1.57 220.07 .440E+06 256.5 0.91 28.56
 1.70 220.20 .541E+06 325.7 0.94 27.70
 1.83 220.33 .663E+06 415.2 0.98 26.62
 1.97 220.47 .787E+06 520.3 1.03 25.21
 2.10 220.60 .913E+06 638.5 1.09 23.82
 2.23 220.73 .104E+07 767.2 1.15 22.62
 2.37 220.87 .117E+07 908.0 1.21 21.54
 2.50 221.00 .131E+07 1060.7 1.27 20.57

<----- hydrograph -----> <-pi pe / channel ->
 AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL

INFLOW : ID= 2 (8212) (ha) (cms) (hrs) (mm) (m) (m/s)
 1174.32 6.24 8.00 12.89 0.67 0.36
 OUTFLOW: ID= 1 (6442) 1174.32 5.46 9.50 12.89 0.66 0.36

 ADD HYD (8214)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0442): 117.26 0.395 8.00 8.37
 + ID2= 2 (6442): 1174.32 5.460 9.50 12.89

 ID = 3 (8214): 1291.58 5.802 9.50 12.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8196)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8194): 44084.05 177.611 14.00 21.93
 + ID2= 2 (8214): 1291.58 5.802 9.50 12.48

 ID = 3 (8196): 45375.62 181.075 14.00 21.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB NASHYD (0444) Area (ha)= 221.65 Curve Number (CN)= 56.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 1.03

Unit Hyd Opeak (cms)= 3.656

PEAK FLOW (cms)= 1.266 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 12.815
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.204

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8216)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0444): 221.65 1.266 7.50 12.82
 + ID2= 2 (8196): 45375.62 181.075 14.00 21.66

 ID = 3 (8216): 45597.28 181.391 14.00 21.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

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V V I SSSS U U A L
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L L
W W I SSSS UUUU A A LLLLL

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0 0 T T H H Y Y M M 0 0
0 0 T T H H Y Y M M 0 0 Company Serial

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***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\Visual Otthymo 2.4\VO2\vo1.n.dat
 Output filename: C:\Users\amanluc\AppData\Local\Temp\cb0c6c38-61aa-40de-82c2-26b771bad59b\Scenario.out
 Summary filename: C:\Users\amanluc\AppData\Local\Temp\cb0c6c38-61aa-40de-82c2-26b771bad59b\Scenario.sum

DATE: 08/29/2012 TIME: 01:57:31
 USER:

COMMENTS: _____

 ** SIMULATION NUMBER: 1 **

READ STORM
 Ptotal = 73.10 mm
 Filename: C:\Users\amanluc\AppData\Local\Temp\cb0c6c38-61aa-40de-82c2-26b771bad59b\5a0ec211
 Comments: 25-Year 12-Hour SCS II Design Storm

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.83	3.25	2.92	6.25	13.16
0.50	1.83	3.50	2.92	6.50	13.16
0.75	1.83	3.75	2.92	6.75	5.85
1.00	1.83	4.00	2.92	7.00	5.85
1.25	1.83	4.25	4.39	7.25	4.39
1.50	1.83	4.50	4.39	7.50	4.39
1.75	1.83	4.75	5.85	7.75	4.39
2.00	1.83	5.00	5.85	8.00	4.39
2.25	2.19	5.25	8.77	8.25	2.56
2.50	2.19	5.50	8.77	8.50	2.56
2.75	2.19	5.75	35.09	8.75	2.56
3.00	2.19	6.00	96.49	9.00	2.56

CALIB STANDHYD (1662)
 ID= 1 DT=15.0 min
 Area (ha)= 141.06
 Total Imp(%)= 70.00 Di r. Conn.(%)= 38.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 98.74 42.32
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 969.74 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 96.49 136.73
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 12.47 (ii) 21.91 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04
 TOTALS
 PEAK FLOW (cms)= 11.26 7.92 16.519 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00

RUNOFF VOLUME (mm)= 71.10 42.93 53.63
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.59 0.73

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1661)
 ID= 1 DT=15.0 min
 Area (ha)= 293.64
 Total Imp(%)= 42.00 Di r. Conn.(%)= 23.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 123.33 170.31
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1399.14 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 96.49 70.19
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 15.54 (ii) 27.86 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.07 0.04
 TOTALS
 PEAK FLOW (cms)= 12.99 14.37 22.140 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 71.10 34.23 42.71
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.47 0.58

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5166)
 IN= 2---> OUT= 1
 DT= 15.0 min
 OUTFLOW (cms) STORAGE (ha.m.)
 0.0000 0.0000 7.2300 6.6504
 0.4310 3.5677 9.1580 7.5825
 1.6680 3.8178 10.9790 8.5125
 3.5970 4.8509 11.3790 8.5225
 5.3070 5.7381 0.0000 0.0000
 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 INFLOW: ID= 2 (1661) 293.640 22.140 6.00 42.71
 OUTFLOW: ID= 1 (5166) 293.640 7.083 7.00 42.70

PEAK FLOW REDUCTION [Qout/Qin] (%) = 31.99
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 6.5871

ADD HYD (8134)
 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (1662): 141.06 16.519 6.00 53.63
 + ID2= 2 (5166): 293.64 7.083 7.00 42.70
 ID= 3 (8134): 434.70 16.866 6.00 46.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1642)
 ID= 1 DT=15.0 min
 Area (ha)= 120.35
 Total Imp(%)= 22.00 Di r. Conn.(%)= 17.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	26.48	93.87	
Dep. Storage over (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	895.73	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	96.49	47.97	
Storage Coeff. (min)=	15.00	30.00	
Unit Hyd. Tpeak (min)=	11.89 (ii)	26.24 (ii)	
Unit Hyd. Tpeak (cms)=	15.00	30.00	
	0.08	0.04	
PEAK FLOW (cms)=	4.37	5.54	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	7.850 (iii)
RUNOFF VOLUME (mm)=	71.10	29.27	36.38
TOTAL RAINFALL (mm)=	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.40	0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1641) ID= 1 DT=15.0 min	Area (ha)= 113.89	Dir. Conn. (%)= 53.00
	Total Imp (%)= 71.00	

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	80.86	33.03	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	871.36	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	96.49	94.31	
Storage Coeff. over (min)=	15.00	30.00	
Unit Hyd. Tpeak (min)=	11.69 (ii)	22.65 (ii)	
Unit Hyd. Tpeak (cms)=	15.00	30.00	
	0.08	0.04	
PEAK FLOW (cms)=	12.98	4.17	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	15.695 (iii)
RUNOFF VOLUME (mm)=	71.10	37.69	55.40
TOTAL RAINFALL (mm)=	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.52	0.76

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5164) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	8.4260	4.5513
	0.3860	1.9228	9.2330	4.9251
	3.3700	2.2339	9.9990	5.2982
	4.5710	3.3613	10.3990	5.3082
	5.7170	4.0568	0.0000	0.0000
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1641)	113.890	15.695	6.00	55.40
OUTFLOW: ID= 1 (5164)	113.890	4.444	6.75	55.39

PEAK FLOW REDUCTION [Qout/Qin] (%) = 28.31
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 3.2831

ADD HYD (8132) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1642):	120.35	7.850	6.00	36.38
+ ID2= 2 (5164):	113.89	4.444	6.75	55.39
ID = 3 (8132):	234.24	11.416	6.25	45.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0152) ID= 1 DT=15.0 min	Area (ha)= 124.37	Curve Number (CN)= 59.0
	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50
	U. H. Tp (hrs)= 0.89	

Unit Hyd Opeak (cms)= 2.372

PEAK FLOW (cms)=	1.174 (i)
TIME TO PEAK (hrs)=	7.000
RUNOFF VOLUME (mm)=	18.731
TOTAL RAINFALL (mm)=	73.100
RUNOFF COEFFICIENT =	0.256

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1481) ID= 1 DT=15.0 min	Area (ha)= 61.75	Dir. Conn. (%)= 17.00
	Total Imp (%)= 47.00	

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	29.02	32.73	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	641.61	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	96.49	11.21	
Storage Coeff. over (min)=	15.00	45.00	
Unit Hyd. Tpeak (min)=	9.73 (ii)	35.41 (ii)	
Unit Hyd. Tpeak (cms)=	15.00	45.00	
	0.09	0.03	

PEAK FLOW (cms)=	2.39	0.59	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.50	2.631 (iii)
RUNOFF VOLUME (mm)=	71.10	8.17	18.87
TOTAL RAINFALL (mm)=	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.11	0.26

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 23.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5148) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	4.1310	1.6343
	0.2090	0.7802	5.1870	1.8598
	1.1960	0.9062	6.0380	2.0805
	2.2280	1.1562	6.4380	2.0905
	3.0910	1.3741	0.0000	0.0000
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1481)	61.750	2.631	6.00	18.87
OUTFLOW: ID= 1 (5148)	61.750	0.222	8.50	18.86

PEAK FLOW REDUCTION [Qout/Qin] (%) = 8.43
 TIME SHIFT OF PEAK FLOW (min) = 150.00
 MAXIMUM STORAGE USED (ha.m.) = 0.7824

CALIB NASHYD (1482) Area (ha) = 356.14 Curve Number (CN) = 31.3
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 1.12

Unit Hyd Qpeak (cms) = 5.438

PEAK FLOW (cms) = 1.087 (i)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 7.349
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.101

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9148)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1482):	356.14	1.087	7.75	7.35
+ ID2= 2 (5148):	61.75	0.222	8.50	18.86
ID = 3 (9148):	417.89	1.293	8.00	9.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1501) Area (ha) = 0.16
 ID= 1 DT=15.0 min Total Imp(%) = 45.00 Dir. Conn. (%) = 15.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	0.07	0.09
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	32.66	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 96.49 over (min) = 15.00
 Storage Coeff. (min) = 1.63 (ii)
 Unit Hyd. Tpeak (min) = 15.00
 Unit Hyd. peak (cms) = 0.11

TOTALS
 PEAK FLOW (cms) = 0.01
 TIME TO PEAK (hrs) = 6.00
 RUNOFF VOLUME (mm) = 71.10
 TOTAL RAINFALL (mm) = 73.10
 RUNOFF COEFFICIENT = 0.97

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 16.4 Ia = Dep Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5150)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.0110	0.0042
0.0010	0.0020	0.0140	0.0049
0.0030	0.0023	0.0160	0.0054
0.0060	0.0030	0.0160	0.0154
0.0080	0.0036	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1501)	0.160	0.007	6.00	15.09
OUTFLOW: ID= 1 (5150)	0.160	0.001	7.50	12.09

PEAK FLOW REDUCTION [Qout/Qin] (%) = 10.26

TIME SHIFT OF PEAK FLOW (min) = 90.00
 MAXIMUM STORAGE USED (ha. m.) = 0.0014

CALIB NASHYD (1502) Area (ha) = 105.72 Curve Number (CN) = 17.2
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 0.58

Unit Hyd Qpeak (cms) = 3.134

PEAK FLOW (cms) = 0.254 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 3.510
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.048

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9150)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1502):	105.72	0.254	6.75	3.51
+ ID2= 2 (5150):	0.16	0.001	7.50	12.09
ID = 3 (9150):	105.88	0.255	6.75	3.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8116)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9148):	417.89	1.293	8.00	9.05
+ ID2= 2 (9150):	105.88	0.255	6.75	3.52
ID = 3 (8116):	523.77	1.506	7.50	7.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6152)
 IN= 2---> OUT= 1 Routing time step (min) = 15.00

<----- DATA FOR SECTION (1521.0) ----->

Distance	Elevation	Manning
0.00	290.00	0.0500
15.34	286.87	0.0500
46.03	284.84	0.0500
88.22	283.07	0.0500
126.57	282.11	0.0500
153.42	280.34	0.0500
157.26	280.07	0.0500
161.09	279.72	0.0500
167.76	278.94	0.0500 / 0.0300
168.01	278.60	0.0300
168.76	278.60	0.0300
169.51	278.60	0.0300
169.76	278.90	0.0300 / 0.0500
173.76	279.40	0.0500
180.27	279.69	0.0500
184.11	279.93	0.0500
218.62	281.86	0.0500
260.82	283.20	0.0500
360.54	289.25	0.0500
379.72	289.80	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.30	278.90	.114E+04	0.4	0.80	45.70
0.87	279.47	.932E+04	4.2	0.98	37.22
1.45	280.05	.353E+05	17.5	1.09	33.68
2.02	280.62	.831E+05	47.8	1.26	29.01
2.59	281.19	.155E+06	102.9	1.46	25.06
3.17	281.77	.250E+06	189.3	1.66	22.03
3.74	282.34	.375E+06	298.5	1.75	20.91

4.32	282.92	.547E+06	456.5	1.83	19.98
4.89	283.49	.767E+06	705.7	2.02	18.12
5.46	284.06	.102E+07	1038.0	2.24	16.34
6.04	284.64	.130E+07	1444.1	2.45	14.98
6.61	285.21	.161E+07	1940.1	2.66	13.79
7.18	285.78	.194E+07	2522.6	2.86	12.80
7.76	286.36	.229E+07	3187.4	3.06	11.98
8.33	286.93	.267E+07	3942.3	3.25	11.28
8.91	287.51	.306E+07	4824.9	3.46	10.58
9.48	288.08	.347E+07	5795.7	3.67	9.99
10.05	288.65	.390E+07	6855.4	3.86	9.48
10.63	289.23	.434E+07	8005.0	4.05	9.04

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8116)	523.77	1.51	7.50	7.93	0.47	0.85
OUTFLOW: ID= 1 (6152)	523.77	1.42	8.50	7.93	0.45	0.84

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0152):	124.37	1.174	7.00	18.73
+ ID2= 2 (6152):	523.77	1.424	8.50	7.93
ID = 3 (8122):	648.14	2.498	8.00	10.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	178.59	61.0
	U.H. Tp(hrs)= 5.00	# of Linear Res. (N)= 1.50
		0.94

Unit Hyd Opeak (cms) = 3.230

PEAK FLOW (cms) = 1.724 (i)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 19.897
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.272

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	83.49	45.0
	U.H. Tp(hrs)= 5.00	# of Linear Res. (N)= 1.50
		0.62

Unit Hyd Opeak (cms) = 2.306

PEAK FLOW (cms) = 0.666 (i)
TIME TO PEAK (hrs) = 6.750
RUNOFF VOLUME (mm) = 11.996
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.164

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD	Area (ha)	Dir. Conn. (%)
ID= 1 DT=15.0 min	11.15	70.00
	Total Imp(%)= 75.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) = 8.36		2.79
Dep. Storage (mm) = 2.00		5.00
Average Slope (%) = 0.50		0.50
Length (m) = 272.64		40.00
Mannings n = 0.013		0.250

Max. Eff. Inten. (mm/hr) = 96.49	9.37
over (min) = 15.00	45.00
Storage Coeff. (min) = 5.82 (ii)	33.41 (ii)
Unit Hyd. Tpeak (min) = 15.00	45.00

Unit Hyd. peak (cms) = 0.10	0.03	*TOTALS*
PEAK FLOW (cms) = 1.99	0.04	2.005 (iii)
TIME TO PEAK (hrs) = 6.00	6.50	6.00
RUNOFF VOLUME (mm) = 71.10	8.91	52.44
TOTAL RAINFALL (mm) = 73.10	73.10	73.10
RUNOFF COEFFICIENT = 0.97	0.12	0.72

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 31.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5154)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1				
DT= 15.0 min				
	0.0000	0.0000	0.7460	0.3544
	0.0380	0.1987	0.9370	0.3839
	0.2160	0.2386	1.0900	0.4129
	0.4020	0.2749	1.4900	0.4229
	0.5580	0.3133	0.0000	0.0000

INFLOW : ID= 2 (1541)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
OUTFLOW: ID= 1 (5154)	11.150	2.005	6.00	52.44
	11.150	0.604	6.25	52.32

PEAK FLOW REDUCTION [Qout/Qin] (%) = 30.15
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha.m.) = 0.3306

CALIB NASHYD	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	189.17	39.1
	U.H. Tp(hrs)= 5.00	# of Linear Res. (N)= 1.50
		0.95

Unit Hyd Opeak (cms) = 3.396

PEAK FLOW (cms) = 0.882 (i)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 9.891
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.135

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (1542):	189.17	0.882	7.25	9.89
+ ID2= 2 (5154):	11.15	0.604	6.25	52.32
ID = 3 (9154):	200.32	1.319	6.50	12.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0156):	83.49	0.666	6.75	12.00
+ ID2= 2 (9154):	200.32	1.319	6.50	12.25
ID = 3 (8118):	283.81	1.978	6.50	12.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6158)	Routing time step (min)
IN= 2--> OUT= 1	15.00

----->
 <----- DATA FOR SECTION (1581.0) ----->

Distance	Elevation	Manning	
0.00	290.00	0.0400	
19.20	288.88	0.0400	
48.00	288.26	0.0400	
119.99	283.68	0.0400	
167.99	282.07	0.0400	
196.79	281.32	0.0400	
210.79	280.90	0.0400	
219.79	280.38	0.0400 / 0.0300	Main Channel
220.04	279.65	0.0300	Main Channel
220.79	279.65	0.0300	Main Channel
221.54	279.65	0.0300	Main Channel
221.79	280.39	0.0300 / 0.0400	Main Channel
225.79	280.59	0.0400	
235.19	280.88	0.0400	
254.39	281.50	0.0400	
273.58	282.56	0.0400	
302.38	283.42	0.0400	
359.98	284.03	0.0400	
436.77	289.37	0.0400	
475.17	290.37	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.37	280.01	.973E+03	0.5	0.86	31.78
0.73	280.38	.210E+04	1.5	1.18	23.16
1.30	280.95	.149E+05	8.6	0.95	28.91
1.86	281.51	.579E+05	40.0	1.13	24.12
2.43	282.08	.133E+06	118.5	1.46	18.75
2.99	282.64	.236E+06	254.6	1.77	15.46
3.56	283.21	.369E+06	450.3	2.00	13.67
4.13	283.78	.542E+06	702.7	2.13	12.86
4.69	284.34	.767E+06	1125.2	2.41	11.36
5.26	284.91	.101E+07	1697.1	2.75	9.93
5.82	285.47	.127E+07	2379.1	3.07	8.91
6.39	286.04	.155E+07	3171.4	3.36	8.14
6.95	286.60	.184E+07	4075.5	3.64	7.53
7.52	287.17	.215E+07	5093.1	3.89	7.03
8.09	287.74	.247E+07	6226.5	4.13	6.62
8.65	288.30	.281E+07	7461.2	4.36	6.28
9.22	288.87	.318E+07	8635.1	4.46	6.13
9.78	289.43	.357E+07	10143.6	4.67	5.86
10.35	290.00	.398E+07	11608.8	4.79	5.71

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8118)	283.81	1.98	6.50	12.18	0.77	1.16
OUTFLOW: ID= 1 (6158)	283.81	1.83	7.00	12.18	0.76	1.17

ADD HYD (8120)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0158):	178.59	1.724	7.25	19.90
+ ID2= 2 (6158):	283.81	1.833	7.00	12.18
ID = 3 (8120):	462.40	3.556	7.25	15.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8124)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (8120):	462.40	3.556	7.25	15.16
+ ID2= 2 (8122):	648.14	2.498	8.00	10.00
ID = 3 (8124):	1110.54	5.935	7.50	12.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6160)
 IN= 2--> OUT= 1 Routing time step (mi n)' = 15.00

----->
 <----- DATA FOR SECTION (1601.0) ----->

Distance	Elevation	Manning	
0.00	261.02	0.0400	
29.89	254.30	0.0400	
40.75	252.58	0.0400	
62.49	251.30	0.0400	
114.11	250.94	0.0400	
130.41	248.58	0.0400	
141.28	247.50	0.0400	
154.86	246.87	0.0400	
155.20	246.70	0.0400	
160.20	246.30	0.0400 / 0.0300	Main Channel
160.30	245.30	0.0300	Main Channel
165.20	245.30	0.0300	Main Channel
165.30	246.30	0.0300 / 0.0400	Main Channel
168.45	246.96	0.0400	
195.62	249.20	0.0400	
203.77	250.82	0.0400	
225.50	256.78	0.0400	
244.52	261.14	0.0400	
255.39	261.95	0.0400	
268.97	261.92	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.50	245.80	.120E+05	2.9	1.18	68.31
1.00	246.30	.242E+05	8.5	1.70	47.47
1.92	247.22	.825E+05	31.7	1.86	43.34
2.84	248.14	.248E+06	102.5	2.00	40.39
3.76	249.06	.506E+06	247.7	2.37	34.03
4.68	249.98	.832E+06	486.8	2.83	28.49
5.60	250.90	.121E+07	814.9	3.27	24.71
6.52	251.82	.180E+07	1157.1	3.11	25.97
7.44	252.74	.253E+07	1840.2	3.52	22.92
8.36	253.66	.331E+07	2727.1	3.98	20.25
9.29	254.59	.414E+07	3782.6	4.43	18.22
10.21	255.51	.499E+07	5010.7	4.86	16.61
11.13	256.43	.589E+07	6397.4	5.26	15.33
12.05	257.35	.681E+07	7932.8	5.64	14.31
12.97	258.27	.777E+07	9621.2	5.99	13.46
13.89	259.19	.877E+07	11469.4	6.33	12.74
14.81	260.11	.980E+07	13478.4	6.66	12.12
15.73	261.03	.109E+08	15651.7	6.97	11.58
16.65	261.95	.120E+08	17492.5	7.07	11.42

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8124)	1110.54	5.93	7.50	12.15	0.77	1.41
OUTFLOW: ID= 1 (6160)	1110.54	5.25	8.50	12.15	0.71	1.35

CALIB STANDBY (1601)	Area Total (ha)	Imp (%)	Di r. Conn. (%)
ID= 1 DT=15.0 mi n	19.03	75.00	69.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	14.27	4.76
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	356.18	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	23.32
over (mi n)	15.00	30.00
Storage Coeff. (mi n)=	6.84 (ii)	25.99 (ii)
Uni t Hyd. Tpeak (mi n)=	15.00	30.00
Uni t Hyd. peak (cms)=	0.10	0.04

	(cms)	(hrs)	(mm)
PEAK FLOW	3.26	0.22	3.396 (iii)
TIME TO PEAK	6.00	6.25	6.00
RUNOFF VOLUME	71.10	20.50	55.41
TOTAL RAI NFALL	73.10	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.28	0.76

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 55.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5160)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	1.2730	0.6034
	0.0640	0.3377	1.5980	0.6540
	0.3690	0.4055	1.8600	0.7037
	0.6870	0.4676	2.2600	0.7137
	0.9520	0.5332	0.0000	0.0000
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1601)	19.030	3.396	6.00	55.41
OUTFLOW: ID= 1 (5160)	19.030	1.063	6.25	55.35

PEAK FLOW REDUCTION [Qout/Qin] (%) = 31.31
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha. m.) = 0.5597

CALIB NASHYD (1602)			
ID= 1 DT=15.0 min			
Area (ha)	= 299.79	Curve Number (CN)	= 60.3
Ia (mm)	= 5.00	# of Linear Res. (N)	= 1.50
U. H. Tp (hrs)	= 1.42		

Unit Hyd Opeak (cms) = 3.592

PEAK FLOW (cms) = 2.078 (i)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 19.588
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.268

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9160)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1602):	299.79	2.078	8.00	19.59
+ ID2= 2 (5160):	19.03	1.063	6.25	55.35
=====				
ID = 3 (9160):	318.82	2.601	6.75	21.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8126)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6160):	1110.54	5.247	8.50	12.15
+ ID2= 2 (9160):	318.82	2.601	6.75	21.72
=====				
ID = 3 (8126):	1429.36	7.512	8.50	14.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1621)			
ID= 1 DT=15.0 min			
Area Total	(ha) = 46.07	Dir. Conn. (%) =	20.00
Imp (%) =	45.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 20.73	25.34
Dep. Storage (mm)	= 2.00	5.00
Average Slope (%)	= 0.50	0.50
Length (m)	= 554.20	40.00
Mannings n	= 0.013	0.250
Max. Eff. Inten. (mm/hr) over (min)	= 96.49 / 15.00	44.70 / 30.00
Storage Coeff. (min)	= 8.91 (ii)	23.68 (ii)
Unit Hyd. Tpeak (min)	= 15.00	30.00

Unit Hyd. peak (cms)	= 0.09	0.04	*TOTALS* 3.064 (iii)
PEAK FLOW (cms)	= 2.15	1.46	6.00
TIME TO PEAK (hrs)	= 6.00	6.25	30.62
RUNOFF VOLUME (mm)	= 71.10	20.50	73.10
TOTAL RAINFALL (mm)	= 73.10	73.10	0.42
RUNOFF COEFFICIENT	= 0.97	0.28	

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 51.1 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5162)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	3.0820	1.1997
	0.1560	0.5629	3.8700	1.3716
	0.8930	0.6512	4.5050	1.5398
	1.6620	0.8407	4.9050	1.5498
	2.3060	1.0035	0.0000	0.0000
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1621)	46.070	3.064	6.00	30.62
OUTFLOW: ID= 1 (5162)	46.070	1.078	6.75	30.60

PEAK FLOW REDUCTION [Qout/Qin] (%) = 35.20
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.7024

CALIB STANDHYD (1622)			
ID= 1 DT=15.0 min			
Area Total	(ha) = 144.07	Dir. Conn. (%) =	11.00
Imp (%) =	26.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 37.46	106.61
Dep. Storage (mm)	= 2.00	5.00
Average Slope (%)	= 0.50	0.50
Length (m)	= 980.03	40.00
Mannings n	= 0.013	0.250
Max. Eff. Inten. (mm/hr) over (min)	= 96.49 / 15.00	19.12 / 45.00
Storage Coeff. (min)	= 12.55 (ii)	33.28 (ii)
Unit Hyd. Tpeak (min)	= 15.00	45.00
Unit Hyd. peak (cms)	= 0.08	0.03

PEAK FLOW (cms)	= 3.32	3.34	4.692 (iii)
TIME TO PEAK (hrs)	= 6.00	6.50	6.00
RUNOFF VOLUME (mm)	= 71.10	17.54	23.43
TOTAL RAINFALL (mm)	= 73.10	73.10	73.10
RUNOFF COEFFICIENT	= 0.97	0.24	0.32

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 51.1 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9162)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1622):	144.07	4.692	6.00	23.43
+ ID2= 2 (5162):	46.07	1.078	6.75	30.60
=====				
ID = 3 (9162):	190.14	5.198	6.50	25.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	(8128)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8126):	1429.36	7.512	8.50	14.29	
+ ID2= 2 (9162):	190.14	5.198	6.50	25.17	
ID = 3 (8128):	1619.50	9.444	6.50	15.56	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	(8130)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8128):	1619.50	9.444	6.50	15.56	
+ ID2= 2 (8132):	234.24	11.416	6.25	45.62	
ID = 3 (8130):	1853.74	18.970	6.25	19.36	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6166)
IN= 2----> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1661.0) ----->

Distance	Elevation	Manning	
0.00	248.36	0.0500	
41.67	246.98	0.0500	
95.82	244.93	0.0500	
149.98	242.42	0.0500	
191.64	241.88	0.0500	
224.97	239.90	0.0500	
229.13	239.35	0.0500	
233.30	237.77	0.0500	
237.47	236.57	0.0500	
240.63	235.60	0.0500 / 0.0300	Main Channel
241.63	234.10	0.0300	Main Channel
247.13	234.10	0.0300	Main Channel
247.53	235.60	0.0300 / 0.0500	Main Channel
254.13	236.25	0.0500	
266.63	236.77	0.0500	
283.29	237.84	0.0500	
291.63	238.89	0.0500	
329.12	244.66	0.0500	
370.78	249.55	0.0500	
412.44	253.17	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.75	234.85	.130E+05	6.0	1.36	36.45
1.50	235.60	.276E+05	18.1	1.95	25.41
2.25	236.35	.544E+05	40.0	2.18	22.69
3.00	237.10	.117E+06	80.1	2.04	24.32
3.75	237.85	.213E+06	148.9	2.07	23.89
4.50	238.60	.334E+06	255.1	2.27	21.85
5.25	239.35	.473E+06	395.8	2.49	19.90
6.00	240.10	.631E+06	563.8	2.65	18.66
6.75	240.85	.824E+06	773.2	2.79	17.77
7.50	241.60	.106E+07	1037.9	2.92	16.97
8.26	242.36	.135E+07	1326.7	2.92	16.94
9.01	243.11	.173E+07	1758.5	3.01	16.44
9.76	243.86	.217E+07	2285.9	3.13	15.81
10.51	244.61	.265E+07	2909.5	3.26	15.17
11.26	245.36	.318E+07	3610.1	3.37	14.67
12.01	246.11	.377E+07	4417.4	3.48	14.21
12.76	246.86	.441E+07	5348.0	3.60	13.75
13.51	247.61	.512E+07	6394.0	3.71	13.34
14.26	248.36	.589E+07	7580.3	3.82	12.95

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8130)	1853.74	18.97	6.25	19.36	1.53

OUTFLOW: ID= 1 (6166) 1853.74 16.67 6.75 19.36 1.40 1.84

ADD HYD 1 + 2 = 3	(8136)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6166):	1853.74	16.670	6.75	19.36	
+ ID2= 2 (8134):	434.70	16.866	6.00	46.25	
ID = 3 (8136):	2288.44	28.554	6.50	24.47	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2021)
ID= 1 DT=15.0 min Area Total (ha)= 107.32 Imp(%)= 45.00 Di r. Conn.(%)= 27.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	48.29	59.03
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	845.85	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	96.49	71.50
over (min)	15.00	30.00
Storage Coeff. (min)=	11.49 (ii)	23.72 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04
PEAK FLOW (cms)=	6.27	5.51
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	71.10	34.84
TOTAL RAINFALL (mm)=	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.48
TOTALS		9.806 (iii)
		6.00
		44.63
		73.10
		0.61

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5202)
IN= 2----> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	5.1480	2.8069
0.3630	1.2717	6.4200	3.2412
1.5670	1.4796	7.4640	3.6910
2.7880	1.9296	7.8640	3.7010
3.8580	2.3404	0.0000	0.0000

INFLOW : ID= 2 (2021)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5202)	107.320	9.806	6.00	44.63
	107.320	3.653	6.75	44.62

PEAK FLOW REDUCTION [Out/Oi n](%)= 37.25
TIME SHIFT OF PEAK FLOW (min)= 45.00
MAXIMUM STORAGE USED (ha. m.)= 2.2697

CALIB STANDHYD (2022)
ID= 1 DT=15.0 min Area Total (ha)= 361.21 Imp(%)= 60.00 Di r. Conn.(%)= 36.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	216.73	144.48
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1551.79	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	96.49	95.42

over (min) 15.00 30.00
 Storage Coeff. (min)= 16.53 (ii) 27.43 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.07 0.04

TOTALS
 PEAK FLOW (cms)= 24.32 16.82 35.206 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 71.10 38.58 50.29
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.53 0.69

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 73.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8110)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2022):	361.21	35.206	6.00	50.29
+ ID2= 2 (5202):	107.32	3.653	6.75	44.62
ID = 3 (8110):	468.53	35.549	6.00	48.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (0102)
 ID= 1 DT=15.0 min

Area (ha)	Imp(%)	Dir. Conn. (%)
466.00	23.00	9.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	107.18	358.82
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1762.57	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 96.49 17.28
 over (min) 15.00 45.00
 Storage Coeff. (min)= 17.85 (ii) 39.44 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.06 0.03

TOTALS
 PEAK FLOW (cms)= 7.57 9.30 11.959 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.50
 RUNOFF VOLUME (mm)= 71.10 16.21 21.15
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.22 0.29

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 49.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0100)
 ID= 1 DT=15.0 min

Area (ha)	Curve Number (CN)	# of Linear Res. (N)
693.84	40.0	1.50

Unit Hyd Opeak (cms)= 6.087

PEAK FLOW (cms)= 1.955 (i)
 TIME TO PEAK (hrs)= 9.250
 RUNOFF VOLUME (mm)= 10.287
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.141

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8000)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0100):	693.84	1.955	9.25	10.29
+ ID2= 2 (0102):	466.00	11.959	6.50	21.15
ID = 3 (8000):	1159.84	13.215	6.50	14.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6104)
 IN= 2----> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1041.0) ----->

Distance	Elevation	Manning
0.00	330.16	0.0450
50.03	328.36	0.0450
89.34	326.33	0.0450
117.94	324.30	0.0450
160.82	322.75	0.0450
178.34	319.96	0.0450
183.34	319.81	0.0450
184.34	319.38	0.0450 / 0.0300
185.09	318.78	0.0300
185.84	318.72	0.0300
186.84	319.32	0.0300 / 0.0450
193.84	319.70	0.0450
198.84	320.38	0.0450
200.13	320.81	0.0450
218.00	322.49	0.0450
239.44	323.49	0.0450
250.17	323.96	0.0450
275.18	325.20	0.0450
310.92	326.47	0.0450
353.81	330.00	0.0450

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.30	319.02	.624E+03	0.1	0.45	73.00
0.60	319.32	.181E+04	0.6	0.67	49.46
1.23	319.95	.128E+05	4.9	0.77	43.38
1.86	320.58	.396E+05	19.1	0.96	34.50
2.48	321.20	.757E+05	43.5	1.15	28.98
3.11	321.83	.125E+06	81.2	1.30	25.58
3.74	322.46	.187E+06	135.9	1.45	22.92
4.37	323.09	.269E+06	195.6	1.45	22.90
5.00	323.72	.387E+06	287.8	1.48	22.44
5.63	324.35	.545E+06	428.4	1.56	21.22
6.25	324.97	.735E+06	630.5	1.71	19.42
6.88	325.60	.952E+06	876.1	1.83	18.12
7.51	326.23	.120E+07	1177.0	1.95	17.03
8.14	326.86	.149E+07	1554.4	2.08	15.93
8.77	327.49	.179E+07	2009.7	2.23	14.88
9.40	328.12	.213E+07	2533.9	2.37	13.99
10.02	328.74	.249E+07	3112.6	2.49	13.31
10.65	329.37	.288E+07	3760.9	2.60	12.75
11.28	330.00	.330E+07	4494.8	2.71	12.23

<---- hydrograph ----> <- pipe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8000)	1159.84	13.22	6.50	14.65	1.59	0.87
OUTFLOW: ID= 1 (6104)	1159.84	9.58	6.75	14.65	1.43	0.82

CALIB STANDHYD (1041)
 ID= 1 DT=15.0 min

Area (ha)	Imp(%)	Dir. Conn. (%)
0.67	75.00	70.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	0.50	0.17
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	66.83	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 96.49 11.52
 over (min) 15.00 30.00

Storage Coeff. (min)= 2.51 (ii) 27.90 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.11 0.04

PEAK FLOW (cms)= 0.13 0.00 *TOTALS* 0.128 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 71.10 10.87 53.01
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.15 0.73

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 36.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5104)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.0450	0.0213
0.0020	0.0120	0.0560	0.0231
0.0130	0.0144	0.0660	0.0248
0.0240	0.0165	0.4660	0.0348
0.0340	0.0189	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
0.670	0.128	6.00	53.01
0.670	0.039	6.25	50.77

INFLOW: ID= 2 (1041)
 OUTFLOW: ID= 1 (5104)

PEAK FLOW REDUCTION [Qout/Qin](%) = 30.22
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 0.0207

CALIB NASHYD (1042)
 ID= 1 DT=15.0 min

Area (ha)= 527.14 Curve Number (CN)= 42.6
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.34

Unit Hyd Qpeak (cms)= 6.731

PEAK FLOW (cms)= 2.172 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 11.227
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.154

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9104)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1042):	527.14	2.172	8.00	11.23
+ ID2= 2 (5104):	0.67	0.039	6.25	50.77
ID = 3 (9104):	527.81	2.182	8.00	11.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8002)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6104):	1159.84	9.578	6.75	14.65
+ ID2= 2 (9104):	527.81	2.182	8.00	11.28
ID = 3 (8002):	1687.65	11.460	7.00	13.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANHDYD (1061)
 ID= 1 DT=15.0 min

Area (ha)= 32.26
 Total Imp(%)= 67.00
 Dir. Conn. (%)= 51.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 21.61 10.65
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 463.75 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 96.49 54.02
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 8.01 (ii) 21.70 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.10 0.05

PEAK FLOW (cms)= 3.95 0.78 *TOTALS* 4.438 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 71.10 24.08 48.06
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.33 0.66

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 56.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5106)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.1580	0.9744
0.1090	0.5252	2.7090	1.0694
0.6250	0.6260	3.1540	1.1625
1.1640	0.7386	3.5540	1.1725
1.6140	0.8505	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
32.260	4.438	6.00	48.06
32.260	1.391	6.50	48.02

INFLOW: ID= 2 (1061)
 OUTFLOW: ID= 1 (5106)

PEAK FLOW REDUCTION [Qout/Qin](%) = 31.34
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 0.8033

CALIB NASHYD (1062)
 ID= 1 DT=15.0 min

Area (ha)= 227.27 Curve Number (CN)= 57.5
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.09

Unit Hyd Qpeak (cms)= 3.556

PEAK FLOW (cms)= 1.761 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 17.964
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.246

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9106)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1062):	227.27	1.761	7.50	17.96
+ ID2= 2 (5106):	32.26	1.391	6.50	48.02
ID = 3 (9106):	259.53	2.901	6.50	21.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8002):	1687.65	11.460	7.00	13.60
+ ID2= 2 (9106):	259.53	2.901	6.50	21.70

ID = 3 (8004):	1947.18	14.352	6.75	14.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6108)
IN= 2---> OUT= 1 Routing time step (min)'= 15.00

Distance	Elevation	Manning	
0.00	326.06	0.0400	
12.06	325.74	0.0400	
30.15	324.28	0.0400	
59.27	320.35	0.0400	
72.37	317.60	0.0400	
87.45	314.52	0.0400	
93.48	313.91	0.0400	
105.54	313.55	0.0400	
106.17	313.40	0.0400	
110.62	313.04	0.0400 / 0.0300	Main Channel
110.82	312.97	0.0300	Main Channel
111.57	313.08	0.0300	Main Channel
111.67	313.48	0.0300 / 0.0400	Main Channel
129.66	316.62	0.0400	
150.77	318.95	0.0400	
180.92	319.61	0.0400	
205.04	321.23	0.0400	
232.18	322.09	0.0400	
268.37	322.31	0.0400	
298.52	326.00	0.0400	

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.43	313.40	.244E+04	0.7	0.59	58.83
1.09	314.06	.214E+05	10.4	1.01	34.24
1.76	314.73	.600E+05	40.2	1.39	24.89
2.42	315.39	.109E+06	92.1	1.75	19.80
3.08	316.05	.169E+06	166.5	2.05	16.88
3.75	316.72	.237E+06	264.4	2.31	14.97
4.41	317.38	.318E+06	386.3	2.52	13.72
5.07	318.04	.411E+06	539.3	2.72	12.71
5.74	318.71	.517E+06	725.5	2.91	11.88
6.40	319.37	.642E+06	909.2	2.94	11.77
7.06	320.03	.806E+06	1179.3	3.04	11.38
7.72	320.69	.989E+06	1515.9	3.18	10.87
8.39	321.36	.119E+07	1895.5	3.30	10.49
9.05	322.02	.143E+07	2310.2	3.36	10.30
9.71	322.68	.172E+07	2763.2	3.33	10.39
10.38	323.35	.205E+07	3477.6	3.53	9.81
11.04	324.01	.238E+07	4284.3	3.73	9.27
11.70	324.67	.274E+07	5154.0	3.91	8.85
12.37	325.34	.311E+07	6104.0	4.08	8.48

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8004)	1947.18	14.35	6.75	14.68	1.18
OUTFLOW: ID= 1 (6108)	1947.18	12.88	7.50	14.68	1.15

CALIB STANDHYD (1081)
ID= 1 DT=15.0 min
Area (ha)= 40.81
Total Imp(%)= 75.00
Dir. Conn.(%)= 70.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	30.61
Dep. Storage (mm)=	2.00
Average Slope (%)=	0.50
Length (m)=	521.60
Mannings n =	0.013
Max. Eff. Inten. (mm/hr)=	96.49
over (min)	15.00
	8.94
	45.00

Storage Coeff. (min)=	8.60 (ii)	36.70 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.09	0.03

PEAK FLOW (cms)=	6.74	0.14	*TOTALS* 6.799 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	71.10	8.51	52.32
TOTAL RAINFALL (mm)=	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.12	0.72

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 30.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5108)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.7300	1.2970
0.1380	0.7271	3.4270	1.4050
0.7910	0.8733	3.9900	1.5110
1.4720	1.0061	4.3900	1.5210
2.0420	1.1467	0.0000	0.0000

INFLOW : ID= 2 (1081)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5108)	40.810	6.799	6.00	52.32
	40.810	2.090	6.25	52.29

PEAK FLOW REDUCTION [Qout/Qin] (%) = 30.73
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha. m.) = 1.1595

CALIB NASHYD (1082)
ID= 1 DT=15.0 min

Area (ha)= 166.24
Ia (mm)= 5.00
U. H. Tp(hrs)= 0.89
Curve Number (CN)= 32.2
of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 3.171

PEAK FLOW (cms)=	0.621 (i)
TIME TO PEAK (hrs)=	7.250
RUNOFF VOLUME (mm)=	7.599
TOTAL RAINFALL (mm)=	73.100
RUNOFF COEFFICIENT =	0.104

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9108)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1082):	166.24	0.621	7.60
+ ID2= 2 (5108):	40.81	2.090	6.25

ID = 3 (9108):	207.05	2.569	6.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8006)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6108):	1947.18	12.880	7.50
+ ID2= 2 (9108):	207.05	2.569	6.25

ID = 3 (8006):	2154.23	14.415	7.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (1101)
ID= 1 DT=15.0 min

Area (ha)= 60.97
Total Imp(%)= 69.00 Dir. Conn.(%)= 55.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 42.07 18.90
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 637.55 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 96.49 46.68
over (min)= 15.00 30.00
Storage Coeff. (min)= 9.70 (ii) 24.20 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.09 0.04

TOTALS
PEAK FLOW (cms)= 7.66 1.13 8.361 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 71.10 21.40 48.74
TOTAL RAINFALL (mm)= 73.10 73.10 73.10
RUNOFF COEFFICIENT = 0.97 0.29 0.67

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 52.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5110)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.0780	1.8615
0.2060	1.0120	5.1210	2.0372
1.1810	1.2083	5.9610	2.2096
2.2000	1.4181	6.3610	2.2196
3.0510	1.6293	0.0000	0.0000

AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
INFLOW: ID= 2 (1101) 60.970 8.361 6.00 48.74
OUTFLOW: ID= 1 (5110) 60.970 2.671 6.50 48.72

PEAK FLOW REDUCTION [Oout/Oin] (%) = 31.95
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 1.5507

CALIB
NASHYD (1102)
ID= 1 DT=15.0 min

Area (ha)= 262.95 Curve Number (CN)= 55.4
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 1.29

Unit Hyd Opeak (cms)= 3.483
PEAK FLOW (cms)= 1.692 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 16.894
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.231

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9110)
1 + 2 = 3

Area (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
ID1= 1 (1102): 262.95 1.692 8.00 16.89
+ ID2= 2 (5110): 60.97 2.671 6.50 48.72

ID = 3 (9110): 323.92 4.015 6.50 22.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8008)
1 + 2 = 3

AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
ID1= 1 (8006): 2154.23 14.415 7.25 14.84
+ ID2= 2 (9110): 323.92 4.015 6.50 22.88

ID = 3 (8008): 2478.15 17.644 7.25 15.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6112)
IN= 2---> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (1121.0) ----->

Distance	Elevation	Manning
0.00	320.59	0.0400
13.48	317.93	0.0400
40.45	311.52	0.0400
64.04	307.68	0.0400
74.15	307.01	0.0400
91.00	307.22	0.0400
107.86	305.21	0.0400
111.23	304.86	0.0400
113.32	304.60	0.0400
117.77	304.24	0.0400 / 0.0300
117.97	304.17	0.0300
118.72	304.28	0.0300
118.82	304.68	0.0300 / 0.0400
124.71	304.76	0.0400
171.90	306.30	0.0400
205.60	307.24	0.0400
239.31	308.24	0.0400
262.90	310.64	0.0400
289.86	314.34	0.0400
333.68	317.66	0.0400

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.43	304.60	.372E+04	0.6	0.54	97.65
1.12	305.29	.463E+05	12.5	0.85	61.89
1.80	305.97	.149E+06	53.0	1.13	46.85
2.49	306.66	.311E+06	134.2	1.37	38.62
3.18	307.35	.554E+06	261.6	1.50	35.27
3.87	308.04	.894E+06	490.7	1.74	30.37
4.55	308.72	.129E+07	833.0	2.05	25.75
5.24	309.41	.171E+07	1269.8	2.36	22.39
5.93	310.10	.215E+07	1789.0	2.64	20.01
6.62	310.79	.261E+07	2392.7	2.90	18.21
7.30	311.47	.310E+07	3087.8	3.16	16.74
7.99	312.16	.361E+07	3873.6	3.40	15.52
8.68	312.85	.413E+07	4741.0	3.64	14.52
9.37	313.54	.467E+07	5689.3	3.86	13.69
10.05	314.22	.523E+07	6718.4	4.07	12.97
10.74	314.91	.581E+07	7770.7	4.24	12.46
11.43	315.60	.641E+07	8897.5	4.40	12.01
12.12	316.29	.704E+07	10113.2	4.55	11.61
12.80	316.97	.770E+07	11420.1	4.70	11.24

<----- hydrograph -----> <- pipe / channel ->
AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm) MAX DEPTH (m) MAX VEL (m/s)
INFLOW: ID= 2 (8008) 2478.15 17.64 7.25 15.89 1.21 0.88
OUTFLOW: ID= 1 (6112) 2478.15 14.59 8.25 15.89 1.15 0.86

CALIB
STANDHYD (1121)
ID= 1 DT=15.0 min

Area (ha)= 9.96
Total Imp(%)= 55.00 Dir. Conn.(%)= 25.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 5.48 4.48
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 257.68 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 96.49 14.85
over (min)= 15.00 30.00

Storage Coeff. (min)= 5.63 (ii) 28.57 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.11 0.04

PEAK FLOW (cms)= 0.64 0.13 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.25 0.715 (iii)
 RUNOFF VOLUME (mm)= 71.10 10.10 25.35
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.14 0.35

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 26.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5112)
 IN= 2----> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.6660	0.2787
0.0340	0.1406	0.8360	0.3122
0.1930	0.1653	0.9740	0.3450
0.3590	0.2034	1.3740	0.3550
0.4980	0.2383	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
9.960	0.715	6.00	25.35
9.960	0.112	6.75	25.25

INFLOW : ID= 2 (1121)
 OUTFLOW: ID= 1 (5112)

PEAK FLOW REDUCTION [Qout/Qin](%) = 15.60
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 0.1531

CALIB NASHYD (1122)
 ID= 1 DT=15.0 min

Area (ha) = 404.93 Curve Number (CN) = 31.1
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.90

Unit Hyd Qpeak (cms) = 3.635

PEAK FLOW (cms) = 0.823 (i)
 TIME TO PEAK (hrs) = 9.250
 RUNOFF VOLUME (mm) = 7.323
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.100

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9112)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
404.93	0.823	9.25	7.32
9.96	0.112	6.75	25.25
414.89	0.866	8.50	7.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8010)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
2478.15	14.592	8.25	15.89
414.89	0.866	8.50	7.75
2893.04	15.458	8.25	14.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6114)
 IN= 2----> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1141.0) ----->

Distance	Elevation	Manning
0.00	312.27	0.0500
14.07	309.70	0.0500
28.14	306.51	0.0500
35.17	305.81	0.0500
42.20	304.95	0.0500
52.75	304.24	0.0500
77.37	303.61	0.0500
98.47	301.33	0.0500
131.76	300.40	0.0500 / 0.0300
136.21	300.04	0.0300
136.41	299.97	0.0300
137.16	300.08	0.0300
137.26	300.48	0.0300 / 0.0500
140.68	300.53	0.0500
179.36	302.20	0.0500
214.53	303.82	0.0500
242.66	305.27	0.0500
288.38	307.66	0.0500
316.52	311.76	0.0500
348.17	313.48	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.43	300.40	366E+04	0.7	0.59	88.31
1.05	301.02	505E+05	12.1	0.75	69.49
1.68	301.65	165E+06	52.7	0.99	52.28
2.30	302.27	324E+06	130.3	1.26	41.40
2.93	302.90	520E+06	247.1	1.48	35.08
3.55	303.52	754E+06	406.8	1.68	30.91
4.18	304.15	104E+07	586.1	1.76	29.55
4.80	304.77	138E+07	856.0	1.93	26.96
5.43	305.40	177E+07	1201.1	2.12	24.56
6.05	306.02	219E+07	1612.2	2.30	22.63
6.68	306.65	264E+07	2090.6	2.47	21.07
7.30	307.27	313E+07	2653.0	2.64	19.65
7.93	307.90	364E+07	3303.9	2.83	18.36
8.55	308.52	417E+07	4053.9	3.03	17.14
9.18	309.15	471E+07	4871.9	3.22	16.12
9.80	309.77	527E+07	5756.2	3.41	15.25
10.43	310.40	584E+07	6699.8	3.58	14.52
11.05	311.02	642E+07	7710.5	3.74	13.89
11.68	311.65	702E+07	8788.5	3.90	13.32

<---- hydrograph ---->

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
2893.04	15.46	8.25	14.73	1.11	0.76
2893.04	13.35	9.50	14.73	1.07	0.75

CALIB STANDHYD (1141)
 ID= 1 DT=15.0 min

Area (ha) = 19.45 Dir. Conn. (%) = 15.00
 Total Imp(%) = 45.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 8.75 10.70
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 360.09 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 96.49 24.03
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 6.88 (ii) 25.80 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.10 0.04

TOTALS
 PEAK FLOW (cms) = 0.72 0.52 1.044 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 71.10 17.17 25.26
 TOTAL RAINFALL (mm) = 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.23 0.35

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 43.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5114) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	1.3010	0.5065
	0.0660	0.2376	1.6340	0.5791
	0.3770	0.2749	1.9020	0.6501
	0.7020	0.3549	2.3020	0.6601
	0.9730	0.4237	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1141)	19.450	1.044	6.00	25.26
OUTFLOW: ID= 1 (5114)	19.450	0.292	7.00	25.22

PEAK FLOW REDUCTION [Qout/Qin] (%) = 28.00
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 0.2665

CALIB NASHYD (1142) ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)
	268.19	51.3
	Ia (mm) = 5.00	# of Linear Res. (N) = 1.50
	U.H. Tp(hrs) = 1.09	

Unit Hyd Qpeak (cms) = 4.197
 PEAK FLOW (cms) = 1.708 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 14.863
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.203

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9114) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1142):	268.19	1.708	7.50	14.86
+ ID2= 2 (5114):	19.45	0.292	7.00	25.22
ID = 3 (9114):	287.64	1.951	7.00	15.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8012) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6114):	2893.04	13.354	9.50	14.73
+ ID2= 2 (9114):	287.64	1.951	7.00	15.56
ID = 3 (8012):	3180.68	14.834	9.50	14.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0116) ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)
	206.43	64.0
	Ia (mm) = 5.00	# of Linear Res. (N) = 1.50
	U.H. Tp(hrs) = 0.80	

Unit Hyd Qpeak (cms) = 4.394
 PEAK FLOW (cms) = 2.488 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 21.671
 TOTAL RAINFALL (mm) = 73.100

RUNOFF COEFFICIENT = 0.296

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8014) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0116):	206.43	2.488	7.00	21.67
+ ID2= 2 (8012):	3180.68	14.834	9.50	14.80
ID = 3 (8014):	3387.11	16.440	9.25	15.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6118) IN= 2--> OUT= 1	Routing time step (min) = 15.00
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Distance	Elevation	Manning
0.00	307.18	0.0450
24.89	299.75	0.0450
37.34	296.39	0.0450
71.57	292.75	0.0450
96.46	291.12	0.0450
108.90	289.43	0.0450
112.02	288.86	0.0450
115.13	288.46	0.0450
117.24	288.32	0.0450 / 0.0350
117.49	288.15	0.0350
118.24	288.00	0.0350
118.99	288.20	0.0350
119.24	288.36	0.0350 / 0.0450
121.35	288.40	0.0450
149.35	289.36	0.0450
171.14	290.47	0.0450
208.47	293.14	0.0450
236.48	295.17	0.0450
280.04	299.48	0.0450
308.04	307.00	0.0450

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.32	288.32	.116E+04	0.2	0.62	81.92
1.30	289.30	.631E+05	22.2	1.07	47.43
2.29	290.29	.218E+06	117.5	1.64	30.95
3.27	291.27	.448E+06	314.6	2.15	23.72
4.25	292.25	.756E+06	625.6	2.53	20.13
5.24	293.24	.115E+07	1093.2	2.91	17.50
6.22	294.22	.161E+07	1731.6	3.28	15.50
7.20	295.20	.214E+07	2547.4	3.63	14.01
8.19	296.19	.274E+07	3584.9	4.00	12.72
9.17	297.17	.338E+07	4859.1	4.39	11.60
10.15	298.15	.407E+07	6341.5	4.76	10.70
11.13	299.13	.480E+07	8024.6	5.11	9.97
12.12	300.12	.556E+07	10006.6	5.49	9.27
13.10	301.10	.635E+07	12244.6	5.89	8.64
14.08	302.08	.716E+07	14681.7	6.26	8.13
15.07	303.07	.799E+07	17316.5	6.62	7.69
16.05	304.05	.884E+07	20148.1	6.96	7.31
17.03	305.03	.971E+07	23176.2	7.29	6.98
18.02	306.02	.106E+08	26400.8	7.61	6.69

INFLOW : ID= 2 (8014)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW: ID= 1 (6118)	3387.11	15.44	10.25	15.22	1.05	0.90

CALIB NASHYD (0118) ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)
	340.53	45.0
	Ia (mm) = 5.00	# of Linear Res. (N) = 1.50
	U.H. Tp(hrs) = 1.12	

Unit Hyd Qpeak (cms) = 5.200

PEAK FLOW (cms)= 1.732 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 12.145
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.166

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8016)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0118):	340.53	1.732	7.75	12.15
+ ID2= 2 (6118):	3387.11	15.443	10.25	15.22
ID = 3 (8016):	3727.64	16.791	10.00	14.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6122)
 IN= 2----> OUT= 1
 Routing time step (min)' = 15.00

Distance	Elevation	Manning	Main Channel
0.00	277.00	0.0500	
52.18	272.82	0.0500	
86.97	269.38	0.0500	
134.80	267.35	0.0500	
160.89	263.85	0.0500	
169.58	262.88	0.0500	
178.28	262.45	0.0500	
195.67	262.07	0.0500	
213.07	260.99	0.0500	
226.01	259.80	0.0500 / 0.0300	Main Channel
226.11	259.30	0.0300	Main Channel
230.11	259.30	0.0300	Main Channel
230.21	259.80	0.0300 / 0.0500	Main Channel
235.01	260.20	0.0500	
239.15	260.57	0.0500	
243.50	260.93	0.0500	
300.03	264.43	0.0500	
339.16	268.34	0.0500	
373.95	272.36	0.0500	
430.48	277.46	0.0500	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.50	259.80	.626E+04	2.5	1.23	41.35
1.41	260.71	.462E+05	21.8	1.44	35.27
2.31	261.61	.149E+06	79.6	1.63	31.19
3.22	262.52	.340E+06	200.1	1.80	28.34
4.12	263.42	.636E+06	446.8	2.15	23.71
5.03	264.33	.995E+06	819.9	2.52	20.23
5.93	265.23	.141E+07	1337.0	2.90	17.56
6.84	266.14	.187E+07	1984.8	3.25	15.67
7.74	267.04	.237E+07	2766.0	3.57	14.26
8.65	267.95	.292E+07	3596.9	3.76	13.53
9.55	268.85	.355E+07	4575.4	3.93	12.95
10.46	269.76	.427E+07	5814.9	4.16	12.23
11.36	270.66	.503E+07	7319.0	4.44	11.46
12.27	271.57	.584E+07	9009.7	4.71	10.81
13.17	272.47	.670E+07	10886.3	4.96	10.26
14.08	273.38	.761E+07	12893.2	5.17	9.84
14.98	274.28	.858E+07	15098.1	5.37	9.48
15.89	275.19	.961E+07	17527.6	5.57	9.14
16.79	276.09	.107E+08	20190.3	5.76	8.83

INFLOW: ID= 2 (8016)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW: ID= 1 (6122)	3727.64	16.79	10.00	14.94	1.17	1.38
	3727.64	16.29	10.75	14.94	1.14	1.37

CALIB STANDHYD (1221)
 ID= 1 DT=15.0 mi n
 Area (ha)= 117.91
 Total Imp(%)= 44.00
 Di r. Conn.(%)= 16.00

Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
51.88		66.03
2.00		5.00
0.50		0.50
886.60		40.00
0.013		0.250
96.49		17.82
15.00		45.00
11.82 (ii)		33.14 (ii)
15.00		45.00
0.08		0.03
4.04		1.94
6.00		6.50
71.10		13.31
73.10		73.10
0.97		0.18

TOTALS
 4.834 (iii)
 6.00
 22.56
 73.10
 0.31

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 36.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5122)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	7.8880	3.0390	
0.3990	1.4100	9.9040	3.4850	
2.2850	1.6269	11.5290	3.9214	
4.2550	2.1164	11.9290	3.9314	
5.9010	2.5338	0.0000	0.0000	

INFLOW: ID= 2 (1221)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5122)	117.910	4.834	6.00	22.56
	117.910	1.275	7.25	22.55

PEAK FLOW REDUCTION [Out/Oi n](%)= 26.37
 TIME SHIFT OF PEAK FLOW (mi n)= 75.00
 MAXIMUM STORAGE USED (ha. m.)= 1.5135

CALIB NASHYD (1222)
 ID= 1 DT=15.0 mi n
 Area (ha)= 246.95
 I a (mm)= 5.00
 U. H. Tp (hrs)= 1.13
 Curve Number (CN)= 43.8
 # of Li near Res. (N)= 1.50

Unit Hyd Opeak (cms)= 3.728
 PEAK FLOW (cms)= 1.196 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 11.670
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.160

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8018)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (1222):	246.95	1.196	7.75	11.67
+ ID2= 2 (5122):	117.91	1.275	7.25	22.55
ID = 3 (8018):	364.86	2.453	7.25	15.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8020)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (6122):	3727.64	16.289	10.75	14.94	
+ ID2= 2 (8018):	364.86	2.453	7.25	15.19	

ID = 3 (8020):	4092.50	17.517	10.50	14.96	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	STANDHYD (1261)	Area (ha)	Imp (%)	Di r. Conn. (%)
ID= 1 DT=15.0 min		50.19	36.00	26.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	18.07	32.12
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	578.45	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	16.08
over (min)	15.00	45.00
Storage Coeff. (min)=	9.15 (ii)	31.36 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.09	0.03

			TOTALS
PEAK FLOW (cms)=	3.03	0.87	3.386 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	71.10	15.47	29.93
TOTAL RAINFALL (mm)=	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.21	0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 48.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5126)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min	0.0000	0.0000	3.3570	1.2179
	0.1700	0.5264	4.2160	1.4221
	0.9720	0.5968	4.9080	1.6217
	1.8110	0.8165	5.3080	1.6317
	2.5120	0.9954	0.0000	0.0000

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (1261)	50.190	3.386	6.00	29.93
OUTFLOW: ID= 1 (5126)	50.190	1.173	6.75	29.92

PEAK FLOW REDUCTION [Qout/Qin] (%) = 34.63
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha.m.) = 0.6586

CALIB	NASHYD (1262)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min		(mm)	# of Linear Res. (N)
		83.33	57.7
		5.00	1.50
		U.H. Tp(hrs)= 0.57	

Unit Hyd Opeak (cms) = 2.509

PEAK FLOW (cms)=	1.075 (i)
TIME TO PEAK (hrs)=	6.500
RUNOFF VOLUME (mm)=	17.806
TOTAL RAINFALL (mm)=	73.100
RUNOFF COEFFICIENT =	0.244

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	(8024)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (1262):	83.33	1.075	6.50	17.81	
+ ID2= 2 (5126):	50.19	1.173	6.75	29.92	

ID = 3 (8024):	133.52	2.241	6.75	22.36	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	NASHYD (0124)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min		(mm)	# of Linear Res. (N)
		121.43	43.0
		5.00	1.50
		U.H. Tp(hrs)= 0.51	

Unit Hyd Opeak (cms) = 4.049

PEAK FLOW (cms)=	1.047 (i)
TIME TO PEAK (hrs)=	6.500
RUNOFF VOLUME (mm)=	11.142
TOTAL RAINFALL (mm)=	73.100
RUNOFF COEFFICIENT =	0.152

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	(8026)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0124):	121.43	1.047	6.50	11.14	
+ ID2= 2 (8024):	133.52	2.241	6.75	22.36	

ID = 3 (8026):	254.95	3.280	6.50	17.02	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6128)	Routing time step (min)
IN= 2--> OUT= 1	' = 15.00

<----- DATA FOR SECTION (1281.0) ----->			
Di stance	El evation	Manning	
0.00	270.58	0.0400	
7.71	270.36	0.0400	
15.43	269.48	0.0400	
23.14	265.88	0.0400	
42.43	261.83	0.0400	
61.71	258.31	0.0400	
75.21	256.62	0.0400	
90.64	255.35	0.0400	
92.49	255.00	0.0400 / 0.0300	Mai n Channel
93.49	254.00	0.0300	Mai n Channel
94.49	253.90	0.0300	Mai n Channel
94.99	253.80	0.0300	Mai n Channel
95.49	253.70	0.0300	Mai n Channel
95.99	253.80	0.0300	Mai n Channel
98.49	255.00	0.0300 / 0.0400	Mai n Channel
100.28	255.44	0.0400	
134.99	257.45	0.0400	
148.49	259.67	0.0400	
167.77	263.42	0.0400	
190.92	270.00	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(mi n)
0.65	254.35	.439E+04	2.0	1.24	36.10
1.30	255.00	.131E+05	9.4	1.92	23.39
2.18	255.88	.433E+05	37.0	2.30	19.54
3.06	256.76	.131E+06	113.3	2.34	19.20
3.95	257.65	.273E+06	273.8	2.70	16.64
4.83	258.53	.453E+06	538.5	3.21	14.01
5.71	259.41	.658E+06	902.9	3.70	12.14
6.59	260.29	.886E+06	1368.4	4.16	10.79
7.48	261.18	.114E+07	1936.3	4.59	9.79
8.36	262.06	.141E+07	2610.9	4.99	9.01
9.24	262.94	.171E+07	3398.7	5.37	8.36
10.12	263.82	.202E+07	4309.6	5.75	7.81
11.01	264.71	.235E+07	5347.5	6.13	7.33

11.89	265.59	.270E+07	6501.0	6.48	6.93
12.77	266.47	.307E+07	7808.6	6.86	6.55
13.65	267.35	.345E+07	9250.5	7.23	6.21
14.54	268.24	.384E+07	10808.3	7.59	5.92
15.42	269.12	.424E+07	12482.5	7.94	5.66
16.30	270.00	.466E+07	14127.4	8.18	5.49

<---- hydrograph ----> <- pipe / channel ->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8026)	254.95	3.28	6.50	17.02	0.76	1.32
OUTFLOW: ID= 1 (6128)	254.95	2.76	7.25	17.02	0.72	1.29

CALIB STANDHYD (1281) ID= 1 DT=15.0 min	Area Total	(ha)= 55.87	Di r. Conn. (%)=	29.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	27.93	27.93
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	610.30	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	96.49	46.60
over (min)	15.00	30.00
Storage Coeff. (min)	9.44 (ii)	23.96 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.04
PEAK FLOW (cms)	3.73	1.67
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	71.10	21.81
TOTAL RAINFALL (mm)	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.30

TOTALS
4.770 (iii)
6.00
36.11
73.10
0.49

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 54.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5128) IN= 2--> OUT= 1 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.7380	1.5139
0.1890	0.7401	4.6930	1.7113
1.0830	0.8643	5.4630	1.9045
2.0160	1.0853	5.8630	1.9045
2.7960	1.2819	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (1281)	55.870	4.770	6.00	36.11
OUTFLOW: ID= 1 (5128)	55.870	1.564	6.75	36.09

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 32.78
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.9932

CALIB NASHYD (1282) ID= 1 DT=15.0 min

Area Ia	(ha)= (mm)=	184.36 5.00	Curve Number (CN)=	62.0
U.H. Tp	(hrs)=	0.62	# of Linear Res. (N)=	1.50

Unit Hyd Qpeak (cms)	5.093
PEAK FLOW (cms)	2.546 (i)
TIME TO PEAK (hrs)	6.750
RUNOFF VOLUME (mm)	20.293
TOTAL RAINFALL (mm)	73.100
RUNOFF COEFFICIENT	0.278

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9128) 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1282):	184.36	2.546	6.75	20.29
+ ID2= 2 (5128):	55.87	1.564	6.75	36.09
ID = 3 (9128):	240.23	4.110	6.75	23.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8028) 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6128):	254.95	2.765	7.25	17.02
+ ID2= 2 (9128):	240.23	4.110	6.75	23.97
ID = 3 (8028):	495.18	6.556	7.00	20.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8022) 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8020):	4092.50	17.517	10.50	14.96
+ ID2= 2 (8028):	495.18	6.556	7.00	20.39
ID = 3 (8022):	4587.68	19.611	10.25	15.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1201) ID= 1 DT=15.0 min

Area Total	(ha)= 176.35	Di r. Conn. (%)=	22.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	79.36	96.99
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1084.28	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	96.49	23.17
over (min)	15.00	45.00
Storage Coeff. (min)	13.33 (ii)	32.53 (ii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.08	0.03

PEAK FLOW (cms)	7.95	3.72
TIME TO PEAK (hrs)	6.00	6.50
RUNOFF VOLUME (mm)	71.10	17.99
TOTAL RAINFALL (mm)	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.25

TOTALS
9.486 (iii)
6.00
29.67
73.10
0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 47.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5120) IN= 2--> OUT= 1 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	11.7960	4.5929
0.5970	2.1554	14.8120	5.2509
3.4170	2.4937	17.2430	5.8948
6.3630	3.2187	17.6430	5.9048
8.8260	3.8422	0.0000	0.0000

INFLOW : ID= 2 (1201) AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 176.350 9.486 6.00 29.67
 OUTFLOW: ID= 1 (5120) 176.350 3.584 6.75 29.67

PEAK FLOW REDUCTION [Qout/Qin] (%) = 37.78
 TIME SHIF T OF PEAK FLOW (mi n) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 2.5555

CALIB STANDHYD (1202) | Area (ha) = 70.67
 ID= 1 DT=15.0 mi n | Total Imp(%) = 43.00 Di r. Conn. (%) = 20.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 30.39 40.28
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 686.39 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 96.49 22.72
 over (mi n) = 15.00 30.00
 Storage Coeff. (mi n) = 10.13 (ii) 29.49 (ii)
 Unit Hyd. Tpeak (mi n) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.09 0.04

TOTALS
 PEAK FLOW (cms) = 3.19 1.73 4.244 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 71.10 17.83 28.49
 TOTAL RAINFALL (mm) = 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.24 0.39

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 47.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8030) | Area (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 1 + 2 = 3 |
 ID1= 1 (1202): 70.67 4.244 6.00 28.49
 + ID2= 2 (5120): 176.35 3.584 6.75 29.67
 ID = 3 (8030): 247.02 4.950 6.75 29.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8032) | Area (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 1 + 2 = 3 |
 ID1= 1 (8022): 4587.68 19.611 10.25 15.55
 + ID2= 2 (8030): 247.02 4.950 6.75 29.33
 ID = 3 (8032): 4834.70 20.540 10.25 16.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6130) | Routing time step (mi n)' = 15.00
 IN= 2----> OUT= 1 |

<----- DATA FOR SECTION (1301.0) ----->
 Di stance El evati on Manni ng
 0.00 257.18 0.0350
 62.45 254.70 0.0350
 109.29 252.74 0.0350
 150.92 251.55 0.0350
 192.56 248.96 0.0350
 239.39 247.16 0.0350

291.84	246.00	0.0350		
296.84	245.00	0.0350		
298.84	245.00	0.0350 / 0.0300	Mai n	Channel
299.24	244.60	0.0300	Mai n	Channel
301.84	244.60	0.0300	Mai n	Channel
303.44	244.60	0.0300	Mai n	Channel
303.84	245.00	0.0300 / 0.0350	Mai n	Channel
306.84	245.00	0.0350		
309.84	246.00	0.0350		
317.46	247.22	0.0350		
348.68	254.83	0.0350		
359.09	256.70	0.0350		
452.76	258.23	0.0350		
515.22	258.33	0.0350		

<----- TRAVEL TIME TABLE ----->
 DEPTH (m) ELEV (m) VOLUME (cu. m.) FLOW RATE (cms) VELOCITY (m/s) TRAV. TIME (mi n)
 0.40 245.00 .870E+04 1.2 0.64 124.12
 1.04 245.64 .468E+05 9.5 0.96 81.81
 1.68 246.28 .109E+06 25.6 1.11 70.86
 2.32 246.92 .257E+06 63.5 1.17 67.51
 2.96 247.56 .498E+06 142.7 1.36 58.18
 3.61 248.21 .800E+06 266.7 1.58 50.01
 4.25 248.85 .116E+07 437.4 1.78 44.24
 4.89 249.49 .157E+07 669.1 2.01 39.20
 5.53 250.13 .203E+07 954.9 2.23 35.36
 6.17 250.77 .252E+07 1294.4 2.43 32.42
 6.81 251.41 .305E+07 1689.6 2.62 30.07
 7.45 252.05 .363E+07 2090.7 2.73 28.93
 8.09 252.69 .429E+07 2553.1 2.82 27.98
 8.73 253.33 .501E+07 3138.4 2.96 26.60
 9.37 253.97 .579E+07 3803.8 3.11 25.36
 10.02 254.62 .662E+07 4549.3 3.25 24.25
 10.66 255.26 .751E+07 5364.9 3.38 23.32
 11.30 255.90 .845E+07 6265.7 3.51 22.49
 11.94 256.54 .946E+07 7260.1 3.63 21.72

<---- hydrograph ----> <-pi pe / channel ->
 AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm) MAX DEPTH (m) MAX VEL (m/s)
 INFLOW : ID= 2 (8032) 4834.70 20.54 10.25 16.25 1.48 1.06
 OUTFLOW: ID= 1 (6130) 4834.70 19.27 11.25 16.25 1.43 1.05

CALIB STANDHYD (1301) | Area (ha) = 320.20
 ID= 1 DT=15.0 mi n | Total Imp(%) = 64.00 Di r. Conn. (%) = 47.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 204.93 115.27
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 1461.05 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 96.49 87.73
 over (mi n) = 15.00 30.00
 Storage Coeff. (mi n) = 15.95 (ii) 27.22 (ii)
 Unit Hyd. Tpeak (mi n) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.07 0.04

TOTALS
 PEAK FLOW (cms) = 28.61 12.38 36.611 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 71.10 38.47 53.81
 TOTAL RAINFALL (mm) = 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.53 0.74

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5130) |
 IN= 2----> OUT= 1 |
 DT= 15.0 mi n |
 OUTFLOW (cms) STORAGE (ha. m.) | OUTFLOW (cms) STORAGE (ha. m.)
 0.0000 0.0000 | 18.6040 9.7083

1. 1320	5. 3237	22. 6740	10. 8355
6. 6360	6. 1006	30. 4650	12. 2155
10. 3780	7. 2341	31. 8650	12. 2255
14. 1960	8. 4233	0. 0000	0. 0000

INFLOW : ID= 2 (1301)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5130)	320. 200	36. 611	6. 00	53. 81
	320. 200	13. 954	6. 75	53. 80

PEAK FLOW REDUCTION [Qout/Qin] (%) = 38. 11
 TIME SHIFT OF PEAK FLOW (min) = 45. 00
 MAXIMUM STORAGE USED (ha. m.) = 8. 4490

CALIB NASHYD (1302)	Area (ha)	Curve Number (CN)
ID= 1 DT=15. 0 min	65. 86	80. 0
	U. H. Tp (hrs) = 5. 00	# of Linear Res. (N) = 1. 50
		0. 69

Unit Hyd Qpeak (cms) = 1. 620

PEAK FLOW (cms)	1. 474 (i)
TIME TO PEAK (hrs)	6. 750
RUNOFF VOLUME (mm)	34. 623
TOTAL RAINFALL (mm)	73. 100
RUNOFF COEFFICIENT	0. 474

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8036)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (1302):	65. 86	1. 474	6. 75	34. 62
+ ID2= 2 (5130):	320. 20	13. 954	6. 75	53. 80
ID = 3 (8036):	386. 06	15. 428	6. 75	50. 53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8034)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (6130):	4834. 70	19. 265	11. 25	16. 25
+ ID2= 2 (8036):	386. 06	15. 428	6. 75	50. 53
ID = 3 (8034):	5220. 76	22. 423	7. 25	18. 79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1861)	Area (ha)	Dir. Conn. (%)
ID= 1 DT=15. 0 min	6. 85	27. 00
	Total Imp (%) = 51. 00	

Surface Area (ha)	3. 49	PERVIOUS (i)	3. 36
Dep. Storage (mm)	2. 00		5. 00
Average Slope (%)	0. 50		0. 50
Length (m)	213. 70		40. 00
Mannings n	0. 013		0. 250

Max. Eff. Inten. (mm/hr) over (min)	96. 49	48. 62
Storage Coeff. (min)	15. 00	30. 00
Unit Hyd. Tpeak (min)	5. 03 (ii)	19. 31 (ii)
Unit Hyd. peak (cms)	15. 00	30. 00
	0. 11	0. 05

PEAK FLOW (cms)	0. 48	0. 23	*TOTALS*
TIME TO PEAK (hrs)	6. 00	6. 25	0. 625 (iii)
RUNOFF VOLUME (mm)	71. 10	21. 72	6. 00
TOTAL RAINFALL (mm)	73. 10	73. 10	35. 05
RUNOFF COEFFICIENT	0. 97	0. 30	73. 10
			0. 48

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 52. 5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5186)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2--> OUT= 1				
DT= 15. 0 min	0. 0000	0. 0000	0. 4580	0. 1865
	0. 0230	0. 0917	0. 5750	0. 2106
	0. 1330	0. 1071	0. 6700	0. 2341
	0. 2470	0. 1341	1. 0700	0. 2441
	0. 3430	0. 1582	0. 0000	0. 0000

INFLOW : ID= 2 (1861)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5186)	6. 850	0. 625	6. 00	35. 05
	6. 850	0. 186	6. 50	34. 90

PEAK FLOW REDUCTION [Qout/Qin] (%) = 29. 82
 TIME SHIFT OF PEAK FLOW (min) = 30. 00
 MAXIMUM STORAGE USED (ha. m.) = 0. 1212

CALIB STANDHYD (1862)	Area (ha)	Dir. Conn. (%)
ID= 1 DT=15. 0 min	344. 94	23. 00
	Total Imp (%) = 43. 00	

Surface Area (ha)	148. 32	PERVIOUS (i)	196. 62
Dep. Storage (mm)	2. 00		5. 00
Average Slope (%)	0. 50		0. 50
Length (m)	1516. 44		40. 00
Mannings n	0. 013		0. 250

Max. Eff. Inten. (mm/hr) over (min)	96. 49	24. 87
Storage Coeff. (min)	15. 00	45. 00
Unit Hyd. Tpeak (min)	16. 31 (ii)	34. 97 (ii)
Unit Hyd. peak (cms)	15. 00	45. 00
	0. 07	0. 03

PEAK FLOW (cms)	14. 93	7. 79	*TOTALS*
TIME TO PEAK (hrs)	6. 00	6. 50	18. 164 (iii)
RUNOFF VOLUME (mm)	71. 10	20. 11	6. 00
TOTAL RAINFALL (mm)	73. 10	73. 10	31. 84
RUNOFF COEFFICIENT	0. 97	0. 28	73. 10
			0. 44

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 52. 5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9186)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (1862):	344. 94	18. 164	6. 00	31. 84
+ ID2= 2 (5186):	6. 85	0. 186	6. 50	34. 90
ID = 3 (9186):	351. 79	18. 183	6. 00	31. 90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1881)	Area (ha)	Dir. Conn. (%)
ID= 1 DT=15. 0 min	1. 12	46. 00
	Total Imp (%) = 75. 00	

Surface Area (ha)	0. 84	PERVIOUS (i)	0. 28
Dep. Storage (mm)	2. 00		5. 00
Average Slope (%)	0. 50		0. 50
Length (m)	86. 41		40. 00
Mannings n	0. 013		0. 250

Max. Eff. Inten. (mm/hr)= 96.49 109.30
 over (min)= 15.00 15.00
 Storage Coeff. (min)= 2.92 (ii) 13.25 (ii)
 Unit Hyd. Tpeak (min)= 15.00 15.00
 Unit Hyd. peak (cms)= 0.11 0.08

TOTALS
 PEAK FLOW (cms)= 0.14 0.06 0.200 (iii)
 TIME TO PEAK (hrs)= 6.00 6.00
 RUNOFF VOLUME (mm)= 71.10 33.15 50.60
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.45 0.69

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5188)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0750	0.0357
0.0040	0.0200	0.0940	0.0386
0.0220	0.0240	0.1100	0.0416
0.0410	0.0277	0.5100	0.0516
0.0560	0.0315	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1881)	1.120	0.200	6.00	50.60
OUTFLOW: ID= 1 (5188)	1.120	0.054	6.25	49.48

PEAK FLOW REDUCTION [Qout/Qin] (%) = 26.76
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0311

CALIB STANDHYD (1882)
 ID= 1 DT=15.0 min

Area (ha)= 307.41
 Total Imp(%)= 41.00
 Dir. Conn. (%) = 25.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	126.04	181.37
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1431.57	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 96.49 44.71
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 15.75 (ii) 30.51 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.07 0.03

TOTALS
 PEAK FLOW (cms)= 14.69 8.45 18.232 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50
 RUNOFF VOLUME (mm)= 71.10 23.25 35.21
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.32 0.48

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9188)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1882):	307.41	18.232	6.00	35.21
+ ID2= 2 (5188):	1.12	0.054	6.25	49.48
-----	-----	-----	-----	-----
ID = 3 (9188):	308.53	18.249	6.00	35.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8068)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9186):	351.79	18.183	6.00	31.90
+ ID2= 2 (9188):	308.53	18.249	6.00	35.27
-----	-----	-----	-----	-----
ID = 3 (8068):	660.32	36.432	6.00	33.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1821)
 ID= 1 DT=15.0 min

Area (ha)= 111.80
 Total Imp(%)= 40.00
 Dir. Conn. (%) = 17.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	44.72	67.08
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	863.33	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 96.49 25.60
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 11.63 (ii) 30.08 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03

TOTALS
 PEAK FLOW (cms)= 4.10 2.95 5.324 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 71.10 20.20 28.86
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.28 0.39

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 52.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5182)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	5.7800	2.5656
0.3790	1.0207	7.3720	3.0829
1.6240	1.1939	8.7710	3.5956
3.0930	1.6631	9.1710	3.6056
4.3530	2.1032	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1821)	111.800	5.324	6.00	28.86
OUTFLOW: ID= 1 (5182)	111.800	2.297	7.00	28.85

PEAK FLOW REDUCTION [Qout/Qin] (%) = 43.14
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 1.4169

CALIB NASHYD (1822)
 ID= 1 DT=15.0 min

Area (ha)= 44.40
 Ia (mm)= 5.00
 U. H. Tp(hrs)= 0.79
 Curve Number (CN)= 57.0
 # of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 0.966

PEAK FLOW (cms)= 0.437 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 17.596

TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.241

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8062) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1822):	44.40	0.437	7.00	17.60
+ ID2= 2 (5182):	111.80	2.297	7.00	28.85

ID = 3 (8062):	156.20	2.734	7.00	25.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1841) ID= 1 DT=15.0 min	Area (ha)	Total Imp(%)	Dir. Conn. (%)
	145.07	48.00	19.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	69.63	75.44
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	983.43	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (min)	96.49 / 15.00	57.93 / 30.00
Storage Coeff. (min)	12.57 (ii)	25.88 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04

TOTALS
 PEAK FLOW (cms) = 5.77
 TIME TO PEAK (hrs) = 6.00
 RUNOFF VOLUME (mm) = 71.10
 TOTAL RAINFALL (mm) = 73.10
 RUNOFF COEFFICIENT = 0.97

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 55.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5184) IN= 2--- OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	5.9270	2.3472
	0.2980	1.0088	7.4630	2.7585
	1.6660	1.1775	8.7490	3.1642
	3.1100	1.4809	9.1490	3.1742
	4.4100	1.8897	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1841)	145.070	9.164	6.00	33.44
OUTFLOW: ID= 1 (5184)	145.070	4.432	6.75	33.43

PEAK FLOW REDUCTION [Qout/Qin] (%) = 48.36
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 1.9293

CALIB STANDHYD (1842) ID= 1 DT=15.0 min	Area (ha)	Total Imp(%)	Dir. Conn. (%)
	117.45	42.00	17.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	49.33	68.12
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50

Length (m)	= 884.87	40.00
Mannings n	= 0.013	0.250
Max. Eff. Inten. (mm/hr) over (min)	96.49 / 15.00	49.90 / 30.00
Storage Coeff. (min)	11.80 (ii)	25.93 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04

TOTALS
 PEAK FLOW (cms) = 4.28
 TIME TO PEAK (hrs) = 6.00
 RUNOFF VOLUME (mm) = 71.10
 TOTAL RAINFALL (mm) = 73.10
 RUNOFF COEFFICIENT = 0.97

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 55.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8060) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1842):	117.45	6.897	6.00	31.27
+ ID2= 2 (5184):	145.07	4.432	6.75	33.43

ID = 3 (8060):	262.52	9.572	6.25	32.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8064) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8060):	262.52	9.572	6.25	32.46
+ ID2= 2 (8062):	156.20	2.734	7.00	25.65

ID = 3 (8064):	418.72	10.794	6.50	29.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1781) ID= 1 DT=15.0 min	Area (ha)	Total Imp(%)	Dir. Conn. (%)
	55.50	39.00	13.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	21.65	33.86
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	608.28	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (min)	96.49 / 15.00	81.70 / 30.00
Storage Coeff. (min)	9.43 (ii)	21.02 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.05

TOTALS
 PEAK FLOW (cms) = 1.66
 TIME TO PEAK (hrs) = 6.00
 RUNOFF VOLUME (mm) = 71.10
 TOTAL RAINFALL (mm) = 73.10
 RUNOFF COEFFICIENT = 0.97

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 74.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5178)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	6.1870	0.7248
0.1880	0.3394	9.3670	0.8313
1.7260	0.4003	10.7440	0.9285
3.2000	0.4933	11.1440	0.9385
4.6710	0.6039	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1781)	55.500	4.373	6.25	41.44
OUTFLOW: ID= 1 (5178)	55.500	3.784	6.25	41.43

PEAK FLOW REDUCTION [Qout/Oin] (%) = 86.54
TIME SHIFT OF PEAK FLOW (min) = 0.00
MAXIMUM STORAGE USED (ha. m.) = 0.5637

CALIB NASHYD (1782)
ID= 1 DT=15.0 min

Area (ha) = 219.91
Curve Number (CN) = 77.6
U. H. Tp (hrs) = 0.76
of Linear Res. (N) = 1.50

Unit Hyd Opeak (cms) = 4.921

PEAK FLOW (cms) = 4.212 (i)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 32.294
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.442

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8046)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1782):	219.91	4.212	7.00	32.29
+ ID2= 2 (5178):	55.50	3.784	6.25	41.43
=====				
ID = 3 (8046):	275.41	7.727	6.50	34.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0180)
ID= 1 DT=15.0 min

Area (ha) = 226.65
Curve Number (CN) = 65.0
U. H. Tp (hrs) = 1.17
of Linear Res. (N) = 1.50

Unit Hyd Opeak (cms) = 3.299

PEAK FLOW (cms) = 2.096 (i)
TIME TO PEAK (hrs) = 7.750
RUNOFF VOLUME (mm) = 22.455
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.307

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8050)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0180):	226.65	2.096	7.75	22.45
+ ID2= 2 (8046):	275.41	7.727	6.50	34.14
=====				
ID = 3 (8050):	502.06	9.487	6.50	28.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1741)
ID= 1 DT=15.0 min

Area Total (ha) = 56.30
Imp (%) = 35.00
Dir. Conn. (%) = 18.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 19.70
Dep. Storage (mm) = 2.00
Average Slope (%) = 0.50
Length (m) = 612.64
Mannings n = 0.013

Max. Eff. Inten. (mm/hr) = 96.49
over (min) = 15.00
Storage Coeff. (min) = 9.47 (ii)
Unit Hyd. Tpeak (min) = 15.00
Unit Hyd. peak (cms) = 0.09

PEAK FLOW (cms) = 2.33
TIME TO PEAK (hrs) = 6.00
RUNOFF VOLUME (mm) = 71.10
TOTAL RAINFALL (mm) = 73.10
RUNOFF COEFFICIENT = 0.97

TOTALS

3.833 (iii)
6.00
34.29
73.10
0.47

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5174)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	1.1370	1.3070
0.0580	0.5427	1.4280	1.5354
0.3290	0.6196	1.6620	1.7593
0.6130	0.8627	2.0620	1.7693
0.8510	1.0621	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1741)	56.300	3.833	6.00	34.29
OUTFLOW: ID= 1 (5174)	56.300	0.866	7.25	34.25

PEAK FLOW REDUCTION [Qout/Oin] (%) = 22.58
TIME SHIFT OF PEAK FLOW (min) = 75.00
MAXIMUM STORAGE USED (ha. m.) = 1.0796

CALIB NASHYD (1742)
ID= 1 DT=15.0 min

Area (ha) = 82.80
Curve Number (CN) = 71.0
U. H. Tp (hrs) = 0.69
of Linear Res. (N) = 1.50

Unit Hyd Opeak (cms) = 2.055

PEAK FLOW (cms) = 1.397 (i)
TIME TO PEAK (hrs) = 6.750
RUNOFF VOLUME (mm) = 26.508
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.363

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8044)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1742):	82.80	1.397	6.75	26.51
+ ID2= 2 (5174):	56.30	0.866	7.25	34.25
=====				
ID = 3 (8044):	139.10	2.246	7.00	29.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB NASHYD (0172) | Area (ha)= 202.23 | Curve Number (CN)= 78.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.89

Unit Hyd Qpeak (cms)= 3.875

PEAK FLOW (cms)= 3.482 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 32.785
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.448

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB NASHYD (0168) | Area (ha)= 247.13 | Curve Number (CN)= 52.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.75

Unit Hyd Qpeak (cms)= 5.611

PEAK FLOW (cms)= 2.144 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 15.089
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.206

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB NASHYD (0170) | Area (ha)= 166.91 | Curve Number (CN)= 65.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.80

Unit Hyd Qpeak (cms)= 3.553

PEAK FLOW (cms)= 2.075 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 22.317
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.305

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8040) | AREA | OPEAK | TPEAK | R. V.
 1 + 2 = 3 | (ha) | (cms) | (hrs) | (mm)
 ID1= 1 (0168): 247.13 2.144 7.00 15.09
 + ID2= 2 (0170): 166.91 2.075 7.00 22.32
 ID= 3 (8040): 414.04 4.218 7.00 18.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8042) | AREA | OPEAK | TPEAK | R. V.
 1 + 2 = 3 | (ha) | (cms) | (hrs) | (mm)
 ID1= 1 (0172): 202.23 3.482 7.00 32.79
 + ID2= 2 (8040): 414.04 4.218 7.00 18.00
 ID= 3 (8042): 616.27 7.700 7.00 22.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6174) | Routing time step (min)'= 15.00
 IN= 2--> OUT= 1 |
 <----- DATA FOR SECTION (1741.0) ----->

Distance	Elevation	Manning
0.00	296.00	0.0450
13.36	288.94	0.0450
26.71	288.64	0.0450
51.19	287.96	0.0450
73.45	287.32	0.0450
86.81	286.48	0.0450
102.39	285.08	0.0450
110.73	282.76	0.0450
115.73	281.06	0.0450 / 0.0350
115.74	280.35	0.0350
120.73	280.35	0.0350
120.74	281.10	0.0350 / 0.0450
125.74	282.64	0.0450
135.78	286.87	0.0450
149.13	289.58	0.0450
162.49	292.29	0.0450
175.84	293.95	0.0450
195.88	295.13	0.0450
213.68	296.02	0.0450
220.36	296.09	0.0450

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.71	281.06	.136E+05	6.0	1.69	37.89
1.50	281.85	.358E+05	22.5	2.42	26.43
2.28	282.63	.726E+05	53.8	2.85	22.47
3.07	283.42	.124E+06	104.1	3.24	19.79
3.86	284.21	.189E+06	176.7	3.60	17.81
4.64	284.99	.268E+06	274.9	3.94	16.26
5.43	285.78	.369E+06	390.8	4.07	15.73
6.21	286.56	.501E+06	551.6	4.23	15.15
7.00	287.35	.674E+06	750.5	4.28	14.97
7.79	288.14	.920E+06	993.3	4.15	15.44
8.57	288.92	.126E+07	1361.5	4.14	15.47
9.36	289.71	.167E+07	1986.0	4.56	14.04
10.15	290.50	.210E+07	2739.7	5.02	12.76
10.93	291.28	.254E+07	3615.0	5.47	11.70
11.72	292.07	.300E+07	4608.8	5.91	10.83
12.50	292.85	.347E+07	5693.3	6.30	10.16
13.29	293.64	.397E+07	6892.9	6.67	9.60
14.08	294.43	.450E+07	8150.4	6.97	9.19
14.86	295.21	.506E+07	9525.0	7.23	8.86

<---- hydrograph ----> <- pipe / channel -->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8042)	616.27	7.70	7.00	22.85	0.79	1.74
OUTFLOW: ID= 1 (6174)	616.27	7.05	7.75	22.85	0.76	1.72

 ADD HYD (8048) | AREA | OPEAK | TPEAK | R. V.
 1 + 2 = 3 | (ha) | (cms) | (hrs) | (mm)
 ID1= 1 (6174): 616.27 7.054 7.75 22.85
 + ID2= 2 (8044): 139.10 2.246 7.00 29.64
 ID= 3 (8048): 755.37 9.077 7.75 24.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8052) | AREA | OPEAK | TPEAK | R. V.
 1 + 2 = 3 | (ha) | (cms) | (hrs) | (mm)
 ID1= 1 (8048): 755.37 9.077 7.75 24.10
 + ID2= 2 (8050): 502.06 9.487 6.50 28.86
 ID= 3 (8052): 1257.43 16.293 7.00 26.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8058) | AREA | OPEAK | TPEAK | R. V.
 1 + 2 = 3 | (ha) | (cms) | (hrs) | (mm)

ID1= 1 (8052):	1257.43	16.293	7.00	26.00
+ ID2= 2 (8064):	418.72	10.794	6.50	29.92

ID = 3 (8058):	1676.15	26.528	6.50	26.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1762) ID= 1 DT=15.0 min	Area (ha)= 162.00 Total Imp(%)= 26.00	Di r. Conn. (%)= 11.00
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	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	(ha)= 42.12	119.88
Dep. Storage	(mm)= 2.00	5.00
Average Slope	(%)= 0.50	0.50
Length	(m)= 1039.23	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	24.46
over (min)	15.00	45.00
Storage Coeff. (min)=	13.00 (ii)	31.79 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03

TOTALS	(cms)= 3.69	4.89	5.848 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.50
RUNOFF VOLUME (mm)=	71.10	22.04	27.43
TOTAL RAINFALL (mm)=	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.30	0.38

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1761) ID= 1 DT=15.0 min	Area (ha)= 48.80 Total Imp(%)= 50.00	Di r. Conn. (%)= 21.00
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	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	(ha)= 24.40	24.40
Dep. Storage	(mm)= 2.00	5.00
Average Slope	(%)= 0.50	0.50
Length	(m)= 570.38	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	64.60
over (min)	15.00	30.00
Storage Coeff. (min)=	9.07 (ii)	21.81 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.05

TOTALS	(cms)= 2.38	2.13	3.733 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	71.10	26.93	36.20
TOTAL RAINFALL (mm)=	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.37	0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5176) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	3.2640	1.3175
	0.1650	0.6418	4.0090	1.4908
	0.9450	0.7489	4.7720	1.6604

1.7610	0.9426	5.1720	1.6704
2.4420	1.1144	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1761)	48.800	3.733	6.00	36.20
OUTFLOW: ID= 1 (5176)	48.800	1.394	6.75	36.18

PEAK FLOW REDUCTION [Qout/Qin] (%) = 37.36
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha.m.) = 0.8625

ADD HYD (8056) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1762):	162.00	5.848	6.50	27.43
+ ID2= 2 (5176):	48.80	1.394	6.75	36.18

ID = 3 (8056):	210.80	7.163	6.50	29.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8066) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8056):	210.80	7.163	6.50	29.46
+ ID2= 2 (8058):	1676.15	26.528	6.50	26.98

ID = 3 (8066):	1886.95	33.691	6.50	27.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8070) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8066):	1886.95	33.691	6.50	27.26
+ ID2= 2 (8068):	660.32	36.432	6.00	33.47

ID = 3 (8070):	2547.27	59.559	6.50	28.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6190) IN= 2--> OUT= 1	Routing time step (min)' = 15.00
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<----- DATA FOR SECTION (1901.0) ----->

Distance	Elevation	Manning	
0.00	261.34	0.0600	
19.76	261.64	0.0600	
59.28	261.29	0.0600	
79.04	261.09	0.0600	
103.74	255.99	0.0600	
153.13	253.42	0.0600	
192.65	252.78	0.0600	
212.29	249.80	0.0600	
217.29	249.50	0.0600 / 0.0350	Main Channel
217.30	249.05	0.0350	Main Channel
222.29	249.05	0.0350	Main Channel
222.30	249.50	0.0350 / 0.0600	Main Channel
227.30	249.80	0.0600	
232.17	252.31	0.0600	
276.63	253.53	0.0600	
326.03	256.97	0.0600	
355.67	257.30	0.0600	
400.12	260.08	0.0600	
469.28	261.25	0.0600	
489.04	262.53	0.0600	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	TRAVEL TIME (min)	TRAVEL TIME (min)
0.45	249.50	436E+04	1.0	75.97
1.07	250.12	205E+05	5.4	62.96

1.70	250.75	.451E+05	14.8	0.64	50.78
2.32	251.37	.762E+05	29.2	0.74	43.46
2.94	251.99	.114E+06	49.1	0.84	38.61
3.57	252.62	.161E+06	70.2	0.85	38.18
4.19	253.24	.248E+06	93.4	0.73	44.29
4.81	253.86	.398E+06	154.9	0.76	42.79
5.44	254.49	.576E+06	246.1	0.83	38.99
6.06	255.11	.779E+06	362.7	0.90	35.79
6.68	255.73	.101E+07	506.5	0.98	33.15
7.30	256.35	.126E+07	687.6	1.06	30.52
7.93	256.98	.153E+07	902.8	1.15	28.18
8.55	257.60	.183E+07	1114.3	1.18	27.39
9.17	258.22	.216E+07	1407.8	1.26	25.55
9.80	258.85	.250E+07	1735.4	1.35	24.03
10.42	259.47	.286E+07	2097.5	1.42	22.73
11.04	260.09	.324E+07	2492.3	1.49	21.63
11.67	260.72	.364E+07	2837.8	1.51	21.39

<---- hydrograph ---->						<-pi pe / channel -->	
AREA	QPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL		
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW : ID= 2 (8070)	2547.27	59.56	6.50	28.87	3.25	0.84	
OUTFLOW: ID= 1 (6190)	2547.27	45.52	6.75	28.87	2.84	0.82	

CALIB STANDHYD (1901)			
ID= 1 DT=15.0 min	Area (ha)= 13.49	Total Imp(%)= 58.00	Di r. Conn.(%)= 31.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)= 7.82		5.67	
Dep. Storage (mm)= 2.00		5.00	
Average Slope (%)= 0.50		0.50	
Length (m)= 299.89		40.00	
Mannings n = 0.013		0.250	
Max. Eff. Inten. (mm/hr)= 96.49		92.22	
over (min)= 15.00		30.00	
Storage Coeff. (min)= 6.17 (ii)		17.22 (ii)	
Unit Hyd. Tpeak (min)= 15.00		30.00	
Unit Hyd. peak (cms)= 0.10		0.05	
PEAK FLOW (cms)= 1.06		0.78	*TOTALS*
TIME TO PEAK (hrs)= 6.00		6.25	1.571 (iii)
RUNOFF VOLUME (mm)= 71.10		36.42	6.00
TOTAL RAINFALL (mm)= 73.10		73.10	47.17
RUNOFF COEFFICIENT = 0.97		0.50	73.10
			0.65

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5190)			
IN= 2--> OUT= 1			
DT= 15.0 min			
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.9030	0.3849
0.0460	0.1975	1.1330	0.4289
0.2610	0.2331	1.3190	0.4720
0.4870	0.2836	1.7190	0.4820
0.6750	0.3308	0.0000	0.0000
AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1901)	13.490	1.571	6.00
OUTFLOW: ID= 1 (5190)	13.490	0.587	47.17

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 37.35
 TIME SHIF T OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 0.3104

CALIB STANDHYD (1902)			
ID= 1 DT=15.0 min	Area (ha)= 274.50	Total Imp(%)= 52.00	Di r. Conn.(%)= 28.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)= 142.74		131.76	
Dep. Storage (mm)= 2.00		5.00	
Average Slope (%)= 0.50		0.50	
Length (m)= 1352.77		40.00	
Mannings n = 0.013		0.250	
Max. Eff. Inten. (mm/hr)= 96.49		80.00	
over (min)= 15.00		30.00	
Storage Coeff. (min)= 15.23 (ii)		26.92 (ii)	
Unit Hyd. Tpeak (min)= 15.00		30.00	
Unit Hyd. peak (cms)= 0.07		0.04	
PEAK FLOW (cms)= 14.91		12.92	*TOTALS*
TIME TO PEAK (hrs)= 6.00		6.25	23.175 (iii)
RUNOFF VOLUME (mm)= 71.10		34.61	6.00
TOTAL RAINFALL (mm)= 73.10		73.10	44.83
RUNOFF COEFFICIENT = 0.97		0.47	73.10
			0.61

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9190)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1902):	274.50	23.175	6.00	44.83
+ ID2= 2 (5190):	13.49	0.587	6.50	47.09
ID = 3 (9190):	287.99	23.219	6.00	44.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8072)				
1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6190):	2547.27	45.516	6.75	28.87
+ ID2= 2 (9190):	287.99	23.219	6.00	44.94
ID = 3 (8072):	2835.26	57.916	6.50	30.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6192)		Routing time step (min)' = 15.00
IN= 2--> OUT= 1		

<----- DATA FOR SECTION (1921.0) ----->			
Distance	Elevation	Manning	
0.00	264.40	0.0500	
3.64	263.94	0.0500	
14.54	261.05	0.0500	
29.08	256.78	0.0500	
36.36	254.96	0.0500	
50.90	252.20	0.0500	
65.44	249.78	0.0500	
76.35	246.86	0.0500	
77.24	245.90	0.0500	
82.24	245.45	0.0500 / 0.0350	Main Channel
82.25	244.90	0.0350	Main Channel
87.25	244.90	0.0350	Main Channel
87.26	245.45	0.0350 / 0.0500	Main Channel
94.53	246.89	0.0500	
170.87	247.16	0.0500	
218.13	248.69	0.0500	
265.40	249.22	0.0500	
301.75	250.78	0.0500	
338.11	253.47	0.0500	
359.92	264.00	0.0500	

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)

0.55	245.45	.345E+04	2.0	0.71	29.20
1.53	246.43	.174E+05	14.6	1.05	19.92
2.50	247.40	.767E+05	52.8	0.86	24.20
3.48	248.38	.224E+06	199.8	1.11	18.71
4.46	249.36	.429E+06	441.2	1.29	16.19
5.43	250.33	.691E+06	874.3	1.58	13.18
6.41	251.31	.987E+06	1468.3	1.86	11.20
7.38	252.28	1.31E+07	2224.8	2.13	9.79
8.36	253.26	1.65E+07	3132.4	2.37	8.78
9.34	254.24	2.01E+07	4256.4	2.65	7.87
10.31	255.21	2.38E+07	5551.0	2.92	7.15
11.29	256.19	2.76E+07	6996.8	3.17	6.57
12.27	257.17	3.14E+07	8585.9	3.42	6.10
13.24	258.14	3.53E+07	10315.9	3.65	5.71
14.22	259.12	3.93E+07	12180.7	3.87	5.38
15.19	260.09	4.34E+07	14178.2	4.09	5.10
16.17	261.07	4.75E+07	16306.5	4.29	4.85
17.15	262.05	5.17E+07	18558.6	4.49	4.64
18.12	263.02	5.59E+07	20939.7	4.68	4.45

<---- hydrograph ---->						
	AREA	OPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8072)	2835.26	57.92	6.50	30.50	2.54	0.87
OUTFLOW: ID= 1 (6192)	2835.26	49.96	7.00	30.50	2.43	0.87

CALIB STANDHYD (1921)			
ID= 1 DT=15.0 min	Area (ha)= 72.27	Imp(%)= 42.00	Dir. Conn.(%)= 19.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)=	30.35	41.92	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	694.12	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	96.49	49.46	
over (min)	15.00	30.00	
Storage Coeff. (min)=	10.20 (ii)	24.38 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.09	0.04	
PEAK FLOW (cms)=	3.09	2.65	*TOTALS* 4.739 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	71.10	23.44	32.50
TOTAL RAINFALL (mm)=	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.32	0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 57.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5192)			
IN= 2--> OUT= 1			
DT= 15.0 min			
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.1390	1.4346
0.2060	0.7072	3.8730	1.6637
0.9550	0.8170	4.8310	1.9184
1.7940	1.0081	5.2310	1.9284
2.4250	1.1871	0.0000	0.0000

	AREA	OPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (1921)	72.270	4.739	6.00	32.50
OUTFLOW: ID= 1 (5192)	72.270	1.931	6.75	32.49

PEAK FLOW REDUCTION [Qout/Qin] (%) = 40.74
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 1.0570

CALIB STANDHYD (1922)			
ID= 1 DT=15.0 min	Area (ha)= 174.34	Imp(%)= 39.00	Dir. Conn.(%)= 18.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)=	67.99	106.35	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	1078.08	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	96.49	46.21	
over (min)	15.00	30.00	
Storage Coeff. (min)=	13.29 (ii)	27.85 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.08	0.04	

TOTALS			
PEAK FLOW (cms)=	6.44	5.85	10.060 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	71.10	22.78	31.47
TOTAL RAINFALL (mm)=	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.31	0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 57.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8074)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1922):	174.34	10.060	6.00	31.47
+ ID2= 2 (5192):	72.27	1.931	6.75	32.49
=====				
ID = 3 (8074):	246.61	10.226	6.00	31.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8076)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6192):	2835.26	49.959	7.00	30.50
+ ID2= 2 (8074):	246.61	10.226	6.00	31.77
=====				
ID = 3 (8076):	3081.87	55.392	6.75	30.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0196)			
ID= 1 DT=15.0 min	Area (ha)= 41.47	Curve Number (CN)= 70.0	Ia (mm)= 5.00
		# of Linear Res. (N)= 1.50	U.H. Tp(hrs)= 0.24

Unit Hyd Opeak (cms)=	2.950
PEAK FLOW (cms)=	1.446 (i)
TIME TO PEAK (hrs)=	6.000
RUNOFF VOLUME (mm)=	24.056
TOTAL RAINFALL (mm)=	73.100
RUNOFF COEFFICIENT =	0.329

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1941)			
ID= 1 DT=15.0 min	Area (ha)= 0.76	Imp(%)= 45.00	Dir. Conn.(%)= 15.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 0.34 0.42
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 71.18 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 96.49 77.93
 over (min)= 15.00 15.00
 Storage Coeff. (min)= 2.60 (ii) 14.42 (ii)
 Unit Hyd. Tpeak (min)= 15.00 15.00
 Unit Hyd. peak (cms)= 0.11 0.07

TOTALS
 PEAK FLOW (cms)= 0.03 0.06 0.095 (iii)
 TIME TO PEAK (hrs)= 6.00 6.00 6.00
 RUNOFF VOLUME (mm)= 71.10 32.84 38.56
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.45 0.53

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 67.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5194)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0510	0.0197
0.0030	0.0093	0.0640	0.0226
0.0150	0.0107	0.0740	0.0253
0.0270	0.0138	0.0740	0.0353
0.0380	0.0165	0.0000	0.0000

INFLOW : ID= 2 (1941)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5194)	0.760	0.095	6.00	38.56
	0.760	0.028	6.50	37.55

PEAK FLOW REDUCTION [Qout/Qin] (%) = 29.80
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0142

CALIB NASHYD (1942)
 ID= 1 DT=15.0 min

Area (ha)= 255.33 Curve Number (CN)= 69.5
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.68

Unit Hyd Opeak (cms)= 6.448
 PEAK FLOW (cms)= 4.165 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 25.356
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.347

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9194)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1942):	255.33	4.165	6.75	25.36
+ ID2= 2 (5194):	0.76	0.028	6.50	37.55

ID = 3 (9194):	256.09	4.191	6.75	25.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8078)
 1 + 2 = 3

AREA	OPEAK	TPEAK	R. V.
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	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0196):	41.47	1.446	6.00	24.06
+ ID2= 2 (9194):	256.09	4.191	6.75	25.39

ID = 3 (8078):	297.56	5.339	6.50	25.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6198)
 IN= 2---> OUT= 1

Routing time step (min)= 15.00

<----- DATA FOR SECTION (1981.0) ----->

Distance	Elevation	Manning
0.00	267.15	0.0500
22.99	265.94	0.0500
51.73	261.39	0.0500
74.72	258.75	0.0500
94.83	257.79	0.0500
114.95	254.36	0.0500
126.44	254.06	0.0500
132.19	253.68	0.0500
135.06	253.35	0.0500
137.94	252.93	0.0500 / 0.0300
140.81	252.41	0.0300
143.69	251.89	0.0300
146.56	252.51	0.0300
149.43	253.36	0.0300 / 0.0500
158.05	255.89	0.0500
172.42	256.25	0.0500
204.03	259.15	0.0500
229.90	260.36	0.0500
252.89	260.46	0.0500
284.50	260.45	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.35	252.24	.111E+04	0.6	0.97	31.07
0.69	252.58	.442E+04	3.8	1.55	19.42
1.04	252.93	.974E+04	11.1	2.06	14.65
1.51	253.40	.203E+05	31.2	2.77	10.87
1.98	253.87	.357E+05	65.0	3.30	9.14
2.45	254.34	.595E+05	112.9	3.43	8.80
2.92	254.81	.935E+05	186.8	3.62	8.34
3.39	255.28	.131E+06	282.1	3.89	7.75
3.86	255.75	.172E+06	398.2	4.18	7.22
4.33	256.22	.221E+06	527.0	4.31	6.99
4.80	256.69	.282E+06	697.3	4.47	6.75
5.27	257.16	.350E+06	898.5	4.64	6.50
5.74	257.63	.425E+06	1131.4	4.82	6.26
6.21	258.10	.508E+06	1367.4	4.88	6.19
6.68	258.57	.603E+06	1635.6	4.91	6.14
7.15	259.04	.709E+06	1977.2	5.04	5.98
7.62	259.51	.826E+06	2360.5	5.17	5.83
8.09	259.98	.954E+06	2790.7	5.29	5.70
8.56	260.45	.110E+07	3179.6	5.25	5.74

INFLOW : ID= 2 (8078)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW: ID= 1 (6198)	297.56	5.34	6.50	25.21	0.77	1.64
	297.56	5.13	6.75	25.21	0.76	1.63

CALIB STANDBYD (1981)
 ID= 1 DT=15.0 min

Area (ha)= 149.95
 Total Imp (%) = 44.00
 Dir. Conn. (%) = 19.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	65.98	83.97
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	999.83	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 96.49 53.21
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 12.70 (ii) 26.47 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

PEAK FLOW (cms) = 5.95 5.47 *TOTALS* 9.359 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 71.10 24.32 33.21
 TOTAL RAINFALL (mm) = 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.33 0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 57.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5198) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	10.9910	3.4957
	0.4390	1.7800	13.3530	3.9512
	2.6180	2.0449	15.3540	4.3979
	5.1680	2.5932	15.7540	4.4079
	7.9230	3.0040	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (1981)	149.950	9.359	6.00	33.21
OUTFLOW: ID= 1 (5198)	149.950	3.993	6.75	33.20

PEAK FLOW REDUCTION [Qout/Qin] (%) = 42.66
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 2.3488

CALIB STANDHYD (1982) ID= 1 DT=15.0 min	Area (ha)	Imp (%) = 20.00	Di r. Conn. (%) = 9.00
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	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	48.12	192.49
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1266.52	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	96.49	21.05
over (min)	15.00	45.00
Storage Coeff. (min)	14.64 (ii)	34.59 (ii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.07	0.03

PEAK FLOW (cms) = 4.27 6.50 *TOTALS* 7.736 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.50
 RUNOFF VOLUME (mm) = 71.10 20.19 24.78
 TOTAL RAINFALL (mm) = 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.28 0.34

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 57.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8080) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1982):	240.61	7.736	6.50	24.78
+ ID2= 2 (5198):	149.95	3.993	6.75	33.20
ID = 3 (8080):	390.56	11.369	6.50	28.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8082) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6198):	297.56	5.128	6.75	25.21
+ ID2= 2 (8080):	390.56	11.369	6.50	28.01
ID = 3 (8082):	688.12	16.120	6.50	26.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8084) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8076):	3081.87	55.392	6.75	30.60
+ ID2= 2 (8082):	688.12	16.120	6.50	26.80
ID = 3 (8084):	3769.99	70.397	6.75	29.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6200) IN= 2---> OUT= 1	Routing time step (min) = 15.00
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<----- DATA FOR SECTION (2001.0) ----->		
Distance	Elevation	Manning
0.00	252.47	0.0500
10.38	252.29	0.0500
20.76	252.28	0.0500
41.52	247.66	0.0500
62.28	247.47	0.0500
80.45	247.34	0.0500
90.83	246.85	0.0500
103.81	245.51	0.0500
106.00	244.50	0.0500
108.99	244.05	0.0500 / 0.0300
109.00	243.50	0.0300
114.00	243.50	0.0300
114.01	244.05	0.0300 / 0.0500
118.99	244.50	0.0500
132.36	245.09	0.0500
150.52	245.83	0.0500
199.83	249.24	0.0500
207.62	250.16	0.0500
236.16	253.11	0.0500
256.92	253.83	0.0500

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)
0.28	243.77	.365E+04	0.7	0.54	81.42
0.55	244.05	.730E+04	2.2	0.81	54.59
1.05	244.55	.197E+05	7.2	0.97	45.51
1.54	245.04	.463E+05	17.2	0.99	44.76
2.04	245.54	.895E+05	34.8	1.03	42.81
2.53	246.03	.152E+06	62.8	1.09	40.43
3.03	246.53	.232E+06	104.7	1.20	36.91
3.52	247.02	.328E+06	159.7	1.29	34.19
4.02	247.52	.450E+06	223.4	1.32	33.56
4.51	248.01	.627E+06	326.9	1.38	31.99
5.01	248.51	.820E+06	463.0	1.50	29.52
5.50	249.00	.103E+07	626.0	1.62	27.30
6.00	249.50	.124E+07	820.1	1.75	25.24
6.49	249.99	.147E+07	1044.4	1.89	23.42
6.99	250.49	.170E+07	1292.6	2.01	21.95
7.48	250.98	.195E+07	1565.6	2.13	20.71
7.98	251.48	.220E+07	1864.5	2.25	19.65
8.47	251.97	.246E+07	2189.4	2.36	18.73
8.97	252.47	.274E+07	2427.5	2.35	18.80

		AREA	OPEAK	TPEAK	R.V.	<--- pi pe / channel --->	
		(ha)	(cms)	(hrs)	(mm)	MAX DEPTH	MAX VEL
		(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW: ID= 2 (8084)	3769.99	70.40	6.75	29.91	2.62	1.11	
OUTFLOW: ID= 1 (6200)	3769.99	58.48	7.50	29.91	2.46	1.08	

CALIB
STANDHYD (2001)
ID= 1 DT=15.0 min

Area (ha)	= 42.48	Dir. Conn. (%)	= 19.00
Total Imp (%)	= 46.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 19.54	22.94
Dep. Storage (mm)	= 2.00	5.00
Average Slope (%)	= 0.50	0.50
Length (m)	= 532.17	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr)	= 96.49	80.41
over (min)	= 15.00	30.00
Storage Coeff. (min)	= 8.70 (ii)	20.37 (ii)
Unit Hyd. Tpeak (min)	= 15.00	30.00
Unit Hyd. peak (cms)	= 0.09	0.05

PEAK FLOW (cms)	= 1.90	2.58	*TOTALS*
TIME TO PEAK (hrs)	= 6.00	6.25	3.571 (iii)
RUNOFF VOLUME (mm)	= 71.10	34.78	6.00
TOTAL RAINFALL (mm)	= 73.10	73.10	41.68
RUNOFF COEFFICIENT	= 0.97	0.48	73.10
			0.57

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.7 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5200)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.7210	1.0525
0.1450	0.4892	3.4130	1.1947
0.7930	0.5722	4.0030	1.3346
1.4410	0.7540	4.4030	1.3446
2.0370	0.8927	0.0000	0.0000

INFLOW : ID= 2 (2001)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5200)	42.480	3.571	6.00	41.68
	42.480	1.580	6.75	41.66

PEAK FLOW REDUCTION [Qout/Oin] (%) = 44.23
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha.m.) = 0.7981

CALIB
STANDHYD (2002)
ID= 1 DT=15.0 min

Area (ha)	= 72.79	Dir. Conn. (%)	= 13.00
Total Imp (%)	= 31.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 22.56	50.23
Dep. Storage (mm)	= 2.00	5.00
Average Slope (%)	= 0.50	0.50
Length (m)	= 696.61	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr)	= 96.49	60.78
over (min)	= 15.00	30.00
Storage Coeff. (min)	= 10.22 (ii)	23.28 (ii)
Unit Hyd. Tpeak (min)	= 15.00	30.00
Unit Hyd. peak (cms)	= 0.09	0.04

PEAK FLOW (cms)	= 2.13	4.00	*TOTALS*
TIME TO PEAK (hrs)	= 6.00	6.25	4.761 (iii)
RUNOFF VOLUME (mm)	= 71.10	31.32	6.25
TOTAL RAINFALL (mm)	= 73.10	73.10	36.49
RUNOFF COEFFICIENT	= 0.97	0.43	73.10
			0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.7 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8086)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2002):	72.79	4.761	6.25	36.49
+ ID2= 2 (5200):	42.48	1.580	6.75	41.66
ID = 3 (8086):	115.27	5.877	6.25	38.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8088)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6200):	3769.99	58.485	7.50	29.91
+ ID2= 2 (8086):	115.27	5.877	6.25	38.40
ID = 3 (8088):	3885.26	60.424	7.50	30.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8038)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8034):	5220.76	22.423	7.25	18.79
+ ID2= 2 (8088):	3885.26	60.424	7.50	30.16
ID = 3 (8038):	9106.02	82.640	7.50	23.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (1365)
IN= 2--> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1362.0) ----->

Distance	Elevation	Manning
0.00	252.00	0.0400
2.35	244.60	0.0400
39.94	243.14	0.0400
58.74	242.65	0.0400
91.63	242.03	0.0400
101.03	241.69	0.0400
103.38	241.66	0.0400
105.73	240.76	0.0400
105.93	240.59	0.0400
109.18	240.28	0.0400 / 0.0300
110.43	239.81	0.0300
111.68	239.81	0.0300
113.43	240.09	0.0300 / 0.0400
118.43	240.50	0.0400
138.62	241.68	0.0400
152.72	241.60	0.0400
166.82	242.68	0.0400
185.62	249.09	0.0400
197.36	251.00	0.0400
232.61	252.28	0.0400

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.28	240.09	.118E+04	0.1	0.20	142.09
0.91	240.72	.114E+05	2.4	0.35	79.92
1.53	241.34	.352E+05	9.4	0.45	62.17
2.16	241.97	.831E+05	22.8	0.46	60.69
2.79	242.60	.172E+06	54.2	0.53	52.74

3.41	243.22	.297E+06	111.4	0.63	44.49
4.04	243.85	.445E+06	194.1	0.74	38.22
4.67	244.48	.612E+06	301.1	0.83	33.88
5.29	245.10	.793E+06	443.0	0.94	29.82
5.92	245.73	.976E+06	611.3	1.06	26.60
6.55	246.36	.116E+07	802.0	1.17	24.12
7.18	246.99	.135E+07	1014.0	1.27	22.16
7.80	247.61	.154E+07	1246.2	1.37	20.56
8.43	248.24	.173E+07	1498.0	1.46	19.24
9.06	248.87	.192E+07	1768.8	1.55	18.12
9.68	249.49	.212E+07	2049.6	1.63	17.24
10.31	250.12	.232E+07	2344.7	1.71	16.49
10.94	250.75	.252E+07	2658.5	1.78	15.83
11.56	251.37	.274E+07	2931.2	1.81	15.56

		<---- hydrograph ---->		<-pi pe / channel ->			
	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL	
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW :	ID= 2 (8038)	9106.02	82.64	7.50	23.64	3.10	0.58
OUTFLOW:	ID= 1 (1365)	9106.02	69.51	8.25	23.64	2.96	0.56

CALIB STANDHYD (1321)		Area (ha)= 208.90		Dir. Conn. (%)= 46.00	
ID= 1 DT=15.0 min		Total Imp(%)= 67.00			

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	139.96	68.94
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	1180.11	40.00
Mannings n	=	0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	78.44
over (min)	15.00	30.00
Storage Coeff. (min)=	14.03 (ii)	25.82 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.07	0.04

PEAK FLOW (cms)=	19.29	6.75	23.586 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	71.10	31.33	49.62
TOTAL RAINFALL (mm)=	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.43	0.68

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5132)		OUTFLOW		STORAGE	
IN= 2--> OUT= 1		(cms)		(ha.m.)	
DT= 15.0 min		DT= 15.0 min		DT= 15.0 min	
		0.0000	0.0000	7.9970	7.9437
		0.9210	5.2645	12.4650	8.5839
		3.2690	5.3433	16.9060	9.2236
		6.3980	5.6925	17.3060	9.2336
		7.1860	6.8155	0.0000	0.0000

	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
INFLOW :	ID= 2 (1321)	208.900	23.586	6.00	49.62
OUTFLOW:	ID= 1 (5132)	208.900	6.560	6.75	49.62

PEAK FLOW REDUCTION [Qout/Oin] (%) = 27.81
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha.m.) = 5.9275

CALIB STANDHYD (1322)		Area (ha)= 81.94		Dir. Conn. (%)= 25.00	
ID= 1 DT=15.0 min		Total Imp(%)= 37.00			

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	30.32	51.62

Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	739.10	40.00
Mannings n	=	0.013	0.250
Max. Eff. Inten. (mm/hr)=	96.49	45.78	
over (min)	15.00	30.00	
Storage Coeff. (min)=	10.59 (ii)	25.22 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.09	0.04	

PEAK FLOW (cms)=	4.55	2.96	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	6.402 (iii)
RUNOFF VOLUME (mm)=	71.10	25.27	36.73
TOTAL RAINFALL (mm)=	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.35	0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD. (8090)		AREA		OPEAK		TPEAK		R. V.	
1 + 2 = 3		(ha)		(cms)		(hrs)		(mm)	
ID= 1 (1322):	81.94	6.402	6.00	36.73					
+ ID= 2 (5132):	208.90	6.560	6.75	49.62					
ID= 3 (8090):	290.84	9.558	6.50	45.99					

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1342)		Area (ha)= 97.16		Dir. Conn. (%)= 28.00	
ID= 1 DT=15.0 min		Total Imp(%)= 41.00			

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	39.84	57.32
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	804.82	40.00
Mannings n	=	0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	22.02
over (min)	15.00	45.00
Storage Coeff. (min)=	11.15 (ii)	30.75 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03

PEAK FLOW (cms)=	5.95	2.15	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.50	6.835 (iii)
RUNOFF VOLUME (mm)=	71.10	19.73	34.11
TOTAL RAINFALL (mm)=	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.27	0.47

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 54.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1341)		Area (ha)= 58.79		Dir. Conn. (%)= 20.00	
ID= 1 DT=15.0 min		Total Imp(%)= 30.00			

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	17.64	41.15
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	626.05	40.00
Mannings n	=	0.013	0.250

Max. Eff. Inten. (mm/hr)= 96.49 19.45
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 9.59 (ii) 30.18 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. Tpeak (cms)= 0.09 0.03

PEAK FLOW (cms)= 2.69 1.38 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 3.260 (iii)
 RUNOFF VOLUME (mm)= 71.10 18.68
 TOTAL RAINFALL (mm)= 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.26 0.40

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 54.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5134)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha. m.)	(cms)	(ha. m.)
	0.0000	0.0000	1.1250	1.0755
	0.1070	0.6119	1.7570	1.1641
	0.4130	0.6225	2.3860	1.2526
	0.8380	0.7018	2.7860	1.2626
	0.9790	0.8884	0.0000	0.0000

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (1341)	58.790	3.260	6.00	29.17
OUTFLOW: ID= 1 (5134)	58.790	0.938	7.00	29.14

PEAK FLOW REDUCTION [Qout/Qin] (%) = 28.78
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 0.8386

ADD HYD (9134)				
1 + 2 = 3				
	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1342):	97.16	6.835	6.00	34.11
+ ID2= 2 (5134):	58.79	0.938	7.00	29.14
=====				
ID = 3 (9134):	155.95	6.910	6.00	32.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8092)				
1 + 2 = 3				
	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8090):	290.84	9.558	6.50	45.99
+ ID2= 2 (9134):	155.95	6.910	6.00	32.24
=====				
ID = 3 (8092):	446.79	13.908	6.50	41.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (1366)
 IN= 2--> OUT= 1
 Routing time step (min) = 15.00

<----- DATA FOR SECTION (1361.0) ----->		
Distance	Elevation	Manning
0.00	262.81	0.0400
37.23	260.54	0.0400
79.12	256.98	0.0400
111.70	254.74	0.0400
172.20	252.76	0.0400
251.32	248.82	0.0400
255.97	248.75	0.0400
260.43	248.76	0.0400

265.43	248.49	0.0400		
268.68	248.18	0.0400 / 0.0300	Main	Channel
269.93	247.71	0.0300	Main	Channel
271.18	247.71	0.0300	Main	Channel
272.93	247.99	0.0300	Main	Channel
277.93	248.40	0.0300 / 0.0400	Main	Channel
283.90	248.88	0.0400		
307.17	249.60	0.0400		
321.13	251.59	0.0400		
363.01	257.08	0.0400		
400.25	257.58	0.0400		
460.75	261.91	0.0400		

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)
0.47	248.18	.881E+04	1.3	0.76	114.66
1.19	248.90	.742E+05	17.3	1.22	71.44
1.92	249.63	.278E+06	78.3	1.48	59.26
2.64	250.35	.589E+06	210.3	1.87	46.69
3.36	251.07	.974E+06	414.3	2.23	39.19
4.08	251.79	.143E+07	696.2	2.55	34.32
4.81	252.52	.197E+07	1063.0	2.83	30.86
5.53	253.24	.259E+07	1505.7	3.06	28.62
6.25	253.96	.331E+07	2051.3	3.26	26.85
6.97	254.68	.413E+07	2720.9	3.46	25.30
7.70	255.41	.504E+07	3584.5	3.73	23.43
8.42	256.13	.601E+07	4580.9	4.00	21.87
9.14	256.85	.704E+07	5707.9	4.25	20.57
9.86	257.57	.818E+07	6636.1	4.26	20.53
10.59	258.30	.945E+07	8115.0	4.50	19.42
11.31	259.02	.108E+08	9759.2	4.74	18.45
12.03	259.74	.122E+08	11569.5	4.96	17.61
12.75	260.46	.137E+08	13550.6	5.18	16.87
13.48	261.19	.153E+08	15636.4	5.37	16.29

	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW: ID= 2 (8092)	446.79	13.91	6.50	41.19	1.04	1.08
OUTFLOW: ID= 1 (1366)	446.79	7.37	7.25	41.19	0.74	0.89

CALIB
 STANDHYD (1362)
 ID= 1 DT=15.0 min
 Area (ha) = 371.79
 Total Imp (%) = 26.00
 Dir. Conn. (%) = 14.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	96.67	275.12
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1574.36	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 96.49 44.93
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 16.68 (ii) 31.41 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. Tpeak (cms)= 0.07 0.03

PEAK FLOW (cms)= 9.70 12.70 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.50 15.892 (iii)
 RUNOFF VOLUME (mm)= 71.10 25.38 6.50
 TOTAL RAINFALL (mm)= 73.10 73.10 31.78
 RUNOFF COEFFICIENT = 0.97 0.35 0.43

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (1361)
 ID= 1 DT=15.0 min
 Area (ha) = 140.62
 Total Imp (%) = 55.00
 Dir. Conn. (%) = 29.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 77.34 63.28
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 968.23 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 96.49 75.42
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 12.46 (ii) 24.43 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

TOTALS
 PEAK FLOW (cms)= 8.57 6.13 12.474 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 71.10 31.23 42.79
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.43 0.59

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5136)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.7060	4.3486
0.3100	2.4365	5.9200	5.0324
1.7670	2.5993	7.0820	5.7151
3.4020	2.8510	7.4820	5.7251
4.0310	3.5962	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1361)	140.620	12.474	6.00	42.79
OUTFLOW: ID= 1 (5136)	140.620	3.711	6.75	42.77

PEAK FLOW REDUCTION [Qout/Qin] (%) = 29.75
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 3.2408

ADD HYD (8096)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1362):	371.79	15.892	6.50	31.78
+ ID2= 2 (5136):	140.62	3.711	6.75	42.77
ID = 3 (8096):	512.41	19.473	6.50	34.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8094)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1366):	446.79	7.375	7.25	41.19
+ ID2= 2 (8096):	512.41	19.473	6.50	34.80
ID = 3 (8094):	959.20	24.505	6.50	37.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8098)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1365):	9106.02	69.511	8.25	23.64
+ ID2= 2 (8094):	959.20	24.505	6.50	37.77
ID = 3 (8098):	10065.22	82.569	7.75	24.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1381)
 ID= 1 DT=15.0 min

Area (ha)= 96.24
 Total Imp(%)= 59.00
 Dir. Conn.(%)= 34.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	56.78	39.46
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	801.00	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 96.49 14.69
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 11.12 (ii) 34.16 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03

TOTALS
 PEAK FLOW (cms)= 7.16 0.94 7.543 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 71.10 10.34 31.00
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.14 0.42

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 28.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5138)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	6.4380	2.7648
0.3260	1.4281	8.0830	3.0750
1.8650	1.6876	9.4100	3.3790
3.4730	2.0447	9.8100	3.3890
4.8170	2.3809	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1381)	96.240	7.543	6.00	31.00
OUTFLOW: ID= 1 (5138)	96.240	1.802	6.75	30.99

PEAK FLOW REDUCTION [Qout/Qin] (%) = 23.89
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 1.6876

CALIB
 NASHYD (1382)
 ID= 1 DT=15.0 min

Area (ha)= 318.55
 Ia (mm)= 5.00
 U. H. Tp(hrs)= 1.09
 Curve Number (CN)= 33.2
 # of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 4.976

PEAK FLOW (cms)= 1.068 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 7.936
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.109

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9138)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1382):	318.55	1.068	7.75	7.94
+ ID2= 2 (5138):	96.24	1.802	6.75	30.99
ID = 3 (9138):	414.79	2.780	6.75	13.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (1402)
ID= 1 DT=15.0 min

Area Total	(ha)= 131.22	Dir. Conn. (%)= 21.00
Imp (%)	= 29.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 38.05	93.17
Dep. Storage (mm)	= 2.00	5.00
Average Slope (%)	= 0.50	0.50
Length (m)	= 935.31	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr) over (min)	= 96.49 / 15.00	10.40 / 45.00
Storage Coeff. (min)	= 12.20 (ii)	38.66 (ii)
Unit Hyd. Tpeak (min)	= 15.00	45.00
Unit Hyd. peak (cms)	= 0.08	0.03

TOTALS

PEAK FLOW (cms)	= 5.84	1.48	6.426 (iii)
TIME TO PEAK (hrs)	= 6.00	6.50	6.00
RUNOFF VOLUME (mm)	= 71.10	10.60	23.30
TOTAL RAINFALL (mm)	= 73.10	73.10	73.10
RUNOFF COEFFICIENT	= 0.97	0.14	0.32

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 37.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (1401)
ID= 1 DT=15.0 min

Area Total	(ha)= 27.37	Dir. Conn. (%)= 33.00
Imp (%)	= 45.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 12.32	15.05
Dep. Storage (mm)	= 2.00	5.00
Average Slope (%)	= 0.50	0.50
Length (m)	= 427.16	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr) over (min)	= 96.49 / 15.00	12.46 / 45.00
Storage Coeff. (min)	= 7.62 (ii)	32.23 (ii)
Unit Hyd. Tpeak (min)	= 15.00	45.00
Unit Hyd. peak (cms)	= 0.10	0.03

TOTALS

PEAK FLOW (cms)	= 2.19	0.31	2.319 (iii)
TIME TO PEAK (hrs)	= 6.00	6.50	6.00
RUNOFF VOLUME (mm)	= 71.10	11.55	31.20
TOTAL RAINFALL (mm)	= 73.10	73.10	73.10
RUNOFF COEFFICIENT	= 0.97	0.16	0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 37.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5140)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.8310	0.7127
0.0930	0.3344	2.2990	0.8148
0.5300	0.3869	2.6760	0.9148
0.9880	0.4994	3.0760	0.9248
1.3700	0.5962	0.0000	0.0000

INFLOW : ID= 2 (1401)	27.370	2.319	6.00	31.20
OUTFLOW: ID= 1 (5140)	27.370	0.660	6.50	31.17

PEAK FLOW REDUCTION [Qout/Qin] (%) = 28.45
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha.m.) = 0.4197

ADD HYD (9140)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1402):	131.22	6.426	6.00	23.30
+ ID2= 2 (5140):	27.37	0.660	6.50	31.17
ID = 3 (9140):	158.59	6.510	6.00	24.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8102)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (9138):	414.79	2.780	6.75	13.28
+ ID2= 2 (9140):	158.59	6.510	6.00	24.66
ID = 3 (8102):	573.38	7.308	6.00	16.43

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6142)
IN= 2--> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1421.0) ----->

Distance	Elevation	Manning	
0.00	295.50	0.0400	
7.25	295.36	0.0400	
14.50	295.22	0.0400	
21.75	295.22	0.0400	
29.00	294.23	0.0400	
52.56	286.45	0.0400	
67.06	281.44	0.0400	
77.94	278.28	0.0400	
82.81	277.20	0.0400 / 0.0300	Main Channel
83.21	276.80	0.0300	Main Channel
88.81	276.80	0.0300	Main Channel
96.41	276.80	0.0300	Main Channel
96.81	277.20	0.0300 / 0.0400	Main Channel
103.31	278.96	0.0400	
108.75	280.16	0.0400	
117.81	282.73	0.0400	
135.94	290.30	0.0400	
159.50	292.14	0.0400	
174.00	293.07	0.0400	
179.44	293.37	0.0400	

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.40	277.20	238E+05	6.9	1.27	57.28
1.25	278.05	889E+05	50.9	2.50	29.13
2.10	278.90	179E+06	133.4	3.26	22.36
2.95	279.75	293E+06	256.7	3.83	19.03
3.80	280.60	432E+06	426.0	4.31	16.90
4.66	281.46	593E+06	644.3	4.75	15.35
5.51	282.31	776E+06	915.3	5.16	14.12
6.36	283.16	978E+06	1242.2	5.55	13.12
7.21	284.01	120E+07	1626.2	5.94	12.27
8.06	284.86	143E+07	2065.6	6.30	11.57
8.91	285.71	169E+07	2562.4	6.64	10.97
9.76	286.56	196E+07	3118.0	6.97	10.46
10.61	287.41	224E+07	3733.3	7.28	10.02
11.46	288.26	255E+07	4411.9	7.57	9.62
12.31	289.11	287E+07	5155.6	7.86	9.27
13.17	289.97	321E+07	5966.6	8.14	8.96
14.02	290.82	357E+07	6705.0	8.22	8.87
14.87	291.67	398E+07	7494.3	8.24	8.85
15.72	292.52	444E+07	8405.3	8.28	8.80

----- hydrograph -----> <- pipe / channel ->

AREA	OPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
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INFLOW : ID= 2 (8102) (ha) (cms) (hrs) (mm) (m) (m/s)
 573.38 7.31 6.00 16.43 0.41 1.28
 OUTFLOW: ID= 1 (6142) 573.38 4.31 7.25 16.43 0.25 1.27

CALIB
 STANDHYD (1421)
 ID= 1 DT=15.0 mi n Area (ha)= 69.77
 Total Imp(%)= 45.00 Di r. Conn.(%)= 15.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 31.40 38.37
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 682.01 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 96.49 62.21
 over (mi n) 15.00 30.00
 Storage Coeff. (mi n)= 10.10 (ii) 23.03 (ii)
 Uni t Hyd. Tpeak (mi n)= 15.00 30.00
 Uni t Hyd. peak (cms)= 0.09 0.04

TOTALS
 PEAK FLOW (cms)= 2.36 3.14 4.343 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 71.10 26.52 33.20
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.36 0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5142)
 IN= 2--> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.6670	1.8169
0.2360	0.8525	5.8600	2.0773
1.3520	0.9863	6.8220	2.3321
2.5180	1.2732	7.2220	2.3421
3.4920	1.5199	0.0000	0.0000

INFLOW : ID= 2 (1421) AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 69.770 4.343 6.00 33.20
 OUTFLOW: ID= 1 (5142) 69.770 1.860 6.75 33.19

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 42.82
 TIME SHIF T OF PEAK FLOW (mi n) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 1.1135

CALIB
 NASHYD (1422)
 ID= 1 DT=15.0 mi n Area (ha)= 326.77 Curve Number (CN)= 61.2
 Ia (mm)= 5.00 # of Li near Res. (N)= 1.50
 U. H. Tp(hrs)= 1.21

Uni t Hyd Qpeak (cms)= 4.626
 PEAK FLOW (cms)= 2.636 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 20.083
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.275

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9142)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
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ID1= 1 (1422): 326.77 2.636 7.75 20.08
 + ID2= 2 (5142): 69.77 1.860 6.75 33.19

 ID = 3 (9142): 396.54 4.268 7.00 22.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8104)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
573.38	4.307	7.25	16.43
396.54	4.268	7.00	22.39
969.92	8.541	7.00	18.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1441)
 ID= 1 DT=15.0 mi n Area (ha)= 6.29
 Total Imp(%)= 45.00 Di r. Conn.(%)= 15.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 2.83 3.46
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 204.78 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 96.49 81.98
 over (mi n) 15.00 30.00
 Storage Coeff. (mi n)= 4.90 (ii) 16.49 (ii)
 Uni t Hyd. Tpeak (mi n)= 15.00 30.00
 Uni t Hyd. peak (cms)= 0.11 0.05

TOTALS
 PEAK FLOW (cms)= 0.25 0.43 0.528 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 71.10 34.46 39.95
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.47 0.55

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5144)
 IN= 2--> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.4210	0.1639
0.0210	0.0769	0.5290	0.1874
0.1220	0.0890	0.6150	0.2104
0.2270	0.1148	1.0150	0.2204
0.3150	0.1371	0.0000	0.0000

INFLOW : ID= 2 (1441) AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 6.290 0.528 6.00 39.95
 OUTFLOW: ID= 1 (5144) 6.290 0.227 6.75 39.81

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 42.94
 TIME SHIF T OF PEAK FLOW (mi n) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 0.1169

CALIB
 NASHYD (1442)
 ID= 1 DT=15.0 mi n Area (ha)= 129.40 Curve Number (CN)= 74.9
 Ia (mm)= 5.00 # of Li near Res. (N)= 1.50
 U. H. Tp(hrs)= 0.70

Uni t Hyd Qpeak (cms)= 3.147

PEAK FLOW (cms) = 2.431 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 29.746
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.407

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9144) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1442):	129.40	2.431	6.75	29.75
+ ID2= 2 (5144):	6.29	0.227	6.75	39.81

ID = 3 (9144):	135.69	2.658	6.75	30.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8106) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8104):	969.92	8.541	7.00	18.87
+ ID2= 2 (9144):	135.69	2.658	6.75	30.21

ID = 3 (8106):	1105.61	11.152	7.00	20.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6146)
 IN= 2----> OUT= 1
 Routing time step (mi n)' = 15.00

<----- DATA FOR SECTION (1461.0) ----->

Distance	Elevation	Manning	
0.00	262.00	0.0350	
39.58	257.13	0.0350	
123.69	257.51	0.0350	
183.07	256.83	0.0350	
202.86	255.73	0.0350	
212.75	254.73	0.0350	
222.65	252.86	0.0350	
232.44	250.70	0.0350 / 0.0300	Main Channel
236.44	250.20	0.0300	Main Channel
236.84	249.80	0.0300	Main Channel
242.44	249.80	0.0300	Main Channel
250.04	249.80	0.0300	Main Channel
250.44	250.20	0.0300	Main Channel
252.33	250.57	0.0300 / 0.0350	Main Channel
286.97	253.52	0.0350	
331.50	255.80	0.0350	
380.98	255.55	0.0350	
445.30	258.40	0.0350	
455.19	259.82	0.0350	
489.83	262.22	0.0350	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.39	250.19	.169E+05	5.4	1.04	52.00
0.77	250.57	.373E+05	16.6	1.44	37.46
1.44	251.24	.911E+05	60.3	2.14	25.19
2.11	251.91	.169E+06	136.3	2.61	20.64
2.79	252.59	.270E+06	249.7	2.99	18.05
3.46	253.26	.396E+06	405.4	3.31	16.28
4.13	253.93	.549E+06	603.3	3.56	15.15
4.80	254.60	.737E+06	857.8	3.77	14.31
5.48	255.28	.963E+06	1174.6	3.95	13.67
6.15	255.95	.128E+07	1449.5	3.66	14.74
6.82	256.62	.173E+07	2046.4	3.83	14.07
7.49	257.29	.227E+07	2665.2	3.81	14.17
8.17	257.97	.310E+07	3705.2	3.87	13.93
8.84	258.64	.400E+07	5127.4	4.15	12.99
9.51	259.31	.492E+07	6858.6	4.51	11.95
10.18	259.98	.587E+07	8815.5	4.86	11.09
10.86	260.66	.684E+07	10954.6	5.18	10.41
11.53	261.33	.785E+07	13324.3	5.49	9.82

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8106)	1105.61	11.15	7.00	20.26	0.58	1.21
OUTFLOW: ID= 1 (6146)	1105.61	9.48	7.75	20.26	0.52	1.15

CALIB STANDHYD (1462)
 ID= 1 DT=15.0 mi n
 Area (ha)= 112.22
 Total Imp(%)= 32.00
 Dir. Conn.(%)= 13.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	35.91	76.31
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	864.95	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	66.16
over (mi n)	15.00	30.00
Storage Coeff. (mi n)=	11.64 (ii)	24.26 (ii)
Unit Hyd. Tpeak (mi n)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04

TOTALS
 PEAK FLOW (cms) = 3.14
 TIME TO PEAK (hrs) = 6.00
 RUNOFF VOLUME (mm) = 71.10
 TOTAL RAINFALL (mm) = 73.10
 RUNOFF COEFFICIENT = 0.97

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1461)
 ID= 1 DT=15.0 mi n
 Area (ha)= 83.29
 Total Imp(%)= 38.00
 Dir. Conn.(%)= 16.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	31.65	51.64
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	745.16	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	72.49
over (mi n)	15.00	30.00
Storage Coeff. (mi n)=	10.65 (ii)	22.81 (ii)
Unit Hyd. Tpeak (mi n)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.04

TOTALS
 PEAK FLOW (cms) = 2.96
 TIME TO PEAK (hrs) = 6.00
 RUNOFF VOLUME (mm) = 71.10
 TOTAL RAINFALL (mm) = 73.10
 RUNOFF COEFFICIENT = 0.97

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5146)
 IN= 2----> OUT= 1
 DT= 15.0 mi n
 OUTFLOW STORAGE | OUTFLOW STORAGE

	(cms)	(ha. m.)	(cms)	(ha. m.)
0.0000	0.0000		8.9200	2.5813
0.2820	0.8782		11.0570	3.0328
2.5360	1.0372		12.9860	3.4993
4.7880	1.7148		13.3860	3.5093
6.7570	2.1134		0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1461)	83.290	6.156	6.00	40.47
OUTFLOW: ID= 1 (5146)	83.290	3.396	6.50	40.46

PEAK TIME	FLOW SHIFT	REDUCTION	[Out/Oi n] (%)
MAXIMUM STORAGE USED			55.17
			(min) = 30.00
			(ha. m.) = 1.3245

ADD HYD	(8099)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3					
ID1= 1 (1462)		112.22	7.757	6.25	38.37
+ ID2= 2 (5146)		83.29	3.396	6.50	40.46
ID = 3 (8099)		195.51	10.572	6.25	39.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8108)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3					
ID1= 1 (6146)		1105.61	9.479	7.75	20.26
+ ID2= 2 (8099)		195.51	10.572	6.25	39.26
ID = 3 (8108)		1301.12	13.236	6.25	23.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8100)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3					
ID1= 1 (8098)		10065.22	82.569	7.75	24.99
+ ID2= 2 (8108)		1301.12	13.236	6.25	23.11
ID = 3 (8100)		11366.34	94.493	7.75	24.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6202)
IN= 2--> OUT= 1
Routing time step (min)' = 15.00

Distance	Elevation	Manning	
0.00	245.48	0.0400	
29.68	242.80	0.0400	
74.20	240.39	0.0400	
155.81	237.12	0.0400	
200.33	234.48	0.0400	
244.85	234.67	0.0400	
259.69	233.96	0.0400	
274.53	233.84	0.0400	
281.68	233.40	0.0400	
285.78	232.40	0.0400 / 0.0300	Main Channel
286.78	231.40	0.0300	Main Channel
296.78	231.40	0.0300	Main Channel
299.78	232.40	0.0300 / 0.0400	Main Channel
311.62	233.74	0.0400	
333.88	234.34	0.0400	
497.11	235.32	0.0400	
571.31	237.32	0.0400	
675.18	239.10	0.0400	
704.86	242.71	0.0400	
734.54	242.62	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.50	231.90	221E+05	4.7	0.85	78.67
1.00	232.40	482E+05	15.3	1.28	52.40
1.60	233.00	913E+05	38.3	1.68	39.77
2.20	233.60	154E+06	72.9	1.90	35.24
2.80	234.20	278E+06	124.2	1.79	37.30
3.40	234.80	599E+06	214.3	1.44	46.59
4.01	235.41	125E+07	445.0	1.43	46.90
4.61	236.01	205E+07	838.4	1.64	40.81
5.21	236.61	293E+07	1361.6	1.86	35.89
5.81	237.21	389E+07	2011.2	2.08	32.23
6.41	237.81	494E+07	2753.5	2.24	29.91
7.01	238.41	612E+07	3635.0	2.39	28.04
7.61	239.01	741E+07	4672.5	2.53	26.43
8.21	239.61	880E+07	5999.3	2.74	24.44
8.82	240.22	102E+08	7510.0	2.94	22.72
9.42	240.82	117E+08	9201.9	3.15	21.23
10.02	241.42	132E+08	11059.7	3.35	19.95
10.62	242.02	148E+08	13073.4	3.54	18.87
11.22	242.62	164E+08	15242.1	3.73	17.93

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8100)	*****	94.49	7.75	24.77	2.45	1.85
OUTFLOW: ID= 1 (6202)	*****	89.19	8.50	24.77	2.39	1.86

ADD HYD	(8112)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3					
ID1= 1 (6202)		11366.34	89.191	8.50	24.77
+ ID2= 2 (8110)		468.53	35.549	6.00	48.99
ID = 3 (8112)		11834.87	94.268	8.25	25.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5555)	IN= 2--> OUT= 1	DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
			0.0000	0.0000	*****	19.2100
			2.8500	0.5200	*****	24.7300
			8.1800	1.2600	*****	30.8600
			15.0900	2.2100	*****	37.6100
			23.2700	3.3900	*****	41.2200
			32.5300	4.7800	*****	48.9000
			36.5100	5.3900	*****	66.4700
			42.7600	6.3700	*****	100.1200
			53.8500	8.1300	*****	140.4100
			91.7300	14.3200	*****	140.4200

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (8112)	*****	94.268	8.25	25.73
OUTFLOW: ID= 1 (5555)	*****	92.215	8.75	25.73

PEAK TIME	FLOW SHIFT	REDUCTION	[Out/Oi n] (%)
MAXIMUM STORAGE USED			97.82
			(min) = 30.00
			(ha. m.) = 14.4154

ADD HYD	(8114)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3					
ID1= 1 (5555)		11834.87	92.215	8.75	25.73
+ ID2= 2 (8136)		2288.44	28.554	6.50	24.47
ID = 3 (8114)		14123.31	109.643	8.50	25.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| CALIB |

STANDHYD (2061)
 ID= 1 DT=15.0 min

Area (ha)= 49.84
 Total Imp(%)= 76.00 Dir. Conn.(%)= 50.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	37.88	11.96
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	576.43	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	144.99
over (min)	15.00	30.00
Storage Coeff. (min)=	9.13 (ii)	18.35 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.05

TOTALS

PEAK FLOW (cms)=	5.79	2.56	7.517 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	71.10	45.16	58.13
TOTAL RAINFALL (mm)=	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.62	0.80

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5206)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.5640	2.4459
0.1690	0.8986	4.2460	2.7624
1.3810	1.2959	4.8070	3.0770
2.2440	1.6904	5.2070	3.0870
2.8510	2.0623	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2061)	49.840	7.517	6.00	58.13
OUTFLOW: ID= 1 (5206)	49.840	2.005	6.75	58.10

PEAK FLOW REDUCTION [Qout/Qin](%)= 26.66
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 1.6016

CALIB
 STANDHYD (2062)
 ID= 1 DT=15.0 min

Area (ha)= 225.70
 Total Imp(%)= 64.00 Dir. Conn.(%)= 42.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	144.45	81.25
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1226.65	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	100.39
over (min)	15.00	30.00
Storage Coeff. (min)=	14.36 (ii)	25.04 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.07	0.04

TOTALS

PEAK FLOW (cms)=	18.85	10.44	25.665 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	71.10	40.26	53.21
TOTAL RAINFALL (mm)=	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.55	0.73

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8144)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2062):	225.70	25.665	6.00	53.21
+ ID2= 2 (5206):	49.84	2.005	6.75	58.10
=====				
ID = 3 (8144):	275.54	26.063	6.00	54.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (2041)
 ID= 1 DT=15.0 min

Area (ha)= 209.90
 Total Imp(%)= 37.00 Dir. Conn.(%)= 21.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	77.66	132.24
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1182.93	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	45.72
over (min)	15.00	30.00
Storage Coeff. (min)=	14.05 (ii)	28.68 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.07	0.04

TOTALS

PEAK FLOW (cms)=	8.84	7.08	13.233 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	71.10	24.05	33.93
TOTAL RAINFALL (mm)=	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.33	0.46

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5204)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	13.0280	5.1901
0.5340	2.0405	16.1840	5.6427
3.0020	2.3552	19.0840	6.7700
5.5440	3.8411	19.4840	6.7800
8.4130	4.5335	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2041)	209.900	13.233	6.00	33.93
OUTFLOW: ID= 1 (5204)	209.900	4.614	7.00	33.93

PEAK FLOW REDUCTION [Qout/Qin](%)= 34.87
 TIME SHIFT OF PEAK FLOW (min)= 60.00
 MAXIMUM STORAGE USED (ha.m.)= 3.3284

CALIB
 STANDHYD (2042)
 ID= 1 DT=15.0 min

Area (ha)= 114.52
 Total Imp(%)= 61.00 Dir. Conn.(%)= 34.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	69.86	44.66
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	873.77	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	76.42
over (min)	15.00	30.00
Storage Coeff. (min)=	11.71 (ii)	23.63 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00

Unit Hyd. peak (cms)= 0.08 0.04
 PEAK FLOW (cms)= 8.37 4.45
 TIME TO PEAK (hrs)= 6.00 6.25
 RUNOFF VOLUME (mm)= 71.10 29.62
 TOTAL RAINFALL (mm)= 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.41

TOTALS
 11.198 (iii)
 6.00
 43.72
 73.10
 0.60

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2042):	114.52	11.198	6.00	43.72
+ ID2= 2 (5204):	209.90	4.614	7.00	33.93
ID = 3 (8140):	324.42	11.630	6.00	37.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6206)
 IN= 2----> OUT= 1 Routing time step (min)' = 15.00

Distance	Elevation	Manning	
0.00	253.07	0.0350	
13.98	252.89	0.0350	
27.95	251.99	0.0350	
34.94	252.18	0.0350	
41.93	252.02	0.0350	
73.38	250.35	0.0350	
94.34	247.09	0.0350	
111.81	244.79	0.0350	
115.31	244.24	0.0350	
117.55	244.20	0.0350 / 0.0300	Main Channel
118.80	243.60	0.0300	Main Channel
122.30	243.60	0.0300	Main Channel
122.55	244.60	0.0300 / 0.0350	Main Channel
146.76	244.74	0.0350	
164.23	247.26	0.0350	
213.15	248.92	0.0350	
241.10	249.51	0.0350	
272.55	250.87	0.0350	
300.50	252.26	0.0350	
345.93	253.67	0.0350	

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(mi n)
0.30	243.90	.310E+04	1.4	1.23	36.25
0.60	244.20	.676E+04	4.6	1.81	24.64
1.12	244.72	.221E+05	17.1	2.07	21.60
1.64	245.24	.754E+05	64.1	2.28	19.60
2.17	245.77	.139E+06	149.8	2.88	15.50
2.69	246.29	.214E+06	273.3	3.43	13.04
3.21	246.81	.299E+06	436.1	3.91	11.43
3.73	247.33	.395E+06	632.6	4.30	10.40
4.25	247.85	.511E+06	836.4	4.39	10.18
4.77	248.37	.653E+06	1114.9	4.58	9.76
5.30	248.90	.822E+06	1472.1	4.81	9.30
5.82	249.42	.102E+07	1872.2	4.91	9.10
6.34	249.94	.126E+07	2444.6	5.22	8.56
6.86	250.46	.151E+07	3113.1	5.52	8.09
7.38	250.98	.180E+07	3843.5	5.74	7.79
7.90	251.50	.211E+07	4717.8	6.00	7.45
8.43	252.03	.245E+07	5701.4	6.24	7.16
8.95	252.55	.284E+07	6727.5	6.36	7.03
9.47	253.07	.326E+07	7885.3	6.48	6.90

INFLOW : ID= 2 (8140) AREA 324.42 OPEAK 11.63 TPEAK 6.00 R. V. 37.38
 MAX DEPTH 0.89 MAX VEL 1.95

OUTFLOW: ID= 1 (6206) 324.42 9.36 6.50 37.38 0.80 1.90

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6206):	324.42	9.357	6.50	37.38
+ ID2= 2 (8144):	275.54	26.063	6.00	54.09
ID = 3 (8142):	599.96	30.664	6.00	45.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8114):	14123.31	109.643	8.50	25.53
+ ID2= 2 (8142):	599.96	30.664	6.00	45.06
ID = 3 (8138):	14723.27	117.483	8.25	26.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6208)
 IN= 2----> OUT= 1 Routing time step (min)' = 15.00

Distance	Elevation	Manning	
0.00	249.19	0.1000	
73.11	244.13	0.1000	
121.86	243.39	0.1000	
243.71	240.01	0.1000	
353.38	238.75	0.1000	
450.87	235.40	0.1000	
499.61	231.40	0.1000	
511.80	230.96	0.1000	
523.98	230.12	0.1000	
534.50	227.80	0.1000 / 0.0500	Main Channel
536.50	226.70	0.0500	Main Channel
546.50	226.70	0.0500	Main Channel
548.50	227.80	0.0500 / 0.1000	Main Channel
572.72	230.25	0.1000	
682.40	230.50	0.1000	
804.25	233.95	0.1000	
926.11	235.83	0.1000	
1047.96	240.70	0.1000	
1145.45	243.26	0.1000	
1206.38	247.03	0.1000	

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(mi n)
0.55	227.25	.111E+05	2.0	0.33	92.44
1.10	227.80	.242E+05	6.5	0.50	61.56
2.23	228.93	.700E+05	26.0	0.68	44.91
3.36	230.06	.150E+06	60.8	0.74	41.04
4.49	231.19	.446E+06	130.8	0.54	56.83
5.62	232.32	.925E+06	287.4	0.57	53.66
6.76	233.46	.152E+07	522.9	0.63	48.32
7.89	234.59	.223E+07	827.2	0.68	44.91
9.02	235.72	.312E+07	1219.5	0.72	42.61
10.15	236.85	.418E+07	1780.6	0.78	39.11
11.28	237.98	.537E+07	2479.2	0.85	36.07
12.41	239.11	.669E+07	3282.9	0.90	33.95
13.54	240.24	.824E+07	4228.6	0.94	32.47
14.67	241.37	.997E+07	5384.6	0.99	30.87
15.81	242.51	.119E+08	6698.4	1.03	29.56
16.94	243.64	.140E+08	8235.2	1.08	28.25
18.07	244.77	.162E+08	10104.2	1.14	26.73
19.20	245.90	.185E+08	12251.5	1.21	25.21
20.33	247.03	.209E+08	14597.0	1.28	23.89

INFLOW : ID= 2 (8138) AREA 117.48 OPEAK 8.25 TPEAK 26.32 R. V. 4.28
 MAX DEPTH 0.57 MAX VEL 0.57

OUTFLOW: ID= 1 (6208) ***** 106.44 9.25 26.32 4.09 0.60

CALIB
STANDHYD (2082)
ID= 1 DT=15.0 min

Area (ha)	= 426.60	Dir. Conn. (%)	= 37.00
Total Imp (%)	= 64.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 273.02	153.58
Dep. Storage (mm)	= 2.00	5.00
Average Slope (%)	= 0.50	0.50
Length (m)	= 1686.42	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr)	= 96.49	100.43
over (min)	= 15.00	30.00
Storage Coeff. (min)	= 17.38 (ii)	28.06 (ii)
Unit Hyd. Tpeak (min)	= 15.00	30.00
Unit Hyd. peak (cms)	= 0.07	0.04

TOTALS
40.830 (iii)

PEAK FLOW (cms)	= 28.85	18.58	40.830 (iii)
TIME TO PEAK (hrs)	= 6.00	6.25	6.00
RUNOFF VOLUME (mm)	= 71.10	37.25	49.78
TOTAL RAINFALL (mm)	= 73.10	73.10	73.10
RUNOFF COEFFICIENT	= 0.97	0.51	0.68

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (2081)
ID= 1 DT=15.0 min

Area (ha)	= 209.70	Dir. Conn. (%)	= 20.00
Total Imp (%)	= 35.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 73.39	136.31
Dep. Storage (mm)	= 2.00	5.00
Average Slope (%)	= 0.50	0.50
Length (m)	= 1182.37	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr)	= 96.49	57.25
over (min)	= 15.00	30.00
Storage Coeff. (min)	= 14.04 (ii)	27.42 (ii)
Unit Hyd. Tpeak (min)	= 15.00	30.00
Unit Hyd. peak (cms)	= 0.07	0.04

TOTALS
14.345 (iii)

PEAK FLOW (cms)	= 8.42	9.42	14.345 (iii)
TIME TO PEAK (hrs)	= 6.00	6.25	6.00
RUNOFF VOLUME (mm)	= 71.10	30.26	38.43
TOTAL RAINFALL (mm)	= 73.10	73.10	73.10
RUNOFF COEFFICIENT	= 0.97	0.41	0.53

- ***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
- ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (2084)
ID= 1 DT=15.0 min

Area (ha)	= 56.00	Dir. Conn. (%)	= 70.00
Total Imp (%)	= 75.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 42.00	14.00
Dep. Storage (mm)	= 2.00	5.00
Average Slope (%)	= 0.50	0.50
Length (m)	= 611.01	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr)	= 96.49	77.22
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over (min)	= 15.00	30.00
Storage Coeff. (min)	= 9.45 (ii)	21.31 (ii)
Unit Hyd. Tpeak (min)	= 15.00	30.00
Unit Hyd. peak (cms)	= 0.09	0.05

TOTALS

PEAK FLOW (cms)	= 9.02	1.49	9.998 (iii)
TIME TO PEAK (hrs)	= 6.00	6.25	6.00
RUNOFF VOLUME (mm)	= 71.10	41.18	62.12
TOTAL RAINFALL (mm)	= 73.10	73.10	73.10
RUNOFF COEFFICIENT	= 0.97	0.56	0.85

- ***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 82.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9289)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.8000	1.5745
0.1900	0.9984	3.7500	1.7809
1.0900	1.1991	4.7100	1.9291
2.0200	1.3813	5.4800	2.0745

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2084)	56.000	9.998	6.00	62.12
OUTFLOW: ID= 1 (9289)	56.000	3.477	6.50	62.10

PEAK FLOW REDUCTION [Qout/Qin] (%) = 34.77
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 1.7553

ADD HYD (9262)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2081):	209.70	14.345	6.00	38.43
+ ID2= 2 (9289):	56.00	3.477	6.50	62.10
=====				
ID = 3 (9262):	265.70	16.727	6.25	43.42

- NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5208)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	16.8530	8.3371
0.8360	4.7353	21.1630	9.3975
4.7810	5.1070	25.1140	10.4406
8.9050	6.2484	25.5140	10.4506
12.6070	7.1796	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9262)	265.700	16.727	6.25	43.42
OUTFLOW: ID= 1 (5208)	265.700	6.353	7.00	43.41

PEAK FLOW REDUCTION [Qout/Qin] (%) = 37.98
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 5.5756

ADD HYD (8148)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2082):	426.60	40.830	6.00	49.78
+ ID2= 2 (5208):	265.70	6.353	7.00	43.41
=====				
ID = 3 (8148):	692.30	41.170	6.00	47.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8146) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6208):	14723.27	106.443	9.25	26.32
+ ID2= 2 (8148):	692.30	41.170	6.00	47.33
ID = 3 (8146):	15415.57	110.966	9.25	27.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6210)
IN= 2---> OUT= 1 | Routing time step (min)' = 15.00

----- DATA FOR SECTION (2101.0) ----->

Distance	Elevation	Manning	
0.00	241.81	0.0800	
28.43	241.78	0.0800	
85.30	237.04	0.0800	
149.27	231.32	0.0800	
170.60	230.10	0.0800	
255.89	229.37	0.0800	
277.22	228.68	0.0800	
284.33	228.42	0.0800	
291.43	228.16	0.0800	
297.76	227.00	0.0800 / 0.0350	Main Channel
298.76	225.00	0.0350	Main Channel
312.76	225.00	0.0350	Main Channel
313.76	227.00	0.0350 / 0.0800	Main Channel
326.97	229.50	0.0800	
348.30	230.29	0.0800	
376.73	229.76	0.0800	
490.46	235.75	0.0800	
568.65	236.24	0.0800	
661.06	241.85	0.0800	
703.71	246.04	0.0800	

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.67	225.67	.141E+05	6.6	0.69	35.86
1.33	226.33	.289E+05	20.4	1.04	23.65
2.00	227.00	.443E+05	39.3	1.31	18.81
2.93	227.93	.730E+05	77.8	1.57	15.64
3.85	228.85	.123E+06	131.1	1.57	15.65
4.78	229.78	.228E+06	205.6	1.33	18.51
5.70	230.70	.503E+06	356.7	1.05	23.49
6.63	231.63	.846E+06	608.7	1.06	23.17
7.55	232.55	.123E+07	946.4	1.14	21.65
8.48	233.48	.165E+07	1367.0	1.22	20.13
9.41	234.41	.211E+07	1871.8	1.31	18.80
10.33	235.33	.261E+07	2463.2	1.40	17.65
11.26	236.26	.317E+07	2999.4	1.40	17.63
12.18	237.18	.384E+07	3848.3	1.48	16.63
13.11	238.11	.454E+07	4806.0	1.56	15.75
14.03	239.03	.528E+07	5876.9	1.64	14.97
14.96	239.96	.605E+07	7062.7	1.72	14.29
15.88	240.88	.687E+07	8365.0	1.80	13.68
16.81	241.81	.771E+07	9506.9	1.82	13.52

----- hydrograph -----> <- pipe / channel -->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8146)	*****	110.97	9.25	27.27	3.50	1.57
OUTFLOW: ID= 1 (6210)	*****	110.53	9.50	27.27	3.49	1.57

CALIB STANDHYD (2104)
ID= 1 DT=15.0 min | Area (ha)= 192.70
Total Imp(%)= 73.00 | Dir. Conn.(%)= 68.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	140.67	52.03
Dep. Storage	1.00	5.00
Average Slope	1.00	2.00
Length	1133.43	40.00

Mannings n	=	0.013	0.250
Max. Eff. Inten. (mm/hr)=	over (min)	96.49	73.69
Storage Coeff. (min)=	(ii)	15.00	30.00
Unit Hyd. Tpeak (min)=		11.12	19.10 (ii)
Unit Hyd. peak (cms)=		15.00	30.00
		0.08	0.05
PEAK FLOW (cms)=		28.67	5.53
TIME TO PEAK (hrs)=		6.00	6.25
RUNOFF VOLUME (mm)=		72.10	39.82
TOTAL RAINFALL (mm)=		73.10	73.10
RUNOFF COEFFICIENT =		0.99	0.54

TOTALS
32.306 (iii)
6.00
61.77
73.10
0.85

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 81.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9283)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.5000	8.0000
0.3000	2.0000	3.5000	10.0000
0.9000	5.0000	4.5000	13.0000
2.0000	7.5000	5.5000	14.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2104)	192.700	32.306	6.00	61.77
OUTFLOW: ID= 1 (9283)	192.700	2.670	7.25	61.76

PEAK FLOW REDUCTION [Qout/Qin](%) = 8.27
TIME SHIFT OF PEAK FLOW (min) = 75.00
MAXIMUM STORAGE USED (ha. m.) = 8.3493

CALIB STANDHYD (2103)
ID= 1 DT=15.0 min | Area (ha)= 11.10
Total Imp(%)= 30.00 | Dir. Conn.(%)= 10.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	3.33	7.77
Dep. Storage	1.00	5.00
Average Slope	1.00	2.00
Length	272.03	40.00
Mannings n	=	0.013

Max. Eff. Inten. (mm/hr)=	over (min)	96.49	90.03
Storage Coeff. (min)=	(ii)	15.00	15.00
Unit Hyd. Tpeak (min)=		4.72	12.08 (ii)
Unit Hyd. peak (cms)=		15.00	15.00
		0.11	0.08

PEAK FLOW (cms)=	0.29	1.50	*TOTALS* 1.790 (iii)
TIME TO PEAK (hrs)=	6.00	6.00	6.00
RUNOFF VOLUME (mm)=	72.10	44.83	47.56
TOTAL RAINFALL (mm)=	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.99	0.61	0.65

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9284)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.5600	0.2067

0.0400	0.1045	0.7400	0.2572
0.2200	0.1166	0.9300	0.3046
0.4000	0.1670	1.0900	0.3510

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2103)	11.100	1.790	6.00	47.56
OUTFLOW: ID= 1 (9284)	11.100	0.621	6.25	47.50

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 34.70
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 0.2272

ADD HYD (9258)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9283):	192.70	2.670	7.25	61.76
+ ID2= 2 (9284):	11.10	0.621	6.25	47.50

ID = 3 (9258):	203.80	3.071	6.75	60.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5210)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.0060	0.0024
0.0020	0.0013	0.0080	0.0027
0.0030	0.0017	0.0090	0.0031
0.0050	0.0020	0.0090	0.0131

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9258)	203.800	3.071	6.75	60.99
OUTFLOW: ID= 1 (5210)	203.800	3.098	7.00	60.98

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 100.88
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 0.0817

**** WARNING : HYDROGRAPH PEAK WAS NOT REDUCED.
 CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.

CALIB NASHYD (2102)
 ID= 1 DT=15.0 min

Area (ha) = 14.38 Curve Number (CN) = 80.0
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp (hrs) = 1.08

Unit Hyd Qpeak (cms) = 0.228
 PEAK FLOW (cms) = 0.226 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 34.918
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.478

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9210)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2102):	14.38	0.226	7.25	34.92
+ ID2= 2 (5210):	203.80	3.098	7.00	60.98

ID = 3 (9210):	218.18	3.322	7.00	59.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8150)
 1 + 2 = 3

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
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ID1= 1 (6210):	15415.57	110.530	9.50	27.27
+ ID2= 2 (9210):	218.18	3.322	7.00	59.27

ID = 3 (8150):	15633.75	113.031	9.50	27.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2121)
 ID= 1 DT=15.0 min

Area (ha) = 56.89
 Total Imp(%) = 91.00 Dir. Conn. (%) = 74.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	51.77	5.12
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	615.85	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	96.49	203.64
over (min)	15.00	30.00
Storage Coeff. (min)	9.50 (ii)	17.54 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.05

TOTALS
 (cms) = 9.67 1.57
 (hrs) = 6.00 6.25 10.739 (iii)
 (mm) = 71.10 46.02 6.00
 (mm) = 73.10 73.10 64.58
 = 0.97 0.63 73.10
 = 0.88

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2124)
 ID= 1 DT=15.0 min

Area (ha) = 4.40
 Total Imp(%) = 75.00 Dir. Conn. (%) = 70.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	3.30	1.10
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	171.27	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	96.49	53.33
over (min)	15.00	30.00
Storage Coeff. (min)	4.41 (ii)	18.16 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.11	0.05

TOTALS
 (cms) = 0.81 0.09 0.863 (iii)
 (hrs) = 6.00 6.25 6.00
 (mm) = 71.10 28.97 58.46
 (mm) = 73.10 73.10 73.10
 = 0.97 0.40 0.80

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9290)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.2200	0.1226
0.0100	0.0778	0.2500	0.1387
0.0800	0.0934	0.3700	0.1503
0.1600	0.1076	0.4300	0.1616

INFLOW : ID= 2 (2124) AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 4.400 0.863 6.00 58.46
 OUTFLOW: ID= 1 (9290) 4.400 0.278 6.25 58.01

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 32.28
 TIME SHIF T OF PEAK FLOW (mi n) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 0.1386

ADD HYD (9264)
 1 + 2 = 3
 ID1= 1 (2121): AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 56.89 10.739 6.00 64.58
 + ID2= 2 (9290): 4.40 0.278 6.25 58.01
 ID= 3 (9264): 61.29 10.894 6.00 64.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5212)
 IN= 2--> OUT= 1
 DT= 15.0 mi n
 OUTFLOW (cms) STORAGE (ha. m.) OUTFLOW (cms) STORAGE (ha. m.)
 0.0000 0.0000 4.1000 2.1402
 0.2080 1.2793 5.1480 2.2661
 1.1880 1.5546 5.9930 2.3900
 2.2120 1.7255 6.3930 2.4000
 3.0680 1.9336 0.0000 0.0000

INFLOW : ID= 2 (9264) AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 61.290 10.894 6.00 64.11
 OUTFLOW: ID= 1 (5212) 61.290 3.571 6.50 64.08

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 32.77
 TIME SHIF T OF PEAK FLOW (mi n) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 2.0574

CALIB STANDHYD (2122)
 ID= 1 DT=15.0 mi n
 Area (ha)= 267.10
 Total Imp(%)= 29.00 Di r. Conn.(%)= 24.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 77.46 189.64
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1334.42 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 96.49 48.16
 over (mi n) = 15.00 30.00
 Storage Coeff. (mi n)= 15.10 (ii) 29.43 (ii)
 Uni t Hyd. Tpeak (mi n)= 15.00 30.00
 Uni t Hyd. peak (cms)= 0.07 0.04

TOTALS
 PEAK FLOW (cms)= 12.48 10.58 19.083 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 71.10 29.22 39.27
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.40 0.54

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9312)
 IN= 2--> OUT= 1
 DT= 15.0 mi n
 OUTFLOW (cms) STORAGE (ha. m.) OUTFLOW (cms) STORAGE (ha. m.)
 0.0000 0.0000 13.3600 4.9180
 0.9100 2.4658 17.8700 6.1368
 5.1800 2.7404 22.4400 7.2882

9.6400 3.9605 | 26.1200 8.4139

INFLOW : ID= 2 (2122) AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 267.100 19.083 6.00 39.27
 OUTFLOW: ID= 1 (9312) 267.100 9.869 6.75 39.27

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 51.72
 TIME SHIF T OF PEAK FLOW (mi n) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 4.0932

ADD HYD (8152)
 1 + 2 = 3
 ID1= 1 (5212): AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 61.29 3.571 6.50 64.08
 + ID2= 2 (9312): 267.10 9.869 6.75 39.27
 ID= 3 (8152): 328.39 13.378 6.50 43.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6214)
 IN= 2--> OUT= 1
 Routing time step (mi n) = 15.00

<----- DATA FOR SECTION (2141.0) ----->
 Di stance El evation Manni ng
 0.00 259.29 0.0500
 15.33 258.91 0.0500
 38.33 258.57 0.0500
 65.16 257.93 0.0500
 95.82 253.44 0.0500
 111.15 251.21 0.0500
 130.32 249.67 0.0500
 149.48 248.51 0.0500
 150.66 248.50 0.0500 /0.0370 Mai n Channel
 150.81 247.50 0.0370 Mai n Channel
 154.66 247.50 0.0370 Mai n Channel
 155.66 248.50 0.0370 /0.0500 Mai n Channel
 160.98 248.50 0.0500
 187.81 249.78 0.0500
 199.31 252.13 0.0500
 245.30 254.04 0.0500
 264.46 253.99 0.0500
 298.96 255.23 0.0500
 329.62 257.02 0.0500
 379.45 258.82 0.0500

----- TRAVEL TIME TABLE ----->
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV. TIME
 (m) (m) (cu. m.) (cms) (m/s) (mi n)
 0.50 248.00 .648E+04 2.1 1.00 52.30
 1.00 248.50 .139E+05 6.4 1.44 36.35
 1.61 249.11 .570E+05 22.6 1.24 42.01
 2.21 249.71 .143E+06 63.6 1.39 37.61
 2.82 250.32 .263E+06 142.9 1.70 30.69
 3.43 250.93 .403E+06 257.2 2.00 26.11
 4.04 251.54 .562E+06 410.4 2.29 22.82
 4.64 252.14 .735E+06 601.3 2.56 20.37
 5.25 252.75 .933E+06 791.0 2.66 19.67
 5.86 253.36 .117E+07 1038.0 2.79 18.74
 6.46 253.96 .144E+07 1344.4 2.93 17.82
 7.07 254.57 .178E+07 1664.1 2.93 17.83
 7.68 255.18 .217E+07 2140.2 3.10 16.87
 8.28 255.78 .259E+07 2722.9 3.30 15.83
 8.89 256.39 .304E+07 3387.1 3.50 14.94
 9.50 257.00 .351E+07 4132.7 3.69 14.16
 10.11 257.61 .402E+07 4913.0 3.83 13.64
 10.71 258.21 .458E+07 5648.3 3.87 13.50
 11.32 258.82 .521E+07 6343.1 3.82 13.68

<---- hydrograph ----> <-pi pe / channel ->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8152) 328.39 13.38 6.50 43.90 1.26 1.35
 OUTFLOW: ID= 1 (6214) 328.39 10.19 7.25 43.90 1.14 1.39

CALIB
STANDHYD (2144)
ID= 1 DT=15.0 min

Area (ha)= 93.10
Total Imp(%)= 75.00 Dir. Conn.(%)= 70.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	69.82	23.27
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	787.82	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	54.88
over (min)	15.00	30.00
Storage Coeff. (min)=	11.01 (ii)	24.61 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04

TOTALS
15.335 (iii)
6.00
58.70
73.10
0.80

PEAK FLOW (cms)=	14.31	1.63
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	71.10	29.76
TOTAL RAINFALL (mm)=	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9285)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.6500	2.6150
0.3200	1.6583	6.2300	2.9579
1.8100	1.9916	7.8200	3.2041
3.3600	2.2943	9.1000	3.4456

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2144)	93.100	15.335	6.00	58.70
OUTFLOW: ID= 1 (9285)	93.100	5.284	6.50	58.69

PEAK FLOW REDUCTION [Qout/Qin] (%) = 34.46
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 2.7959

CALIB
STANDHYD (2143)
ID= 1 DT=15.0 min

Area (ha)= 158.30
Total Imp(%)= 26.00 Dir. Conn.(%)= 9.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	41.16	117.14
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1027.29	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	60.50
over (min)	15.00	30.00
Storage Coeff. (min)=	12.91 (ii)	25.99 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04

TOTALS
10.088 (iii)
6.25
35.43
73.10
0.48

PEAK FLOW (cms)=	2.96	8.81
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	71.10	31.91
TOTAL RAINFALL (mm)=	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9286)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	7.9200	2.8224
0.5400	1.3792	10.5900	3.5534
3.0700	1.5175	13.3000	4.2519
5.7200	2.2534	15.4900	4.9349

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2143)	158.300	10.088	6.25	35.43
OUTFLOW: ID= 1 (9286)	158.300	5.185	6.75	35.43

PEAK FLOW REDUCTION [Qout/Qin] (%) = 51.40
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 2.1083

ADD HYD (9260)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9285):	93.10	5.284	6.50	58.69
+ ID2= 2 (9286):	158.30	5.185	6.75	35.43

ID = 3 (9260): 251.40 10.106 6.50 44.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5214)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.5370	0.9877
0.1280	0.4635	3.1860	1.1293
0.5900	0.5362	3.7090	1.2678
1.3690	0.6921	3.9980	1.2778
1.8980	0.8262	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9260)	251.400	10.106	6.50	44.04
OUTFLOW: ID= 1 (5214)	251.400	10.733	6.75	44.04

PEAK FLOW REDUCTION [Qout/Qin] (%) = 106.20
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha. m.) = 1.5521

**** WARNING : HYDROGRAPH PEAK WAS NOT REDUCED.
CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.

CALIB
STANDHYD (2142)
ID= 1 DT=15.0 min

Area (ha)= 65.54
Total Imp(%)= 27.00 Dir. Conn.(%)= 10.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	17.70	47.84
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	661.01	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	19.99
over (min)	15.00	45.00
Storage Coeff. (min)=	9.91 (ii)	30.28 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.09	0.03

TOTALS
2.163 (iii)
6.00
23.18
73.10
0.32

PEAK FLOW (cms)=	1.49	1.64
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	71.10	17.86
TOTAL RAINFALL (mm)=	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.24

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 51.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2142):	65.54	2.163	6.00	23.18
+ ID2= 2 (5214):	251.40	10.733	6.75	44.04

ID = 3 (9214):	316.94	12.150	6.75	39.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6214):	328.39	10.187	7.25	43.90
+ ID2= 2 (9214):	316.94	12.150	6.75	39.73

ID = 3 (8154):	645.33	20.647	6.75	41.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2163) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	69.70 27.00	Di r. Conn. (%)=	9.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	18.82	50.88
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	681.66	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	73.12
over (min)	15.00	30.00
Storage Coeff. (min)=	10.09 (ii)	22.22 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.04

	TOTALS
PEAK FLOW (cms)=	1.42
TIME TO PEAK (hrs)=	6.00
RUNOFF VOLUME (mm)=	71.10
TOTAL RAINFALL (mm)=	73.10
RUNOFF COEFFICIENT =	0.97

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 78.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9271) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	3.4900	1.2605
	0.2400	0.6230	4.6600	1.5808
	1.3500	0.6885	5.8600	1.8854
	2.5200	1.0102	6.8200	2.1832

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2163)	69.700	5.514	6.25	40.72

OUTFLOW: ID= 1 (9271) 69.700 2.833 6.75 40.71

PEAK FLOW REDUCTION [Qout/Qin] (%) = 51.37
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 1.1058

CALIB STANDHYD (2169) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	3.50 30.00	Di r. Conn. (%)=	10.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	1.05	2.45
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	152.75	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	59.87
over (min)	15.00	30.00
Storage Coeff. (min)=	4.11 (ii)	17.25 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.11	0.05

	TOTALS
PEAK FLOW (cms)=	0.09
TIME TO PEAK (hrs)=	6.00
RUNOFF VOLUME (mm)=	71.10
TOTAL RAINFALL (mm)=	73.10
RUNOFF COEFFICIENT =	0.97

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9272) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	0.1800	0.0659
	0.0100	0.0333	0.2400	0.0820
	0.0700	0.0372	0.3000	0.0971
	0.1300	0.0532	0.3500	0.1119

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2169)	3.500	0.234	6.25	34.40
OUTFLOW: ID= 1 (9272)	3.500	0.117	6.75	34.16

PEAK FLOW REDUCTION [Qout/Qin] (%) = 49.96
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 0.0508

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9271):	69.70	2.833	6.75	40.71
+ ID2= 2 (9272):	3.50	0.117	6.75	34.16

ID = 3 (9251):	73.20	2.950	6.75	40.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5216) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	3.7400	2.4559
	0.1890	1.6831	4.6960	2.6646
	1.0830	1.7903	5.4670	2.8688
	2.0170	2.0202	5.8670	2.8788

2.7980 2.2179 | 0.0000 0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9251)	73.200	2.950	6.75	40.40
OUTFLOW: ID= 1 (5216)	73.200	0.464	9.00	40.37

PEAK FLOW REDUCTION [Qout/Qin] (%) = 15.74
 TIME SHIFT OF PEAK FLOW (min) = 135.00
 MAXIMUM STORAGE USED (ha. m.) = 1.7162

CALIB STANDHYD (2162) ID= 1 DT=15.0 min	Area (ha)	Total Imp(%)	Dir. Conn. (%)
	72.57	20.00	7.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	14.51	58.06
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	695.56	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	96.49	50.68
Storage Coeff. (min)	15.00	30.00
Unit Hyd. Tpeak (min)	10.22 (ii)	24.25 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.04

TOTALS
 PEAK FLOW (cms) = 1.14 3.77 4.177 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.25
 RUNOFF VOLUME (mm) = 71.10 28.43 31.41
 TOTAL RAINFALL (mm) = 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.39 0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9316)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.0000	0.3000
2.0000	0.1000	4.5000	0.4000
3.0000	0.2000	5.0000	0.5000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2162)	72.570	4.177	6.25	31.41
OUTFLOW: ID= 1 (9316)	72.570	3.408	6.50	31.41

PEAK FLOW REDUCTION [Qout/Qin] (%) = 81.58
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 0.2633

ADD HYD (9216)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (5216):	73.20	0.464	9.00	40.37
+ ID2= 2 (9316):	72.57	3.408	6.50	31.41
ID = 3 (9216):	145.77	3.449	6.50	35.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8156)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)

ID1= 1 (8154):	645.33	20.647	6.75	41.85
+ ID2= 2 (9216):	145.77	3.449	6.50	35.91
ID = 3 (8156):	791.10	23.552	6.75	40.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6218)
 IN= 2--> OUT= 1 Routing time step (min) = 15.00

Distance	Elevation	Manning
0.00	256.38	0.0450
7.70	254.85	0.0450
15.40	253.20	0.0450
19.25	252.29	0.0450
23.10	251.17	0.0450
27.94	236.16	0.0450
31.79	235.80	0.0450
35.64	235.45	0.0450
39.49	234.60	0.0450 / 0.0300
43.34	233.60	0.0300
47.19	233.60	0.0300
51.04	233.60	0.0300
54.89	234.60	0.0300 / 0.0450
58.74	235.13	0.0450
62.59	235.20	0.0450
66.44	235.32	0.0450
70.29	239.95	0.0450
74.14	244.80	0.0450
77.99	245.00	0.0450
81.84	256.00	0.0450

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
1.00	234.60	.181E+05	6.0	0.66	50.64
2.13	235.73	.518E+05	23.8	0.93	36.27
3.25	236.85	.119E+06	60.0	1.02	33.03
4.38	237.98	.217E+06	120.6	1.12	29.99
5.51	239.11	.346E+06	210.5	1.23	27.39
6.63	240.23	.506E+06	334.1	1.33	25.22
7.76	241.36	.697E+06	495.6	1.43	23.44
8.88	242.48	.920E+06	699.8	1.53	21.91
10.01	243.61	.117E+07	950.8	1.63	20.59
11.14	244.74	.146E+07	1252.5	1.73	19.45
12.26	245.86	.178E+07	1619.6	1.83	18.37
13.39	246.99	.212E+07	2078.5	1.97	17.01
14.52	248.12	.247E+07	2589.0	2.11	15.88
15.64	249.24	.282E+07	3149.7	2.25	14.94
16.77	250.37	.319E+07	3759.8	2.37	14.14
17.89	251.49	.356E+07	4419.5	2.50	13.44
19.02	252.62	.395E+07	5126.4	2.61	12.84
20.15	253.75	.435E+07	5871.6	2.72	12.34
21.27	254.87	.476E+07	6663.2	2.82	11.89

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8156)	791.10	23.55	6.75	40.76	2.11	0.92
OUTFLOW: ID= 1 (6218)	791.10	17.09	7.25	40.75	1.71	0.81

CALIB
 STANDHYD (2183)
 ID= 1 DT=15.0 min

Area (ha)	Total Imp(%)	Dir. Conn. (%)
69.30	22.00	7.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	15.25	54.05
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	679.71	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	96.49	72.27
Storage Coeff. (min)	15.00	30.00
Unit Hyd. Tpeak (min)	10.08 (ii)	22.26 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.04

TOTALS

PEAK FLOW (cms)= 1.10 5.27 5.652 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.25
 RUNOFF VOLUME (mm)= 71.10 38.87 41.12
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.53 0.56

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 80.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9282)				
IN= 2---> OUT= 1				
DT= 15.0 min				
OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)	
0.0000	0.0000	4.5000	3.5000	
0.8000	0.1000	6.0000	4.0000	
1.1000	2.0000	6.5000	4.5000	
2.5000	3.0000	7.0000	5.5000	

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2183)	69.300	5.652	6.25	41.12
OUTFLOW: ID= 1 (9282)	69.300	0.999	7.50	41.12

PEAK FLOW REDUCTION [Qout/Qi n](%)= 17.67
 TIME SHIFT OF PEAK FLOW (min)= 75.00
 MAXIMUM STORAGE USED (ha. m.)= 1.3608

RESERVOIR (5218)				
IN= 2---> OUT= 1				
DT= 15.0 min				
OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)	
0.0000	0.0000	4.5090	1.7553	
0.2280	0.8236	5.6620	2.0069	
1.3060	0.9528	6.5910	2.2530	
2.4320	1.2300	6.9910	2.2630	
3.3740	1.4683	0.0000	0.0000	

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9282)	69.300	0.999	7.50	41.12
OUTFLOW: ID= 1 (5218)	69.300	0.956	9.50	41.11

PEAK FLOW REDUCTION [Qout/Qi n](%)= 95.72
 TIME SHIFT OF PEAK FLOW (min)=120.00
 MAXIMUM STORAGE USED (ha. m.)= 0.9110

CALIB NASHYD (2182)				
ID= 1 DT=15.0 min				
Area (ha)=	82.95	Curve Number (CN)=	67.7	
Ia (mm)=	5.00	# of Linear Res. (N)=	1.50	
U. H. Tp(hrs)=	0.79			

Unit Hyd Opeak (cms)= 1.786
 PEAK FLOW (cms)= 1.131 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 24.148
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.330

- (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9218)				
1 + 2 = 3				
ID1= 1 (2182):	82.95	1.131	7.00	24.15
+ ID2= 2 (5218):	69.30	0.956	9.50	41.11

ID = 3 (9218):	152.25	1.735	8.75	31.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8158)				
1 + 2 = 3				
ID1= 1 (6218):	791.10	17.085	7.25	40.75
+ ID2= 2 (9218):	152.25	1.735	8.75	31.87

ID = 3 (8158):	943.35	18.362	7.25	39.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8151)				
1 + 2 = 3				
ID1= 1 (8150):	15633.75	113.031	9.50	27.71
+ ID2= 2 (8158):	943.35	18.362	7.25	39.32

ID = 3 (8151):	16577.10	125.120	8.50	28.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5504)				
IN= 2---> OUT= 1				
DT= 15.0 min				
OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)	
0.0000	0.0000	33.0100	0.6700	
2.6400	0.0500	37.4800	0.7800	
4.8300	0.0900	47.5400	1.0100	
7.4100	0.1300	60.2300	1.2800	
10.3300	0.1900	71.9530	1.5258	
13.5300	0.2500	*****	3.0529	
16.9900	0.3200	*****	5.3299	
20.6800	0.3900	*****	8.4148	
24.6000	0.4800	*****	11.7637	
28.7100	0.5700	*****	11.7737	

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (8151)	*****	125.120	8.50	28.37
OUTFLOW: ID= 1 (5504)	*****	125.142	8.50	28.37

PEAK FLOW REDUCTION [Qout/Qi n](%)=100.02
 TIME SHIFT OF PEAK FLOW (min)= 0.00
 MAXIMUM STORAGE USED (ha. m.)= 2.6552

**** WARNING : HYDROGRAPH PEAK WAS NOT REDUCED.
 CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.

ROUTE CHN (6220)				
IN= 2---> OUT= 1				
Routing time step (min)= 15.00				
<----- DATA FOR SECTION (2201.0) ----->				
Distance	Elevation	Manning		
0.00	245.59	0.0600		
5.76	245.45	0.0600		
23.03	242.27	0.0600		
57.58	232.35	0.0600		
86.37	226.18	0.0600		
109.41	224.31	0.0600		
115.17	223.86	0.0600		
120.92	222.78	0.0600		
126.68	221.93	0.0600		
128.06	221.50	0.0600 / 0.0300	Main Channel	
129.56	220.00	0.0300	Main Channel	
141.56	220.00	0.0300	Main Channel	
143.06	221.50	0.0300 / 0.0600	Main Channel	
149.72	223.77	0.0600		
155.47	225.09	0.0600		
253.36	229.21	0.0600		
333.98	227.50	0.0600		
454.90	229.25	0.0600		
500.97	233.11	0.0600		
570.07	237.42	0.0600		

TRAVEL TIME TABLE						
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)	
0.75	220.75	.262E+05	5.5	0.57	79.44	
1.50	221.50	.555E+05	17.5	0.86	52.89	
2.44	222.44	.103E+06	42.7	1.14	40.03	
3.37	223.37	.171E+06	79.4	1.27	35.98	
4.31	224.31	.263E+06	128.1	1.33	34.22	
5.25	225.25	.393E+06	191.7	1.34	34.19	
6.18	226.18	.594E+06	276.7	1.28	35.77	
7.12	227.12	.872E+06	399.9	1.26	36.33	
8.06	228.06	.127E+07	542.7	1.17	38.92	
8.99	228.99	.199E+07	770.0	1.06	43.01	
9.93	229.93	.297E+07	1151.8	1.06	42.95	
10.86	230.86	.400E+07	1652.9	1.13	40.33	
11.80	231.80	.507E+07	2251.8	1.22	37.53	
12.74	232.74	.618E+07	2946.5	1.31	34.97	
13.67	233.67	.733E+07	3726.8	1.39	32.79	
14.61	234.61	.853E+07	4591.0	1.48	30.96	
15.55	235.55	.977E+07	5544.1	1.55	29.37	
16.48	236.48	.111E+08	6586.7	1.63	27.99	
17.42	237.42	.124E+08	7719.2	1.71	26.77	

		<---- hydrograph ---->				<- pi pe / channel ->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW: ID= 2 (5504)	*****	125.14	8.50	28.37	4.25	1.33	
OUTFLOW: ID= 1 (6220)	*****	121.25	9.50	28.37	4.18	1.33	

CALIB STANDHYD (2207)			
ID= 1 DT=15.0 mi n	Area Total (ha)	(ha)= 57.80	Di r. Conn. (%)= 8.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	12.14	45.66
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	620.75	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)= over (mi n)	96.49	73.88
Storage Coeff. (mi n)=	9.54 (ii)	21.62 (ii)
Uni t Hyd. Tpeak (mi n)=	15.00	30.00
Uni t Hyd. peak (cms)=	0.09	0.05

TOTALS		
PEAK FLOW (cms)=	1.06	4.62
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	71.10	40.57
TOTAL RAI NFALL (mm)=	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.56

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 82.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9278)			
IN= 2 ---> OUT= 1	DT= 15.0 mi n	OUTFLOW (cms)	STORAGE (ha. m.)
		0.0000	0.0000
		0.2000	0.3000
		0.3000	0.9000
		0.4000	2.0000
			0.5000
			0.6000
			0.7000
			0.8000
			0.9000
			1.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2207)	57.800	4.972	6.25	43.02
OUTFLOW: ID= 1 (9278)	57.800	0.375	9.00	43.01

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 7.54
 TIME SHIFT OF PEAK FLOW (mi n) = 165.00

MAXIMUM STORAGE USED (ha. m.) = 1.7232

CALIB STANDHYD (2204)			
ID= 1 DT=15.0 mi n	Area Total (ha)	(ha)= 13.30	Di r. Conn. (%)= 70.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	9.98	3.33
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	297.77	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)= over (mi n)	96.49	70.94
Storage Coeff. (mi n)=	6.14 (ii)	18.41 (ii)
Uni t Hyd. Tpeak (mi n)=	15.00	30.00
Uni t Hyd. peak (cms)=	0.10	0.05

TOTALS		
PEAK FLOW (cms)=	2.35	0.34
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	71.10	37.95
TOTAL RAI NFALL (mm)=	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.52

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9279)			
IN= 2 ---> OUT= 1	DT= 15.0 mi n	OUTFLOW (cms)	STORAGE (ha. m.)
		0.0000	0.0000
		0.0500	0.2376
		0.2600	0.2854
		0.4800	0.3287
			0.6700
			0.8900
			1.1200
			1.3000
			0.3747
			0.4238
			0.4591
			0.4937

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2204)	13.300	2.578	6.00	61.16
OUTFLOW: ID= 1 (9279)	13.300	0.847	6.25	61.07

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 32.86
 TIME SHIFT OF PEAK FLOW (mi n) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 0.4148

ADD HYD (9255)			
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)
ID1= 1 (9278):	57.80	0.375	9.00
+ ID2= 2 (9279):	13.30	0.847	6.25
=====			
ID = 3 (9255):	71.10	1.131	6.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5220)			
IN= 2 ---> OUT= 1	DT= 15.0 mi n	OUTFLOW (cms)	STORAGE (ha. m.)
		0.0000	0.0000
		0.1320	0.4759
		0.7550	0.5505
		1.4050	0.7107
		1.9490	0.8484
			2.6050
			3.2710
			3.8080
			4.2080
			0.0000
			1.0142
			1.1595
			1.3018
			1.3118
			0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9255)	71.100	1.131	6.25	46.39
OUTFLOW: ID= 1 (5220)	71.100	0.614	8.00	46.37

PEAK FLOW REDUCTION [Oout/Oin] (%) = 54.26
 TIME SHIFT OF PEAK FLOW (min) = 105.00
 MAXIMUM STORAGE USED (ha.m.) = 0.5340

CALIB NASHYD (2202) Area (ha) = 98.00 Curve Number (CN) = 74.3
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.50

Unit Hyd Qpeak (cms) = 1.115

PEAK FLOW (cms) = 1.000 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 29.570
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.405

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9220)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (2202):	98.00	1.000	8.00	29.57
+ ID2= 2 (5220):	71.10	0.614	8.00	46.37
ID = 3 (9220):	169.10	1.613	8.00	36.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8160)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6220):	16577.10	121.252	9.50	28.37
+ ID2= 2 (9220):	169.10	1.613	8.00	36.64
ID = 3 (8160):	16746.20	122.675	9.50	28.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2221) Area (ha) = 110.64 Dir. Conn. (%) = 20.00
 ID= 1 DT=15.0 min Total Imp(%) = 37.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 40.94 69.70
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 858.84 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 96.49 69.84
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 11.59 (ii) 23.94 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.08 0.04

TOTALS
 PEAK FLOW (cms) = 4.77 6.32 8.837 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 71.10 35.49 42.61
 TOTAL RAINFALL (mm) = 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.49 0.58

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.3 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2224) Area (ha) = 43.20 Dir. Conn. (%) = 66.00
 ID= 1 DT=15.0 min Total Imp(%) = 72.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 31.10 12.10
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 536.66 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 96.49 72.23
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 8.74 (ii) 20.93 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.09 0.05

TOTALS
 PEAK FLOW (cms) = 6.70 1.21 7.488 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 71.10 38.19 59.91
 TOTAL RAINFALL (mm) = 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.52 0.82

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9288) IN= 2---> OUT= 1 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	2.1600	1.1836
	0.1500	0.7428	2.8900	1.3455
	0.8400	0.8895	3.6300	1.4651
	1.5600	1.0342	4.2300	1.5823

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (2224)	43.200	7.488	6.00	59.91
OUTFLOW: ID= 1 (9288)	43.200	2.563	6.50	59.88

PEAK FLOW REDUCTION [Oout/Oin] (%) = 34.23
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 1.2977

ADD HYD (9261)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (2221):	110.64	8.837	6.00	42.61
+ ID2= 2 (9288):	43.20	2.563	6.50	59.88
ID = 3 (9261):	153.84	10.743	6.25	47.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5222) IN= 2---> OUT= 1 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	7.2210	4.0213
	0.4910	2.1348	9.0080	4.5380
	2.1830	2.4433	15.0420	4.9977
	3.9370	2.9883	15.4420	5.0077
	5.4550	3.4692	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (9261)	153.840	10.743	6.25	47.46
OUTFLOW: ID= 1 (5222)	153.840	4.394	7.00	47.46

PEAK FLOW REDUCTION [Oout/Oin] (%) = 40.90
 TIME SHIFT OF PEAK FLOW (min) = 45.00

MAXIMUM STORAGE USED (ha. m.) = 3.1342

CALIB STANDHYD (2222)
ID= 1 DT=15.0 min
Area (ha)= 65.69
Total Imp(%)= 84.00
Dir. Conn. (%) = 45.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	55.18	10.51
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	661.77	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	96.49	278.81
over (min)	15.00	30.00
Storage Coeff. (min)	9.91 (ii)	17.01 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.05

			TOTALS
PEAK FLOW (cms)	6.71	4.49	9.850 (iii)
TIME TO PEAK (hrs)	6.00	6.25	6.00
RUNOFF VOLUME (mm)	71.10	53.53	61.44
TOTAL RAINFALL (mm)	73.10	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.73	0.84

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8164) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2222):	65.69	9.850	6.00	61.44
+ ID2= 2 (5222):	153.84	4.394	7.00	47.46
=====				
ID = 3 (8164):	219.53	10.118	6.00	51.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2242)
ID= 1 DT=15.0 min
Area (ha)= 52.75
Total Imp(%)= 36.00
Dir. Conn. (%) = 27.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	18.99	33.76
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	593.01	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	96.49	65.61
over (min)	15.00	30.00
Storage Coeff. (min)	9.28 (ii)	21.95 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.04

			TOTALS
PEAK FLOW (cms)	3.29	3.00	5.232 (iii)
TIME TO PEAK (hrs)	6.00	6.25	6.00
RUNOFF VOLUME (mm)	71.10	36.92	46.15
TOTAL RAINFALL (mm)	73.10	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.51	0.63

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

STANDHYD (2244)
ID= 1 DT=15.0 min
Area (ha)= 87.70
Total Imp(%)= 70.00
Dir. Conn. (%) = 64.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	61.39	26.31
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	764.63	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	96.49	70.94
over (min)	15.00	30.00
Storage Coeff. (min)	10.81 (ii)	23.08 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04

			TOTALS
PEAK FLOW (cms)	12.40	2.47	13.997 (iii)
TIME TO PEAK (hrs)	6.00	6.25	6.00
RUNOFF VOLUME (mm)	71.10	37.95	59.17
TOTAL RAINFALL (mm)	73.10	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.52	0.81

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9287) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	4.3900	2.3776
	0.3000	1.4858	5.8700	2.7087
	1.7000	1.7770	7.3700	2.9557
	3.1700	2.0741	8.5800	3.1980

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2244)	87.700	13.997	6.00	59.17
OUTFLOW: ID= 1 (9287)	87.700	5.083	6.50	59.15

PEAK FLOW REDUCTION [Oout/Oin] (%) = 36.32
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 2.5642

RESERVOIR (5224) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	0.4470	0.1810
	0.0230	0.0884	0.5620	0.2047
	0.1300	0.1032	0.6540	0.2278
	0.2410	0.1297	1.0540	0.2378
	0.3350	0.1532	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9287)	87.700	5.083	6.50	59.15
OUTFLOW: ID= 1 (5224)	87.700	5.376	6.50	59.14

PEAK FLOW REDUCTION [Oout/Oin] (%) = 105.75
TIME SHIFT OF PEAK FLOW (min) = 0.00
MAXIMUM STORAGE USED (ha. m.) = 0.3590

**** WARNING: HYDROGRAPH PEAK WAS NOT REDUCED.
CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.

ADD HYD (9224) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2242):	52.75	5.232	6.00	46.15
+ ID2= 2 (5224):	87.70	5.376	6.50	59.14
=====				
ID = 3 (9224):	140.45	8.011	6.50	54.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8166)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (8164):	219.53	10.118	6.00	51.64
+ ID2= 2 (9224):	140.45	8.011	6.50	54.26
ID = 3 (8166):	359.98	15.727	6.00	52.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6226)
IN= 2---> OUT= 1 Routing time step (min)' = 15.00

----- DATA FOR SECTION (2261.0) ----->

Distance	Elevation	Manning	
0.00	245.35	0.0600	
8.27	242.72	0.0600	
16.54	240.01	0.0600	
31.02	235.31	0.0600	
41.36	233.14	0.0600	
90.99	227.73	0.0600	
93.06	227.50	0.0600	
95.13	227.27	0.0600	
95.83	227.00	0.0600	
100.83	226.50	0.0600 / 0.0350	Main Channel
101.33	225.50	0.0350	Main Channel
101.83	226.50	0.0350 / 0.0600	Main Channel
105.83	227.00	0.0600	
107.54	227.39	0.0600	
109.60	227.62	0.0600	
111.67	227.86	0.0600	
146.83	231.76	0.0600	
173.71	234.77	0.0600	
186.12	238.23	0.0600	
202.67	238.35	0.0600	

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	226.00	.403E+03	0.0	0.16	330.59
1.00	226.50	.161E+04	0.1	0.26	208.26
1.70	227.20	.173E+05	1.5	0.27	195.72
2.39	227.89	.541E+05	5.9	0.35	152.27
3.09	228.59	.119E+06	16.0	0.43	124.08
3.79	229.29	.212E+06	33.8	0.51	104.65
4.49	229.99	.334E+06	61.2	0.59	90.97
5.18	230.68	.484E+06	99.7	0.66	80.88
5.88	231.38	.663E+06	151.0	0.73	73.13
6.58	232.08	.870E+06	216.5	0.80	66.95
7.27	232.77	.111E+07	297.5	0.87	61.91
7.97	233.47	.137E+07	398.1	0.94	57.29
8.67	234.17	.165E+07	518.9	1.01	53.12
9.36	234.86	.196E+07	658.9	1.08	49.59
10.06	235.56	.228E+07	827.0	1.17	46.02
10.76	236.26	.262E+07	1015.4	1.25	42.96
11.46	236.96	.296E+07	1221.2	1.33	40.41
12.15	237.65	.332E+07	1444.6	1.40	38.26
12.85	238.35	.368E+07	1598.2	1.40	38.42

----- hydrograph ----->

INFLOW : ID= 2 (8166)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW: ID= 1 (6226)	359.98	15.73	6.00	52.66	3.07	0.43
	359.98	6.55	7.25	52.66	2.44	0.36

CALIB NASHYD (2267)
ID= 1 DT=15.0 min
Area (ha)= 10.50
Ia (mm)= 5.00
U. H. Tp(hrs)= 0.47
Curve Number (CN)= 82.0
of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 0.383

PEAK FLOW (cms)= 0.341 (i)
TIME TO PEAK (hrs)= 6.500

RUNOFF VOLUME (mm)= 36.272
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.496

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9276)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.5300	0.1297
0.0400	0.0402	0.7100	0.1837
0.2000	0.0341	0.8900	0.2402
0.3800	0.0910	1.0300	0.2954

INFLOW : ID= 2 (2267)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (9276)	10.500	0.341	6.50	36.27
	10.500	0.282	7.25	36.25

PEAK FLOW REDUCTION [Qout/Qin] (%) = 82.68
TIME SHIF T OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.0608

**** ERROR : CHECK THE STORAGE-DISCHARGE TABLE.

CALIB STANDHYD (2264)
ID= 1 DT=15.0 min

Area (ha)= 117.80
Total Imp(%)= 75.00
Dir. Conn. (%) = 70.00

Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
88.35		29.45
Dep. Storage (mm)= 2.00		5.00
Average Slope (%)= 0.50		0.50
Length (m)= 886.19		40.00
Mannings n = 0.013		0.250

Max. Eff. Inten. (mm/hr)= 96.49
over (min)= 15.00
Storage Coeff. (min)= 11.81 (ii)
Unit Hyd. Tpeak (min)= 15.00
Unit Hyd. peak (cms)= 0.08

PEAK FLOW (cms)= 17.67
TIME TO PEAK (hrs)= 6.00
RUNOFF VOLUME (mm)= 71.10
TOTAL RAINFALL (mm)= 73.10
RUNOFF COEFFICIENT = 0.97

TOTALS

19.425 (iii)
6.00
61.16
73.10
0.84

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9277)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.5000	8.0000
0.3000	2.0000	3.5000	10.0000
0.9000	5.0000	4.5000	13.0000
2.0000	7.5000	5.5000	14.0000

INFLOW : ID= 2 (2264)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (9277)	117.800	19.425	6.00	61.16
	117.800	1.071	8.25	61.14

PEAK FLOW REDUCTION [Qout/Qin] (%) = 5.51
TIME SHIF T OF PEAK FLOW (min)=135.00
MAXIMUM STORAGE USED (ha. m.) = 5.3891

ADD HYD (9253)

1 + 2 = 3		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9276):		10.50	0.282	7.25	36.25
+ ID2= 2 (9277):		117.80	1.071	8.25	61.14
=====					
ID = 3 (9253):		128.30	1.314	7.75	59.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5226)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.7530	0.6823
0.0890	0.3202	2.2010	0.7801
0.5080	0.3704	2.5620	0.8758
0.9450	0.4782	2.9620	0.8858
1.3110	0.5708	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9253)	128.300	1.314	7.75	59.10
OUTFLOW: ID= 1 (5226)	128.300	1.229	8.50	59.10

PEAK FLOW REDUCTION [Qout/Qin] (%) = 93.59
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha.m.) = 0.5524

CALIB
NASHYD (2262)
ID= 1 DT=15.0 min

Area (ha) = 119.28 Curve Number (CN) = 77.1
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U.H. Tp(hrs) = 1.05

Unit Hyd Opeak (cms) = 1.934

PEAK FLOW (cms) = 1.744 (i)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 32.003
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.438

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9226)
1 + 2 = 3

1 + 2 = 3		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2262):		119.28	1.744	7.25	32.00
+ ID2= 2 (5226):		128.30	1.229	8.50	59.10
=====					
ID = 3 (9226):		247.58	2.854	8.00	46.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8168)
1 + 2 = 3

1 + 2 = 3		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6226):		359.98	6.549	7.25	52.66
+ ID2= 2 (9226):		247.58	2.854	8.00	46.04
=====					
ID = 3 (8168):		607.56	9.283	7.50	49.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8162)
1 + 2 = 3

1 + 2 = 3		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8160):		16746.20	122.675	9.50	28.46
+ ID2= 2 (8168):		607.56	9.283	7.50	49.96
=====					
ID = 3 (8162):		17353.76	129.804	9.00	29.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6228)
IN= 2--> OUT= 1

Routing time step (min) = 15.00

DATA FOR SECTION (2281.0)		
Distance	Elevation	Manning
0.00	245.00	0.0600
18.60	244.74	0.0600
37.20	242.47	0.0600
93.01	233.71	0.0600
125.57	226.95	0.0600
213.93	221.63	0.0600
218.58	221.15	0.0600
223.23	220.45	0.0600
225.03	220.55	0.0600 / 0.0300
225.53	218.05	0.0300
232.53	218.05	0.0300
240.03	218.05	0.0300
241.03	220.55	0.0300 / 0.0600
246.48	221.00	0.0600
251.13	221.27	0.0600
255.78	221.22	0.0600
325.54	225.25	0.0600
367.40	228.91	0.0600
404.60	231.75	0.0600
460.41	245.00	0.0600

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
1.25	219.30	.338E+05	16.0	0.86	35.15
2.50	220.55	.696E+05	48.3	1.26	24.02
3.94	221.99	.168E+06	116.0	1.26	24.14
5.38	223.43	.392E+06	247.5	1.15	26.37
6.81	224.86	.743E+06	472.6	1.16	26.20
8.25	226.30	.122E+07	821.6	1.23	24.67
9.69	227.74	.179E+07	1320.1	1.34	22.59
11.13	229.18	.243E+07	1965.7	1.47	20.58
12.57	230.62	.313E+07	2744.2	1.59	19.02
14.01	232.06	.390E+07	3683.4	1.72	17.66
15.44	233.49	.472E+07	4824.1	1.86	16.30
16.88	234.93	.557E+07	6092.4	1.99	15.23
18.32	236.37	.646E+07	7503.3	2.11	14.35
19.76	237.81	.739E+07	9061.2	2.23	13.59
21.20	239.25	.836E+07	10767.6	2.34	12.94
22.64	240.69	.937E+07	12624.8	2.45	12.37
24.07	242.12	.104E+08	14634.9	2.56	11.87
25.51	243.56	.115E+08	16762.8	2.65	11.45
26.95	245.00	.127E+08	18699.9	2.69	11.28

		<--- hydrograph --->				<- pi pe / channel ->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (8162)	*****	129.80	9.00	29.21	4.09	1.24	
OUTFLOW: ID= 1 (6228)	*****	127.89	9.75	29.21	4.07	1.25	

CALIB
STANDHYD (2287)
ID= 1 DT=15.0 min

Area (ha) = 130.50
Total Imp(%) = 23.00 Di r. Conn. (%) = 10.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 30.01 100.48
Dep. Storage (mm) = 2.00 5.00
Average Slope (%) = 0.50 0.50
Length (m) = 932.74 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 96.49 24.67
over (min) = 15.00 45.00
Storage Coeff. (min) = 12.18 (ii) 30.91 (ii)
Unit Hyd. Tpeak (min) = 15.00 45.00
Unit Hyd. peak (cms) = 0.08 0.03

TOTALS
PEAK FLOW (cms) = 2.77 4.19
TIME TO PEAK (hrs) = 6.00 6.50
RUNOFF VOLUME (mm) = 71.10 22.80
TOTAL RAINFALL (mm) = 73.10 73.10
RUNOFF COEFFICIENT = 0.97 0.31 0.38

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9275)				
IN= 2---> OUT= 1				
DT= 15.0 min				
OUTFLOW	STORAGE	OUTFLOW	STORAGE	
(cms)	(ha. m.)	(cms)	(ha. m.)	
0.0000	0.0000	4.5000	3.5000	
0.8000	0.1000	6.0000	4.0000	
1.5000	2.0000	6.5000	4.5000	
2.5000	3.0000	7.0000	5.5000	
	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (2287)	130.500	4.864	6.50	27.63
OUTFLOW: ID= 1 (9275)	130.500	1.329	7.75	27.63

PEAK FLOW REDUCTION [Qout/Qin] (%) = 27.33
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha. m.) = 1.5374

RESERVOIR (5228)				
IN= 2---> OUT= 1				
DT= 15.0 min				
OUTFLOW	STORAGE	OUTFLOW	STORAGE	
(cms)	(ha. m.)	(cms)	(ha. m.)	
0.0000	0.0000	6.7260	2.6184	
0.3410	1.2286	8.4460	2.9937	
1.9480	1.4214	9.8320	3.3609	
3.6280	1.8348	10.2320	3.3709	
5.0330	2.1904	0.0000	0.0000	
	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (9275)	130.500	1.329	7.75	27.63
OUTFLOW: ID= 1 (5228)	130.500	1.208	9.75	27.62

PEAK FLOW REDUCTION [Qout/Qin] (%) = 90.87
 TIME SHIFT OF PEAK FLOW (min) = 120.00
 MAXIMUM STORAGE USED (ha. m.) = 1.3327

CALIB NASHYD (2282)			
ID= 1 DT=15.0 min			
Area	(ha) = 180.03	Curve Number	(CN) = 61.9
Ia	(mm) = 5.00	# of Linear Res.	(N) = 1.50
U. H.	U. H. Tp (hrs) = 1.12		

Unit Hyd Qpeak (cms) = 2.754
 PEAK FLOW (cms) = 1.571 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 20.484
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.280

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9228)				
1 + 2 = 3				
ID= 1 (2282):	180.03	1.571	7.50	20.48
+ ID= 2 (5228):	130.50	1.208	9.75	27.62
ID = 3 (9228):	310.53	2.492	9.25	23.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8170)				
1 + 2 = 3				
ID= 1 (6228):	17353.76	127.893	9.75	29.21
+ ID= 2 (9228):	310.53	2.492	9.25	23.48
ID = 3 (8170):	17664.29	130.347	9.75	29.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2302)		
ID= 1 DT=15.0 min		
Area	(ha) = 30.47	
Total Imp	(%) = 29.00	Dir. Conn. (%) = 11.00
	IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha) = 8.84	21.63
Dep. Storage	(mm) = 2.00	5.00
Average Slope	(%) = 0.50	0.50
Length	(m) = 450.70	40.00
Mannings n	= 0.013	0.250
Max. Eff. Inten.	(mm/hr) = 96.49	53.63
over	(min) = 15.00	30.00
Storage Coeff.	(min) = 7.87 (ii)	21.60 (ii)
Unit Hyd. Tpeak	(min) = 15.00	30.00
Unit Hyd. peak	(cms) = 0.10	0.05
PEAK FLOW	(cms) = 0.81	1.57
TIME TO PEAK	(hrs) = 6.00	6.25
RUNOFF VOLUME	(mm) = 71.10	27.98
TOTAL RAINFALL	(mm) = 73.10	73.10
RUNOFF COEFFICIENT	= 0.97	0.38

TOTALS
 1.802 (iii)
 6.00
 32.72
 73.10
 0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2307)		
ID= 1 DT=15.0 min		
Area	(ha) = 175.00	
Total Imp	(%) = 25.00	Dir. Conn. (%) = 9.00
	IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha) = 43.75	131.25
Dep. Storage	(mm) = 2.00	5.00
Average Slope	(%) = 0.50	0.50
Length	(m) = 1080.12	40.00
Mannings n	= 0.013	0.250
Max. Eff. Inten.	(mm/hr) = 96.49	64.44
over	(min) = 15.00	30.00
Storage Coeff.	(min) = 13.30 (ii)	26.06 (ii)
Unit Hyd. Tpeak	(min) = 15.00	30.00
Unit Hyd. peak	(cms) = 0.08	0.04
PEAK FLOW	(cms) = 3.23	10.52
TIME TO PEAK	(hrs) = 6.00	6.25
RUNOFF VOLUME	(mm) = 71.10	34.29
TOTAL RAINFALL	(mm) = 73.10	73.10
RUNOFF COEFFICIENT	= 0.97	0.47

TOTALS
 11.953 (iii)
 6.25
 37.60
 73.10
 0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9280)

IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	8.7500	3.0604
0.5900	1.4722	11.7100	3.8736
3.3900	1.6096	14.7000	4.6556
6.3200	2.4308	17.1100	5.4201

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (2307)	175.000	11.953	6.25	37.60
OUTFLOW: ID= 1 (9280)	175.000	6.275	6.75	37.60

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 52.50
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 2.4284

RESERVOIR (5230)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	8.6830	2.7782
0.4700	1.3193	10.7480	3.1570
2.5870	1.5374	12.3570	3.5304
5.1590	1.9534	12.7570	3.5404
6.7830	2.3333	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (9280)	175.000	6.275	6.75	37.60
OUTFLOW: ID= 1 (5230)	175.000	4.585	7.50	37.60

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 73.06
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 1.8626

ADD HYD (8172)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (2302):	30.47	1.802	6.00	32.72
+ ID2= 2 (5230):	175.00	4.585	7.50	37.60
ID = 3 (8172):	205.47	4.907	7.50	36.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (2329)
ID= 1 DT=15.0 min

Area (ha)	Curve Number (CN)
1.80	69.0
5.00	# of Linear Res. (N) = 1.50
U.H. Tp (hrs) = 0.22	

Unit Hyd Opeak (cms) = 0.143

PEAK FLOW (cms) = 0.066 (i)
TIME TO PEAK (hrs) = 6.000
RUNOFF VOLUME (mm) = 23.008
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.315

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9269)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.0900	0.0300
0.0100	0.0136	0.1200	0.0386
0.0400	0.0146	0.1500	0.0471
0.0700	0.0234	0.1800	0.0554

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (2329)	1.800	0.066	6.00	23.01
OUTFLOW: ID= 1 (9269)	1.800	0.042	6.75	22.83

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 63.22
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.0153

CALIB STANHYD (2327)
ID= 1 DT=15.0 min

Area Total (ha)	Imp (%)	Dir. Conn. (%)
184.00	24.00	8.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 44.16 139.84
Dep. Storage (mm) = 2.00 5.00
Average Slope (%) = 0.50 0.50
Length (m) = 1107.55 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 96.49 49.65
over (min) = 15.00 30.00
Storage Coeff. (min) = 13.50 (ii) 27.66 (ii)
Unit Hyd. Tpeak (min) = 15.00 30.00
Unit Hyd. peak (cms) = 0.08 0.04

PEAK FLOW (cms) = 3.00 8.31 *TOTALS* (iii)
TIME TO PEAK (hrs) = 6.00 6.25 9.662 (iii)
RUNOFF VOLUME (mm) = 71.10 26.87 6.25
TOTAL RAINFALL (mm) = 73.10 73.10 30.40
RUNOFF COEFFICIENT = 0.97 0.37 73.10 0.42

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 66.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9270)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.0000	4.0000
1.1000	0.3000	3.5000	4.5000
1.8000	1.8000	4.5000	6.0000
2.0000	2.5000	5.5000	8.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (2327)	184.000	9.662	6.25	30.40
OUTFLOW: ID= 1 (9270)	184.000	2.082	7.50	30.40

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 21.55
TIME SHIFT OF PEAK FLOW (min) = 75.00
MAXIMUM STORAGE USED (ha. m.) = 2.6271

ADD HYD (9249)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (9269):	1.80	0.042	6.75	22.83
+ ID2= 2 (9270):	184.00	2.082	7.50	30.40
ID = 3 (9249):	185.80	2.102	7.50	30.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5232)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	11.1130	4.3262
0.5630	2.0299	13.9540	4.9462
3.2190	2.3484	16.2450	5.5530
5.9950	3.0315	16.6450	5.5630
8.3150	3.6189	0.0000	0.0000

INFLOW : ID= 2 (9249) 185.800 2.102 7.50 30.33
 OUTFLOW: ID= 1 (5232) 185.800 1.835 10.25 30.33

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 87.29
 TIME SHIFT OF PEAK FLOW (min) = 165.00
 MAXIMUM STORAGE USED (ha. m.) = 2.1830

 CALIB (2322) | Area (ha) = 129.00 Curve Number (CN) = 58.1
 NASHYD (2322) | Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs) = 1.00

Unit Hyd Qpeak (cms) = 2.209

PEAK FLOW (cms) = 1.090 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 18.267
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.250

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (9232) | AREA OPEAK TPEAK R.V.
 1 + 2 = 3 (ha) (cms) (hrs) (mm)
 ID1= 1 (2322): 129.00 1.090 7.25 18.27
 + ID2= 2 (5232): 185.80 1.835 10.25 30.33

 ID = 3 (9232): 314.80 2.602 9.75 25.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8173) | AREA OPEAK TPEAK R.V.
 1 + 2 = 3 (ha) (cms) (hrs) (mm)
 ID1= 1 (8172): 205.47 4.907 7.50 36.87
 + ID2= 2 (9232): 314.80 2.602 9.75 25.38

 ID = 3 (8173): 520.27 6.335 7.50 29.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8174) | AREA OPEAK TPEAK R.V.
 1 + 2 = 3 (ha) (cms) (hrs) (mm)
 ID1= 1 (8170): 17664.29 130.347 9.75 29.11
 + ID2= 2 (8173): 520.27 6.335 7.50 29.92

 ID = 3 (8174): 18184.56 134.108 9.75 29.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6234) | Routing time step (min) = 15.00
 IN= 2--> OUT= 1

<----- DATA FOR SECTION (2341.0) ----->

Distance	Elevation	Manning
0.00	228.00	0.0550
42.86	223.21	0.0550
85.72	219.56	0.0550
107.15	219.42	0.0550
128.58	219.39	0.0550
214.30	219.30	0.0550
225.02	219.26	0.0550
235.73	219.24	0.0550
255.16	219.20	0.0550 / 0.0350
257.16	217.20	0.0350
303.16	217.20	0.0350
305.16	219.20	0.0350 / 0.0550

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	217.70	887E+05	7.2	0.31	206.54
1.00	218.20	179E+06	22.7	0.48	131.74
1.50	218.70	272E+06	44.5	0.62	101.75
2.00	219.20	366E+06	71.8	0.75	84.97
2.59	219.79	993E+06	136.1	0.52	121.59
3.17	220.37	189E+07	258.6	0.52	121.80
3.76	220.96	294E+07	434.7	0.56	112.82
4.35	221.55	415E+07	665.9	0.61	103.88
4.93	222.13	551E+07	955.0	0.66	96.22
5.52	222.72	703E+07	1305.1	0.71	89.80
6.11	223.31	868E+07	1744.9	0.77	82.91
6.69	223.89	104E+08	2261.4	0.83	76.46
7.28	224.48	121E+08	2838.0	0.89	71.12
7.87	225.07	139E+08	3472.9	0.95	66.63
8.45	225.65	157E+08	4165.0	1.01	62.82
9.04	226.24	176E+08	4913.5	1.07	59.54
9.63	226.83	194E+08	5717.7	1.12	56.68
10.21	227.41	214E+08	6577.1	1.17	54.18
10.80	228.00	235E+08	6919.5	1.12	56.61

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	217.70	887E+05	7.2	0.31	206.54
1.00	218.20	179E+06	22.7	0.48	131.74
1.50	218.70	272E+06	44.5	0.62	101.75
2.00	219.20	366E+06	71.8	0.75	84.97
2.59	219.79	993E+06	136.1	0.52	121.59
3.17	220.37	189E+07	258.6	0.52	121.80
3.76	220.96	294E+07	434.7	0.56	112.82
4.35	221.55	415E+07	665.9	0.61	103.88
4.93	222.13	551E+07	955.0	0.66	96.22
5.52	222.72	703E+07	1305.1	0.71	89.80
6.11	223.31	868E+07	1744.9	0.77	82.91
6.69	223.89	104E+08	2261.4	0.83	76.46
7.28	224.48	121E+08	2838.0	0.89	71.12
7.87	225.07	139E+08	3472.9	0.95	66.63
8.45	225.65	157E+08	4165.0	1.01	62.82
9.04	226.24	176E+08	4913.5	1.07	59.54
9.63	226.83	194E+08	5717.7	1.12	56.68
10.21	227.41	214E+08	6577.1	1.17	54.18
10.80	228.00	235E+08	6919.5	1.12	56.61

<---- hydrograph ----> <- pi pe / channel ->

INFLOW : ID= 2 (8174)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW: ID= 1 (6234)	*****	134.11	9.75	29.13	2.57	0.53
	*****	114.77	12.00	29.13	2.39	0.58

 CALIB (2347) | Area (ha) = 266.10 Curve Number (CN) = 56.0
 NASHYD (2347) | Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs) = 2.00

Unit Hyd Qpeak (cms) = 2.276

PEAK FLOW (cms) = 1.242 (i)
 TIME TO PEAK (hrs) = 9.250
 RUNOFF VOLUME (mm) = 17.263
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.236

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR (9281) | IN= 2--> OUT= 1
 DT= 15.0 min |

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	13.3100	4.0635
0.9000	1.7152	17.8000	5.3537
5.1600	1.7685	22.3500	6.6446
9.6100	3.0976	26.0300	7.9060

INFLOW : ID= 2 (2347)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
OUTFLOW: ID= 1 (9281)	266.100	1.242	9.25	17.26
	266.100	0.828	13.75	17.26

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 66.63
 TIME SHIFT OF PEAK FLOW (min) = 270.00
 MAXIMUM STORAGE USED (ha. m.) = 1.5781

 RESERVOIR (5284) | IN= 2--> OUT= 1
 DT= 15.0 min |

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.5630	0.1745
0.1300	0.0819	0.6550	0.1995

0.2420	0.0947	0.7760	0.2240
0.3350	0.1223	1.0550	0.2340
0.4480	0.1460	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9281)	266.100	0.828	13.75	17.26
OUTFLOW: ID= 1 (5234)	266.100	0.828	14.00	17.26

PEAK FLOW REDUCTION [Qout/Qin](%) = 99.99
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha.m.) = 0.2259

CALIB STANDHYD (2342)
 ID= 1 DT=15.0 min
 Area (ha) = 1.06
 Total Imp(%) = 22.00
 Dir. Conn.(%) = 8.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	0.23	0.83
Dep. Storage	1.00	5.00
Average Slope	1.00	2.00
Length	84.06	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	20.55
over (min)	15.00	30.00
Storage Coeff. (min)=	2.34 (ii)	15.63 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.11	0.05

	PEAK FLOW (cms)	REDUCTION (%)	TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
	0.02	0.03	6.00	72.10	73.10	0.99
	0.039 (iii)	6.25	11.85	73.10	73.10	0.16
	0.03	6.00	16.66	73.10	73.10	0.23

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 39.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9234)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2342):	1.06	0.039	6.00	16.66
+ ID2= 2 (5234):	266.10	0.828	14.00	17.26

ID = 3 (9234):	267.16	0.828	14.00	17.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8176)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6234):	18184.56	114.765	12.00	29.13
+ ID2= 2 (9234):	267.16	0.828	14.00	17.26

ID = 3 (8176):	18451.72	115.500	12.00	28.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2369)
 ID= 1 DT=15.0 min
 Area (ha) = 441.90
 Total Imp(%) = 23.00
 Dir. Conn.(%) = 8.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	101.64	340.26
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50

Length (m)=	1716.39	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	96.49	52.94
over (min)	15.00	45.00
Storage Coeff. (min)=	17.56 (ii)	31.36 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.07	0.03

PEAK FLOW (cms)=	6.43	18.56	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.50	20.782 (iii)
RUNOFF VOLUME (mm)=	71.10	28.88	6.50
TOTAL RAINFALL (mm)=	73.10	73.10	32.26
RUNOFF COEFFICIENT =	0.97	0.40	73.10
			0.44

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9266)
 IN= 2---> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	2.3000	8.0000
	0.2000	2.0000	3.5000	10.0000
	1.9000	7.0000	4.5000	13.0000
	2.0000	7.5000	5.5000	14.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2369)	441.900	20.782	6.50	32.26
OUTFLOW: ID= 1 (9266)	441.900	3.151	8.75	32.26

PEAK FLOW REDUCTION [Qout/Qin](%) = 15.16
 TIME SHIFT OF PEAK FLOW (min) = 135.00
 MAXIMUM STORAGE USED (ha.m.) = 9.4271

RESERVOIR (5236)
 IN= 2---> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	3.1380	1.5897
	1.0220	0.7542	3.7950	2.0204
	1.7530	0.8200	4.4970	2.4373
	2.3290	1.2567	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9266)	441.900	3.151	8.75	32.26
OUTFLOW: ID= 1 (5236)	441.900	2.829	10.75	32.26

PEAK FLOW REDUCTION [Qout/Qin](%) = 89.77
 TIME SHIFT OF PEAK FLOW (min) = 120.00
 MAXIMUM STORAGE USED (ha.m.) = 1.4630

CALIB NASHYD (2362)
 ID= 1 DT=15.0 min
 Area (ha) = 52.59
 Curve Number (CN) = 58.7
 U. H. Tp(hrs) = 1.24
 # of Linear Res. (N) = 1.50

Unit Hyd Opeak (cms)=	0.726
PEAK FLOW (cms)=	0.386 (i)
TIME TO PEAK (hrs)=	8.000
RUNOFF VOLUME (mm)=	18.650
TOTAL RAINFALL (mm)=	73.100
RUNOFF COEFFICIENT =	0.255

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2362):	52.59	0.386	8.00	18.65
+ ID2= 2 (5236):	441.90	2.829	10.75	32.26
ID = 3 (9236):	494.49	3.117	10.50	30.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6238)
IN= 2--> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (2381.0) ----->

Distance	Elevation	Manning	
0.00	241.54	0.0380	
602.55	227.00	0.0380	
1702.00	224.50	0.0380	
1721.25	223.00	0.0380	
1725.10	222.60	0.0380	
1780.94	222.50	0.0380	
1782.87	222.45	0.0380	
1784.79	222.40	0.0380 / 0.0300	Main Channel
1786.57	221.75	0.0300	Main Channel
1787.07	220.75	0.0300	Main Channel
1787.57	220.75	0.0300	Main Channel
1791.57	220.75	0.0300	Main Channel
1794.07	221.75	0.0300	Main Channel
1798.27	222.00	0.0300	Main Channel
1800.19	222.35	0.0300 / 0.0380	Main Channel
1802.12	222.47	0.0380	
1840.63	223.00	0.0380	
1900.00	225.00	0.0380	
1950.00	226.00	0.0380	
2242.61	240.00	0.0380	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.80	221.55	.101E+05	5.2	1.14	32.36
1.60	222.35	.296E+05	18.7	1.40	26.36
2.64	223.39	.249E+06	151.8	1.35	27.33
3.68	224.43	.613E+06	512.2	1.85	19.94
4.71	225.46	.152E+07	1066.8	1.55	23.80
5.75	226.50	.359E+07	2824.9	1.75	21.16
6.79	227.54	.663E+07	6528.0	2.18	16.93
7.83	228.58	.994E+07	12158.6	2.71	13.62
8.87	229.62	.134E+08	19268.8	3.19	11.58
9.91	230.66	.170E+08	27785.3	3.63	10.19
10.94	231.69	.207E+08	37667.1	4.03	9.18
11.98	232.73	.246E+08	48891.7	4.40	8.40
13.02	233.77	.287E+08	61448.0	4.75	7.78
14.06	234.81	.329E+08	75332.7	5.08	7.28
15.10	235.85	.372E+08	90547.8	5.39	6.85
16.14	236.89	.417E+08	*****	5.69	6.49
17.17	237.92	.464E+08	*****	5.98	6.18
18.21	238.96	.512E+08	*****	6.25	5.91
19.25	240.00	.561E+08	*****	6.52	5.67

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX. DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (9236)	494.49	3.12	10.50	30.81	0.48	1.14
OUTFLOW: ID= 1 (6238)	494.49	3.07	11.00	30.81	0.47	1.14

CALIB STANDHYD (2389)
ID= 1 DT=15.0 mi n
Area (ha)= 4.70
Total Imp(%)= 21.00
Di r. Conn.(%)= 7.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	0.99	3.71
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	177.01	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)=	96.49	51.62
over (mi n)	15.00	30.00
Storage Coeff. (mi n)=	4.49 (ii)	18.43 (ii)

Unit Hyd. Tpeak (mi n)= 15.00 30.00
Unit Hyd. peak (cms)= 0.11 0.05

TOTALS
0.293 (iii)
6.25
31.57
73.10
0.43

PEAK FLOW (cms)= 0.09 0.28
TIME TO PEAK (hrs)= 6.00 6.25
RUNOFF VOLUME (mm)= 71.10 28.59
TOTAL RAINFALL (mm)= 73.10 73.10
RUNOFF COEFFICIENT = 0.97 0.39

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9267)
IN= 2--> OUT= 1
DT= 15.0 mi n

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	0.2300	0.0785
	0.0200	0.0364	0.3100	0.1005
	0.0900	0.0393	0.3900	0.1220
	0.1700	0.0616	0.4600	0.1429

INFLOW : ID= 2 (2389) 4.700 0.293 6.25 31.57
OUTFLOW: ID= 1 (9267) 4.700 0.149 6.75 31.47

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 50.87
TIME SHIFT OF PEAK FLOW (mi n) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 0.0570

CALIB NASHYD (2387)
ID= 1 DT=15.0 mi n

Area (ha)= 90.20 Curve Number (CN)= 54.0
Ia (mm)= 5.00 # of Li near Res. (N)= 1.50
U. H. Tp(hrs)= 0.86

Unit Hyd Opeak (cms)= 1.791

PEAK FLOW (cms)= 0.752 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 16.095
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.220

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9268)
IN= 2--> OUT= 1
DT= 15.0 mi n

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	4.5100	1.3564
	0.3100	0.5624	6.0400	1.7959
	1.7500	0.5748	7.5800	2.2374
	3.2600	1.0285	8.8300	2.6688

INFLOW : ID= 2 (2387) 90.200 0.752 7.00 16.09
OUTFLOW: ID= 1 (9268) 90.200 0.601 9.25 16.09

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 79.97
TIME SHIFT OF PEAK FLOW (mi n)=135.00
MAXIMUM STORAGE USED (ha. m.) = 0.5676

ADD HYD (9247)
1 + 2 = 3

AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
ID1= 1 (9267): 4.70 0.149 6.75 31.47
+ ID2= 2 (9268): 90.20 0.601 9.25 16.09

 ID = 3 (9247): 94.90 0.624 9.25 16.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5238)
 IN= 2 ---> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.8780	1.5095
0.1960	0.7083	4.8600	1.7258
1.1230	0.8194	5.6680	1.9375
2.0920	1.0578	6.0680	1.9475
2.9010	1.2627	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9247)	94.900	0.624	9.25	16.85
OUTFLOW: ID= 1 (5238)	94.900	0.243	15.25	16.84

PEAK FLOW REDUCTION [Qout/Qin] (%) = 38.93
 TIME SHIFT OF PEAK FLOW (mi n) = 360.00
 MAXIMUM STORAGE USED (ha. m.) = 0.7140

CALIB NASHYD (2382)
 ID= 1 DT=15.0 mi n

Area (ha) = 216.80 Curve Number (CN) = 56.8
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.56

Unit Hyd Opeak (cms) = 2.368

PEAK FLOW (cms) = 1.255 (i)
 TIME TO PEAK (hrs) = 8.250
 RUNOFF VOLUME (mm) = 17.656
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.242

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9238)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2382):	216.80	1.255	8.25	17.66
+ ID2= 2 (5238):	94.90	0.243	15.25	16.84
ID = 3 (9238):	311.70	1.310	8.50	17.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8180)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6238):	494.49	3.072	11.00	30.81
+ ID2= 2 (9238):	311.70	1.310	8.50	17.41
ID = 3 (8180):	806.19	4.270	10.75	25.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8178)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8176):	18451.72	115.500	12.00	28.96
+ ID2= 2 (8180):	806.19	4.270	10.75	25.63
ID = 3 (8178):	19257.91	119.582	12.00	28.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6240)
 IN= 2 ---> OUT= 1

Routing time step (mi n)' = 15.00

<----- DATA FOR SECTION (2401.0) ----->

Distance	Elevation	Manning
0.00	222.00	0.0450
11.46	221.00	0.0450
208.98	221.00	0.0450
404.04	220.97	0.0450
808.08	220.83	0.0450
905.60	220.17	0.0450
919.53	219.43	0.0450
933.47	219.22	0.0450
945.26	219.21	0.0450 / 0.0300
946.26	217.81	0.0300
975.26	217.81	0.0300
1003.26	217.81	0.0300
1005.26	219.21	0.0300 / 0.0450
1017.06	219.28	0.0450
1030.99	219.26	0.0450
1044.92	219.23	0.0450
1058.86	219.23	0.0450
1253.91	219.22	0.0450
1323.57	221.05	0.0450
1379.30	222.00	0.0450

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.20	218.01	307E+05	2.2	0.20	227.52
0.40	218.21	617E+05	7.1	0.31	143.95
0.60	218.41	928E+05	14.0	0.41	110.33
0.80	218.61	124E+06	22.6	0.49	91.46
1.00	218.81	156E+06	32.8	0.57	79.15
1.20	219.01	188E+06	44.5	0.64	70.38
1.40	219.21	220E+06	57.5	0.70	63.77
1.63	219.44	413E+06	82.5	0.54	83.52
1.87	219.68	632E+06	120.9	0.51	87.03
2.10	219.91	858E+06	170.0	0.53	84.11
2.33	220.14	109E+07	228.7	0.56	79.62
2.56	220.37	134E+07	294.4	0.59	76.03
2.80	220.61	162E+07	371.1	0.61	72.75
3.03	220.84	192E+07	456.8	0.64	70.18
3.26	221.07	251E+07	552.1	0.59	75.74
3.49	221.30	333E+07	711.8	0.57	78.07
3.73	221.54	417E+07	907.0	0.58	76.62
3.96	221.77	502E+07	1134.1	0.61	73.70
4.19	222.00	587E+07	1390.9	0.64	70.34

<---- hydrograph ----> <- pipe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8178)	*****	119.58	12.00	28.82	1.86	0.51
OUTFLOW: ID= 1 (6240)	*****	111.00	13.50	28.82	1.80	0.52

CALIB NASHYD (0240)
 ID= 1 DT=15.0 mi n

Area (ha) = 345.47 Curve Number (CN) = 46.0
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 3.60

Unit Hyd Opeak (cms) = 1.637

PEAK FLOW (cms) = 0.749 (i)
 TIME TO PEAK (hrs) = 12.000
 RUNOFF VOLUME (mm) = 12.642
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.173

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2407)
 ID= 1 DT=15.0 mi n

Area (ha) = 88.90
 Total Imp(%) = 20.00 Dir. Conn.(%) = 7.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 17.78 71.12
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 769.85 40.00

Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 96.49 44.95
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 10.86 (ii) 25.59 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.08 0.04

TOTALS
 PEAK FLOW (cms) = 1.37 3.98 4.495 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.25
 RUNOFF VOLUME (mm) = 71.10 25.39 28.59
 TOTAL RAINFALL (mm) = 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.35 0.39

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9273)				
IN= 2--> OUT= 1				
DT= 15.0 min				
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)	
0.0000	0.0000	4.4500	5.4723	
0.3000	4.6746	5.9500	5.8930	
1.7300	4.7225	7.4700	6.3047	
3.2100	5.1511	8.7000	6.7071	

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2407)	88.900	4.495	6.25	28.59
OUTFLOW: ID= 1 (9273)	88.900	0.143	12.25	28.54

PEAK FLOW REDUCTION [Qout/Qin] (%) = 3.19
 TIME SHIFT OF PEAK FLOW (min) = 360.00
 MAXIMUM STORAGE USED (ha.m.) = 2.2355

ADD HYD (9252)				
1 + 2 = 3				
ID	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0240):	345.47	0.749	12.00	12.64
+ ID2= 2 (9273):	88.90	0.143	12.25	28.54
=====				
ID = 3 (9252):	434.37	0.892	12.00	15.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8182)				
1 + 2 = 3				
ID	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6240):	19257.91	110.995	13.50	28.82
+ ID2= 2 (9252):	434.37	0.892	12.00	15.90
=====				
ID = 3 (8182):	19692.28	111.844	13.50	28.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (2427)
 ID= 1 DT=15.0 min
 Area (ha) = 5.40 Curve Number (CN) = 74.0
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp (hrs) = 0.37

Unit Hyd Opeak (cms) = 0.249
 PEAK FLOW (cms) = 0.157 (i)
 TIME TO PEAK (hrs) = 6.250
 RUNOFF VOLUME (mm) = 28.175
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.385

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9274)				
IN= 2--> OUT= 1				
DT= 15.0 min				
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)	
0.0000	0.0000	0.2700	0.0857	
0.0200	0.0379	0.3600	0.1115	
0.1000	0.0399	0.4500	0.1369	
0.1900	0.0663	0.5300	0.1618	

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2427)	5.400	0.157	6.25	28.18
OUTFLOW: ID= 1 (9274)	5.400	0.122	7.00	28.09

PEAK FLOW REDUCTION [Qout/Qin] (%) = 77.44
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0463

RESERVOIR (5242)				
IN= 2--> OUT= 1				
DT= 15.0 min				
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)	
0.0000	0.0000	0.0480	1.0185	
0.0020	1.0087	0.0600	1.0212	
0.0140	1.0101	0.0700	1.0238	
0.0260	1.0130	0.4700	1.0338	
0.0360	1.0155	0.0000	0.0000	

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9274)	5.400	0.122	7.00	28.09
OUTFLOW: ID= 1 (5242)	5.400	0.000	35.00	4.29

PEAK FLOW REDUCTION [Qout/Qin] (%) = 0.24
 TIME SHIFT OF PEAK FLOW (min) = *****
 MAXIMUM STORAGE USED (ha.m.) = 0.1490

CALIB NASHYD (2422)
 ID= 1 DT=15.0 min
 Area (ha) = 652.48 Curve Number (CN) = 51.6
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp (hrs) = 5.37

Unit Hyd Opeak (cms) = 2.073
 PEAK FLOW (cms) = 1.196 (i)
 TIME TO PEAK (hrs) = 12.750
 RUNOFF VOLUME (mm) = 15.126
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.207

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9242)				
1 + 2 = 3				
ID	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2422):	652.48	1.196	12.75	15.13
+ ID2= 2 (5242):	5.40	0.000	35.00	4.29
=====				
ID = 3 (9242):	657.88	1.197	12.75	15.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8184)				
1 + 2 = 3				
ID	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8182):	19692.28	111.844	13.50	28.54
+ ID2= 2 (9242):	657.88	1.197	12.75	15.04
=====				
ID = 3 (8184):	20350.16	113.037	13.50	28.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6244)
IN= 2--> OUT= 1

Routing time step (min) = 15.00

DATA FOR SECTION (2441.0) ----->

Distance	Elevation	Manning		
0.00	225.00	0.0450		
2.50	220.10	0.0450		
3.00	220.06	0.0450		
3.64	220.07	0.0450		
7.28	220.08	0.0450		
10.91	220.09	0.0450		
14.55	219.81	0.0450		
18.19	219.29	0.0450		
24.13	219.21	0.0450 / 0.0300	Main Channel	
24.63	217.81	0.0300	Main Channel	
69.13	217.81	0.0300	Main Channel	
114.13	217.81	0.0300	Main Channel	
115.13	219.21	0.0300 / 0.0450	Main Channel	
120.06	219.25	0.0450		
123.70	219.32	0.0450		
127.34	219.77	0.0450		
130.98	219.88	0.0450		
134.61	219.99	0.0450		
350.00	220.18	0.0450		
360.18	225.00	0.0450		

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.35	218.16	.108E+06	9.0	0.29	200.52
0.70	218.51	.216E+06	28.4	0.45	126.89
1.05	218.86	.325E+06	55.7	0.59	97.27
1.40	219.21	.434E+06	89.7	0.71	80.65
1.79	219.60	.574E+06	135.9	0.81	70.36
2.17	219.98	.726E+06	190.8	0.90	63.38
2.56	220.37	.111E+07	264.3	0.82	69.82
2.94	220.75	.157E+07	371.7	0.81	70.36
3.33	221.14	.203E+07	503.4	0.85	67.30
3.72	221.53	.250E+07	656.6	0.90	63.40
4.10	221.91	.296E+07	829.6	0.96	59.54
4.49	222.30	.343E+07	1021.1	1.02	56.00
4.87	222.68	.390E+07	1230.2	1.08	52.83
5.26	223.07	.437E+07	1456.2	1.14	50.01
5.65	223.46	.484E+07	1698.4	1.20	47.51
6.03	223.84	.531E+07	1956.3	1.26	45.28
6.42	224.23	.579E+07	2229.3	1.32	43.28
6.80	224.61	.626E+07	2517.0	1.38	41.48
7.19	225.00	.674E+07	2819.2	1.44	39.85

hydrograph ----->

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8184) *****	113.04	13.50	28.10	1.59	0.76
OUTFLOW: ID= 1 (6244) *****	108.54	14.50	28.10	1.56	0.75

CALIB STANDHYD (2441)
ID= 1 DT=15.0 min

Area (ha) = 91.52
Total Imp(%) = 45.00 Di r. Conn. (%) = 15.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	41.18	50.34
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	781.11	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) =	96.49	65.46
over (min)	15.00	30.00
Storage Coeff. (min) =	10.95 (ii)	23.62 (ii)
Unit Hyd. Tpeak (min) =	15.00	30.00
Unit Hyd. peak (cms) =	0.08	0.04

PEAK FLOW (cms) =	3.02	4.29	*TOTALS* 5.729 (iii)
TIME TO PEAK (hrs) =	6.00	6.25	6.00
RUNOFF VOLUME (mm) =	71.10	27.83	34.32
TOTAL RAINFALL (mm) =	73.10	73.10	73.10

RUNOFF COEFFICIENT = 0.97 0.38 0.47

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 60.9 Ia = Dep Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2449)
ID= 1 DT=15.0 min

Area (ha) = 143.20
Total Imp(%) = 34.00 Di r. Conn. (%) = 17.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	48.69	94.51
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	977.07	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) =	96.49	57.70
over (min)	15.00	30.00
Storage Coeff. (min) =	12.53 (ii)	25.86 (ii)
Unit Hyd. Tpeak (min) =	15.00	30.00
Unit Hyd. peak (cms) =	0.08	0.04

TOTALS

PEAK FLOW (cms) =	5.11	6.78	9.385 (iii)
TIME TO PEAK (hrs) =	6.00	6.25	6.00
RUNOFF VOLUME (mm) =	71.10	29.89	36.89
TOTAL RAINFALL (mm) =	73.10	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.41	0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9265)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	7.1600	2.7857
0.4900	1.4537	9.5800	3.4259
2.7800	1.6401	12.0300	4.0179
5.1700	2.2743	14.0100	4.5968

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2449)	143.200	9.385	6.00	36.89
OUTFLOW: ID= 1 (9265)	143.200	4.706	6.75	36.89

PEAK FLOW REDUCTION [Qout/Qin] (%) = 50.15
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 2.1696

ADD HYD (9245)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2441):	91.52	5.729	6.00	34.32
+ ID2= 2 (9265):	143.20	4.706	6.75	36.89
ID= 3 (9245):	234.72	8.656	6.25	35.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5244)
IN= 2--> OUT= 1

DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	15.7000	11.1120
	0.7950	7.8678	19.7140	11.9879
	4.5470	8.3178	22.9500	12.8451
	8.4690	9.2829	23.3500	12.8551
	11.7470	10.1128	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (9245)	234.720	8.656	6.25	35.89
OUTFLOW: ID= 1 (5244)	234.720	0.588	12.50	35.88

PEAK FLOW REDUCTION [Qout/Qin](%) = 6.79
 TIME SHIFT OF PEAK FLOW (min) = 375.00
 MAXIMUM STORAGE USED (ha.m.) = 5.8170

CALIB NASHYD (2442)	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 DT=15.0 min	1674.00	8.741	9.500	20.72
	U.H. Tp (hrs)			
		2.20		

Unit Hyd Qpeak (cms) = 12.991
 PEAK FLOW (cms) = 8.741 (i)
 TIME TO PEAK (hrs) = 9.500
 RUNOFF VOLUME (mm) = 20.720
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.283

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9244)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (2442)	1674.00	8.741	9.50	20.72
+ ID2= 2 (5244)	234.72	0.588	12.50	35.88
ID = 3 (9244)	1908.72	9.301	9.75	22.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8186)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (6244)	20350.16	108.544	14.50	28.10
+ ID2= 2 (9244)	1908.72	9.301	9.75	22.58
ID = 3 (8186)	22258.88	114.902	14.00	27.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ HYD (8190)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
DT=15.0 min				
		0.00		
		0.00		

File name: S:\2010 Projects\D-EE (E10)\NR\W10-487 East Gwillimbury SWM Master Plan\Calcs\Hydrologic Model\s\Ex Comments:

HOWEVER THE TABLE BELOW AND HYDROGRAPH FILES DO.

TIME FLOW	ACTUAL FLOW	PEAK FLOW	PEAK (0.00) + BASE (*****)	TIME FLOW	TIME FLOW	TIME FLOW	TIME FLOW
hrs cms	hrs cms	hrs cms	hrs cms	hrs cms	hrs cms	hrs cms	hrs cms
0.00	*****	64.00	*****	128.00	*****	192.00	*****
0.25	*****	64.25	*****	128.25	*****	192.25	*****
0.50	*****	64.50	*****	128.50	*****	192.50	*****
0.75	*****	64.75	*****	128.75	*****	192.75	*****
1.00	*****	65.00	*****	129.00	*****	193.00	*****
1.25	*****	65.25	*****	129.25	*****	193.25	*****
1.50	*****	65.50	*****	129.50	*****	193.50	*****
1.75	*****	65.75	*****	129.75	*****	193.75	*****
2.00	*****	66.00	*****	130.00	*****	194.00	*****
2.25	*****	66.25	*****	130.25	*****	194.25	*****
2.50	*****	66.50	*****	130.50	*****	194.50	*****
2.75	*****	66.75	*****	130.75	*****	194.75	*****

3.00	*****	67.00	*****	131.00	*****	195.00	*****	259.00	*****
3.25	*****	67.25	*****	131.25	*****	195.25	*****	259.25	*****
3.50	*****	67.50	*****	131.50	*****	195.50	*****	259.50	*****
3.75	*****	67.75	*****	131.75	*****	195.75	*****	259.75	*****
4.00	*****	68.00	*****	132.00	*****	196.00	*****	260.00	*****
4.25	*****	68.25	*****	132.25	*****	196.25	*****	260.25	*****
4.50	*****	68.50	*****	132.50	*****	196.50	*****	260.50	*****
4.75	*****	68.75	*****	132.75	*****	196.75	*****	260.75	*****
5.00	*****	69.00	*****	133.00	*****	197.00	*****	261.00	*****
5.25	*****	69.25	*****	133.25	*****	197.25	*****	261.25	*****
5.50	*****	69.50	*****	133.50	*****	197.50	*****	261.50	*****
5.75	*****	69.75	*****	133.75	*****	197.75	*****	261.75	*****
6.00	*****	70.00	*****	134.00	*****	198.00	*****	262.00	*****
6.25	*****	70.25	*****	134.25	*****	198.25	*****	262.25	*****
6.50	*****	70.50	*****	134.50	*****	198.50	*****	262.50	*****
6.75	*****	70.75	*****	134.75	*****	198.75	*****	262.75	*****
7.00	*****	71.00	*****	135.00	*****	199.00	*****	263.00	*****
7.25	*****	71.25	*****	135.25	*****	199.25	*****	263.25	*****
7.50	*****	71.50	*****	135.50	*****	199.50	*****	263.50	*****
7.75	*****	71.75	*****	135.75	*****	199.75	*****	263.75	*****
8.00	*****	72.00	*****	136.00	*****	200.00	*****	264.00	*****
8.25	*****	72.25	*****	136.25	*****	200.25	*****	264.25	*****
8.50	*****	72.50	*****	136.50	*****	200.50	*****	264.50	*****
8.75	*****	72.75	*****	136.75	*****	200.75	*****	264.75	*****
9.00	*****	73.00	*****	137.00	*****	201.00	*****	265.00	*****
9.25	*****	73.25	*****	137.25	*****	201.25	*****	265.25	*****
9.50	*****	73.50	*****	137.50	*****	201.50	*****	265.50	*****
9.75	*****	73.75	*****	137.75	*****	201.75	*****	265.75	*****
10.00	*****	74.00	*****	138.00	*****	202.00	*****	266.00	*****
10.25	*****	74.25	*****	138.25	*****	202.25	*****	266.25	*****
10.50	*****	74.50	*****	138.50	*****	202.50	*****	266.50	*****
10.75	*****	74.75	*****	138.75	*****	202.75	*****	266.75	*****
11.00	*****	75.00	*****	139.00	*****	203.00	*****	267.00	*****
11.25	*****	75.25	*****	139.25	*****	203.25	*****	267.25	*****
11.50	*****	75.50	*****	139.50	*****	203.50	*****	267.50	*****
11.75	*****	75.75	*****	139.75	*****	203.75	*****	267.75	*****
12.00	*****	76.00	*****	140.00	*****	204.00	*****	268.00	*****
12.25	*****	76.25	*****	140.25	*****	204.25	*****	268.25	*****
12.50	*****	76.50	*****	140.50	*****	204.50	*****	268.50	*****
12.75	*****	76.75	*****	140.75	*****	204.75	*****	268.75	*****
13.00	*****	77.00	*****	141.00	*****	205.00	*****	269.00	*****
13.25	*****	77.25	*****	141.25	*****	205.25	*****	269.25	*****
13.50	*****	77.50	*****	141.50	*****	205.50	*****	269.50	*****
13.75	*****	77.75	*****	141.75	*****	205.75	*****	269.75	*****
14.00	*****	78.00	*****	142.00	*****	206.00	*****	270.00	*****
14.25	*****	78.25	*****	142.25	*****	206.25	*****	270.25	*****
14.50	*****	78.50	*****	142.50	*****	206.50	*****	270.50	*****
14.75	*****	78.75	*****	142.75	*****	206.75	*****	270.75	*****
15.00	*****	79.00	*****	143.00	*****	207.00	*****	271.00	*****
15.25	*****	79.25	*****	143.25	*****	207.25	*****	271.25	*****
15.50	*****	79.50	*****	143.50	*****	207.50	*****	271.50	*****
15.75	*****	79.75	*****	143.75	*****	207.75	*****	271.75	*****
16.00	*****	80.00	*****	144.00	*****	208.00	*****	272.00	*****
16.25	*****	80.25	*****	144.25	*****	208.25	*****	272.25	*****
16.50	*****	80.50	*****	144.50	*****	208.50	*****	272.50	*****
16.75	*****	80.75	*****	144.75	*****	208.75	*****	272.75	*****
17.00	*****	81.00	*****	145.00	*****	209.00	*****	273.00	*****
17.25	*****	81.25	*****	145.25	*****	209.25	*****	273.25	*****
17.50	*****	81.50	*****	145.50	*****	209.50	*****	273.50	*****
17.75	*****	81.75	*****	145.75	*****	209.75	*****	273.75	*****
18.00	*****	82.00	*****	146.00	*****	210.00	*****	274.00	*****
18.25	*****	82.25	*****	146.25	*****	210.25	*****	274.25	*****
18.50	*****	82.50	*****	146.50	*****	210.50	*****	274.50	*****
18.75	*****	82.75	*****	146.75	*****	210.75	*****	274.75	*****
19.00	*****	83.00	*****	147.00	*****	211.00	*****	275.00	*****
19.25	*****	83.25	*****	147.25	*****	211.25	*****	275.25	*****
19.50	*****	83.50	*****	147.50	*****	211.50	*****	275.50	*****
19.75	*****	83.75	*****	147.75	*****	211.75	*****	275.75	*****
20.00	*****	84.00	*****	148.00	*****	212.00	*****	276.00	*****
20.25	*****	84.25	*****	148.25	*****	212.25	*****	276.25	*****
20.50	*****	84.50	*****	148.50	*****	212.50	*****	276.50	*****
20.75	*****	84.75	*****	148.75	*****	212.75	*****	276.75	*****
21.00	*****	85.00	*****	149.00	*****	213.00	*****	277.00	*****
21.25	*****	85.25	*****	149.25	*****	213.25	*****	277.25	*****
21.50	*****	85.50	*****	149.50	*****	213.50	*****	277.50	*****
21.75	*****	85.75	*****	149.75	*****	213.75	*****	277.75	*****
22.00	*****	86.00	*****	150.00	*****	214.00	*****	278.00	*****
22.25	*****	86.25	*****	150.25	*****	214.25	*****	278.25	*****
22.50	*****	86.50	*****	150.50	*****	214.50	*****	278.50	*****
22.75	*****	86.75	*****	150.75	*****	214.75	*****	278.75	*****
23.00	*****	87.00	*****	151.00	*****	215.00	*****	279.00	*****
23.25	*****	87.25	*****	151.25	*****	215.25	*****	279.25	*****
23.50	*****	87.50	*****	151.50	*****	215.50	*****	279.50	*****
23.75	*****	87.75	*****	151.75	*****	215.75	*****	279.75	*****

 ID = 3 (8188): 42609.04 223.362 14.50 27.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB NASHYD (4462) Area (ha)=1238.89 Curve Number (CN)= 60.9
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 4.02

Unit Hyd Qpeak (cms)= 5.256
 PEAK FLOW (cms)= 3.894 (i)
 TIME TO PEAK (hrs)= 12.000
 RUNOFF VOLUME (mm)= 20.035
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.274

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB STANDHYD (4461) Area (ha)= 236.12 Dir. Conn. (%)= 17.00
 ID= 1 DT=15.0 min Total Imp(%)= 47.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	110.98	125.14
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1254.64	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	66.94
over (min)	15.00	30.00
Storage Coeff. (min)=	14.55 (ii)	27.11 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.07	0.04

TOTALS
 PEAK FLOW (cms)= 7.94 10.17 14.338 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 71.10 28.08 35.39
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.38 0.48

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 60.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR (5446) IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	10.1800	6.2621
0.8000	2.9957	12.5050	7.1218
3.0350	3.4813	14.4090	7.9633
5.6510	4.4353	14.8090	7.9733
7.7470	5.2683	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (4461)	236.120	14.338	6.00	35.39
OUTFLOW: ID= 1 (5446)	236.120	5.138	7.00	35.39

PEAK FLOW REDUCTION [Oout/Oin] (%) = 35.83
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 4.2593

 ADD HYD (8192)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
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ID1= 1 (4462): 1238.89 3.894 12.00 20.04
 + ID2= 2 (5446): 236.12 5.138 7.00 35.39

 ID = 3 (8192): 1475.01 7.364 7.25 22.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8194)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8188):	42609.04	223.362	14.50	27.85
+ ID2= 2 (8192):	1475.01	7.364	7.25	22.49

ID = 3 (8194):	44084.05	227.586	14.50	27.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB NASHYD (0442) Area (ha)= 117.26 Curve Number (CN)= 43.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.17

Unit Hyd Qpeak (cms)= 1.705
 PEAK FLOW (cms)= 0.538 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 11.364
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.155

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB NASHYD (0440) Area (ha)= 226.35 Curve Number (CN)= 60.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.11

Unit Hyd Qpeak (cms)= 3.481
 PEAK FLOW (cms)= 1.870 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 19.362
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.265

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB NASHYD (0438) Area (ha)= 130.70 Curve Number (CN)= 51.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.60

Unit Hyd Qpeak (cms)= 3.738
 PEAK FLOW (cms)= 1.306 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 14.532
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.199

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB NASHYD (0436) Area (ha)= 187.51 Curve Number (CN)= 55.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.73

Unit Hyd Qpeak (cms)= 4.391
 PEAK FLOW (cms)= 1.830 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 16.534
 TOTAL RAINFALL (mm)= 73.100

RUNOFF COEFFICIENT = 0.226

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha)= Ia (mm)= U. H. Tp(hrs)=	56.64 5.00 0.52	Curve Number (CN)= 46.0 # of Linear Res. (N)= 1.50
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Unit Hyd Qpeak (cms)= 1.867

PEAK FLOW (cms)= 0.537 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 12.320
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.169

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8206) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0434):	56.64	0.537	6.50	12.32
+ ID2= 2 (0436):	187.51	1.830	7.00	16.53
ID = 3 (8206):	244.15	2.351	6.75	15.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6438) IN= 2---> OUT= 1	Routing time step (min)' = 15.00
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<----- DATA FOR SECTION (4381.0) ----->

Distance	Elevation	Manning	
0.00	256.00	0.0600	
9.23	255.50	0.0600	
27.69	255.00	0.0600	
50.77	254.90	0.0600	
64.62	254.85	0.0600	
78.47	252.26	0.0600	
96.93	249.44	0.0600	
129.24	245.94	0.0600	
145.40	245.76	0.0600	
146.20	245.80	0.0600 / 0.0350	Main Channel
146.70	245.30	0.0350	Main Channel
147.70	245.30	0.0350	Main Channel
148.20	245.80	0.0350 / 0.0600	Main Channel
150.01	245.78	0.0600	
152.32	245.79	0.0600	
163.86	246.72	0.0600	
175.40	249.07	0.0600	
186.94	253.15	0.0600	
196.17	255.38	0.0600	
228.48	255.55	0.0600	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	TRAVEL TIME (hrs)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	245.80	221E+04	0.9	0.9	1.02	40.44
1.01	246.31	352E+05	14.0	0.99	41.77	41.77
1.53	246.83	841E+05	47.3	1.39	29.61	29.61
2.04	247.34	144E+06	102.0	1.76	23.49	23.49
2.55	247.85	213E+06	177.3	2.07	19.99	19.99
3.07	248.37	291E+06	274.4	2.34	17.66	17.66
3.58	248.88	378E+06	394.6	2.58	15.97	15.97
4.09	249.39	474E+06	541.4	2.83	14.60	14.60
4.61	249.91	578E+06	719.9	3.08	13.38	13.38
5.12	250.42	688E+06	923.8	3.33	12.40	12.40
5.63	250.93	803E+06	1153.3	3.56	11.61	11.61
6.14	251.44	925E+06	1408.6	3.77	10.95	10.95
6.66	251.96	105E+07	1690.6	3.98	10.38	10.38
7.17	252.47	119E+07	2002.3	4.18	9.88	9.88
7.68	252.98	133E+07	2345.4	4.38	9.43	9.43
8.20	253.50	147E+07	2710.1	4.56	9.05	9.05
8.71	254.01	162E+07	3101.0	4.73	8.72	8.72
9.22	254.52	178E+07	3521.8	4.90	8.43	8.43
9.74	255.04	195E+07	3467.6	4.39	9.39	9.39

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8206)	244.15	2.35	6.75	15.56	0.56	1.02
OUTFLOW: ID= 1 (6438)	244.15	2.06	7.75	15.56	0.54	1.02

ADD HYD (8208) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0438):	130.70	1.306	6.75	14.53
+ ID2= 2 (6438):	244.15	2.063	7.75	15.56
ID = 3 (8208):	374.85	3.183	7.25	15.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8210) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0440):	226.35	1.870	7.50	19.36
+ ID2= 2 (8208):	374.85	3.183	7.25	15.20
ID = 3 (8210):	601.20	5.052	7.50	16.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha)= Ia (mm)= U. H. Tp(hrs)=	114.21 5.00 1.21	Curve Number (CN)= 53.0 # of Linear Res. (N)= 1.50
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Unit Hyd Qpeak (cms)= 1.617

PEAK FLOW (cms)= 0.714 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 15.687
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.215

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha)= Ia (mm)= U. H. Tp(hrs)=	111.64 5.00 0.52	Curve Number (CN)= 55.0 # of Linear Res. (N)= 1.50
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Unit Hyd Qpeak (cms)= 3.687

PEAK FLOW (cms)= 1.425 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 16.354
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.224

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha)= Ia (mm)= U. H. Tp(hrs)=	50.53 5.00 0.50	Curve Number (CN)= 51.0 # of Linear Res. (N)= 1.50
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Unit Hyd Qpeak (cms)= 1.725

PEAK FLOW (cms)= 0.582 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 14.436
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.197

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0426) Area (ha) = 247.17 Curve Number (CN) = 61.0
 ID= 1 DT=15.0 min I a (mm) = 5.00 # of Li near Res. (N) = 1.50
 U. H. Tp(hrs) = 0.98

Unit Hyd Qpeak (cms) = 4.315

PEAK FLOW (cms) = 2.322 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 19.908
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.272

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0424) Area (ha) = 49.57 Curve Number (CN) = 59.0
 ID= 1 DT=15.0 min I a (mm) = 5.00 # of Li near Res. (N) = 1.50
 U. H. Tp(hrs) = 0.37

Unit Hyd Qpeak (cms) = 2.293

PEAK FLOW (cms) = 0.900 (i)
 TIME TO PEAK (hrs) = 6.500
 RUNOFF VOLUME (mm) = 18.121
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.248

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	(8198)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0424):	49.57	0.900	6.50	18.12	
+ ID2= 2 (0426):	247.17	2.322	7.25	19.91	
ID = 3 (8198):	296.74	3.036	6.75	19.61	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6428) Routing time step (min)' = 15.00
 IN= 2--> OUT= 1

Distance	Elevation	Manning	
0.00	246.54	0.0900	
8.80	246.29	0.0900	
30.78	246.03	0.0900	
41.78	243.97	0.0900	
63.77	237.16	0.0900	
85.76	232.53	0.0900	
96.75	230.16	0.0900	
109.94	228.40	0.0900	
112.14	228.28	0.0900 / 0.0450	Main Channel
112.84	228.00	0.0450	Main Channel
113.34	227.50	0.0450	Main Channel
114.34	227.50	0.0450	Main Channel
114.84	228.00	0.0450	Main Channel
116.54	228.29	0.0450 / 0.0900	Main Channel
118.74	228.40	0.0900	
120.94	228.52	0.0900	
149.52	229.51	0.0900	
169.31	230.69	0.0900	
186.00	234.27	0.0900	
217.69	243.56	0.0900	

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(mi n)
0.78	228.28	205E+04	1.6	0.97	21.60
1.58	229.08	202E+05	16.6	1.02	20.37
2.39	229.89	665E+05	64.8	1.22	17.11
3.19	230.69	133E+06	158.1	1.49	14.05
4.00	231.50	213E+06	308.6	1.82	11.48

4.80	232.30	299E+06	504.9	2.11	9.89
5.61	233.11	394E+06	746.6	2.37	8.79
6.41	233.91	496E+06	1034.5	2.61	7.99
7.21	234.71	605E+06	1373.3	2.84	7.35
8.02	235.52	722E+06	1763.3	3.06	6.82
8.82	236.32	844E+06	2202.0	3.27	6.39
9.63	237.13	974E+06	2690.5	3.46	6.03
10.43	237.93	111E+07	3239.5	3.66	5.71
11.23	238.73	125E+07	3839.0	3.84	5.43
12.04	239.54	140E+07	4489.1	4.03	5.18
12.84	240.34	155E+07	5190.5	4.20	4.97
13.65	241.15	170E+07	5943.8	4.36	4.78
14.45	241.95	187E+07	6749.8	4.53	4.61
15.26	242.76	204E+07	7609.4	4.68	4.46

	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW: ID= 2 (8198)	296.74	3.04	6.75	19.61	0.86	0.97
OUTFLOW: ID= 1 (6428)	296.74	2.92	7.25	19.61	0.85	0.97

ADD HYD	(8200)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0428):	50.53	0.582	6.50	14.44	
+ ID2= 2 (6428):	296.74	2.916	7.25	19.61	
ID = 3 (8200):	347.27	3.410	7.25	18.86	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8202)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0430):	111.64	1.425	6.50	16.35	
+ ID2= 2 (8200):	347.27	3.410	7.25	18.86	
ID = 3 (8202):	458.91	4.703	7.00	18.25	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6432) Routing time step (min)' = 15.00
 IN= 2--> OUT= 1

Distance	Elevation	Manning	
0.00	221.00	0.0600	
15.29	219.85	0.0600	
45.86	219.86	0.0600	
76.44	219.96	0.0600	
110.84	220.23	0.0600	
112.00	220.15	0.0600	
114.66	220.09	0.0600	
118.48	219.73	0.0600	
120.80	219.70	0.0600 / 0.0350	Main Channel
121.30	219.20	0.0350	Main Channel
122.30	219.20	0.0350	Main Channel
122.80	219.70	0.0350 / 0.0600	Main Channel
129.95	220.11	0.0600	
175.81	220.24	0.0600	
214.03	220.55	0.0600	
252.25	220.61	0.0600	
290.47	220.70	0.0600	
328.69	221.04	0.0600	
347.80	221.08	0.0600	
378.37	221.04	0.0600	

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(mi n)
0.08	219.28	185E+03	0.0	0.23	149.38
0.17	219.37	399E+03	0.1	0.34	100.62
0.25	219.45	641E+03	0.1	0.42	80.98
0.33	219.53	911E+03	0.2	0.49	69.79
0.42	219.62	121E+04	0.3	0.55	62.35

0.50	219.70	.154E+04	0.5	0.60	56.91
0.61	219.81	.267E+04	0.7	0.56	61.28
0.71	219.91	.901E+04	1.3	0.30	112.96
0.82	220.02	.249E+05	3.4	0.28	122.71
0.92	220.12	.452E+05	6.8	0.31	109.98
1.03	220.23	.747E+05	12.2	0.33	102.05
1.14	220.34	.112E+06	21.3	0.39	87.59
1.24	220.44	.153E+06	33.0	0.44	77.01
1.35	220.55	.196E+06	47.2	0.49	69.22
1.46	220.66	.248E+06	62.3	0.51	66.48
1.56	220.76	.310E+06	83.4	0.55	62.06
1.67	220.87	.376E+06	108.8	0.59	57.56
1.77	220.97	.444E+06	137.7	0.64	53.77
1.88	221.08	.517E+06	163.6	0.65	52.70

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8202)	458.91	4.70	7.00	18.25	0.86	0.29
OUTFLOW: ID= 1 (6432)	458.91	3.07	9.25	18.25	0.80	0.28

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0432):	114.21	0.714	8.00	15.69
+ ID2= 2 (6432):	458.91	3.071	9.25	18.25
ID = 3 (8204):	573.12	3.717	9.00	17.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8204):	573.12	3.717	9.00	17.74
+ ID2= 2 (8210):	601.20	5.052	7.50	16.77
ID = 3 (8212):	1174.32	8.390	8.00	17.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6442)
IN= 2--> OUT= 1 Routing time step (min)' = 15.00

Distance	Elevation	Manning	
0.00	221.00	0.0350	
26.73	220.80	0.0350	
53.47	220.60	0.0350	
66.83	220.40	0.0350	
80.20	220.20	0.0350	
347.52	220.00	0.0350	
354.21	219.59	0.0350	
360.89	219.50	0.0350	
367.57	219.08	0.0350	
368.76	219.00	0.0350 / 0.0300	Main Channel
369.26	218.50	0.0300	Main Channel
374.26	218.50	0.0300	Main Channel
374.76	219.00	0.0300 / 0.0350	Main Channel
380.94	220.33	0.0350	
387.62	219.62	0.0350	
394.31	219.23	0.0350	
454.45	219.07	0.0350	
514.60	219.09	0.0350	
660.00	219.19	0.0350	
661.63	221.00	0.0350	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.12	218.62	.100E+04	0.2	0.28	92.89
0.25	218.75	.205E+04	0.6	0.43	60.09
0.37	218.87	.315E+04	1.1	0.55	46.99
0.50	219.00	.430E+04	1.8	0.66	39.67
0.63	219.13	.141E+05	3.4	0.37	69.99

0.77	219.27	.652E+05	13.7	0.33	79.14
0.90	219.40	.124E+06	35.5	0.45	58.02
1.03	219.53	.183E+06	65.7	0.56	46.45
1.17	219.67	.245E+06	103.8	0.66	39.30
1.30	219.80	.307E+06	149.3	0.76	34.30
1.43	219.93	.371E+06	201.6	0.85	30.65
1.57	220.07	.440E+06	256.5	0.91	28.56
1.70	220.20	.541E+06	325.7	0.94	27.70
1.83	220.33	.669E+06	415.2	0.98	26.62
1.97	220.47	.787E+06	520.3	1.03	25.21
2.10	220.60	.913E+06	638.5	1.09	23.82
2.23	220.73	.104E+07	767.2	1.15	22.62
2.37	220.87	.117E+07	908.0	1.21	21.54
2.50	221.00	.131E+07	1060.7	1.27	20.57

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8212)	1174.32	8.39	8.00	17.24	0.70	0.35
OUTFLOW: ID= 1 (6442)	1174.32	7.28	9.50	17.24	0.68	0.35

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0442):	117.26	0.538	8.00	11.36
+ ID2= 2 (6442):	1174.32	7.285	9.50	17.24
ID = 3 (8214):	1291.58	7.748	9.50	16.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8194):	44084.05	227.586	14.50	27.67
+ ID2= 2 (8214):	1291.58	7.748	9.50	16.71
ID = 3 (8196):	45375.62	231.812	14.00	27.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHYD (0444) Area (ha)= 221.65 Curve Number (CN)= 56.0
ID= 1 DT=15.0 min I a (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 1.03

Unit Hyd Opeak (cms)= 3.656

PEAK FLOW (cms)= 1.705 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 17.158
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.235

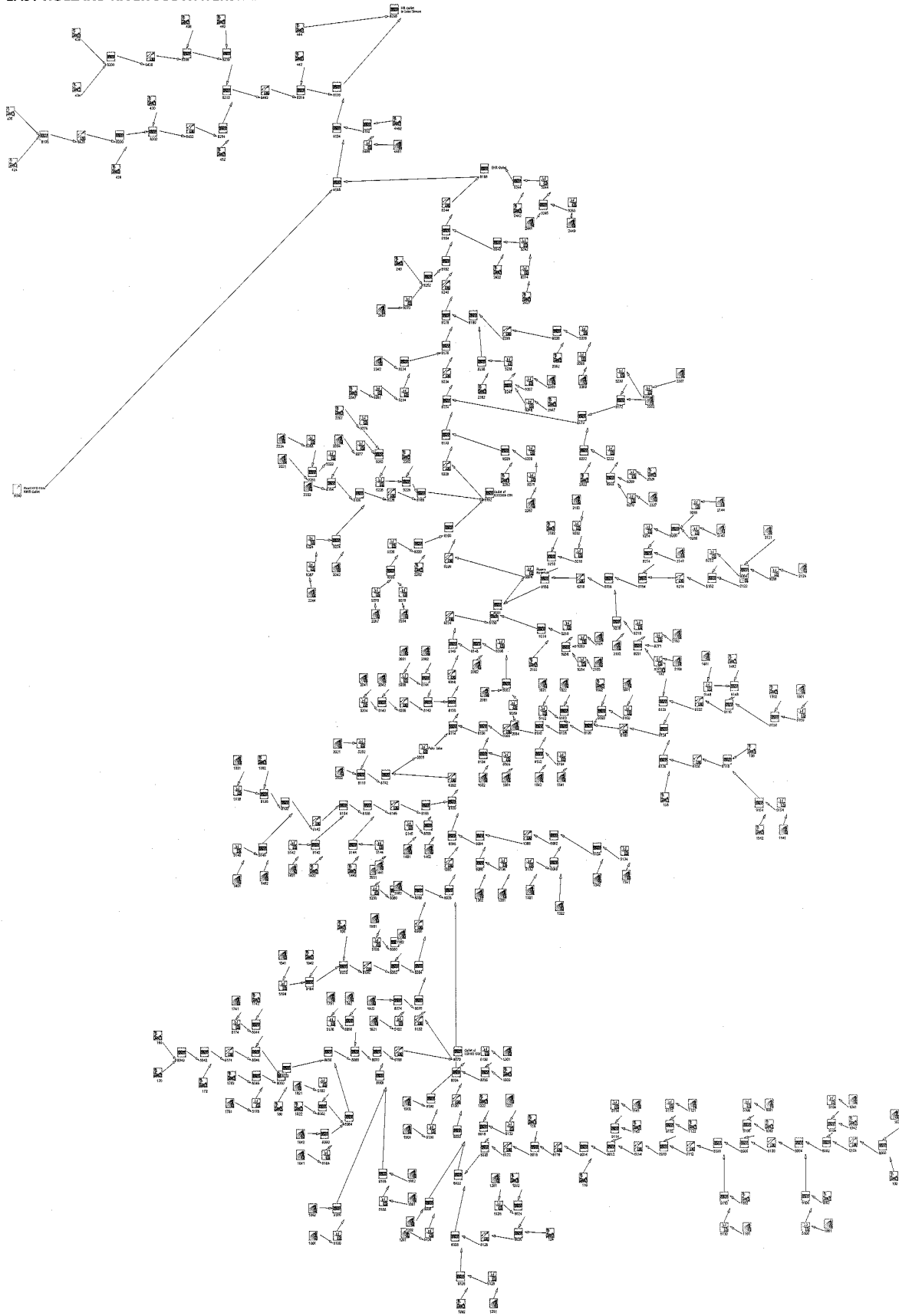
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0444):	221.65	1.705	7.50	17.16
+ ID2= 2 (8196):	45375.62	231.812	14.00	27.36
ID = 3 (8216):	45597.28	232.228	14.00	27.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

EAST HOLLAND RIVER SUBWATERSHED



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V V I SSSS U U A L
V V I SS U U A A L
V V I SS U U A A A A L
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***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\Visual Otthymo 2.4\VO2\voi.n.dat
 Output filename: C:\Users\amanluc\AppData\Local\Temp\b18f9d7e-4227-4676-9f98-8e2612e3c8a8\Scenario.out
 Summary filename: C:\Users\amanluc\AppData\Local\Temp\b18f9d7e-4227-4676-9f98-8e2612e3c8a8\Scenario.sum

DATE: 08/29/2012 TIME: 01:58:21
 USER:

COMMENTS: _____

 ** SIMULATION NUMBER: 1 **

READ STORM
 Ptotal = 80.80 mm
 Filename: C:\Users\amanluc\AppData\Local\Temp\b18f9d7e-4227-4676-9f98-8e2612e3c8a8\170567d
 Comments: 50-Year 12-Hour SCS II Design Storm

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	2.02	3.25	3.23	6.25	14.54
0.50	2.02	3.50	3.23	6.50	14.54
0.75	2.02	3.75	3.23	6.75	6.46
1.00	2.02	4.00	3.23	7.00	6.46
1.25	2.02	4.25	4.85	7.25	4.85
1.50	2.02	4.50	4.85	7.50	4.85
1.75	2.02	4.75	6.46	7.75	4.85
2.00	2.02	5.00	6.46	8.00	4.85
2.25	2.42	5.25	9.70	8.25	2.83
2.50	2.42	5.50	9.70	8.50	2.83
2.75	2.42	5.75	38.78	8.75	2.83
3.00	2.42	6.00	106.66	9.00	2.83

CALIB
 STANDHYD (1662)
 ID= 1 DT=15.0 min
 Area (ha)= 141.06
 Total Imp(%)= 70.00 Di r. Conn.(%)= 38.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 98.74 42.32
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 969.74 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 106.66 157.43
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 11.98 (ii) 20.90 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.05
 TOTALS
 PEAK FLOW (cms)= 12.63 9.33 18.880 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00

RUNOFF VOLUME (mm)= 78.80 49.52 60.64
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.61 0.75

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (1661)
 ID= 1 DT=15.0 min
 Area (ha)= 293.64
 Total Imp(%)= 42.00 Di r. Conn.(%)= 23.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 123.33 170.31
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1399.14 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 106.66 82.16
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 14.93 (ii) 26.50 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.07 0.04
 TOTALS
 PEAK FLOW (cms)= 14.60 17.32 25.737 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 78.80 40.05 48.97
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.50 0.61

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5166)
 IN= 2--> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	7.2300	6.6504
	0.4310	3.5677	9.1580	7.5825
	1.6680	3.8178	10.9790	8.5125
	3.5970	4.8509	11.3790	8.5225
	5.3070	5.7381	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (1661)	293.640	25.737	6.00	48.97
OUTFLOW: ID= 1 (5166)	293.640	8.741	7.00	48.96

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 33.96
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 7.4082

ADD HYD (8134)
 1 + 2 = 3
 ID1= 1 (1662): 141.06 18.880 6.00 60.64
 + ID2= 2 (5166): 293.64 8.741 7.00 48.96
 ID= 3 (8134): 434.70 19.285 6.00 52.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1642)
 ID= 1 DT=15.0 min
 Area (ha)= 120.35
 Total Imp(%)= 22.00 Di r. Conn.(%)= 17.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	26.48	93.87
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	895.73	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	106.66	56.71
over (min)=	15.00	30.00
Storage Coeff. (min)=	11.42 (ii)	24.84 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04

TOTALS
 PEAK FLOW (cms)= 4.90 6.76 9.183 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 78.80 34.56 42.08
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.43 0.52

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1641) ID= 1 DT=15.0 min	Area Total (ha)=	Imp(%)=	Dir. Conn.(%)=
	113.89	71.00	53.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	80.86	33.03
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	871.36	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	106.66	109.65
over (min)=	15.00	30.00
Storage Coeff. (min)=	11.24 (ii)	21.55 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.05

TOTALS
 PEAK FLOW (cms)= 14.55 4.97 17.813 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 78.80 43.84 62.37
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.54 0.77

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5164) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	8.4260	4.5513
	0.3860	1.9228	9.2330	4.9251
	3.3700	2.2339	9.9990	5.2982
	4.5710	3.3613	10.3990	5.3082
	5.7170	4.0568	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1641)	113.890	17.813	6.00	62.37
OUTFLOW: ID= 1 (5164)	113.890	4.904	6.75	62.37

PEAK FLOW REDUCTION [Qout/Qin] (%) = 27.53
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 3.6077

ADD HYD (8132) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1642):	120.35	9.183	6.00	42.08
+ ID2= 2 (5164):	113.89	4.904	6.75	62.36
=====				
ID = 3 (8132):	234.24	13.055	6.25	51.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0152) ID= 1 DT=15.0 min	Area (ha)=	Imp(%)=	Dir. Conn.(%)=	Curve Number (CN)=	# of Linear Res. (N)=
	124.37	5.00		59.0	1.50

Unit Hyd Opeak (cms)= 2.372

PEAK FLOW (cms)=	1.418 (i)
TIME TO PEAK (hrs)=	7.000
RUNOFF VOLUME (mm)=	22.498
TOTAL RAINFALL (mm)=	80.800
RUNOFF COEFFICIENT =	0.278

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1481) ID= 1 DT=15.0 min	Area Total (ha)=	Imp(%)=	Dir. Conn.(%)=
	61.75	47.00	17.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	29.02	32.73
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	641.61	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	106.66	13.68
over (min)=	15.00	45.00
Storage Coeff. (min)=	9.35 (ii)	33.05 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.09	0.03

TOTALS
 PEAK FLOW (cms)= 2.68 0.74 2.978 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 78.80 9.95 21.65
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.12 0.27

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 23.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5148) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	4.1310	1.6343
	0.2090	0.7802	5.1870	1.8598
	1.1960	0.9062	6.0380	2.0805
	2.2280	1.1562	6.4380	2.0905
	3.0910	1.3741	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1481)	61.750	2.978	6.00	21.65
OUTFLOW: ID= 1 (5148)	61.750	0.491	7.25	21.64

PEAK FLOW REDUCTION [Qout/Qin] (%) = 16.49
 TIME SHIFT OF PEAK FLOW (min) = 75.00

MAXIMUM STORAGE USED (ha. m.) = 0.8175

CALIB NASHYD (1482)
ID= 1 DT=15.0 min
Area (ha) = 356.14
Ia (mm) = 5.00
U. H. Tp(hrs) = 1.12
Curve Number (CN) = 31.3
of Linear Res. (N) = 1.50

Unit Hyd Opeak (cms) = 5.438
PEAK FLOW (cms) = 1.333 (i)
TIME TO PEAK (hrs) = 7.750
RUNOFF VOLUME (mm) = 8.994
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.111

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9148)
1 + 2 = 3
ID1= 1 (1482): 356.14 1.333 7.75 8.99
+ ID2= 2 (5148): 61.75 0.491 7.25 21.64

ID = 3 (9148): 417.89 1.813 7.50 10.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1501)
ID= 1 DT=15.0 min
Area Total Imp(%) = 45.00
Dir. Conn. (%) = 15.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 0.07 0.09
Dep. Storage (mm) = 2.00 5.00
Average Slope (%) = 0.50 0.50
Length (m) = 32.66 40.00
Mannings n = 0.013 0.250
Max. Eff. Inten. (mm/hr) = 106.66 8.82
over (min) = 15.00 30.00
Storage Coeff. (min) = 1.57 (ii) 29.82 (ii)
Unit Hyd. Tpeak (min) = 15.00 30.00
Unit Hyd. peak (cms) = 0.11 0.04

TOTALS
0.008 (iii)
17.29
80.80
0.21

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 16.4 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5150)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.0110	0.0042
0.0010	0.0020	0.0140	0.0049
0.0030	0.0023	0.0160	0.0054
0.0060	0.0030	0.0160	0.0154
0.0080	0.0036	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
0.160	0.008	6.00	17.29
0.160	0.001	7.25	14.30

INFLOW : ID= 2 (1501)
OUTFLOW: ID= 1 (5150)

PEAK FLOW REDUCTION [Qout/Qin] (%) = 10.20
TIME SHIFT OF PEAK FLOW (min) = 75.00
MAXIMUM STORAGE USED (ha. m.) = 0.0016

CALIB NASHYD (1502)
ID= 1 DT=15.0 min
Area (ha) = 105.72
Ia (mm) = 5.00
U. H. Tp(hrs) = 0.58
Curve Number (CN) = 17.2
of Linear Res. (N) = 1.50

Unit Hyd Opeak (cms) = 3.134
PEAK FLOW (cms) = 0.314 (i)
TIME TO PEAK (hrs) = 6.750
RUNOFF VOLUME (mm) = 4.323
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.053

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9150)
1 + 2 = 3
ID1= 1 (1502): 105.72 0.314 6.75 4.32
+ ID2= 2 (5150): 0.16 0.001 7.25 14.30

ID = 3 (9150): 105.88 0.315 6.75 4.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8116)
1 + 2 = 3
ID1= 1 (9148): 417.89 1.813 7.50 10.86
+ ID2= 2 (9150): 105.88 0.315 6.75 4.34

ID = 3 (8116): 523.77 2.095 7.25 9.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6152)
IN= 2---> OUT= 1
Routing time step (min) = 15.00

<----- DATA FOR SECTION (1521.0) ----->

Distance	Elevation	Manning
0.00	290.00	0.0500
15.34	286.87	0.0500
46.03	284.84	0.0500
88.22	283.07	0.0500
126.57	282.11	0.0500
153.42	280.34	0.0500
157.26	280.07	0.0500
161.09	279.72	0.0500
167.76	278.94	0.0500 / 0.0300
168.01	278.60	0.0300
168.76	278.60	0.0300
169.51	278.60	0.0300
169.76	278.90	0.0300 / 0.0500
173.76	279.40	0.0500
180.27	279.69	0.0500
184.11	279.93	0.0500
218.62	281.86	0.0500
260.82	283.20	0.0500
360.54	289.25	0.0500
379.72	289.80	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.30	278.90	.114E+04	0.4	0.80	45.70
0.87	279.47	.932E+04	4.2	0.98	37.22
1.45	280.05	.353E+05	17.5	1.09	33.68
2.02	280.62	.831E+05	47.8	1.26	29.01
2.59	281.19	.155E+06	102.9	1.46	25.06
3.17	281.77	.250E+06	189.3	1.66	22.03

3.74	282.34	.375E+06	298.5	1.75	20.91
4.32	282.92	.547E+06	456.5	1.83	19.98
4.89	283.49	.767E+06	705.7	2.02	18.12
5.46	284.06	.102E+07	1038.0	2.24	16.34
6.04	284.64	.130E+07	1444.1	2.45	14.98
6.61	285.21	.161E+07	1940.1	2.66	13.79
7.18	285.78	.194E+07	2522.6	2.86	12.80
7.76	286.36	.229E+07	3187.4	3.06	11.98
8.33	286.93	.267E+07	3942.3	3.25	11.28
8.91	287.51	.306E+07	4824.9	3.46	10.58
9.48	288.08	.347E+07	5795.7	3.67	9.99
10.05	288.65	.390E+07	6855.4	3.86	9.48
10.63	289.23	.434E+07	8005.0	4.05	9.04

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8116)	523.77	2.09	7.25	9.54	0.56	0.87
OUTFLOW: ID= 1 (6152)	523.77	1.89	8.25	9.54	0.52	0.86

ADD HYD (8122)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0152):	124.37	1.418	7.00	22.50
+ ID2= 2 (6152):	523.77	1.886	8.25	9.54
ID = 3 (8122):	648.14	3.206	8.00	12.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0158)	Area (ha)	Ia (mm)	U. H. Tp (hrs)	Curve Number (CN)	# of Linear Res. (N)
ID= 1 DT=15.0 min	178.59	5.00	0.94	61.0	1.50

Uni t Hyd Qpeak (cms)	= 3.230
PEAK FLOW (cms)	= 2.076 (i)
TIME TO PEAK (hrs)	= 7.250
RUNOFF VOLUME (mm)	= 23.854
TOTAL RAINFALL (mm)	= 80.800
RUNOFF COEFFICIENT	= 0.295

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0156)	Area (ha)	Ia (mm)	U. H. Tp (hrs)	Curve Number (CN)	# of Linear Res. (N)
ID= 1 DT=15.0 min	83.49	5.00	0.62	45.0	1.50

Uni t Hyd Qpeak (cms)	= 2.306
PEAK FLOW (cms)	= 0.812 (i)
TIME TO PEAK (hrs)	= 6.750
RUNOFF VOLUME (mm)	= 14.566
TOTAL RAINFALL (mm)	= 80.800
RUNOFF COEFFICIENT	= 0.180

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1541)	Area Total	(ha)	Imp (%)	Di r. Conn. (%)
ID= 1 DT=15.0 min	11.15	75.00		70.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	8.36	2.79
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	272.64	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	106.66	11.46
over (min)	15.00	45.00
Storage Coeff. (min)	5.60 (ii)	31.04 (ii)

Uni t Hyd. Tpeak (min)	= 15.00	45.00
Uni t Hyd. peak (cms)	= 0.11	0.03

TOTALS
2.231 (iii)
6.00
58.41
80.80
0.72

PEAK FLOW (cms)	= 2.21	0.05	2.231 (iii)
TIME TO PEAK (hrs)	= 6.00	6.50	6.00
RUNOFF VOLUME (mm)	= 78.80	10.85	58.41
TOTAL RAINFALL (mm)	= 80.80	80.80	80.80
RUNOFF COEFFICIENT	= 0.98	0.13	0.72

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 31.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5154)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2--> OUT= 1				
DT= 15.0 min				
	0.0000	0.0000	0.7460	0.3544
	0.0380	0.1987	0.9370	0.3839
	0.2160	0.2386	1.0900	0.4129
	0.4020	0.2749	1.4900	0.4229
	0.5580	0.3133	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1541)	11.150	2.231	6.00	58.41
OUTFLOW: ID= 1 (5154)	11.150	0.724	6.25	58.30

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 32.44
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha. m.) = 0.3603

CALIB NASHYD (1542)	Area (ha)	Ia (mm)	U. H. Tp (hrs)	Curve Number (CN)	# of Linear Res. (N)
ID= 1 DT=15.0 min	189.17	5.00	0.95	39.1	1.50

Uni t Hyd Qpeak (cms)	= 3.396
PEAK FLOW (cms)	= 1.079 (i)
TIME TO PEAK (hrs)	= 7.250
RUNOFF VOLUME (mm)	= 12.054
TOTAL RAINFALL (mm)	= 80.800
RUNOFF COEFFICIENT	= 0.149

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9154)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (1542):	189.17	1.079	7.25	12.05
+ ID2= 2 (5154):	11.15	0.724	6.25	58.30
ID = 3 (9154):	200.32	1.587	6.50	14.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8118)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0156):	83.49	0.812	6.75	14.57
+ ID2= 2 (9154):	200.32	1.587	6.50	14.63
ID = 3 (8118):	283.81	2.391	6.50	14.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ROUTE CHN (6158) |

| IN= 2---> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1581.0) ----->

Distance	Elevation	Manning	
0.00	290.00	0.0400	
19.20	288.88	0.0400	
48.00	288.26	0.0400	
119.99	283.68	0.0400	
167.99	282.07	0.0400	
196.79	281.32	0.0400	
210.79	280.90	0.0400	
219.79	280.38	0.0400 / 0.0300	Main Channel
220.04	279.65	0.0300	Main Channel
220.79	279.65	0.0300	Main Channel
221.54	279.65	0.0300	Main Channel
221.79	280.39	0.0300 / 0.0400	Main Channel
225.79	280.59	0.0400	
235.19	280.88	0.0400	
254.39	281.50	0.0400	
273.58	282.56	0.0400	
302.38	283.42	0.0400	
359.98	284.03	0.0400	
436.77	289.37	0.0400	
475.17	290.37	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.37	280.01	.973E+03	0.5	0.86	31.78
0.73	280.38	.210E+04	1.5	1.18	23.16
1.30	280.95	.149E+05	8.6	0.95	28.91
1.86	281.51	.579E+05	40.0	1.13	24.12
2.43	282.08	.133E+06	118.5	1.46	18.75
2.99	282.64	.236E+06	254.6	1.77	15.46
3.56	283.21	.369E+06	450.3	2.00	13.67
4.13	283.78	.542E+06	702.7	2.13	12.86
4.69	284.34	.767E+06	1125.2	2.41	11.36
5.26	284.91	.101E+07	1697.1	2.75	9.93
5.82	285.47	.127E+07	2379.1	3.07	8.91
6.39	286.04	.155E+07	3171.4	3.36	8.14
6.95	286.60	.184E+07	4075.5	3.64	7.53
7.52	287.17	.215E+07	5093.1	3.89	7.03
8.09	287.74	.247E+07	6226.5	4.13	6.62
8.65	288.30	.281E+07	7461.2	4.36	6.28
9.22	288.87	.318E+07	8635.1	4.46	6.13
9.78	289.43	.357E+07	10143.6	4.67	5.86
10.35	290.00	.398E+07	11608.8	4.79	5.71

<---- hydrograph ----> <- pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8118)	283.81	2.39	6.50	14.61	0.80	1.15
OUTFLOW: ID= 1 (6158)	283.81	2.21	7.25	14.61	0.79	1.15

ADD HYD (8120)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0158):	178.59	2.076	7.25	23.85
+ ID2= 2 (6158):	283.81	2.207	7.25	14.61
=====				
ID = 3 (8120):	462.40	4.283	7.25	18.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8124)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8120):	462.40	4.283	7.25	18.18
+ ID2= 2 (8122):	648.14	3.206	8.00	12.03
=====				
ID = 3 (8124):	1110.54	7.328	7.50	14.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ROUTE CHN (6160) |

| IN= 2---> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1601.0) ----->

Distance	Elevation	Manning	
0.00	261.02	0.0400	
29.89	254.30	0.0400	
40.75	252.58	0.0400	
62.49	251.30	0.0400	
114.11	250.94	0.0400	
130.41	248.58	0.0400	
141.28	247.50	0.0400	
154.86	246.87	0.0400	
155.20	246.70	0.0400	
160.20	246.30	0.0400 / 0.0300	Main Channel
160.30	245.30	0.0300	Main Channel
165.20	245.30	0.0300	Main Channel
165.30	246.30	0.0300 / 0.0400	Main Channel
168.45	246.96	0.0400	
195.62	249.20	0.0400	
203.77	250.82	0.0400	
225.50	256.78	0.0400	
244.52	261.14	0.0400	
255.39	261.95	0.0400	
268.97	261.92	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.50	245.80	120E+05	2.9	1.18	68.31
1.00	246.30	242E+05	8.5	1.70	47.47
1.92	247.22	825E+05	31.7	1.86	43.34
2.84	248.14	248E+06	102.5	2.00	40.39
3.76	249.06	506E+06	247.7	2.37	34.03
4.68	249.98	832E+06	486.8	2.83	28.49
5.60	250.90	121E+07	814.9	3.27	24.71
6.52	251.82	180E+07	1157.1	3.11	25.97
7.44	252.74	253E+07	1840.2	3.52	22.92
8.36	253.66	331E+07	2727.1	3.98	20.25
9.29	254.59	414E+07	3782.6	4.43	18.22
10.21	255.51	499E+07	5010.7	4.86	16.61
11.13	256.43	589E+07	6397.4	5.26	15.33
12.05	257.35	681E+07	7932.8	5.64	14.31
12.97	258.27	777E+07	9621.2	5.99	13.46
13.89	259.19	877E+07	11469.4	6.33	12.74
14.81	260.11	980E+07	13478.4	6.66	12.12
15.73	261.03	109E+08	15651.7	6.97	11.58
16.65	261.95	120E+08	17492.5	7.07	11.42

<---- hydrograph ----> <- pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8124)	1110.54	7.33	7.50	14.59	0.90	1.56
OUTFLOW: ID= 1 (6160)	1110.54	6.62	8.50	14.59	0.83	1.48

CALIB STANDHYD (1601)
ID= 1 DT=15.0 mi n

Area Total	(ha)= 19.03
Imp(%)	= 75.00
Dir. Conn.(%)	= 69.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)= 14.27	4.76
Dep. Storage	(mm)= 2.00	5.00
Average Slope	(%)= 0.50	0.50
Length	(m)= 356.18	40.00
Mannings n	= 0.013	0.250
Max. Eff. Inten.(mm/hr)=	106.66	45.81
over (mi n)	15.00	30.00
Storage Coeff. (mi n)=	6.57 (ii)	21.19 (ii)
Unit Hyd. Tpeak (mi n)=	15.00	30.00
Unit Hyd. peak (cms)=	0.10	0.05

TOTALS

PEAK FLOW	(cms)= 3.63	0.30	3.813 (iii)
TIME TO PEAK	(hrs)= 6.00	6.25	6.00
RUNOFF VOLUME	(mm)= 78.80	24.52	61.97
TOTAL RAI NFALL	(mm)= 80.80	80.80	80.80
RUNOFF COEFFICIENT	= 0.98	0.30	0.77

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 55.6 Ia = Dep. Storage (Above)

- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5160)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	1.2730	0.6034
0.0640	0.3377	1.5980	0.6540
0.3690	0.4055	1.8600	0.7037
0.6870	0.4676	2.2600	0.7137
0.9520	0.5332	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1601)	19.030	3.813	6.00	61.97
OUTFLOW: ID= 1 (5160)	19.030	1.294	6.25	61.90

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 33.94
TIME SHIFT OF PEAK FLOW (mi n) = 15.00
MAXIMUM STORAGE USED (ha. m.) = 0.6135

CALIB NASHYD (1602)
ID= 1 DT=15.0 mi n

Area (ha) = 299.79
U. H. Tp (hrs) = 1.42
Curve Number (CN) = 60.3
of Linear Res. (N) = 1.50

Unit Hyd Opeak (cms) = 3.592

PEAK FLOW (cms) = 2.502 (i)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 23.499
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.291

- (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8126)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1602):	299.79	2.502	8.00	23.50
+ ID2= 2 (5160):	19.03	1.294	6.25	61.90
-----	-----	-----	-----	-----
ID = 3 (9160):	318.82	3.100	6.75	25.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8126)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6160):	1110.54	6.624	8.50	14.59
+ ID2= 2 (9160):	318.82	3.100	6.75	25.79
-----	-----	-----	-----	-----
ID = 3 (8126):	1429.36	9.369	8.25	17.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1621)
ID= 1 DT=15.0 mi n

Area Total (ha) = 46.07
Imp (%) = 45.00
Di r. Conn. (%) = 20.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	20.73	25.34
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	554.20	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (mi n)	106.66	53.51
Storage Coeff. (mi n)	8.56 (ii)	22.30 (ii)

Unit Hyd. Tpeak (mi n) = 15.00
Unit Hyd. peak (cms) = 0.09

30.00
0.04

TOTALS
3.537 (iii)
6.00
35.34
80.80
0.44

PEAK FLOW (cms) = 2.41
TIME TO PEAK (hrs) = 6.00
RUNOFF VOLUME (mm) = 78.80
TOTAL RAINFALL (mm) = 80.80
RUNOFF COEFFICIENT = 0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 51.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5162)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.0820	1.1997
0.1560	0.5629	3.8700	1.3716
0.8930	0.6512	4.5050	1.5398
1.6620	0.8407	4.9050	1.5498
2.3060	1.0035	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1621)	46.070	3.537	6.00	35.34
OUTFLOW: ID= 1 (5162)	46.070	1.309	6.75	35.33

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 37.00
TIME SHIFT OF PEAK FLOW (mi n) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.7628

CALIB STANDHYD (1622)
ID= 1 DT=15.0 mi n

Area Total (ha) = 144.07
Imp (%) = 26.00
Di r. Conn. (%) = 11.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	37.46	106.61
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	980.03	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) over (mi n) = 106.66
Storage Coeff. (mi n) = 12.06 (ii)
Unit Hyd. Tpeak (mi n) = 15.00
Unit Hyd. peak (cms) = 0.08

PEAK FLOW (cms) = 3.73
TIME TO PEAK (hrs) = 6.00
RUNOFF VOLUME (mm) = 78.80
TOTAL RAINFALL (mm) = 80.80
RUNOFF COEFFICIENT = 0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 51.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9162)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1622):	144.07	5.449	6.00	27.42
+ ID2= 2 (5162):	46.07	1.309	6.75	35.33
-----	-----	-----	-----	-----

ID = 3 (9162): 190.14 6.326 6.50 29.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8126):	1429.36	9.369	8.25	17.09
+ ID2= 2 (9162):	190.14	6.326	6.50	29.34

ID = 3 (8128):	1619.50	11.529	6.50	18.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8128):	1619.50	11.529	6.50	18.53
+ ID2= 2 (8132):	234.24	13.055	6.25	51.94

ID = 3 (8130):	1853.74	22.677	6.25	22.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6166)
IN= 2---> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1661.0) ----->

Distance	Elevation	Manning	
0.00	248.36	0.0500	
41.67	246.98	0.0500	
95.82	244.93	0.0500	
149.98	242.42	0.0500	
191.64	241.88	0.0500	
224.97	239.90	0.0500	
229.13	239.35	0.0500	
233.30	237.77	0.0500	
237.47	236.57	0.0500	
240.63	235.60	0.0500 / 0.0300	Main Channel
241.63	234.10	0.0300	Main Channel
247.13	234.10	0.0300	Main Channel
247.53	235.60	0.0300 / 0.0500	Main Channel
254.13	236.25	0.0500	
266.63	236.77	0.0500	
283.29	237.84	0.0500	
291.63	238.89	0.0500	
329.12	244.66	0.0500	
370.78	249.55	0.0500	
412.44	253.17	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.75	234.85	.130E+05	6.0	1.36	36.45
1.50	235.60	.276E+05	18.1	1.95	25.41
2.25	236.35	.544E+05	40.0	2.18	22.69
3.00	237.10	.117E+06	80.1	2.04	24.32
3.75	237.85	.213E+06	148.9	2.07	23.89
4.50	238.60	.334E+06	255.1	2.27	21.85
5.25	239.35	.473E+06	395.8	2.49	19.90
6.00	240.10	.631E+06	563.8	2.65	18.66
6.75	240.85	.824E+06	773.2	2.79	17.77
7.50	241.60	.106E+07	1037.9	2.92	16.97
8.26	242.36	.135E+07	1326.7	2.92	16.94
9.01	243.11	.173E+07	1758.5	3.01	16.44
9.76	243.86	.217E+07	2285.9	3.13	15.81
10.51	244.61	.265E+07	2909.5	3.26	15.17
11.26	245.36	.318E+07	3610.1	3.37	14.67
12.01	246.11	.377E+07	4417.4	3.48	14.21
12.76	246.86	.441E+07	5348.0	3.60	13.75
13.51	247.61	.512E+07	6394.0	3.71	13.34
14.26	248.36	.589E+07	7580.3	3.82	12.95

<----- hydrograph -----> <- pipe / channel ->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)

INFLOW : ID= 2 (8130) 1853.74 22.68 6.25 22.75 1.66 1.99
 OUTFLOW: ID= 1 (6166) 1853.74 20.22 6.75 22.75 1.57 1.97

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6166):	1853.74	20.219	6.75	22.75
+ ID2= 2 (8134):	434.70	19.285	6.00	52.75

ID = 3 (8136):	2288.44	34.994	6.50	28.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2021)
 ID= 1 DT=15.0 min Area Total (ha)= 107.32
 Imp(%)= 45.00 Dir. Conn.(%)= 27.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	48.29	59.03
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	845.85	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	106.66	83.60
over (min)	15.00	30.00
Storage Coeff. (min)=	11.04 (ii)	22.53 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04
PEAK FLOW (cms)=	7.03	6.61
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	78.80	40.73
TOTAL RAINFALL (mm)=	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.50
TOTALS (iii)		11.313

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 73.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5202)
 IN= 2---> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000		5.1480	2.8069
0.3630	1.2717		6.4200	3.2412
1.5670	1.4796		7.4640	3.6910
2.7880	1.9296		7.8640	3.7010
3.8580	2.3404		0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2021)	107.320	11.313	6.00	51.01
OUTFLOW: ID= 1 (5202)	107.320	4.362	6.75	51.00

	PEAK FLOW REDUCTION [Out/Oi n] (%) =	38.56
TIME SHIFT OF PEAK FLOW (min) =	45.00	
MAXIMUM STORAGE USED (ha. m.) =	2.5421	

CALIB STANDHYD (2022)
 ID= 1 DT=15.0 min Area Total (ha)= 361.21
 Imp(%)= 60.00 Dir. Conn.(%)= 36.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	216.73	144.48
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1551.79	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 106.66 110.77
 over (min) = 15.00 30.00
 Storage Coeff. (min)= 15.88 (ii) 26.15 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.07 0.04

TOTALS
 PEAK FLOW (cms)= 27.37 20.06 40.472 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 78.80 44.82 57.05
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.55 0.71

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 73.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8110)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (2022):	361.21	40.472	6.00	57.05
+ ID2= 2 (5202):	107.32	4.362	6.75	51.00
ID = 3 (8110):	468.53	41.367	6.00	55.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (0102)	Area (ha)	Imp(%)	Dir. Conn. (%)
ID= 1 DT=15.0 min	466.00	23.00	9.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	107.18	358.82
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1762.57	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 106.66 20.93
 over (min) = 15.00 45.00
 Storage Coeff. (min)= 17.15 (ii) 37.14 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.07 0.03

TOTALS
 PEAK FLOW (cms)= 8.53 11.62 14.506 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.50
 RUNOFF VOLUME (mm)= 78.80 19.53 24.86
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.24 0.31

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 49.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0100)	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
ID= 1 DT=15.0 min	693.84	40.0	1.50
	Ia (mm)= 5.00		
	U. H. Tp (hrs)= 1.95		

Unit Hyd Opeak (cms)= 6.087

PEAK FLOW (cms)= 2.384 (i)
 TIME TO PEAK (hrs)= 9.250
 RUNOFF VOLUME (mm)= 12.530
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.155

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8000)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0100):	693.84	2.384	9.25	12.53
+ ID2= 2 (0102):	466.00	14.506	6.50	24.86
ID = 3 (8000):	1159.84	16.049	6.50	17.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6104)	Routing time step (min)
IN= 2--> OUT= 1	15.00

Distance	Elevation	Manning
0.00	330.16	0.0450
50.03	328.36	0.0450
89.34	326.33	0.0450
117.94	324.30	0.0450
160.82	322.75	0.0450
178.34	319.96	0.0450
183.34	319.81	0.0450
184.34	319.38	0.0450 / 0.0300
185.09	318.78	0.0300
185.84	318.72	0.0300
186.84	319.32	0.0300 / 0.0450
193.84	319.70	0.0450
198.84	320.38	0.0450
200.13	320.81	0.0450
218.00	322.49	0.0450
239.44	323.49	0.0450
250.17	323.96	0.0450
275.18	325.20	0.0450
310.92	326.47	0.0450
353.81	330.00	0.0450

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.30	319.02	.624E+03	0.1	0.45	73.00
0.60	319.32	.181E+04	0.6	0.67	49.46
1.23	319.95	.128E+05	4.9	0.77	43.38
1.86	320.58	.396E+05	19.1	0.96	34.50
2.48	321.20	.757E+05	43.5	1.15	28.98
3.11	321.83	.125E+06	81.2	1.30	25.58
3.74	322.46	.187E+06	135.9	1.45	22.92
4.37	323.09	.269E+06	195.6	1.45	22.90
5.00	323.72	.387E+06	287.8	1.48	22.44
5.63	324.35	.545E+06	428.4	1.56	21.22
6.25	324.97	.735E+06	630.5	1.71	19.42
6.88	325.60	.952E+06	876.1	1.83	18.12
7.51	326.23	.120E+07	1177.0	1.95	17.03
8.14	326.86	.149E+07	1554.4	2.08	15.93
8.77	327.49	.179E+07	2009.7	2.23	14.88
9.40	328.12	.213E+07	2533.9	2.37	13.99
10.02	328.74	.249E+07	3112.6	2.49	13.31
10.65	329.37	.288E+07	3760.9	2.60	12.75
11.28	330.00	.330E+07	4494.8	2.71	12.23

INFLOW	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
ID= 2 (8000)	1159.84	16.05	6.50	17.48	1.72	0.91
OUTFLOW: ID= 1 (6104)	1159.84	11.71	6.75	17.48	1.53	0.85

CALIB STANDHYD (1041)	Area (ha)	Imp(%)	Dir. Conn. (%)
ID= 1 DT=15.0 min	0.67	75.00	70.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	0.50	0.17
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	66.83	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 106.66 14.06

over (min) 15.00 30.00
 Storage Coeff. (min) = 2.41 (ii) 25.86 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.11 0.04

TOTALS
 PEAK FLOW (cms) = 0.14 0.00 0.142 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 78.80 13.20 59.10
 TOTAL RAINFALL (mm) = 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.16 0.73

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 36.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5104)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0450	0.0213
0.0020	0.0120	0.0560	0.0231
0.0130	0.0144	0.0660	0.0248
0.0240	0.0165	0.4660	0.0348
0.0340	0.0189	0.0000	0.0000

AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 INFLOW: ID= 2 (1041) 0.670 0.142 6.00 59.10
 OUTFLOW: ID= 1 (5104) 0.670 0.046 6.25 56.85

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 32.81
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0226

CALIB NASHYD (1042)
 ID= 1 DT=15.0 min

Area (ha) = 527.14 Curve Number (CN) = 42.6
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.34

Unit Hyd Opeak (cms) = 6.731
 PEAK FLOW (cms) = 2.648 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 13.653
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.169

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9104)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
ID1= 1 (1042):	527.14	2.648	8.00	13.65
+ ID2= 2 (5104):	0.67	0.046	6.25	56.85

ID = 3 (9104):	527.81	2.659	8.00	13.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8002)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
ID1= 1 (6104):	1159.84	11.706	6.75	17.48
+ ID2= 2 (9104):	527.81	2.659	8.00	13.71

ID = 3 (8002):	1687.65	14.006	6.75	16.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDBYD (1061)
 ID= 1 DT=15.0 min

Area (ha) = 32.26
 Total Imp(%) = 67.00
 Dir. Conn. (%) = 51.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 21.61 10.65
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 463.75 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 106.66 64.30
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 7.70 (ii) 20.46 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.10 0.05

TOTALS
 PEAK FLOW (cms) = 4.41 0.95 5.008 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 78.80 28.60 54.20
 TOTAL RAINFALL (mm) = 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.35 0.67

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 56.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5106)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.1580	0.9744
0.1090	0.5252	2.7090	1.0694
0.6250	0.6260	3.1540	1.1625
1.1640	0.7386	3.5540	1.1725
1.6140	0.8505	0.0000	0.0000

AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 INFLOW: ID= 2 (1061) 32.260 5.008 6.00 54.20
 OUTFLOW: ID= 1 (5106) 32.260 1.655 6.50 54.16

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 33.05
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 0.8729

CALIB NASHYD (1062)
 ID= 1 DT=15.0 min

Area (ha) = 227.27 Curve Number (CN) = 57.5
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.09

Unit Hyd Opeak (cms) = 3.556
 PEAK FLOW (cms) = 2.126 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 21.606
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.267

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9106)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
ID1= 1 (1062):	227.27	2.126	7.50	21.61
+ ID2= 2 (5106):	32.26	1.655	6.50	54.16

ID = 3 (9106):	259.53	3.487	6.50	25.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8002):	1687.65	14.006	6.75	16.30
+ ID2= 2 (9106):	259.53	3.487	6.50	25.65
=====				
ID = 3 (8004):	1947.18	17.463	6.75	17.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6108) IN= 2--> OUT= 1	Routing time step (min)' = 15.00
-------------------------------------	----------------------------------

<----- DATA FOR SECTION (1081.0) ----->				
Distance	Elevation	Manning		
0.00	326.06	0.0400		
12.06	325.74	0.0400		
30.15	324.28	0.0400		
59.27	320.35	0.0400		
72.37	317.60	0.0400		
87.45	314.52	0.0400		
93.48	313.91	0.0400		
105.54	313.55	0.0400		
106.17	313.40	0.0400 / 0.0300	Main Channel	
110.62	313.04	0.0300	Main Channel	
110.82	312.97	0.0300	Main Channel	
111.57	313.08	0.0300	Main Channel	
111.67	313.48	0.0300 / 0.0400	Main Channel	
129.66	316.62	0.0400		
150.77	318.95	0.0400		
180.92	319.61	0.0400		
205.04	321.23	0.0400		
232.18	322.09	0.0400		
268.37	322.31	0.0400		
298.52	326.00	0.0400		

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.43	313.40	.244E+04	0.7	0.59	58.83
1.09	314.06	.214E+05	10.4	1.01	34.24
1.76	314.73	.600E+05	40.2	1.39	24.89
2.42	315.39	.109E+06	92.1	1.75	19.80
3.08	316.05	.169E+06	166.5	2.05	16.88
3.75	316.72	.237E+06	264.4	2.31	14.97
4.41	317.38	.318E+06	386.3	2.52	13.72
5.07	318.04	.411E+06	539.3	2.72	12.71
5.74	318.71	.517E+06	725.5	2.91	11.88
6.40	319.37	.642E+06	909.2	2.94	11.77
7.06	320.03	.806E+06	1179.3	3.04	11.38
7.72	320.69	.989E+06	1515.9	3.18	10.87
8.39	321.36	.119E+07	1895.5	3.30	10.49
9.05	322.02	.143E+07	2310.2	3.36	10.30
9.71	322.68	.172E+07	2763.2	3.33	10.39
10.38	323.35	.205E+07	3477.6	3.53	9.81
11.04	324.01	.238E+07	4284.3	3.73	9.27
11.70	324.67	.274E+07	5154.0	3.91	8.85
12.37	325.34	.311E+07	6104.0	4.08	8.48

<----- hydrograph ----->					<- pi pe / channel ->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW: ID= 2 (8004)	17.46	6.75	17.55	1.25	1.08	
OUTFLOW: ID= 1 (6108)	15.48	7.50	17.55	1.20	1.06	

CALIB STANDHYD (1081) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	40.81 75.00	Di r. Conn. (%)=	70.00
Surface Area	(ha)=	30.61	10.20		
Dep. Storage	(mm)=	2.00	5.00		
Average Slope	(%)=	0.50	0.50		
Length	(m)=	521.60	40.00		
Mannings n	=	0.013	0.250		
Max. Eff. Inten. (mm/hr)=		106.66	10.94		

Storage Coeff. (min)=	15.00	45.00	
Unit Hyd. Tpeak (min)=	8.26 (ii)	34.18 (ii)	
Unit Hyd. peak (cms)=	15.00	45.00	
	0.09	0.03	
			TOTALS
PEAK FLOW (cms)=	7.53	0.18	7.600 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	78.80	10.38	58.27
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.13	0.72

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 30.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5108) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	2.7300	1.2970
	0.1380	0.7271	3.4270	1.4050
	0.7910	0.8733	3.9900	1.5110
	1.4720	1.0061	4.3900	1.5210
	2.0420	1.1467	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1081)	40.810	7.600	6.00	58.27
OUTFLOW: ID= 1 (5108)	40.810	2.518	6.25	58.24

PEAK FLOW REDUCTION [Qout/Qi n] (%)=	33.13
TIME SHIFT OF PEAK FLOW (min)=	15.00
MAXIMUM STORAGE USED (ha. m.)=	1.2595

CALIB NASHYD (1082) ID= 1 DT=15.0 min	Area Ia	(ha)= (mm)=	166.24 5.00	Curve Number (CN)=	32.2
	U. H. Tp(hrs)=	0.89	# of Li near Res. (N)=	1.50	

Unit Hyd Opeak (cms)= 3.171

PEAK FLOW (cms)=	0.762 (i)
TIME TO PEAK (hrs)=	7.250
RUNOFF VOLUME (mm)=	9.296
TOTAL RAINFALL (mm)=	80.800
RUNOFF COEFFICIENT =	0.115

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9108) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1082):	166.24	0.762	7.25	9.30
+ ID2= 2 (5108):	40.81	2.518	6.25	58.24
=====				
ID = 3 (9108):	207.05	3.109	6.25	18.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8006) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6108):	1947.18	15.485	7.50	17.55
+ ID2= 2 (9108):	207.05	3.109	6.25	18.94
=====				
ID = 3 (8006):	2154.23	17.380	7.25	17.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (1101)
ID= 1 DT=15.0 min

Area (ha)= 60.97
Total Imp(%)= 69.00 Dir. Conn.(%)= 55.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	42.07	18.90
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	637.55	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	106.66	55.81
over (min)	15.00	30.00
Storage Coeff. (min)	9.31 (ii)	22.82 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.04

TOTALS

PEAK FLOW (cms)	8.56	1.39	9.435 (iii)
TIME TO PEAK (hrs)	6.00	6.25	6.00
RUNOFF VOLUME (mm)	78.80	25.53	54.83
TOTAL RAINFALL (mm)	80.80	80.80	80.80
RUNOFF COEFFICIENT	0.98	0.32	0.68

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 52.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5110)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.0780	1.8615
0.2060	1.0120	5.1210	2.0372
1.1810	1.2083	5.9610	2.2096
2.2000	1.4181	6.3610	2.2196
3.0510	1.6293	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1101)	60.970	9.435	6.00	54.83
OUTFLOW: ID= 1 (5110)	60.970	3.172	6.50	54.81

PEAK FLOW REDUCTION [Oout/Oin](%)= 33.62
TIME SHIFT OF PEAK FLOW (min)= 30.00
MAXIMUM STORAGE USED (ha.m.)= 1.6821

CALIB
NASHYD (1102)
ID= 1 DT=15.0 min

Area (ha)= 262.95 Curve Number (CN)= 55.4
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 1.29

Unit Hyd Opeak (cms)	3.483
PEAK FLOW (cms)	2.044 (i)
TIME TO PEAK (hrs)	8.000
RUNOFF VOLUME (mm)	20.356
TOTAL RAINFALL (mm)	80.800
RUNOFF COEFFICIENT	0.252

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9110)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1102):	262.95	2.044	8.00	20.36
+ ID2= 2 (5110):	60.97	3.172	6.50	54.81
=====				
ID = 3 (9110):	323.92	4.807	6.50	26.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8008)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8006):	2154.23	17.380	7.25	17.68
+ ID2= 2 (9110):	323.92	4.807	6.50	26.84
=====				
ID = 3 (8008):	2478.15	21.243	7.25	18.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6112)
IN= 2--> OUT= 1

Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1121.0) ----->

Distance	Elevation	Manning
0.00	320.59	0.0400
13.48	317.93	0.0400
40.45	311.52	0.0400
64.04	307.68	0.0400
74.15	307.01	0.0400
91.00	307.22	0.0400
107.86	305.21	0.0400
111.23	304.86	0.0400
113.32	304.60	0.0400 / 0.0300
117.77	304.24	0.0300
117.97	304.17	0.0300
118.72	304.28	0.0300
118.82	304.68	0.0300 / 0.0400
124.71	304.76	0.0400
171.90	306.30	0.0400
205.60	307.24	0.0400
239.31	308.24	0.0400
262.90	310.64	0.0400
289.86	314.34	0.0400
333.68	317.66	0.0400

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.43	304.60	.372E+04	0.6	0.54	97.65
1.12	305.29	.463E+05	12.5	0.85	61.89
1.80	305.97	.149E+06	53.0	1.13	46.85
2.49	306.66	.311E+06	134.2	1.37	38.62
3.18	307.35	.554E+06	261.6	1.50	35.27
3.87	308.04	.894E+06	490.7	1.74	30.37
4.55	308.72	.129E+07	833.0	2.05	25.75
5.24	309.41	.171E+07	1269.8	2.36	22.39
5.93	310.10	.215E+07	1789.0	2.64	20.01
6.62	310.79	.261E+07	2392.7	2.90	18.21
7.30	311.47	.310E+07	3087.8	3.16	16.74
7.99	312.16	.361E+07	3873.6	3.40	15.52
8.68	312.85	.413E+07	4741.0	3.64	14.52
9.37	313.54	.467E+07	5689.3	3.86	13.69
10.05	314.22	.523E+07	6718.4	4.07	12.97
10.74	314.91	.581E+07	7770.7	4.24	12.46
11.43	315.60	.641E+07	8897.5	4.40	12.01
12.12	316.29	.704E+07	10113.2	4.55	11.61
12.80	316.97	.770E+07	11420.1	4.70	11.24

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8008)	2478.15	21.24	7.25	18.88	1.27	0.90
OUTFLOW: ID= 1 (6112)	2478.15	17.50	8.25	18.88	1.20	0.88

CALIB
STANDHYD (1121)
ID= 1 DT=15.0 min

Area (ha)= 9.96
Total Imp(%)= 55.00 Dir. Conn.(%)= 25.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	5.48	4.48
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	257.68	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	106.66	18.08
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over (min) 15.00 30.00
 Storage Coeff. (min) = 5.41 (ii) 26.61 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.11 0.04

TOTALS
 PEAK FLOW (cms) = 0.71 0.16 0.807 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 78.80 12.25 28.89
 TOTAL RAINFALL (mm) = 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.15 0.36

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 26.8 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5112)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.6660	0.2787
0.0340	0.1406	0.8360	0.3122
0.1930	0.1653	0.9740	0.3450
0.3590	0.2034	1.3740	0.3550
0.4980	0.2383	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1121)	9.960	0.807	6.00	28.89
OUTFLOW: ID= 1 (5112)	9.960	0.170	6.75	28.78

PEAK FLOW REDUCTION [Qout/Oin] (%) = 21.05
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 0.1630

CALIB NASHYD (1122)
 ID= 1 DT=15.0 min

Area (ha) = 404.93 Curve Number (CN) = 31.1
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.90

Unit Hyd Opeak (cms) = 3.635
 PEAK FLOW (cms) = 1.009 (i)
 TIME TO PEAK (hrs) = 9.000
 RUNOFF VOLUME (mm) = 8.963
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.111

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9112)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1122):	404.93	1.009	9.00	8.96
+ ID2= 2 (5112):	9.96	0.170	6.75	28.78
ID = 3 (9112):	414.89	1.058	8.25	9.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8010)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6112):	2478.15	17.500	8.25	18.88
+ ID2= 2 (9112):	414.89	1.058	8.25	9.44
ID = 3 (8010):	2893.04	18.559	8.25	17.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6114)
 IN= 2---> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (1141.0) ----->

Distance	Elevation	Manning
0.00	312.27	0.0500
14.07	309.70	0.0500
28.14	302.51	0.0500
35.17	305.81	0.0500
42.20	304.95	0.0500
52.75	304.24	0.0500
77.37	303.61	0.0500
98.47	301.33	0.0500
131.76	300.40	0.0500 / 0.0300
136.21	300.04	0.0300
136.41	299.97	0.0300
137.16	300.08	0.0300
137.26	300.48	0.0300 / 0.0500
140.68	300.53	0.0500
179.36	302.20	0.0500
214.53	303.82	0.0500
242.66	305.27	0.0500
288.38	307.66	0.0500
316.52	311.76	0.0500
348.17	313.48	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.43	300.40	.366E+04	0.7	0.59	88.31
1.05	301.02	.505E+05	12.1	0.75	69.49
1.68	301.65	.165E+06	52.7	0.99	52.28
2.30	302.27	.324E+06	130.3	1.26	41.40
2.93	302.90	.520E+06	247.1	1.48	35.08
3.55	303.52	.754E+06	406.8	1.68	30.91
4.18	304.15	.104E+07	586.1	1.76	29.55
4.80	304.77	.138E+07	856.0	1.93	26.96
5.43	305.40	.177E+07	1201.1	2.12	24.56
6.05	306.02	.219E+07	1612.2	2.30	22.63
6.68	306.65	.264E+07	2090.6	2.47	21.07
7.30	307.27	.313E+07	2653.0	2.64	19.65
7.93	307.90	.364E+07	3303.9	2.83	18.36
8.55	308.52	.417E+07	4053.9	3.03	17.14
9.18	309.15	.471E+07	4871.9	3.22	16.12
9.80	309.77	.527E+07	5756.2	3.41	15.25
10.43	310.40	.584E+07	6699.8	3.58	14.52
11.05	311.02	.642E+07	7710.5	3.74	13.89
11.68	311.65	.702E+07	8788.5	3.90	13.32

<---- hydrograph ----> <- pipe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8010)	2893.04	18.56	8.25	17.53	1.15	0.78
OUTFLOW: ID= 1 (6114)	2893.04	16.23	9.75	17.53	1.12	0.77

CALIB STANDHYD (1141)
 ID= 1 DT=15.0 min

Area Total (ha) = 19.45
 Imp(%) = 45.00
 Dir. Conn. (%) = 15.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 8.75 10.70
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 360.09 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 106.66 47.44
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 6.61 (ii) 21.03 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.10 0.05

TOTALS
 PEAK FLOW (cms) = 0.80 0.69 1.238 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 78.80 20.61 29.34
 TOTAL RAINFALL (mm) = 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.26 0.36

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%

YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 43.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5114)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.3010	0.5065
0.0660	0.2376	1.6340	0.5791
0.3770	0.2749	1.9020	0.6501
0.7020	0.3549	2.3020	0.6601
0.9730	0.4237	0.0000	0.0000

INFLOW : ID= 2 (1141)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5114)	19.450	1.238	6.00	29.34
	19.450	0.430	6.75	29.29

PEAK FLOW REDUCTION [Qout/Qin] (%) = 34.76
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha.m.) = 0.2897

CALIB NASHYD (1142)
ID= 1 DT=15.0 min

Area (ha) = 268.19 Curve Number (CN) = 51.3
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 1.09

Unit Hyd Qpeak (cms) = 4.197

PEAK FLOW (cms) = 2.073 (i)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 17.967
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.222

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9114)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1142):	268.19	2.073	7.50	17.97
+ ID2= 2 (5114):	19.45	0.430	6.75	29.29
ID = 3 (9114):	287.64	2.417	7.00	18.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8012)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6114):	2893.04	16.234	9.75	17.53
+ ID2= 2 (9114):	287.64	2.417	7.00	18.73
ID = 3 (8012):	3180.68	18.013	9.50	17.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0116)
ID= 1 DT=15.0 min

Area (ha) = 206.43 Curve Number (CN) = 64.0
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 0.80

Unit Hyd Qpeak (cms) = 4.394

PEAK FLOW (cms) = 2.988 (i)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 25.903

TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.321

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8014)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0116):	206.43	2.988	7.00	25.90
+ ID2= 2 (8012):	3180.68	18.013	9.50	17.64
ID = 3 (8014):	3387.11	19.883	9.25	18.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6118)
IN= 2--> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (1181.0) ----->

Distance	Elevation	Manning
0.00	307.18	0.0450
24.89	299.75	0.0450
37.34	296.39	0.0450
71.57	292.75	0.0450
96.46	291.12	0.0450
108.90	289.43	0.0450
112.02	288.86	0.0450
115.13	288.46	0.0450
117.24	288.32	0.0450 / 0.0350
117.49	288.15	0.0350
118.24	288.00	0.0350
118.99	288.20	0.0350
119.24	288.36	0.0350 / 0.0450
121.35	288.40	0.0450
149.35	289.36	0.0450
171.14	290.47	0.0450
208.47	293.14	0.0450
236.48	295.17	0.0450
280.04	299.48	0.0450
308.04	307.00	0.0450

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.32	288.32	.116E+04	0.2	0.62	81.92
1.30	289.30	.631E+05	22.2	1.07	47.43
2.29	290.29	.218E+06	117.5	1.64	30.95
3.27	291.27	.448E+06	314.6	2.15	23.72
4.25	292.25	.756E+06	625.6	2.53	20.13
5.24	293.24	.115E+07	1093.2	2.91	17.50
6.22	294.22	.161E+07	1731.6	3.28	15.50
7.20	295.20	.214E+07	2547.4	3.63	14.01
8.19	296.19	.274E+07	3584.9	4.00	12.72
9.17	297.17	.338E+07	4859.1	4.39	11.60
10.15	298.15	.407E+07	6341.5	4.76	10.70
11.13	299.13	.480E+07	8024.6	5.11	9.97
12.12	300.12	.556E+07	10006.6	5.49	9.27
13.10	301.10	.635E+07	12244.6	5.89	8.64
14.08	302.08	.716E+07	14681.7	6.26	8.13
15.07	303.07	.799E+07	17316.5	6.62	7.69
16.05	304.05	.884E+07	20148.1	6.96	7.31
17.03	305.03	.971E+07	23176.2	7.29	6.98
18.02	306.02	.106E+08	26400.8	7.61	6.69

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8014)	3387.11	19.88	9.25	18.14	1.20	1.00
OUTFLOW: ID= 1 (6118)	3387.11	19.17	10.00	18.14	1.16	0.97

CALIB NASHYD (0118)
ID= 1 DT=15.0 min

Area (ha) = 340.53 Curve Number (CN) = 45.0
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 1.12

Unit Hyd Qpeak (cms) = 5.200

PEAK FLOW (cms) = 2.109 (i)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 14.747
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.183

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8016) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0118):	340.53	2.109	7.75	14.75
+ ID2= 2 (6118):	3387.11	19.174	10.00	18.14

ID = 3 (8016):	3727.64	20.817	10.00	17.83

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6122)
 IN= 2----> OUT= 1
 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1221.0) ----->

Distance	Elevation	Manning	
0.00	277.00	0.0500	
52.18	272.82	0.0500	
86.97	269.38	0.0500	
134.80	267.35	0.0500	
160.89	263.85	0.0500	
169.58	262.88	0.0500	
178.28	262.45	0.0500	
195.67	262.07	0.0500	
213.07	260.99	0.0500	
226.01	259.80	0.0500 / 0.0300	Main Channel
226.11	259.30	0.0300	Main Channel
230.11	259.30	0.0300	Main Channel
230.21	259.80	0.0300 / 0.0500	Main Channel
235.01	260.20	0.0500	
239.15	260.57	0.0500	
243.50	260.93	0.0500	
300.03	264.43	0.0500	
339.16	268.34	0.0500	
373.95	272.36	0.0500	
430.48	277.46	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.50	259.80	.626E+04	2.5	1.23	41.35
1.41	260.71	.462E+05	21.8	1.44	35.27
2.31	261.61	.149E+06	79.6	1.63	31.19
3.22	262.52	.340E+06	200.1	1.80	28.34
4.12	263.42	.636E+06	446.8	2.15	23.71
5.03	264.33	.995E+06	819.9	2.52	20.23
5.93	265.23	.141E+07	1337.0	2.90	17.56
6.84	266.14	.187E+07	1984.8	3.25	15.67
7.74	267.04	.237E+07	2766.0	3.57	14.26
8.65	267.95	.292E+07	3596.9	3.76	13.53
9.55	268.85	.355E+07	4575.4	3.93	12.95
10.46	269.76	.427E+07	5814.9	4.16	12.23
11.36	270.66	.503E+07	7319.0	4.44	11.46
12.27	271.57	.584E+07	9009.7	4.71	10.81
13.17	272.47	.670E+07	10886.3	4.96	10.26
14.08	273.38	.761E+07	12893.2	5.17	9.84
14.98	274.28	.858E+07	15098.1	5.37	9.48
15.89	275.19	.961E+07	17527.6	5.57	9.14
16.79	276.09	.107E+08	20190.3	5.76	8.83

<----- hydrograph -----> <----- pi pe / channel ----->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8016)	3727.64	20.82	10.00	17.83	1.36	1.43
OUTFLOW: ID= 1 (6122)	3727.64	20.23	10.50	17.83	1.32	1.42

CALIB STANDHYD (1221) Area (ha) = 117.91

|| ID= 1 DT=15.0 mi n | Total Imp(%)= 44.00 Di r. Conn.(%)= 16.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	51.88	66.03
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	886.60	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	106.66	21.62
over (min)	15.00	45.00
Storage Coeff. (min)	11.35 (ii)	31.09 (ii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.08	0.03
PEAK FLOW (cms)	4.53	2.42
TIME TO PEAK (hrs)	6.00	6.50
RUNOFF VOLUME (mm)	78.80	16.07
TOTAL RAINFALL (mm)	80.80	80.80
RUNOFF COEFFICIENT	0.98	0.20

TOTALS
 5.529 (iii)
 6.00
 26.11
 80.80
 0.32

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 36.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5122)
 IN= 2----> OUT= 1
 DT= 15.0 mi n

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	7.8880	3.0390
	0.3990	1.4100	9.9040	3.4850
	2.2850	1.6269	11.5290	3.9214
	4.2550	2.1164	11.9290	3.9314
	5.9010	2.5338	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1221)	117.910	5.529	6.00	26.11
OUTFLOW: ID= 1 (5122)	117.910	1.956	7.00	26.10

PEAK FLOW REDUCTION [Out/Oi n] (%) = 35.38
 TIME SHIFT OF PEAK FLOW (mi n) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 1.5926

CALIB NASHYD (1222) Area (ha) = 246.95 Curve Number (CN) = 43.8
 ID= 1 DT=15.0 mi n Ia (mm) = 5.00 # of Li near Res. (N) = 1.50
 U. H. Tp (hrs) = 1.13

Unit Hyd Opeak (cms) = 3.728
 PEAK FLOW (cms) = 1.457 (i)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 14.181
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.176

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8018) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1222):	246.95	1.457	7.75	14.18
+ ID2= 2 (5122):	117.91	1.956	7.00	26.10

ID = 3 (8018):	364.86	3.357	7.00	18.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6122):	3727.64	20.234	10.50	17.83
+ ID2= 2 (8018):	364.86	3.357	7.00	18.03
=====				
ID = 3 (8020):	4092.50	21.682	10.50	17.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1261) ID= 1 DT=15.0 min	Area (ha)	Imp (%)	Di r. Conn. (%)
	50.19	36.00	26.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	18.07	32.12
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	578.45	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	106.66	19.51
over (min)	15.00	30.00
Storage Coeff. (min)	8.79 (ii)	29.35 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.04

TOTALS		
PEAK FLOW (cms)	3.39	1.19
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	78.80	18.66
TOTAL RAINFALL (mm)	80.80	80.80
RUNOFF COEFFICIENT	0.98	0.23
		4.110 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 48.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5126) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	3.3570	1.2179
	0.1700	0.5264	4.2160	1.4221
	0.9720	0.5968	4.9080	1.6217
	1.8110	0.8165	5.3080	1.6317
	2.5120	0.9954	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1261)	50.190	4.110	6.00	34.29
OUTFLOW: ID= 1 (5126)	50.190	1.576	6.50	34.28

PEAK FLOW REDUCTION [Qout/Qin] (%) = 38.36
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 0.7567

CALIB NASHYD (1262) ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	# of Li near Res. (N)
	83.33	57.7	1.50

Unit Hyd Opeak (cms) = 2.509

PEAK FLOW (cms)	1.300 (i)
TIME TO PEAK (hrs)	6.500
RUNOFF VOLUME (mm)	21.412
TOTAL RAINFALL (mm)	80.800
RUNOFF COEFFICIENT	0.265

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1262):	83.33	1.300	6.50	21.41
+ ID2= 2 (5126):	50.19	1.576	6.50	34.28
=====				
ID = 3 (8024):	133.52	2.877	6.50	26.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0124) ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	# of Li near Res. (N)
	121.43	43.0	1.50

Unit Hyd Opeak (cms) = 4.049

PEAK FLOW (cms)	1.279 (i)
TIME TO PEAK (hrs)	6.500
RUNOFF VOLUME (mm)	13.547
TOTAL RAINFALL (mm)	80.800
RUNOFF COEFFICIENT	0.168

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0124):	121.43	1.279	6.50	13.55
+ ID2= 2 (8024):	133.52	2.877	6.50	26.25
=====				
ID = 3 (8026):	254.95	4.155	6.50	20.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6128) IN= 2--> OUT= 1	Routing time step (min)
	15.00

<----- DATA FOR SECTION (1281.0) ----->		
Distance	Elevation	Manning
0.00	270.58	0.0400
7.71	270.36	0.0400
15.43	269.48	0.0400
23.14	265.88	0.0400
42.43	261.83	0.0400
61.71	258.31	0.0400
75.21	256.62	0.0400
90.64	255.35	0.0400
92.49	255.00	0.0400 / 0.0300
93.49	254.00	0.0300
94.49	253.90	0.0300
94.99	253.80	0.0300
95.49	253.70	0.0300
95.99	253.80	0.0300
98.49	255.00	0.0300 / 0.0400
100.28	255.44	0.0400
134.99	257.45	0.0400
148.49	259.67	0.0400
167.77	263.42	0.0400
190.92	270.00	0.0400

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.65	254.35	.439E+04	2.0	1.24	36.10
1.30	255.00	.131E+05	9.4	1.92	23.39
2.18	255.88	.433E+05	37.0	2.30	19.54
3.06	256.76	.131E+06	113.3	2.34	19.20
3.95	257.65	.273E+06	273.8	2.70	16.64
4.83	258.53	.453E+06	538.5	3.21	14.01
5.71	259.41	.658E+06	902.9	3.70	12.14
6.59	260.29	.886E+06	1368.4	4.16	10.79
7.48	261.18	.114E+07	1936.3	4.59	9.79
8.36	262.06	.141E+07	2610.9	4.99	9.01
9.24	262.94	.171E+07	3398.7	5.37	8.36
10.12	263.82	.202E+07	4309.6	5.75	7.81

11.01	264.71	.235E+07	5347.5	6.13	7.33
11.89	265.59	.270E+07	6501.0	6.48	6.93
12.77	266.47	.307E+07	7808.6	6.86	6.55
13.65	267.35	.345E+07	9250.5	7.23	6.21
14.54	268.24	.384E+07	10808.3	7.59	5.92
15.42	269.12	.424E+07	12482.5	7.94	5.66
16.30	270.00	.466E+07	14127.4	8.18	5.49

<---- hydrograph ----> <- pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8026)	254.95	4.16	6.50	20.20	0.84	1.39
OUTFLOW: ID= 1 (6128)	254.95	3.43	7.25	20.20	0.77	1.33

CALIB STANDHYD (1281) ID= 1 DT=15.0 min	Area Total	(ha)= 55.87 Imp(%)= 50.00	Di r. Conn. (%)= 29.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	27.93	27.93
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	610.30	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (min)	106.66 / 15.00	55.68 / 30.00
Storage Coeff. (min)	9.07 (ii)	22.59 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.04
PEAK FLOW (cms)	4.17	2.06
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	78.80	26.00
TOTAL RAINFALL (mm)	80.80	80.80
RUNOFF COEFFICIENT	0.98	0.32

TOTALS
5.460 (iii)
6.00
41.32
80.80
0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 54.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5128)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.7380	1.5139
0.1890	0.7401	4.6930	1.7113
1.0830	0.8643	5.4630	1.9045
2.0160	1.0853	5.8630	1.9045
2.7960	1.2819	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1281)	55.870	5.460	6.00	41.32
OUTFLOW: ID= 1 (5128)	55.870	2.014	6.50	41.30

PEAK FLOW REDUCTION [Qout/Qin] (%) = 36.88
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 1.0935

CALIB NASHYD (1282) ID= 1 DT=15.0 min	Area Ia	(ha)= 184.36 (mm)= 5.00	Curve Number (CN)= 62.0 # of Linear Res. (N)= 1.50
	U. H.	0.62	

Unit Hyd Opeak (cms)	5.093
PEAK FLOW (cms)	3.065 (i)
TIME TO PEAK (hrs)	6.750
RUNOFF VOLUME (mm)	24.306
TOTAL RAINFALL (mm)	80.800
RUNOFF COEFFICIENT	0.301

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9128) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1282):	184.36	3.065	6.75	24.31
+ ID2= 2 (5128):	55.87	2.014	6.50	41.30
ID= 3 (9128):	240.23	5.062	6.50	28.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8028) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6128):	254.95	3.432	7.25	20.20
+ ID2= 2 (9128):	240.23	5.062	6.50	28.26
ID= 3 (8028):	495.18	8.073	7.00	24.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8022) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8020):	4092.50	21.682	10.50	17.85
+ ID2= 2 (8028):	495.18	8.073	7.00	24.11
ID= 3 (8022):	4587.68	24.304	10.25	18.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1201) ID= 1 DT=15.0 min	Area Total	(ha)= 176.35 Imp(%)= 45.00	Di r. Conn. (%)= 22.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	79.36	96.99
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1084.28	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) over (min)	106.66 / 15.00	45.71 / 30.00
Storage Coeff. (min)	12.81 (ii)	27.44 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04

	PEAK FLOW (cms)	TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
	8.92	5.31	6.00	80.80	0.42
	6.00	6.25	21.57	80.80	0.27
	78.80	21.57	80.80	80.80	0.42
	80.80	80.80	80.80	80.80	0.42
	0.98	0.27	0.42	0.42	0.42

TOTALS
12.202 (iii)
6.00
34.16
80.80
0.42

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 47.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5120) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	11.7960	4.5929
	0.5970	2.1554	14.8120	5.2509
	3.4170	2.4937	17.2430	5.8948
	6.3630	3.2187	17.6430	5.9048

8. 8260 3. 8422 | 0. 0000 0. 0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1201)	176. 350	12. 202	6. 00	34. 16
OUTFLOW: ID= 1 (5120)	176. 350	4. 726	6. 75	34. 16

PEAK FLOW REDUCTION [Qout/Qin] (%) = 38. 73
 TIME SHIFT OF PEAK FLOW (min) = 45. 00
 MAXIMUM STORAGE USED (ha. m.) = 2. 8382

CALIB STANDHYD (1202) ID= 1 DT=15. 0 min	Area (ha)	Total Imp(%)	Di r. Conn. (%)
	70. 67	43. 00	20. 00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	30. 39	40. 28
Dep. Storage (mm)=	2. 00	5. 00
Average Slope (%)=	0. 50	0. 50
Length (m)=	686. 39	40. 00
Mannings n =	0. 013	0. 250

Max. Eff. Inten. (mm/hr)=	106. 66	44. 85
Storage Coeff. (min)=	15. 00	30. 00
Unit Hyd. Tpeak (min)=	9. 74 (ii)	24. 48 (ii)
Unit Hyd. peak (cms)=	15. 00	30. 00
	0. 09	0. 04

TOTALS
 PEAK FLOW (cms)= 3. 56 2. 30 4. 987 (iii)
 TIME TO PEAK (hrs)= 6. 00 6. 25 6. 00
 RUNOFF VOLUME (mm)= 78. 80 21. 39 32. 87
 TOTAL RAINFALL (mm)= 80. 80 80. 80 80. 80
 RUNOFF COEFFICIENT = 0. 98 0. 26 0. 41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 47. 3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8030) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1202):	70. 67	4. 987	6. 00	32. 87
+ ID2= 2 (5120):	176. 35	4. 726	6. 75	34. 16
ID = 3 (8030):	247. 02	6. 852	6. 50	33. 79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8032) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8022):	4587. 68	24. 304	10. 25	18. 52
+ ID2= 2 (8030):	247. 02	6. 852	6. 50	33. 79
ID = 3 (8032):	4834. 70	25. 389	10. 00	19. 30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6130) IN= 2---> OUT= 1	Routing time step (min)
	15. 00

<----- DATA FOR SECTION (1301. 0) ----->

Distance	Elevation	Manning
0. 00	257. 18	0. 0350
62. 45	254. 70	0. 0350
109. 29	252. 74	0. 0350
150. 92	251. 55	0. 0350
192. 56	248. 96	0. 0350

239. 39	247. 16	0. 0350
291. 84	246. 00	0. 0350
296. 84	245. 00	0. 0350
298. 84	245. 00	0. 0350 / 0. 0300
299. 24	244. 60	0. 0300
301. 84	244. 60	0. 0300
303. 44	244. 60	0. 0300
303. 84	245. 00	0. 0300 / 0. 0350
306. 84	245. 00	0. 0350
309. 84	246. 00	0. 0350
317. 46	247. 22	0. 0350
348. 68	254. 83	0. 0350
359. 09	256. 70	0. 0350
452. 76	258. 23	0. 0350
515. 22	258. 33	0. 0350

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0. 40	245. 00	. 870E+04	1. 2	0. 64	124. 12
1. 04	245. 64	. 468E+05	9. 5	0. 96	81. 81
1. 68	246. 28	. 109E+06	25. 6	1. 11	70. 86
2. 32	246. 92	. 257E+06	63. 5	1. 17	67. 51
2. 96	247. 56	. 498E+06	142. 7	1. 36	58. 18
3. 61	248. 21	. 800E+06	266. 7	1. 58	50. 01
4. 25	248. 85	. 116E+07	437. 4	1. 78	44. 24
4. 89	249. 49	. 157E+07	669. 1	2. 01	39. 20
5. 53	250. 13	. 203E+07	954. 9	2. 23	35. 36
6. 17	250. 77	. 252E+07	1294. 4	2. 43	32. 42
6. 81	251. 41	. 305E+07	1689. 6	2. 62	30. 07
7. 45	252. 05	. 363E+07	2090. 7	2. 73	28. 93
8. 09	252. 69	. 429E+07	2553. 1	2. 82	27. 98
8. 73	253. 33	. 501E+07	3138. 4	2. 96	26. 60
9. 37	253. 97	. 579E+07	3803. 8	3. 11	25. 36
10. 02	254. 62	. 662E+07	4549. 3	3. 25	24. 25
10. 66	255. 26	. 751E+07	5364. 9	3. 38	23. 32
11. 30	255. 90	. 845E+07	6265. 7	3. 51	22. 49
11. 94	256. 54	. 946E+07	7260. 1	3. 63	21. 72

<---- hydrograph ----> <- pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8032)	4834. 70	25. 39	10. 00	19. 30	1. 68	1. 11
OUTFLOW: ID= 1 (6130)	4834. 70	23. 78	11. 00	19. 30	1. 60	1. 09

CALIB STANDHYD (1301) ID= 1 DT=15. 0 min	Area (ha)	Total Imp(%)	Di r. Conn. (%)
	320. 20	64. 00	47. 00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	204. 93	115. 27
Dep. Storage (mm)=	2. 00	5. 00
Average Slope (%)=	0. 50	0. 50
Length (m)=	1461. 05	40. 00
Mannings n =	0. 013	0. 250

Max. Eff. Inten. (mm/hr)=	106. 66	101. 88
Storage Coeff. (min)=	15. 00	30. 00
Unit Hyd. Tpeak (min)=	15. 32 (ii)	25. 94 (ii)
Unit Hyd. peak (cms)=	15. 00	30. 00
	0. 07	0. 04

TOTALS
 PEAK FLOW (cms)= 32. 18 14. 78 41. 822 (iii)
 TIME TO PEAK (hrs)= 6. 00 6. 25 6. 00
 RUNOFF VOLUME (mm)= 78. 80 44. 71 60. 73
 TOTAL RAINFALL (mm)= 80. 80 80. 80 80. 80
 RUNOFF COEFFICIENT = 0. 98 0. 55 0. 75

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75. 3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5130) IN= 2---> OUT= 1 DT= 15. 0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)

0.0000	0.0000	18.6040	9.7083
1.1320	5.3237	22.6740	10.8355
6.6360	6.1006	30.4650	12.2155
10.3780	7.2341	31.8650	12.2255
14.1960	8.4233	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1301)	320.200	41.822	6.00	60.73
OUTFLOW: ID= 1 (5130)	320.200	16.531	6.75	60.73

PEAK FLOW REDUCTION [Qout/Qin] (%) = 39.53
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 9.2611

CALIB NASHYD (1302)	Area (ha) = 65.86	Curve Number (CN) = 80.0
ID= 1 DT=15.0 min	Ia (mm) = 5.00	# of Linear Res. (N) = 1.50
U.H. Tp (hrs) = 0.69		

Unit Hyd Opeak (cms) = 1.620

PEAK FLOW (cms) = 1.734 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 40.524
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.502

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8036)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (1302):	65.86	1.734	6.75	40.52
+ ID2= 2 (5130):	320.20	16.531	6.75	60.73
ID = 3 (8036):	386.06	18.265	6.75	57.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8034)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (6130):	4834.70	23.779	11.00	19.30
+ ID2= 2 (8036):	386.06	18.265	6.75	57.28
ID = 3 (8034):	5220.76	27.601	7.25	22.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1861)	Area (ha) = 6.85	Di r. Conn. (%) = 27.00
ID= 1 DT=15.0 min	Total Imp (%) = 51.00	

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	3.49	3.36
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	213.70	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 106.66 over (min) = 15.00
 Storage Coeff. (min) = 4.83 (ii) 18.13 (ii)
 Unit Hyd. Tpeak (min) = 15.00
 Unit Hyd. peak (cms) = 0.11 0.05

TOTALS
 PEAK FLOW (cms) = 0.53 0.29 0.712 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 78.80 25.89 40.17
 TOTAL RAINFALL (mm) = 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.32 0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 52.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5186)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1				
DT= 15.0 min	0.0000	0.0000	0.4580	0.1865
	0.0230	0.0917	0.5750	0.2106
	0.1330	0.1071	0.6700	0.2341
	0.2470	0.1341	1.0700	0.2441
	0.3430	0.1582	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1861)	6.850	0.712	6.00	40.17
OUTFLOW: ID= 1 (5186)	6.850	0.242	6.50	40.03

PEAK FLOW REDUCTION [Qout/Qin] (%) = 34.02
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 0.1335

CALIB STANDHYD (1862)	Area (ha) = 344.94	Di r. Conn. (%) = 23.00
ID= 1 DT=15.0 min	Total Imp (%) = 43.00	

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	148.32	196.62
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1516.44	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 106.66 over (min) = 15.00
 Storage Coeff. (min) = 15.67 (ii) 29.92 (ii)
 Unit Hyd. Tpeak (min) = 15.00
 Unit Hyd. peak (cms) = 0.07 0.04

TOTALS
 PEAK FLOW (cms) = 16.80 10.99 23.593 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 78.80 24.04 36.64
 TOTAL RAINFALL (mm) = 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.30 0.45

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 52.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9186)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (1862):	344.94	23.593	6.00	36.64
+ ID2= 2 (5186):	6.85	0.242	6.50	40.03
ID = 3 (9186):	351.79	23.615	6.00	36.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1881)	Area (ha) = 1.12	Di r. Conn. (%) = 46.00
ID= 1 DT=15.0 min	Total Imp (%) = 75.00	

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	0.84	0.28
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	86.41	40.00

Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 106.66 127.97
over (min) = 15.00 15.00

Storage Coeff. (mi n)= 2.81 (ii) 12.50 (ii)

Unit Hyd. Tpeak (mi n)= 15.00 15.00

Unit Hyd. peak (cms)= 0.11 0.08

PEAK FLOW (cms)= 0.15 0.08 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 0.228 (iii)
6.00
RUNOFF VOLUME (mm)= 78.80 38.78 57.18
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.48 0.71

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 59.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5188)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.0750	0.0357
0.0040	0.0200	0.0940	0.0386
0.0220	0.0240	0.1100	0.0416
0.0410	0.0277	0.5100	0.0516
0.0560	0.0315	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1.120	0.228	6.00	57.18
1.120	0.067	6.25	56.05

INFLOW : ID= 2 (1881)
OUTFLOW: ID= 1 (5188)

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 29.30
TIME SHIF T OF PEAK FLOW (mi n) = 15.00
MAXIMUM STORAGE USED (ha. m.) = 0.0344

CALIB STANDHYD (1882)
ID= 1 DT=15.0 mi n

Area (ha)	Imp (%)	Di r. Conn. (%)
307.41	41.00	25.00

IMPERVIOUS (ha)	PERVIOUS (i)
126.04	181.37
2.00	5.00
0.50	0.50
1431.57	40.00
0.013	0.250

Surface Area (ha)= 126.04
Dep. Storage (mm)= 2.00
Average Slope (%)= 0.50
Length (m)= 1431.57
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 106.66 53.35
over (mi n) = 15.00 30.00

Storage Coeff. (mi n)= 15.13 (ii) 28.89 (ii)

Unit Hyd. Tpeak (mi n)= 15.00 30.00

Unit Hyd. peak (cms)= 0.07 0.04

PEAK FLOW (cms)= 16.52 11.32 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.25 23.564 (iii)
6.00
RUNOFF VOLUME (mm)= 78.80 27.68 40.46
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.34 0.50

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 59.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9188)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
307.41	23.564	6.00	40.46
1.12	0.067	6.25	56.05

ID1= 1 (1882):
+ ID2= 2 (5188):

ID = 3 (9188): 308.53 23.593 6.00 40.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8068)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
351.79	23.615	6.00	36.70
308.53	23.593	6.00	40.52
660.32	47.208	6.00	38.49

ID1= 1 (9186):
+ ID2= 2 (9188):
ID = 3 (8068):

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1821)
ID= 1 DT=15.0 mi n

Area (ha)	Imp (%)	Di r. Conn. (%)
111.80	40.00	17.00

IMPERVIOUS (ha)	PERVIOUS (i)
44.72	67.08
2.00	5.00
0.50	0.50
863.33	40.00
0.013	0.250

Surface Area (ha)= 44.72
Dep. Storage (mm)= 2.00
Average Slope (%)= 0.50
Length (m)= 863.33
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 106.66 50.21
over (mi n) = 15.00 30.00

Storage Coeff. (mi n)= 11.17 (ii) 25.26 (ii)

Unit Hyd. Tpeak (mi n)= 15.00 30.00

Unit Hyd. peak (cms)= 0.08 0.04

PEAK FLOW (cms)= 4.59 4.22 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.25 7.214 (iii)
6.00
RUNOFF VOLUME (mm)= 78.80 24.15 33.44
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.30 0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 52.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5182)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	5.7800	2.5656
0.3790	1.0207	7.3720	3.0829
1.6240	1.1939	8.7710	3.5956
3.0930	1.6631	9.1710	3.6056
4.3530	2.1032	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
111.800	7.214	6.00	33.44
111.800	2.963	6.75	33.44

INFLOW : ID= 2 (1821)
OUTFLOW: ID= 1 (5182)

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 41.08
TIME SHIF T OF PEAK FLOW (mi n) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 1.6292

CALIB NASHYD (1822)
ID= 1 DT=15.0 mi n

Area (ha)	Imp (%)	Curve Number (CN)
44.40	5.00	57.0

Ia (mm)= 5.00
U. H. Tp(hrs)= 0.79
of Li near Res. (N)= 1.50

Unit Hyd Opeak (cms)= 0.966

PEAK FLOW (cms)= 0.529 (i)
TIME TO PEAK (hrs)= 7.000

RUNOFF VOLUME (mm) = 21.172
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.262

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8062) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1822):	44.40	0.529	7.00	21.17
+ ID2= 2 (5182):	111.80	2.963	6.75	33.44

ID = 3 (8062):	156.20	3.486	6.75	29.95

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1841) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	145.07 48.00	Dir. Conn. (%)=	19.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	69.63	75.44
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	983.43	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)= over (min)	106.66 15.00	68.88 30.00
Storage Coeff. (min)=	12.08 (ii)	24.50 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04

			TOTALS (iii)
PEAK FLOW (cms)=	6.48	6.64	10.660
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	78.80	29.19	38.62
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.36	0.48

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 55.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5184) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	5.9270	2.3472
	0.2980	1.0088	7.4630	2.7585
	1.6660	1.1775	8.7490	3.1642
	3.1100	1.4809	9.1490	3.1742
	4.4100	1.8897	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1841)	145.070	10.660	6.00	38.62
OUTFLOW: ID= 1 (5184)	145.070	5.204	6.75	38.61

PEAK FLOW REDUCTION [Qout/Qin] (%) = 48.82
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 2.1717

CALIB STANDHYD (1842) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	117.45 42.00	Dir. Conn. (%)=	17.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	49.33	68.12
Dep. Storage	2.00	5.00

Average Slope (%) = 0.50
 Length (m) = 884.87
 Mannings n = 0.013

Max. Eff. Inten. (mm/hr)= 106.66
 over (min) = 15.00
 Storage Coeff. (min)= 11.34 (ii)
 Unit Hyd. Tpeak (min)= 15.00
 Unit Hyd. peak (cms)= 0.08

			TOTALS (iii)
PEAK FLOW (cms)=	4.80	5.17	8.039
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	78.80	27.50	36.22
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.34	0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 55.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8060) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1842):	117.45	8.039	6.00	36.22
+ ID2= 2 (5184):	145.07	5.204	6.75	38.61

ID = 3 (8060):	262.52	11.310	6.25	37.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8064) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8060):	262.52	11.310	6.25	37.54
+ ID2= 2 (8062):	156.20	3.486	6.75	29.95

ID = 3 (8064):	418.72	13.822	6.25	34.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1781) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	55.50 39.00	Dir. Conn. (%)=	13.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	21.65	33.86
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	608.28	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)= over (min)	106.66 15.00	95.13 30.00
Storage Coeff. (min)=	9.06 (ii)	19.97 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.05

			TOTALS (iii)
PEAK FLOW (cms)=	1.86	4.56	5.155
TIME TO PEAK (hrs)=	6.00	6.25	6.25
RUNOFF VOLUME (mm)=	78.80	43.11	47.75
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.53	0.59

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 74.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5178)				
IN= 2--> OUT= 1				
DT= 15.0 mi n				
OUTFLOW	STORAGE	OUTFLOW	STORAGE	
(cms)	(ha. m.)	(cms)	(ha. m.)	
0.0000	0.0000	6.1870	0.7248	
0.1880	0.3394	9.3670	0.8313	
1.7260	0.4003	10.7440	0.9285	
3.2000	0.4933	11.1440	0.9385	
4.6710	0.6039	0.0000	0.0000	
AREA	OPEAK	TPEAK	R. V.	
(ha)	(cms)	(hrs)	(mm)	
INFLOW : ID= 2 (1781)	55.500	5.155	6.25	47.75
OUTFLOW: ID= 1 (5178)	55.500	4.411	6.25	47.74

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 85.57
 TIME SHIF T OF PEAK FLOW (mi n) = 0.00
 MAXIMUM STORAGE USED (ha. m.) = 0.6178

CALIB NASHYD (1782)			
ID= 1 DT=15.0 mi n			
Area (ha)	= 219.91	Curve Number (CN)	= 77.6
Ia (mm)	= 5.00	# of Li near Res. (N)	= 1.50
U. H. Tp (hrs)	= 0.76		

Unit Hyd Qpeak (cms) = 4.921

PEAK FLOW (cms) = 4.976 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 37.944
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.470

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8046)				
1 + 2 = 3				
	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1782):	219.91	4.976	6.75	37.94
+ ID2= 2 (5178):	55.50	4.411	6.25	47.74
=====				
ID = 3 (8046):	275.41	9.089	6.50	39.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0180)			
ID= 1 DT=15.0 mi n			
Area (ha)	= 226.65	Curve Number (CN)	= 65.0
Ia (mm)	= 5.00	# of Li near Res. (N)	= 1.50
U. H. Tp (hrs)	= 1.17		

Unit Hyd Qpeak (cms) = 3.299

PEAK FLOW (cms) = 2.512 (i)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 26.812
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.332

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8050)				
1 + 2 = 3				
	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0180):	226.65	2.512	7.75	26.81
+ ID2= 2 (8046):	275.41	9.089	6.50	39.92
=====				
ID = 3 (8050):	502.06	11.210	6.50	34.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1741)			
ID= 1 DT=15.0 mi n			
Area Total (ha)	= 56.30	Dir. Conn. (%)	= 18.00
Imp (%)	= 35.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 19.70	36.60
Dep. Storage (mm)	= 2.00	5.00
Average Slope (%)	= 0.50	0.50
Length (m)	= 612.64	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr)	= 106.66	59.79
over (mi n)	= 15.00	30.00
Storage Coeff. (mi n)	= 9.09 (ii)	22.24 (ii)
Unit Hyd. Tpeak (mi n)	= 15.00	30.00
Unit Hyd. peak (cms)	= 0.09	0.04

PEAK FLOW (cms)	= 2.60	2.93	*TOTALS*
TIME TO PEAK (hrs)	= 6.00	6.25	4.458 (iii)
RUNOFF VOLUME (mm)	= 78.80	31.06	39.65
TOTAL RAINFALL (mm)	= 80.80	80.80	80.80
RUNOFF COEFFICIENT	= 0.98	0.38	0.49

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 64.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5174)				
IN= 2--> OUT= 1				
DT= 15.0 mi n				
OUTFLOW	STORAGE	OUTFLOW	STORAGE	
(cms)	(ha. m.)	(cms)	(ha. m.)	
0.0000	0.0000	1.1370	1.3070	
0.0580	0.5427	1.4280	1.5354	
0.3290	0.6196	1.6620	1.7593	
0.6130	0.8627	2.0620	1.7693	
0.8510	1.0621	0.0000	0.0000	

INFLOW : ID= 2 (1741)	AREA	OPEAK	TPEAK	R. V.
OUTFLOW: ID= 1 (5174)	(ha)	(cms)	(hrs)	(mm)
56.300	56.300	4.458	6.00	39.65
56.300	56.300	1.050	7.00	39.61

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 23.55
 TIME SHIF T OF PEAK FLOW (mi n) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 1.2370

CALIB NASHYD (1742)			
ID= 1 DT=15.0 mi n			
Area (ha)	= 82.80	Curve Number (CN)	= 71.0
Ia (mm)	= 5.00	# of Li near Res. (N)	= 1.50
U. H. Tp (hrs)	= 0.69		

Unit Hyd Qpeak (cms) = 2.055

PEAK FLOW (cms) = 1.665 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 31.433
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.389

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8044)				
1 + 2 = 3				
	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1742):	82.80	1.665	6.75	31.43
+ ID2= 2 (5174):	56.30	1.050	7.00	39.61
=====				
ID = 3 (8044):	139.10	2.696	7.00	34.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha)= 202.23 Ia (mm)= 5.00 U. H. Tp(hrs)= 0.89	Curve Number (CN)= 78.0 # of Linear Res. (N)= 1.50
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Unit Hyd Qpeak (cms)= 3.875
 PEAK FLOW (cms)= 4.110 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 38.497
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.476

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha)= 247.13 Ia (mm)= 5.00 U. H. Tp(hrs)= 0.75	Curve Number (CN)= 52.0 # of Linear Res. (N)= 1.50
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Unit Hyd Qpeak (cms)= 5.611
 PEAK FLOW (cms)= 2.601 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 18.230
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.226

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha)= 166.91 Ia (mm)= 5.00 U. H. Tp(hrs)= 0.80	Curve Number (CN)= 65.0 # of Linear Res. (N)= 1.50
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Unit Hyd Qpeak (cms)= 3.553
 PEAK FLOW (cms)= 2.489 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 26.647
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.330

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0168):	247.13	2.601	7.00	18.23
+ ID2= 2 (0170):	166.91	2.489	7.00	26.65
ID = 3 (8040):	414.04	5.090	7.00	21.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0172):	202.23	4.110	7.00	38.50
+ ID2= 2 (8040):	414.04	5.090	7.00	21.62
ID = 3 (8042):	616.27	9.200	7.00	27.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6174) IN= 2--> OUT= 1	Routing time step (min)'= 15.00
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<----- DATA FOR SECTION (1741.0) ----->

Distance	Elevation	Manning
0.00	296.00	0.0450
13.36	288.94	0.0450
26.71	288.64	0.0450
51.19	287.96	0.0450
73.45	287.32	0.0450
86.81	286.48	0.0450
102.39	285.08	0.0450
110.73	282.76	0.0450
115.73	281.06	0.0450 / 0.0350
115.74	280.35	0.0350
120.73	280.35	0.0350
120.74	281.10	0.0350 / 0.0450
125.74	282.64	0.0450
135.78	286.87	0.0450
149.13	289.58	0.0450
162.49	292.29	0.0450
175.84	293.95	0.0450
195.88	295.13	0.0450
213.68	296.02	0.0450
220.36	296.09	0.0450

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.71	281.06	.136E+05	6.0	1.69	37.89
1.50	281.85	.358E+05	22.5	2.42	26.43
2.28	282.63	.726E+05	53.8	2.85	22.47
3.07	283.42	.124E+06	104.1	3.24	19.79
3.86	284.21	.189E+06	176.7	3.60	17.81
4.64	284.99	.268E+06	274.9	3.94	16.26
5.43	285.78	.369E+06	390.8	4.07	15.73
6.21	286.56	.501E+06	551.6	4.23	15.15
7.00	287.35	.674E+06	750.5	4.28	14.97
7.79	288.14	.920E+06	993.3	4.15	15.44
8.57	288.92	.126E+07	1361.5	4.14	15.47
9.36	289.71	.167E+07	1986.0	4.56	14.04
10.15	290.50	.210E+07	2739.7	5.02	12.76
10.93	291.28	.254E+07	3615.0	5.47	11.70
11.72	292.07	.300E+07	4608.8	5.91	10.83
12.50	292.85	.347E+07	5693.3	6.30	10.16
13.29	293.64	.397E+07	6892.9	6.67	9.60
14.08	294.43	.450E+07	8150.4	6.97	9.19
14.86	295.21	.506E+07	9525.0	7.23	8.86

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8042)	616.27	9.20	7.00	27.16	0.86	1.80
OUTFLOW: ID= 1 (6174)	616.27	8.48	7.75	27.16	0.83	1.77

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6174):	616.27	8.476	7.75	27.16
+ ID2= 2 (8044):	139.10	2.696	7.00	34.74
ID = 3 (8048):	755.37	10.888	7.50	28.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8048):	755.37	10.888	7.50	28.56
+ ID2= 2 (8050):	502.06	11.210	6.50	34.00
ID = 3 (8052):	1257.43	19.451	7.00	30.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.
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	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8052):	1257.43	19.451	7.00	30.73
+ ID2= 2 (8064):	418.72	13.822	6.25	34.71

ID = 3 (8058):	1676.15	32.073	6.50	31.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area (ha)	Imp(%)	Di r. Conn.(%)
STANDHYD (1762)	162.00	26.00	11.00
ID= 1 DT=15.0 min			

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	42.12	119.88
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1039.23	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	106.66	47.86
over (min)	15.00	30.00
Storage Coeff. (min)=	12.49 (ii)	26.85 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04

PEAK FLOW (cms)=	4.14	6.97	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	8.715 (iii)
RUNOFF VOLUME (mm)=	78.80	26.29	6.25
TOTAL RAINFALL (mm)=	80.80	80.80	32.07
RUNOFF COEFFICIENT =	0.98	0.33	80.80
			0.40

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Imp(%)	Di r. Conn.(%)
STANDHYD (1761)	48.80	50.00	21.00
ID= 1 DT=15.0 min			

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	24.40	24.40
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	570.38	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	106.66	76.51
over (min)	15.00	30.00
Storage Coeff. (min)=	8.71 (ii)	20.62 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.05

PEAK FLOW (cms)=	2.66	2.59	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	4.318 (iii)
RUNOFF VOLUME (mm)=	78.80	31.83	6.00
TOTAL RAINFALL (mm)=	80.80	80.80	41.69
RUNOFF COEFFICIENT =	0.98	0.39	80.80
			0.52

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5176)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1				
DT= 15.0 min				
	0.0000	0.0000	3.2640	1.3175
	0.1650	0.6418	4.0090	1.4908

0.9450	0.7489	4.7720	1.6604
1.7610	0.9426	5.1720	1.6704
2.4420	1.1144	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1761)	48.800	4.318	6.00	41.69
OUTFLOW: ID= 1 (5176)	48.800	1.734	6.75	41.67

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 40.15
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 0.9502

ADD HYD (8056)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (1762):	162.00	8.715	6.25	32.07
+ ID2= 2 (5176):	48.80	1.734	6.75	41.67

ID = 3 (8056):	210.80	9.902	6.25	34.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8066)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (8056):	210.80	9.902	6.25	34.29
+ ID2= 2 (8058):	1676.15	32.073	6.50	31.72

ID = 3 (8066):	1886.95	39.971	6.50	32.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8070)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (8066):	1886.95	39.971	6.50	32.01
+ ID2= 2 (8068):	660.32	47.208	6.00	38.49

ID = 3 (8070):	2547.27	78.882	6.25	33.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6190)	Routing time step (min)
IN= 2--> OUT= 1	15.00

<----- DATA FOR SECTION (1901.0) ----->			
Distance	Elevation	Manning	
0.00	261.34	0.0600	
19.76	261.64	0.0600	
59.28	261.29	0.0600	
79.04	261.09	0.0600	
103.74	255.99	0.0600	
153.13	253.42	0.0600	
192.65	252.78	0.0600	
212.29	249.80	0.0600	
217.29	249.50	0.0600 / 0.0350	Main Channel
217.30	249.05	0.0350	Main Channel
222.29	249.05	0.0350	Main Channel
222.30	249.50	0.0350 / 0.0600	Main Channel
227.30	249.80	0.0600	
232.17	252.31	0.0600	
276.63	253.53	0.0600	
326.03	256.97	0.0600	
355.67	257.30	0.0600	
400.12	260.08	0.0600	
469.28	261.25	0.0600	
489.04	262.53	0.0600	

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.45	249.50	.436E+04	1.0	0.43	75.97

1.07	250.12	.205E+05	5.4	0.51	62.96
1.70	250.75	.451E+05	14.8	0.64	50.78
2.32	251.37	.762E+05	29.2	0.74	43.46
2.94	251.99	.114E+06	49.1	0.84	38.61
3.57	252.62	.161E+06	70.2	0.85	38.18
4.19	253.24	.248E+06	93.4	0.73	44.29
4.81	253.86	.398E+06	154.9	0.76	42.79
5.44	254.49	.576E+06	246.1	0.83	38.99
6.06	255.11	.779E+06	362.7	0.90	35.79
6.68	255.73	.101E+07	506.5	0.98	33.15
7.30	256.35	.126E+07	687.6	1.06	30.52
7.93	256.98	.153E+07	902.8	1.15	28.18
8.55	257.60	.183E+07	1114.3	1.18	27.39
9.17	258.22	.216E+07	1407.8	1.26	25.55
9.80	258.85	.250E+07	1735.4	1.35	24.03
10.42	259.47	.286E+07	2097.5	1.42	22.73
11.04	260.09	.324E+07	2492.3	1.49	21.63
11.67	260.72	.364E+07	2837.8	1.51	21.39

<---- hydrograph ---->						<-pi pe / channel -->	
AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL		
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW : ID= 2 (8070)	2547.27	78.88	6.25	33.69	3.80	0.80	
OUTFLOW : ID= 1 (6190)	2547.27	55.51	6.50	33.69	3.13	0.84	

CALIB STANDHYD (1901)			
ID= 1 DT=15.0 min	Area (ha)= 13.49	Total Imp(%)= 58.00	Di r. Conn.(%)= 31.00

IMPERVIOUS PERVIOUS (i)			
Surface Area (ha)=	7.82	5.67	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	299.89	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	106.66	107.45	
over (min)	15.00	30.00	
Storage Coeff. (min)=	5.92 (ii)	16.32 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.10	0.05	
PEAK FLOW (cms)=	1.17	0.93	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	1.794 (iii)
RUNOFF VOLUME (mm)=	78.80	42.44	6.00
TOTAL RAINFALL (mm)=	80.80	80.80	53.71
RUNOFF COEFFICIENT =	0.98	0.53	80.80
			0.66

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5190)				
IN= 2--> OUT= 1				
DT= 15.0 min				
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)	
0.0000	0.0000	0.9030	0.3849	
0.0460	0.1975	1.1330	0.4289	
0.2610	0.2331	1.3190	0.4720	
0.4870	0.2836	1.7190	0.4820	
0.6750	0.3308	0.0000	0.0000	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
INFLOW : ID= 2 (1901)	13.490	1.794	6.00	53.71
OUTFLOW : ID= 1 (5190)	13.490	0.713	6.50	53.63

PEAK FLOW REDUCTION [Oout/Oin] (%) = 39.72
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 0.3398

CALIB STANDHYD (1902)	
Area (ha)=	274.50

|| ID= 1 DT=15.0 min | Total Imp(%)= 52.00 Di r. Conn.(%)= 28.00

IMPERVIOUS PERVIOUS (i)		
Surface Area (ha)=	142.74	131.76
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1352.77	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	106.66	93.54
over (min)	15.00	30.00
Storage Coeff. (min)=	14.63 (ii)	25.62 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.07	0.04
PEAK FLOW (cms)=	16.76	15.54
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	78.80	40.46
TOTAL RAINFALL (mm)=	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.50
		TOTALS
		26.790 (iii)
		6.00
		51.19
		80.80
		0.63

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9190)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1902):	274.50	26.790	6.00	51.19
+ ID2= 2 (5190):	13.49	0.713	6.50	53.63
ID = 3 (9190):	287.99	26.934	6.00	51.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8072)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6190):	2547.27	55.512	6.50	33.69
+ ID2= 2 (9190):	287.99	26.934	6.00	51.31
ID = 3 (8072):	2835.26	73.708	6.25	35.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6192)	
IN= 2--> OUT= 1	Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1921.0) ----->			
Distance	Elevation	Manning	
0.00	264.40	0.0500	
3.64	263.94	0.0500	
14.54	261.05	0.0500	
29.08	256.78	0.0500	
36.36	254.96	0.0500	
50.90	252.20	0.0500	
65.44	249.78	0.0500	
76.35	246.86	0.0500	
77.24	245.90	0.0500	
82.24	245.45	0.0500 / 0.0350	Main Channel
82.25	244.90	0.0350	Main Channel
87.25	244.90	0.0350	Main Channel
87.26	245.45	0.0350 / 0.0500	Main Channel
94.53	246.89	0.0500	
170.87	247.16	0.0500	
218.13	248.69	0.0500	
265.40	249.22	0.0500	
301.75	250.78	0.0500	
338.11	253.47	0.0500	
359.92	264.00	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.55	245.45	.345E+04	2.0	0.71	29.20
1.53	246.43	.174E+05	14.6	1.05	19.92
2.50	247.40	.767E+05	52.8	0.86	24.20
3.48	248.38	.224E+06	199.8	1.11	18.71
4.46	249.36	.429E+06	441.2	1.29	16.19
5.43	250.33	.691E+06	874.3	1.58	13.18
6.41	251.31	.987E+06	1468.3	1.86	11.20
7.38	252.28	.131E+07	2224.8	2.13	9.79
8.36	253.26	.165E+07	3132.4	2.37	8.78
9.34	254.24	.201E+07	4256.4	2.65	7.87
10.31	255.21	.238E+07	5551.0	2.92	7.15
11.29	256.19	.276E+07	6996.8	3.17	6.57
12.27	257.17	.314E+07	8585.9	3.42	6.10
13.24	258.14	.353E+07	10315.9	3.65	5.71
14.22	259.12	.393E+07	12180.7	3.87	5.38
15.19	260.09	.434E+07	14178.2	4.09	5.10
16.17	261.07	.475E+07	16306.5	4.29	4.85
17.15	262.05	.517E+07	18558.6	4.49	4.64
18.12	263.02	.559E+07	20939.7	4.68	4.45

		<---- hydrograph ---->				<- pi pe / channel -->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW : ID= 2 (8072)	2835.26	73.71	6.25	35.48	2.64	0.89	
OUTFLOW: ID= 1 (6192)	2835.26	60.49	7.00	35.48	2.55	0.87	

CALIB STANDHYD (1921)		Area (ha)= 72.27		Di r. Conn. (%)= 19.00	
ID= 1 DT=15.0 mi n	Total Imp(%)= 42.00				

		IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=		30.35	41.92
Dep. Storage (mm)=		2.00	5.00
Average Slope (%)=		0.50	0.50
Length (m)=		694.12	40.00
Mannings n =		0.013	0.250

Max. Eff. Inten. (mm/hr)=	106.66	58.95	
over (mi n)	15.00	30.00	
Storage Coeff. (mi n)=	15.00 (ii)	23.02 (ii)	
Unit Hyd. Tpeak (mi n)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.09	0.04	

PEAK FLOW (cms)=	3.46	3.25	5.499 (ii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	78.80	27.88	37.56
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.35	0.46

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 57.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5192)		OUTFLOW		STORAGE	
IN= 2--> OUT= 1	DT= 15.0 mi n	(cms)	(ha. m.)	(cms)	(ha. m.)
		0.0000	0.0000	3.1390	1.4346
		0.2060	0.7072	3.8730	1.6637
		0.9550	0.8170	4.8310	1.9184
		1.7940	1.0081	5.2310	1.9284
		2.4250	1.1871	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1921)	72.270	5.499	6.00	37.56
OUTFLOW: ID= 1 (5192)	72.270	2.362	6.75	37.54

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 42.96
 TIME SHIFT OF PEAK FLOW (mi n) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 1.1870

CALIB STANDHYD (1922)		Area (ha)= 174.34		Di r. Conn. (%)= 18.00	
ID= 1 DT=15.0 mi n	Total Imp(%)= 39.00				

Surface Area (ha)=	67.99	PERVIOUS (i)	106.35
Dep. Storage (mm)=	2.00		5.00
Average Slope (%)=	0.50		0.50
Length (m)=	1078.08		40.00
Mannings n =	0.013		0.250

Max. Eff. Inten. (mm/hr)=	106.66	55.16	
over (mi n)	15.00	30.00	
Storage Coeff. (mi n)=	12.77 (ii)	26.34 (ii)	
Unit Hyd. Tpeak (mi n)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.08	0.04	

PEAK FLOW (cms)=	7.23	7.21	11.725 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	78.80	27.12	36.43
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.34	0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 57.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8074)		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3					
ID1= 1 (1922):	174.34	11.725	6.00	36.43	
+ ID2= 2 (5192):	72.27	2.362	6.75	37.54	
=====					
ID = 3 (8074):	246.61	12.126	6.25	36.75	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8076)		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3					
ID1= 1 (6192):	2835.26	60.489	7.00	35.48	
+ ID2= 2 (8074):	246.61	12.126	6.25	36.75	
=====					
ID = 3 (8076):	3081.87	67.752	6.75	35.58	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0196)		Area (ha)= 41.47	Curve Number (CN)= 70.0
ID= 1 DT=15.0 mi n	Ia (mm)= 5.00	# of Li near Res. (N)= 1.50	

Unit Hyd Opeak (cms)= 2.950

PEAK FLOW (cms)=	1.731 (i)
TIME TO PEAK (hrs)=	6.000
RUNOFF VOLUME (mm)=	28.561
TOTAL RAINFALL (mm)=	80.800
RUNOFF COEFFICIENT =	0.353

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1941)		Area (ha)= 0.76		Di r. Conn. (%)= 15.00	
ID= 1 DT=15.0 mi n	Total Imp(%)= 45.00				

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.34	0.42
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	71.18	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	106.66	91.39
over (min)	15.00	15.00
Storage Coeff. (min)=	2.50 (ii)	13.59 (ii)
Unit Hyd. Tpeak (min)=	15.00	15.00
Unit Hyd. peak (cms)=	0.11	0.08
PEAK FLOW (cms)=	0.03	0.08
TIME TO PEAK (hrs)=	6.00	6.00
RUNOFF VOLUME (mm)=	78.80	38.48
TOTAL RAINFALL (mm)=	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.48

TOTALS
0.111 (iii)
6.00
44.52
80.80
0.55

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 67.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5194)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.0510	0.0197
0.0030	0.0093	0.0640	0.0226
0.0150	0.0107	0.0740	0.0253
0.0270	0.0138	0.0740	0.0353
0.0380	0.0165	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1941)	0.760	0.111	6.00	44.52
OUTFLOW: ID= 1 (5194)	0.760	0.034	6.50	43.51

PEAK FLOW REDUCTION [Oout/Oi n] (%) = 30.31
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 0.0157

CALIB NASHYD (1942)
ID= 1 DT=15.0 min

Area (ha) = 255.33 Curve Number (CN) = 69.5
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp (hrs) = 0.68

Unit Hyd Opeak (cms) = 6.448
PEAK FLOW (cms) = 4.975 (i)
TIME TO PEAK (hrs) = 6.750
RUNOFF VOLUME (mm) = 30.123
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.373

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9194)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1942):	255.33	4.975	6.75	30.12
+ ID2= 2 (5194):	0.76	0.034	6.50	43.51
ID = 3 (9194):	256.09	5.005	6.75	30.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8078)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0196):	41.47	1.731	6.00	28.56
+ ID2= 2 (9194):	256.09	5.005	6.75	30.16
ID = 3 (8078):	297.56	6.379	6.50	29.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6198)
IN= 2---> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (1981.0) ----->

Distance	Elevation	Manning
0.00	267.15	0.0500
22.99	265.94	0.0500
51.73	261.39	0.0500
74.72	258.75	0.0500
94.83	257.79	0.0500
114.95	254.36	0.0500
126.44	254.06	0.0500
132.19	253.68	0.0500
135.06	253.35	0.0500
137.94	252.93	0.0500 / 0.0300
140.81	252.41	0.0300
143.69	251.89	0.0300
146.56	252.51	0.0300
149.43	253.36	0.0300 / 0.0500
158.05	255.89	0.0500
172.42	256.25	0.0500
204.03	259.15	0.0500
229.90	260.36	0.0500
252.89	260.46	0.0500
284.50	260.45	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.35	252.24	.111E+04	0.6	0.97	31.07
0.69	252.58	.442E+04	3.8	1.55	19.42
1.04	252.93	.974E+04	11.1	2.06	14.65
1.51	253.40	2.03E+05	31.2	2.77	10.87
1.98	253.87	3.57E+05	65.0	3.30	9.14
2.45	254.34	5.96E+05	112.9	3.43	8.80
2.92	254.81	9.35E+05	186.8	3.62	8.34
3.39	255.28	1.31E+06	282.1	3.89	7.75
3.86	255.75	1.72E+06	398.2	4.18	7.22
4.33	256.22	2.21E+06	527.0	4.31	6.99
4.80	256.69	2.82E+06	697.3	4.47	6.75
5.27	257.16	3.50E+06	898.5	4.64	6.50
5.74	257.63	4.25E+06	1131.4	4.82	6.26
6.21	258.10	5.08E+06	1367.4	4.88	6.19
6.68	258.57	6.03E+06	1635.6	4.91	6.14
7.15	259.04	7.09E+06	1977.2	5.04	5.98
7.62	259.51	8.26E+06	2360.5	5.17	5.83
8.09	259.98	9.54E+06	2790.7	5.29	5.70
8.56	260.45	1.10E+07	3179.6	5.25	5.74

<---- hydrograph ----> <- pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8078)	297.56	6.38	6.50	29.94	0.82	1.70
OUTFLOW: ID= 1 (6198)	297.56	6.14	6.75	29.94	0.80	1.69

CALIB STANDHYD (1981)
ID= 1 DT=15.0 min

Area (ha) = 149.95
Total Imp (%) = 44.00
Di r. Conn. (%) = 19.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	65.98	83.97
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	999.83	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr) = 106.66
over (min) = 15.00
Storage Coeff. (min) = 12.20 (ii)
25.04 (ii)

Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

PEAK FLOW (cms)= 6.67 6.71
 TIME TO PEAK (hrs)= 6.00 6.25
 RUNOFF VOLUME (mm)= 78.80 28.88
 TOTAL RAINFALL (mm)= 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.36

TOTALS
 10.891 (iii)
 6.00
 38.36
 80.80
 0.47

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 57.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5198) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	10.9910	3.4957
	0.4390	1.7800	13.3530	3.9512
	2.6180	2.0449	15.3540	4.3979
	5.1680	2.5932	15.7540	4.4079
	7.9230	3.0040	0.0000	0.0000

INFLOW : ID= 2 (1981) 149.950 10.891 6.00 38.36
 OUTFLOW: ID= 1 (5198) 149.950 4.924 6.75 38.36

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 45.21
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 2.5648

CALIB STANDHYD (1982) ID= 1 DT=15.0 min	Area (ha)	IMPERVIOUS (%)	PERVIOUS (i)	Dir. Conn. (%)
	240.61	20.00	192.49	9.00
Surface Area (ha)=	48.12		192.49	
Dep. Storage (mm)=	2.00		5.00	
Average Slope (%)=	0.50		0.50	
Length (m)=	1266.52		40.00	
Mannings n =	0.013		0.250	
Max. Eff. Inten. (mm/hr)=	106.66		25.36	
over (min)=	15.00		45.00	
Storage Coeff. (min)=	14.06 (ii)		32.58 (ii)	
Unit Hyd. Tpeak (min)=	15.00		45.00	
Unit Hyd. peak (cms)=	0.07		0.03	

TOTALS
 9.390 (iii)
 6.50
 29.09
 80.80
 0.36

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 57.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8080) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1982):	240.61	9.390	6.50	29.09
+ ID2= 2 (5198):	149.95	4.924	6.75	38.36

ID = 3 (8080): 390.56 14.052 6.50 32.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8082) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6198):	297.56	6.137	6.75	29.94
+ ID2= 2 (8080):	390.56	14.052	6.50	32.65
ID = 3 (8082):	688.12	19.849	6.50	31.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8084) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8076):	3081.87	67.752	6.75	35.58
+ ID2= 2 (8082):	688.12	19.849	6.50	31.48
ID = 3 (8084):	3769.99	85.860	6.75	34.83

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6200)
 IN= 2---> OUT= 1
 Routing time step (min)' = 15.00

Distance	Elevation	Manning
0.00	252.47	0.0500
10.38	252.29	0.0500
20.76	252.28	0.0500
41.52	247.66	0.0500
62.28	247.47	0.0500
80.45	247.34	0.0500
90.83	246.85	0.0500
103.81	245.51	0.0500
106.00	244.50	0.0500
108.99	244.05	0.0500 / 0.0300
109.00	243.50	0.0300
114.00	243.50	0.0300
114.01	244.05	0.0300 / 0.0500
118.99	244.50	0.0500
132.36	245.09	0.0500
150.52	245.83	0.0500
199.83	249.24	0.0500
207.62	250.16	0.0500
236.16	253.11	0.0500
256.92	253.83	0.0500

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.28	243.77	.365E+04	0.7	0.54	81.42
0.55	244.05	.730E+04	2.2	0.81	54.59
1.05	244.55	.197E+05	7.2	0.97	45.51
1.54	245.04	.463E+05	17.2	0.99	44.76
2.04	245.54	.895E+05	34.8	1.03	42.81
2.53	246.03	.152E+06	62.8	1.09	40.43
3.03	246.53	.232E+06	104.7	1.20	36.91
3.52	247.02	.328E+06	159.7	1.29	34.19
4.02	247.52	.450E+06	223.4	1.32	33.56
4.51	248.01	.627E+06	326.9	1.38	31.99
5.01	248.51	.820E+06	463.0	1.50	29.52
5.50	249.00	.103E+07	626.0	1.62	27.30
6.00	249.50	.124E+07	820.1	1.75	25.24
6.49	249.99	.147E+07	1044.4	1.89	23.42
6.99	250.49	.170E+07	1292.6	2.01	21.95
7.48	250.98	.195E+07	1565.6	2.13	20.71
7.98	251.48	.220E+07	1864.5	2.25	19.65
8.47	251.97	.246E+07	2189.4	2.36	18.73
8.97	252.47	.274E+07	2427.5	2.35	18.80

<--- hydrograph ---> <--- pipe / channel --->
 AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)

INFLOW : ID= 2 (8084) 3769.99 85.86 6.75 34.83 2.80 1.15
 OUTFLOW: ID= 1 (6200) 3769.99 70.39 7.25 34.83 2.62 1.11

CALIB
 STANDHYD (2001)
 ID= 1 DT=15.0 min

Area (ha)	= 42.48	Dir. Conn. (%)	= 19.00
Total Imp (%)	= 46.00		

Surface Area (ha)	= 19.54	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)	= 2.00		
Average Slope (%)	= 0.50		
Length (m)	= 532.17		
Mannings n	= 0.013		
Max. Eff. Inten. (mm/hr)	= 106.66		
over (min)	= 15.00		
Storage Coeff. (min)	= 8.36 (ii)		19.32 (ii)
Unit Hyd. Tpeak (min)	= 15.00		30.00
Unit Hyd. peak (cms)	= 0.09		0.05
PEAK FLOW (cms)	= 2.12		3.09
TIME TO PEAK (hrs)	= 6.00		4.142 (iii)
RUNOFF VOLUME (mm)	= 78.80		47.89
TOTAL RAINFALL (mm)	= 80.80		80.80
RUNOFF COEFFICIENT	= 0.98		0.59

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.7 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5200)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.7210	1.0525
0.1450	0.4892	3.4130	1.1947
0.7930	0.5722	4.0030	1.3346
1.4410	0.7540	4.4030	1.3446
2.0370	0.8927	0.0000	0.0000

INFLOW : ID= 2 (2001)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5200)	42.480	4.142	6.00	47.89
	42.480	1.982	6.50	47.87

PEAK FLOW REDUCTION [Qout/Qin] (%) = 47.84
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 0.8950

CALIB
 STANDHYD (2002)
 ID= 1 DT=15.0 min

Area (ha)	= 72.79	Dir. Conn. (%)	= 13.00
Total Imp (%)	= 31.00		

Surface Area (ha)	= 22.56	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)	= 2.00		
Average Slope (%)	= 0.50		
Length (m)	= 696.61		
Mannings n	= 0.013		
Max. Eff. Inten. (mm/hr)	= 106.66		
over (min)	= 15.00		
Storage Coeff. (min)	= 9.82 (ii)		22.05 (ii)
Unit Hyd. Tpeak (min)	= 15.00		30.00
Unit Hyd. peak (cms)	= 0.09		0.04
PEAK FLOW (cms)	= 2.38		4.85
TIME TO PEAK (hrs)	= 6.00		5.661 (iii)
RUNOFF VOLUME (mm)	= 78.80		42.28
TOTAL RAINFALL (mm)	= 80.80		80.80
RUNOFF COEFFICIENT	= 0.98		0.52

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.7 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8086)
 1 + 2 = 3

ID1= 1 (2002):	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
+ ID2= 2 (5200):	42.48	1.982	6.50	47.87
ID = 3 (8086):	115.27	7.146	6.25	44.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8088)
 1 + 2 = 3

ID1= 1 (6200):	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
+ ID2= 2 (8086):	115.27	7.146	6.25	44.34
ID = 3 (8088):	3885.26	73.095	7.25	35.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8038)
 1 + 2 = 3

ID1= 1 (8034):	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
+ ID2= 2 (8088):	3885.26	73.095	7.25	35.11
ID = 3 (8038):	9106.02	100.696	7.25	27.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (1365)
 IN= 2---> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1362.0) ----->

Distance	Elevation	Manning
0.00	252.00	0.0400
2.35	244.60	0.0400
39.94	243.14	0.0400
58.74	242.65	0.0400
91.63	242.03	0.0400
101.03	241.69	0.0400
103.38	241.66	0.0400
105.73	240.76	0.0400
105.93	240.59	0.0400
109.18	240.28	0.0400 / 0.0300
110.43	239.81	0.0300
111.68	239.81	0.0300
113.43	240.09	0.0300 / 0.0400
118.43	240.50	0.0400
138.62	241.68	0.0400
152.72	241.60	0.0400
166.82	242.68	0.0400
185.62	249.09	0.0400
197.36	251.00	0.0400
232.61	252.28	0.0400

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (Cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.28	240.09	.118E+04	0.1	0.20	142.09
0.91	240.72	.114E+05	2.4	0.35	79.92
1.53	241.34	.352E+05	9.4	0.45	62.17

2.16	241.97	.831E+05	22.8	0.46	60.69
2.79	242.60	.172E+06	54.2	0.53	52.74
3.41	243.22	.297E+06	111.4	0.63	44.49
4.04	243.85	.445E+06	194.1	0.74	38.22
4.67	244.48	.612E+06	301.1	0.83	33.88
5.29	245.10	.793E+06	443.0	0.94	29.82
5.92	245.73	.976E+06	611.3	1.06	26.60
6.55	246.36	.116E+07	802.0	1.17	24.12
7.18	246.99	.135E+07	1014.0	1.27	22.16
7.80	247.61	.154E+07	1246.2	1.37	20.56
8.43	248.24	.173E+07	1498.0	1.46	19.24
9.06	248.87	.192E+07	1768.8	1.55	18.12
9.68	249.49	.212E+07	2049.6	1.63	17.24
10.31	250.12	.232E+07	2344.7	1.71	16.49
10.94	250.75	.252E+07	2658.5	1.78	15.83
11.56	251.37	.274E+07	2931.2	1.81	15.56

<---- hydrograph ---->						
<-pi pe / channel -->						
	AREA	QPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW: ID= 2 (8038)	9106.02	100.70	7.25	27.66	3.30	0.61
OUTFLOW: ID= 1 (1365)	9106.02	84.06	8.00	27.66	3.12	0.58

CALIB STANDHYD (1321) ID= 1 DT=15.0 min			
Area (ha)	= 208.90		
Total Imp(%)	= 67.00	Di r. Conn. (%)	= 46.00

Surface Area (ha)	= 139.96	IMPERVIOUS	68.94	PERVIOUS (i)	
Dep. Storage (mm)	= 2.00		5.00		
Average Slope (%)	= 0.50		0.50		
Length (m)	= 1180.11		40.00		
Mannings n	= 0.013		0.250		

Max. Eff. Inten. (mm/hr)	= 106.66	over (min)	15.00	92.20	30.00
Storage Coeff. (min)	= 13.48	(ii)	24.53	(ii)	
Unit Hyd. Tpeak (min)	= 15.00		30.00		
Unit Hyd. peak (cms)	= 0.08		0.04		

PEAK FLOW (cms)	= 21.67	8.16	*TOTALS*	
TIME TO PEAK (hrs)	= 6.00	6.25	26.904	(iii)
RUNOFF VOLUME (mm)	= 78.80	36.79	56.11	
TOTAL RAINFALL (mm)	= 80.80	80.80	80.80	
RUNOFF COEFFICIENT	= 0.98	0.46	0.69	

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5132) IN= 2--> OUT= 1 DT= 15.0 min			
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	7.9970	7.9437
0.9210	5.2645	12.4650	8.5839
3.2690	5.3433	16.9060	9.2236
6.3980	5.6925	17.3060	9.2336
7.1860	6.8155	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1321)	208.900	26.904	6.00
OUTFLOW: ID= 1 (5132)	208.900	6.970	56.11

PEAK FLOW REDUCTION [Oout/Oi n] (%) = 25.91
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha.m.) = 6.5192

CALIB STANDHYD (1322) ID= 1 DT=15.0 min			
Area (ha)	= 81.94		
Total Imp(%)	= 37.00	Di r. Conn. (%)	= 25.00

IMPERVIOUS PERVIOUS (i)			
Surface Area (ha)	= 30.32	51.62	
Dep. Storage (mm)	= 2.00	5.00	
Average Slope (%)	= 0.50	0.50	
Length (m)	= 739.10	40.00	
Mannings n	= 0.013	0.250	
Max. Eff. Inten. (mm/hr)	= 106.66	54.46	
over (min)	= 15.00	30.00	
Storage Coeff. (min)	= 10.18	(ii)	23.82 (ii)
Unit Hyd. Tpeak (min)	= 15.00	30.00	
Unit Hyd. peak (cms)	= 0.09	0.04	
PEAK FLOW (cms)	= 5.10	3.64	*TOTALS*
TIME TO PEAK (hrs)	= 6.00	6.25	7.384 (iii)
RUNOFF VOLUME (mm)	= 78.80	30.00	42.20
TOTAL RAINFALL (mm)	= 80.80	80.80	80.80
RUNOFF COEFFICIENT	= 0.98	0.37	0.52

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8090) 1 + 2 = 3				
	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1322):	81.94	7.384	6.00	42.20
+ ID2= 2 (5132):	208.90	6.970	6.75	56.11
ID= 3 (8090):	290.84	10.422	6.50	52.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1342) ID= 1 DT=15.0 min			
Area (ha)	= 97.16		
Total Imp(%)	= 41.00	Di r. Conn. (%)	= 28.00

Surface Area (ha)	= 39.84	IMPERVIOUS	57.32	PERVIOUS (i)	
Dep. Storage (mm)	= 2.00		5.00		
Average Slope (%)	= 0.50		0.50		
Length (m)	= 804.82		40.00		
Mannings n	= 0.013		0.250		

Max. Eff. Inten. (mm/hr)	= 106.66	over (min)	15.00	43.36	30.00
Storage Coeff. (min)	= 10.71	(ii)	25.66	(ii)	
Unit Hyd. Tpeak (min)	= 15.00		30.00		
Unit Hyd. peak (cms)	= 0.09		0.04		

PEAK FLOW (cms)	= 6.66	3.09	*TOTALS*	
TIME TO PEAK (hrs)	= 6.00	6.25	8.572	(iii)
RUNOFF VOLUME (mm)	= 78.80	23.62	39.07	
TOTAL RAINFALL (mm)	= 80.80	80.80	80.80	
RUNOFF COEFFICIENT	= 0.98	0.29	0.48	

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 54.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1341) ID= 1 DT=15.0 min			
Area (ha)	= 58.79		
Total Imp(%)	= 30.00	Di r. Conn. (%)	= 20.00

Surface Area (ha)	= 17.64	IMPERVIOUS	41.15	PERVIOUS (i)	
Dep. Storage (mm)	= 2.00		5.00		
Average Slope (%)	= 0.50		0.50		
Length (m)	= 626.05		40.00		

Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 106.66 23.48
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 9.21 (ii) 28.31 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.04
 TOTALS
 PEAK FLOW (cms)= 3.01 1.86 4.158 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 78.80 22.42 33.70
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.28 0.42

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 54.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5134)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.1250	1.0755
0.1070	0.6119	1.7570	1.1641
0.4130	0.6225	2.3860	1.2526
0.8380	0.7018	2.7860	1.2626
0.9790	0.8884	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
58.790	4.158	6.00	33.70
58.790	1.062	7.00	33.67

INFLOW : ID= 2 (1341)
 OUTFLOW: ID= 1 (5134)

PEAK FLOW REDUCTION [Qout/Qin] (%) = 25.53
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 0.9962

ADD HYD (9134)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1342):	97.16	8.572	6.00	39.07
+ ID2= 2 (5134):	58.79	1.062	7.00	33.67
ID = 3 (9134):	155.95	8.664	6.00	37.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8092)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8090):	290.84	10.422	6.50	52.19
+ ID2= 2 (9134):	155.95	8.664	6.00	37.04
ID = 3 (8092):	446.79	16.689	6.00	46.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (1366)
 IN= 2--> OUT= 1
 Routing time step (min) = 15.00

<----- DATA FOR SECTION (1361.0) ----->

Distance	Elevation	Manning
0.00	262.81	0.0400
37.23	260.54	0.0400
79.12	256.98	0.0400
111.70	254.74	0.0400
172.20	252.76	0.0400
251.32	248.82	0.0400

255.97	248.75	0.0400
260.43	248.76	0.0400
265.43	248.49	0.0400
268.68	248.18	0.0400 / 0.0300
269.93	247.71	0.0300
271.18	247.71	0.0300
272.93	247.99	0.0300
277.93	248.40	0.0300 / 0.0400
283.90	248.88	0.0400
307.17	249.60	0.0400
321.13	251.59	0.0400
363.01	257.08	0.0400
400.25	257.58	0.0400
460.75	261.91	0.0400

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.47	248.18	.881E+04	1.3	0.76	114.66
1.19	248.90	.742E+05	17.3	1.22	71.44
1.92	249.63	.278E+06	78.3	1.48	59.26
2.64	250.35	.589E+06	210.3	1.87	46.69
3.36	251.07	.974E+06	414.3	2.23	39.19
4.08	251.79	.143E+07	696.2	2.55	34.32
4.81	252.52	.197E+07	1063.0	2.83	30.86
5.53	253.24	.259E+07	1505.7	3.06	28.62
6.25	253.96	.331E+07	2051.3	3.26	26.85
6.97	254.68	.413E+07	2720.9	3.46	25.30
7.70	255.41	.504E+07	3584.5	3.73	23.43
8.42	256.13	.601E+07	4580.9	4.00	21.87
9.14	256.85	.704E+07	5707.9	4.25	20.57
9.86	257.57	.818E+07	6636.1	4.26	20.53
10.59	258.30	.945E+07	8115.0	4.50	19.42
11.31	259.02	.108E+08	9759.2	4.74	18.45
12.03	259.74	.122E+08	11569.5	4.96	17.61
12.75	260.46	.137E+08	13550.6	5.18	16.87
13.48	261.19	.153E+08	15636.4	5.37	16.29

<---- hydrograph ----> <- pi pe / channel ->

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
446.79	16.69	6.00	46.90	1.16	1.20
446.79	8.82	7.50	46.90	0.81	0.93

INFLOW : ID= 2 (8092)
 OUTFLOW: ID= 1 (1366)

CALIB
 STANDHYD (1362)
 ID= 1 DT=15.0 min
 Area (ha) = 371.79
 Total Imp (%) = 26.00
 Di r. Conn. (%) = 14.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	96.67 275.12
Dep. Storage (mm)=	2.00 5.00
Average Slope (%)=	0.50 0.50
Length (m)=	1574.36 40.00
Mannings n =	0.013 0.250

Max. Eff. Inten. (mm/hr)= 106.66 53.45
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 16.02 (ii) 29.77 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.07 0.04

TOTALS
 PEAK FLOW (cms)= 10.91 16.93 22.492 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.25
 RUNOFF VOLUME (mm)= 78.80 30.14 36.95
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.37 0.46

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (1361)
 ID= 1 DT=15.0 min
 Area (ha) = 140.62
 Total Imp (%) = 55.00
 Di r. Conn. (%) = 29.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	77.34	63.28
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	968.23	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	106.66	88.68
over (min)	15.00	30.00
Storage Coeff. (min)=	11.97 (ii)	23.19 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04
	TOTALS	
PEAK FLOW (cms)=	9.62	7.40
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	78.80	36.68
TOTAL RAINFALL (mm)=	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.45
		14.370 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5136)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.7060	4.3486
0.3100	2.4365	5.9200	5.0324
1.7670	2.5993	7.0820	5.7151
3.4020	2.8510	7.4820	5.7251
4.0310	3.5962	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1361)	140.620	14.370	6.00	48.89
OUTFLOW: ID= 1 (5136)	140.620	4.023	7.00	48.88
PEAK FLOW REDUCTION [Qout/Qin] (%) =	27.99			
TIME SHIFT OF PEAK FLOW (min) =	60.00			
MAXIMUM STORAGE USED (ha. m.) =	3.6177			

ADD HYD (8096)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1362):	371.79	22.492	6.25	36.95
+ ID2= 2 (5136):	140.62	4.023	7.00	48.88

ID = 3 (8096):	512.41	25.912	6.25	40.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8094)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1366):	446.79	8.816	7.50	46.90
+ ID2= 2 (8096):	512.41	25.912	6.25	40.22

ID = 3 (8094):	959.20	31.109	6.25	43.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8098)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1365):	9106.02	84.063	8.00	27.66
+ ID2= 2 (8094):	959.20	31.109	6.25	43.33

ID = 3 (8098): 10065.22 100.447 7.75 29.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANHYD (1381)
ID= 1 DT=15.0 min

Area (ha)=	96.24		
Total Imp(%)=	59.00	Dir. Conn.(%)=	34.00
	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	56.78	39.46	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	801.00	40.00	
Mannings n =	0.013	0.250	

Max. Eff. Inten. (mm/hr)=	106.66	17.89
over (min)	15.00	45.00
Storage Coeff. (min)=	10.68 (ii)	31.98 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.09	0.03

	TOTALS	
PEAK FLOW (cms)=	8.02	1.19
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	78.80	12.54
TOTAL RAINFALL (mm)=	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.16
		8.504 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 28.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5138)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	6.4380	2.7648
0.3260	1.4281	8.0830	3.0750
1.8650	1.6876	9.4100	3.3790
3.4730	2.0447	9.8100	3.3890
4.8170	2.3809	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1381)	96.240	8.504	6.00	35.07
OUTFLOW: ID= 1 (5138)	96.240	2.259	6.75	35.06

PEAK FLOW REDUCTION [Qout/Qin] (%) =	26.56		
TIME SHIFT OF PEAK FLOW (min) =	45.00		
MAXIMUM STORAGE USED (ha. m.) =	1.7956		

CALIB NASHYD (1382)
ID= 1 DT=15.0 min

Area (ha)=	318.55	Curve Number (CN)=	33.2
Ia (mm)=	5.00	# of Linear Res. (N)=	1.50
U. H. Tp(hrs)=	1.09		

Unit Hyd Opeak (cms)=	4.976
-----------------------	-------

PEAK FLOW (cms)=	1.309 (i)
TIME TO PEAK (hrs)=	7.750
RUNOFF VOLUME (mm)=	9.703
TOTAL RAINFALL (mm)=	80.800
RUNOFF COEFFICIENT =	0.120

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9138)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1382):	318.55	1.309	7.75	9.70
+ ID2= 2 (5138):	96.24	2.259	6.75	35.06

ID = 3 (9138): 414.79 3.461 6.75 15.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1402) ID= 1 DT=15.0 min		Area (ha)= 131.22 Total Imp(%)= 29.00	Dir. Conn. (%)= 21.00
		IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	38.05	93.17	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	935.31	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	106.66	12.70	
over (min)	15.00	45.00	
Storage Coeff. (min)=	11.72 (ii)	36.14 (ii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.08	0.03	
		TOTALS (iii)	
PEAK FLOW (cms)=	6.55	1.87	7.298
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	78.80	12.88	26.73
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.16	0.33

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 37.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1401) ID= 1 DT=15.0 min		Area (ha)= 27.37 Total Imp(%)= 45.00	Dir. Conn. (%)= 33.00
		IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	12.32	15.05	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	427.16	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	106.66	15.19	
over (min)	15.00	45.00	
Storage Coeff. (min)=	7.33 (ii)	30.06 (ii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.10	0.03	
		TOTALS (iii)	
PEAK FLOW (cms)=	2.44	0.40	2.605
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	78.80	14.01	35.39
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.17	0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 37.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5140) IN= 2----> OUT= 1 DT= 15.0 min			
OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	1.8310	0.7127
0.0930	0.3344	2.2990	0.8148
0.5300	0.3869	2.6760	0.9148
0.9880	0.4994	3.0760	0.9248
1.3700	0.5962	0.0000	0.0000
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)

INFLOW : ID= 2 (1401) 27.370 2.605 6.00 35.39
OUTFLOW: ID= 1 (5140) 27.370 0.820 6.50 35.36

PEAK FLOW REDUCTION [Qout/Qin] (%) = 31.47
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 0.4592

ADD HYD (9140) 1 + 2 = 3		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1402):	131.22	7.298	6.00	26.73	
+ ID2= 2 (5140):	27.37	0.820	6.50	35.36	
ID = 3 (9140):	158.59	7.402	6.00	28.22	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8102) 1 + 2 = 3		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9138):	414.79	3.461	6.75	15.59	
+ ID2= 2 (9140):	158.59	7.402	6.00	28.22	
ID = 3 (8102):	573.38	8.363	6.00	19.08	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6142)
IN= 2----> OUT= 1
Routing time step (min)' = 15.00

DATA FOR SECTION (1421.0) ----->		
Distance	Elevation	Manning
0.00	295.50	0.0400
7.25	295.36	0.0400
14.50	295.22	0.0400
21.75	295.22	0.0400
29.00	294.23	0.0400
52.56	286.45	0.0400
67.06	281.44	0.0400
77.94	278.28	0.0400
82.81	277.20	0.0400 / 0.0300
83.21	276.80	0.0300
88.81	276.80	0.0300
96.41	276.80	0.0300
96.81	277.20	0.0300 / 0.0400
103.31	278.96	0.0400
108.75	280.16	0.0400
117.81	282.73	0.0400
135.94	290.30	0.0400
159.50	292.14	0.0400
174.00	293.07	0.0400
179.44	293.37	0.0400

TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.40	277.20	.238E+05	6.9	1.27	57.28
1.25	278.05	.889E+05	50.9	2.50	29.13
2.10	278.90	.179E+06	133.4	3.26	22.36
2.95	279.75	.293E+06	256.7	3.83	19.03
3.80	280.60	.432E+06	426.0	4.31	16.90
4.66	281.46	.593E+06	644.3	4.75	15.35
5.51	282.31	.776E+06	915.3	5.16	14.12
6.36	283.16	.978E+06	1242.2	5.55	13.12
7.21	284.01	.120E+07	1626.2	5.94	12.27
8.06	284.86	.143E+07	2065.6	6.30	11.57
8.91	285.71	.169E+07	2562.4	6.64	10.97
9.76	286.56	.196E+07	3118.0	6.97	10.46
10.61	287.41	.224E+07	3733.3	7.28	10.02
11.46	288.26	.255E+07	4411.9	7.57	9.62
12.31	289.11	.287E+07	5155.6	7.86	9.27
13.17	289.97	.321E+07	5966.6	8.14	8.96
14.02	290.82	.357E+07	6705.0	8.22	8.87
14.87	291.67	.398E+07	7494.3	8.24	8.85
15.72	292.52	.444E+07	8405.3	8.28	8.80

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8102)	573.38	8.36	6.00	19.08	0.43	1.29
OUTFLOW: ID= 1 (6142)	573.38	5.25	7.25	19.08	0.30	1.27

CALIB
STANDHYD (1421)
ID= 1 DT=15.0 min

Area (ha)= 69.77
Total Imp(%)= 45.00 Di r. Conn.(%)= 15.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	31.40	38.37
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	682.01	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	106.66	73.73
over (min)	15.00	30.00
Storage Coeff. (min)=	9.70 (ii)	21.78 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.05

TOTALS
PEAK FLOW (cms)= 2.64 3.83
TIME TO PEAK (hrs)= 6.00 6.25
RUNOFF VOLUME (mm)= 78.80 31.37
TOTAL RAINFALL (mm)= 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.39

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5142) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	4.6670	1.8169
	0.2360	0.8525	5.8600	2.0773
	1.3520	0.9863	6.8220	2.3321
	2.5180	1.2732	7.2220	2.3421
	3.4920	1.5199	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1421)	69.770	5.077	6.00	38.49
OUTFLOW: ID= 1 (5142)	69.770	2.261	6.75	38.47

PEAK FLOW REDUCTION [Qout/Qin] (%) = 44.54
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha.m.) = 1.2181

CALIB
NASHYD (1422)
ID= 1 DT=15.0 min

Area (ha)= 326.77
Ia (mm)= 5.00
U.H. Tp(hrs)= 1.21

Curve Number (CN)= 61.2
of Linear Res. (N)= 1.50

Unit Hyd Qpeak (cms)=	4.626
PEAK FLOW (cms)=	3.170 (i)
TIME TO PEAK (hrs)=	7.750
RUNOFF VOLUME (mm)=	24.073
TOTAL RAINFALL (mm)=	80.800
RUNOFF COEFFICIENT =	0.298

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9142)

1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1422):	326.77	3.170	7.75	24.07
+ ID2= 2 (5142):	69.77	2.261	6.75	38.47
ID = 3 (9142):	396.54	5.133	7.00	26.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8104) 1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6142):	573.38	5.250	7.25	19.08
+ ID2= 2 (9142):	396.54	5.133	7.00	26.61
ID = 3 (8104):	969.92	10.342	7.00	22.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (1441)
ID= 1 DT=15.0 min

Area (ha)= 6.29
Total Imp(%)= 45.00 Di r. Conn.(%)= 15.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	2.83	3.46
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	204.78	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	106.66	95.87
over (min)	15.00	30.00
Storage Coeff. (min)=	4.71 (ii)	15.59 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.11	0.05

TOTALS
PEAK FLOW (cms)= 0.27 0.52 0.613 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 78.80 40.28 46.06
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.50 0.57

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5144) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	0.4210	0.1639
	0.0210	0.0769	0.5290	0.1874
	0.1220	0.0890	0.6150	0.2104
	0.2270	0.1148	1.0150	0.2204
	0.3150	0.1371	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1441)	6.290	0.613	6.00	46.06
OUTFLOW: ID= 1 (5144)	6.290	0.285	6.50	45.92

PEAK FLOW REDUCTION [Qout/Qin] (%) = 46.39
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha.m.) = 0.1313

CALIB
NASHYD (1442)
ID= 1 DT=15.0 min

Area (ha)= 129.40
Ia (mm)= 5.00
U.H. Tp(hrs)= 0.70

Curve Number (CN)= 74.9
of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms) = 3.147

PEAK FLOW (cms) = 2.884 (i)
TIME TO PEAK (hrs) = 6.750
RUNOFF VOLUME (mm) = 35.090
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.434

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1442):	129.40	2.884	6.75	35.09
+ ID2= 2 (5144):	6.29	0.285	6.50	45.92

ID = 3 (9144):	135.69	3.163	6.75	35.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8104):	969.92	10.342	7.00	22.16
+ ID2= 2 (9144):	135.69	3.163	6.75	35.59

ID = 3 (8106):	1105.61	13.439	7.00	23.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6146)
IN= 2----> OUT= 1
Routing time step (min) = 15.00

----- DATA FOR SECTION (1461.0) -----

Distance	Elevation	Manning	
0.00	262.00	0.0350	
39.58	257.13	0.0350	
123.69	257.51	0.0350	
183.07	256.83	0.0350	
202.86	255.73	0.0350	
212.75	254.73	0.0350	
222.65	252.86	0.0350	
232.44	250.70	0.0350 / 0.0300	Main Channel
236.44	250.20	0.0300	Main Channel
236.84	249.80	0.0300	Main Channel
242.44	249.80	0.0300	Main Channel
250.04	249.80	0.0300	Main Channel
250.44	250.20	0.0300	Main Channel
252.33	250.57	0.0300 / 0.0350	Main Channel
286.97	253.52	0.0350	
331.50	255.80	0.0350	
380.98	255.55	0.0350	
445.30	258.40	0.0350	
455.19	259.82	0.0350	
489.83	262.22	0.0350	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.39	250.19	.169E+05	5.4	1.04	52.00
0.77	250.57	.373E+05	16.6	1.44	37.46
1.44	251.24	.911E+05	60.3	2.14	25.19
2.11	251.91	.169E+06	136.3	2.61	20.64
2.79	252.59	.270E+06	249.7	2.99	18.05
3.46	253.26	.396E+06	405.4	3.31	16.28
4.13	253.93	.549E+06	603.3	3.56	15.15
4.80	254.60	.737E+06	857.8	3.77	14.31
5.48	255.28	.963E+06	1174.6	3.95	13.67
6.15	255.95	.128E+07	1449.5	3.66	14.74
6.82	256.62	.173E+07	2046.4	3.83	14.07
7.49	257.29	.227E+07	2665.2	3.81	14.17
8.17	257.97	.310E+07	3705.2	3.87	13.93
8.84	258.64	.400E+07	5127.4	4.15	12.99
9.51	259.31	.492E+07	6858.6	4.51	11.95
10.18	259.98	.587E+07	8815.5	4.86	11.09

10.86 260.66 .684E+07 10954.6 5.18 10.41
11.53 261.33 .785E+07 13324.3 5.49 9.82
12.20 262.00 .890E+07 15923.2 5.79 9.31

<---- hydrograph ----> <-pi pe / channel ->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8106) 1105.61 13.44 7.00 23.81 0.66 1.30
 OUTFLOW: ID= 1 (6146) 1105.61 11.69 7.75 23.81 0.60 1.23

CALIB
 STANDHYD (1462)
 ID= 1 DT=15.0 min
 Area (ha) = 112.22
 Total Imp(%) = 32.00 Dir. Conn.(%) = 13.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 35.91 76.31
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 864.95 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 106.66 77.56
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 11.19 (ii) 23.03 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.08 0.04

TOTALS
 PEAK FLOW (cms) = 3.52 7.84 9.195 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.25
 RUNOFF VOLUME (mm) = 78.80 39.23 44.38
 TOTAL RAINFALL (mm) = 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.49 0.55

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (1461)
 ID= 1 DT=15.0 min
 Area (ha) = 83.29
 Total Imp(%) = 38.00 Dir. Conn.(%) = 16.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 31.65 51.64
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 745.16 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 106.66 84.79
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 10.23 (ii) 21.66 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.09 0.05

TOTALS
 PEAK FLOW (cms) = 3.31 5.97 7.189 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 78.80 40.50 46.63
 TOTAL RAINFALL (mm) = 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.50 0.58

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| RESERVOIR (5146) |

IN= 2----> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	8.9200	2.5813
0.2820	0.8782	11.0570	3.0328
2.5360	1.0372	12.9860	3.4993
4.7880	1.7148	13.3860	3.5093
6.7570	2.1134	0.0000	0.0000

INFLOW : ID= 2 (1461)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
83.290	83.290	7.189	6.00	46.63
OUTFLOW: ID= 1 (5146)	83.290	4.081	6.50	46.62

PEAK FLOW REDUCTION [Out/Oi n] (%) = 56.77
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 1.5279

ADD HYD (8099)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1462):	112.22	9.195	6.25	44.38
+ ID2= 2 (5146):	83.29	4.081	6.50	46.62
=====				
ID = 3 (8099):	195.51	12.636	6.25	45.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8108)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6146):	1105.61	11.693	7.75	23.81
+ ID2= 2 (8099):	195.51	12.636	6.25	45.33
=====				
ID = 3 (8108):	1301.12	16.094	7.25	27.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8100)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8098):	10065.22	100.447	7.75	29.15
+ ID2= 2 (8108):	1301.12	16.094	7.25	27.04
=====				
ID = 3 (8100):	11366.34	115.330	7.75	28.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6202)
IN= 2----> OUT= 1

Routing time step (min) = 15.00

----- DATA FOR SECTION (2021.0) ----->

Di stance	Elevation	Manning	
0.00	245.48	0.0400	
29.68	242.80	0.0400	
74.20	240.39	0.0400	
155.81	237.12	0.0400	
200.33	234.48	0.0400	
244.85	234.67	0.0400	
259.69	233.96	0.0400	
274.53	233.84	0.0400	
281.68	233.40	0.0400	
285.78	232.40	0.0400 / 0.0300	Main Channel
286.78	231.40	0.0300	Main Channel
296.78	231.40	0.0300	Main Channel
299.78	232.40	0.0300 / 0.0400	Main Channel
311.62	233.74	0.0400	
333.88	234.34	0.0400	
497.11	235.32	0.0400	
571.31	237.32	0.0400	
675.18	239.10	0.0400	
704.86	242.71	0.0400	
734.54	242.62	0.0400	

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	231.90	.221E+05	4.7	0.85	78.67
1.00	232.40	.482E+05	15.3	1.28	52.40
1.60	233.00	.913E+05	38.3	1.68	39.77
2.20	233.60	.154E+06	72.9	1.90	35.24
2.80	234.20	.279E+06	124.2	1.79	37.30
3.40	234.80	.599E+06	214.3	1.44	46.59
4.01	235.41	.125E+07	445.0	1.43	46.90
4.61	236.01	.205E+07	838.4	1.64	40.81
5.21	236.61	.293E+07	1361.6	1.86	35.89
5.81	237.21	.389E+07	2011.2	2.08	32.23
6.41	237.81	.494E+07	2753.5	2.24	29.91
7.01	238.41	.612E+07	3635.0	2.39	28.04
7.61	239.01	.741E+07	4672.5	2.53	26.43
8.21	239.61	.880E+07	5999.3	2.74	24.44
8.82	240.22	.102E+08	7510.0	2.94	22.72
9.42	240.82	.117E+08	9201.9	3.15	21.23
10.02	241.42	.132E+08	11059.7	3.35	19.95
10.62	242.02	.148E+08	13073.4	3.54	18.87
11.22	242.62	.164E+08	15242.1	3.73	17.93

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8100)	*****	115.33	7.75	28.91	2.70	1.81
OUTFLOW: ID= 1 (6202)	*****	107.68	8.50	28.91	2.61	1.83

ADD HYD (8112)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6202):	11366.34	107.680	8.50	28.91
+ ID2= 2 (8110):	468.53	41.367	6.00	55.67
=====				
ID = 3 (8112):	11834.87	113.592	8.25	29.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5555)
IN= 2----> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	*****	19.2100
2.8500	0.5200	*****	24.7300
8.1800	1.2600	*****	30.8600
15.0900	2.2100	*****	37.6100
23.2700	3.3900	*****	41.2200
32.5300	4.7800	*****	48.9000
36.5100	5.3900	*****	66.4700
42.7600	6.3700	*****	100.1200
53.8500	8.1300	*****	140.4100
91.7300	14.3200	*****	140.4200

INFLOW : ID= 2 (8112)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
*****	*****	113.592	8.25	29.97
OUTFLOW: ID= 1 (5555)	*****	110.767	8.75	29.97

PEAK FLOW REDUCTION [Out/Oi n] (%) = 97.51
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 17.5861

ADD HYD (8114)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (5555):	11834.87	110.767	8.75	29.97
+ ID2= 2 (8136):	2288.44	34.994	6.50	28.45
=====				
ID = 3 (8114):	14123.31	131.798	8.50	29.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (2061)
ID= 1 DT=15.0 min

Area (ha)= 49.84
Total Imp(%)= 76.00 Dir. Conn.(%)= 50.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	37.88	11.96
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	576.43	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	106.66	166.27
over (min)	15.00	30.00
Storage Coeff. (min)=	8.77 (ii)	17.50 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.05

TOTALS

PEAK FLOW (cms)=	6.47	3.00	8.509 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	78.80	51.92	65.36
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.64	0.81

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5206)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.5640	2.4459
0.1690	0.8986	4.2460	2.7624
1.3810	1.2959	4.8070	3.0770
2.2440	1.6904	5.2070	3.0870
2.8510	2.0623	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2061)	49.840	8.509	6.00	65.36
OUTFLOW: ID= 1 (5206)	49.840	2.322	6.75	65.33

PEAK FLOW REDUCTION [Out/Oi n](%)= 27.29
TIME SHIFT OF PEAK FLOW (min)= 45.00
MAXIMUM STORAGE USED (ha. m.)= 1.7660

CALIB
STANDHYD (2062)
ID= 1 DT=15.0 min

Area (ha)= 225.70
Total Imp(%)= 64.00 Dir. Conn.(%)= 42.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	144.45	81.25
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1226.65	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	106.66	116.18
over (min)	15.00	30.00
Storage Coeff. (min)=	13.79 (ii)	23.87 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04

TOTALS

PEAK FLOW (cms)=	21.18	12.40	29.347 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	78.80	46.65	60.15
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.58	0.74

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8144)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2062):	225.70	29.347	6.00	60.15
+ ID2= 2 (5206):	49.84	2.322	6.75	65.33
ID = 3 (8144):	275.54	30.105	6.00	61.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (2041)
ID= 1 DT=15.0 min

Area (ha)= 209.90
Total Imp(%)= 37.00 Dir. Conn.(%)= 21.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	77.66	132.24
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1182.93	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	106.66	54.48
over (min)	15.00	30.00
Storage Coeff. (min)=	13.50 (ii)	27.14 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04

TOTALS

PEAK FLOW (cms)=	9.93	8.72	15.379 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	78.80	28.60	39.14
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.35	0.48

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5204)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	13.0280	5.1901
0.5340	2.0405	16.1840	5.6427
3.0020	2.3552	19.0840	6.7700
5.5440	3.8411	19.4840	6.7800
8.4130	4.5335	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2041)	209.900	15.379	6.00	39.14
OUTFLOW: ID= 1 (5204)	209.900	5.473	6.75	39.14

PEAK FLOW REDUCTION [Out/Oi n](%)= 35.59
TIME SHIFT OF PEAK FLOW (min)= 45.00
MAXIMUM STORAGE USED (ha. m.)= 3.8363

CALIB
STANDHYD (2042)
ID= 1 DT=15.0 min

Area (ha)= 114.52
Total Imp(%)= 61.00 Dir. Conn.(%)= 34.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	69.86	44.66
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	873.77	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	106.66	90.08
over (min)	15.00	30.00

Storage Coeff. (min)= 11.25 (ii) 22.41 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

TOTALS
 12.834 (iii)
 6.00
 49.80
 80.80
 0.62

PEAK FLOW (cms)= 9.38 5.39
 TIME TO PEAK (hrs)= 6.00 6.25
 RUNOFF VOLUME (mm)= 78.80 34.87
 TOTAL RAINFALL (mm)= 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
 CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8140) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2042):	114.52	12.834	6.00	49.80
+ ID2= 2 (5204):	209.90	5.473	6.75	39.14
ID = 3 (8140):	324.42	13.334	6.00	42.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6206)
 IN= 2----> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (2061.0) ----->

Distance	Elevation	Manning	
0.00	253.07	0.0350	
13.98	252.89	0.0350	
27.95	251.99	0.0350	
34.94	252.18	0.0350	
41.93	252.02	0.0350	
73.38	250.35	0.0350	
94.34	247.09	0.0350	
111.81	244.79	0.0350	
115.31	244.24	0.0350	
117.55	244.20	0.0350 / 0.0300	Main Channel
118.80	243.60	0.0300	Main Channel
122.30	243.60	0.0300	Main Channel
122.55	244.60	0.0300 / 0.0350	Main Channel
146.76	244.74	0.0350	
164.23	247.26	0.0350	
213.15	248.92	0.0350	
241.10	249.51	0.0350	
272.55	250.87	0.0350	
300.50	252.26	0.0350	
345.93	253.67	0.0350	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.30	243.90	.310E+04	1.4	1.23	36.25
0.60	244.20	.676E+04	4.6	1.81	24.64
1.12	244.72	.221E+05	17.1	2.07	21.60
1.64	245.24	.754E+05	64.1	2.28	19.60
2.17	245.77	.139E+06	149.8	2.88	15.50
2.69	246.29	.214E+06	273.3	3.43	13.04
3.21	246.81	.299E+06	436.1	3.91	11.43
3.73	247.33	.395E+06	632.6	4.30	10.40
4.25	247.85	.511E+06	836.4	4.39	10.18
4.77	248.37	.653E+06	1114.9	4.58	9.76
5.30	248.90	.822E+06	1472.1	4.81	9.30
5.82	249.42	.102E+07	1872.2	4.91	9.10
6.34	249.94	.126E+07	2444.6	5.22	8.56
6.86	250.46	.151E+07	3113.1	5.52	8.09
7.38	250.98	.180E+07	3843.5	5.74	7.79
7.90	251.50	.211E+07	4717.8	6.00	7.45
8.43	252.03	.245E+07	5701.4	6.24	7.16
8.95	252.55	.284E+07	6727.5	6.36	7.03
9.47	253.07	.326E+07	7885.3	6.48	6.90

<---- hydrograph ----> <- pi pe / channel ->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL

INFLOW : ID= 2 (8140) 324.42 13.33 6.00 42.90 0.97 1.99
 OUTFLOW: ID= 1 (6206) 324.42 10.91 6.50 42.90 0.86 1.94

ADD HYD (8142) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6206):	324.42	10.914	6.50	42.90
+ ID2= 2 (8144):	275.54	30.105	6.00	61.09
ID = 3 (8142):	599.96	35.502	6.00	51.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8138) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8114):	14123.31	131.798	8.50	29.72
+ ID2= 2 (8142):	599.96	35.502	6.00	51.25
ID = 3 (8138):	14723.27	140.381	8.25	30.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6208)
 IN= 2----> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (2081.0) ----->

Distance	Elevation	Manning	
0.00	249.19	0.1000	
73.11	244.13	0.1000	
121.86	243.39	0.1000	
243.71	240.01	0.1000	
353.38	238.75	0.1000	
450.87	235.40	0.1000	
499.61	231.40	0.1000	
511.80	230.96	0.1000	
523.98	230.12	0.1000	
534.50	227.80	0.1000 / 0.0500	Main Channel
536.50	226.70	0.0500	Main Channel
546.50	226.70	0.0500	Main Channel
548.50	227.80	0.0500 / 0.1000	Main Channel
572.72	230.25	0.1000	
682.40	230.50	0.1000	
804.25	233.95	0.1000	
926.11	235.83	0.1000	
1047.96	240.70	0.1000	
1145.45	243.26	0.1000	
1206.38	247.03	0.1000	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.55	227.25	.111E+05	2.0	0.33	92.44
1.10	227.80	.242E+05	6.5	0.50	61.56
2.23	228.93	.700E+05	26.0	0.68	44.91
3.36	230.06	.150E+06	60.8	0.74	41.04
4.49	231.19	.446E+06	130.8	0.54	56.83
5.62	232.32	.925E+06	287.4	0.57	53.66
6.76	233.46	.152E+07	522.9	0.63	48.32
7.89	234.59	.223E+07	827.2	0.68	44.91
9.02	235.72	.312E+07	1219.5	0.72	42.61
10.15	236.85	.418E+07	1780.6	0.78	39.11
11.28	237.98	.537E+07	2479.2	0.85	36.07
12.41	239.11	.669E+07	3282.9	0.90	33.95
13.54	240.24	.824E+07	4228.6	0.94	32.47
14.67	241.37	.997E+07	5384.6	0.99	30.87
15.81	242.51	.119E+08	6698.4	1.03	29.56
16.94	243.64	.140E+08	8235.2	1.08	28.25
18.07	244.77	.162E+08	10104.2	1.14	26.73
19.20	245.90	.185E+08	12251.5	1.21	25.21
20.33	247.03	.209E+08	14597.0	1.28	23.89

<---- hydrograph ----> <- pi pe / channel ->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL

INFLOW : ID= 2 (8138) ***** (ha) (cms) (hrs) (mm) (m) (m/s)
 140.38 8.25 30.60 4.56 0.54
 OUTFLOW: ID= 1 (6208) ***** 125.01 9.25 30.60 4.41 0.55

CALIB
 STANDHYD (2082) Area (ha)= 426.60
 ID= 1 DT=15.0 min Total Imp(%)= 64.00 Dir. Conn.(%)= 37.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 273.02 153.58
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1686.42 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 106.66 116.83
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 16.70 (ii) 26.75 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.07 0.04

TOTALS
 PEAK FLOW (cms)= 32.49 22.21 46.933 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 78.80 43.35 56.47
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.54 0.70

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (2081) Area (ha)= 209.70
 ID= 1 DT=15.0 min Total Imp(%)= 35.00 Dir. Conn.(%)= 20.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 73.39 136.31
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1182.37 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 106.66 67.52
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 13.49 (ii) 26.01 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

TOTALS
 PEAK FLOW (cms)= 9.45 11.44 16.724 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 78.80 35.65 44.28
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.44 0.55

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (2084) Area (ha)= 56.00
 ID= 1 DT=15.0 min Total Imp(%)= 75.00 Dir. Conn.(%)= 70.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 42.00 14.00
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 611.01 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 106.66 89.27
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 9.08 (ii) 20.27 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.05

TOTALS
 PEAK FLOW (cms)= 10.08 1.77 11.253 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 78.80 47.71 69.47
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.59 0.86

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 82.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9289)
 IN= 2--> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	2.8000	1.5745
	0.1900	0.9984	3.7500	1.7809
	1.0900	1.1991	4.7100	1.9291
	2.0200	1.3813	5.4800	2.0745

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (2084)	56.000	11.253	6.00	69.47
OUTFLOW: ID= 1 (9289)	56.000	4.253	6.25	69.45

PEAK FLOW REDUCTION [Qout/Qin] (%) = 37.79
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha.m.) = 1.8912

ADD HYD (9262)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (2081):	209.70	16.724	6.00	44.28
+ ID2= 2 (9289):	56.00	4.253	6.25	69.45
=====				
ID = 3 (9262):	265.70	19.943	6.25	49.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5208)
 IN= 2--> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	16.8530	8.3371
	0.8360	4.7353	21.1630	9.3975
	4.7810	5.1070	25.1140	10.4406
	8.9050	6.2484	25.5140	10.4506
	12.6070	7.1796	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (9262)	265.700	19.943	6.25	49.58
OUTFLOW: ID= 1 (5208)	265.700	8.016	7.00	49.58

PEAK FLOW REDUCTION [Qout/Qin] (%) = 40.19
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 6.0029

ADD HYD (8148)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (2082):	426.60	46.933	6.00	56.47
+ ID2= 2 (5208):	265.70	8.016	7.00	49.58
=====				

ID = 3 (8148): 692.30 47.333 6.00 53.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6208):	14723.27	125.007	9.25	30.60
+ ID2= 2 (8148):	692.30	47.333	6.00	53.82
ID = 3 (8146):	15415.57	130.153	8.75	31.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6210)
IN= 2--> OUT= 1 Routing time step (mi n)' = 15.00

<----- DATA FOR SECTION (2101.0) ----->

Distance	Elevation	Manning	
0.00	241.81	0.0800	
28.43	241.78	0.0800	
85.30	237.04	0.0800	
149.27	231.32	0.0800	
170.60	230.10	0.0800	
255.89	229.37	0.0800	
277.22	228.68	0.0800	
284.33	228.42	0.0800	
291.43	228.16	0.0800	
297.76	227.00	0.0800 / 0.0350	Main Channel
298.76	225.00	0.0350	Main Channel
312.76	225.00	0.0350	Main Channel
313.76	227.00	0.0350 / 0.0800	Main Channel
326.97	229.50	0.0800	
348.30	230.29	0.0800	
376.73	229.76	0.0800	
490.46	235.75	0.0800	
568.65	236.24	0.0800	
661.06	241.85	0.0800	
703.71	246.04	0.0800	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.67	225.67	.141E+05	6.6	0.69	35.86
1.33	226.33	.289E+05	20.4	1.04	23.65
2.00	227.00	.443E+05	39.3	1.31	18.81
2.93	227.93	.730E+05	77.8	1.57	15.64
3.85	228.85	.123E+06	131.1	1.57	15.65
4.78	229.78	.228E+06	205.6	1.33	18.51
5.70	230.70	.503E+06	356.7	1.05	23.49
6.63	231.63	.846E+06	608.7	1.06	23.17
7.55	232.55	.123E+07	946.4	1.14	21.65
8.48	233.48	.165E+07	1367.0	1.22	20.13
9.41	234.41	.211E+07	1871.8	1.31	18.80
10.33	235.33	.261E+07	2463.2	1.40	17.65
11.26	236.26	.317E+07	2999.4	1.40	17.63
12.18	237.18	.384E+07	3848.3	1.48	16.63
13.11	238.11	.454E+07	4806.0	1.56	15.75
14.03	239.03	.528E+07	5876.9	1.64	14.97
14.96	239.96	.605E+07	7062.7	1.72	14.29
15.88	240.88	.687E+07	8365.0	1.80	13.68
16.81	241.81	.771E+07	9506.9	1.82	13.52

<----- hydrograph ----->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8146)	*****	130.15	8.75	31.64	3.84	1.57
OUTFLOW: ID= 1 (6210)	*****	129.71	9.50	31.64	3.83	1.57

CALIB STANDHYD (2104)
ID= 1 DT=15.0 mi n
Area (ha)= 192.70
Total Imp(%)= 73.00 Di r. Conn.(%)= 68.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	140.67	52.03
Dep. Storage (mm)=	1.00	5.00

Average Slope (%)= 1.00 2.00
Length (m)= 1133.43 40.00
Mannings n = 0.013 0.250
Max. Eff. Inten. (mm/hr)= 106.66 85.42
Storage over (mi n)= 15.00 30.00
Storage Coeff. (mi n)= 10.69 (ii) 18.20 (ii)
Unit Hyd. Tpeak (mi n)= 15.00 30.00
Unit Hyd. peak (cms)= 0.09 0.05

PEAK FLOW (cms)= 32.11 6.55 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.25 36.463 (iii)
RUNOFF VOLUME (mm)= 79.80 46.23 6.00
TOTAL RAINFALL (mm)= 80.80 80.80 69.06
RUNOFF COEFFICIENT = 0.99 0.57 80.80
0.85

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 81.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9283)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.5000	8.0000
0.3000	2.0000	3.5000	10.0000
0.9000	5.0000	4.5000	13.0000
2.0000	7.5000	5.5000	14.0000

AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
INFLOW : ID= 2 (2104) 192.700 36.463 6.00 69.06
OUTFLOW: ID= 1 (9283) 192.700 3.127 7.25 69.05

PEAK FLOW REDUCTION [Qout/Qin] (%) = 8.58
TIME SHIFT OF PEAK FLOW (mi n) = 75.00
MAXIMUM STORAGE USED (ha. m.) = 9.2541

CALIB STANDHYD (2103)
ID= 1 DT=15.0 mi n

Area (ha)= 11.10
Total Imp(%)= 30.00 Di r. Conn.(%)= 10.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 3.33 7.77
Dep. Storage (mm)= 1.00 5.00
Average Slope (%)= 1.00 2.00
Length (m)= 272.03 40.00
Mannings n = 0.013 0.250
Max. Eff. Inten. (mm/hr)= 106.66 103.34
Storage over (mi n)= 15.00 15.00
Storage Coeff. (mi n)= 4.54 (ii) 11.50 (ii)
Unit Hyd. Tpeak (mi n)= 15.00 15.00
Unit Hyd. peak (cms)= 0.11 0.08

PEAK FLOW (cms)= 0.32 1.76 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.00 2.080 (iii)
RUNOFF VOLUME (mm)= 79.80 51.64 6.00
TOTAL RAINFALL (mm)= 80.80 80.80 54.46
RUNOFF COEFFICIENT = 0.99 0.64 80.80
0.67

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9284)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW STORAGE | OUTFLOW STORAGE

(cms)	(ha. m.)	(cms)	(ha. m.)
0.0000	0.0000	0.5600	0.2067
0.0400	0.1045	0.7400	0.2572
0.2200	0.1166	0.9300	0.3046
0.4000	0.1670	1.0900	0.3510

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2103)	11.100	2.080	6.00	54.46
OUTFLOW: ID= 1 (9284)	11.100	0.739	6.25	54.40

PEAK FLOW REDUCTION [Qout/Qin] (%) = 35.54
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 0.2587

ADD HYD 1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9283):	192.70	3.127	7.25	69.05
+ ID2= 2 (9284):	11.10	0.739	6.25	54.40
ID = 3 (9258):	203.80	3.622	6.75	68.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5210) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	0.0060	0.0024
	0.0020	0.0013	0.0080	0.0027
	0.0030	0.0017	0.0090	0.0031
	0.0050	0.0020	0.4090	0.0131

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9258)	203.800	3.622	6.75	68.25
OUTFLOW: ID= 1 (5210)	203.800	3.612	6.75	68.25

PEAK FLOW REDUCTION [Qout/Qin] (%) = 99.75
 TIME SHIFT OF PEAK FLOW (min) = 0.00
 MAXIMUM STORAGE USED (ha. m.) = 0.0936

CALIB NASHYD (2102) ID= 1 DT=15.0 min	Area (ha)	(ha)=	Curve Number (CN)=	(CN)=
	14.38	5.00	80.0	1.50
	U. H.	1.08		

Unit Hyd Qpeak (cms) = 0.228
 PEAK FLOW (cms) = 0.266 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 40.870
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.506

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2102):	14.38	0.266	7.25	40.87
+ ID2= 2 (5210):	203.80	3.612	6.75	68.25
ID = 3 (9210):	218.18	3.875	7.00	66.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8150):	14.38	0.266	7.25	40.87
+ ID2= 2 (5210):	203.80	3.612	6.75	68.25
ID = 3 (9210):	218.18	3.875	7.00	66.45

ID1= 1 (6210):	15415.57	129.714	9.50	31.64
+ ID2= 2 (9210):	218.18	3.875	7.00	66.45
ID = 3 (8150):	15633.75	132.737	9.50	32.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2121) ID= 1 DT=15.0 min	Area (ha)	(ha)=	Total Imp(%)=	Dir. Conn.(%)=
	56.89	91.00	74.00	

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	51.77	5.12
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	615.85	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	106.66	233.15
over (min)	15.00	30.00
Storage Coeff. (min)=	9.12 (ii)	16.75 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.05

TOTALS
 PEAK FLOW (cms) = 10.81 1.83 12.066 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 78.80 52.81 72.04
 TOTAL RAINFALL (mm) = 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.65 0.89

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2124) ID= 1 DT=15.0 min	Area (ha)	(ha)=	Total Imp(%)=	Dir. Conn.(%)=
	4.40	75.00	70.00	

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	3.30	1.10
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	171.27	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	106.66	63.04
over (min)	15.00	30.00
Storage Coeff. (min)=	4.23 (ii)	17.10 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.11	0.05

TOTALS
 PEAK FLOW (cms) = 0.90 0.10 0.963 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 78.80 34.20 65.42
 TOTAL RAINFALL (mm) = 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.42 0.81

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9290) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	0.2200	0.1226
	0.0100	0.0778	0.2500	0.1387
	0.0800	0.0934	0.3700	0.1503
	0.1600	0.1076	0.4300	0.1616

INFLOW : ID= 2 (2124) AREA (ha) 4.400 OPEAK (cms) 0.963 TPEAK (hrs) 6.00 R. V. (mm) 65.42
 OUTFLOW: ID= 1 (9290) AREA (ha) 4.400 OPEAK (cms) 0.353 TPEAK (hrs) 6.25 R. V. (mm) 64.97

PEAK FLOW REDUCTION [Qout/Qin] (%) = 36.65
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 0.1524

ADD HYD (9264)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2121):	56.89	12.066	6.00	72.04
+ ID2= 2 (9290):	4.40	0.353	6.25	64.97

ID = 3 (9264):	61.29	12.269	6.00	71.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5212)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.1000	2.1402
0.2080	1.2793	5.1480	2.2661
1.1880	1.5546	5.9930	2.3900
2.2120	1.7255	6.3930	2.4000
3.0680	1.9336	0.0000	0.0000

INFLOW : ID= 2 (9264) AREA (ha) 61.290 OPEAK (cms) 12.269 TPEAK (hrs) 6.00 R. V. (mm) 71.53
 OUTFLOW: ID= 1 (5212) AREA (ha) 61.290 OPEAK (cms) 4.365 TPEAK (hrs) 6.50 R. V. (mm) 71.51

PEAK FLOW REDUCTION [Qout/Qin] (%) = 35.58
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 2.2167

CALIB STANDHYD (2122)
 ID= 1 DT=15.0 min

Area (ha) = 267.10
 Total Imp(%) = 29.00
 Dir. Conn. (%) = 24.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	77.46	189.64
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1334.42	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	106.66	56.94	
over (min)	15.00	30.00	
Storage Coeff. (min)	14.51 (ii)	27.91 (ii)	
Unit Hyd. Tpeak (min)	15.00	30.00	
Unit Hyd. peak (cms)	0.07	0.04	
			TOTALS
PEAK FLOW (cms)	14.03	12.91	22.168 (iii)
TIME TO PEAK (hrs)	6.00	6.25	6.00
RUNOFF VOLUME (mm)	78.80	34.51	45.14
TOTAL RAINFALL (mm)	80.80	80.80	80.80
RUNOFF COEFFICIENT	0.98	0.43	0.56

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9312)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	13.3600	4.9180
0.9100	2.4658	17.8700	6.1368

5.1800 2.7404 22.4400 7.2882
 9.6400 3.9605 26.1200 8.4139

INFLOW : ID= 2 (2122) AREA (ha) 267.100 OPEAK (cms) 22.168 TPEAK (hrs) 6.00 R. V. (mm) 45.14
 OUTFLOW: ID= 1 (9312) AREA (ha) 267.100 OPEAK (cms) 11.595 TPEAK (hrs) 6.50 R. V. (mm) 45.14

PEAK FLOW REDUCTION [Qout/Qin] (%) = 52.30
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 4.5566

ADD HYD (8152)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (5212):	61.29	4.365	6.50	71.51
+ ID2= 2 (9312):	267.10	11.595	6.50	45.14

ID = 3 (8152):	328.39	15.960	6.50	50.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6214)
 IN= 2--> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (2141.0) ----->

Distance	Elevation	Manning
0.00	259.29	0.0500
15.33	258.91	0.0500
38.33	258.57	0.0500
65.16	257.93	0.0500
95.82	253.44	0.0500
111.15	251.21	0.0500
130.32	249.67	0.0500
149.48	248.51	0.0500
150.66	248.50	0.0500 / 0.0370
150.81	247.50	0.0370
154.66	247.50	0.0370
155.66	248.50	0.0370 / 0.0500
160.98	248.50	0.0500
187.81	249.78	0.0500
199.31	252.13	0.0500
245.30	254.04	0.0500
264.46	253.99	0.0500
298.96	255.23	0.0500
329.62	257.02	0.0500
379.45	258.82	0.0500

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	248.00	.648E+04	2.1	1.00	52.30
1.00	248.50	.139E+05	6.4	1.44	36.35
1.61	249.11	.570E+05	22.6	1.24	42.01
2.21	249.71	.143E+06	63.6	1.39	37.61
2.82	250.32	.263E+06	142.9	1.70	30.69
3.43	250.93	.403E+06	257.2	2.00	26.11
4.04	251.54	.562E+06	410.4	2.29	22.82
4.64	252.14	.735E+06	601.3	2.56	20.37
5.25	252.75	.933E+06	791.0	2.66	19.67
5.86	253.36	.117E+07	1038.0	2.79	18.74
6.46	253.96	.144E+07	1344.4	2.93	17.82
7.07	254.57	.178E+07	1664.1	2.93	17.83
7.68	255.18	.217E+07	2140.2	3.10	16.87
8.28	255.78	.259E+07	2722.9	3.30	15.83
8.89	256.39	.304E+07	3387.1	3.50	14.94
9.50	257.00	.351E+07	4132.7	3.69	14.16
10.11	257.61	.402E+07	4913.0	3.83	13.64
10.71	258.21	.458E+07	5648.3	3.87	13.50
11.32	258.82	.521E+07	6343.1	3.82	13.68

<---- hydrograph ----> <-pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8152)	328.39	15.96	6.50	50.06	1.36	1.32
OUTFLOW: ID= 1 (6214)	328.39	11.91	7.25	50.06	1.21	1.36

CALIB
STANDHYD (2144)
ID= 1 DT=15.0 min

Area (ha)= 93.10
Total Imp(%)= 75.00 Dir. Conn.(%)= 70.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 69.82 23.27
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 787.82 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 106.66 64.78
over (min)= 15.00 30.00
Storage Coeff. (min)= 10.58 (ii) 23.30 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.09 0.04

TOTALS

PEAK FLOW (cms)= 16.02 1.98 17.283 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 78.80 35.09 65.69
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.43 0.81

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9285)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.6500	2.6150
0.3200	1.6583	6.2300	2.9579
1.8100	1.9916	7.8200	3.2041
3.3600	2.2943	9.1000	3.4456

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
93.100	17.283	6.00	65.69
93.100	6.224	6.50	65.67

INFLOW : ID= 2 (2144)
OUTFLOW: ID= 1 (9285)

PEAK FLOW REDUCTION [Qout/Qin] (%) = 36.01
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha.m.) = 3.0214

CALIB
STANDHYD (2143)
ID= 1 DT=15.0 min

Area (ha)= 158.30
Total Imp(%)= 26.00 Dir. Conn.(%)= 9.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 41.16 117.14
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1027.29 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 106.66 71.14
over (min)= 15.00 30.00
Storage Coeff. (min)= 12.40 (ii) 24.66 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04

TOTALS

PEAK FLOW (cms)= 3.32 10.66 12.053 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.25
RUNOFF VOLUME (mm)= 78.80 37.49 41.21
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.46 0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9286)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	7.9200	2.8224
0.5400	1.3792	10.5900	3.5534
3.0700	1.5175	13.3000	4.2519
5.7200	2.2534	15.4900	4.9349

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
158.300	12.053	6.25	41.21
158.300	6.361	6.75	41.20

PEAK FLOW REDUCTION [Qout/Qin] (%) = 52.77
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha.m.) = 2.4403

ADD HYD (9260)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
93.10	6.224	6.50	65.67
158.30	6.361	6.75	41.20
251.40	12.265	6.50	50.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5214)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.5370	0.9877
0.1280	0.4635	3.1860	1.1293
0.5900	0.5362	3.7090	1.2678
1.3690	0.6921	3.9980	1.2778
1.8980	0.8262	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
251.400	12.265	6.50	50.27
251.400	12.455	6.75	50.26

PEAK FLOW REDUCTION [Qout/Qin] (%) = 101.55
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha.m.) = 1.6030

**** WARNING : HYDROGRAPH PEAK WAS NOT REDUCED.
CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.

CALIB
STANDHYD (2142)
ID= 1 DT=15.0 min

Area (ha)= 65.54
Total Imp(%)= 27.00 Dir. Conn.(%)= 10.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 17.70 47.84
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 661.01 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 106.66 24.14
over (min)= 15.00 30.00
Storage Coeff. (min)= 9.52 (ii) 28.41 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.09 0.04

TOTALS

PEAK FLOW (cms)= 1.66 2.22 3.031 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 78.80 21.44 27.18
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.27 0.34

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 51.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD. (9214) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2142):	65.54	3.031	6.00	27.18
+ ID2= 2 (5214):	251.40	12.455	6.75	50.26

ID = 3 (9214):	316.94	13.920	6.75	45.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD. (8154) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6214):	328.39	11.913	7.25	50.06
+ ID2= 2 (9214):	316.94	13.920	6.75	45.49

ID = 3 (8154):	645.33	24.281	6.75	47.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2163) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	69.70 27.00	Di r. Conn. (%)=	9.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	18.82	50.88
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	681.66	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)= over (min)	106.66 15.00	85.07 30.00
Storage Coeff. (min)=	9.70 (ii)	21.11 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.05
PEAK FLOW (cms)=	1.58	5.99
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	78.80	43.92
TOTAL RAINFALL (mm)=	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.54

TOTALS
6.525 (iii)
6.25
47.06
80.80
0.58

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 78.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9271) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	3.4900	1.2605
	0.2400	0.6230	4.6600	1.5808
	1.3500	0.6885	5.8600	1.8854
	2.5200	1.0102	6.8200	2.1832
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)

INFLOW : ID= 2 (2163) 69.700 6.525 6.25 47.06
 OUTFLOW: ID= 1 (9271) 69.700 3.430 6.75 47.05

PEAK FLOW REDUCTION [Qout/Qin] (%) = 52.57
 TIME SHIF T OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 1.2715

CALIB STANDHYD (2169) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	3.50 30.00	Di r. Conn. (%)=	10.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	1.05	2.45
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	152.75	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)= over (min)	106.66 15.00	70.57 30.00
Storage Coeff. (min)=	3.95 (ii)	16.25 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.11	0.05

TOTALS
0.281 (iii)
6.25
40.01
80.80
0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9272) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	0.1800	0.0659
	0.0100	0.0333	0.2400	0.0820
	0.0700	0.0372	0.3000	0.0971
	0.1300	0.0532	0.3500	0.1119

INFLOW : ID= 2 (2169) 3.500 0.281 6.25 40.01
 OUTFLOW: ID= 1 (9272) 3.500 0.146 6.50 39.77

PEAK FLOW REDUCTION [Qout/Qin] (%) = 52.06
 TIME SHIF T OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 0.0583

ADD HYD. (9251) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9271):	69.70	3.430	6.75	47.05
+ ID2= 2 (9272):	3.50	0.146	6.50	39.77

ID = 3 (9251):	73.20	3.574	6.75	46.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5216) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	3.7400	2.4559
	0.1890	1.6831	4.6960	2.6646
	1.0830	1.7903	5.4670	2.8688

2.0170 2.0202 | 5.8670 2.8788
 2.7980 2.2179 | 0.0000 0.0000

AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (9251) 73.200 3.574 6.75 46.70
 OUTFLOW: ID= 1 (5216) 73.200 1.258 8.25 46.67

PEAK FLOW REDUCTION [Qout/Qin] (%) = 35.21
 TIME SHIFT OF PEAK FLOW (min) = 90.00
 MAXIMUM STORAGE USED (ha.m.) = 1.8398

CALIB STANDHYD (2162)
 ID= 1 DT=15.0 min
 Area (ha) = 72.57
 Total Imp(%) = 20.00
 Dir. Conn. (%) = 7.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 14.51 58.06
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 695.56 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr) = 106.66 59.98
 Storage Coeff. over (min) = 15.00 30.00
 Unit Hyd. Tpeak (min) = 9.81 (ii) 22.94 (ii)
 Unit Hyd. peak (cms) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.09 0.04
 TOTALS
 PEAK FLOW (cms) = 1.28 4.60 5.035 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.25
 RUNOFF VOLUME (mm) = 78.80 33.59 36.76
 TOTAL RAINFALL (mm) = 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.42 0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9316)
 IN= 2--> OUT= 1
 DT= 15.0 min
 OUTFLOW STORAGE OUTFLOW STORAGE
 (cms) (ha.m.) (cms) (ha.m.)
 0.0000 0.0000 4.0000 0.3000
 2.0000 0.1000 4.5000 0.4000
 3.0000 0.2000 5.0000 0.5000

AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (2162) 72.570 5.035 6.25 36.76
 OUTFLOW: ID= 1 (9316) 72.570 4.033 6.50 36.76

PEAK FLOW REDUCTION [Qout/Qin] (%) = 80.10
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha.m.) = 0.3340

ADD HYD (9216)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (5216): 73.20 1.258 8.25 46.67
 + ID2= 2 (9316): 72.57 4.033 6.50 36.76
 ID= 3 (9216): 145.77 4.084 6.50 41.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8156)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.

(ha) (cms) (hrs) (mm)
 ID1= 1 (8154): 645.33 24.281 6.75 47.81
 + ID2= 2 (9216): 145.77 4.084 6.50 41.74
 ID= 3 (8156): 791.10 27.724 6.75 46.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6218)
 IN= 2--> OUT= 1
 Routing time step (min) = 15.00

DATA FOR SECTION (2181.0) ----->

Distance	Elevation	Manning
0.00	256.38	0.0450
7.70	254.85	0.0450
15.40	253.20	0.0450
19.25	252.29	0.0450
23.10	251.17	0.0450
78.94	236.16	0.0450
80.87	235.80	0.0450
82.79	235.45	0.0450
84.57	234.60	0.0450 / 0.0300
85.07	233.60	0.0300
88.57	233.60	0.0300
93.57	233.60	0.0300
94.07	234.60	0.0300 / 0.0450
96.27	235.13	0.0450
98.19	235.20	0.0450
100.12	235.32	0.0450
138.63	239.95	0.0450
180.98	244.80	0.0450
188.69	245.00	0.0450
190.61	256.00	0.0450

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
1.00	234.60	.181E+05	6.0	0.66	50.64
2.13	235.73	.518E+05	23.8	0.93	36.27
3.25	236.85	.119E+06	60.0	1.02	33.03
4.38	237.98	.217E+06	120.6	1.12	29.99
5.51	239.11	.346E+06	210.5	1.23	27.39
6.63	240.23	.506E+06	334.1	1.33	25.22
7.76	241.36	.697E+06	495.6	1.43	23.44
8.88	242.48	.920E+06	699.8	1.53	21.91
10.01	243.61	.117E+07	950.8	1.63	20.59
11.14	244.74	.146E+07	1252.5	1.73	19.45
12.26	245.86	.178E+07	1619.6	1.83	18.37
13.39	246.99	.212E+07	2078.5	1.97	17.01
14.52	248.12	.247E+07	2589.0	2.11	15.88
15.64	249.24	.282E+07	3149.7	2.25	14.94
16.77	250.37	.319E+07	3759.8	2.37	14.14
17.89	251.49	.356E+07	4419.5	2.50	13.44
19.02	252.62	.395E+07	5126.4	2.61	12.84
20.15	253.75	.435E+07	5871.6	2.72	12.34
21.27	254.87	.476E+07	6663.2	2.82	11.89

hydrograph -----> <- pi pe / channel ->
 AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8156) 791.10 27.72 6.75 46.69 2.25 0.93
 OUTFLOW: ID= 1 (6218) 791.10 21.03 7.25 46.69 1.96 0.87

CALIB STANDHYD (2183)
 ID= 1 DT=15.0 min
 Area (ha) = 69.30
 Total Imp(%) = 22.00
 Dir. Conn. (%) = 7.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 15.25 54.05
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 679.71 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr) = 106.66 83.92
 Storage Coeff. over (min) = 15.00 30.00
 Unit Hyd. Tpeak (min) = 9.68 (ii) 21.15 (ii)
 Unit Hyd. peak (cms) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.09 0.05

TOTALS
 PEAK FLOW (cms)= 1.23 6.28 6.690 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.25
 RUNOFF VOLUME (mm)= 78.80 45.19 47.54
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.56 0.59

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 80.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9282)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.5000	3.5000
0.8000	0.1000	6.0000	4.0000
1.1000	2.0000	6.5000	4.5000
2.5000	3.0000	7.0000	5.5000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2183)	69.300	6.690	6.25	47.54
OUTFLOW: ID= 1 (9282)	69.300	1.045	7.50	47.54

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 15.61
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha. m.) = 1.6501

RESERVOIR (5218)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.5090	1.7553
0.2280	0.8236	5.6620	2.0069
1.3060	0.9528	6.5910	2.2530
2.4320	1.2300	6.9910	2.2630
3.3740	1.4683	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9282)	69.300	1.045	7.50	47.54
OUTFLOW: ID= 1 (5218)	69.300	1.010	9.25	47.53

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 96.66
 TIME SHIFT OF PEAK FLOW (min) = 105.00
 MAXIMUM STORAGE USED (ha. m.) = 0.9173

CALIB NASHYD (2182)
 ID= 1 DT=15.0 min

Area (ha) = 82.95 Curve Number (CN) = 67.7
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp (hrs) = 0.79

Unit Hyd Qpeak (cms) = 1.786
 PEAK FLOW (cms) = 1.353 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 28.748
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.356

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9218)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2182):	82.95	1.353	7.00	28.75
+ ID2= 2 (5218):	69.30	1.010	9.25	47.53

ID = 3 (9218): 152.25 1.990 8.50 37.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8158)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6218):	791.10	21.034	7.25	46.69
+ ID2= 2 (9218):	152.25	1.990	8.50	37.30
ID = 3 (8158):	943.35	22.542	7.25	45.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8151)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8150):	15633.75	132.737	9.50	32.13
+ ID2= 2 (8158):	943.35	22.542	7.25	45.18
ID = 3 (8151):	16577.10	147.742	8.50	32.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5504)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	33.0100	0.6700
2.6400	0.0500	37.4800	0.7800
4.8300	0.0900	47.5400	1.0100
7.4100	0.1300	60.2300	1.2800
10.3300	0.1900	71.9530	1.5258
13.5300	0.2500	*****	3.0529
16.9900	0.3200	*****	5.3299
20.6800	0.3900	*****	8.4148
24.6000	0.4800	*****	11.7637
28.7100	0.5700	*****	11.7737

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (8151)	*****	147.742	8.50	32.87
OUTFLOW: ID= 1 (5504)	*****	147.808	8.50	32.87

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 100.04
 TIME SHIFT OF PEAK FLOW (min) = 0.00
 MAXIMUM STORAGE USED (ha. m.) = 3.1869

**** WARNING : HYDROGRAPH PEAK WAS NOT REDUCED.
 CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.

ROUTE CHN (6220)
 IN= 2--> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (2201,0) ----->

Distance	Elevation	Manning	
0.00	245.59	0.0600	
5.76	245.45	0.0600	
23.03	242.27	0.0600	
57.58	232.35	0.0600	
86.37	226.18	0.0600	
109.41	224.31	0.0600	
115.17	223.86	0.0600	
120.92	222.78	0.0600	
126.68	221.93	0.0600	
128.06	221.50	0.0600 / 0.0300	Main Channel
129.56	220.00	0.0300	Main Channel
141.56	220.00	0.0300	Main Channel
143.06	221.50	0.0300 / 0.0600	Main Channel
149.72	223.77	0.0600	
155.47	225.09	0.0600	
253.36	229.21	0.0600	
333.98	227.50	0.0600	
454.90	229.25	0.0600	
500.97	233.11	0.0600	

570.07 237.42 0.0600

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.75	220.75	.262E+05	5.5	0.57	79.44
1.50	221.50	.555E+05	17.5	0.86	52.89
2.44	222.44	.103E+06	42.7	1.14	40.03
3.37	223.37	.171E+06	79.4	1.27	35.98
4.31	224.31	.263E+06	128.1	1.33	34.22
5.25	225.25	.393E+06	191.7	1.34	34.19
6.18	226.18	.594E+06	276.7	1.28	35.77
7.12	227.12	.872E+06	399.9	1.26	36.33
8.06	228.06	.127E+07	542.7	1.17	38.92
8.99	228.99	.199E+07	770.0	1.06	43.01
9.93	229.93	.297E+07	1151.8	1.06	42.95
10.86	230.86	.400E+07	1652.9	1.13	40.33
11.80	231.80	.507E+07	2251.8	1.22	37.53
12.74	232.74	.618E+07	2946.5	1.31	34.97
13.67	233.67	.733E+07	3726.8	1.39	32.79
14.61	234.61	.853E+07	4591.0	1.48	30.96
15.55	235.55	.977E+07	5544.1	1.55	29.37
16.48	236.48	.111E+08	6586.7	1.63	27.99
17.42	237.42	.124E+08	7719.2	1.71	26.77

----- hydrograph ----- <- pipe / channel ->

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (5504) *****	147.81	8.50	32.87	4.60	1.33
OUTFLOW: ID= 1 (6220) *****	142.35	9.50	32.87	4.53	1.33

CALIB STANDHYD (2207) ID= 1 DT=15.0 mi n

Area Total	(ha)= 57.80	Imp(%)= 21.00	Di r. Conn.(%)= 8.00
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	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	12.14	45.66
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	620.75	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (mi n)	106.66 / 15.00	85.52 / 30.00
Storage Coeff. (mi n)	9.17 (ii)	20.56 (ii)
Unit Hyd. Tpeak (mi n)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.05

TOTALS
5.861 (iii)
6.25
49.60
80.80
0.61

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 82.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9278) IN= 2---> OUT= 1 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.5000	2.5000
0.2000	0.3000	0.6000	3.5000
0.3000	0.9000	0.7000	4.5000
0.4000	2.0000	0.8000	5.5000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (2207)	57.800	5.861	6.25
OUTFLOW: ID= 1 (9278)	57.800	0.405	49.59

PEAK FLOW REDUCTION [Qout/Qi n](%)= 6.91

TIME SHIFT OF PEAK FLOW (mi n)=165.00
MAXIMUM STORAGE USED (ha. m.)= 2.0243

CALIB STANDHYD (2204) ID= 1 DT=15.0 mi n

Area Total	(ha)= 13.30	Imp(%)= 75.00	Di r. Conn.(%)= 70.00
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	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	9.98	3.33
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	297.77	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (mi n)	106.66 / 15.00	82.51 / 30.00
Storage Coeff. (mi n)	5.90 (ii)	17.45 (ii)
Unit Hyd. Tpeak (mi n)	15.00	30.00
Unit Hyd. peak (cms)	0.10	0.05

TOTALS
2.887 (iii)
6.00
68.42
80.80
0.85

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9279) IN= 2---> OUT= 1 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.6700	0.3747
0.0500	0.2376	0.9900	0.4238
0.2600	0.2854	1.1200	0.4591
0.4800	0.3287	1.3000	0.4937

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (2204)	13.300	2.887	6.00
OUTFLOW: ID= 1 (9279)	13.300	1.063	68.33

PEAK FLOW REDUCTION [Qout/Qi n](%)= 36.83
TIME SHIFT OF PEAK FLOW (mi n)= 15.00
MAXIMUM STORAGE USED (ha. m.)= 0.4549

ADD HYD 1 + 2 = 3 (9255)

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (9278):	57.80	0.405	9.00
+ ID2= 2 (9279):	13.30	1.063	68.33
ID = 3 (9255):	71.10	1.368	53.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5220) IN= 2---> OUT= 1 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.6050	1.0142
0.1320	0.4759	3.2710	1.1595
0.7550	0.5505	3.8080	1.3018
1.4050	0.7107	4.2080	1.3118
1.9490	0.8484	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (9255)	71.100	1.368	53.10

OUTFLOW: ID= 1 (5220) 71.100 0.746 7.50 53.08

PEAK FLOW REDUCTION [Out/Oi n] (%) = 54.56
TIME SHIFT OF PEAK FLOW (min) = 75.00
MAXIMUM STORAGE USED (ha. m.) = 0.5510

CALIB NASHYD (2202) Area (ha) = 98.00 Curve Number (CN) = 74.3
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp (hrs) = 1.50

Unit Hyd Qpeak (cms) = 1.115

PEAK FLOW (cms) = 1.185 (i)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 34.912
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.432

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9220)
1 + 2 = 3
ID1= 1 (2202): 98.00 1.185 8.00 34.91
+ ID2= 2 (5220): 71.10 0.746 7.50 53.08
ID = 3 (9220): 169.10 1.917 7.75 42.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8160)
1 + 2 = 3
ID1= 1 (6220): 16577.10 142.353 9.50 32.87
+ ID2= 2 (9220): 169.10 1.917 7.75 42.55
ID = 3 (8160): 16746.20 143.986 9.50 32.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2221) Area (ha) = 110.64
ID= 1 DT=15.0 min Total Imp(%) = 37.00 Di r. Conn. (%) = 20.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 40.94 69.70
Dep. Storage (mm) = 2.00 5.00
Average Slope (%) = 0.50 0.50
Length (m) = 858.84 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 106.66 81.58
over (min) = 15.00 30.00
Storage Coeff. (min) = 11.14 (ii) 22.74 (ii)
Unit Hyd. Tpeak (min) = 15.00 30.00
Unit Hyd. peak (cms) = 0.08 0.04

PEAK FLOW (cms) = 5.35 7.59 *TOTALS*
TIME TO PEAK (hrs) = 6.00 6.25 10.274 (iii)
RUNOFF VOLUME (mm) = 78.80 41.47 48.93
TOTAL RAINFALL (mm) = 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.51 0.61

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.3 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2224) Area (ha) = 43.20
ID= 1 DT=15.0 min Total Imp(%) = 72.00 Di r. Conn. (%) = 66.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 31.10 12.10
Dep. Storage (mm) = 2.00 5.00
Average Slope (%) = 0.50 0.50
Length (m) = 536.66 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 106.66 83.97
over (min) = 15.00 30.00
Storage Coeff. (min) = 8.40 (ii) 19.87 (ii)
Unit Hyd. Tpeak (min) = 15.00 30.00
Unit Hyd. peak (cms) = 0.09 0.05

PEAK FLOW (cms) = 7.48 1.44 *TOTALS*
TIME TO PEAK (hrs) = 6.00 6.25 8.430 (iii)
RUNOFF VOLUME (mm) = 78.80 44.45 67.12
TOTAL RAINFALL (mm) = 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.55 0.83

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 79.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9288)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW STORAGE OUTFLOW STORAGE
(cms) (ha. m.) (cms) (ha. m.)
0.0000 0.0000 2.1600 1.1836
0.1500 0.7428 2.8900 1.3455
0.8400 0.8895 3.6300 1.4651
1.5600 1.0342 4.2300 1.5823

AREA OPEAK TPEAK R. V.
(ha) (cms) (hrs) (mm)
INFLOW: ID= 2 (2224) 43.200 8.430 6.00 67.12
OUTFLOW: ID= 1 (9288) 43.200 3.098 6.25 67.09

PEAK FLOW REDUCTION [Out/Oi n] (%) = 36.74
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha. m.) = 1.4042

ADD HYD (9261)
1 + 2 = 3
ID1= 1 (2221): 110.64 10.274 6.00 48.93
+ ID2= 2 (9288): 43.20 3.098 6.25 67.09
ID = 3 (9261): 153.84 12.741 6.25 54.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5222)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW STORAGE OUTFLOW STORAGE
(cms) (ha. m.) (cms) (ha. m.)
0.0000 0.0000 7.2210 4.0213
0.4910 2.1348 9.0080 4.5380
2.1830 2.4433 15.0420 4.9977
3.9370 2.9883 15.4420 5.0077
5.4550 3.4692 0.0000 0.0000

AREA OPEAK TPEAK R. V.
(ha) (cms) (hrs) (mm)
INFLOW: ID= 2 (9261) 153.840 12.741 6.25 54.03
OUTFLOW: ID= 1 (5222) 153.840 5.401 7.00 54.02

PEAK FLOW REDUCTION [Out/Oi n] (%) = 42.39

TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 3.4714

CALIB STANDHYD (2222) ID= 1 DT=15.0 min			
Area Total	(ha) = 65.69	Dir. Conn. (%) = 45.00	
Imp (%) = 84.00			
	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha) =	55.18	10.51	
Dep. Storage (mm) =	2.00	5.00	
Average Slope (%) =	0.50	0.50	
Length (m) =	661.77	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr) =	106.66	315.10	
over (min) =	15.00	30.00	
Storage Coeff. (min) =	9.53 (ii)	16.28 (ii)	
Unit Hyd. Tpeak (min) =	15.00	30.00	
Unit Hyd. peak (cms) =	0.09	0.05	
PEAK FLOW (cms) =	7.50	5.17	*TOTALS* 11.143 (iii)
TIME TO PEAK (hrs) =	6.00	6.25	6.00
RUNOFF VOLUME (mm) =	78.80	60.78	68.89
TOTAL RAINFALL (mm) =	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.75	0.85

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8164) 1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2222):	65.69	11.143	6.00	68.89
+ ID2= 2 (5222):	153.84	5.401	7.00	54.02
=====				
ID = 3 (8164):	219.53	11.458	6.00	58.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2242) ID= 1 DT=15.0 min			
Area Total	(ha) = 52.75	Dir. Conn. (%) = 27.00	
Imp (%) = 36.00			
	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha) =	18.99	33.76	
Dep. Storage (mm) =	2.00	5.00	
Average Slope (%) =	0.50	0.50	
Length (m) =	593.01	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr) =	106.66	76.47	
over (min) =	15.00	30.00	
Storage Coeff. (min) =	8.92 (ii)	20.83 (ii)	
Unit Hyd. Tpeak (min) =	15.00	30.00	
Unit Hyd. peak (cms) =	0.09	0.05	
PEAK FLOW (cms) =	3.68	3.59	*TOTALS* 6.024 (iii)
TIME TO PEAK (hrs) =	6.00	6.25	6.00
RUNOFF VOLUME (mm) =	78.80	43.06	52.71
TOTAL RAINFALL (mm) =	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.53	0.65

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2244)
ID= 1 DT=15.0 min

CALIB STANDHYD (2244) ID= 1 DT=15.0 min			
Area Total	(ha) = 87.70	Dir. Conn. (%) = 64.00	
Imp (%) = 70.00			
	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha) =	61.39	26.31	
Dep. Storage (mm) =	2.00	5.00	
Average Slope (%) =	0.50	0.50	
Length (m) =	764.63	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr) =	106.66	82.51	
over (min) =	15.00	30.00	
Storage Coeff. (min) =	10.39 (ii)	21.94 (ii)	
Unit Hyd. Tpeak (min) =	15.00	30.00	
Unit Hyd. peak (cms) =	0.09	0.04	
PEAK FLOW (cms) =	13.88	2.95	*TOTALS* 15.809 (iii)
TIME TO PEAK (hrs) =	6.00	6.25	6.00
RUNOFF VOLUME (mm) =	78.80	44.19	66.34
TOTAL RAINFALL (mm) =	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.55	0.82

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9287) IN= 2---> OUT= 1 DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	4.3900	2.3776
	0.3000	1.4858	5.8700	2.7087
	1.7000	1.7770	7.3700	2.9557
	3.1700	2.0741	8.5800	3.1980
		AREA (ha)	OPEAK (cms)	TPEAK (hrs)
		87.700	15.809	6.00
INFLOW: ID= 2 (2244)		87.700	6.026	6.50
OUTFLOW: ID= 1 (9287)		87.700	6.026	6.50

PEAK FLOW REDUCTION [Out/Oi n] (%) = 38.12
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 2.7862

RESERVOIR (5224) IN= 2---> OUT= 1 DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	0.4470	0.1810
	0.0230	0.0884	0.5620	0.2047
	0.1300	0.1032	0.6540	0.2278
	0.2410	0.1297	1.0540	0.2378
	0.3350	0.1532	0.0000	0.0000
		AREA (ha)	OPEAK (cms)	TPEAK (hrs)
		87.700	6.026	6.50
INFLOW: ID= 2 (9287)		87.700	6.128	6.50
OUTFLOW: ID= 1 (5224)		87.700	6.128	6.50

PEAK FLOW REDUCTION [Out/Oi n] (%) = 101.69
 TIME SHIFT OF PEAK FLOW (min) = 0.00
 MAXIMUM STORAGE USED (ha. m.) = 0.3692

**** WARNING : HYDROGRAPH PEAK WAS NOT REDUCED.
 CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.

ADD HYD (9224) 1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2242):	52.75	6.024	6.00	52.71
+ ID2= 2 (5224):	87.70	6.128	6.50	66.31
=====				
ID = 3 (9224):	140.45	9.852	6.25	61.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8166)	AREA	OPEAK	TPEAK	R. V.
1 + 2 =	3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8164):		219.53	11.458	6.00	58.47
+ ID2= 2 (9224):		140.45	9.852	6.25	61.20
ID = 3 (8166):		359.98	19.373	6.25	59.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6226)
IN= 2---> OUT= 1
Routing time step (mi n)' = 15.00

<----- DATA FOR SECTION (2261.0) ----->

Distance	Elevation	Manning	
0.00	245.35	0.0600	
8.27	242.72	0.0600	
16.54	240.01	0.0600	
31.02	235.31	0.0600	
41.36	233.14	0.0600	
90.99	227.73	0.0600	
93.06	227.50	0.0600	
95.13	227.27	0.0600	
95.83	227.00	0.0600	
100.83	226.50	0.0600 / 0.0350	Main Channel
101.33	225.50	0.0350	Main Channel
101.83	226.50	0.0350 / 0.0600	Main Channel
105.83	227.00	0.0600	
107.54	227.39	0.0600	
109.60	227.62	0.0600	
111.67	227.86	0.0600	
146.83	231.76	0.0600	
173.71	234.77	0.0600	
186.12	238.23	0.0600	
202.67	238.35	0.0600	

DEPTH	ELEV	VOLUME	TRAVEL TIME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(hrs)	(cms)	(m/s)	(mi n)
0.50	226.00	.403E+03	0.0	0.0	0.16	330.59
1.00	226.50	.161E+04	0.1	0.26	0.26	208.26
1.70	227.20	.173E+05	1.5	0.27	0.27	195.72
2.39	227.89	.541E+05	5.9	0.35	0.35	152.27
3.09	228.59	.119E+06	16.0	0.43	0.43	124.08
3.79	229.29	.212E+06	33.8	0.51	0.51	104.65
4.49	229.99	.334E+06	61.2	0.59	0.59	90.97
5.18	230.68	.484E+06	99.7	0.66	0.66	80.88
5.88	231.38	.663E+06	151.0	0.73	0.73	73.13
6.58	232.08	.870E+06	216.5	0.80	0.80	66.95
7.27	232.77	.111E+07	297.5	0.87	0.87	61.91
7.97	233.47	.137E+07	398.1	0.94	0.94	57.29
8.67	234.17	.165E+07	518.9	1.01	1.01	53.12
9.36	234.86	.196E+07	658.9	1.08	1.08	49.59
10.06	235.56	.228E+07	827.0	1.17	1.17	46.02
10.76	236.26	.262E+07	1015.4	1.25	1.25	42.96
11.46	236.96	.296E+07	1221.2	1.33	1.33	40.41
12.15	237.65	.332E+07	1444.6	1.40	1.40	38.26
12.85	238.35	.368E+07	1598.2	1.40	1.40	38.42

	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8166)	359.98	19.37	6.25	59.54	3.22	0.45
OUTFLOW: ID= 1 (6226)	359.98	7.93	7.25	59.53	2.53	0.37

CALIB NASHYD (2267)
ID= 1 DT=15.0 mi n
Area (ha)= 10.50
Ia (mm)= 5.00
U. H. Tp(hrs)= 0.47
Curve Number (CN)= 82.0
of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 0.383
PEAK FLOW (cms)= 0.400 (i)

TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 42.308
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.524

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9276)	IN= 2---> OUT= 1	DT= 15.0 mi n	OUTFLOW	STORAGE	OUTFLOW	STORAGE
			(cms)	(ha. m.)	(cms)	(ha. m.)
			0.0000	0.0000	0.5300	0.1297
			0.0400	0.0402	0.7100	0.1837
			0.2000	0.0341	0.8900	0.2402
			0.3800	0.0910	1.0300	0.2954
INFLOW : ID= 2 (2267)			10.500	0.400	6.50	42.31
OUTFLOW: ID= 1 (9276)			10.500	0.327	7.25	42.29

PEAK FLOW REDUCTION [Qout/Qin] (%) = 81.77
TIME SHIFT OF PEAK FLOW (mi n) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.0749

**** ERROR : CHECK THE STORAGE-DISCHARGE TABLE.

CALIB STANDHYD (2264)
ID= 1 DT=15.0 mi n
Area (ha)= 117.80
Total Imp(%)= 75.00
Di r. Conn. (%) = 70.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	88.35	29.45
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	886.19	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	106.66	82.51
Storage Coeff. (mi n)=	11.35 (ii)	22.90 (ii)
Unit Hyd. peak (cms)=	0.08	0.04
PEAK FLOW (cms)=	19.81	3.24
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	78.80	44.19
TOTAL RAINFALL (mm)=	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.55

TOTALS
21.923 (iii)
6.00
68.42
80.80
0.85

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9277)	IN= 2---> OUT= 1	DT= 15.0 mi n	OUTFLOW	STORAGE	OUTFLOW	STORAGE
			(cms)	(ha. m.)	(cms)	(ha. m.)
			0.0000	0.0000	2.5000	8.0000
			0.3000	2.0000	3.5000	10.0000
			0.9000	5.0000	4.5000	13.0000
			2.0000	7.5000	5.5000	14.0000
INFLOW : ID= 2 (2264)			117.800	21.923	6.00	68.42
OUTFLOW: ID= 1 (9277)			117.800	1.316	8.25	68.40

PEAK FLOW REDUCTION [Qout/Qin] (%) = 6.00
TIME SHIFT OF PEAK FLOW (mi n)=135.00
MAXIMUM STORAGE USED (ha. m.) = 5.9530

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9276):	10.50	0.327	7.25	42.29
+ ID2= 2 (9277):	117.80	1.316	8.25	68.40

ID = 3 (9253):	128.30	1.610	7.75	66.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5226)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.7530	0.6823
0.0890	0.3202	2.2010	0.7801
0.5080	0.3704	2.5620	0.8758
0.9450	0.4782	2.9620	0.8858
1.3110	0.5708	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9253)	128.300	1.610	7.75	66.26
OUTFLOW: ID= 1 (5226)	128.300	1.517	8.75	66.26

PEAK FLOW REDUCTION [Qout/Qin] (%) = 94.27
TIME SHIFT OF PEAK FLOW (min) = 60.00
MAXIMUM STORAGE USED (ha.m.) = 0.6229

CALIB
NASHYD (2262)
ID= 1 DT=15.0 min

Area (ha) =	119.28	Curve Number (CN) =	77.1
Ia (mm) =	5.00	# of Linear Res. (N) =	1.50
U.H. Tp (hrs) =	1.05		

Unit Hyd Opeak (cms) = 1.934

PEAK FLOW (cms) = 2.061 (i)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 37.631
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.466

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9226)
1 + 2 = 3

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2262):	119.28	2.061	7.25	37.63
+ ID2= 2 (5226):	128.30	1.517	8.75	66.26

ID = 3 (9226):	247.58	3.434	8.00	52.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8168)
1 + 2 = 3

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6226):	359.98	7.933	7.25	59.53
+ ID2= 2 (9226):	247.58	3.434	8.00	52.47

ID = 3 (8168):	607.56	11.152	7.50	56.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8162)
1 + 2 = 3

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8160):	16746.20	143.986	9.50	32.97
+ ID2= 2 (8168):	607.56	11.152	7.50	56.65

ID = 3 (8162):	17353.76	152.748	9.00	33.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6228)
IN= 2---> OUT= 1

Routing time step (min) = 15.00

DATA FOR SECTION (2281.0) ----->		
Distance	Elevation	Manning
0.00	245.00	0.0600
18.60	244.74	0.0600
37.20	242.47	0.0600
93.01	233.71	0.0600
125.57	226.95	0.0600
213.93	221.63	0.0600
218.58	221.15	0.0600
223.23	220.45	0.0600
225.03	220.55	0.0600 / 0.0300
225.53	218.05	0.0300
232.53	218.05	0.0300
240.03	218.05	0.0300
241.03	220.55	0.0300 / 0.0600
246.48	221.00	0.0600
251.13	221.27	0.0600
255.78	221.22	0.0600
325.54	225.25	0.0600
367.40	228.91	0.0600
404.60	231.75	0.0600
460.41	245.00	0.0600

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
1.25	219.30	.338E+05	16.0	0.86	35.15
2.50	220.55	.696E+05	48.3	1.26	24.02
3.94	221.99	.168E+06	116.0	1.26	24.14
5.38	223.43	.392E+06	247.5	1.15	26.37
6.81	224.86	.743E+06	472.6	1.16	26.20
8.25	226.30	.122E+07	821.6	1.23	24.67
9.69	227.74	.179E+07	1320.1	1.34	22.59
11.13	229.18	.243E+07	1965.7	1.47	20.58
12.57	230.62	.313E+07	2744.2	1.59	19.02
14.01	232.06	.390E+07	3683.4	1.72	17.66
15.44	233.49	.472E+07	4824.1	1.86	16.30
16.88	234.93	.557E+07	6092.4	1.99	15.23
18.32	236.37	.646E+07	7503.3	2.11	14.35
19.76	237.81	.739E+07	9061.2	2.23	13.59
21.20	239.25	.836E+07	10767.6	2.34	12.94
22.64	240.69	.937E+07	12624.8	2.45	12.37
24.07	242.12	.104E+08	14634.9	2.56	11.87
25.51	243.56	.115E+08	16762.8	2.65	11.45
26.95	245.00	.127E+08	18699.9	2.69	11.28

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8162)	*****	152.75	9.00	33.80	4.34	1.22
OUTFLOW: ID= 1 (6228)	*****	150.18	9.75	33.80	4.32	1.23

CALIB
STANDHYD (2287)
ID= 1 DT=15.0 min

Area (ha) = 130.50
Total Imp (%) = 23.00
Di r. Conn. (%) = 10.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	30.01	100.48
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	932.74	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) =	106.66	48.19
over (min) =	15.00	30.00
Storage Coeff. (min) =	11.70 (ii)	26.03 (ii)
Unit Hyd. Tpeak (min) =	15.00	30.00
Unit Hyd. peak (cms) =	0.08	0.04

PEAK FLOW (cms) =	3.10	5.98	*TOTALS*
TIME TO PEAK (hrs) =	6.00	6.25	7.220 (iii)
RUNOFF VOLUME (mm) =	78.80	27.18	6.25
TOTAL RAINFALL (mm) =	80.80	80.80	32.34
			80.80

RUNOFF COEFFICIENT = 0.98 0.34 0.40

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9275)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.5000	3.5000
0.8000	0.1000	6.0000	4.0000
1.5000	2.0000	6.5000	4.5000
2.5000	3.0000	7.0000	5.5000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (2287)	130.500	7.220	6.25	32.34
OUTFLOW: ID= 1 (9275)	130.500	1.484	7.50	32.34

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 20.55
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha.m.) = 1.9574

RESERVOIR (5228)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	6.7260	2.6184
0.3410	1.2286	8.4460	2.9937
1.9480	1.4214	9.8320	3.3609
3.6280	1.8348	10.2320	3.3709
5.0330	2.1904	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (9275)	130.500	1.484	7.50	32.34
OUTFLOW: ID= 1 (5228)	130.500	1.368	9.25	32.33

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 92.23
 TIME SHIFT OF PEAK FLOW (min) = 105.00
 MAXIMUM STORAGE USED (ha.m.) = 1.3521

CALIB NASHYD (2282)
 ID= 1 DT=15.0 min

Area (ha) = 180.03 Curve Number (CN) = 61.9
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp (hrs) = 1.12

Unit Hyd Opeak (cms) = 2.754
 PEAK FLOW (cms) = 1.889 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 24.536
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.304

- (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9228)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (2282):	180.03	1.889	7.50	24.54
+ ID2= 2 (5228):	130.50	1.368	9.25	32.33
ID = 3 (9228):	310.53	3.011	9.00	27.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8170)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6228):	17353.76	150.185	9.75	33.80
+ ID2= 2 (9228):	310.53	3.011	9.00	27.81
ID = 3 (8170):	17664.29	153.025	9.75	33.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2302)
 ID= 1 DT=15.0 min

Area (ha) = 30.47
 Total Imp (%) = 29.00 Dir. Conn. (%) = 11.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 8.84 21.63
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 450.70 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 106.66 63.48
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 7.56 (ii) 20.39 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.10 0.05

TOTALS

PEAK FLOW (cms) = 0.90 1.91 2.155 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.25
 RUNOFF VOLUME (mm) = 78.80 33.07 38.10
 TOTAL RAINFALL (mm) = 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.41 0.47

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2307)
 ID= 1 DT=15.0 min

Area (ha) = 175.00
 Total Imp (%) = 25.00 Dir. Conn. (%) = 9.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 43.75 131.25
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 1080.12 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 106.66 75.45
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 12.78 (ii) 24.75 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.08 0.04

TOTALS

PEAK FLOW (cms) = 3.62 12.67 14.234 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.25
 RUNOFF VOLUME (mm) = 78.80 40.15 43.62
 TOTAL RAINFALL (mm) = 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.50 0.54

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9280)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	8.7500	3.0604
0.5900	1.4722	11.7100	3.8736
3.3900	1.6096	14.7000	4.6556
6.3200	2.4308	17.1100	5.4201

INFLOW : ID= 2 (2307)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
175.000	14.234	6.25	43.62	
OUTFLOW: ID= 1 (9280)	175.000	7.665	6.75	43.62

PEAK FLOW REDUCTION [Qout/Qin] (%) = 53.85
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 2.8117

RESERVOIR (5230)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	8.6830	2.7782
0.4700	1.3193	10.7480	3.1570
2.5870	1.5374	12.3570	3.5304
5.1590	1.9534	12.7570	3.5404
6.7830	2.3333	0.0000	0.0000

INFLOW : ID= 2 (9280)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
175.000	7.665	6.75	43.62	
OUTFLOW: ID= 1 (5230)	175.000	5.542	7.50	43.62

PEAK FLOW REDUCTION [Qout/Qin] (%) = 72.30
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 2.0497

ADD HYD (8172)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2302):	30.47	2.155	6.25	38.10
+ ID2= 2 (5230):	175.00	5.542	7.50	43.62

ID = 3 (8172):	205.47	5.904	7.50	42.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (2329)
ID= 1 DT=15.0 min

Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1.80	5.00	6.25	35.57
U. H. Tp (hrs) =	0.22		

Unit Hyd Qpeak (cms) = 0.143

PEAK FLOW (cms) = 0.080 (i)
TIME TO PEAK (hrs) = 6.000
RUNOFF VOLUME (mm) = 27.350
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.338

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9269)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.0900	0.0300
0.0100	0.0136	0.1200	0.0386
0.0400	0.0146	0.1500	0.0471
0.0700	0.0234	0.1800	0.0554

INFLOW : ID= 2 (2329)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1.800	0.080	6.00	27.35	
OUTFLOW: ID= 1 (9269)	1.800	0.048	6.75	27.17

PEAK FLOW REDUCTION [Qout/Qin] (%) = 60.09
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.0169

CALIB STANDHYD (2327)
ID= 1 DT=15.0 min

Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
184.00	24.00	8.00	

Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
44.16		139.84
Dep. Storage (mm) =	2.00	5.00
Average Slope (%) =	0.50	0.50
Length (m) =	1107.55	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr) =	106.66	58.90
over (min) =	15.00	30.00
Storage Coeff. (min) =	12.97 (ii)	26.19 (ii)
Unit Hyd. Tpeak (min) =	15.00	30.00
Unit Hyd. peak (cms) =	0.08	0.04

TOTALS
PEAK FLOW (cms) = 3.37 10.17 11.641 (iii)
TIME TO PEAK (hrs) = 6.00 6.25 6.25
RUNOFF VOLUME (mm) = 78.80 31.62 35.57
TOTAL RAINFALL (mm) = 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.39 0.44

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
**** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 66.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9270)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.0000	4.0000
1.1000	0.3000	3.5000	4.5000
1.8000	1.8000	4.5000	6.0000
2.0000	2.5000	5.5000	8.0000

INFLOW : ID= 2 (2327)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
184.000	11.641	6.25	35.57	
OUTFLOW: ID= 1 (9270)	184.000	2.440	7.50	35.57

PEAK FLOW REDUCTION [Qout/Qin] (%) = 20.96
TIME SHIFT OF PEAK FLOW (min) = 75.00
MAXIMUM STORAGE USED (ha. m.) = 3.1605

ADD HYD (9249)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9269):	1.80	0.048	6.75	27.17
+ ID2= 2 (9270):	184.00	2.440	7.50	35.57

ID = 3 (9249):	185.80	2.469	7.50	35.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5232)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	11.1130	4.3262
0.5630	0.0299	13.9540	4.9462
3.2190	2.3484	16.2450	5.5530
5.9950	3.0315	16.6450	5.5630
8.3150	3.6189	0.0000	0.0000

INFLOW : ID= 2 (9249) AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 185.800 2.469 7.50 35.49
 OUTFLOW: ID= 1 (5232) 185.800 2.084 9.50 35.49

PEAK FLOW REDUCTION [Qout/Qin] (%) = 84.41
 TIME SHIFT OF PEAK FLOW (min) = 120.00
 MAXIMUM STORAGE USED (ha. m.) = 2.2134

CALIB NASHYD (2322) | Area (ha) = 129.00 Curve Number (CN) = 58.1
 ID= 1 DT=15.0 min | Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.00

Unit Hyd Opeak (cms) = 2.209

PEAK FLOW (cms) = 1.317 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 21.958
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.272

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9232) | AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 1 + 2 = 3 |
 ID1= 1 (2322): 129.00 1.317 7.25 21.96
 + ID2= 2 (5232): 185.80 2.084 9.50 35.49
 ID = 3 (9232): 314.80 3.123 9.25 29.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8173) | AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 1 + 2 = 3 |
 ID1= 1 (8172): 205.47 5.904 7.50 42.80
 + ID2= 2 (9232): 314.80 3.123 9.25 29.94
 ID = 3 (8173): 520.27 7.605 7.50 35.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8174) | AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 1 + 2 = 3 |
 ID1= 1 (8170): 17664.29 153.025 9.75 33.69
 + ID2= 2 (8173): 520.27 7.605 7.50 35.02
 ID = 3 (8174): 18184.56 157.370 9.75 33.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6234) | Routing time step (min)' = 15.00
 IN= 2--> OUT= 1

<----- DATA FOR SECTION (2341.0) ----->

Distance	Elevation	Manning
0.00	228.00	0.0550
42.86	223.21	0.0550
85.72	219.56	0.0550
107.15	219.42	0.0550
128.58	219.39	0.0550
214.30	219.30	0.0550
225.02	219.26	0.0550
235.73	219.24	0.0550
255.16	219.20	0.0550 / 0.0350
257.16	217.20	0.0350
303.16	217.20	0.0350

Main Channel
Main Channel
Main Channel

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	217.70	887E+05	7.2	0.31	206.54
1.00	218.20	179E+06	22.7	0.48	131.74
1.50	218.70	272E+06	44.5	0.62	101.75
2.00	219.20	366E+06	71.8	0.75	84.97
2.59	219.79	993E+06	136.1	0.52	121.59
3.17	220.37	189E+07	258.6	0.52	121.80
3.76	220.96	294E+07	434.7	0.56	112.82
4.35	221.55	415E+07	665.9	0.61	103.88
4.93	222.13	551E+07	955.0	0.66	96.22
5.52	222.72	703E+07	1305.1	0.71	89.80
6.11	223.31	868E+07	1744.9	0.77	82.91
6.69	223.89	104E+08	2261.4	0.83	76.46
7.28	224.48	121E+08	2838.0	0.89	71.12
7.87	225.07	139E+08	3472.9	0.95	66.63
8.45	225.65	157E+08	4165.0	1.01	62.82
9.04	226.24	176E+08	4913.5	1.07	59.54
9.63	226.83	194E+08	5717.7	1.12	56.68
10.21	227.41	214E+08	6577.1	1.17	54.18
10.80	228.00	235E+08	6919.5	1.12	56.61

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	217.70	887E+05	7.2	0.31	206.54
1.00	218.20	179E+06	22.7	0.48	131.74
1.50	218.70	272E+06	44.5	0.62	101.75
2.00	219.20	366E+06	71.8	0.75	84.97
2.59	219.79	993E+06	136.1	0.52	121.59
3.17	220.37	189E+07	258.6	0.52	121.80
3.76	220.96	294E+07	434.7	0.56	112.82
4.35	221.55	415E+07	665.9	0.61	103.88
4.93	222.13	551E+07	955.0	0.66	96.22
5.52	222.72	703E+07	1305.1	0.71	89.80
6.11	223.31	868E+07	1744.9	0.77	82.91
6.69	223.89	104E+08	2261.4	0.83	76.46
7.28	224.48	121E+08	2838.0	0.89	71.12
7.87	225.07	139E+08	3472.9	0.95	66.63
8.45	225.65	157E+08	4165.0	1.01	62.82
9.04	226.24	176E+08	4913.5	1.07	59.54
9.63	226.83	194E+08	5717.7	1.12	56.68
10.21	227.41	214E+08	6577.1	1.17	54.18
10.80	228.00	235E+08	6919.5	1.12	56.61

<---- hydrograph ----> <- pipe / channel ->
 AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm) MAX DEPTH (m) MAX VEL (m/s)
 INFLOW : ID= 2 (8174) ***** 157.37 9.75 33.73 2.69 0.52
 OUTFLOW: ID= 1 (6234) ***** 132.26 12.25 33.73 2.54 0.54

CALIB NASHYD (2347) | Area (ha) = 266.10 Curve Number (CN) = 56.0
 ID= 1 DT=15.0 min | Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 2.00

Unit Hyd Opeak (cms) = 2.276

PEAK FLOW (cms) = 1.499 (i)
 TIME TO PEAK (hrs) = 9.000
 RUNOFF VOLUME (mm) = 20.789
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.257

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9281) | IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	13.3100	4.0635
0.9000	1.7152	17.8000	5.3537
5.1600	1.7685	22.3500	6.6446
9.6100	3.0976	26.0300	7.9060

INFLOW : ID= 2 (2347)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
266.100	266.100	1.499	9.00	20.79
OUTFLOW: ID= 1 (9281)	266.100	1.543	11.75	20.79

PEAK FLOW REDUCTION [Qout/Qin] (%) = 102.96
 TIME SHIFT OF PEAK FLOW (min) = 165.00
 MAXIMUM STORAGE USED (ha. m.) = 1.7474

**** WARNING : HYDROGRAPH PEAK WAS NOT REDUCED.
 CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.

RESERVOIR (5234) | IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW	STORAGE	OUTFLOW	STORAGE
---------	---------	---------	---------

(cms)	(ha. m.)	(cms)	(ha. m.)
0.0000	0.0000	0.5630	0.1745
0.1300	0.0819	0.6550	0.1995
0.2420	0.0947	0.7760	0.2240
0.3350	0.1223	1.0550	0.2340
0.4480	0.1460	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9281)	266.100	1.543	11.75	20.79
OUTFLOW: ID= 1 (5234)	266.100	1.364	12.00	20.79

PEAK FLOW REDUCTION [Qout/Oin] (%) = 88.40
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 0.2538

CALIB STANDHYD (2342) ID= 1 DT=15.0 min	Area Total Imp (%) = 22.00	(ha) = 1.06	Di r. Conn. (%) = 8.00
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	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) =	0.23	0.83
Dep. Storage (mm) =	1.00	5.00
Average Slope (%) =	1.00	2.00
Length (m) =	84.06	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr) =	106.66	24.97
over (min) =	15.00	15.00
Storage Coeff. (min) =	2.24 (ii)	14.54 (ii)
Unit Hyd. Tpeak (min) =	15.00	15.00
Unit Hyd. peak (cms) =	0.11	0.07

TOTALS
 PEAK FLOW (cms) = 0.03
 TIME TO PEAK (hrs) = 6.00
 RUNOFF VOLUME (mm) = 79.80
 TOTAL RAINFALL (mm) = 80.80
 RUNOFF COEFFICIENT = 0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 39.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9234) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2342):	1.06	0.065	6.00	19.60
+ ID2= 2 (5234):	266.10	1.364	12.00	20.79
ID = 3 (9234):	267.16	1.366	12.00	20.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8176) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6234):	18184.56	132.264	12.25	33.73
+ ID2= 2 (9234):	267.16	1.366	12.00	20.78
ID = 3 (8176):	18451.72	133.602	12.00	33.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2369) ID= 1 DT=15.0 min	Area Total Imp (%) = 23.00	(ha) = 441.90	Di r. Conn. (%) = 8.00
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IMPERVIOUS PERVIOUS (i)

Surface Area (ha) =	101.64	340.26
Dep. Storage (mm) =	2.00	5.00
Average Slope (%) =	0.50	0.50
Length (m) =	1716.39	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr) =	106.66	62.59
over (min) =	15.00	30.00
Storage Coeff. (min) =	15.87 (ii)	29.78 (ii)
Unit Hyd. Tpeak (min) =	15.00	30.00
Unit Hyd. peak (cms) =	0.07	0.04

TOTALS
 PEAK FLOW (cms) = 7.24
 TIME TO PEAK (hrs) = 6.00
 RUNOFF VOLUME (mm) = 78.80
 TOTAL RAINFALL (mm) = 80.80
 RUNOFF COEFFICIENT = 0.98

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9266) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	2.3000	8.0000
	0.2000	2.0000	3.5000	10.0000
	1.9000	7.0000	4.5000	13.0000
	2.0000	7.5000	5.5000	14.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2369)	441.900	28.431	6.25	37.68
OUTFLOW: ID= 1 (9266)	441.900	3.792	8.50	37.67

PEAK FLOW REDUCTION [Qout/Oin] (%) = 13.34
 TIME SHIFT OF PEAK FLOW (min) = 135.00
 MAXIMUM STORAGE USED (ha. m.) = 10.8896

RESERVOIR (5236) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	3.1380	1.5897
	1.0220	0.7542	3.7950	2.0204
	1.7530	0.8200	4.4970	2.4373
	2.3290	1.2567	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9266)	441.900	3.792	8.50	37.67
OUTFLOW: ID= 1 (5236)	441.900	3.437	10.75	37.67

PEAK FLOW REDUCTION [Qout/Oin] (%) = 90.64
 TIME SHIFT OF PEAK FLOW (min) = 135.00
 MAXIMUM STORAGE USED (ha. m.) = 1.7877

CALIB NASHYD (2362) ID= 1 DT=15.0 min	Area Ia U. H. Tp (hrs) =	(ha) = 52.59 (mm) = 5.00 1.24	Curve Number (CN) = 58.7 # of Linear Res. (N) = 1.50
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Unit Hyd Opeak (cms) = 0.726

PEAK FLOW (cms) =	0.465 (i)
TIME TO PEAK (hrs) =	8.000
RUNOFF VOLUME (mm) =	22.407
TOTAL RAINFALL (mm) =	80.800
RUNOFF COEFFICIENT =	0.277

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9236) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2362):	52.59	0.465	8.00	22.41
+ ID2= 2 (5236):	441.90	3.437	10.75	37.67
=====				
ID = 3 (9236):	494.49	3.778	10.50	36.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6238)
IN= 2---> OUT= 1 Routing time step (min)' = 15.00

----- DATA FOR SECTION (2381.0) -----				
Distance	Elevation	Manning		
0.00	241.54	0.0380		
602.55	227.00	0.0380		
1702.00	224.50	0.0380		
1721.25	223.00	0.0380		
1725.10	222.60	0.0380		
1780.94	222.50	0.0380		
1782.87	222.45	0.0380		
1784.79	222.40	0.0380 / 0.0300	Main Channel	
1786.57	221.75	0.0300	Main Channel	
1787.07	220.75	0.0300	Main Channel	
1787.57	220.75	0.0300	Main Channel	
1791.57	220.75	0.0300	Main Channel	
1794.07	221.75	0.0300	Main Channel	
1798.27	222.00	0.0300	Main Channel	
1800.19	222.35	0.0300 / 0.0380	Main Channel	
1802.12	222.47	0.0380		
1840.63	223.00	0.0380		
1900.00	225.00	0.0380		
1950.00	226.00	0.0380		
2242.61	240.00	0.0380		

----- TRAVEL TIME TABLE -----						
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)	
0.80	221.55	.101E+05	5.2	1.14	32.36	
1.60	222.35	.296E+05	18.7	1.40	26.36	
2.64	223.39	.249E+06	151.8	1.35	27.33	
3.68	224.43	.613E+06	512.2	1.85	19.94	
4.71	225.46	.152E+07	1066.8	1.55	23.80	
5.75	226.50	.359E+07	2824.9	1.75	21.16	
6.79	227.54	.663E+07	6528.0	2.18	16.93	
7.83	228.58	.994E+07	12158.6	2.71	13.62	
8.87	229.62	.134E+08	19268.8	3.19	11.58	
9.91	230.66	.170E+08	27785.3	3.63	10.19	
10.94	231.69	.207E+08	37667.1	4.03	9.18	
11.98	232.73	.246E+08	48891.7	4.40	8.40	
13.02	233.77	.287E+08	61448.0	4.75	7.78	
14.06	234.81	.329E+08	75332.7	5.08	7.28	
15.10	235.85	.372E+08	90547.8	5.39	6.85	
16.14	236.89	.417E+08	*****	5.69	6.49	
17.17	237.92	.464E+08	*****	5.98	6.18	
18.21	238.96	.512E+08	*****	6.25	5.91	
19.25	240.00	.561E+08	*****	6.52	5.67	

----- hydrograph -----					<- pipe / channel ->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (9236)	494.49	3.78	10.50	36.05	0.58	1.14
OUTFLOW: ID= 1 (6238)	494.49	3.74	11.00	36.05	0.57	1.14

CALIB STANDHYD (2389)
ID= 1 DT=15.0 mi n

Area (ha)	Imp (%)	IMPERVIOUS	PERVIOUS (i)
4.70	21.00		
Di r. Conn. (%) = 7.00			
Surface Area (ha) = 0.99			3.71
Dep. Storage (mm) = 2.00			5.00
Average Slope (%) = 0.50			0.50
Length (m) = 177.01			40.00
Mannings n = 0.013			0.250

Max. Eff. Inten. (mm/hr)=	106.66	61.07
Storage over (mi n)	15.00	30.00
Storage Coeff. (mi n)=	4.32 (ii)	17.35 (ii)
Unit Hyd. Tpeak (mi n)=	15.00	30.00
Unit Hyd. peak (cms)=	0.11	0.05

PEAK FLOW (cms)=	0.10	0.34	*TOTALS* 0.354 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.25
RUNOFF VOLUME (mm)=	78.80	33.78	36.93
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.42	0.46

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9267)
IN= 2---> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.2300	0.0785
0.0200	0.0364	0.3100	0.1005
0.0900	0.0393	0.3900	0.1220
0.1700	0.0616	0.4600	0.1429

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2389)	4.700	0.354	6.25	36.93
OUTFLOW: ID= 1 (9267)	4.700	0.182	6.50	36.84

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 51.39
TIME SHI FT OF PEAK FLOW (mi n) = 15.00
MAXI MUM STORAGE USED (ha. m.) = 0.0665

CALIB NASHYD (2387)
ID= 1 DT=15.0 mi n

Area (ha) = 90.20 Curve Number (CN) = 54.0
Ia (mm) = 5.00 # of Li near Res. (N) = 1.50
U. H. Tp (hrs) = 0.86

Unit Hyd Opeak (cms)=	1.791
PEAK FLOW (cms)=	0.911 (i)
TIME TO PEAK (hrs)=	7.000
RUNOFF VOLUME (mm)=	19.414
TOTAL RAINFALL (mm)=	80.800
RUNOFF COEFFICIENT =	0.240

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9268)
IN= 2---> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.5100	1.3564
0.3100	0.5624	6.0400	1.7959
1.7500	0.5748	7.5800	2.2374
3.2600	1.0285	8.8300	2.6688

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2387)	90.200	0.911	7.00	19.41
OUTFLOW: ID= 1 (9268)	90.200	0.793	8.50	19.41

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 87.06
TIME SHI FT OF PEAK FLOW (mi n) = 90.00
MAXI MUM STORAGE USED (ha. m.) = 0.5677

ADD HYD (9247)
1 + 2 = 3

AREA OPEAK TPEAK R. V.

	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (9267):	4.70	0.182	6.50	36.84
+ ID2= 2 (9268):	90.20	0.793	8.50	19.41

ID = 3 (9247):	94.90	0.827	8.50	20.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5238)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.8780	1.5095
0.1960	0.7083	4.8600	1.7258
1.1230	0.8194	5.6680	1.9375
2.0920	1.0578	6.0680	1.9475
2.9010	1.2627	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9247)	94.900	0.827	8.50	20.27
OUTFLOW: ID= 1 (5238)	94.900	0.406	11.50	20.26

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 49.07
TIME SHIFT OF PEAK FLOW (min) = 180.00
MAXIMUM STORAGE USED (ha.m.) = 0.7337

CALIB NASHYD (2382)
ID= 1 DT=15.0 min

Area (ha) =	216.80	Curve Number (CN) =	56.8
Ia (mm) =	5.00	# of Linear Res. (N) =	1.50
U.H. Tp (hrs) =	1.56		

Unit Hyd Opeak (cms) = 2.368

PEAK FLOW (cms) = 1.515 (i)
TIME TO PEAK (hrs) = 8.250
RUNOFF VOLUME (mm) = 21.248
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.263

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9238)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2382):	216.80	1.515	8.25	21.25
+ ID2= 2 (5238):	94.90	0.406	11.50	20.26

ID = 3 (9238):	311.70	1.611	11.25	20.95

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8180)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6238):	494.49	3.744	11.00	36.05
+ ID2= 2 (9238):	311.70	1.611	11.25	20.95

ID = 3 (8180):	806.19	5.354	11.25	30.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8178)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8176):	18451.72	133.602	12.00	33.54
+ ID2= 2 (8180):	806.19	5.354	11.25	30.21

ID = 3 (8178):	19257.91	138.760	12.00	33.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6240)
IN= 2--> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (2401.0) ----->

Distance	Elevation	Manning	
0.00	222.00	0.0450	
11.46	221.00	0.0450	
208.98	221.00	0.0450	
404.04	220.97	0.0450	
808.08	220.83	0.0450	
905.60	220.17	0.0450	
919.53	219.43	0.0450	
933.47	219.22	0.0450	
945.26	219.21	0.0450 / 0.0300	Main Channel
946.26	217.81	0.0300	Main Channel
975.26	217.81	0.0300	Main Channel
1003.26	217.81	0.0300	Main Channel
1005.26	219.21	0.0300 / 0.0450	Main Channel
1017.06	219.28	0.0450	
1030.99	219.26	0.0450	
1044.92	219.23	0.0450	
1058.86	219.23	0.0450	
1253.91	219.22	0.0450	
1323.57	221.05	0.0450	
1379.30	222.00	0.0450	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.20	218.01	.307E+05	2.2	0.20	227.52
0.40	218.21	.617E+05	7.1	0.31	143.95
0.60	218.41	.928E+05	14.0	0.41	110.33
0.80	218.61	.124E+06	22.6	0.49	91.46
1.00	218.81	.156E+06	32.8	0.57	79.15
1.20	219.01	.188E+06	44.5	0.64	70.38
1.40	219.21	.220E+06	57.5	0.70	63.77
1.63	219.44	.413E+06	82.5	0.54	83.52
1.87	219.68	.632E+06	120.9	0.51	87.03
2.10	219.91	.858E+06	170.0	0.53	84.11
2.33	220.14	.109E+07	228.7	0.56	79.62
2.56	220.37	.134E+07	294.4	0.59	76.03
2.80	220.61	.162E+07	371.1	0.61	72.75
3.03	220.84	.192E+07	456.8	0.64	70.18
3.26	221.07	.251E+07	552.1	0.59	75.74
3.49	221.30	.333E+07	711.8	0.57	78.07
3.73	221.54	.417E+07	907.0	0.58	76.62
3.96	221.77	.502E+07	1134.1	0.61	73.70
4.19	222.00	.587E+07	1390.9	0.64	70.34

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8178)	*****	138.76	12.00	33.40	1.95	0.52
OUTFLOW: ID= 1 (6240)	*****	128.73	13.75	33.40	1.90	0.52

CALIB NASHYD (Q240)
ID= 1 DT=15.0 min

Area (ha) =	345.47	Curve Number (CN) =	46.0
Ia (mm) =	5.00	# of Linear Res. (N) =	1.50
U.H. Tp (hrs) =	3.60		

Unit Hyd Opeak (cms) = 1.637

PEAK FLOW (cms) = 0.908 (i)
TIME TO PEAK (hrs) = 12.000
RUNOFF VOLUME (mm) = 15.341
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.190

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2407)
ID= 1 DT=15.0 min

Area (ha) =	88.90	Dir. Conn. (%) =	7.00
Total Imp (%) =	20.00		

Surface Area (ha) = IMPERVIOUS 17.78 PERVIOUS (i) 71.12

Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 769.85 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 106.66 53.47
 over (min) 15.00 30.00
 Storage Coeff. (min)= 10.43 (ii) 24.17 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.04

TOTALS
 PEAK FLOW (cms)= 1.54 4.88 5.440 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.25
 RUNOFF VOLUME (mm)= 78.80 30.14 33.55
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.37 0.42

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9273)
 IN= 2--> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	4.4500	5.4723
	0.3000	4.6746	5.9500	5.8930
	1.7300	4.7225	7.4700	6.3047
	3.2100	5.1511	8.7000	6.7071

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2407)	88.900	5.440	6.25	33.55
OUTFLOW: ID= 1 (9273)	88.900	0.168	12.25	33.50

PEAK FLOW REDUCTION [Qout/Qin] (%) = 3.09
 TIME SHIFT OF PEAK FLOW (min) = 360.00
 MAXIMUM STORAGE USED (ha.m.) = 2.6236

ADD HYD (9252)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0240):	345.47	0.908	12.00	15.34
+ ID2= 2 (9273):	88.90	0.168	12.25	33.50

ID = 3 (9252):	434.37	1.076	12.00	19.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8182)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6240):	19257.91	128.735	13.75	33.40
+ ID2= 2 (9252):	434.37	1.076	12.00	19.06

ID = 3 (8182):	19692.28	129.745	13.75	33.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (2427)
 ID= 1 DT=15.0 min

Area (ha)= 5.40 Curve Number (CN)= 74.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.37

Unit Hyd Opeak (cms)= 0.249

PEAK FLOW (cms)= 0.187 (i)
 TIME TO PEAK (hrs)= 6.250

RUNOFF VOLUME (mm)= 33.279
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.412

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9274)
 IN= 2--> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	0.2700	0.0857
	0.0200	0.0379	0.3600	0.1115
	0.1000	0.0399	0.4500	0.1369
	0.1900	0.0663	0.5300	0.1618

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2427)	5.400	0.187	6.25	33.28
OUTFLOW: ID= 1 (9274)	5.400	0.134	7.25	33.19

PEAK FLOW REDUCTION [Qout/Qin] (%) = 71.88
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0503

RESERVOIR (5242)
 IN= 2--> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	0.0480	1.0185
	0.0020	1.0087	0.0600	1.0212
	0.0140	1.0101	0.0700	1.0238
	0.0260	1.0130	0.4700	1.0338
	0.0360	1.0155	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9274)	5.400	0.134	7.25	33.19
OUTFLOW: ID= 1 (5242)	5.400	0.000	34.25	9.39

PEAK FLOW REDUCTION [Qout/Qin] (%) = 0.26
 TIME SHIFT OF PEAK FLOW (min) = *****
 MAXIMUM STORAGE USED (ha.m.) = 0.1760

CALIB
 NASHYD (2422)
 ID= 1 DT=15.0 min

Area (ha)= 652.48 Curve Number (CN)= 51.6
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 5.37

Unit Hyd Opeak (cms)= 2.073

PEAK FLOW (cms)= 1.446 (i)
 TIME TO PEAK (hrs)= 12.750
 RUNOFF VOLUME (mm)= 18.280
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.226

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9242)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2422):	652.48	1.446	12.75	18.28
+ ID2= 2 (5242):	5.40	0.000	34.25	9.39

ID = 3 (9242):	657.88	1.446	12.75	18.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8184)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8182):	19692.28	129.745	13.75	33.09

+ ID2= 2 (9242): 657.88 1.446 12.75 18.21

 ID = 3 (8184): 20350.16 131.181 13.75 32.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6244)
 IN= 2--> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (2441.0) ----->

Distance	Elevation	Manning	
0.00	225.00	0.0450	
2.50	220.10	0.0450	
3.00	220.06	0.0450	
3.64	220.07	0.0450	
7.28	220.08	0.0450	
10.91	220.09	0.0450	
14.55	219.81	0.0450	
18.19	219.29	0.0450	
24.13	219.21	0.0450 / 0.0300	Main Channel
24.63	217.81	0.0300	Main Channel
69.13	217.81	0.0300	Main Channel
114.13	217.81	0.0300	Main Channel
115.13	219.21	0.0300 / 0.0450	Main Channel
120.06	219.25	0.0450	
123.70	219.32	0.0450	
127.34	219.77	0.0450	
130.98	219.88	0.0450	
134.61	219.99	0.0450	
350.00	220.18	0.0450	
360.18	225.00	0.0450	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.35	218.16	.108E+06	9.0	0.29	200.52
0.70	218.51	.216E+06	28.4	0.45	126.89
1.05	218.86	.325E+06	55.7	0.59	97.27
1.40	219.21	.434E+06	89.7	0.71	80.65
1.79	219.60	.574E+06	135.9	0.81	70.36
2.17	219.98	.726E+06	190.8	0.90	63.38
2.56	220.37	.111E+07	264.3	0.82	69.82
2.94	220.75	.157E+07	371.7	0.81	70.36
3.33	221.14	.203E+07	503.4	0.85	67.30
3.72	221.53	.250E+07	656.6	0.90	63.40
4.10	221.91	.296E+07	829.6	0.96	59.54
4.49	222.30	.343E+07	1021.1	1.02	56.00
4.87	222.68	.390E+07	1230.2	1.08	52.83
5.26	223.07	.437E+07	1456.2	1.14	50.01
5.65	223.46	.484E+07	1698.4	1.20	47.51
6.03	223.84	.531E+07	1956.3	1.26	45.28
6.42	224.23	.579E+07	2229.3	1.32	43.28
6.80	224.61	.626E+07	2517.0	1.38	41.48
7.19	225.00	.674E+07	2819.2	1.44	39.85

<----- hydrograph -----> <-- pi pe / channel -->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8184)	*****	131.18	13.75	32.61	1.75
OUTFLOW: ID= 1 (6244)	*****	126.51	14.50	32.61	0.79

CALIB STANDHYD (2441)
 ID= 1 DT=15.0 min

Area (ha)= 91.52
 Total Imp(%)= 45.00 Di r. Conn.(%)= 15.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	41.18	50.34
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	781.11	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) over (min)	106.66	77.42	15.00	30.00
Storage Coeff. (min)	10.52 (ii)	22.37 (ii)	15.00	30.00
Unit Hyd. Tpeak (min)	15.00	30.00		
Unit Hyd. peak (cms)	0.09	0.04		

TOTALS
 PEAK FLOW (cms)= 3.38 5.21 6.704 (iii)

TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 78.80 32.86 39.75
 TOTAL RAI NFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.41 0.49

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 60.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2449)
 ID= 1 DT=15.0 min

Area (ha)= 143.20
 Total Imp(%)= 34.00 Di r. Conn.(%)= 17.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	48.69	94.51
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	977.07	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (min)	106.66	68.08
Storage Coeff. (min)	12.03 (ii)	24.51 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04

PEAK FLOW (cms)= 5.73 8.24 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.25 10.974 (iii)
 RUNOFF VOLUME (mm)= 78.80 35.22 42.63
 TOTAL RAI NFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.44 0.53

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9265)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	7.1600	2.7857
0.4900	1.4537	9.5800	3.4259
2.7800	1.6401	12.0300	4.0179
5.1700	2.2743	14.0100	4.5968

AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 INFLOW: ID= 2 (2449) 143.200 10.974 6.00 42.63
 OUTFLOW: ID= 1 (9265) 143.200 5.766 6.75 42.62

PEAK FLOW REDUCTION [Out/Oi n](%)= 52.54
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha. m.)= 2.4646

ADD HYD (9245)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2441):	91.52	6.704	6.00	39.75
+ ID2= 2 (9265):	143.20	5.766	6.75	42.62
ID = 3 (9245):	234.72	10.736	6.25	41.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5244)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	15.7000	11.1120
0.7950	7.8678	19.7140	11.9879
4.5470	8.3178	22.9500	12.8451
8.4690	9.2829	23.3500	12.8551
11.7470	10.1128	0.0000	0.0000

INFLOW : ID= 2 (9245)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
234.720	234.720	10.736	6.25	41.50
OUTFLOW: ID= 1 (5244)	234.720	0.696	12.25	41.49

PEAK FLOW REDUCTION [Qout/Qin](%) = 6.48
 TIME SHIFT OF PEAK FLOW (min) = 360.00
 MAXIMUM STORAGE USED (ha.m.) = 6.8880

CALIB NASHYD (2442)
 ID= 1 DT=15.0 min

Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1674.00	10.483	9.50	24.81
U.H. Tp(hrs)= 2.20	0.696	12.25	41.49

Unit Hyd Opeak (cms) = 12.991

PEAK FLOW (cms) = 10.483 (i)
 TIME TO PEAK (hrs) = 9.500
 RUNOFF VOLUME (mm) = 24.814
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.307

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9244)
 1 + 2 = 3

ID	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2442):	1674.00	10.483	9.50	24.81
+ ID2= 2 (5244):	234.72	0.696	12.25	41.49
ID = 3 (9244):	1908.72	11.148	9.75	26.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8186)
 1 + 2 = 3

ID	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6244):	20350.16	126.510	14.50	32.61
+ ID2= 2 (9244):	1908.72	11.148	9.75	26.87
ID = 3 (8186):	22258.88	134.375	14.00	32.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ HYD (8190)
 DT=15.0 min

AREA (ha) = *****
 TPEAK (hrs) = 0.00
 VOLUME (mm) = 0.00

Filename: S:\2010 Projects\D-EE (E10)\NR\W10-487 East Gwillimbury SWM Master Plan\Calcs\Hydrologi c. Model s\Ex Comments:

HOWEVER THE TABLE BELOW AND HYDROGRAPH FILES DO.

TIME hrs	ACTUAL FLOW cms	PEAK FLOW cms	PEAK TIME hrs	BASE FLOW cms	ACTUAL + BASE FLOW cms
0.00	65.00	130.00	195.00	260.00	260.00
0.25	65.25	130.25	195.25	260.25	260.25
0.50	65.50	130.50	195.50	260.50	260.50
0.75	65.75	130.75	195.75	260.75	260.75
1.00	66.00	131.00	196.00	261.00	261.00
1.25	66.25	131.25	196.25	261.25	261.25
1.50	66.50	131.50	196.50	261.50	261.50
1.75	66.75	131.75	196.75	261.75	261.75
2.00	67.00	132.00	197.00	262.00	262.00

2.25	67.25	132.25	197.25	262.25	262.25
2.50	67.50	132.50	197.50	262.50	262.50
2.75	67.75	132.75	197.75	262.75	262.75
3.00	68.00	133.00	198.00	263.00	263.00
3.25	68.25	133.25	198.25	263.25	263.25
3.50	68.50	133.50	198.50	263.50	263.50
3.75	68.75	133.75	198.75	263.75	263.75
4.00	69.00	134.00	199.00	264.00	264.00
4.25	69.25	134.25	199.25	264.25	264.25
4.50	69.50	134.50	199.50	264.50	264.50
4.75	69.75	134.75	199.75	264.75	264.75
5.00	70.00	135.00	200.00	265.00	265.00
5.25	70.25	135.25	200.25	265.25	265.25
5.50	70.50	135.50	200.50	265.50	265.50
5.75	70.75	135.75	200.75	265.75	265.75
6.00	71.00	136.00	201.00	266.00	266.00
6.25	71.25	136.25	201.25	266.25	266.25
6.50	71.50	136.50	201.50	266.50	266.50
6.75	71.75	136.75	201.75	266.75	266.75
7.00	72.00	137.00	202.00	267.00	267.00
7.25	72.25	137.25	202.25	267.25	267.25
7.50	72.50	137.50	202.50	267.50	267.50
7.75	72.75	137.75	202.75	267.75	267.75
8.00	73.00	138.00	203.00	268.00	268.00
8.25	73.25	138.25	203.25	268.25	268.25
8.50	73.50	138.50	203.50	268.50	268.50
8.75	73.75	138.75	203.75	268.75	268.75
9.00	74.00	139.00	204.00	269.00	269.00
9.25	74.25	139.25	204.25	269.25	269.25
9.50	74.50	139.50	204.50	269.50	269.50
9.75	74.75	139.75	204.75	269.75	269.75
10.00	75.00	140.00	205.00	270.00	270.00
10.25	75.25	140.25	205.25	270.25	270.25
10.50	75.50	140.50	205.50	270.50	270.50
10.75	75.75	140.75	205.75	270.75	270.75
11.00	76.00	141.00	206.00	271.00	271.00
11.25	76.25	141.25	206.25	271.25	271.25
11.50	76.50	141.50	206.50	271.50	271.50
11.75	76.75	141.75	206.75	271.75	271.75
12.00	77.00	142.00	207.00	272.00	272.00
12.25	77.25	142.25	207.25	272.25	272.25
12.50	77.50	142.50	207.50	272.50	272.50
12.75	77.75	142.75	207.75	272.75	272.75
13.00	78.00	143.00	208.00	273.00	273.00
13.25	78.25	143.25	208.25	273.25	273.25
13.50	78.50	143.50	208.50	273.50	273.50
13.75	78.75	143.75	208.75	273.75	273.75
14.00	79.00	144.00	209.00	274.00	274.00
14.25	79.25	144.25	209.25	274.25	274.25
14.50	79.50	144.50	209.50	274.50	274.50
14.75	79.75	144.75	209.75	274.75	274.75
15.00	80.00	145.00	210.00	275.00	275.00
15.25	80.25	145.25	210.25	275.25	275.25
15.50	80.50	145.50	210.50	275.50	275.50
15.75	80.75	145.75	210.75	275.75	275.75
16.00	81.00	146.00	211.00	276.00	276.00
16.25	81.25	146.25	211.25	276.25	276.25
16.50	81.50	146.50	211.50	276.50	276.50
16.75	81.75	146.75	211.75	276.75	276.75
17.00	82.00	147.00	212.00	277.00	277.00
17.25	82.25	147.25	212.25	277.25	277.25
17.50	82.50	147.50	212.50	277.50	277.50
17.75	82.75	147.75	212.75	277.75	277.75
18.00	83.00	148.00	213.00	278.00	278.00
18.25	83.25	148.25	213.25	278.25	278.25
18.50	83.50	148.50	213.50	278.50	278.50
18.75	83.75	148.75	213.75	278.75	278.75
19.00	84.00	149.00	214.00	279.00	279.00
19.25	84.25	149.25	214.25	279.25	279.25
19.50	84.50	149.50	214.50	279.50	279.50
19.75	84.75	149.75	214.75	279.75	279.75
20.00	85.00	150.00	215.00	280.00	280.00
20.25	85.25	150.25	215.25	280.25	280.25
20.50	85.50	150.50	215.50	280.50	280.50
20.75	85.75	150.75	215.75	280.75	280.75
21.00	86.00	151.00	216.00	281.00	281.00
21.25	86.25	151.25	216.25	281.25	281.25
21.50	86.50	151.50	216.50	281.50	281.50
21.75	86.75	151.75	216.75	281.75	281.75
22.00	87.00	152.00	217.00	282.00	282.00
22.25	87.25	152.25	217.25	282.25	282.25
22.50	87.50	152.50	217.50	282.50	282.50
22.75	87.75	152.75	217.75	282.75	282.75
23.00	88.00	153.00	218.00	283.00	283.00

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8186):	22258.88	134.375	14.00	32.11
+ ID2= 2 (8190):	20350.16	126.510	14.50	32.61
=====				
ID = 3 (8188):	42609.04	260.744	14.00	32.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
(4462)	1238.89	60.9	1.50
Imp (%) =	5.00		
U. H. Tp (hrs) =	4.02		

Unit Hyd Opeak (cms) = 5.256
 PEAK FLOW (cms) = 4.668 (i)
 TIME TO PEAK (hrs) = 12.000
 RUNOFF VOLUME (mm) = 24.022
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.297

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
(4461)	236.12	43.0	1.50
Total Imp (%) =	47.00		
Di r. Conn. (%) =	17.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) =	110.98	125.14
Dep. Storage (mm) =	2.00	5.00
Average Slope (%) =	0.50	0.50
Length (m) =	1254.64	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr) =	106.66	79.13
over (min) =	15.00	30.00
Storage Coeff. (min) =	13.98 (ii)	25.73 (ii)
Unit Hyd. Tpeak (min) =	15.00	30.00
Unit Hyd. peak (cms) =	0.07	0.04

TOTALS
 PEAK FLOW (cms) = 8.92 12.38 16.774 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 78.80 33.14 40.90
 TOTAL RAINFALL (mm) = 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.41 0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 60.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5446) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	10.1800	6.2621
	0.8000	2.9957	12.5050	7.1218
	3.0350	3.4813	14.4090	7.9633
	5.6510	4.4353	14.8090	7.9733
	7.7470	5.2683	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (4461)	236.120	16.774	6.00	40.90
OUTFLOW: ID= 1 (5446)	236.120	6.305	7.00	40.90

PEAK FLOW REDUCTION [Qout/Qin] (%) = 37.59
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 4.7288

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4462):	1238.89	4.668	12.00	24.02
+ ID2= 2 (5446):	236.12	6.305	7.00	40.90
=====				
ID = 3 (8192):	1475.01	8.982	7.00	26.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8188):	42609.04	260.744	14.00	32.35
+ ID2= 2 (8192):	1475.01	8.982	7.00	26.72
=====				
ID = 3 (8194):	44084.05	265.826	14.00	32.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
(0442)	117.26	43.0	1.50
Imp (%) =	5.00		
U. H. Tp (hrs) =	1.17		

Unit Hyd Opeak (cms) = 1.705
 PEAK FLOW (cms) = 0.655 (i)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 13.817
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.171

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
(0440)	226.35	60.0	1.50
Imp (%) =	5.00		
U. H. Tp (hrs) =	1.11		

Unit Hyd Opeak (cms) = 3.481
 PEAK FLOW (cms) = 2.254 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 23.234
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.288

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
(0438)	130.70	51.0	1.50
Imp (%) =	5.00		
U. H. Tp (hrs) =	0.60		

Unit Hyd Opeak (cms) = 3.738
 PEAK FLOW (cms) = 1.586 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 17.571
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.217

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
(0436)	187.51	55.0	1.50
Imp (%) =	5.00		
U. H. Tp (hrs) =	0.73		

Unit Hyd Qpeak (cms) = 4.391

PEAK FLOW (cms) = 2.215 (i)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 19.928
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.247

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0434) Area (ha) = 56.64 Curve Number (CN) = 46.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U.H. Tp(hrs) = 0.52

Unit Hyd Qpeak (cms) = 1.867

PEAK FLOW (cms) = 0.654 (i)
TIME TO PEAK (hrs) = 6.500
RUNOFF VOLUME (mm) = 14.950
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.185

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8206) 1 + 2 = 3 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 1 (0434): 56.64 0.654 6.50 14.95
+ ID2= 2 (0436): 187.51 2.215 7.00 19.93
ID = 3 (8206): 244.15 2.853 6.75 18.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6438) IN= 2----> OUT= 1 Routing time step (min)' = 15.00

DATA FOR SECTION (4381.0) ----->

Distance	Elevation	Manning	
0.00	256.00	0.0600	
9.23	255.50	0.0600	
27.69	255.00	0.0600	
50.77	254.90	0.0600	
64.62	254.85	0.0600	
78.47	252.26	0.0600	
96.93	249.44	0.0600	
129.24	245.94	0.0600	
145.40	245.76	0.0600	
146.20	245.80	0.0600 / 0.0350	Main Channel
146.70	245.30	0.0350	Main Channel
147.70	245.30	0.0350	Main Channel
148.20	245.80	0.0350 / 0.0600	Main Channel
150.01	245.78	0.0600	
152.32	245.79	0.0600	
163.86	246.72	0.0600	
175.40	249.07	0.0600	
186.94	253.15	0.0600	
196.17	255.38	0.0600	
228.48	255.55	0.0600	

TRAVEL TIME TABLE <----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.50	245.80	.221E+04	0.9	1.02	40.44
1.01	246.31	.352E+05	14.0	0.99	41.77
1.53	246.83	.841E+05	47.3	1.39	29.61
2.04	247.34	.144E+06	102.0	1.76	23.49
2.55	247.85	.213E+06	177.3	2.07	19.99
3.07	248.37	.291E+06	274.4	2.34	17.66
3.58	248.88	.378E+06	394.6	2.58	15.97
4.09	249.39	.474E+06	541.4	2.83	14.60
4.61	249.91	.578E+06	719.9	3.08	13.38
5.12	250.42	.688E+06	923.8	3.33	12.40
5.63	250.93	.803E+06	1153.3	3.56	11.61
6.14	251.44	.925E+06	1408.6	3.77	10.95

	6.66	251.96	.105E+07	1690.6	3.98	10.38
	7.17	252.47	.119E+07	2002.3	4.18	9.88
	7.68	252.98	.133E+07	2345.4	4.38	9.43
	8.20	253.50	.147E+07	2710.1	4.56	9.05
	8.71	254.01	.162E+07	3101.0	4.73	8.72
	9.22	254.52	.178E+07	3521.8	4.90	8.43
	9.74	255.04	.195E+07	3467.6	4.39	9.39

<---- hydrograph ----> <-pi pe / channel ->
INFLOW : ID= 2 (8206) AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm) MAX DEPTH (m) MAX VEL (m/s)
OUTFLOW: ID= 1 (6438) 244.15 2.85 6.75 18.77 0.58 1.02
244.15 2.50 7.75 18.77 0.56 1.02

ADD HYD (8208) 1 + 2 = 3 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 1 (0438): 130.70 1.586 6.75 17.57
+ ID2= 2 (6438): 244.15 2.498 7.75 18.77
ID = 3 (8208): 374.85 3.861 7.25 18.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8210) 1 + 2 = 3 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 1 (0440): 226.35 2.254 7.50 23.23
+ ID2= 2 (8208): 374.85 3.861 7.25 18.35
ID = 3 (8210): 601.20 6.108 7.50 20.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0432) Area (ha) = 114.21 Curve Number (CN) = 53.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U.H. Tp(hrs) = 1.21

Unit Hyd Qpeak (cms) = 1.617

PEAK FLOW (cms) = 0.865 (i)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 18.938
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.234

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0430) Area (ha) = 111.64 Curve Number (CN) = 55.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U.H. Tp(hrs) = 0.52

Unit Hyd Qpeak (cms) = 3.687

PEAK FLOW (cms) = 1.726 (i)
TIME TO PEAK (hrs) = 6.500
RUNOFF VOLUME (mm) = 19.711
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.244

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0428) Area (ha) = 50.53 Curve Number (CN) = 51.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U.H. Tp(hrs) = 0.50

Unit Hyd Qpeak (cms) = 1.725

PEAK FLOW (cms)= 0.707 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 17.454
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.216

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0426) Area (ha)= 247.17 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.98

Unit Hyd Qpeak (cms)= 4.315

PEAK FLOW (cms)= 2.796 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 23.867
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.295

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0424) Area (ha)= 49.57 Curve Number (CN)= 59.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.37

Unit Hyd Qpeak (cms)= 2.293

PEAK FLOW (cms)= 1.085 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 21.765
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.269

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8198)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0424):	49.57	1.085	6.50	21.77
+ ID2= 2 (0426):	247.17	2.796	7.25	23.87
ID = 3 (8198):	296.74	3.662	6.75	23.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6428)
 IN= 2--> OUT= 1 Routing time step (min)'= 15.00

----- DATA FOR SECTION (4281.0) ----->

Distance	Elevation	Manning	
0.00	246.54	0.0900	
8.80	246.29	0.0900	
30.78	246.03	0.0900	
41.78	243.97	0.0900	
63.77	237.16	0.0900	
85.76	232.53	0.0900	
96.75	230.16	0.0900	
109.94	228.40	0.0900	
112.14	228.28	0.0900 / 0.0450	Main Channel
112.84	228.00	0.0450	Main Channel
113.34	227.50	0.0450	Main Channel
114.34	227.50	0.0450	Main Channel
114.84	228.00	0.0450	Main Channel
116.54	228.29	0.0450 / 0.0900	Main Channel
118.74	228.40	0.0900	
120.94	228.52	0.0900	
149.52	229.51	0.0900	
169.31	230.69	0.0900	
186.00	234.27	0.0900	
217.69	243.56	0.0900	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.78	228.28	.205E+04	1.6	0.97	21.60
1.58	229.08	.202E+05	16.6	1.02	20.37
2.39	229.89	.665E+05	64.8	1.22	17.11
3.19	230.69	.133E+06	158.1	1.49	14.05
4.00	231.50	.213E+06	308.6	1.82	11.48
4.80	232.30	.299E+06	504.9	2.11	9.89
5.61	233.11	.394E+06	746.6	2.37	8.79
6.41	233.91	.496E+06	1034.5	2.61	7.99
7.21	234.71	.605E+06	1373.3	2.84	7.35
8.02	235.52	.722E+06	1763.3	3.06	6.82
8.82	236.32	.844E+06	2202.0	3.27	6.39
9.63	237.13	.974E+06	2690.5	3.46	6.03
10.43	237.93	.111E+07	3239.5	3.66	5.71
11.23	238.73	.125E+07	3839.0	3.84	5.43
12.04	239.54	.140E+07	4489.1	4.03	5.18
12.84	240.34	.155E+07	5190.5	4.20	4.97
13.65	241.15	.170E+07	5943.8	4.36	4.78
14.45	241.95	.187E+07	6749.8	4.53	4.61
15.26	242.76	.204E+07	7609.4	4.68	4.46

<---- hydrograph ----> <-pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8198)	296.74	3.66	6.75	23.52	0.89	0.97
OUTFLOW: ID= 1 (6428)	296.74	3.52	7.25	23.52	0.88	0.97

ADD HYD (8200)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0428):	50.53	0.707	6.50	17.45
+ ID2= 2 (6428):	296.74	3.517	7.25	23.52
ID = 3 (8200):	347.27	4.114	7.25	22.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8202)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0430):	111.64	1.726	6.50	19.71
+ ID2= 2 (8200):	347.27	4.114	7.25	22.63
ID = 3 (8202):	458.91	5.684	7.00	21.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6432)
 IN= 2--> OUT= 1 Routing time step (min)'= 15.00

----- DATA FOR SECTION (4321.0) ----->

Distance	Elevation	Manning	
0.00	221.00	0.0600	
15.29	219.85	0.0600	
45.86	219.86	0.0600	
76.44	219.96	0.0600	
110.84	220.23	0.0600	
112.00	220.15	0.0600	
114.66	220.09	0.0600	
118.48	219.73	0.0600	
120.80	219.70	0.0600 / 0.0350	Main Channel
121.30	219.20	0.0350	Main Channel
122.30	219.20	0.0350	Main Channel
122.80	219.70	0.0350 / 0.0600	Main Channel
129.95	220.11	0.0600	
175.81	220.24	0.0600	
214.03	220.55	0.0600	
252.25	220.61	0.0600	
290.47	220.70	0.0600	
328.69	221.04	0.0600	
347.80	221.08	0.0600	
378.37	221.04	0.0600	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.08	219.28	.185E+03	0.0	0.23	149.38
0.17	219.37	.399E+03	0.1	0.34	100.62
0.25	219.45	.641E+03	0.1	0.42	80.98
0.33	219.53	.911E+03	0.2	0.49	69.79
0.42	219.62	.121E+04	0.3	0.55	62.35
0.50	219.70	.154E+04	0.5	0.60	56.91
0.61	219.81	.267E+04	0.7	0.56	61.28
0.71	219.91	.901E+04	1.3	0.30	112.96
0.82	220.02	.249E+05	3.4	0.28	122.71
0.92	220.12	.452E+05	6.8	0.31	109.98
1.03	220.23	.747E+05	12.2	0.33	102.05
1.14	220.34	.112E+06	21.3	0.39	87.59
1.24	220.44	.153E+06	33.0	0.44	77.01
1.35	220.55	.196E+06	47.2	0.49	69.22
1.46	220.66	.248E+06	62.3	0.51	66.48
1.56	220.76	.310E+06	83.4	0.55	62.06
1.67	220.87	.376E+06	108.8	0.59	57.56
1.77	220.97	.444E+06	137.7	0.64	53.77
1.88	221.08	.517E+06	163.6	0.65	52.70

		<---- hydrograph ---->			<-pi pe / channel -->		
		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW :	ID= 2 (8202)	458.91	5.68	7.00	21.92	0.89	0.30
OUTFLOW:	ID= 1 (6432)	458.91	3.70	9.00	21.92	0.83	0.28

ADD HYD (8204)		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3					
ID1=	1 (0432):	114.21	0.865	8.00	18.94
+ ID2=	2 (6432):	458.91	3.698	9.00	21.92
ID = 3 (8204):		573.12	4.494	8.75	21.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8212)		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3					
ID1=	1 (8204):	573.12	4.494	8.75	21.33
+ ID2=	2 (8210):	601.20	6.108	7.50	20.19
ID = 3 (8212):		1174.32	10.140	8.00	20.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6442)
IN= 2--> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (4421.0) ----->		
Distance	Elevation	Manning
0.00	221.00	0.0350
26.73	220.80	0.0350
53.47	220.60	0.0350
66.83	220.40	0.0350
80.20	220.20	0.0350
347.52	220.00	0.0350
354.21	219.59	0.0350
360.89	219.50	0.0350
367.57	219.08	0.0350
368.76	219.00	0.0350 / 0.0300
369.26	218.50	0.0300
374.26	218.50	0.0300
374.76	219.00	0.0300 / 0.0350
380.94	220.33	0.0350
387.62	219.62	0.0350
394.31	219.23	0.0350
454.45	219.07	0.0350
514.60	219.09	0.0350
660.00	219.19	0.0350
661.63	221.00	0.0350

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.12	218.62	.100E+04	0.2	0.28	92.89
0.25	218.75	.205E+04	0.6	0.43	60.09
0.37	218.87	.315E+04	1.1	0.55	46.99
0.50	219.00	.430E+04	1.8	0.66	39.67
0.63	219.13	.141E+05	3.4	0.37	69.99
0.77	219.27	.652E+05	13.7	0.33	79.14
0.90	219.40	.124E+06	35.5	0.45	58.02
1.03	219.53	.183E+06	65.7	0.56	46.45
1.17	219.67	.245E+06	103.8	0.66	39.30
1.30	219.80	.307E+06	149.3	0.76	34.30
1.43	219.93	.371E+06	201.6	0.85	30.65
1.57	220.07	.440E+06	256.5	0.91	28.56
1.70	220.20	.541E+06	325.7	0.94	27.70
1.83	220.33	.663E+06	415.2	0.98	26.62
1.97	220.47	.787E+06	520.3	1.03	25.21
2.10	220.60	.913E+06	638.5	1.09	23.82
2.23	220.73	.104E+07	767.2	1.15	22.62
2.37	220.87	.117E+07	908.0	1.21	21.54
2.50	221.00	.131E+07	1060.7	1.27	20.57

		<---- hydrograph ---->			<-pi pe / channel -->		
		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW :	ID= 2 (8212)	1174.32	10.14	8.00	20.75	0.72	0.34
OUTFLOW:	ID= 1 (6442)	1174.32	8.76	9.50	20.75	0.70	0.35

ADD HYD (8214)		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3					
ID1=	1 (0442):	117.26	0.655	7.75	13.82
+ ID2=	2 (6442):	1174.32	8.763	9.50	20.75
ID = 3 (8214):		1291.58	9.326	9.50	20.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8196)		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3					
ID1=	1 (8194):	44084.05	265.826	14.00	32.16
+ ID2=	2 (8214):	1291.58	9.326	9.50	20.12
ID = 3 (8196):		45375.62	270.883	14.00	31.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0444)
ID= 1 DT=15.0 min Area (ha)= 221.65 Curve Number (CN)= 56.0
U. H. Tp(hrs)= 1.03 # of Li near Res. (N)= 1.50

Unit Hyd Opeak (cms)= 3.656
PEAK FLOW (cms)= 2.061 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 20.663
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.256

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8216)		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3					
ID1=	1 (0444):	221.65	2.061	7.50	20.66
+ ID2=	2 (8196):	45375.62	270.883	14.00	31.82
ID = 3 (8216):		45597.28	271.380	14.00	31.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH
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V V I SSSS U U A L
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V V I SS U U A A A A L
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***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\Visual Otthymo 2.4\VO2\vo1.nat
 Output filename: C:\Users\amanluc\AppData\Local\Temp\9d9ff6f-b151-4d0f-9c8a-18195be1719c\Scenario.out
 Summary filename: C:\Users\amanluc\AppData\Local\Temp\9d9ff6f-b151-4d0f-9c8a-18195be1719c\Scenario.sum

DATE: 08/29/2012 TIME: 01:59:04

USER:

COMMENTS: _____

 ** SIMULATION NUMBER: 1 **

READ STORM
 Ptotal = 88.50 mm
 Filename: C:\Users\amanluc\AppData\Local\Temp\9d9ff6f-b151-4d0f-9c8a-18195be1719c\79ab8986
 Comments: 100-Year 12-Hour SCS II Design Storm

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	2.21	3.25	3.54	6.25	15.93	9.25	3.10
0.50	2.21	3.50	3.54	6.50	15.93	9.50	3.10
0.75	2.21	3.75	3.54	6.75	7.08	9.75	3.10
1.00	2.21	4.00	3.54	7.00	7.08	10.00	3.10
1.25	2.21	4.25	5.31	7.25	5.31	10.25	1.77
1.50	2.21	4.50	5.31	7.50	5.31	10.50	1.77
1.75	2.21	4.75	7.08	7.75	5.31	10.75	1.77
2.00	2.21	5.00	7.08	8.00	5.31	11.00	1.77
2.25	2.65	5.25	10.62	8.25	3.10	11.25	1.77
2.50	2.65	5.50	10.62	8.50	3.10	11.50	1.77
2.75	2.65	5.75	42.48	8.75	3.10	11.75	1.77
3.00	2.65	6.00	116.82	9.00	3.10	12.00	1.77

CALIB
 STANDHYD (1662)
 ID= 1 DT=15.0 min
 Area (ha)= 141.06
 Total Imp(%)= 70.00 Di r. Conn.(%)= 38.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 98.74 42.32
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 969.74 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.82 178.42
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 11.55 (ii) 20.04 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.05

TOTALS
 PEAK FLOW (cms)= 14.02 10.78 21.296 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00

RUNOFF VOLUME (mm)= 86.50 56.23 67.73
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.64 0.77

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (1661)
 ID= 1 DT=15.0 min
 Area (ha)= 293.64
 Total Imp(%)= 42.00 Di r. Conn.(%)= 23.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 123.33 170.31
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1399.14 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.82 94.48
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 14.39 (ii) 25.34 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.07 0.04

TOTALS
 PEAK FLOW (cms)= 16.24 20.42 29.480 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 86.50 46.07 55.37
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.52 0.63

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5166)
 IN= 2--> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	7.2300	6.6504
	0.4310	3.5677	9.1580	7.5825
	1.6680	3.8178	10.9790	8.5125
	3.5970	4.8509	11.3790	8.5225
	5.3070	5.7381	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1661)	293.640	29.480	6.00	55.37
OUTFLOW: ID= 1 (5166)	293.640	10.286	7.00	55.36

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 34.89
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 8.2153

ADD HYD (8134)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1662):	141.06	21.296	6.00	67.73
+ ID2= 2 (5166):	293.64	10.286	7.00	55.36
-----	-----	-----	-----	-----
ID = 3 (8134):	434.70	22.861	6.00	59.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1642)
 ID= 1 DT=15.0 min
 Area (ha)= 120.35
 Total Imp(%)= 22.00 Di r. Conn.(%)= 17.00

 IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 26.48 93.87
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 895.73 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.82 65.80
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 11.01 (ii) 23.66 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

TOTALS
 PEAK FLOW (cms)= 5.44 8.05 10.580 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 86.50 40.07 47.96
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.45 0.54

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.3 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB STANDHYD (1641)
 ID= 1 DT=15.0 min
 Area (ha)= 113.89
 Total Imp(%)= 71.00 Dir. Conn.(%)= 53.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 80.86 33.03
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 871.36 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.82 125.34
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 10.83 (ii) 20.61 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.05

TOTALS
 PEAK FLOW (cms)= 16.13 5.81 19.974 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 86.50 50.16 69.42
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.57 0.78

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.3 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR (5164)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	8.4260	4.5513
0.3860	1.9228	9.2330	4.9251
3.3700	2.2339	9.9990	5.2982
4.5710	3.3613	10.3990	5.3082
5.7170	4.0568	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1641)	113.890	19.974	6.00	69.42
OUTFLOW: ID= 1 (5164)	113.890	5.584	6.75	69.41

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 27.96
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 4.0335

 ADD HYD (8132)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1642):	120.35	10.580	6.00	47.96
+ ID2= 2 (5164):	113.89	5.584	6.75	69.41
=====				
ID = 3 (8132):	234.24	15.032	6.25	58.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB NASHYD (0152)
 ID= 1 DT=15.0 min
 Area (ha)= 124.37
 Ia (mm)= 5.00 Curve Number (CN)= 59.0
 U. H. Tp(hrs)= 0.89 # of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 2.372

PEAK FLOW (cms)= 1.677 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 26.493
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.299

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB STANDHYD (1481)
 ID= 1 DT=15.0 min
 Area (ha)= 61.75
 Total Imp(%)= 47.00 Dir. Conn.(%)= 17.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 29.02 32.73
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 641.61 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.82 16.38
 over (min)= 15.00 45.00
 Storage Coeff. (min)= 9.02 (ii) 31.08 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.09 0.03

TOTALS
 PEAK FLOW (cms)= 2.96 0.91 3.335 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 86.50 11.87 24.55
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.13 0.28

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 23.5 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR (5148)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.1310	1.6343
0.2090	0.7802	5.1870	1.8598
1.1960	0.9062	6.0380	2.0805
2.2280	1.1562	6.4380	2.0905
3.0910	1.3741	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1481)	61.750	3.335	6.00	24.55
OUTFLOW: ID= 1 (5148)	61.750	0.781	7.00	24.54

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 23.41
 TIME SHIFT OF PEAK FLOW (min) = 60.00

MAXIMUM STORAGE USED (ha. m.) = 0.8534

CALIB NASHYD (1482)
ID= 1 DT=15.0 min
Area (ha) = 356.14
Ia (mm) = 5.00
U.H. Tp(hrs) = 1.12
Curve Number (CN) = 31.3
of Linear Res. (N) = 1.50

Unit Hyd Opeak (cms) = 5.438
PEAK FLOW (cms) = 1.601 (i)
TIME TO PEAK (hrs) = 7.750
RUNOFF VOLUME (mm) = 10.783
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.122

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9148)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (1482):	356.14	1.601	7.75	10.78
+ ID2= 2 (5148):	61.75	0.781	7.00	24.54
ID = 3 (9148):	417.89	2.321	7.00	12.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1501)
ID= 1 DT=15.0 min
Area Total Imp(%) = 45.00
Dir. Conn. (%) = 15.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	0.07	0.09
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	32.66	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (min)	116.82 / 15.00	10.60 / 30.00
Storage Coeff. (min)	1.51 (ii)	27.76 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.11	0.04

TOTALS
0.009 (iii)

PEAK FLOW (cms)	0.01	0.00
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	86.50	7.88
TOTAL RAINFALL (mm)	88.50	88.50
RUNOFF COEFFICIENT	0.98	0.09

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 16.4 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5150)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.0110	0.0042
0.0010	0.0020	0.0140	0.0049
0.0030	0.0023	0.0160	0.0054
0.0060	0.0030	0.0160	0.0154
0.0080	0.0036	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (1501)	0.160	0.009	6.00	19.54
OUTFLOW: ID= 1 (5150)	0.160	0.001	7.25	16.45

PEAK FLOW REDUCTION [Qout/Qin] (%) = 10.36
TIME SHIFT OF PEAK FLOW (min) = 75.00
MAXIMUM STORAGE USED (ha. m.) = 0.0018

CALIB NASHYD (1502)
ID= 1 DT=15.0 min
Area (ha) = 105.72
Ia (mm) = 5.00
U.H. Tp(hrs) = 0.58
Curve Number (CN) = 17.2
of Linear Res. (N) = 1.50

Unit Hyd Opeak (cms) = 3.134
PEAK FLOW (cms) = 0.379 (i)
TIME TO PEAK (hrs) = 6.750
RUNOFF VOLUME (mm) = 5.215
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.059

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9150)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (1502):	105.72	0.379	6.75	5.21
+ ID2= 2 (5150):	0.16	0.001	7.25	16.45
ID = 3 (9150):	105.88	0.380	6.75	5.23

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8116)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (9148):	417.89	2.321	7.00	12.82
+ ID2= 2 (9150):	105.88	0.380	6.75	5.23
ID = 3 (8116):	523.77	2.691	7.00	11.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6152)
IN= 2---> OUT= 1
Routing time step (min) = 15.00

Distance	Elevation	Manning	
0.00	290.00	0.0500	
15.34	286.87	0.0500	
46.03	284.84	0.0500	
88.22	283.07	0.0500	
126.57	282.11	0.0500	
153.42	280.34	0.0500	
157.26	280.07	0.0500	
161.09	279.72	0.0500	
167.76	278.94	0.0500 / 0.0300	Main Channel
168.01	278.60	0.0300	Main Channel
168.76	278.60	0.0300	Main Channel
169.51	278.60	0.0300	Main Channel
169.76	278.90	0.0300 / 0.0500	Main Channel
173.76	279.40	0.0500	
180.27	279.69	0.0500	
184.11	279.93	0.0500	
218.62	281.86	0.0500	
260.82	283.20	0.0500	
360.54	289.25	0.0500	
379.72	289.80	0.0500	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.30	278.90	.114E+04	0.4	0.80	45.70
0.87	279.47	.932E+04	4.2	0.98	37.22
1.45	280.05	.353E+05	17.5	1.09	33.68
2.02	280.62	.831E+05	47.8	1.26	29.01
2.59	281.19	.155E+06	102.9	1.46	25.06
3.17	281.77	.250E+06	189.3	1.66	22.03

3.74	282.34	.375E+06	298.5	1.75	20.91
4.32	282.92	.547E+06	456.5	1.83	19.98
4.89	283.49	.767E+06	705.7	2.02	18.12
5.46	284.06	.102E+07	1038.0	2.24	16.34
6.04	284.64	.130E+07	1444.1	2.45	14.98
6.61	285.21	.161E+07	1940.1	2.66	13.79
7.18	285.78	.194E+07	2522.6	2.86	12.80
7.76	286.36	.229E+07	3187.4	3.06	11.98
8.33	286.93	.267E+07	3942.3	3.25	11.28
8.91	287.51	.306E+07	4824.9	3.46	10.58
9.48	288.08	.347E+07	5795.7	3.67	9.99
10.05	288.65	.390E+07	6855.4	3.86	9.48
10.63	289.23	.434E+07	8005.0	4.05	9.04

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8116)	523.77	2.69	7.00	11.28	0.65	0.90
OUTFLOW: ID= 1 (6152)	523.77	2.34	8.00	11.28	0.59	0.88

ADD HYD (8122)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0152):	124.37	1.677	7.00	26.49
+ ID2= 2 (6152):	523.77	2.342	8.00	11.28
ID = 3 (8122):	648.14	3.933	7.75	14.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0158)	Area (ha)	Ia (mm)	U. H. Tp (hrs)	Curve Number (CN)	# of Linear Res. (N)
ID= 1 DT=15.0 min	178.59	5.00	0.94	61.0	1.50

Uni t Hyd Qpeak (cms)	= 3.230
PEAK FLOW (cms)	= 2.450 (i)
TIME TO PEAK (hrs)	= 7.250
RUNOFF VOLUME (mm)	= 28.040
TOTAL RAINFALL (mm)	= 88.500
RUNOFF COEFFICIENT	= 0.317

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0156)	Area (ha)	Ia (mm)	U. H. Tp (hrs)	Curve Number (CN)	# of Linear Res. (N)
ID= 1 DT=15.0 min	83.49	5.00	0.62	45.0	1.50

Uni t Hyd Qpeak (cms)	= 2.306
PEAK FLOW (cms)	= 0.970 (i)
TIME TO PEAK (hrs)	= 6.730
RUNOFF VOLUME (mm)	= 17.331
TOTAL RAINFALL (mm)	= 88.500
RUNOFF COEFFICIENT	= 0.196

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1541)	Area Total	(ha)	Imp (%)	Di r. Conn. (%)
ID= 1 DT=15.0 min	11.15	75.00		70.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	8.36	2.79
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	272.64	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	116.82	13.74
over (min)	15.00	30.00
Storage Coeff. (min)	5.40 (ii)	29.06 (ii)

Uni t Hyd. Tpeak (min)	= 15.00	30.00
Uni t Hyd. peak (cms)	= 0.11	0.04

TOTALS			
PEAK FLOW (cms)	= 2.43	0.07	2.474 (iii)
TIME TO PEAK (hrs)	= 6.00	6.25	6.00
RUNOFF VOLUME (mm)	= 86.50	12.96	64.44
TOTAL RAINFALL (mm)	= 88.50	88.50	88.50
RUNOFF COEFFICIENT	= 0.98	0.15	0.73

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 31.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5154)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2--> OUT= 1				
DT= 15.0 min				
	0.0000	0.0000	0.7460	0.3544
	0.0380	0.1987	0.9370	0.3839
	0.2160	0.2386	1.0900	0.4129
	0.4020	0.2749	1.4900	0.4229
	0.5580	0.3133	0.0000	0.0000
INFLOW : ID= 2 (1541)	11.150	2.474	6.00	64.44
OUTFLOW: ID= 1 (5154)	11.150	0.897	6.25	64.32

PEAK FLOW REDUCTION [Qout/Qi n] (%)	= 36.24
TIME SHIFT OF PEAK FLOW (min)	= 15.00
MAXIMUM STORAGE USED (ha. m.)	= 0.3934

CALIB NASHYD (1542)	Area (ha)	Ia (mm)	U. H. Tp (hrs)	Curve Number (CN)	# of Linear Res. (N)
ID= 1 DT=15.0 min	189.17	5.00	0.95	39.1	1.50

Uni t Hyd Qpeak (cms)	= 3.396
PEAK FLOW (cms)	= 1.292 (i)
TIME TO PEAK (hrs)	= 7.250
RUNOFF VOLUME (mm)	= 14.392
TOTAL RAINFALL (mm)	= 88.500
RUNOFF COEFFICIENT	= 0.163

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9154)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (1542):	189.17	1.292	7.25	14.39
+ ID2= 2 (5154):	11.15	0.897	6.25	64.32
ID = 3 (9154):	200.32	1.882	6.25	17.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8118)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0156):	83.49	0.970	6.75	17.33
+ ID2= 2 (9154):	200.32	1.882	6.25	17.17
ID = 3 (8118):	283.81	2.834	6.50	17.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ROUTE CHN (6158) |

| IN= 2---> OUT= 1 | Routing time step (min)'= 15.00

----->>> DATA FOR SECTION (1581.0) <<<----->

Distance	Elevation	Manning	
0.00	290.00	0.0400	
19.20	288.88	0.0400	
48.00	288.26	0.0400	
119.99	283.68	0.0400	
167.99	282.07	0.0400	
196.79	281.32	0.0400	
210.79	280.90	0.0400	
219.79	280.38	0.0400 / 0.0300	Main Channel
220.04	279.65	0.0300	Main Channel
220.79	279.65	0.0300	Main Channel
221.54	279.65	0.0300	Main Channel
221.79	280.39	0.0300 / 0.0400	Main Channel
225.79	280.59	0.0400	
235.19	280.88	0.0400	
254.39	281.50	0.0400	
273.58	282.56	0.0400	
302.38	283.42	0.0400	
359.98	284.03	0.0400	
436.77	289.37	0.0400	
475.17	290.37	0.0400	

----->>> TRAVEL TIME TABLE <<<----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.37	280.01	.973E+03	0.5	0.86	31.78
0.73	280.38	.210E+04	1.5	1.18	23.16
1.30	280.95	.149E+05	8.6	0.95	28.91
1.86	281.51	.579E+05	40.0	1.13	24.12
2.43	282.08	.133E+06	118.5	1.46	18.75
2.99	282.64	.236E+06	254.6	1.77	15.46
3.56	283.21	.369E+06	450.3	2.00	13.67
4.13	283.78	.542E+06	702.7	2.13	12.86
4.69	284.34	.767E+06	1125.2	2.41	11.36
5.26	284.91	.101E+07	1697.1	2.75	9.93
5.82	285.47	.127E+07	2379.1	3.07	8.91
6.39	286.04	.155E+07	3171.4	3.36	8.14
6.95	286.60	.184E+07	4075.5	3.64	7.53
7.52	287.17	.215E+07	5093.1	3.89	7.03
8.09	287.74	.247E+07	6226.5	4.13	6.62
8.65	288.30	.281E+07	7461.2	4.36	6.28
9.22	288.87	.318E+07	8635.1	4.46	6.13
9.78	289.43	.357E+07	10143.6	4.67	5.86
10.35	290.00	.398E+07	11608.8	4.79	5.71

----->>> hydrograph <<<----->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8118)	283.81	2.83	6.50	17.22	0.84	1.13
OUTFLOW: ID= 1 (6158)	283.81	2.60	7.25	17.22	0.82	1.14

----->>> ADD HYD (8120) <<<----->

1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0158):	178.59	2.450	7.25	28.04
+ ID2= 2 (6158):	283.81	2.604	7.25	17.22
----->>>				
ID = 3 (8120):	462.40	5.055	7.25	21.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----->>> ADD HYD (8124) <<<----->

1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8120):	462.40	5.055	7.25	21.40
+ ID2= 2 (8122):	648.14	3.933	7.75	14.20
----->>>				
ID = 3 (8124):	1110.54	8.870	7.50	17.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ROUTE CHN (6160) |

| IN= 2---> OUT= 1 | Routing time step (min)'= 15.00

----->>> DATA FOR SECTION (1601.0) <<<----->

Distance	Elevation	Manning	
0.00	261.02	0.0400	
29.89	254.30	0.0400	
40.75	252.58	0.0400	
62.49	251.30	0.0400	
114.11	250.94	0.0400	
130.41	248.58	0.0400	
141.28	247.50	0.0400	
154.86	246.87	0.0400	
155.20	246.70	0.0400	
160.20	246.30	0.0400 / 0.0300	Main Channel
160.30	245.30	0.0300	Main Channel
165.20	245.30	0.0300	Main Channel
165.30	246.30	0.0300 / 0.0400	Main Channel
168.45	246.96	0.0400	
195.62	249.20	0.0400	
203.77	250.82	0.0400	
225.50	256.78	0.0400	
244.52	261.14	0.0400	
255.39	261.95	0.0400	
268.97	261.92	0.0400	

----->>> TRAVEL TIME TABLE <<<----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.50	245.80	.120E+05	2.9	1.18	68.31
1.00	246.30	.242E+05	8.5	1.70	47.47
1.92	247.22	.825E+05	31.7	1.86	43.34
2.84	248.14	.248E+06	102.5	2.00	40.39
3.76	249.06	.506E+06	247.7	2.37	34.03
4.68	249.98	.832E+06	486.8	2.83	28.49
5.60	250.90	.121E+07	814.9	3.27	24.71
6.52	251.82	.180E+07	1157.1	3.11	25.97
7.44	252.74	.253E+07	1840.2	3.52	22.92
8.36	253.66	.331E+07	2727.1	3.98	20.25
9.29	254.59	.414E+07	3782.6	4.43	18.22
10.21	255.51	.499E+07	5010.7	4.86	16.61
11.13	256.43	.589E+07	6397.4	5.26	15.33
12.05	257.35	.681E+07	7932.8	5.64	14.31
12.97	258.27	.777E+07	9621.2	5.99	13.46
13.89	259.19	.877E+07	11469.4	6.33	12.74
14.81	260.11	.980E+07	13478.4	6.66	12.12
15.73	261.03	.109E+08	15651.7	6.97	11.58
16.65	261.95	.120E+08	17492.5	7.07	11.42

----->>> hydrograph <<<----->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8124)	1110.54	8.87	7.50	17.20	1.01	1.70
OUTFLOW: ID= 1 (6160)	1110.54	8.22	8.25	17.20	0.97	1.66

----->>> CALIB STANDHYD (1601) <<<----->

ID= 1 DT=15.0 min	Area Total (ha)	Imp(%)= 19.03	Di r. Conn.(%)= 69.00
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----->>> IMPERVIOUS PERVIOUS (i) <<<----->

Surface Area (ha)=	14.27	4.76
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	356.18	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	116.82	53.85
over (min)=	15.00	30.00
Storage Coeff. (min)=	6.33 (ii)	20.04 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.10	0.05

----->>> *TOTALS* <<<----->

PEAK FLOW (cms)=	4.00	0.36	4.223 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	86.50	28.76	68.60
TOTAL RAI NFALL (mm)=	88.50	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.33	0.78

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 55.6 Ia = Dep. Storage (Above)

- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5160)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	1.2730	0.6034
0.0640	0.3377	1.5980	0.6540
0.3690	0.4055	1.8600	0.7037
0.6870	0.4676	2.2600	0.7137
0.9520	0.5332	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1601)	19.030	4.223	6.00	68.60
OUTFLOW: ID= 1 (5160)	19.030	1.601	6.25	68.53

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 37.90
TIME SHIFT OF PEAK FLOW (mi n) = 15.00
MAXIMUM STORAGE USED (ha. m.) = 0.6698

CALIB NASHYD (1602)
ID= 1 DT=15.0 mi n

Area (ha) = 299.79
U.H. Tp (hrs) = 1.42
Curve Number (CN) = 60.3
of Linear Res. (N) = 1.50

Unit Hyd Opeak (cms) = 3.592

PEAK FLOW (cms) = 2.951 (i)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 27.640
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.312

- (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9160)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1602):	299.79	2.951	8.00	27.64
+ ID2= 2 (5160):	19.03	1.601	6.25	68.53
ID = 3 (9160):	318.82	3.641	6.50	30.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8126)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6160):	1110.54	8.216	8.25	17.20
+ ID2= 2 (9160):	318.82	3.641	6.50	30.08
ID = 3 (8126):	1429.36	11.465	8.25	20.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1621)
ID= 1 DT=15.0 mi n

Area Total (ha) = 46.07
Imp(%) = 45.00
Di r. Conn. (%) = 20.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	20.73	25.34
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	554.20	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (mi n)	116.82	62.85
Storage Coeff. (mi n)	8.26 (ii)	21.14 (ii)

Unit Hyd. Tpeak (mi n) = 15.00
Unit Hyd. peak (cms) = 0.09

30.00
0.05

TOTALS

PEAK FLOW (cms)	2.66	2.18	4.032 (iii)
TIME TO PEAK (hrs)	6.00	6.25	6.00
RUNOFF VOLUME (mm)	86.50	28.69	40.25
TOTAL RAINFALL (mm)	88.50	88.50	88.50
RUNOFF COEFFICIENT	0.98	0.32	0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 51.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5162)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.0820	1.1997
0.1560	0.5629	3.8700	1.3716
0.8930	0.6512	4.5050	1.5398
1.6620	0.8407	4.9050	1.5498
2.3060	1.0035	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1621)	46.070	4.032	6.00	40.25
OUTFLOW: ID= 1 (5162)	46.070	1.609	6.50	40.23

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 39.91
TIME SHIFT OF PEAK FLOW (mi n) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 0.8420

CALIB STANDHYD (1622)
ID= 1 DT=15.0 mi n

Area Total (ha) = 144.07
Imp(%) = 26.00
Di r. Conn. (%) = 11.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	37.46	106.61
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	980.03	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) over (mi n)	116.82	44.78
Storage Coeff. (mi n)	11.62 (ii)	26.38 (ii)
Unit Hyd. Tpeak (mi n)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04

PEAK FLOW (cms)	4.13	5.84	7.750 (iii)
TIME TO PEAK (hrs)	6.00	6.25	6.00
RUNOFF VOLUME (mm)	86.50	24.83	31.62
TOTAL RAINFALL (mm)	88.50	88.50	88.50
RUNOFF COEFFICIENT	0.98	0.28	0.36

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 51.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9162)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1622):	144.07	7.750	6.00	31.62
+ ID2= 2 (5162):	46.07	1.609	6.50	40.23

ID = 3 (9162): 190.14 8.729 6.25 33.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8126):	1429.36	11.465	8.25	20.07
+ ID2= 2 (9162):	190.14	8.729	6.25	33.71
=====				
ID = 3 (8128):	1619.50	13.746	6.25	21.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8128):	1619.50	13.746	6.25	21.67
+ ID2= 2 (8132):	234.24	15.032	6.25	58.39
=====				
ID = 3 (8130):	1853.74	28.778	6.25	26.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6166)
IN= 2---> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1661.0) ----->

Distance	Elevation	Manning	
0.00	248.36	0.0500	
41.67	246.98	0.0500	
95.82	244.93	0.0500	
149.98	242.42	0.0500	
191.64	241.88	0.0500	
224.97	239.90	0.0500	
229.13	239.35	0.0500	
233.30	237.77	0.0500	
237.47	236.57	0.0500	
240.63	235.60	0.0500 / 0.0300	Main Channel
241.63	234.10	0.0300	Main Channel
247.13	234.10	0.0300	Main Channel
247.53	235.60	0.0300 / 0.0500	Main Channel
254.13	236.25	0.0500	
266.63	236.77	0.0500	
283.29	237.84	0.0500	
291.63	238.89	0.0500	
329.12	244.66	0.0500	
370.78	249.55	0.0500	
412.44	253.17	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.75	234.85	.130E+05	6.0	1.36	36.45
1.50	235.60	.276E+05	18.1	1.95	25.41
2.25	236.35	.544E+05	40.0	2.18	22.69
3.00	237.10	.117E+06	80.1	2.04	24.32
3.75	237.85	.213E+06	148.9	2.07	23.89
4.50	238.60	.334E+06	255.1	2.27	21.85
5.25	239.35	.473E+06	395.8	2.49	19.90
6.00	240.10	.631E+06	563.8	2.65	18.66
6.75	240.85	.824E+06	773.2	2.79	17.77
7.50	241.60	.106E+07	1037.9	2.92	16.97
8.26	242.36	.135E+07	1326.7	2.92	16.94
9.01	243.11	.173E+07	1758.5	3.01	16.44
9.76	243.86	.217E+07	2285.9	3.13	15.81
10.51	244.61	.265E+07	2909.5	3.26	15.17
11.26	245.36	.318E+07	3610.1	3.37	14.67
12.01	246.11	.377E+07	4417.4	3.48	14.21
12.76	246.86	.441E+07	5348.0	3.60	13.75
13.51	247.61	.512E+07	6394.0	3.71	13.34
14.26	248.36	.589E+07	7580.3	3.82	12.95

<----- hydrograph -----> <- pipe / channel ->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)

INFLOW : ID= 2 (8130) 1853.74 28.78 6.25 26.31 1.87 2.06
 OUTFLOW: ID= 1 (6166) 1853.74 23.56 6.75 26.31 1.69 2.00

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6166):	1853.74	23.555	6.75	26.31
+ ID2= 2 (8134):	434.70	22.861	6.00	59.38
=====				
ID = 3 (8136):	2288.44	42.207	6.50	32.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2021)
 ID= 1 DT=15.0 min Area Total (ha)= 107.32
 Imp(%)= 45.00 Dir. Conn.(%)= 27.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	48.29	59.03
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	845.85	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	116.82	96.04
over (min)	15.00	30.00
Storage Coeff. (min)=	10.64 (ii)	21.51 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.05
PEAK FLOW (cms)=	7.79	7.78
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	86.50	46.81
TOTAL RAINFALL (mm)=	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.53
TOTALS		
TIME TO PEAK (hrs)=	6.00	6.00 (iii)
RUNOFF VOLUME (mm)=	86.50	57.53
TOTAL RAINFALL (mm)=	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.65

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 73.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5202)
 IN= 2---> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000		5.1480	2.8069
0.3630	1.2717		6.4200	3.2412
1.5670	1.4796		7.4640	3.6910
2.7880	1.9296		7.8640	3.7010
3.8580	2.3404		0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2021)	107.320	12.872	6.00	57.53
OUTFLOW: ID= 1 (5202)	107.320	5.070	6.75	57.52

PEAK FLOW REDUCTION [Out/Oi n] (%) = 39.39
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 2.8093

CALIB STANDHYD (2022)
 ID= 1 DT=15.0 min Area Total (ha)= 361.21
 Imp(%)= 60.00 Dir. Conn.(%)= 36.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	216.73	144.48
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1551.79	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 116.82 126.43
 over (min) = 15.00 30.00
 Storage Coeff. (min)= 15.32 (ii) 25.06 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.07 0.04

PEAK FLOW (cms)= 30.46 23.45
 TIME TO PEAK (hrs)= 6.00 6.25
 RUNOFF VOLUME (mm)= 86.50 51.22
 TOTAL RAINFALL (mm)= 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.58

TOTALS
 45.898 (iii)
 6.00
 63.92
 88.50
 0.72

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 73.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8110)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (2022):	361.21	45.898	6.00	63.92	
+ ID2= 2 (5202):	107.32	5.070	6.75	57.52	
ID = 3 (8110):	468.53	47.586	6.00	62.45	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (0102)			
ID= 1 DT=15.0 min			
Area Total	(ha)	Imp(%)	Dir. Conn. (%)
	466.00	23.00	9.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 107.18 358.82
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1762.57 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.82 24.85
 over (min) = 15.00 45.00
 Storage Coeff. (min)= 16.53 (ii) 35.20 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.07 0.03

TOTALS
 17.272 (iii)
 6.50
 28.77
 88.50
 0.33

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 49.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0100)			
ID= 1 DT=15.0 min			
Area	(ha)	Curve Number (CN)	# of Linear Res. (N)
	693.84	40.0	1.50
Ia	5.00		
U. H. Tp	(hrs)= 1.95		

Unit Hyd Opeak (cms)= 6.087

PEAK FLOW (cms)= 2.848 (i)
 TIME TO PEAK (hrs)= 9.000
 RUNOFF VOLUME (mm)= 14.953
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.169

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8000)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0100):	693.84	2.848	9.00	14.95
+ ID2= 2 (0102):	466.00	17.272	6.50	28.77
ID = 3 (8000):	1159.84	19.127	6.50	20.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6104)		Routing time step (min)'= 15.00
IN= 2--> OUT= 1		

<----- DATA FOR SECTION (1041.0) ----->			
Distance	Elevation	Manning	
0.00	330.16	0.0450	
50.03	328.36	0.0450	
89.34	326.33	0.0450	
117.94	324.30	0.0450	
160.82	322.75	0.0450	
178.34	319.96	0.0450	
183.34	319.81	0.0450	
184.34	319.38	0.0450 / 0.0300	Main Channel
185.09	318.78	0.0300	Main Channel
185.84	318.72	0.0300	Main Channel
186.84	319.32	0.0300 / 0.0450	Main Channel
193.84	319.70	0.0450	
198.84	320.38	0.0450	
200.13	320.81	0.0450	
218.00	322.49	0.0450	
239.44	323.49	0.0450	
250.17	323.96	0.0450	
275.18	325.20	0.0450	
310.92	326.47	0.0450	
353.81	330.00	0.0450	

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.30	319.02	.624E+03	0.1	0.45	73.00
0.60	319.32	.181E+04	0.6	0.67	49.46
1.23	319.95	.128E+05	4.9	0.77	43.38
1.86	320.58	.396E+05	19.1	0.96	34.50
2.48	321.20	.757E+05	43.5	1.15	28.98
3.11	321.83	.125E+06	81.2	1.30	25.58
3.74	322.46	.187E+06	135.9	1.45	22.92
4.37	323.09	.269E+06	195.6	1.45	22.90
5.00	323.72	.387E+06	287.8	1.48	22.44
5.63	324.35	.545E+06	428.4	1.56	21.22
6.25	324.97	.735E+06	630.5	1.71	19.42
6.88	325.60	.952E+06	876.1	1.83	18.12
7.51	326.23	.120E+07	1177.0	1.95	17.03
8.14	326.86	.149E+07	1554.4	2.08	15.93
8.77	327.49	.179E+07	2009.7	2.23	14.88
9.40	328.12	.213E+07	2533.9	2.37	13.99
10.02	328.74	.249E+07	3112.6	2.49	13.31
10.65	329.37	.288E+07	3760.9	2.60	12.75
11.28	330.00	.330E+07	4494.8	2.71	12.23

<---- hydrograph ---->					<- pi pe / channel ->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8000)	1159.84	19.13	6.50	20.50	1.86	0.96
OUTFLOW: ID= 1 (6104)	1159.84	14.10	6.75	20.50	1.63	0.88

CALIB STANDHYD (1041)			
ID= 1 DT=15.0 min			
Area Total	(ha)	Imp(%)	Dir. Conn. (%)
	0.67	75.00	70.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 0.50 0.17
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 66.83 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.82 16.81

over (min) 15.00 30.00
 Storage Coeff. (min) = 2.32 (ii) 24.15 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.11 0.04

TOTALS
 PEAK FLOW (cms) = 0.15 0.01 0.156 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 86.50 15.71 65.24
 TOTAL RAINFALL (mm) = 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.18 0.74

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 36.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5104)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0450	0.0213
0.0020	0.0120	0.0560	0.0231
0.0130	0.0144	0.0660	0.0248
0.0240	0.0165	0.4660	0.0348
0.0340	0.0189	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (1041)	0.670	0.156	6.00	65.24
OUTFLOW: ID= 1 (5104)	0.670	0.056	6.25	62.98

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 36.18
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0245

CALIB NASHYD (1042)
 ID= 1 DT=15.0 min

Area (ha) = 527.14 Curve Number (CN) = 42.6
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.34

Unit Hyd Opeak (cms) = 6.731
 PEAK FLOW (cms) = 3.163 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 16.268
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.184

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9104)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1042):	527.14	3.163	8.00	16.27
+ ID2= 2 (5104):	0.67	0.056	6.25	62.98
ID = 3 (9104):	527.81	3.174	8.00	16.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8002)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6104):	1159.84	14.095	6.75	20.50
+ ID2= 2 (9104):	527.81	3.174	8.00	16.33
ID = 3 (8002):	1687.65	16.849	6.75	19.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANHYD (1061)
 ID= 1 DT=15.0 min

Area (ha) = 32.26
 Total Imp(%) = 67.00
 Dir. Conn. (%) = 51.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 21.61 10.65
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 463.75 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 116.82 75.10
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 7.42 (ii) 19.42 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.10 0.05

TOTALS
 PEAK FLOW (cms) = 4.86 1.14 5.590 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 86.50 33.34 60.45
 TOTAL RAINFALL (mm) = 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.38 0.68

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 56.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5106)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.1580	0.9744
0.1090	0.5252	2.7090	1.0694
0.6250	0.6260	3.1540	1.1625
1.1640	0.7386	3.5540	1.1725
1.6140	0.8505	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (1061)	32.260	5.590	6.00	60.45
OUTFLOW: ID= 1 (5106)	32.260	1.940	6.50	60.42

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 34.70
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 0.9432

CALIB NASHYD (1062)
 ID= 1 DT=15.0 min

Area (ha) = 227.27 Curve Number (CN) = 57.5
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.09

Unit Hyd Opeak (cms) = 3.556
 PEAK FLOW (cms) = 2.516 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 25.475
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.288

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9106)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1062):	227.27	2.516	7.50	25.47
+ ID2= 2 (5106):	32.26	1.940	6.50	60.42
ID = 3 (9106):	259.53	4.116	6.50	29.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8002):	1687.65	16.849	6.75	19.20
+ ID2= 2 (9106):	259.53	4.116	6.50	29.82
=====				
ID = 3 (8004):	1947.18	20.897	6.75	20.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6108)
IN= 2--> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1081.0) ----->

Distance	Elevation	Manning	
0.00	326.06	0.0400	
12.06	325.74	0.0400	
30.15	324.28	0.0400	
59.27	320.35	0.0400	
72.37	317.60	0.0400	
87.45	314.52	0.0400	
93.48	313.91	0.0400	
105.54	313.55	0.0400	
106.17	313.40	0.0400 / 0.0300	Main Channel
110.62	313.04	0.0300	Main Channel
110.82	312.97	0.0300	Main Channel
111.57	313.08	0.0300	Main Channel
111.67	313.48	0.0300 / 0.0400	Main Channel
129.66	316.62	0.0400	
150.77	318.95	0.0400	
180.92	319.61	0.0400	
205.04	321.23	0.0400	
232.18	322.09	0.0400	
268.37	322.31	0.0400	
298.52	326.00	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.43	313.40	.244E+04	0.7	0.59	58.83
1.09	314.06	.214E+05	10.4	1.01	34.24
1.76	314.73	.600E+05	40.2	1.39	24.89
2.42	315.39	.109E+06	92.1	1.75	19.80
3.08	316.05	.169E+06	166.5	2.05	16.88
3.75	316.72	.237E+06	264.4	2.31	14.97
4.41	317.38	.318E+06	386.3	2.52	13.72
5.07	318.04	.411E+06	539.3	2.72	12.71
5.74	318.71	.517E+06	725.5	2.91	11.88
6.40	319.37	.642E+06	909.2	2.94	11.77
7.06	320.03	.806E+06	1179.3	3.04	11.38
7.72	320.69	.989E+06	1515.9	3.18	10.87
8.39	321.36	.119E+07	1895.5	3.30	10.49
9.05	322.02	.143E+07	2310.2	3.36	10.30
9.71	322.68	.172E+07	2763.2	3.33	10.39
10.38	323.35	.205E+07	3477.6	3.53	9.81
11.04	324.01	.238E+07	4284.3	3.73	9.27
11.70	324.67	.274E+07	5154.0	3.91	8.85
12.37	325.34	.311E+07	6104.0	4.08	8.48

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8004)	1947.18	20.90	6.75	20.61	1.33	1.12
OUTFLOW: ID= 1 (6108)	1947.18	18.33	7.25	20.61	1.27	1.09

CALIB STANDHYD (1081)
ID= 1 DT=15.0 min

Area (ha)= 40.81
Total Imp(%)= 75.00
Dir. Conn.(%)= 70.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	30.61	10.20
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	521.60	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)=	116.82	13.12

over (min)	15.00	45.00
Storage Coeff. (min)=	7.96 (ii)	32.06 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.10	0.03

TOTALS

PEAK FLOW (cms)=	8.31	0.22	8.406 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	86.50	12.40	64.27
TOTAL RAINFALL (mm)=	88.50	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.14	0.73

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 30.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5108)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.7500	1.2970
0.1380	0.7271	3.4270	1.4050
0.7910	0.8733	3.9900	1.5110
1.4720	1.0061	4.3900	1.5210
2.0420	1.1467	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1081)	40.810	8.406	6.00	64.27
OUTFLOW: ID= 1 (5108)	40.810	3.043	6.25	64.24

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 36.20
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha. m.) = 1.3699

CALIB NASHYD (1082)
ID= 1 DT=15.0 min

Area (ha)= 166.24
Ia (mm)= 5.00
U. H. Tp(hrs)= 0.89
Curve Number (CN)= 32.2
of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 3.171

PEAK FLOW (cms)=	0.915 (i)
TIME TO PEAK (hrs)=	7.250
RUNOFF VOLUME (mm)=	11.140
TOTAL RAINFALL (mm)=	88.500
RUNOFF COEFFICIENT =	0.126

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9108)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1082):	166.24	0.915	11.14
+ ID2= 2 (5108):	40.81	3.043	64.24
=====			
ID = 3 (9108):	207.05	3.755	6.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8006)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6108):	1947.18	18.326	7.25
+ ID2= 2 (9108):	207.05	3.755	6.25
=====			
ID = 3 (8006):	2154.23	20.620	7.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (1101)
ID= 1 DT=15.0 min

Area (ha)= 60.97
Total Imp(%)= 69.00 Dir. Conn.(%)= 55.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	42.07	18.90
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	637.55	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	116.82	65.45
over (min)	15.00	30.00
Storage Coeff. (min)	8.98 (ii)	21.66 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.05

TOTALS
PEAK FLOW (cms)= 9.47 1.68 10.530 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 86.50 29.88 61.02
TOTAL RAINFALL (mm)= 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.34 0.69

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 52.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5110)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.0780	1.8615
0.2060	1.0120	5.1210	2.0372
1.1810	1.2083	5.9610	2.2096
2.2000	1.4181	6.3610	2.2196
3.0510	1.6293	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1101)	60.970	10.530	6.00	61.02
OUTFLOW: ID= 1 (5110)	60.970	3.703	6.50	61.00

PEAK FLOW REDUCTION [Oout/Oin](%)= 35.17
TIME SHIFT OF PEAK FLOW (min)= 30.00
MAXIMUM STORAGE USED (ha.m.)= 1.8128

CALIB
NASHYD (1102)
ID= 1 DT=15.0 min

Area (ha)= 262.95 Curve Number (CN)= 55.4
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 1.29

Unit Hyd Opeak (cms)	3.483
PEAK FLOW (cms)	2.420 (i)
TIME TO PEAK (hrs)	8.000
RUNOFF VOLUME (mm)	24.041
TOTAL RAINFALL (mm)	88.500
RUNOFF COEFFICIENT	0.272

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9110)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1102):	262.95	2.420	8.00	24.04
+ ID2= 2 (5110):	60.97	3.703	6.50	61.00
ID = 3 (9110):	323.92	5.649	6.50	31.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8008)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8006):	2154.23	20.620	7.25	20.71
+ ID2= 2 (9110):	323.92	5.649	6.50	31.00
ID = 3 (8008):	2478.15	25.143	7.25	22.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6112)
IN= 2---> OUT= 1

Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1121.0) ----->

Distance	Elevation	Manning
0.00	320.59	0.0400
13.48	317.93	0.0400
40.45	311.52	0.0400
64.04	307.68	0.0400
74.15	307.01	0.0400
91.00	307.22	0.0400
107.86	305.21	0.0400
111.23	304.86	0.0400
113.32	304.60	0.0400 / 0.0300
117.77	304.24	0.0300
117.97	304.17	0.0300
118.72	304.28	0.0300
118.82	304.68	0.0300 / 0.0400
124.71	304.76	0.0400
171.90	306.30	0.0400
205.60	307.24	0.0400
239.31	308.24	0.0400
262.90	310.64	0.0400
289.86	314.34	0.0400
333.68	317.66	0.0400

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.43	304.60	.372E+04	0.6	0.54	97.65
1.12	305.29	.463E+05	12.5	0.85	61.89
1.80	305.97	.149E+06	53.0	1.13	46.85
2.49	306.66	.311E+06	134.2	1.37	38.62
3.18	307.35	.554E+06	261.6	1.50	35.27
3.87	308.04	.894E+06	490.7	1.74	30.37
4.55	308.72	.129E+07	833.0	2.05	25.75
5.24	309.41	.171E+07	1269.8	2.36	22.39
5.93	310.10	.215E+07	1789.0	2.64	20.01
6.62	310.79	.261E+07	2392.7	2.90	18.21
7.30	311.47	.310E+07	3087.8	3.16	16.74
7.99	312.16	.361E+07	3873.6	3.40	15.52
8.68	312.85	.413E+07	4741.0	3.64	14.52
9.37	313.54	.467E+07	5689.3	3.86	13.69
10.05	314.22	.523E+07	6718.4	4.07	12.97
10.74	314.91	.581E+07	7770.7	4.24	12.46
11.43	315.60	.641E+07	8897.5	4.40	12.01
12.12	316.29	.704E+07	10113.2	4.55	11.61
12.80	316.97	.770E+07	11420.1	4.70	11.24

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8008)	2478.15	25.14	7.25	22.05	1.33	0.92
OUTFLOW: ID= 1 (6112)	2478.15	20.65	8.25	22.05	1.25	0.90

CALIB
STANDHYD (1121)
ID= 1 DT=15.0 min

Area (ha)= 9.96
Total Imp(%)= 55.00 Dir. Conn.(%)= 25.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	5.48	4.48
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	257.68	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	116.82	21.58
--------------------------	--------	-------

over (min) 15.00 30.00
 Storage Coeff. (min) = 5.22 (ii) 24.97 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.11 0.04

TOTALS
 PEAK FLOW (cms) = 0.78 0.20 0.901 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 86.50 14.57 32.55
 TOTAL RAINFALL (mm) = 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.16 0.37

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 26.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5112)
 IN= 2----> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.6660	0.2787
0.0340	0.1406	0.8360	0.3122
0.1930	0.1653	0.9740	0.3450
0.3590	0.2034	1.3740	0.3550
0.4980	0.2383	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1121)	9.960	0.901	6.00	32.55
OUTFLOW: ID= 1 (5112)	9.960	0.217	6.50	32.45

PEAK FLOW REDUCTION [Qout/Oin] (%) = 24.14
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 0.1728

CALIB NASHBY (1122)
 ID= 1 DT=15.0 min

Area (ha) = 404.93 Curve Number (CN) = 31.1
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp (hrs) = 1.90

Unit Hyd Opeak (cms) = 3.635

PEAK FLOW (cms) = 1.211 (i)
 TIME TO PEAK (hrs) = 9.000
 RUNOFF VOLUME (mm) = 10.747
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.121

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9112)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1122):	404.93	1.211	9.00	10.75
+ ID2= 2 (5112):	9.96	0.217	6.50	32.45
ID = 3 (9112):	414.89	1.268	8.25	11.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8010)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6112):	2478.15	20.646	8.25	22.05
+ ID2= 2 (9112):	414.89	1.268	8.25	11.27
ID = 3 (8010):	2893.04	21.914	8.25	20.51

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6114)
 IN= 2----> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (1141.0) ----->

Distance	Elevation	Manning
0.00	312.27	0.0500
14.07	309.70	0.0500
28.14	302.51	0.0500
35.17	305.81	0.0500
42.20	304.95	0.0500
52.75	304.24	0.0500
77.37	303.61	0.0500
98.47	301.33	0.0500
131.76	300.40	0.0500 / 0.0300
136.21	300.04	0.0300
136.41	299.97	0.0300
137.16	300.08	0.0300
137.26	300.48	0.0300 / 0.0500
140.68	300.53	0.0500
179.36	302.20	0.0500
214.53	303.82	0.0500
242.66	305.27	0.0500
288.38	307.66	0.0500
316.52	311.76	0.0500
348.17	313.48	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.43	300.40	.366E+04	0.7	0.59	88.31
1.05	301.02	.505E+05	12.1	0.75	69.49
1.68	301.65	.165E+06	52.7	0.99	52.28
2.30	302.27	.324E+06	130.3	1.26	41.40
2.93	302.90	.520E+06	247.1	1.48	35.08
3.55	303.52	.754E+06	406.8	1.68	30.91
4.18	304.15	.104E+07	586.1	1.76	29.55
4.80	304.77	.138E+07	856.0	1.93	26.96
5.43	305.40	.177E+07	1201.1	2.12	24.56
6.05	306.02	.219E+07	1612.2	2.30	22.63
6.68	306.65	.264E+07	2090.6	2.47	21.07
7.30	307.27	.313E+07	2653.0	2.64	19.65
7.93	307.90	.364E+07	3303.9	2.83	18.36
8.55	308.52	.417E+07	4053.9	3.03	17.14
9.18	309.15	.471E+07	4871.9	3.22	16.12
9.80	309.77	.527E+07	5756.2	3.41	15.25
10.43	310.40	.584E+07	6699.8	3.58	14.52
11.05	311.02	.642E+07	7710.5	3.74	13.89
11.68	311.65	.702E+07	8788.5	3.90	13.32

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8010)	2893.04	21.91	8.25	20.51	1.21	0.80
OUTFLOW: ID= 1 (6114)	2893.04	19.18	9.50	20.51	1.16	0.78

CALIB STANDHYD (1141)
 ID= 1 DT=15.0 min

Area Total (ha) = 19.45
 Imp (%) = 45.00
 Dir. Conn. (%) = 15.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 8.75 10.70
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 360.09 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 116.82 55.97
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 6.38 (ii) 19.87 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.10 0.05

PEAK FLOW (cms) = 0.89 0.84 *TOTALS*
 TIME TO PEAK (hrs) = 6.00 6.25 1.415 (iii)
 RUNOFF VOLUME (mm) = 86.50 24.27 33.60
 TOTAL RAINFALL (mm) = 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.27 0.38

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%

YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 43.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5114)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.3010	0.5065
0.0660	0.2376	1.6340	0.5791
0.3770	0.2749	1.9020	0.6501
0.7020	0.3549	2.3020	0.6601
0.9730	0.4237	0.0000	0.0000

INFLOW : ID= 2 (1141)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5114)	19.450	1.415	6.00	33.60
	19.450	0.520	6.75	33.55

PEAK FLOW REDUCTION [Qout/Qin] (%) = 36.79
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha.m.) = 0.3135

CALIB NASHYD (1142)
ID= 1 DT=15.0 min

Area (ha) = 268.19 Curve Number (CN) = 51.3
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 1.09

Unit Hyd Qpeak (cms) = 4.197

PEAK FLOW (cms) = 2.463 (i)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 21.285
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.241

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9114)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1142):	268.19	2.463	7.50	21.29
+ ID2= 2 (5114):	19.45	0.520	6.75	33.55
ID= 3 (9114):	287.64	2.877	7.00	22.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8012)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6114):	2893.04	19.182	9.50	20.51
+ ID2= 2 (9114):	287.64	2.877	7.00	22.11
ID= 3 (8012):	3180.68	21.284	9.50	20.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0116)
ID= 1 DT=15.0 min

Area (ha) = 206.43 Curve Number (CN) = 64.0
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 0.80

Unit Hyd Qpeak (cms) = 4.394

PEAK FLOW (cms) = 3.517 (i)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 30.364

TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.343

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8014)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0116):	206.43	3.517	7.00	30.36
+ ID2= 2 (8012):	3180.68	21.284	9.50	20.65
ID= 3 (8014):	3387.11	23.532	9.25	21.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6118)
IN= 2--> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (1181.0) ----->

Distance	Elevation	Manning
0.00	307.18	0.0450
24.89	299.75	0.0450
37.34	296.39	0.0450
71.57	292.75	0.0450
96.46	291.12	0.0450
108.90	289.43	0.0450
112.02	288.86	0.0450
115.13	288.46	0.0450
117.24	288.32	0.0450 / 0.0350
117.49	288.15	0.0350
118.24	288.00	0.0350
118.99	288.20	0.0350
119.24	288.36	0.0350 / 0.0450
121.35	288.40	0.0450
149.35	289.36	0.0450
171.14	290.47	0.0450
208.47	293.14	0.0450
236.48	295.17	0.0450
280.04	299.48	0.0450
308.04	307.00	0.0450

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.32	288.32	.116E+04	0.2	0.62	81.92
1.30	289.30	.631E+05	22.2	1.07	47.43
2.29	290.29	.218E+06	117.5	1.64	30.95
3.27	291.27	.448E+06	314.6	2.15	23.72
4.25	292.25	.756E+06	625.6	2.53	20.13
5.24	293.24	.115E+07	1093.2	2.91	17.50
6.22	294.22	.161E+07	1731.6	3.28	15.50
7.20	295.20	.214E+07	2547.4	3.63	14.01
8.19	296.19	.274E+07	3584.9	4.00	12.72
9.17	297.17	.338E+07	4859.1	4.39	11.60
10.15	298.15	.407E+07	6341.5	4.76	10.70
11.13	299.13	.480E+07	8024.6	5.11	9.97
12.12	300.12	.556E+07	10006.6	5.49	9.27
13.10	301.10	.635E+07	12244.6	5.89	8.64
14.08	302.08	.716E+07	14681.7	6.26	8.13
15.07	303.07	.799E+07	17316.5	6.62	7.69
16.05	304.05	.884E+07	20148.1	6.96	7.31
17.03	305.03	.971E+07	23176.2	7.29	6.98
18.02	306.02	.106E+08	26400.8	7.61	6.69

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8014)	3387.11	23.53	9.25	21.24	1.32	1.08
OUTFLOW: ID= 1 (6118)	3387.11	22.90	10.00	21.24	1.31	1.08

CALIB NASHYD (0118)
ID= 1 DT=15.0 min

Area (ha) = 340.53 Curve Number (CN) = 45.0
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 1.12

Unit Hyd Qpeak (cms) = 5.200

PEAK FLOW (cms) = 2.515 (i)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 17.546
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.198

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8016) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0118):	340.53	2.515	7.75	17.55
+ ID2= 2 (6118):	3387.11	22.904	10.00	21.24

ID = 3 (8016):	3727.64	24.896	9.75	20.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6122)
 IN= 2----> OUT= 1
 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1221.0) ----->

Distance	Elevation	Manning	
0.00	277.00	0.0500	
52.18	272.82	0.0500	
86.97	269.38	0.0500	
134.80	267.35	0.0500	
160.89	263.85	0.0500	
169.58	262.88	0.0500	
178.28	262.45	0.0500	
195.67	262.07	0.0500	
213.07	260.99	0.0500	
226.01	259.80	0.0500 / 0.0300	Main Channel
226.11	259.30	0.0300	Main Channel
230.11	259.30	0.0300	Main Channel
230.21	259.80	0.0300 / 0.0500	Main Channel
235.01	260.20	0.0500	
239.15	260.57	0.0500	
243.50	260.93	0.0500	
300.03	264.43	0.0500	
339.16	268.34	0.0500	
373.95	272.36	0.0500	
430.48	277.46	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.50	259.80	.626E+04	2.5	1.23	41.35
1.41	260.71	.462E+05	21.8	1.44	35.27
2.31	261.61	.149E+06	79.6	1.63	31.19
3.22	262.52	.340E+06	200.1	1.80	28.34
4.12	263.42	.636E+06	446.8	2.15	23.71
5.03	264.33	.995E+06	819.9	2.52	20.23
5.93	265.23	.141E+07	1337.0	2.90	17.56
6.84	266.14	.187E+07	1984.8	3.25	15.67
7.74	267.04	.237E+07	2766.0	3.57	14.26
8.65	267.95	.292E+07	3596.9	3.76	13.53
9.55	268.85	.355E+07	4575.4	3.93	12.95
10.46	269.76	.427E+07	5814.9	4.16	12.23
11.36	270.66	.503E+07	7319.0	4.44	11.46
12.27	271.57	.584E+07	9009.7	4.71	10.81
13.17	272.47	.670E+07	10886.3	4.96	10.26
14.08	273.38	.761E+07	12893.2	5.17	9.84
14.98	274.28	.858E+07	15098.1	5.37	9.48
15.89	275.19	.961E+07	17527.6	5.57	9.14
16.79	276.09	.107E+08	20190.3	5.76	8.83

<----- hydrograph -----> <--pi pe / channel -->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8016)	3727.64	24.90	9.75	20.91	1.45	1.45
OUTFLOW: ID= 1 (6122)	3727.64	24.30	10.50	20.91	1.44	1.45

CALIB STANDHYD (1221) Area (ha) = 117.91

|| ID= 1 DT=15.0 mi n | Total Imp(%)= 44.00 Di r. Conn.(%)= 16.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	51.88	66.03
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	886.60	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	116.82	25.71
over (min)	15.00	30.00
Storage Coeff. (min)	10.95 (ii)	29.36 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04
PEAK FLOW (cms)	5.02	3.22
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	86.50	19.04
TOTAL RAINFALL (mm)	88.50	88.50
RUNOFF COEFFICIENT	0.98	0.22

TOTALS
 6.995 (iii)
 6.00
 29.83
 88.50
 0.34

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 36.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5122)
 IN= 2----> OUT= 1
 DT= 15.0 mi n

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	7.8880	3.0390
	0.3990	1.4100	9.9040	3.4850
	2.2850	1.6269	11.5290	3.9214
	4.2550	2.1164	11.9290	3.9314
	5.9010	2.5338	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1221)	117.910	6.995	6.00	29.83
OUTFLOW: ID= 1 (5122)	117.910	2.589	6.75	29.82

PEAK FLOW REDUCTION [Out/Oi n] (%) = 37.02
 TIME SHIFT OF PEAK FLOW (mi n) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 1.7069

CALIB NASHYD (1222)
 ID= 1 DT=15.0 mi n

Area (ha) = 246.95 Curve Number (CN) = 43.8
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp (hrs) = 1.13

Unit Hyd Opeak (cms) = 3.728
 PEAK FLOW (cms) = 1.739 (i)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 16.885
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.191

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8018)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1222):	246.95	1.739	7.75	16.89
+ ID2= 2 (5122):	117.91	2.589	6.75	29.82

ID = 3 (8018):	364.86	4.182	6.75	21.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6122):	3727.64	24.300	10.50	20.91
+ ID2= 2 (8018):	364.86	4.182	6.75	21.07
=====				
ID = 3 (8020):	4092.50	25.994	10.25	20.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1261) ID= 1 DT=15.0 min	Area (ha)	Imp(%)	Di r. Conn. (%)
	50.19	36.00	26.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	18.07	32.12
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	578.45	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	116.82	23.19
over (min)	15.00	30.00
Storage Coeff. (min)	8.47 (ii)	27.67 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.04

TOTALS		
PEAK FLOW (cms)	3.74	1.46
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	86.50	22.06
TOTAL RAINFALL (mm)	88.50	88.50
RUNOFF COEFFICIENT	0.98	0.25

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 48.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5126) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	3.3570	1.2179
	0.1700	0.5264	4.2160	1.4221
	0.9720	0.5968	4.9080	1.6217
	1.8110	0.8165	5.3080	1.6317
	2.5120	0.9954	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1261)	50.190	4.636	6.00	38.81
OUTFLOW: ID= 1 (5126)	50.190	1.825	6.50	38.80

PEAK FLOW REDUCTION [Qout/Qin] (%) = 39.36
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 0.8225

CALIB NASHYD (1262) ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	# of Li near Res. (N)
	83.33	57.7	1.50

Unit Hyd Opeak (cms) = 2.509

PEAK FLOW (cms)	1.541 (i)
TIME TO PEAK (hrs)	6.500
RUNOFF VOLUME (mm)	25.241
TOTAL RAINFALL (mm)	88.500
RUNOFF COEFFICIENT	0.285

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1262):	83.33	1.541	6.50	25.24
+ ID2= 2 (5126):	50.19	1.825	6.50	38.80
=====				
ID = 3 (8024):	133.52	3.366	6.50	30.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0124) ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	# of Li near Res. (N)
	121.43	43.0	1.50

Unit Hyd Opeak (cms) = 4.049

PEAK FLOW (cms)	1.530 (i)
TIME TO PEAK (hrs)	6.500
RUNOFF VOLUME (mm)	16.138
TOTAL RAINFALL (mm)	88.500
RUNOFF COEFFICIENT	0.182

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0124):	121.43	1.530	6.50	16.14
+ ID2= 2 (8024):	133.52	3.366	6.50	30.34
=====				
ID = 3 (8026):	254.95	4.895	6.50	23.57

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6128) IN= 2--> OUT= 1	Routing time step (min)
	15.00

<----- DATA FOR SECTION (1281.0) ----->		
Distance	Elevation	Manning
0.00	270.58	0.0400
7.71	270.36	0.0400
15.43	269.48	0.0400
23.14	265.88	0.0400
42.43	261.83	0.0400
61.71	258.31	0.0400
75.21	256.62	0.0400
90.64	255.35	0.0400
92.49	255.00	0.0400 / 0.0300
93.49	254.00	0.0300
94.49	253.90	0.0300
94.99	253.80	0.0300
95.49	253.70	0.0300
95.99	253.80	0.0300
98.49	255.00	0.0300 / 0.0400
100.28	255.44	0.0400
134.99	257.45	0.0400
148.49	259.67	0.0400
167.77	263.42	0.0400
190.92	270.00	0.0400

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.65	254.35	.439E+04	2.0	1.24	36.10
1.30	255.00	.131E+05	9.4	1.92	23.39
2.18	255.88	.433E+05	37.0	2.30	19.54
3.06	256.76	.131E+06	113.3	2.34	19.20
3.95	257.65	.273E+06	273.8	2.70	16.64
4.83	258.53	.453E+06	538.5	3.21	14.01
5.71	259.41	.658E+06	902.9	3.70	12.14
6.59	260.29	.886E+06	1368.4	4.16	10.79
7.48	261.18	.114E+07	1936.3	4.59	9.79
8.36	262.06	.141E+07	2610.9	4.99	9.01
9.24	262.94	.171E+07	3398.7	5.37	8.36
10.12	263.82	.202E+07	4309.6	5.75	7.81

11.01	264.71	.235E+07	5347.5	6.13	7.33
11.89	265.59	.270E+07	6501.0	6.48	6.93
12.77	266.47	.307E+07	7808.6	6.86	6.55
13.65	267.35	.345E+07	9250.5	7.23	6.21
14.54	268.24	.384E+07	10808.3	7.59	5.92
15.42	269.12	.424E+07	12482.5	7.94	5.66
16.30	270.00	.466E+07	14127.4	8.18	5.49

<---- hydrograph ----> <- pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8026)	254.95	4.90	6.50	23.57	0.90	1.44
OUTFLOW: ID= 1 (6128)	254.95	4.09	7.25	23.57	0.83	1.38

CALIB STANDHYD (1281) ID= 1 DT=15.0 min	Area Total	(ha)= 55.87 Imp(%)= 50.00	Di r. Conn. (%)= 29.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	27.93	27.93
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	610.30	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (min)	116.82 / 15.00	65.27 / 30.00
Storage Coeff. (min)	8.75 (ii)	21.44 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.05
PEAK FLOW (cms)	4.61	2.48
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	86.50	30.42
TOTAL RAINFALL (mm)	88.50	88.50
RUNOFF COEFFICIENT	0.98	0.34

TOTALS
6.176 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 54.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5128)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.7380	1.5139
0.1890	0.7401	4.6930	1.7113
1.0830	0.8643	5.4630	1.9045
2.0160	1.0853	5.8630	1.9045
2.7960	1.2819	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1281)	55.870	6.176	6.00	46.69
OUTFLOW: ID= 1 (5128)	55.870	2.432	6.50	46.67

PEAK FLOW REDUCTION [Qout/Qin] (%) = 39.38
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 1.1938

CALIB NASHYD (1282) ID= 1 DT=15.0 min	Area Ia	(ha)= 184.36 (mm)= 5.00	Curve Number (CN)= 62.0 # of Linear Res. (N)= 1.50
	U. H. Tp	(hrs)= 0.62	

Unit Hyd. Qpeak (cms)	5.093
PEAK FLOW (cms)	3.615 (i)
TIME TO PEAK (hrs)	6.750
RUNOFF VOLUME (mm)	28.545
TOTAL RAINFALL (mm)	88.500
RUNOFF COEFFICIENT	0.323

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9128) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1282):	184.36	3.615	6.75	28.54
+ ID2= 2 (5128):	55.87	2.432	6.50	46.67
ID = 3 (9128):	240.23	6.033	6.50	32.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8028) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6128):	254.95	4.089	7.25	23.57
+ ID2= 2 (9128):	240.23	6.033	6.50	32.76
ID = 3 (8028):	495.18	9.649	6.75	28.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8022) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8020):	4092.50	25.994	10.25	20.92
+ ID2= 2 (8028):	495.18	9.649	6.75	28.03
ID = 3 (8022):	4587.68	29.109	10.00	21.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1201) ID= 1 DT=15.0 min	Area Total	(ha)= 176.35 Imp(%)= 45.00	Di r. Conn. (%)= 22.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	79.36	96.99
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1084.28	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (min)	116.82 / 15.00	53.89 / 30.00
Storage Coeff. (min)	12.35 (ii)	26.05 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04

TOTALS
9.91 6.45 13.912 (iii)
6.00 6.25 6.00
86.50 25.38 38.82
88.50 88.50 88.50
0.98 0.29 0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 47.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5120)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	11.7960	4.5929
0.5970	2.1554	14.8120	5.2509
3.4170	2.4937	17.2430	5.8948
6.3630	3.2187	17.6430	5.9048

8. 8260 3. 8422 | 0. 0000 0. 0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1201)	176. 350	13. 912	6. 00	38. 82
OUTFLOW: ID= 1 (5120)	176. 350	5. 685	6. 75	38. 82

PEAK FLOW REDUCTION [Qout/Qin] (%) = 40. 87
 TIME SHIFT OF PEAK FLOW (min) = 45. 00
 MAXIMUM STORAGE USED (ha. m.) = 3. 0912

CALIB STANDHYD (1202) ID= 1 DT=15. 0 min	Area (ha)	Total Imp(%)	Di r. Conn. (%)
	70. 67	43. 00	20. 00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	30. 39	40. 28
Dep. Storage (mm)	2. 00	5. 00
Average Slope (%)	0. 50	0. 50
Length (m)	686. 39	40. 00
Mannings n	0. 013	0. 250

Max. Eff. Inten. (mm/hr)	116. 82	52. 89
over (min)	15. 00	30. 00
Storage Coeff. (min)	9. 39 (ii)	23. 19 (ii)
Unit Hyd. Tpeak (min)	15. 00	30. 00
Unit Hyd. peak (cms)	0. 09	0. 04

TOTALS
 PEAK FLOW (cms) = 3. 94 2. 79 5. 682 (iii)
 TIME TO PEAK (hrs) = 6. 00 6. 25 6. 00
 RUNOFF VOLUME (mm) = 86. 50 25. 17 37. 44
 TOTAL RAINFALL (mm) = 88. 50 88. 50 88. 50
 RUNOFF COEFFICIENT = 0. 98 0. 28 0. 42

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 47. 3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8030) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1202):	70. 67	5. 682	6. 00	37. 44
+ ID2= 2 (5120):	176. 35	5. 685	6. 75	38. 82
ID = 3 (8030):	247. 02	8. 261	6. 50	38. 42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8032) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8022):	4587. 68	29. 109	10. 00	21. 69
+ ID2= 2 (8030):	247. 02	8. 261	6. 50	38. 42
ID = 3 (8032):	4834. 70	30. 404	10. 00	22. 54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6130) IN= 2--> OUT= 1	Routing time step (min)
	15. 00

<----- DATA FOR SECTION (1301.0) ----->

Distance	Elevation	Manning
0. 00	257. 18	0. 0350
62. 45	254. 70	0. 0350
109. 29	252. 74	0. 0350
150. 92	251. 55	0. 0350
192. 56	248. 96	0. 0350

239. 39	247. 16	0. 0350
291. 84	246. 00	0. 0350
296. 84	245. 00	0. 0350
298. 84	245. 00	0. 0350 / 0. 0300
299. 24	244. 60	0. 0300
301. 84	244. 60	0. 0300
303. 44	244. 60	0. 0300
303. 84	245. 00	0. 0300 / 0. 0350
306. 84	245. 00	0. 0350
309. 84	246. 00	0. 0350
317. 46	247. 22	0. 0350
348. 68	254. 83	0. 0350
359. 09	256. 70	0. 0350
452. 76	258. 23	0. 0350
515. 22	258. 33	0. 0350

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0. 40	245. 00	. 870E+04	1. 2	0. 64	124. 12
1. 04	245. 64	. 468E+05	9. 5	0. 96	81. 81
1. 68	246. 28	. 109E+06	25. 6	1. 11	70. 86
2. 32	246. 92	. 257E+06	63. 5	1. 17	67. 51
2. 96	247. 56	. 498E+06	142. 7	1. 36	58. 18
3. 61	248. 21	. 800E+06	266. 7	1. 58	50. 01
4. 25	248. 85	. 116E+07	437. 4	1. 78	44. 24
4. 89	249. 49	. 157E+07	669. 1	2. 01	39. 20
5. 53	250. 13	. 203E+07	954. 9	2. 23	35. 36
6. 17	250. 77	. 252E+07	1294. 4	2. 43	32. 42
6. 81	251. 41	. 305E+07	1689. 6	2. 62	30. 07
7. 45	252. 05	. 363E+07	2090. 7	2. 73	28. 93
8. 09	252. 69	. 429E+07	2553. 1	2. 82	27. 98
8. 73	253. 33	. 501E+07	3138. 4	2. 96	26. 60
9. 37	253. 97	. 579E+07	3803. 8	3. 11	25. 36
10. 02	254. 62	. 662E+07	4549. 3	3. 25	24. 25
10. 66	255. 26	. 751E+07	5364. 9	3. 38	23. 32
11. 30	255. 90	. 845E+07	6265. 7	3. 51	22. 49
11. 94	256. 54	. 946E+07	7260. 1	3. 63	21. 72

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8032)	4834. 70	30. 40	10. 00	22. 54	1. 76	1. 12
OUTFLOW: ID= 1 (6130)	4834. 70	28. 55	11. 00	22. 54	1. 73	1. 12

CALIB STANDHYD (1301) ID= 1 DT=15. 0 min	Area (ha)	Total Imp(%)	Di r. Conn. (%)
	320. 20	64. 00	47. 00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	204. 93	115. 27
Dep. Storage (mm)	2. 00	5. 00
Average Slope (%)	0. 50	0. 50
Length (m)	1461. 05	40. 00
Mannings n	0. 013	0. 250

Max. Eff. Inten. (mm/hr)	116. 82	116. 32
over (min)	15. 00	30. 00
Storage Coeff. (min)	14. 77 (ii)	24. 84 (ii)
Unit Hyd. Tpeak (min)	15. 00	30. 00
Unit Hyd. peak (cms)	0. 07	0. 04

TOTALS
 PEAK FLOW (cms) = 35. 80 17. 28 47. 168 (iii)
 TIME TO PEAK (hrs) = 6. 00 6. 25 6. 00
 RUNOFF VOLUME (mm) = 86. 50 51. 11 67. 75
 TOTAL RAINFALL (mm) = 88. 50 88. 50 88. 50
 RUNOFF COEFFICIENT = 0. 98 0. 58 0. 77

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75. 3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5130) IN= 2--> OUT= 1 DT= 15. 0 min	OUTFLOW	STORAGE	OUTFLOW	STORAGE

(cms)	(ha. m.)	(cms)	(ha. m.)
0.0000	0.0000	18.6040	9.7083
1.1320	5.3237	22.6740	10.8355
6.6360	6.1006	30.4650	12.2155
10.3780	7.2341	31.8650	12.2255
14.1960	8.4233	0.0000	0.0000

INFLOW : ID= 2 (1301) 320.200 47.168 67.75
 OUTFLOW: ID= 1 (5130) 320.200 19.101 6.50 67.74

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 40.50
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 10.0016

CALIB NASHYD (1302) ID= 1 DT=15.0 mi n
 Area (ha) = 65.86
 U. H. Tp (hrs) = 0.69
 Curve Number (CN) = 80.0
 # of Linear Res. (N) = 1.50

Unit Hyd Qpeak (cms) = 1.620
 PEAK FLOW (cms) = 2.002 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 46.599
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.527

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8036) 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 ID1= 1 (1302): 65.86 2.002 6.75 46.60
 + ID2= 2 (5130): 320.20 19.101 6.50 67.74
 ID = 3 (8036): 386.06 21.078 6.50 64.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8034) 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 ID1= 1 (6130): 4834.70 28.549 11.00 22.54
 + ID2= 2 (8036): 386.06 21.078 6.50 64.13
 ID = 3 (8034): 5220.76 32.952 7.00 25.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1861) ID= 1 DT=15.0 mi n
 Area (ha) = 6.85
 Total Imp (%) = 51.00 Di r. Conn. (%) = 27.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 3.49 3.36
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 213.70 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr) = 116.82 68.09
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 4.66 (ii) 17.14 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.11 0.05

TOTALS
 PEAK FLOW (cms) = 0.58 0.34 0.803 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 86.50 30.28 45.46
 TOTAL RAINFALL (mm) = 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.34 0.51

PEAK FLOW (cms) = 0.58 0.34 0.803 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 86.50 30.28 45.46
 TOTAL RAINFALL (mm) = 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.34 0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 52.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5186)
 IN= 2---> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.4580	0.1865
0.0230	0.0917	0.5750	0.2106
0.1330	0.1071	0.6700	0.2341
0.2470	0.1341	1.0700	0.2441
0.3430	0.1582	0.0000	0.0000

INFLOW : ID= 2 (1861) 6.850 0.803 6.00 45.46
 OUTFLOW: ID= 1 (5186) 6.850 0.293 6.50 45.31

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 36.50
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 0.1459

CALIB STANDHYD (1862) ID= 1 DT=15.0 mi n
 Area (ha) = 344.94
 Total Imp (%) = 43.00 Di r. Conn. (%) = 23.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 148.32 196.62
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 1516.44 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr) = 116.82 57.40
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 15.11 (ii) 28.46 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.07 0.04

TOTALS
 PEAK FLOW (cms) = 18.70 13.30 26.971 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 86.50 28.21 41.62
 TOTAL RAINFALL (mm) = 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.32 0.47

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 52.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9186) 1 + 2 = 3

AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 ID1= 1 (1862): 344.94 26.971 6.00 41.62
 + ID2= 2 (5186): 6.85 0.293 6.50 45.31
 ID = 3 (9186): 351.79 27.021 6.00 41.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1881) ID= 1 DT=15.0 mi n
 Area (ha) = 1.12
 Total Imp (%) = 75.00 Di r. Conn. (%) = 46.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 0.84 0.28
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50

Length (m) = 86.41 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr) = 116.82 147.26
 over (min) = 15.00 15.00
 Storage Coeff. (min) = 2.71 (ii) 11.87 (ii)
 Unit Hyd. Tpeak (min) = 15.00 15.00
 Unit Hyd. peak (cms) = 0.11 0.08

TOTALS
 PEAK FLOW (cms) = 0.17 0.09 0.255 (iii)
 TIME TO PEAK (hrs) = 6.00 6.00 6.00
 RUNOFF VOLUME (mm) = 86.50 44.60 63.87
 TOTAL RAINFALL (mm) = 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.50 0.72

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5188)
 IN= 2---> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0750	0.0357
0.0040	0.0200	0.0940	0.0386
0.0220	0.0240	0.1100	0.0416
0.0410	0.0277	0.5100	0.0516
0.0560	0.0315	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (1881)	1.120	0.255	6.00	63.87
OUTFLOW: ID= 1 (5188)	1.120	0.082	6.25	62.75

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 32.14
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0377

CALIB STANDHYD (1882)
 ID= 1 DT=15.0 mi n

Area (ha) = 307.41
 Total Imp(%) = 41.00 Di r. Conn. (%) = 25.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	126.04	181.37
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1431.57	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 116.82 62.43
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 14.59 (ii) 27.51 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.07 0.04

PEAK FLOW (cms) = 18.38 13.63 26.918 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 86.50 32.34 45.88
 TOTAL RAINFALL (mm) = 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.37 0.52

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 59.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9188)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1882):	307.41	26.918	6.00	45.88

+ ID2= 2 (5188):	1.12	0.082	6.25	62.75
ID = 3 (9188):	308.53	26.961	6.00	45.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8068)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (9186):	351.79	27.021	6.00	41.69
+ ID2= 2 (9188):	308.53	26.961	6.00	45.94
ID = 3 (8068):	660.32	53.982	6.00	43.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1821)
 ID= 1 DT=15.0 mi n

Area (ha) = 111.80
 Total Imp(%) = 40.00 Di r. Conn. (%) = 17.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	44.72	67.08
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	863.33	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 116.82 59.01
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 10.77 (ii) 23.98 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.09 0.04

PEAK FLOW (cms) = 5.09 5.10 8.281 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 86.50 28.33 38.22
 TOTAL RAINFALL (mm) = 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.32 0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 52.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5182)
 IN= 2---> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	5.7800	2.5656
0.3790	1.0207	7.3720	3.0829
1.6240	1.1939	8.7710	3.5956
3.0930	1.6631	9.1710	3.6056
4.3530	2.1032	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (1821)	111.800	8.281	6.00	38.22
OUTFLOW: ID= 1 (5182)	111.800	3.544	6.75	38.21

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 42.80
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 1.8363

CALIB NASHYD (1822)
 ID= 1 DT=15.0 mi n

Area (ha) = 44.40 Curve Number (CN) = 57.0
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 0.79

Unit Hyd Qpeak (cms) = 0.966

PEAK FLOW (cms)= 0.626 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 24.973
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.282

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8062)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (1822):	44.40	0.626	7.00	24.97
+ ID2= 2 (5182):	111.80	3.544	6.75	38.21
ID = 3 (8062):	156.20	4.164	6.75	34.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1841)	Area Total (ha)	Imp(%)=	Dir. Conn.(%)=
ID= 1 DT=15.0 min	145.07	48.00	19.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 69.63 75.44
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 983.43 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.82 80.37
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 11.65 (ii) 23.32 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

PEAK FLOW (cms)= 7.19 7.95 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.25 12.231 (iii)
 RUNOFF VOLUME (mm)= 86.50 34.00 6.00
 TOTAL RAINFALL (mm)= 88.50 88.50 43.98
 RUNOFF COEFFICIENT = 0.98 0.38 88.50
 0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 55.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5184)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1 DT= 15.0 min				
	0.0000	0.0000	5.9270	2.3472
	0.2980	1.0088	7.4630	2.7585
	1.6660	1.1775	8.7490	3.1642
	3.1100	1.4809	9.1490	3.1742
	4.4100	1.8897	0.0000	0.0000

AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 INFLOW : ID= 2 (1841) 145.070 12.231 6.00 43.98
 OUTFLOW: ID= 1 (5184) 145.070 6.068 6.50 43.97

PEAK FLOW REDUCTION [Qout/Qin] (%)= 49.61
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 2.4359

CALIB STANDHYD (1842)	Area Total (ha)	Imp(%)=	Dir. Conn.(%)=
ID= 1 DT=15.0 min	117.45	42.00	17.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 49.33 68.12
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 884.87 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.82 69.61
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 10.93 (ii) 23.30 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

PEAK FLOW (cms)= 5.32 6.21 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.25 9.241 (iii)
 RUNOFF VOLUME (mm)= 86.50 32.11 6.00
 TOTAL RAINFALL (mm)= 88.50 88.50 41.36
 RUNOFF COEFFICIENT = 0.98 0.36 88.50
 0.47

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 55.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8060)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (1842):	117.45	9.241	6.00	41.36
+ ID2= 2 (5184):	145.07	6.068	6.50	43.97
ID = 3 (8060):	262.52	13.238	6.25	42.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8064)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (8060):	262.52	13.238	6.25	42.80
+ ID2= 2 (8062):	156.20	4.164	6.75	34.45
ID = 3 (8064):	418.72	16.419	6.25	39.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1781)	Area Total (ha)	Imp(%)=	Dir. Conn.(%)=
ID= 1 DT=15.0 min	55.50	39.00	13.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 21.65 33.86
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 608.28 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.82 108.88
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 8.73 (ii) 19.07 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.05

PEAK FLOW (cms)= 2.05 5.34 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.25 5.967 (iii)
 RUNOFF VOLUME (mm)= 86.50 49.39 6.25
 TOTAL RAINFALL (mm)= 88.50 88.50 54.21
 RUNOFF COEFFICIENT = 0.98 0.56 88.50
 0.61

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

- CN* = 74.4 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5178)				
IN= 2----> OUT= 1				
DT= 15.0 mi n				
OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)	
0.0000	0.0000	6.1870	0.7248	
0.1880	0.3394	9.3670	0.8313	
1.7260	0.4003	10.7440	0.9285	
3.2000	0.4933	11.1440	0.9385	
4.6710	0.6039	0.0000	0.0000	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
INFLOW : ID= 2 (1781)	55.500	5.967	6.25	54.21
OUTFLOW: ID= 1 (5178)	55.500	5.092	6.25	54.20
PEAK FLOW REDUCTION [Qout/Qi n] (%) = 85.35				
TIME SHIFT OF PEAK FLOW (mi n) = 0.00				
MAXIMUM STORAGE USED (ha. m.) = 0.6769				

CALIB NASHYD (1782)				
ID= 1 DT=15.0 mi n				
Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res. (N)	
219.91	5.00	77.6	1.50	
U. H. Tp(hrs)= 0.76				

Unit Hyd Qpeak (cms) = 4.921

PEAK FLOW (cms) = 5.770 (i)
TIME TO PEAK (hrs) = 6.750
RUNOFF VOLUME (mm) = 43.783
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.495

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8046)				
1 + 2 = 3				
ID	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1782):	219.91	5.770	6.75	43.78
+ ID2= 2 (5178):	55.50	5.092	6.25	54.20
=====				
ID = 3 (8046):	275.41	10.512	6.50	45.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0180)				
ID= 1 DT=15.0 mi n				
Area (ha)	Ia (mm)	Curve Number (CN)	# of Linear Res. (N)	
226.65	5.00	65.0	1.50	
U. H. Tp(hrs)= 1.17				

Unit Hyd Qpeak (cms) = 3.299

PEAK FLOW (cms) = 2.950 (i)
TIME TO PEAK (hrs) = 7.750
RUNOFF VOLUME (mm) = 31.398
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.355

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8050)				
1 + 2 = 3				
ID	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0180):	226.65	2.950	7.75	31.40
+ ID2= 2 (8046):	275.41	10.512	6.50	45.88
=====				
ID = 3 (8050):	502.06	13.018	6.50	39.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1741)			
ID= 1 DT=15.0 mi n			
Area Total (ha)	Imp (%)	Dir. Conn. (%)	
56.30	35.00	18.00	

	IMPERVIOUS (ha)	PERVIOUS (i)	
Surface Area	19.70	36.60	
Dep. Storage	2.00	5.00	
Average Slope	0.50	0.50	
Length	612.64	40.00	
Mannings n	0.013	0.250	
Max. Eff. Inten. (mm/hr) over (mi n)	116.82	69.65	
Storage Coeff. (mi n)	15.00	30.00	
Unit Hyd. Tpeak (mi n)	8.77 (ii)	21.13 (ii)	
Unit Hyd. peak (cms)	15.00	30.00	
	0.09	0.05	
PEAK FLOW (cms)	2.88	3.50	*TOTALS* 5.113 (iii)
TIME TO PEAK (hrs)	6.00	6.25	6.00
RUNOFF VOLUME (mm)	86.50	36.13	45.20
TOTAL RAINFALL (mm)	88.50	88.50	88.50
RUNOFF COEFFICIENT	0.98	0.41	0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5174)				
IN= 2----> OUT= 1				
DT= 15.0 mi n				
OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)	
0.0000	0.0000	1.1370	1.3070	
0.0580	0.5427	1.4280	1.5354	
0.3290	0.6196	1.6620	1.7593	
0.6130	0.8627	2.0620	1.7693	
0.8510	1.0621	0.0000	0.0000	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
INFLOW : ID= 2 (1741)	56.300	5.113	6.00	45.20
OUTFLOW: ID= 1 (5174)	56.300	1.236	7.00	45.16
PEAK FLOW REDUCTION [Qout/Qi n] (%) = 24.16				
TIME SHIFT OF PEAK FLOW (mi n) = 60.00				
MAXIMUM STORAGE USED (ha. m.) = 1.3867				

Unit Hyd Qpeak (cms) = 2.055

PEAK FLOW (cms) = 1.947 (i)
TIME TO PEAK (hrs) = 6.750
RUNOFF VOLUME (mm) = 36.575
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.413

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8044)				
1 + 2 = 3				
ID	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1742):	82.80	1.947	6.75	36.57
+ ID2= 2 (5174):	56.30	1.236	7.00	45.16
=====				

ID = 3 (8044): 139.10 3.157 7.00 40.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	(0172)	Area (ha)= 202.23 Ia (mm)= 5.00 U. H. Tp(hrs)= 0.89	Curve Number (CN)= 78.0 # of Linear Res. (N)= 1.50
--------------------------------------	--------	---	---

Unit Hyd Opeak (cms)= 3.875

PEAK FLOW (cms)= 4.760 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 44.397
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.502

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	(0168)	Area (ha)= 247.13 Ia (mm)= 5.00 U. H. Tp(hrs)= 0.75	Curve Number (CN)= 52.0 # of Linear Res. (N)= 1.50
--------------------------------------	--------	---	---

Unit Hyd Opeak (cms)= 5.611

PEAK FLOW (cms)= 3.090 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 21.587
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.244

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	(0170)	Area (ha)= 166.91 Ia (mm)= 5.00 U. H. Tp(hrs)= 0.80	Curve Number (CN)= 65.0 # of Linear Res. (N)= 1.50
--------------------------------------	--------	---	---

Unit Hyd Opeak (cms)= 3.553

PEAK FLOW (cms)= 2.928 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 31.205
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.353

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	(8040)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0168):		247.13	3.090	7.00	21.59
+ ID2= 2 (0170):		166.91	2.928	7.00	31.21
=====					
ID = 3 (8040):		414.04	6.018	7.00	25.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	(8042)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0172):		202.23	4.760	7.00	44.40
+ ID2= 2 (8040):		414.04	6.018	7.00	25.46
=====					
ID = 3 (8042):		616.27	10.778	7.00	31.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| ROUTE CHN (6174) |

| IN= 2----> OUT= 1 | Routing time step (min)'= 15.00

<----- DATA FOR SECTION (1741.0) ----->

Distance	Elevation	Manning
0.00	296.00	0.0450
13.36	288.94	0.0450
26.71	288.64	0.0450
51.19	287.96	0.0450
73.45	287.32	0.0450
86.81	286.48	0.0450
102.39	285.08	0.0450
110.73	282.76	0.0450
115.73	281.06	0.0450 / 0.0350
115.74	280.35	0.0350
120.73	280.35	0.0350
120.74	281.10	0.0350 / 0.0450
125.74	282.64	0.0450
135.78	286.87	0.0450
149.13	289.58	0.0450
162.49	292.29	0.0450
175.84	293.95	0.0450
195.88	295.13	0.0450
213.68	296.02	0.0450
220.36	296.09	0.0450

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.71	281.06	.136E+05	6.0	1.69	37.89
1.50	281.85	.358E+05	22.5	2.42	26.43
2.28	282.63	.726E+05	53.8	2.85	22.47
3.07	283.42	.124E+06	104.1	3.24	19.79
3.86	284.21	.189E+06	176.7	3.60	17.81
4.64	284.99	.268E+06	274.9	3.94	16.26
5.43	285.78	.369E+06	390.8	4.07	15.73
6.21	286.56	.501E+06	551.6	4.23	15.15
7.00	287.35	.674E+06	750.5	4.28	14.97
7.79	288.14	.920E+06	993.3	4.15	15.44
8.57	288.92	.126E+07	1361.5	4.14	15.47
9.36	289.71	.167E+07	1986.0	4.56	14.04
10.15	290.50	.210E+07	2739.7	5.02	12.76
10.93	291.28	.254E+07	3615.0	5.47	11.70
11.72	292.07	.300E+07	4608.8	5.91	10.83
12.50	292.85	.347E+07	5693.3	6.30	10.16
13.29	293.64	.397E+07	6892.9	6.67	9.60
14.08	294.43	.450E+07	8150.4	6.97	9.19
14.86	295.21	.506E+07	9525.0	7.23	8.86

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8042)	616.27	10.78	7.00	31.68	0.94	1.85
OUTFLOW: ID= 1 (6174)	616.27	9.98	7.75	31.68	0.90	1.82

ADD HYD 1 + 2 = 3	(8048)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6174):		616.27	9.979	7.75	31.68
+ ID2= 2 (8044):		139.10	3.157	7.00	40.05
=====					
ID = 3 (8048):		755.37	12.829	7.50	33.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	(8052)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8048):		755.37	12.829	7.50	33.22
+ ID2= 2 (8050):		502.06	13.018	6.50	39.34
=====					
ID = 3 (8052):		1257.43	22.829	7.00	35.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8052):	1257.43	22.829	7.00	35.66
+ ID2= 2 (8064):	418.72	16.419	6.25	39.69
=====				
ID = 3 (8058):	1676.15	37.611	6.50	36.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1762) ID= 1 DT=15.0 min	Area (ha)	Imp(%)	Di r. Conn. (%)
	162.00	26.00	11.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	42.12	119.88
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1039.23	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) over (min)	116.82	56.13
Storage Coeff. (min)	15.00	30.00
Unit Hyd. Tpeak (min)	12.04 (ii)	25.52 (ii)
Unit Hyd. peak (cms)	15.00	30.00
	0.08	0.04

TOTALS			
PEAK FLOW (cms)	4.59	8.41	10.290 (iii)
TIME TO PEAK (hrs)	6.00	6.25	6.25
RUNOFF VOLUME (mm)	86.50	30.78	36.91
TOTAL RAINFALL (mm)	88.50	88.50	88.50
RUNOFF COEFFICIENT	0.98	0.35	0.42

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1761) ID= 1 DT=15.0 min	Area (ha)	Imp(%)	Di r. Conn. (%)
	48.80	50.00	21.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	24.40	24.40
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	570.38	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) over (min)	116.82	88.96
Storage Coeff. (min)	15.00	30.00
Unit Hyd. Tpeak (min)	8.40 (ii)	19.61 (ii)
Unit Hyd. peak (cms)	15.00	30.00
	0.09	0.05

TOTALS			
PEAK FLOW (cms)	2.94	3.08	4.929 (iii)
TIME TO PEAK (hrs)	6.00	6.25	6.00
RUNOFF VOLUME (mm)	86.50	36.96	47.36
TOTAL RAINFALL (mm)	88.50	88.50	88.50
RUNOFF COEFFICIENT	0.98	0.42	0.54

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 59.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5176) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)

0.0000	0.0000	3.2640	1.3175
0.1650	0.6418	4.0090	1.4908
0.9450	0.7489	4.7720	1.6604
1.7610	0.9426	5.1720	1.6704
2.4420	1.1144	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (1761)	48.800	4.929	6.00	47.36
OUTFLOW: ID= 1 (5176)	48.800	2.117	6.50	47.34

PEAK FLOW REDUCTION [Out/Oi n] (%) = 42.96
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 1.0495

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1762):	162.00	10.290	6.25	36.91
+ ID2= 2 (5176):	48.80	2.117	6.50	47.34
=====				
ID = 3 (8056):	210.80	11.917	6.25	39.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8056):	210.80	11.917	6.25	39.32
+ ID2= 2 (8058):	1676.15	37.611	6.50	36.67
=====				
ID = 3 (8066):	1886.95	47.195	6.25	36.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8066):	1886.95	47.195	6.25	36.97
+ ID2= 2 (8068):	660.32	53.982	6.00	43.67
=====				
ID = 3 (8070):	2547.27	92.364	6.25	38.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6190) IN= 2--> OUT= 1	Routing time step (min)' = 15.00		

<----- DATA FOR SECTION (1901.0) ----->			
Distance	Elevation	Manning	
0.00	261.34	0.0600	
19.76	261.64	0.0600	
59.28	261.29	0.0600	
79.04	261.09	0.0600	
103.74	255.99	0.0600	
153.13	253.42	0.0600	
192.65	252.78	0.0600	
212.29	249.80	0.0600	
217.29	249.50	0.0600 / 0.0350	Main Channel
217.30	249.05	0.0350	Main Channel
222.29	249.05	0.0350	Main Channel
222.30	249.50	0.0350 / 0.0600	Main Channel
227.30	249.80	0.0600	
232.17	252.31	0.0600	
276.63	253.53	0.0600	
326.03	256.97	0.0600	
355.67	257.30	0.0600	
400.12	260.08	0.0600	
469.28	261.25	0.0600	
489.04	262.53	0.0600	

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV. TIME

(m)	(m)	(cu. m.)	(cms)	(m/s)	(mi n)
0.45	249.50	.436E+04	1.0	0.43	75.97
1.07	250.12	.205E+05	5.4	0.51	62.96
1.70	250.75	.451E+05	14.8	0.64	50.78
2.32	251.37	.762E+05	29.2	0.74	43.46
2.94	251.99	.114E+06	49.1	0.84	38.61
3.57	252.62	.161E+06	70.2	0.85	38.18
4.19	253.24	.248E+06	93.4	0.73	44.29
4.81	253.86	.398E+06	154.9	0.76	42.79
5.44	254.49	.576E+06	246.1	0.83	38.99
6.06	255.11	.779E+06	362.7	0.90	35.79
6.68	255.73	.101E+07	506.5	0.98	33.15
7.30	256.35	.126E+07	687.6	1.06	30.52
7.93	256.98	.153E+07	902.8	1.15	28.18
8.55	257.60	.183E+07	1114.3	1.18	27.39
9.17	258.22	.216E+07	1407.8	1.26	25.55
9.80	258.85	.250E+07	1735.4	1.35	24.03
10.42	259.47	.286E+07	2097.5	1.42	22.73
11.04	260.09	.324E+07	2492.3	1.49	21.63
11.67	260.72	.364E+07	2837.8	1.51	21.39

	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8070)	2547.27	92.36	6.25	38.70	4.16	0.73
OUTFLOW: ID= 1 (6190)	2547.27	63.86	6.50	38.70	3.37	0.84

CALIB	Area	(ha)=	Di r. Conn. (%)=
STANDHYD (1901)	13.49		
ID= 1 DT=15.0 mi n	58.00		31.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	7.82	5.67
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	299.89	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	116.82	123.07
over (mi n)	15.00	30.00
Storage Coeff. (mi n)=	5.71 (ii)	15.56 (ii)
Uni t Hyd. Tpeak (mi n)=	15.00	30.00
Uni t Hyd. peak (cms)=	0.11	0.05

PEAK FLOW (cms)=	1.29	1.09	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	2.022 (iii)
RUNOFF VOLUME (mm)=	86.50	48.64	60.37
TOTAL RAINFALL (mm)=	88.50	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.55	0.68

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5190)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1	(cms)	(ha. m.)	(cms)	(ha. m.)
DT= 15.0 mi n	0.0000	0.0000	0.9030	0.3849
	0.0460	0.1975	1.1330	0.4289
	0.2610	0.2331	1.3190	0.4720
	0.4870	0.2836	1.7190	0.4820
	0.6750	0.3308	0.0000	0.0000

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (1901)	13.490	2.022	6.00	60.37
OUTFLOW: ID= 1 (5190)	13.490	0.839	6.50	60.29

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 41.48
 TIME SHI FT OF PEAK FLOW (mi n) = 30.00
 MAXI MUM STORAGE USED (ha. m.) = 0.3716

CALIB	Area	(ha)=	Di r. Conn. (%)=
STANDHYD (1902)	274.50		
ID= 1 DT=15.0 mi n	52.00		28.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	142.74	131.76
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1352.77	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	116.82	107.46
over (mi n)	15.00	30.00
Storage Coeff. (mi n)=	14.11 (ii)	24.50 (ii)
Uni t Hyd. Tpeak (mi n)=	15.00	30.00
Uni t Hyd. peak (cms)=	0.07	0.04

PEAK FLOW (cms)=	18.64	18.29	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	30.538 (iii)
RUNOFF VOLUME (mm)=	86.50	46.49	57.69
TOTAL RAINFALL (mm)=	88.50	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.53	0.65

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9190)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (1902):	274.50	30.538	6.00	57.69
+ ID2= 2 (5190):	13.49	0.839	6.50	60.29
ID = 3 (9190):	287.99	30.814	6.00	57.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8072)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6190):	2547.27	63.858	6.50	38.70
+ ID2= 2 (9190):	287.99	30.814	6.00	57.81
ID = 3 (8072):	2835.26	84.837	6.25	40.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6192)	Routing time step (mi n)=
IN= 2--> OUT= 1	15.00

Di stance	DATA FOR SECTION (1921.0) ----->	El evation	Mann ing
0.00		264.40	0.0500
3.64		263.94	0.0500
14.54		261.05	0.0500
29.08		256.78	0.0500
36.36		254.96	0.0500
50.90		252.20	0.0500
65.44		249.78	0.0500
76.35		246.86	0.0500
77.24		245.90	0.0500
82.24		245.45	0.0500 / 0.0350
82.25		244.90	0.0350
87.25		244.90	0.0350
87.26		245.45	0.0350 / 0.0500
94.53		246.89	0.0500
170.87		247.16	0.0500
218.13		248.69	0.0500
265.40		249.22	0.0500
301.75		250.78	0.0500
338.11		253.47	0.0500
359.92		264.00	0.0500

TRAVEL TIME TABLE						
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)	
0.55	245.45	.345E+04	2.0	0.71	29.20	29.20
1.53	246.43	.174E+05	14.6	1.05	19.92	19.92
2.50	247.40	.767E+05	52.8	0.86	24.20	24.20
3.48	248.38	.224E+06	199.8	1.11	18.71	18.71
4.46	249.36	.429E+06	441.2	1.29	16.19	16.19
5.43	250.33	.691E+06	874.3	1.58	13.18	13.18
6.41	251.31	.987E+06	1468.3	1.86	11.20	11.20
7.38	252.28	.131E+07	2224.8	2.13	9.79	9.79
8.36	253.26	.165E+07	3132.4	2.37	8.78	8.78
9.34	254.24	.201E+07	4256.4	2.65	7.87	7.87
10.31	255.21	.238E+07	5551.0	2.92	7.15	7.15
11.29	256.19	.276E+07	6996.8	3.17	6.57	6.57
12.27	257.17	.314E+07	8585.9	3.42	6.10	6.10
13.24	258.14	.353E+07	10315.9	3.65	5.71	5.71
14.22	259.12	.393E+07	12180.7	3.87	5.38	5.38
15.19	260.09	.434E+07	14178.2	4.09	5.10	5.10
16.17	261.07	.475E+07	16306.5	4.29	4.85	4.85
17.15	262.05	.517E+07	18558.6	4.49	4.64	4.64
18.12	263.02	.559E+07	20939.7	4.68	4.45	4.45

<---- hydrograph ---->						
		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	<- pi pe / channel ->
INFLOW : ID= 2 (8072)	OUTFLOW: ID= 1 (6192)	2835.26	84.84	6.25	40.65	2.72
		2835.26	70.63	6.75	40.65	0.91
						0.89

CALIB STANDHYD (1921)			
ID= 1 DT=15.0 mi n	Area (ha)	Imp(%)	Di r. Conn. (%)
	72.27	42.00	19.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	=	30.35	41.92
Dep. Storage (mm)	=	2.00	5.00
Average Slope (%)	=	0.50	0.50
Length (m)	=	694.12	40.00
Mannings n	=	0.013	0.250
Max. Eff. Inten. (mm/hr)	=	116.82	68.94
over (mi n)	=	15.00	30.00
Storage Coeff. (mi n)	=	9.45 (ii)	21.87 (ii)
Uni t Hyd. Tpeak (mi n)	=	15.00	30.00
Uni t Hyd. peak (cms)	=	0.09	0.05

TOTALS		
PEAK FLOW (cms)	=	3.82
TIME TO PEAK (hrs)	=	6.00
RUNOFF VOLUME (mm)	=	86.50
TOTAL RAINFALL (mm)	=	88.50
RUNOFF COEFFICIENT	=	0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 57.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5192)			
IN= 2---> OUT= 1 DT= 15.0 mi n			
OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.1390	1.4346
0.2060	0.7072	3.8730	1.6637
0.9550	0.8170	4.8310	1.9184
1.7940	1.0081	5.2310	1.9284
2.4250	1.1871	0.0000	0.0000

		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1921)		72.270	6.295	6.00	42.80
OUTFLOW: ID= 1 (5192)		72.270	2.756	6.75	42.79

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 43.77

TIME SHIFT OF PEAK FLOW (mi n) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 1.3250

CALIB STANDHYD (1922)			
ID= 1 DT=15.0 mi n	Area (ha)	Imp(%)	Di r. Conn. (%)
	174.34	39.00	18.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	=	67.99	106.35
Dep. Storage (mm)	=	2.00	5.00
Average Slope (%)	=	0.50	0.50
Length (m)	=	1078.08	40.00
Mannings n	=	0.013	0.250
Max. Eff. Inten. (mm/hr)	=	116.82	64.58
over (mi n)	=	15.00	30.00
Storage Coeff. (mi n)	=	12.31 (ii)	25.05 (ii)
Uni t Hyd. Tpeak (mi n)	=	15.00	30.00
Uni t Hyd. peak (cms)	=	0.08	0.04

TOTALS		
PEAK FLOW (cms)	=	8.02
TIME TO PEAK (hrs)	=	6.00
RUNOFF VOLUME (mm)	=	86.50
TOTAL RAINFALL (mm)	=	88.50
RUNOFF COEFFICIENT	=	0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 57.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8074)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1922):	174.34	13.475	6.00	41.56
+ ID2= 2 (5192):	72.27	2.756	6.75	42.79
ID = 3 (8074):	246.61	14.267	6.25	41.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8076)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6192):	2835.26	70.631	6.75	40.65
+ ID2= 2 (8074):	246.61	14.267	6.25	41.92
ID = 3 (8076):	3081.87	78.994	6.75	40.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0196)			
ID= 1 DT=15.0 mi n	Area (ha)	Curve Number (CN)	# of Li near Res. (N)
	41.47	70.0	1.50
	Ia (mm) = 5.00		
	U.H. Tp (hrs) = 0.24		

Uni t Hyd Opeak (cms)	=	2.950
PEAK FLOW (cms)	=	2.030 (i)
TIME TO PEAK (hrs)	=	6.000
RUNOFF VOLUME (mm)	=	33.271
TOTAL RAINFALL (mm)	=	88.500
RUNOFF COEFFICIENT	=	0.376

- (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

STANDHYD (1941)
 ID= 1 DT=15.0 min

Area (ha)=	0.76		
Total Imp(%)=	45.00	Di r. Conn.(%)=	15.00
IMPERVIOUS PERVIOUS (i)			
Surface Area (ha)=	0.34		0.42
Dep. Storage (mm)=	2.00		5.00
Average Slope (%)=	0.50		0.50
Length (m)=	71.18		40.00
Mannings n =	0.013		0.250

Max. Eff. Inten. (mm/hr)=	116.82	105.29
over (min)	15.00	15.00
Storage Coeff. (min)=	2.41 (ii)	12.89 (ii)
Unit Hyd. Tpeak (min)=	15.00	15.00
Unit Hyd. peak (cms)=	0.11	0.08

TOTALS

PEAK FLOW (cms)=	0.04	0.09	0.128 (iii)
TIME TO PEAK (hrs)=	6.00	6.00	6.00
RUNOFF VOLUME (mm)=	86.50	44.32	50.63
TOTAL RAINFALL (mm)=	88.50	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.50	0.57

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 67.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5194)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0510	0.0197
0.0030	0.0093	0.0640	0.0226
0.0150	0.0107	0.0740	0.0253
0.0270	0.0138	0.0740	0.0353
0.0380	0.0165	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1941) 0.760	0.128	6.00	50.63
OUTFLOW: ID= 1 (5194) 0.760	0.040	6.25	49.62

PEAK FLOW REDUCTION [Oout/Oin](%) = 31.50
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0174

CALIB NASHYD (1942)
 ID= 1 DT=15.0 min

Area (ha)=	255.33	Curve Number (CN)=	69.5
Ia (mm)=	5.00	# of Linear Res. (N)=	1.50
U. H. Tp (hrs)=	0.68		

Unit Hyd Opeak (cms)=	6.448
PEAK FLOW (cms)=	5.827 (i)
TIME TO PEAK (hrs)=	6.750
RUNOFF VOLUME (mm)=	35.110
TOTAL RAINFALL (mm)=	88.500
RUNOFF COEFFICIENT =	0.397

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9194)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1942):	255.33	5.827	35.11
+ ID2= 2 (5194):	0.76	0.040	49.62
=====			
ID = 3 (9194):	256.09	5.862	35.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8078)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0196):	41.47	2.030	33.27
+ ID2= 2 (9194):	256.09	5.862	35.15
=====			
ID = 3 (8078):	297.56	7.472	34.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6198)
 IN= 2---> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1981.0) ----->

Distance	Elevation	Manning	
0.00	267.15	0.0500	
22.99	265.94	0.0500	
51.73	261.39	0.0500	
74.72	258.75	0.0500	
94.83	257.79	0.0500	
114.95	254.36	0.0500	
126.44	254.06	0.0500	
132.19	253.68	0.0500	
135.06	253.35	0.0500	
137.94	252.93	0.0500 / 0.0300	Main Channel
140.81	252.41	0.0300	Main Channel
143.69	251.89	0.0300	Main Channel
146.56	252.51	0.0300	Main Channel
149.43	253.36	0.0300 / 0.0500	Main Channel
158.05	255.89	0.0500	
172.42	256.25	0.0500	
204.03	259.15	0.0500	
229.90	260.36	0.0500	
252.89	260.46	0.0500	
284.50	260.45	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.35	252.24	.111E+04	0.6	0.97	31.07
0.69	252.58	.442E+04	3.8	1.55	19.42
1.04	252.93	.974E+04	11.1	2.06	14.65
1.51	253.40	.203E+05	31.2	2.77	10.87
1.98	253.87	.357E+05	65.0	3.30	9.14
2.45	254.34	.596E+05	112.9	3.43	8.80
2.92	254.81	.935E+05	186.8	3.62	8.34
3.39	255.28	.131E+06	282.1	3.89	7.75
3.86	255.75	.172E+06	398.2	4.18	7.22
4.33	256.22	.221E+06	527.0	4.31	6.99
4.80	256.69	.282E+06	697.3	4.47	6.75
5.27	257.16	.350E+06	898.5	4.64	6.50
5.74	257.63	.425E+06	1131.4	4.82	6.26
6.21	258.10	.508E+06	1367.4	4.88	6.19
6.68	258.57	.603E+06	1635.6	4.91	6.14
7.15	259.04	.709E+06	1977.2	5.04	5.98
7.62	259.51	.826E+06	2360.5	5.17	5.83
8.09	259.98	.954E+06	2790.7	5.29	5.70
8.56	260.45	.110E+07	3179.6	5.25	5.74

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8078)	297.56	7.47	6.50	34.89	0.87	1.77
OUTFLOW: ID= 1 (6198)	297.56	7.24	6.75	34.89	0.86	1.76

CALIB STANDHYD (1981)
 ID= 1 DT=15.0 min

Area (ha)=	149.95	Di r. Conn.(%)=	19.00
Total Imp(%)=	44.00		

Surface Area (ha)=	65.98	PERVIOUS (i)	83.97
Dep. Storage (mm)=	2.00		5.00
Average Slope (%)=	0.50		0.50
Length (m)=	999.83		40.00
Mannings n =	0.013		0.250

Max. Eff. Inten. (mm/hr)=	116.82	73.94
---------------------------	--------	-------

over (min) 15.00 30.00
 Storage Coeff. (min)= 11.77 (ii) 23.84 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

TOTALS
 PEAK FLOW (cms)= 7.40 8.05 12.498 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 86.50 33.66 43.70
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.38 0.49

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 57.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5198)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	10.9910	3.4957
0.4390	1.7800	13.3530	3.9512
2.6180	2.0449	15.3540	4.3979
5.1680	2.5932	15.7540	4.4079
7.9230	3.0040	0.0000	0.0000

INFLOW : ID= 2 (1981)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
149.950	149.950	12.498	6.00	43.70
OUTFLOW: ID= 1 (5198)	149.950	6.191	6.50	43.70

PEAK FLOW REDUCTION [Oout/Oin] (%) = 49.54
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 2.7998

CALIB STANDHYD (1982)
 ID= 1 DT=15.0 min

Area (ha) = 240.61
 Total Imp(%) = 20.00
 Dir. Conn. (%) = 9.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) = 48.12	192.49
Dep. Storage (mm) = 2.00	5.00
Average Slope (%) = 0.50	0.50
Length (m) = 1266.52	40.00
Mannings n = 0.013	0.250

Max. Eff. Inten. (mm/hr) = 116.82 48.80
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 13.56 (ii) 27.81 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.08 0.04

TOTALS
 PEAK FLOW (cms) = 5.34 11.20 13.604 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.25
 RUNOFF VOLUME (mm) = 86.50 28.39 33.62
 TOTAL RAINFALL (mm) = 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.32 0.38

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 57.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8080)
 1 + 2 = 3

ID1= 1 (1982):	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
240.61	13.604	6.25	33.62	

+ ID2= 2 (5198):	149.95	6.191	6.50	43.70
ID = 3 (8080):	390.56	17.801	6.25	37.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8082)
 1 + 2 = 3

ID1= 1 (6198):	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
297.56	7.243	6.75	34.89	
+ ID2= 2 (8080):	390.56	17.801	6.25	37.49
ID = 3 (8082):	688.12	23.047	6.25	36.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8084)
 1 + 2 = 3

ID1= 1 (8076):	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
3081.87	78.994	6.75	40.75	
+ ID2= 2 (8082):	688.12	23.047	6.25	36.37
ID = 3 (8084):	3769.99	99.594	6.75	39.95

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6200)
 IN= 2---> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (2001.0) ----->

Distance	Elevation	Manning
0.00	252.47	0.0500
10.38	252.29	0.0500
20.76	252.28	0.0500
41.52	247.66	0.0500
62.28	247.47	0.0500
80.45	247.34	0.0500
90.83	246.85	0.0500
103.81	245.51	0.0500
106.00	244.50	0.0500
108.99	244.05	0.0500 / 0.0300
109.00	243.50	0.0300
114.00	243.50	0.0300
114.01	244.05	0.0300 / 0.0500
118.99	244.50	0.0500
132.36	245.09	0.0500
150.52	245.83	0.0500
199.83	249.24	0.0500
207.62	250.16	0.0500
236.16	253.11	0.0500
256.92	253.83	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.28	243.77	365E+04	0.7	0.54	81.42
0.55	244.05	730E+04	2.2	0.81	54.59
1.05	244.55	197E+05	7.2	0.97	45.51
1.54	245.04	463E+05	17.2	0.99	44.76
2.04	245.54	895E+05	34.8	1.03	42.81
2.53	246.03	152E+06	62.8	1.09	40.43
3.03	246.53	232E+06	104.7	1.20	36.91
3.52	247.02	328E+06	159.7	1.29	34.19
4.02	247.52	450E+06	223.4	1.32	33.56
4.51	248.01	627E+06	326.9	1.38	31.99
5.01	248.51	820E+06	463.0	1.50	29.52
5.50	249.00	103E+07	626.0	1.62	27.30
6.00	249.50	124E+07	820.1	1.75	25.24
6.49	249.99	147E+07	1044.4	1.89	23.42
6.99	250.49	170E+07	1292.6	2.01	21.95
7.48	250.98	195E+07	1565.6	2.13	20.71
7.98	251.48	220E+07	1864.5	2.25	19.65
8.47	251.97	246E+07	2189.4	2.36	18.73
8.97	252.47	274E+07	2427.5	2.35	18.80

<---- hydrograph ----> <- pipe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8084)	3769.99	99.59	6.75	39.95	2.97	1.18
OUTFLOW: ID= 1 (6200)	3769.99	82.16	7.25	39.95	2.76	1.14

CALIB
STANDHYD (2001)
ID= 1 DT=15.0 min

Area (ha)=	42.48
Total Imp(%)=	46.00
Dir. Conn.(%)=	19.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	19.54	22.94
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	532.17	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	116.82	107.95
over (min)	15.00	30.00
Storage Coeff. (min)=	8.06 (ii)	18.43 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.10	0.05

PEAK FLOW (cms)=	2.34	3.63	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	4.735 (iii)
RUNOFF VOLUME (mm)=	86.50	46.69	54.26
TOTAL RAINFALL (mm)=	88.50	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.53	0.61

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.7 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5200) IN= 2----> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	2.7210	1.0525
	0.1450	0.4892	3.4130	1.1947
	0.7930	0.5722	4.0030	1.3346
	1.4410	0.7540	4.4030	1.3446
	2.0370	0.8927	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2001)	42.480	4.735	6.00	54.26
OUTFLOW: ID= 1 (5200)	42.480	2.343	6.50	54.24

PEAK FLOW REDUCTION [Qout/Qin] (%) = 49.47
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 0.9767

CALIB
STANDHYD (2002)
ID= 1 DT=15.0 min

Area (ha)=	72.79
Total Imp(%)=	31.00
Dir. Conn.(%)=	13.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	22.56	50.23
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	696.61	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	116.82	82.65
over (min)	15.00	30.00
Storage Coeff. (min)=	9.47 (ii)	21.02 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.05

PEAK FLOW (cms)=	2.63	5.73	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	6.608 (iii)
RUNOFF VOLUME (mm)=	86.50	42.53	48.25

TOTAL RAINFALL (mm)=	88.50	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.48	0.55

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.7 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8086) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2002):	72.79	6.608	6.25	48.25
+ ID2= 2 (5200):	42.48	2.343	6.50	54.24
ID = 3 (8086):	115.27	8.417	6.25	50.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8088) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6200):	3769.99	82.160	7.25	39.95
+ ID2= 2 (8086):	115.27	8.417	6.25	50.45
ID = 3 (8088):	3885.26	85.203	7.25	40.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8038) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8034):	5220.76	32.952	7.00	25.62
+ ID2= 2 (8088):	3885.26	85.203	7.25	40.26
ID = 3 (8038):	9106.02	117.801	7.25	31.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (1365)
IN= 2----> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (1362.0) ----->

Distance	Elevation	Manning
0.00	252.00	0.0400
2.35	244.60	0.0400
39.94	243.14	0.0400
58.74	242.65	0.0400
91.63	242.03	0.0400
101.03	241.69	0.0400
103.38	241.66	0.0400
105.73	240.76	0.0400
105.93	240.59	0.0400
109.18	240.28	0.0400 / 0.0300
110.43	239.81	0.0300
111.68	239.81	0.0300
113.43	240.09	0.0300 / 0.0400
118.43	240.50	0.0400
138.62	241.68	0.0400
152.72	241.60	0.0400
166.82	242.68	0.0400
185.62	249.09	0.0400
197.36	251.00	0.0400
232.61	252.28	0.0400

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.28	240.09	.118E+04	0.1	0.20	142.09

0.91	240.72	.114E+05	2.4	0.35	79.92
1.53	241.34	.352E+05	9.4	0.45	62.17
2.16	241.97	.831E+05	22.8	0.46	60.69
2.79	242.60	.172E+06	54.2	0.53	52.74
3.41	243.22	.297E+06	111.4	0.63	44.49
4.04	243.85	.445E+06	194.1	0.74	38.22
4.67	244.48	.612E+06	301.1	0.83	33.88
5.29	245.10	.793E+06	443.0	0.94	29.82
5.92	245.73	.976E+06	611.3	1.06	26.60
6.55	246.36	.116E+07	802.0	1.17	24.12
7.18	246.99	.135E+07	1014.0	1.27	22.16
7.80	247.61	.154E+07	1246.2	1.37	20.56
8.43	248.24	.173E+07	1498.0	1.46	19.24
9.06	248.87	.192E+07	1768.8	1.55	18.12
9.68	249.49	.212E+07	2049.6	1.63	17.24
10.31	250.12	.232E+07	2344.7	1.71	16.49
10.94	250.75	.252E+07	2658.5	1.78	15.83
11.56	251.37	.274E+07	2931.2	1.81	15.56

		<--- hydrograph --->			<--pi pe / channel -->	
AREA	OPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (8038)	9106.02	117.80	7.25	31.87	3.46	
OUTFLOW : ID= 1 (1365)	9106.02	99.11	7.75	31.87	3.29	

CALIB STANDHYD (1321)		Area (ha)= 208.90	Total Imp(%)= 67.00	Di r. Conn.(%)= 46.00
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		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	139.96	68.94
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	1180.11	40.00
Mannings n	=	0.013	0.250
Max. Eff. Inten. (mm/hr)=		116.82	106.46
over (mi n)		15.00	30.00
Storage Coeff. (mi n)=		13.00 (ii)	23.43 (ii)
Uni t Hyd. Tpeak (mi n)=		15.00	30.00
Uni t Hyd. peak (cms)=		0.08	0.04
PEAK FLOW (cms)=		24.07	9.66
TIME TO PEAK (hrs)=		6.00	6.25
RUNOFF VOLUME (mm)=		86.50	42.45
TOTAL RAINFALL (mm)=		88.50	88.50
RUNOFF COEFFICIENT =		0.98	0.48

TOTALS
30.312 (iii)
62.71
88.50
0.71

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5132)		IN= 2--> OUT= 1		DT= 15.0 mi n	
OUTFLOW	STORAGE	OUTFLOW	STORAGE		
(cms)	(ha. m.)	(cms)	(ha. m.)		
0.0000	0.0000	7.9970	7.9437		
0.9210	5.2645	12.4650	8.5839		
3.2690	5.3433	16.9060	9.2236		
6.3980	5.6925	17.3060	9.2336		
7.1860	6.8155	0.0000	0.0000		
AREA	OPEAK	TPEAK	R.V.		
(ha)	(cms)	(hrs)	(mm)		
INFLOW : ID= 2 (1321)	208.900	30.312	6.00	62.71	
OUTFLOW : ID= 1 (5132)	208.900	7.460	6.75	62.70	

PEAK FLOW REDUCTION [Oout/Oi n] (%) = 24.61
TIME SHIF T OF PEAK FLOW (mi n) = 45.00
MAXI MUM STORAGE USED (ha. m.) = 7.2206

CALIB STANDHYD (1322)	Area (ha)= 81.94
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|| ID= 1 DT=15.0 mi n | Total Imp(%)= 37.00 Di r. Conn.(%)= 25.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	30.32	51.62
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	739.10	40.00
Mannings n	=	0.013	0.250
Max. Eff. Inten. (mm/hr)=		116.82	63.56
over (mi n)		15.00	30.00
Storage Coeff. (mi n)=		9.81 (ii)	22.64 (ii)
Uni t Hyd. Tpeak (mi n)=		15.00	30.00
Uni t Hyd. peak (cms)=		0.09	0.04
PEAK FLOW (cms)=		5.64	4.36
TIME TO PEAK (hrs)=		6.00	6.25
RUNOFF VOLUME (mm)=		86.50	34.96
TOTAL RAINFALL (mm)=		88.50	88.50
RUNOFF COEFFICIENT =		0.98	0.40

TOTALS
8.407 (iii)
6.00
47.84
88.50
0.54

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8090)		Area (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3					
ID1= 1 (1322):	81.94	8.407	6.00	47.84	
+ ID2= 2 (5132):	208.90	7.460	6.75	62.70	
ID = 3 (8090):	290.84	12.947	6.25	58.52	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (1342)		Area (ha)= 97.16	Total Imp(%)= 41.00	Di r. Conn.(%)= 28.00
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		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	39.84	57.32
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	804.82	40.00
Mannings n	=	0.013	0.250
Max. Eff. Inten. (mm/hr)=		116.82	51.04
over (mi n)		15.00	30.00
Storage Coeff. (mi n)=		10.33 (ii)	24.33 (ii)
Uni t Hyd. Tpeak (mi n)=		15.00	30.00
Uni t Hyd. peak (cms)=		0.09	0.04
PEAK FLOW (cms)=		7.38	3.74
TIME TO PEAK (hrs)=		6.00	6.25
RUNOFF VOLUME (mm)=		86.50	27.74
TOTAL RAINFALL (mm)=		88.50	88.50
RUNOFF COEFFICIENT =		0.98	0.31

TOTALS
9.712 (iii)
6.00
44.20
88.50
0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 54.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1341)		Area (ha)= 58.79	Total Imp(%)= 30.00	Di r. Conn.(%)= 20.00
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		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	17.64	41.15
Dep. Storage	(mm)=	2.00	5.00

Average Slope (%) = 0.50 0.50
 Length (m) = 626.05 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 116.82 45.39
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 8.88 (ii) 23.56 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.09 0.04

PEAK FLOW (cms) = 3.33 2.42 *TOTALS* 4.838 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 86.50 26.39 38.41
 TOTAL RAINFALL (mm) = 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.30 0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 54.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5134)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.1250	1.0755
0.1070	0.6119	1.7570	1.1641
0.4130	0.6225	2.3860	1.2526
0.8380	0.7018	2.7860	1.2626
0.9790	0.8884	0.0000	0.0000

INFLOW: ID= 2 (1341)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5134)	58.790	4.838	6.00	38.41
	58.790	1.557	6.75	38.39

PEAK FLOW REDUCTION [Qout/Qin] (%) = 32.19
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 1.1365

ADD HYD (9134)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1342):	97.16	9.712	6.00	44.20
+ ID2= 2 (5134):	58.79	1.557	6.75	38.39
ID = 3 (9134):	155.95	9.817	6.00	42.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8092)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8090):	290.84	12.947	6.25	58.52
+ ID2= 2 (9134):	155.95	9.817	6.00	42.01
ID = 3 (8092):	446.79	20.342	6.25	52.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (1366)
 IN= 2---> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (1361.0) ----->

Distance	Elevation	Manning
0.00	262.81	0.0400
37.23	260.54	0.0400
79.12	256.98	0.0400
111.70	254.74	0.0400

172.20	252.76	0.0400
251.32	248.82	0.0400
255.97	248.75	0.0400
260.43	248.76	0.0400
265.43	248.49	0.0400
268.68	248.18	0.0400 / 0.0300
269.93	247.71	0.0300
271.18	247.71	0.0300
272.93	247.99	0.0300
277.93	248.40	0.0300 / 0.0400
283.90	248.88	0.0400
307.17	249.60	0.0400
321.13	251.59	0.0400
363.01	257.08	0.0400
400.25	257.58	0.0400
460.75	261.91	0.0400

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.47	248.18	.881E+04	1.3	0.76	114.66
1.19	248.90	.742E+05	17.3	1.22	71.44
1.92	249.63	.278E+06	78.3	1.48	59.26
2.64	250.35	.589E+06	210.3	1.87	46.69
3.36	251.07	.974E+06	414.3	2.23	39.19
4.08	251.79	.143E+07	696.2	2.55	34.32
4.81	252.52	.197E+07	1063.0	2.83	30.86
5.53	253.24	.259E+07	1505.7	3.06	28.62
6.25	253.96	.331E+07	2051.3	3.26	26.85
6.97	254.68	.413E+07	2720.9	3.46	25.30
7.70	255.41	.504E+07	3584.5	3.73	23.43
8.42	256.13	.601E+07	4580.9	4.00	21.87
9.14	256.85	.704E+07	5707.9	4.25	20.57
9.86	257.57	.818E+07	6636.1	4.26	20.53
10.59	258.30	.945E+07	8115.0	4.50	19.42
11.31	259.02	.108E+08	9759.2	4.74	18.45
12.03	259.74	.122E+08	11569.5	4.96	17.61
12.75	260.46	.137E+08	13550.6	5.18	16.87
13.48	261.19	.153E+08	15636.4	5.37	16.29

<---- hydrograph ----> <- pipe / channel ->

INFLOW: ID= 2 (8092)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW: ID= 1 (1366)	446.79	20.34	6.25	52.75	1.23	1.23
	446.79	10.11	7.25	52.75	0.87	0.96

CALIB
 STANDHYD (1362)
 ID= 1 DT=15.0 min

Area (ha) = 371.79
 Total Imp (%) = 26.00
 Dir. Conn. (%) = 14.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) =	96.67 275.12
Dep. Storage (mm) =	2.00 5.00
Average Slope (%) =	0.50 0.50
Length (m) =	1574.36 40.00
Mannings n =	0.013 0.250

Max. Eff. Inten. (mm/hr) = 116.82 62.37
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 15.45 (ii) 28.37 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.07 0.04

PEAK FLOW (cms) = 12.15 20.34 *TOTALS* 26.369 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.25
 RUNOFF VOLUME (mm) = 86.50 35.12 42.31
 TOTAL RAINFALL (mm) = 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.40 0.48

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

STANDHYD (1361) | Area (ha)= 140.62
 ID= 1 DT=15.0 min | Total Imp(%)= 55.00 Di r. Conn.(%)= 29.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	77.34	63.28
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	968.23	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	116.82	102.42
over (min)	15.00	30.00
Storage Coeff. (min)=	11.54 (ii)	22.14 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04

			TOTALS
PEAK FLOW (cms)=	10.67	8.75	16.335 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	86.50	42.33	55.14
TOTAL RAINFALL (mm)=	88.50	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.48	0.62

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5136)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.7060	4.3486
0.3100	2.4365	5.9200	5.0324
1.7670	2.5993	7.0820	5.7151
3.4020	2.8510	7.4820	5.7251
4.0310	3.5962	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1361)	140.620	16.335	6.00	55.14
OUTFLOW: ID= 1 (5136)	140.620	4.484	7.00	55.12

PEAK FLOW REDUCTION [Qout/Oi n](%)= 27.45
 TIME SHIFT OF PEAK FLOW (min)= 60.00
 MAXIMUM STORAGE USED (ha.m.)= 4.1368

ADD HYD (8096)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1362):	371.79	26.369	6.25	42.31
+ ID2= 2 (5136):	140.62	4.484	7.00	55.12

ID = 3 (8096):	512.41	30.129	6.25	45.83

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8094)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1366):	446.79	10.113	7.25	52.75
+ ID2= 2 (8096):	512.41	30.129	6.25	45.83

ID = 3 (8094):	959.20	36.319	6.25	49.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8098)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1365):	9106.02	99.110	7.75	31.87

+ ID2= 2 (8094): 959.20 36.319 6.25 49.05
 ID = 3 (8098): 10065.22 118.031 7.75 33.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1381)
 ID= 1 DT=15.0 min

Area (ha)= 96.24
 Total Imp(%)= 59.00 Di r. Conn.(%)= 34.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	56.78	39.46
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	801.00	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	116.82	21.34
over (min)	15.00	45.00
Storage Coeff. (min)=	10.30 (ii)	30.14 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.09	0.03

			TOTALS
PEAK FLOW (cms)=	8.89	1.45	9.483 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	86.50	14.91	39.25
TOTAL RAINFALL (mm)=	88.50	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.17	0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 28.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5138)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	6.4380	2.7648
0.3260	1.4281	8.0830	3.0750
1.8650	1.6876	9.4100	3.3790
3.4730	2.0447	9.8100	3.3890
4.8170	2.3809	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1381)	96.240	9.483	6.00	39.25
OUTFLOW: ID= 1 (5138)	96.240	2.801	6.50	39.24

PEAK FLOW REDUCTION [Qout/Oi n](%)= 29.54
 TIME SHIFT OF PEAK FLOW (min)= 30.00
 MAXIMUM STORAGE USED (ha.m.)= 1.9182

CALIB
 NASHYD (1382)
 ID= 1 DT=15.0 min

Area (ha)= 318.55 Curve Number (CN)= 33.2
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.09

Unit Hyd Opeak (cms)= 4.976

PEAK FLOW (cms)=	1.570 (i)
TIME TO PEAK (hrs)=	7.750
RUNOFF VOLUME (mm)=	11.622
TOTAL RAINFALL (mm)=	88.500
RUNOFF COEFFICIENT =	0.131

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9138)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1382):	318.55	1.570	7.75	11.62

+ ID2= 2 (5138): 96.24 2.801 6.50 39.24
 ID= 3 (9138): 414.79 4.206 6.75 18.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (1402)
 ID= 1 DT=15.0 min

Area (ha)	= 131.22	Dir. Conn. (%)	= 21.00
Total Imp (%)	= 29.00		

Surface Area (ha)	= 38.05	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)	= 2.00		
Average Slope (%)	= 0.50		
Length (m)	= 935.31		
Mannings n	= 0.013		

Max. Eff. Inten. (mm/hr)	= 116.82	15.20
over (min)	= 15.00	45.00
Storage Coeff. (min)	= 11.30 (ii)	34.03 (ii)
Unit Hyd. Tpeak (min)	= 15.00	45.00
Unit Hyd. peak (cms)	= 0.08	0.03

PEAK FLOW (cms)	= 7.26	2.30	*TOTALS*
TIME TO PEAK (hrs)	= 6.00	6.50	8.195 (iii)
RUNOFF VOLUME (mm)	= 86.50	15.35	30.29
TOTAL RAINFALL (mm)	= 88.50	88.50	88.50
RUNOFF COEFFICIENT	= 0.98	0.17	0.34

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 37.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (1401)
 ID= 1 DT=15.0 min

Area (ha)	= 27.37	Dir. Conn. (%)	= 33.00
Total Imp (%)	= 45.00		

Surface Area (ha)	= 12.32	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)	= 2.00		
Average Slope (%)	= 0.50		
Length (m)	= 427.16		
Mannings n	= 0.013		

Max. Eff. Inten. (mm/hr)	= 116.82	18.14
over (min)	= 15.00	30.00
Storage Coeff. (min)	= 7.06 (ii)	28.24 (ii)
Unit Hyd. Tpeak (min)	= 15.00	30.00
Unit Hyd. peak (cms)	= 0.10	0.04

PEAK FLOW (cms)	= 2.70	0.53	*TOTALS*
TIME TO PEAK (hrs)	= 6.00	6.25	3.019 (iii)
RUNOFF VOLUME (mm)	= 86.50	16.66	39.71
TOTAL RAINFALL (mm)	= 88.50	88.50	88.50
RUNOFF COEFFICIENT	= 0.98	0.19	0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 37.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5140)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	1.8310	0.7127
0.0930	0.3344	2.2990	0.8148
0.5300	0.3869	2.6760	0.9148
0.9880	0.4994	3.0760	0.9248
1.3700	0.5962	0.0000	0.0000

INFLOW: ID= 2 (1401)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
27.370	27.370	3.019	6.00	39.71
OUTFLOW: ID= 1 (5140)	27.370	0.995	6.50	39.68

PEAK FLOW REDUCTION [Qout/Qin] (%) = 32.95
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 0.5081

ADD HYD (9140)
 1 + 2 = 3

ID1= 1 (1402):	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
131.22	8.195	6.00	30.29	
+ ID2= 2 (5140):	27.37	0.995	6.50	39.68
ID= 3 (9140):	158.59	8.584	6.00	31.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8102)
 1 + 2 = 3

ID1= 1 (9138):	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
414.79	4.206	6.75	18.03	
+ ID2= 2 (9140):	158.59	8.584	6.00	31.91
ID= 3 (8102):	573.38	9.722	6.00	21.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6142)
 IN= 2---> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (1421.0) ----->

Distance	Elevation	Manning	
0.00	295.50	0.0400	
7.25	295.36	0.0400	
14.50	295.22	0.0400	
21.75	295.22	0.0400	
29.00	294.23	0.0400	
52.56	286.45	0.0400	
67.06	281.44	0.0400	
77.94	278.28	0.0400	
82.81	277.20	0.0400 / 0.0300	Mai n Channel
83.21	276.80	0.0300	Mai n Channel
88.81	276.80	0.0300	Mai n Channel
96.41	276.80	0.0300	Mai n Channel
96.81	277.20	0.0300 / 0.0400	Mai n Channel
103.31	278.96	0.0400	
108.75	280.16	0.0400	
117.81	282.73	0.0400	
135.94	290.30	0.0400	
159.50	292.14	0.0400	
174.00	293.07	0.0400	
179.44	293.37	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.40	277.20	238E+05	6.9	1.27	57.28
1.25	278.05	889E+05	50.9	2.50	29.13
2.10	278.90	179E+06	133.4	3.26	22.36
2.95	279.75	293E+06	256.7	3.83	19.03
3.80	280.60	432E+06	426.0	4.31	16.90
4.66	281.46	593E+06	644.3	4.75	15.35
5.51	282.31	776E+06	915.3	5.16	14.12
6.36	283.16	978E+06	1242.2	5.55	13.12
7.21	284.01	120E+07	1626.2	5.94	12.27
8.06	284.86	143E+07	2065.6	6.30	11.57
8.91	285.71	169E+07	2562.4	6.64	10.97
9.76	286.56	196E+07	3118.0	6.97	10.46
10.61	287.41	224E+07	3733.3	7.28	10.02
11.46	288.26	255E+07	4411.9	7.57	9.62
12.31	289.11	287E+07	5155.6	7.86	9.27
13.17	289.97	321E+07	5966.6	8.14	8.96
14.02	290.82	357E+07	6705.0	8.22	8.87
14.87	291.67	398E+07	7494.3	8.24	8.85

15. 72 292. 52 . 444E+07 8405. 3 8. 28 8. 80

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8102)	573. 38	9. 72	6. 00	21. 87	0. 45	1. 31
OUTFLOW: ID= 1 (6142)	573. 38	6. 22	7. 25	21. 87	0. 36	1. 27

CALIB
STANDHYD (1421)
ID= 1 DT=15. 0 min

Area (ha)= 69. 77
Total Imp(%)= 45. 00 Di r. Conn. (%)= 15. 00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	31. 40	38. 37
Dep. Storage	2. 00	5. 00
Average Slope	0. 50	0. 50
Length	682. 01	40. 00
Mannings n	0. 013	0. 250

Max. Eff. Inten. (mm/hr)=	116. 82	85. 78
over (min)	15. 00	30. 00
Storage Coeff. (min)	9. 35 (ii)	20. 73 (ii)
Unit Hyd. Tpeak (min)	15. 00	30. 00
Unit Hyd. peak (cms)	0. 09	0. 05

TOTALS
PEAK FLOW (cms)= 2. 92 4. 56 5. 848 (iii)
TIME TO PEAK (hrs)= 6. 00 6. 25 6. 00
RUNOFF VOLUME (mm)= 86. 50 36. 45 43. 95
TOTAL RAINFALL (mm)= 88. 50 88. 50 88. 50
RUNOFF COEFFICIENT = 0. 98 0. 41 0. 50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 59. 0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5142)
IN= 2---> OUT= 1
DT= 15. 0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0. 0000	0. 0000	4. 6670	1. 8169
0. 2360	0. 8525	5. 8600	2. 0773
1. 3520	0. 9863	6. 8220	2. 3321
2. 5180	1. 2732	7. 2220	2. 3421
3. 4920	1. 5199	0. 0000	0. 0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1421)	69. 770	5. 848	6. 00	43. 95
OUTFLOW: ID= 1 (5142)	69. 770	2. 753	6. 75	43. 94

PEAK FLOW REDUCTION [Qout/Qin] (%) = 47. 08
TIME SHIFT OF PEAK FLOW (min) = 45. 00
MAXIMUM STORAGE USED (ha. m.) = 1. 3505

CALIB
NASHYD (1422)
ID= 1 DT=15. 0 min

Area (ha)= 326. 77 Curve Number (CN)= 61. 2
Ia (mm)= 5. 00 # of Linear Res. (N)= 1. 50
U. H. Tp (hrs)= 1. 21

Unit Hyd Opeak (cms)= 4. 626

PEAK FLOW (cms)=	3. 737 (i)
TIME TO PEAK (hrs)=	7. 750
RUNOFF VOLUME (mm)=	28. 292
TOTAL RAINFALL (mm)=	88. 500
RUNOFF COEFFICIENT =	0. 320

- (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9142)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (1422):	326. 77	3. 737	7. 75	28. 29
+ ID2= 2 (5142):	69. 77	2. 753	6. 75	43. 94
ID = 3 (9142):	396. 54	6. 149	6. 75	31. 05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8104)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6142):	573. 38	6. 218	7. 25	21. 87
+ ID2= 2 (9142):	396. 54	6. 149	6. 75	31. 05
ID = 3 (8104):	969. 92	12. 294	7. 00	25. 62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (1441)
ID= 1 DT=15. 0 min

Area (ha)= 6. 29
Total Imp(%)= 45. 00 Di r. Conn. (%)= 15. 00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	2. 83	3. 46
Dep. Storage	2. 00	5. 00
Average Slope	0. 50	0. 50
Length	204. 78	40. 00
Mannings n	0. 013	0. 250

Max. Eff. Inten. (mm/hr)=	116. 82	110. 16
over (min)	15. 00	15. 00
Storage Coeff. (min)	4. 54 (ii)	14. 84 (ii)
Unit Hyd. Tpeak (min)	15. 00	15. 00
Unit Hyd. peak (cms)	0. 11	0. 07

TOTALS
PEAK FLOW (cms)= 0. 30 0. 74 1. 043 (iii)
TIME TO PEAK (hrs)= 6. 00 6. 00 6. 00
RUNOFF VOLUME (mm)= 86. 50 46. 29 52. 32
TOTAL RAINFALL (mm)= 88. 50 88. 50 88. 50
RUNOFF COEFFICIENT = 0. 98 0. 52 0. 59

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69. 6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5144)
IN= 2---> OUT= 1
DT= 15. 0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0. 0000	0. 0000	0. 4210	0. 1639
0. 0210	0. 0769	0. 5290	0. 1874
0. 1220	0. 0890	0. 6150	0. 2104
0. 2270	0. 1148	1. 0150	0. 2204
0. 3150	0. 1371	0. 0000	0. 0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (1441)	6. 290	1. 043	6. 00	52. 32
OUTFLOW: ID= 1 (5144)	6. 290	0. 344	6. 50	52. 18

PEAK FLOW REDUCTION [Qout/Qin] (%) = 32. 94
TIME SHIFT OF PEAK FLOW (min) = 30. 00
MAXIMUM STORAGE USED (ha. m.) = 0. 1468

CALIB
NASHYD (1442)

Area (ha)= 129. 40 Curve Number (CN)= 74. 9

|| ID= 1 DT=15.0 min | I a (mm)= 5.00 # of Li near Res. (N)= 1.50
 U.H. Tp(hrs)= 0.70

Unit Hyd Opeak (cms)= 3.147

PEAK FLOW (cms)= 3.356 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 40.637
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.459

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9144) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1442):	129.40	3.356	6.75	40.64
+ ID2= 2 (5144):	6.29	0.344	6.50	52.18
-----	-----	-----	-----	-----
ID = 3 (9144):	135.69	3.663	6.75	41.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8106) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8104):	969.92	12.294	7.00	25.62
+ ID2= 2 (9144):	135.69	3.663	6.75	41.17
-----	-----	-----	-----	-----
ID = 3 (8106):	1105.61	15.866	7.00	27.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6146)
IN= 2----> OUT= 1 Routing time step (min)' = 15.00

----- DATA FOR SECTION (1461.0) -----			
Distance	Elevation	Manning	
0.00	262.00	0.0350	
39.58	257.13	0.0350	
123.69	257.51	0.0350	
183.07	256.83	0.0350	
202.86	255.73	0.0350	
212.75	254.73	0.0350	
222.65	252.86	0.0350	
232.44	250.70	0.0350 / 0.0300	Main Channel
236.44	250.20	0.0300	Main Channel
236.84	249.80	0.0300	Main Channel
242.44	249.80	0.0300	Main Channel
250.04	249.80	0.0300	Main Channel
250.44	250.20	0.0300	Main Channel
252.33	250.57	0.0300 / 0.0350	Main Channel
286.97	253.52	0.0350	
331.50	255.80	0.0350	
380.98	255.55	0.0350	
445.30	258.40	0.0350	
455.19	259.82	0.0350	
489.83	262.22	0.0350	

----- TRAVEL TIME TABLE -----					
DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.39	250.19	.169E+05	5.4	1.04	52.00
0.77	250.57	.373E+05	16.6	1.44	37.46
1.44	251.24	.911E+05	60.3	2.14	25.19
2.11	251.91	.169E+06	136.3	2.61	20.64
2.79	252.59	.270E+06	249.7	2.99	18.05
3.46	253.26	.396E+06	405.4	3.31	16.28
4.13	253.93	.549E+06	603.3	3.56	15.15
4.80	254.60	.737E+06	857.8	3.77	14.31
5.48	255.28	.963E+06	1174.6	3.95	13.67
6.15	255.95	.128E+07	1449.5	3.66	14.74
6.82	256.62	.173E+07	2046.4	3.83	14.07
7.49	257.29	.227E+07	2665.2	3.81	14.17
8.17	257.97	.310E+07	3705.2	3.87	13.93
8.84	258.64	.400E+07	5127.4	4.15	12.99

9.51	259.31	.492E+07	6858.6	4.51	11.95
10.18	259.98	.587E+07	8815.5	4.86	11.09
10.86	260.66	.684E+07	10954.6	5.18	10.41
11.53	261.33	.785E+07	13324.3	5.49	9.82
12.20	262.00	.890E+07	15923.2	5.79	9.31

<---- hydrograph ---->							<- pi pe / channel ->	
INFLOW : ID= 2 (8106)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)		
1105.61	1105.61	15.87	7.00	27.53	0.75	1.40		
OUTFLOW: ID= 1 (6146)	1105.61	14.01	7.75	27.53	0.68	1.32		

CALIB STANDHYD (1462)
ID= 1 DT=15.0 min Area Total (ha)= 112.22 Imp(%)= 32.00 Dir. Conn.(%)= 13.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	35.91	76.31
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	864.95	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 116.82 over (min) 89.30
 Storage Coeff. (min)= 10.79 (ii) 21.98 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.04

PEAK FLOW (cms)=	3.90	9.24	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	10.701 (iii)
RUNOFF VOLUME (mm)=	86.50	45.18	6.25
TOTAL RAINFALL (mm)=	88.50	88.50	50.55
RUNOFF COEFFICIENT =	0.98	0.51	88.50
			0.57

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 72.9 I a = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (1461)
ID= 1 DT=15.0 min Area Total (ha)= 83.29 Imp(%)= 38.00 Dir. Conn.(%)= 16.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	31.65	51.64
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	745.16	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 116.82 over (min) 97.43
 Storage Coeff. (min)= 9.86 (ii) 20.67 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.05

PEAK FLOW (cms)=	3.67	7.02	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	8.285 (iii)
RUNOFF VOLUME (mm)=	86.50	46.55	6.25
TOTAL RAINFALL (mm)=	88.50	88.50	52.94
RUNOFF COEFFICIENT =	0.98	0.53	88.50
			0.60

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 72.9 I a = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5146)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	8.9200	2.5813
0.2820	0.8782	11.0570	3.0328
2.5360	1.0372	12.9860	3.4993
4.7880	1.7148	13.3860	3.5093
6.7570	2.1134	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (1461)	83.290	8.285	6.25	52.94
OUTFLOW: ID= 1 (5146)	83.290	4.662	6.50	52.94

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 56.27
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha. m.) = 1.7050

ADD HYD (8099)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (1462):	112.22	10.701	6.25	50.55
+ ID2= 2 (5146):	83.29	4.662	6.50	52.94
ID = 3 (8099):	195.51	14.617	6.25	51.57

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8108)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6146):	1105.61	14.014	7.75	27.53
+ ID2= 2 (8099):	195.51	14.617	6.25	51.57
ID = 3 (8108):	1301.12	19.245	7.00	31.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8100)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8098):	10065.22	118.031	7.75	33.50
+ ID2= 2 (8108):	1301.12	19.245	7.00	31.14
ID = 3 (8100):	11366.34	136.159	7.75	33.23

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6202)
IN= 2--> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (2021.0) ----->

Distance	Elevation	Manning		
0.00	245.48	0.0400		
29.68	242.80	0.0400		
74.20	240.39	0.0400		
155.81	237.12	0.0400		
200.33	234.48	0.0400		
244.85	234.67	0.0400		
259.69	233.96	0.0400		
274.53	233.84	0.0400		
281.68	233.40	0.0400		
285.78	232.40	0.0400 / 0.0300	Main Channel	
286.78	231.40	0.0300	Main Channel	
296.78	231.40	0.0300	Main Channel	
299.78	232.40	0.0300 / 0.0400	Main Channel	
311.62	233.74	0.0400		
333.88	234.34	0.0400		
497.11	235.32	0.0400		
571.31	237.32	0.0400		
675.18	239.10	0.0400		

704.86 242.71 0.0400
734.54 242.62 0.0400

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.50	231.90	.221E+05	4.7	0.85	78.67
1.00	232.40	.482E+05	15.3	1.28	52.40
1.60	233.00	.913E+05	38.3	1.68	39.77
2.20	233.60	1.54E+06	72.9	1.90	35.24
2.80	234.20	2.78E+06	124.2	1.79	37.30
3.40	234.80	5.99E+06	214.3	1.44	46.59
4.01	235.41	1.25E+07	445.0	1.43	46.90
4.61	236.01	2.05E+07	838.4	1.64	40.81
5.21	236.61	2.93E+07	1361.6	1.86	35.89
5.81	237.21	3.89E+07	2011.2	2.08	32.23
6.41	237.81	4.94E+07	2753.5	2.24	29.91
7.01	238.41	6.12E+07	3635.0	2.39	28.04
7.61	239.01	7.41E+07	4672.5	2.53	26.43
8.21	239.61	8.80E+07	5999.3	2.74	24.44
8.82	240.22	1.02E+08	7510.0	2.94	22.72
9.42	240.82	1.17E+08	9201.9	3.15	21.23
10.02	241.42	1.32E+08	11059.7	3.35	19.95
10.62	242.02	1.48E+08	13073.4	3.54	18.87
11.22	242.62	1.64E+08	15242.1	3.73	17.93

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8100)	*****	136.16	7.75	33.23	2.88	1.74
OUTFLOW: ID= 1 (6202)	*****	126.94	8.50	33.23	2.82	1.78

ADD HYD (8112)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6202):	11366.34	126.944	8.50	33.23
+ ID2= 2 (8110):	468.53	47.586	6.00	62.45
ID = 3 (8112):	11834.87	132.996	8.25	34.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5555)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	*****	19.2100
2.8500	0.5200	*****	24.7300
8.1800	1.2600	*****	30.8600
15.0900	2.2100	*****	37.6100
23.2700	3.3900	*****	41.2200
32.5300	4.7800	*****	48.9000
36.5100	5.3900	*****	66.4700
42.7600	6.3700	*****	100.1200
53.8500	8.1300	*****	140.4100
91.7300	14.3200	*****	140.4200

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8112)	*****	132.996	8.25	34.39
OUTFLOW: ID= 1 (5555)	*****	129.805	8.75	34.39

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 97.60
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 20.9443

ADD HYD (8114)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (5555):	11834.87	129.805	8.75	34.39
+ ID2= 2 (8136):	2288.44	42.207	6.50	32.59
ID = 3 (8114):	14123.31	154.321	8.50	34.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (2061)
ID= 1 DT=15.0 min

Area (ha)	= 49.84	Dir. Conn. (%)	= 50.00
Total Imp (%)	= 76.00		

Surface Area (ha)	= 37.88	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)	= 2.00		
Average Slope (%)	= 0.50		
Length (m)	= 576.43		
Mannings n	= 0.013		

Max. Eff. Inten. (mm/hr)	= 116.82	187.78
over (min)	= 15.00	30.00
Storage Coeff. (min)	= 8.45 (ii)	16.77 (ii)
Unit Hyd. Tpeak (min)	= 15.00	30.00
Unit Hyd. peak (cms)	= 0.09	0.05

TOTALS
PEAK FLOW (cms) = 7.15 3.44 9.515 (iii)
TIME TO PEAK (hrs) = 6.00 6.25 6.00
RUNOFF VOLUME (mm) = 86.50 58.79 72.65
TOTAL RAINFALL (mm) = 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.66 0.82

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5206)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.5640	2.4459
0.1690	0.8986	4.2460	2.7624
1.3810	1.2959	4.8070	3.0770
2.2440	1.6904	5.2070	3.0870
2.8510	2.0623	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
49.840	9.515	6.00	72.65
49.840	2.604	6.50	72.62

PEAK FLOW REDUCTION [Out/Oi n] (%) = 27.37
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 1.9396

CALIB
STANDHYD (2062)
ID= 1 DT=15.0 min

Area (ha)	= 225.70	Dir. Conn. (%)	= 42.00
Total Imp (%)	= 64.00		

Surface Area (ha)	= 144.45	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)	= 2.00		
Average Slope (%)	= 0.50		
Length (m)	= 1226.65		
Mannings n	= 0.013		

Max. Eff. Inten. (mm/hr)	= 116.82	132.25
over (min)	= 15.00	30.00
Storage Coeff. (min)	= 13.30 (ii)	22.87 (ii)
Unit Hyd. Tpeak (min)	= 15.00	30.00
Unit Hyd. peak (cms)	= 0.08	0.04

TOTALS
PEAK FLOW (cms) = 23.53 14.43 33.121 (iii)
TIME TO PEAK (hrs) = 6.00 6.25 6.00
RUNOFF VOLUME (mm) = 86.50 53.19 67.18
TOTAL RAINFALL (mm) = 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.60 0.76

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.3 Ia = Dep. Storage (Above)

- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8144)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
225.70	33.121	6.00	67.18
49.84	2.604	6.50	72.62
275.54	34.246	6.00	68.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (2041)
ID= 1 DT=15.0 min

Area (ha)	= 209.90	Dir. Conn. (%)	= 21.00
Total Imp (%)	= 37.00		

Surface Area (ha)	= 77.66	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)	= 2.00		
Average Slope (%)	= 0.50		
Length (m)	= 1182.93		
Mannings n	= 0.013		

Max. Eff. Inten. (mm/hr)	= 116.82	63.68
over (min)	= 15.00	30.00
Storage Coeff. (min)	= 13.01 (ii)	25.83 (ii)
Unit Hyd. Tpeak (min)	= 15.00	30.00
Unit Hyd. peak (cms)	= 0.08	0.04

TOTALS
PEAK FLOW (cms) = 11.04 10.48 17.629 (iii)
TIME TO PEAK (hrs) = 6.00 6.25 6.00
RUNOFF VOLUME (mm) = 86.50 33.37 44.53
TOTAL RAINFALL (mm) = 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.38 0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5204)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	13.0280	5.1901
0.5340	2.0405	16.1840	5.6427
3.0020	2.3552	19.0840	6.7700
5.5440	3.8411	19.4840	6.7800
8.4130	4.5335	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
209.900	17.629	6.00	44.53
209.900	7.078	6.75	44.53

PEAK FLOW REDUCTION [Out/Oi n] (%) = 40.15
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 4.2140

CALIB
STANDHYD (2042)
ID= 1 DT=15.0 min

Area (ha)	= 114.52	Dir. Conn. (%)	= 34.00
Total Imp (%)	= 61.00		

Surface Area (ha)	= 69.86	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)	= 2.00		
Average Slope (%)	= 0.50		
Length (m)	= 873.77		
Mannings n	= 0.013		

Max. Eff. Inten. (mm/hr)= 116.82 104.27
 over (min) 15.00 30.00
 Storage Coeff. (min)= 10.85 (ii) 21.37 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. Tpeak (cms)= 0.08 0.05

PEAK FLOW (cms)= 10.40 6.39
 TIME TO PEAK (hrs)= 6.00 6.25
 RUNOFF VOLUME (mm)= 86.50 40.32
 TOTAL RAINFALL (mm)= 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.46

TOTALS
 14.523 (iii)
 56.02
 88.50
 0.63

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
 CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8140)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2042):	114.52	14.523	6.00	56.02
+ ID2= 2 (5204):	209.90	7.078	6.75	44.53
ID = 3 (8140):	324.42	15.912	6.00	48.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6206)
 IN= 2----> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (2061.0) ----->

Distance	Elevation	Manning	
0.00	253.07	0.0350	
13.98	252.89	0.0350	
27.95	251.99	0.0350	
34.94	252.18	0.0350	
41.93	252.02	0.0350	
73.38	250.35	0.0350	
94.34	247.09	0.0350	
111.81	244.79	0.0350	
115.31	244.24	0.0350	
117.55	244.20	0.0350 / 0.0300	Main Channel
118.80	243.60	0.0300	Main Channel
122.30	243.60	0.0300	Main Channel
122.55	244.60	0.0300 / 0.0350	Main Channel
146.76	244.74	0.0350	
164.23	247.26	0.0350	
213.15	248.92	0.0350	
241.10	249.51	0.0350	
272.55	250.87	0.0350	
300.50	252.26	0.0350	
345.93	253.67	0.0350	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.30	243.90	.310E+04	1.4	1.23	36.25
0.60	244.20	.676E+04	4.6	1.81	24.64
1.12	244.72	.221E+05	17.1	2.07	21.60
1.64	245.24	.754E+05	64.1	2.28	19.60
2.17	245.77	.139E+06	149.8	2.88	15.50
2.69	246.29	.214E+06	273.3	3.43	13.04
3.21	246.81	.299E+06	436.1	3.91	11.43
3.73	247.33	.395E+06	632.6	4.30	10.40
4.25	247.85	.511E+06	836.4	4.39	10.18
4.77	248.37	.653E+06	1114.9	4.58	9.76
5.30	248.90	.822E+06	1472.1	4.81	9.30
5.82	249.42	.102E+07	1872.2	4.91	9.10
6.34	249.94	.126E+07	2444.6	5.22	8.56
6.86	250.46	.151E+07	3113.1	5.52	8.09
7.38	250.98	.180E+07	3843.5	5.74	7.79
7.90	251.50	.211E+07	4717.8	6.00	7.45
8.43	252.03	.245E+07	5701.4	6.24	7.16
8.95	252.55	.284E+07	6727.5	6.36	7.03
9.47	253.07	.326E+07	7885.3	6.48	6.90

<---- hydrograph ----> <-pi pe / channel ->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW: ID= 2 (8140) 324.42 15.91 6.00 48.58 1.07 2.04
 OUTFLOW: ID= 1 (6206) 324.42 12.76 6.50 48.58 0.94 1.97

ADD HYD (8142)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6206):	324.42	12.763	6.50	48.58
+ ID2= 2 (8144):	275.54	34.246	6.00	68.16
ID = 3 (8142):	599.96	40.621	6.00	57.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8138)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8114):	14123.31	154.321	8.50	34.10
+ ID2= 2 (8142):	599.96	40.621	6.00	57.58
ID = 3 (8138):	14723.27	163.513	8.25	35.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6208)
 IN= 2----> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (2081.0) ----->

Distance	Elevation	Manning	
0.00	249.19	0.1000	
73.11	244.13	0.1000	
121.86	243.39	0.1000	
243.71	240.01	0.1000	
353.38	238.75	0.1000	
450.87	235.40	0.1000	
499.61	231.40	0.1000	
511.80	230.96	0.1000	
523.98	230.12	0.1000	
534.50	227.80	0.1000 / 0.0500	Main Channel
536.50	226.70	0.0500	Main Channel
546.50	226.70	0.0500	Main Channel
548.50	227.80	0.0500 / 0.1000	Main Channel
572.72	230.25	0.1000	
682.40	230.50	0.1000	
804.25	233.95	0.1000	
926.11	235.83	0.1000	
1047.96	240.70	0.1000	
1145.45	243.26	0.1000	
1206.38	247.03	0.1000	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.55	227.25	.111E+05	2.0	0.33	92.44
1.10	227.80	.242E+05	6.5	0.50	61.56
2.23	228.93	.700E+05	26.0	0.68	44.91
3.36	230.06	.150E+06	60.8	0.74	41.04
4.49	231.19	.446E+06	130.8	0.54	56.83
5.62	232.32	.925E+06	287.4	0.57	53.66
6.76	233.46	.152E+07	522.9	0.63	48.32
7.89	234.59	.223E+07	827.2	0.68	44.91
9.02	235.72	.312E+07	1219.5	0.72	42.61
10.15	236.85	.418E+07	1780.6	0.78	39.11
11.28	237.98	.537E+07	2479.2	0.85	36.07
12.41	239.11	.669E+07	3282.9	0.90	33.95
13.54	240.24	.824E+07	4228.6	0.94	32.47
14.67	241.37	.997E+07	5384.6	0.99	30.87
15.81	242.51	.119E+08	6698.4	1.03	29.56
16.94	243.64	.140E+08	8235.2	1.08	28.25
18.07	244.77	.162E+08	10104.2	1.14	26.73
19.20	245.90	.185E+08	12251.5	1.21	25.21
20.33	247.03	.209E+08	14597.0	1.28	23.89

		<---- hydrograph ---->	<--pi pe / channel -->
	AREA	OPEAK	TPEAK
	(ha)	(cms)	(hrs)
INFLOW : ID= 2 (8138)	*****	163.51	8.25
OUTFLOW: ID= 1 (6208)	*****	148.29	9.25
		R. V.	(mm)
		MAX DEPTH	MAX VEL
		(m)	(m/s)
		4.73	0.54
		4.62	0.54

CALIB STANDHYD (2082) ID= 1 DT=15.0 min	Area (ha)= 426.60	Dir. Conn. (%)= 37.00
	Total Imp (%)= 64.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	273.02	153.58
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1686.42	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	116.82	133.62
over (min)	15.00	30.00
Storage Coeff. (min)=	16.10 (ii)	25.63 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.07	0.04

TOTALS
53.229 (iii)
6.00
49.61
88.50
0.71

PEAK FLOW (cms)=	36.17	26.02
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	86.50	49.61
TOTAL RAINFALL (mm)=	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.56

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2081) ID= 1 DT=15.0 min	Area (ha)= 209.70	Dir. Conn. (%)= 20.00
	Total Imp (%)= 35.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	73.39	136.31
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1182.37	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	116.82	78.16
over (min)	15.00	30.00
Storage Coeff. (min)=	13.01 (ii)	24.82 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04

TOTALS
19.213 (iii)
6.00
41.24
88.50
0.57

PEAK FLOW (cms)=	10.50	13.60
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	86.50	41.24
TOTAL RAINFALL (mm)=	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.47

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2084) ID= 1 DT=15.0 min	Area (ha)= 56.00	Dir. Conn. (%)= 70.00
	Total Imp (%)= 75.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	42.00	14.00
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50

Length (m)=	611.01	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	116.82	101.50
over (min)	15.00	30.00
Storage Coeff. (min)=	8.76 (ii)	19.39 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.05

TOTALS
12.522 (iii)
6.00
54.38
88.50
0.87

PEAK FLOW (cms)=	11.15	2.05
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	86.50	54.38
TOTAL RAINFALL (mm)=	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.61

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 82.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9289) IN= 2----> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	2.8000	1.5745
	0.1900	0.9984	3.7500	1.7809
	1.0900	1.1991	4.7100	1.9291
	2.0200	1.3813	5.4800	2.0745

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2084)	56.000	12.522	6.00	76.86
OUTFLOW: ID= 1 (9289)	56.000	5.173	6.25	76.84

PEAK FLOW REDUCTION [Qout/Qin] (%) = 41.32
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha. m.) = 2.0305

ADD HYD (9262) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2081):	209.70	19.213	6.00	50.29
+ ID2= 2 (9289):	56.00	5.173	6.25	76.84
=====				
ID = 3 (9262):	265.70	23.356	6.25	55.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5208) IN= 2----> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	16.8530	8.3371
	0.8360	4.7353	21.1630	9.3975
	4.7810	5.1070	25.1140	10.4406
	8.9050	6.2484	25.5140	10.4506
	12.6070	7.1796	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9262)	265.700	23.356	6.25	55.89
OUTFLOW: ID= 1 (5208)	265.700	10.208	7.00	55.88

PEAK FLOW REDUCTION [Qout/Qin] (%) = 43.71
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 6.6347

ADD HYD (8148) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2082):	426.60	53.229	6.00	63.26

+ ID2= 2 (5208): 265.70 10.208 7.00 55.88
 ID = 3 (8148): 692.30 53.690 6.00 60.43

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8146) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6208):	14723.27	148.292	9.25	35.05
+ ID2= 2 (8148):	692.30	53.690	6.00	60.43
ID = 3 (8146):	15415.57	153.994	9.25	36.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6210)
 IN= 2--> OUT= 1 Routing time step (min)= 15.00

DATA FOR SECTION (2101.0) ----->				
Distance	Elevation	Manning		
0.00	241.81	0.0800		
28.43	241.78	0.0800		
85.30	237.04	0.0800		
149.27	231.32	0.0800		
170.60	230.10	0.0800		
255.89	229.37	0.0800		
277.22	228.68	0.0800		
284.33	228.42	0.0800		
291.43	228.16	0.0800		
297.76	227.00	0.0800 / 0.0350	Main Channel	
298.76	225.00	0.0350	Main Channel	
312.76	225.00	0.0350	Main Channel	
313.76	227.00	0.0350 / 0.0800	Main Channel	
326.97	229.50	0.0800		
348.30	230.29	0.0800		
376.73	229.76	0.0800		
490.46	235.75	0.0800		
568.65	236.24	0.0800		
661.06	241.85	0.0800		
703.71	246.04	0.0800		

TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.67	225.67	.141E+05	6.6	0.69	35.86
1.33	226.33	.289E+05	20.4	1.04	23.65
2.00	227.00	.443E+05	39.3	1.31	18.81
2.93	227.93	.730E+05	77.8	1.57	15.64
3.85	228.85	.123E+06	131.1	1.57	15.65
4.78	229.78	.228E+06	205.6	1.33	18.51
5.70	230.70	.503E+06	356.7	1.05	23.49
6.63	231.63	.846E+06	608.7	1.06	23.17
7.55	232.55	.123E+07	946.4	1.14	21.65
8.48	233.48	.165E+07	1367.0	1.22	20.13
9.41	234.41	.211E+07	1871.8	1.31	18.80
10.33	235.33	.261E+07	2463.2	1.40	17.65
11.26	236.26	.317E+07	2999.4	1.40	17.63
12.18	237.18	.384E+07	3848.3	1.48	16.63
13.11	238.11	.454E+07	4806.0	1.56	15.75
14.03	239.03	.528E+07	5876.9	1.64	14.97
14.96	239.96	.605E+07	7062.7	1.72	14.29
15.88	240.88	.687E+07	8365.0	1.80	13.68
16.81	241.81	.771E+07	9506.9	1.82	13.52

<---- hydrograph ---->				<- pipe / channel ->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8146)	*****	153.99	9.25	36.19	4.14
OUTFLOW: ID= 1 (6210)	*****	152.63	9.50	36.19	4.12

CALIB
 STANDHYD (2104)
 ID= 1 DT=15.0 min
 Area (ha)= 192.70
 Total Imp(%)= 73.00 Di r. Conn.(%)= 68.00
 IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 140.67 52.03
 Dep. Storage (mm)= 1.00 5.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 1133.43 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 116.82 97.34
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 10.30 (ii) 17.44 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.05
 PEAK FLOW (cms)= 35.58 7.61 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.25 40.680 (iii)
 RUNOFF VOLUME (mm)= 87.50 52.79 6.00
 TOTAL RAINFALL (mm)= 88.50 88.50 76.39
 RUNOFF COEFFICIENT = 0.99 0.60 88.50
 0.86

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 81.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9283)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.5000	8.0000
0.3000	2.0000	3.5000	10.0000
0.9000	5.0000	4.5000	13.0000
2.0000	7.5000	5.5000	14.0000

AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 INFLOW : ID= 2 (2104) 192.700 40.680 6.00 76.39
 OUTFLOW: ID= 1 (9283) 192.700 3.553 7.25 76.38

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 8.73
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha. m.) = 10.1659

CALIB
 STANDHYD (2103)
 ID= 1 DT=15.0 min

Area (ha)= 11.10
 Total Imp(%)= 30.00 Di r. Conn.(%)= 10.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 3.33 7.77
 Dep. Storage (mm)= 1.00 5.00
 Average Slope (%)= 1.00 2.00
 Length (m)= 272.03 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 116.82 116.77
 over (min)= 15.00 15.00
 Storage Coeff. (min)= 4.38 (ii) 11.01 (ii)
 Unit Hyd. Tpeak (min)= 15.00 15.00
 Unit Hyd. peak (cms)= 0.11 0.08
 PEAK FLOW (cms)= 0.35 2.02 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.00 2.376 (iii)
 RUNOFF VOLUME (mm)= 87.50 58.57 6.00
 TOTAL RAINFALL (mm)= 88.50 88.50 61.46
 RUNOFF COEFFICIENT = 0.99 0.66 88.50
 0.69

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 84.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9284)

IN= 2----> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.5600	0.2067
0.0400	0.1045	0.7400	0.2572
0.2200	0.1166	0.9300	0.3046
0.4000	0.1670	1.0900	0.3510

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2103)	11.100	2.376	6.00	61.46
OUTFLOW: ID= 1 (9284)	11.100	0.872	6.25	61.40

PEAK FLOW REDUCTION [Qout/Qin] (%) = 36.70
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha.m.) = 0.2908

ADD HYD (9258)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9283)	192.70	3.553	7.25	76.38
+ ID2= 2 (9284)	11.10	0.872	6.25	61.40
ID = 3 (9258)	203.80	4.160	6.75	75.57

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5210)
IN= 2----> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0060	0.0024
0.0020	0.0013	0.0080	0.0027
0.0030	0.0017	0.0090	0.0031
0.0050	0.0020	0.4090	0.0131

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9258)	203.800	4.160	6.75	75.57
OUTFLOW: ID= 1 (5210)	203.800	4.139	7.00	75.57

PEAK FLOW REDUCTION [Qout/Qin] (%) = 99.49
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha.m.) = 0.1076

CALIB NASHYD (2102)
ID= 1 DT=15.0 min

Area (ha) = 14.38
Curve Number (CN) = 80.0
U.H. Tp (hrs) = 1.08
of Linear Res. (N) = 1.50

Unit Hyd Opeak (cms) = 0.228
PEAK FLOW (cms) = 0.307 (i)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 46.997
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.531

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9210)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2102)	14.38	0.307	7.25	47.00
+ ID2= 2 (5210)	203.80	4.139	7.00	75.57
ID = 3 (9210)	218.18	4.443	7.00	73.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8150)

1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6210)	15415.57	152.629	9.50	36.19
+ ID2= 2 (9210)	218.18	4.443	7.00	73.68
ID = 3 (8150)	15633.75	156.060	9.50	36.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2121)
ID= 1 DT=15.0 min

Area (ha) = 56.89
Total Imp (%) = 91.00
Dir. Conn. (%) = 74.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 51.77 5.12
Dep. Storage (mm) = 2.00 5.00
Average Slope (%) = 0.50 0.50
Length (m) = 615.85 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 116.82 262.96
over (min) = 15.00 30.00
Storage Coeff. (min) = 8.80 (ii) 16.06 (ii)
Unit Hyd. Tpeak (min) = 15.00 30.00
Unit Hyd. peak (cms) = 0.09 0.05

PEAK FLOW (cms) = 11.96 2.10 *TOTALS*
TIME TO PEAK (hrs) = 6.00 6.25 13.407 (iii)
RUNOFF VOLUME (mm) = 86.50 59.71 6.00
TOTAL RAINFALL (mm) = 88.50 88.50 79.53
RUNOFF COEFFICIENT = 0.98 0.67 88.50 0.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2124)
ID= 1 DT=15.0 min

Area (ha) = 4.40
Total Imp (%) = 75.00
Dir. Conn. (%) = 70.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 3.30 1.10
Dep. Storage (mm) = 2.00 5.00
Average Slope (%) = 0.50 0.50
Length (m) = 171.27 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 116.82 73.13
over (min) = 15.00 30.00
Storage Coeff. (min) = 4.08 (ii) 16.21 (ii)
Unit Hyd. Tpeak (min) = 15.00 30.00
Unit Hyd. peak (cms) = 0.11 0.05

PEAK FLOW (cms) = 0.98 0.12 *TOTALS*
TIME TO PEAK (hrs) = 6.00 6.25 1.063 (iii)
RUNOFF VOLUME (mm) = 86.50 39.64 6.00
TOTAL RAINFALL (mm) = 88.50 88.50 72.44
RUNOFF COEFFICIENT = 0.98 0.45 88.50 0.82

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9290)
IN= 2----> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.2200	0.1226
0.0100	0.0778	0.2900	0.1387
0.0800	0.0934	0.3700	0.1503

0.1600 0.1076 | 0.4300 0.1616

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2124)	4.400	1.063	6.00	72.44
OUTFLOW: ID= 1 (9290)	4.400	0.421	6.25	71.99

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 39.60
 TIME SHIFT OF PEAK FLOW (mi n) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 0.1662

ADD HYD (9264)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2121):	56.89	13.407	6.00	79.53
+ ID2= 2 (9290):	4.40	0.421	6.25	71.99

ID = 3 (9264):	61.29	13.658	6.00	78.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5212)
 IN= 2--> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.1000	2.1402
0.2080	1.2793	5.1480	2.2661
1.1880	1.5546	5.9930	2.3900
2.2120	1.7255	6.3930	2.4000
3.0680	1.9336	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9264)	61.290	13.658	6.00	78.99
OUTFLOW: ID= 1 (5212)	61.290	5.369	6.25	78.97

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 39.31
 TIME SHIFT OF PEAK FLOW (mi n) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 2.3367

CALIB
 STANDHYD (2122)
 ID= 1 DT=15.0 mi n

Area (ha) = 267.10
 Total Imp(%) = 29.00
 Dir. Conn. (%) = 24.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	77.46	189.64
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1334.42	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 116.82
 over (mi n) = 15.00
 Storage Coeff. (mi n) = 13.99 (ii)
 Uni t Hyd. Tpeak (mi n) = 15.00
 Uni t Hyd. peak (cms) = 0.07

PEAK FLOW (cms) = 15.60
 TIME TO PEAK (hrs) = 6.00
 RUNOFF VOLUME (mm) = 86.50
 TOTAL RAINFALL (mm) = 88.50
 RUNOFF COEFFICIENT = 0.98

TOTALS
 (i i i) 25.389

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9312)
 IN= 2--> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.1000	2.1402
0.2080	1.2793	5.1480	2.2661
1.1880	1.5546	5.9930	2.3900
2.2120	1.7255	6.3930	2.4000
3.0680	1.9336	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
0.0000	0.0000	13.3600	4.9180	
0.9100	2.4658	17.8700	6.1368	
5.1800	2.7404	22.4400	7.2882	
9.6400	3.9605	26.1200	8.4139	

INFLOW : ID= 2 (2122) 267.100
 OUTFLOW: ID= 1 (9312) 267.100

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 52.56
 TIME SHIFT OF PEAK FLOW (mi n) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 5.0139

ADD HYD (8152)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (5212):	61.29	5.369	6.25	78.97
+ ID2= 2 (9312):	267.10	13.343	6.50	51.16

ID = 3 (8152):	328.39	18.522	6.50	56.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6214)
 IN= 2--> OUT= 1

Routing time step (mi n)' = 15.00

<----- DATA FOR SECTION (2141.0) ----->

Di stance	Elevation	Manning
0.00	259.29	0.0500
15.33	258.91	0.0500
38.33	258.57	0.0500
65.16	257.93	0.0500
95.82	253.44	0.0500
111.15	251.21	0.0500
130.32	249.67	0.0500
149.48	248.51	0.0500
150.66	248.50	0.0500 / 0.0370
150.81	247.50	0.0370
154.66	247.50	0.0370
155.66	248.50	0.0370 / 0.0500
160.98	248.50	0.0500
187.81	249.78	0.0500
199.31	252.13	0.0500
245.30	254.04	0.0500
264.46	253.99	0.0500
298.96	255.23	0.0500
329.62	257.02	0.0500
379.45	258.82	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.50	248.00	.648E+04	2.1	1.00	52.30
1.00	248.50	.139E+05	6.4	1.44	36.35
1.61	249.11	.570E+05	22.6	1.24	42.01
2.21	249.71	.143E+06	63.6	1.39	37.61
2.82	250.32	.263E+06	142.9	1.70	30.69
3.43	250.93	.403E+06	257.2	2.00	26.11
4.04	251.54	.562E+06	410.4	2.29	22.82
4.64	252.14	.735E+06	601.3	2.56	20.37
5.25	252.75	.933E+06	791.0	2.66	19.67
5.86	253.36	.117E+07	1038.0	2.79	18.74
6.46	253.96	.144E+07	1344.4	2.93	17.82
7.07	254.57	.178E+07	1664.1	2.93	17.83
7.68	255.18	.217E+07	2140.2	3.10	16.87
8.28	255.78	.259E+07	2722.9	3.30	15.83
8.89	256.39	.304E+07	3387.1	3.50	14.94
9.50	257.00	.351E+07	4132.7	3.69	14.16
10.11	257.61	.402E+07	4913.0	3.83	13.64
10.71	258.21	.458E+07	5648.3	3.87	13.50
11.32	258.82	.521E+07	6343.1	3.82	13.68

<---- hydrograph ----> <-pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8152)	328.39	18.52	6.50	56.35	1.45	1.29
OUTFLOW: ID= 1 (6214)	328.39	13.64	7.25	56.35	1.27	1.34

CALIB
STANDHYD (2144)
ID= 1 DT=15.0 min

Area Total	(ha)= 93.10	Dir. Conn. (%)= 70.00
Imp (%)	= 75.00	

Surface Area	(ha)= 69.82	IMPERVIOUS	PERVIOUS (i)
Dep. Storage	(mm)= 2.00		23.27
Average Slope	(%)= 0.50		5.00
Length	(m)= 787.82		40.00
Mannings n	= 0.013		0.250

Max. Eff. Inten. over	(mm/hr)= 116.82	(min)= 15.00	75.06	30.00
Storage Coeff. (min)	= 10.20	(ii)	22.20	(ii)
Unit Hyd. Tpeak	(min)= 15.00		30.00	
Unit Hyd. peak	(cms)= 0.09		0.04	

TOTALS
19.261 (iii)
6.00
72.74
88.50
0.82

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9285)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.6500	2.6150
0.3200	1.6583	6.2300	2.9579
1.8100	1.9916	7.8200	3.2041
3.3600	2.2943	9.1000	3.4456

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
93.100	19.261	6.00	72.74
93.100	7.687	6.25	72.72

INFLOW : ID= 2 (2144)
OUTFLOW: ID= 1 (9285)

PEAK FLOW REDUCTION [Qout/Qin] (%) = 39.91
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha. m.) = 3.2268

CALIB
STANDHYD (2143)
ID= 1 DT=15.0 min

Area Total	(ha)= 158.30	Dir. Conn. (%)= 9.00
Imp (%)	= 26.00	

Surface Area	(ha)= 41.16	IMPERVIOUS	PERVIOUS (i)
Dep. Storage	(mm)= 2.00		117.14
Average Slope	(%)= 0.50		5.00
Length	(m)= 1027.29		40.00
Mannings n	= 0.013		0.250

Max. Eff. Inten. over	(mm/hr)= 116.82	(min)= 15.00	82.13	30.00
Storage Coeff. (min)	= 11.96	(ii)	23.53	(ii)
Unit Hyd. Tpeak	(min)= 15.00		30.00	
Unit Hyd. peak	(cms)= 0.08		0.04	

TOTALS
14.121 (iii)
6.25
47.16
88.50
0.53

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

- CN* = 72.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9286)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	7.9200	2.8224
0.5400	1.3792	10.5900	3.5534
3.0700	1.5175	13.3000	4.2519
5.7200	2.2534	15.4900	4.9349

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
158.300	14.121	6.25	47.16
158.300	7.605	6.75	47.16

INFLOW : ID= 2 (2143)
OUTFLOW: ID= 1 (9286)

PEAK FLOW REDUCTION [Qout/Qin] (%) = 53.85
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 2.7830

ADD HYD. (9260)
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
93.10	7.687	6.25	72.72
158.30	7.605	6.75	47.16
251.40	14.740	6.50	56.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5214)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.5370	0.9877
0.1280	0.4635	3.1860	1.1293
0.5900	0.5362	3.7090	1.2678
1.3690	0.6921	3.9980	1.2778
1.8980	0.8262	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
251.400	14.740	6.50	56.63
251.400	14.251	6.75	56.62

INFLOW : ID= 2 (9260)
OUTFLOW: ID= 1 (5214)

PEAK FLOW REDUCTION [Qout/Qin] (%) = 96.69
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha. m.) = 1.6630

CALIB
STANDHYD (2142)
ID= 1 DT=15.0 min

Area Total	(ha)= 65.54	Dir. Conn. (%)= 10.00
Imp (%)	= 27.00	

Surface Area	(ha)= 17.70	IMPERVIOUS	PERVIOUS (i)
Dep. Storage	(mm)= 2.00		47.84
Average Slope	(%)= 0.50		5.00
Length	(m)= 661.01		40.00
Mannings n	= 0.013		0.250

Max. Eff. Inten. over	(mm/hr)= 116.82	(min)= 15.00	46.70	30.00
Storage Coeff. (min)	= 9.18	(ii)	23.68	(ii)
Unit Hyd. Tpeak	(min)= 15.00		30.00	
Unit Hyd. peak	(cms)= 0.09		0.04	

TOTALS
14.121 (iii)
6.25
47.16
88.50
0.35

PEAK FLOW (cms) = 1.84
TIME TO PEAK (hrs) = 6.00
RUNOFF VOLUME (mm) = 86.50
TOTAL RAINFALL (mm) = 88.50
RUNOFF COEFFICIENT = 0.98

2.89
6.25
25.25
88.50
0.29

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 51.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD. (9214) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2142):	65.54	3.637	6.00	31.38
+ ID2= 2 (5214):	251.40	14.251	6.75	56.62

ID = 3 (9214):	316.94	15.918	6.75	51.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD. (8154) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6214):	328.39	13.643	7.25	56.35
+ ID2= 2 (9214):	316.94	15.918	6.75	51.40

ID = 3 (8154):	645.33	28.045	6.75	53.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2163) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	69.70 27.00	Di r. Conn. (%)=	9.00

Surface Area	(ha)=	18.82	50.88		
Dep. Storage	(mm)=	2.00	5.00		
Average Slope	(%)=	0.50	0.50		
Length	(m)=	681.66	40.00		
Mannings n	=	0.013	0.250		
Max. Eff. Inten.	(mm/hr)=	116.82	97.28		
over	(min)	15.00	30.00		
Storage Coeff.	(min)=	9.35 (ii)	20.17 (ii)		
Unit Hyd. Tpeak	(min)=	15.00	30.00		
Unit Hyd. peak	(cms)=	0.09	0.05		

PEAK FLOW	(cms)=	1.75	7.00	*TOTALS*	
TIME TO PEAK	(hrs)=	6.00	6.25	7.575 (iii)	
RUNOFF VOLUME	(mm)=	86.50	50.28	53.54	
TOTAL RAINFALL	(mm)=	88.50	88.50	88.50	
RUNOFF COEFFICIENT	=	0.98	0.57	0.61	

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 78.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9271) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)	
	0.0000	0.0000	3.4900	1.2605	
	0.2400	0.6230	4.6600	1.5808	
	1.3500	0.6885	5.8600	1.8854	
	2.5200	1.0102	6.8200	2.1832	
		AREA	OPEAK	TPEAK	R. V.
		(ha)	(cms)	(hrs)	(mm)

INFLOW : ID= 2 (2163) 69.700 7.575 6.25 53.54
 OUTFLOW: ID= 1 (9271) 69.700 3.913 6.75 53.53

PEAK FLOW REDUCTION [Qout/Qin] (%) = 51.65
 TIME SHIF T OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 1.4094

CALIB STANDHYD (2169) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	3.50 30.00	Di r. Conn. (%)=	10.00

Surface Area	(ha)=	1.05	2.45		
Dep. Storage	(mm)=	2.00	5.00		
Average Slope	(%)=	0.50	0.50		
Length	(m)=	152.75	40.00		
Mannings n	=	0.013	0.250		
Max. Eff. Inten.	(mm/hr)=	116.82	81.67		
over	(min)	15.00	30.00		
Storage Coeff.	(min)=	3.81 (ii)	15.41 (ii)		
Unit Hyd. Tpeak	(min)=	15.00	30.00		
Unit Hyd. peak	(cms)=	0.11	0.05		

PEAK FLOW	(cms)=	0.11	0.31	*TOTALS*	
TIME TO PEAK	(hrs)=	6.00	6.25	0.330 (iii)	
RUNOFF VOLUME	(mm)=	86.50	41.29	6.25	
TOTAL RAINFALL	(mm)=	88.50	88.50	45.81	
RUNOFF COEFFICIENT	=	0.98	0.47	88.50	
				0.52	

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9272) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)	
	0.0000	0.0000	0.1800	0.0659	
	0.0100	0.0333	0.2400	0.0820	
	0.0700	0.0372	0.3000	0.0971	
	0.1300	0.0532	0.3500	0.1119	
		AREA	OPEAK	TPEAK	R. V.
		(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (2169)		3.500	0.330	6.25	45.81
OUTFLOW: ID= 1 (9272)		3.500	0.176	6.50	45.57

PEAK FLOW REDUCTION [Qout/Qin] (%) =	53.44				
TIME SHIF T OF PEAK FLOW (min) =	15.00				
MAXIMUM STORAGE USED (ha. m.) =	0.0656				

ADD HYD. (9251) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9271):	69.70	3.913	6.75	53.53
+ ID2= 2 (9272):	3.50	0.176	6.50	45.57

ID = 3 (9251):	73.20	4.083	6.75	53.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5216) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	3.7400	2.4559
	0.1890	1.6831	4.6960	2.6646
	1.0830	1.7903	5.4670	2.8688

2.0170 2.0202 | 5.8670 2.8788
 2.7980 2.2179 | 0.0000 0.0000

AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW: ID= 2 (9251) 73.200 4.083 6.75 53.12
 OUTFLOW: ID= 1 (5216) 73.200 1.680 8.25 53.12

PEAK FLOW REDUCTION [Qout/Qin] (%) = 41.16
 TIME SHIFT OF PEAK FLOW (min) = 90.00
 MAXIMUM STORAGE USED (ha.m.) = 1.9425

CALIB STANDHYD (2162)
 ID= 1 DT=15.0 min
 Area (ha) = 72.57
 Total Imp(%) = 20.00
 Dir. Conn. (%) = 7.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 14.51 58.06
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 695.56 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 116.82 69.65
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 9.46 (ii) 21.83 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.09 0.05

TOTALS
 PEAK FLOW (cms) = 1.41 5.48 5.944 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.25
 RUNOFF VOLUME (mm) = 86.50 38.97 42.30
 TOTAL RAINFALL (mm) = 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.44 0.48

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9316)
 IN= 2--> OUT= 1
 DT= 15.0 min
 OUTFLOW STORAGE OUTFLOW STORAGE
 (cms) (ha.m.) (cms) (ha.m.)
 0.0000 0.0000 4.0000 0.3000
 2.0000 0.1000 4.5000 0.4000
 3.0000 0.2000 5.0000 0.5000

AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW: ID= 2 (2162) 72.570 5.944 6.25 42.30
 OUTFLOW: ID= 1 (9316) 72.570 4.494 6.50 42.30

PEAK FLOW REDUCTION [Qout/Qin] (%) = 75.61
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha.m.) = 0.4233

ADD HYD (9216)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (5216): 73.20 1.680 8.25 53.12
 + ID2= 2 (9316): 72.57 4.494 6.50 42.30
 ID= 3 (9216): 145.77 4.562 6.50 47.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8156)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.

(ha) (cms) (hrs) (mm)
 ID1= 1 (8154): 645.33 28.045 6.75 53.92
 + ID2= 2 (9216): 145.77 4.562 6.50 47.74
 ID= 3 (8156): 791.10 32.221 6.75 52.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6218)
 IN= 2--> OUT= 1
 Routing time step (min) = 15.00

DATA FOR SECTION (2181.0)
 Distance Elevation Manning
 0.00 256.38 0.0450
 7.70 254.85 0.0450
 15.40 253.20 0.0450
 19.25 252.29 0.0450
 23.10 251.17 0.0450
 27.94 236.16 0.0450
 30.87 235.80 0.0450
 32.79 235.45 0.0450
 34.57 234.60 0.0450 / 0.0300 Main Channel
 35.07 233.60 0.0300 Main Channel
 35.57 233.60 0.0300 Main Channel
 36.07 233.60 0.0300 Main Channel
 36.57 234.60 0.0300 / 0.0450 Main Channel
 37.07 235.13 0.0450
 37.57 235.20 0.0450
 38.07 235.32 0.0450
 38.57 239.95 0.0450
 39.07 244.80 0.0450
 39.57 245.00 0.0450
 40.07 256.00 0.0450

TRAVEL TIME TABLE
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV. TIME
 (m) (m) (cu. m.) (cms) (m/s) (min)
 1.00 234.60 .181E+05 6.0 0.66 50.64
 2.13 235.73 .518E+05 23.8 0.93 36.27
 3.25 236.85 .119E+06 60.0 1.02 33.03
 4.38 237.98 .217E+06 120.6 1.12 29.99
 5.51 239.11 .346E+06 210.5 1.23 27.39
 6.63 240.23 .506E+06 334.1 1.33 25.22
 7.76 241.36 .697E+06 495.6 1.43 23.44
 8.88 242.48 .920E+06 699.8 1.53 21.91
 10.01 243.61 .117E+07 950.8 1.63 20.59
 11.14 244.74 .146E+07 1252.5 1.73 19.45
 12.26 245.86 .178E+07 1619.6 1.83 18.37
 13.39 246.99 .212E+07 2078.5 1.97 17.01
 14.52 248.12 .247E+07 2589.0 2.11 15.88
 15.64 249.24 .282E+07 3149.7 2.25 14.94
 16.77 250.37 .319E+07 3759.8 2.37 14.14
 17.89 251.49 .356E+07 4419.5 2.50 13.44
 19.02 252.62 .395E+07 5126.4 2.61 12.84
 20.15 253.75 .435E+07 5871.6 2.72 12.34
 21.27 254.87 .476E+07 6663.2 2.82 11.89

hydrograph
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW: ID= 2 (8156) 791.10 32.22 6.75 52.78
 OUTFLOW: ID= 1 (6218) 791.10 24.63 7.25 52.78
 MAX DEPTH MAX VEL
 (m) (m/s)
 2.39 0.95
 2.16 0.93

CALIB STANDHYD (2183)
 ID= 1 DT=15.0 min
 Area (ha) = 69.30
 Total Imp(%) = 22.00
 Dir. Conn. (%) = 7.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 15.25 54.05
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 679.71 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 116.82 95.79
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 9.33 (ii) 20.22 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.09 0.05

TOTALS
 PEAK FLOW (cms)= 1.36 7.32 7.766 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.25
 RUNOFF VOLUME (mm)= 86.50 51.67 54.11
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.58 0.61

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 80.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9282)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.5000	3.5000
0.8000	0.1000	6.0000	4.0000
1.1000	2.0000	6.5000	4.5000
2.5000	3.0000	7.0000	5.5000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2183)	69.300	7.766	6.25	54.11
OUTFLOW: ID= 1 (9282)	69.300	1.093	7.50	54.11

PEAK FLOW REDUCTION [Qout/Oin] (%) = 14.07
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha. m.) = 1.9583

RESERVOIR (5218)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.5090	1.7553
0.2280	0.8236	5.6620	2.0069
1.3060	0.9528	6.5910	2.2530
2.4320	1.2300	6.9910	2.2630
3.3740	1.4683	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9282)	69.300	1.093	7.50	54.11
OUTFLOW: ID= 1 (5218)	69.300	1.064	9.25	54.09

PEAK FLOW REDUCTION [Qout/Oin] (%) = 97.36
 TIME SHIFT OF PEAK FLOW (min) = 105.00
 MAXIMUM STORAGE USED (ha. m.) = 0.9239

CALIB NASHYD (2182)
 ID= 1 DT=15.0 min

Area (ha) = 82.95 Curve Number (CN) = 67.7
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp (hrs) = 0.79

Unit Hyd Qpeak (cms) = 1.786
 PEAK FLOW (cms) = 1.586 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 33.573
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.379

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9218)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2182):	82.95	1.586	7.00	33.57
+ ID2= 2 (5218):	69.30	1.064	9.25	54.09

ID = 3 (9218): 152.25 2.262 8.25 42.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8158)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6218):	791.10	24.631	7.25	52.78
+ ID2= 2 (9218):	152.25	2.262	8.25	42.91
ID = 3 (8158):	943.35	26.380	7.25	51.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8151)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8150):	15633.75	156.060	9.50	36.72
+ ID2= 2 (8158):	943.35	26.380	7.25	51.19
ID = 3 (8151):	16577.10	170.280	9.25	37.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5504)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	33.0100	0.6700
2.6400	0.0500	37.4800	0.7800
4.8300	0.0900	47.5400	1.0100
7.4100	0.1300	60.2300	1.2800
10.3300	0.1900	71.9530	1.5258
13.5300	0.2500	*****	3.0529
16.9900	0.3200	*****	5.3299
20.6800	0.3900	*****	8.4148
24.6800	0.4800	*****	11.7637
28.7100	0.5700	*****	11.7737

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (8151)	*****	170.280	9.25	37.54
OUTFLOW: ID= 1 (5504)	*****	170.284	9.25	37.54

PEAK FLOW REDUCTION [Qout/Oin] (%) = 100.00
 TIME SHIFT OF PEAK FLOW (min) = 0.00
 MAXIMUM STORAGE USED (ha. m.) = 3.9445

**** WARNING : HYDROGRAPH PEAK WAS NOT REDUCED.
 CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.

ROUTE CHN (6220)
 IN= 2--> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (2201,0) ----->

Distance	Elevation	Manning	Main Channel
0.00	245.59	0.0600	
5.76	245.45	0.0600	
23.03	242.27	0.0600	
57.58	232.35	0.0600	
86.37	226.18	0.0600	
109.41	224.31	0.0600	
115.17	223.86	0.0600	
120.92	222.78	0.0600	
126.68	221.93	0.0600	
128.06	221.50	0.0600 / 0.0300	Main Channel
129.56	220.00	0.0300	Main Channel
141.56	220.00	0.0300	Main Channel
143.06	221.50	0.0300 / 0.0600	Main Channel
149.72	223.77	0.0600	
155.47	225.09	0.0600	
253.36	229.21	0.0600	
333.98	227.50	0.0600	
454.90	229.25	0.0600	
500.97	233.11	0.0600	

570.07 237.42 0.0600

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.75	220.75	.262E+05	5.5	0.57	79.44
1.50	221.50	.555E+05	17.5	0.86	52.89
2.44	222.44	.103E+06	42.7	1.14	40.03
3.37	223.37	.171E+06	79.4	1.27	35.98
4.31	224.31	.263E+06	128.1	1.33	34.22
5.25	225.25	.393E+06	191.7	1.34	34.19
6.18	226.18	.594E+06	276.7	1.28	35.77
7.12	227.12	.872E+06	399.9	1.26	36.33
8.06	228.06	.127E+07	542.7	1.17	38.92
8.99	228.99	.199E+07	770.0	1.06	43.01
9.93	229.93	.297E+07	1151.8	1.06	42.95
10.86	230.86	.400E+07	1652.9	1.13	40.33
11.80	231.80	.507E+07	2251.8	1.22	37.53
12.74	232.74	.618E+07	2946.5	1.31	34.97
13.67	233.67	.733E+07	3726.8	1.39	32.79
14.61	234.61	.853E+07	4591.0	1.48	30.96
15.55	235.55	.977E+07	5544.1	1.55	29.37
16.48	236.48	.111E+08	6586.7	1.63	27.99
17.42	237.42	.124E+08	7719.2	1.71	26.77

----- hydrograph ----- <- pipe / channel ->

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (5504) *****	170.28	9.25	37.54	4.93	1.34
OUTFLOW: ID= 1 (6220) *****	165.62	9.75	37.54	4.87	1.34

CALIB STANDHYD (2207) ID= 1 DT=15.0 mi n

Area Total	(ha)= 57.80	Imp(%)= 21.00	Di r. Conn.(%)= 8.00
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	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	12.14	45.66
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	620.75	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)= over (mi n)	116.82 / 15.00	97.33 / 30.00
Storage Coeff. (mi n)	8.84 (ii)	19.65 (ii)
Unit Hyd. Tpeak (mi n)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.05

TOTALS
 PEAK FLOW (cms)= 1.31 6.37
 TIME TO PEAK (hrs)= 6.00 6.25
 RUNOFF VOLUME (mm)= 86.50 53.68
 TOTAL RAINFALL (mm)= 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.61

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 82.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9278) IN= 2---> OUT= 1 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.5000	2.5000
0.2000	0.3000	0.6000	3.5000
0.3000	0.9000	0.7000	4.5000
0.4000	2.0000	0.8000	5.5000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (2207)	57.800	6.779	56.31
OUTFLOW: ID= 1 (9278)	57.800	0.463	56.30

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 6.83

TIME SHIFT OF PEAK FLOW (mi n)=165.00
 MAXIMUM STORAGE USED (ha. m.) = 2.3149

CALIB STANDHYD (2204) ID= 1 DT=15.0 mi n

Area Total	(ha)= 13.30	Imp(%)= 75.00	Di r. Conn.(%)= 70.00
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	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	9.98	3.33
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	297.77	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)= over (mi n)	116.82 / 15.00	94.32 / 30.00
Storage Coeff. (mi n)	5.69 (ii)	16.64 (ii)
Unit Hyd. Tpeak (mi n)	15.00	30.00
Unit Hyd. peak (cms)	0.11	0.05

TOTALS
 PEAK FLOW (cms)= 2.88 0.48
 TIME TO PEAK (hrs)= 6.00 6.25
 RUNOFF VOLUME (mm)= 86.50 50.58
 TOTAL RAINFALL (mm)= 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.57

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9279) IN= 2---> OUT= 1 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.6700	0.3747
0.0500	0.2376	0.8900	0.4238
0.2600	0.2854	1.1200	0.4591
0.4800	0.3287	1.3000	0.4937

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (2204)	13.300	3.199	6.00
OUTFLOW: ID= 1 (9279)	13.300	1.270	6.25

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 39.71
 TIME SHIFT OF PEAK FLOW (mi n)= 15.00
 MAXIMUM STORAGE USED (ha. m.) = 0.4971

ADD HYD 1 + 2 = 3 (9255)

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (9278):	57.80	0.463	56.30
+ ID2= 2 (9279):	13.30	1.270	6.25
-----	-----	-----	-----
ID = 3 (9255):	71.10	1.590	6.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5220) IN= 2---> OUT= 1 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.6050	1.0142
0.1320	0.4759	3.2710	1.1595
0.7550	0.5505	3.8080	1.3018
1.4050	0.7107	4.2080	1.3118
1.9490	0.8484	0.0000	0.0000

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (9255)	71.100	1.590	6.25
			59.92

OUTFLOW: ID= 1 (5220) 71.100 0.862 7.50 59.91

PEAK FLOW REDUCTION [Qout/Qin] (%) = 54.21
TIME SHIFT OF PEAK FLOW (min) = 75.00
MAXIMUM STORAGE USED (ha.m.) = 0.5775

CALIB NASHYD (2202) Area (ha) = 98.00 Curve Number (CN) = 74.3
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U.H. Tp(hrs) = 1.50

Unit Hyd Qpeak (cms) = 1.115

PEAK FLOW (cms) = 1.377 (i)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 40.461
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.457

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9220)
1 + 2 = 3
ID1= 1 (2202): 98.00 1.377 8.00 40.46
+ ID2= 2 (5220): 71.10 0.862 7.50 59.91
ID = 3 (9220): 169.10 2.218 7.75 48.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8160)
1 + 2 = 3
ID1= 1 (6220): 16577.10 165.616 9.75 37.54
+ ID2= 2 (9220): 169.10 2.218 7.75 48.64
ID = 3 (8160): 16746.20 167.454 9.75 37.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2221) Area (ha) = 110.64 Di r. Conn. (%) = 20.00
ID= 1 DT=15.0 min Total Imp(%) = 37.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 40.94 69.70
Dep. Storage (mm) = 2.00 5.00
Average Slope (%) = 0.50 0.50
Length (m) = 858.84 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 116.82 93.63
over (min) = 15.00 30.00
Storage Coeff. (min) = 10.74 (ii) 21.72 (ii)
Unit Hyd. Tpeak (min) = 15.00 30.00
Unit Hyd. peak (cms) = 0.09 0.05

PEAK FLOW (cms) = 5.93 8.92 *TOTALS*
TIME TO PEAK (hrs) = 6.00 6.25 11.767 (iii)
RUNOFF VOLUME (mm) = 86.50 47.62 55.39
TOTAL RAINFALL (mm) = 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.54 0.63

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.3 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2224) Area (ha) = 43.20 Di r. Conn. (%) = 66.00
ID= 1 DT=15.0 min Total Imp(%) = 72.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 31.10 12.10
Dep. Storage (mm) = 2.00 5.00
Average Slope (%) = 0.50 0.50
Length (m) = 536.66 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 116.82 95.95
over (min) = 15.00 30.00
Storage Coeff. (min) = 8.10 (ii) 18.98 (ii)
Unit Hyd. Tpeak (min) = 15.00 30.00
Unit Hyd. peak (cms) = 0.10 0.05

PEAK FLOW (cms) = 8.27 1.68 *TOTALS*
TIME TO PEAK (hrs) = 6.00 6.25 9.385 (iii)
RUNOFF VOLUME (mm) = 86.50 50.86 74.38
TOTAL RAINFALL (mm) = 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.57 0.84

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 79.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9288) IN= 2---> OUT= 1 DT= 15.0 min
OUTFLOW (cms) STORAGE (ha.m.) OUTFLOW (cms) STORAGE (ha.m.)
0.0000 0.0000 2.1600 1.1836
0.1500 0.7428 2.8900 1.3455
0.8400 0.8895 3.6300 1.4651
1.5600 1.0342 4.2300 1.5823

AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (2224) 43.200 9.385 6.00 74.38
OUTFLOW: ID= 1 (9288) 43.200 3.794 6.25 74.35

PEAK FLOW REDUCTION [Qout/Qin] (%) = 40.43
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha.m.) = 1.5084

ADD HYD (9261)
1 + 2 = 3
ID1= 1 (2221): 110.64 11.767 6.00 55.39
+ ID2= 2 (9288): 43.20 3.794 6.25 74.35
ID = 3 (9261): 153.84 14.915 6.25 60.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5222) IN= 2---> OUT= 1 DT= 15.0 min
OUTFLOW (cms) STORAGE (ha.m.) OUTFLOW (cms) STORAGE (ha.m.)
0.0000 0.0000 7.2210 4.0213
0.4910 2.1348 9.0080 4.5380
2.1830 2.4433 15.0420 4.9977
3.9370 2.9883 15.4420 5.0077
5.4550 3.4692 0.0000 0.0000

AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (9261) 153.840 14.915 6.25 60.72
OUTFLOW: ID= 1 (5222) 153.840 6.477 7.00 60.71

PEAK FLOW REDUCTION [Qout/Qin] (%) = 43.42

TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 3.8324

CALIB STANDHYD (2222) ID= 1 DT=15.0 min			
Area Total	(ha) = 65.69	Imp(%) = 84.00	Dir. Conn. (%) = 45.00
Surface Area (ha)	55.18	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)	2.00		
Average Slope (%)	0.50		
Length (m)	661.77		
Mannings n	0.013		
Max. Eff. Inten. (mm/hr) over (min)	116.82 / 15.00		351.43 / 30.00
Storage Coeff. (min)	9.18 (ii)		15.66 (ii)
Unit Hyd. Tpeak (min)	15.00		30.00
Unit Hyd. peak (cms)	0.09		0.05
PEAK FLOW (cms)	8.30		5.85
TIME TO PEAK (hrs)	6.00		6.25
RUNOFF VOLUME (mm)	86.50		68.09
TOTAL RAINFALL (mm)	88.50		88.50
RUNOFF COEFFICIENT	0.98		0.77
			TOTALS 12.451 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 75.3 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8164) 1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2222):	65.69	12.451	6.00	76.37
+ ID2= 2 (5222):	153.84	6.477	7.00	60.71
=====				
ID = 3 (8164):	219.53	12.816	6.00	65.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2242) ID= 1 DT=15.0 min			
Area Total	(ha) = 52.75	Imp(%) = 36.00	Dir. Conn. (%) = 27.00
Surface Area (ha)	18.99	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)	2.00		
Average Slope (%)	0.50		
Length (m)	593.01		
Mannings n	0.013		
Max. Eff. Inten. (mm/hr) over (min)	116.82 / 15.00		87.58 / 30.00
Storage Coeff. (min)	8.60 (ii)		19.88 (ii)
Unit Hyd. Tpeak (min)	15.00		30.00
Unit Hyd. peak (cms)	0.09		0.05
PEAK FLOW (cms)	4.07		4.20
TIME TO PEAK (hrs)	6.00		6.25
RUNOFF VOLUME (mm)	86.50		49.37
TOTAL RAINFALL (mm)	88.50		88.50
RUNOFF COEFFICIENT	0.98		0.56
			TOTALS 6.841 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2244) ID= 1 DT=15.0 min			
Area Total	(ha) = 87.70	Imp(%) = 70.00	Dir. Conn. (%) = 64.00
Surface Area (ha)	61.39	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)	2.00		
Average Slope (%)	0.50		
Length (m)	764.63		
Mannings n	0.013		
Max. Eff. Inten. (mm/hr) over (min)	116.82 / 15.00		94.32 / 30.00
Storage Coeff. (min)	10.02 (ii)		20.97 (ii)
Unit Hyd. Tpeak (min)	15.00		30.00
Unit Hyd. peak (cms)	0.09		0.05
PEAK FLOW (cms)	15.37		3.45
TIME TO PEAK (hrs)	6.00		6.25
RUNOFF VOLUME (mm)	86.50		50.58
TOTAL RAINFALL (mm)	88.50		88.50
RUNOFF COEFFICIENT	0.98		0.57
			TOTALS 17.651 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9287) IN= 2---> OUT= 1 DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	4.3900	2.3776
	0.3000	1.4858	5.8700	2.7087
	1.7000	1.7770	7.3700	2.9557
	3.1700	2.0741	8.5800	3.1980
		AREA (ha)	OPEAK (cms)	TPEAK (hrs)
INFLOW: ID= 2 (2244)		87.700	17.651	6.00
OUTFLOW: ID= 1 (9287)		87.700	7.134	6.25

PEAK FLOW REDUCTION [Out/Oi n] (%) = 40.42
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 2.9926

RESERVOIR (5224) IN= 2---> OUT= 1 DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	0.4470	0.1810
	0.0230	0.0884	0.5620	0.2047
	0.1300	0.1032	0.6540	0.2278
	0.2410	0.1297	1.0540	0.2378
	0.3350	0.1532	0.0000	0.0000
		AREA (ha)	OPEAK (cms)	TPEAK (hrs)
INFLOW: ID= 2 (9287)		87.700	7.134	6.25
OUTFLOW: ID= 1 (5224)		87.700	7.245	6.50

PEAK FLOW REDUCTION [Out/Oi n] (%) = 101.56
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 0.3984

**** WARNING : HYDROGRAPH PEAK WAS NOT REDUCED.
 CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.

ADD HYD (9224) 1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2242):	52.75	6.841	6.00	59.40
+ ID2= 2 (5224):	87.70	7.245	6.50	73.55
=====				
ID = 3 (9224):	140.45	12.139	6.25	68.23

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8166)	AREA	OPEAK	TPEAK	R. V.
1 + 2 =	3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8164):		219.53	12.816	6.00	65.40
+ ID2= 2 (9224):		140.45	12.139	6.25	68.23
ID = 3 (8166):		359.98	23.745	6.25	66.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6226)
IN= 2---> OUT= 1
Routing time step (mi n)' = 15.00

<----- DATA FOR SECTION (2261.0) ----->

Distance	Elevation	Manning	
0.00	245.35	0.0600	
8.27	242.72	0.0600	
16.54	240.01	0.0600	
31.02	235.31	0.0600	
41.36	233.14	0.0600	
90.99	227.73	0.0600	
93.06	227.50	0.0600	
95.13	227.27	0.0600	
95.83	227.00	0.0600	
100.83	226.50	0.0600 / 0.0350	Main Channel
101.33	225.50	0.0350	Main Channel
101.83	226.50	0.0350 / 0.0600	Main Channel
105.83	227.00	0.0600	
107.54	227.39	0.0600	
109.60	227.62	0.0600	
111.67	227.86	0.0600	
146.83	231.76	0.0600	
173.71	234.77	0.0600	
186.12	238.23	0.0600	
202.67	238.35	0.0600	

DEPTH	ELEV	VOLUME	TRAVEL TIME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(hrs)	(cms)	(m/s)	(mi n)
0.50	226.00	.403E+03	0.0	0.0	0.16	330.59
1.00	226.50	.161E+04	0.1	0.26	0.26	208.26
1.70	227.20	.173E+05	1.5	0.27	0.27	195.72
2.39	227.89	.541E+05	5.9	0.35	0.35	152.27
3.09	228.59	.119E+06	16.0	0.43	0.43	124.08
3.79	229.29	.212E+06	33.8	0.51	0.51	104.65
4.49	229.99	.334E+06	61.2	0.59	0.59	90.97
5.18	230.68	.484E+06	99.7	0.66	0.66	80.88
5.88	231.38	.663E+06	151.0	0.73	0.73	73.13
6.58	232.08	.870E+06	216.5	0.80	0.80	66.95
7.27	232.77	.111E+07	297.5	0.87	0.87	61.91
7.97	233.47	.137E+07	398.1	0.94	0.94	57.29
8.67	234.17	.165E+07	518.9	1.01	1.01	53.12
9.36	234.86	.196E+07	658.9	1.08	1.08	49.59
10.06	235.56	.228E+07	827.0	1.17	1.17	46.02
10.76	236.26	.262E+07	1015.4	1.25	1.25	42.96
11.46	236.96	.296E+07	1221.2	1.33	1.33	40.41
12.15	237.65	.332E+07	1444.6	1.40	1.40	38.26
12.85	238.35	.368E+07	1598.2	1.40	1.40	38.42

	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8166)	359.98	23.74	6.25	66.50	3.40	0.46
OUTFLOW: ID= 1 (6226)	359.98	9.43	7.00	66.50	2.64	0.38

CALIB NASHYD (2267)
ID= 1 DT=15.0 mi n
Area (ha)= 10.50
Ia (mm)= 5.00
U. H. Tp(hrs)= 0.47
Curve Number (CN)= 82.0
of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 0.383
PEAK FLOW (cms)= 0.460 (i)

TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 48.502
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.548

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9276)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2---> OUT= 1	(cms)	(ha. m.)	(cms)	(ha. m.)
DT= 15.0 mi n	0.0000	0.0000	0.5300	0.1297
	0.0400	0.0402	0.7100	0.1837
	0.2000	0.0341	0.8900	0.2402
	0.3800	0.0910	1.0300	0.2954
INFLOW : ID= 2 (2267)	10.500	0.460	6.50	48.50
OUTFLOW: ID= 1 (9276)	10.500	0.373	7.25	48.48

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 81.10
TIME SHIF T OF PEAK FLOW (mi n) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.0895

**** ERROR : CHECK THE STORAGE-DISCHARGE TABLE.

CALIB STANDHYD (2264)
ID= 1 DT=15.0 mi n
Area (ha)= 117.80
Total Imp(%)= 75.00
Di r. Conn. (%) = 70.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	88.35	29.45
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	886.19	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	116.82	94.32
over (mi n)	15.00	30.00
Storage Coeff. (mi n)=	10.94 (ii)	21.89 (ii)
Unit Hyd. peak (cms)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.05
PEAK FLOW (cms)=	21.96	3.79
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	86.50	50.58
TOTAL RAINFALL (mm)=	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.57

TOTALS
24.459 (iii)
6.00
75.73
88.50
0.86

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9277)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2---> OUT= 1	(cms)	(ha. m.)	(cms)	(ha. m.)
DT= 15.0 mi n	0.0000	0.0000	2.5000	8.0000
	0.3000	2.0000	3.5000	10.0000
	0.9000	5.0000	4.5000	13.0000
	2.0000	7.5000	5.5000	14.0000
INFLOW : ID= 2 (2264)	117.800	24.459	6.00	75.73
OUTFLOW: ID= 1 (9277)	117.800	1.563	8.00	75.71

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 6.39
TIME SHIF T OF PEAK FLOW (mi n)=120.00
MAXIMUM STORAGE USED (ha. m.) = 6.5131

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9276):	10.50	0.373	7.25	48.48
+ ID2= 2 (9277):	117.80	1.563	8.00	75.71

ID = 3 (9253):	128.30	1.911	7.50	73.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5226)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.7530	0.6823
0.0890	0.3202	2.2010	0.7801
0.5080	0.3704	2.5620	0.8758
0.9450	0.4782	2.9620	0.8858
1.3110	0.5708	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9253)	128.300	1.911	7.50	73.48
OUTFLOW: ID= 1 (5226)	128.300	1.797	8.75	73.48

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 94.05
TIME SHIFT OF PEAK FLOW (min) = 75.00
MAXIMUM STORAGE USED (ha.m.) = 0.6925

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
(2262)	119.28	77.1	1.50
La (mm) =	5.00		
U.H. Tp (hrs) =	1.05		

Unit Hyd Opeak (cms) = 1.934

PEAK FLOW (cms) = 2.390 (i)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 43.452
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.491

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2262):	119.28	2.390	7.25	43.45
+ ID2= 2 (5226):	128.30	1.797	8.75	73.48

ID = 3 (9226):	247.58	4.024	8.00	59.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6226):	359.98	9.432	7.00	66.50
+ ID2= 2 (9226):	247.58	4.024	8.00	59.01

ID = 3 (8168):	607.56	13.161	7.25	63.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8160):	16746.20	167.454	9.75	37.65
+ ID2= 2 (8168):	607.56	13.161	7.25	63.45

ID = 3 (8162):	17353.76	176.667	9.25	38.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6228)
IN= 2--> OUT= 1

Routing time step (min) = 15.00

Di stance	Elevation	Manning		
0.00	245.00	0.0600		
18.60	244.74	0.0600		
37.20	242.47	0.0600		
93.01	233.71	0.0600		
125.57	226.95	0.0600		
213.93	221.63	0.0600		
218.58	221.15	0.0600		
223.23	220.45	0.0600		
225.03	220.55	0.0600 / 0.0300	Mai n	Channel
225.53	218.05	0.0300	Mai n	Channel
232.53	218.05	0.0300	Mai n	Channel
240.03	218.05	0.0300	Mai n	Channel
241.03	220.55	0.0300 / 0.0600	Mai n	Channel
246.48	221.00	0.0600		
251.13	221.27	0.0600		
255.78	221.22	0.0600		
325.54	225.25	0.0600		
367.40	228.91	0.0600		
404.60	231.75	0.0600		
460.41	245.00	0.0600		

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
1.25	219.30	.338E+05	16.0	0.86	35.15
2.50	220.55	.696E+05	48.3	1.26	24.02
3.94	221.99	.168E+06	116.0	1.26	24.14
5.38	223.43	.392E+06	247.5	1.15	26.37
6.81	224.86	.743E+06	472.6	1.16	26.20
8.25	226.30	.122E+07	821.6	1.23	24.67
9.69	227.74	.179E+07	1320.1	1.34	22.59
11.13	229.18	.243E+07	1965.7	1.47	20.58
12.57	230.62	.313E+07	2744.2	1.59	19.02
14.01	232.06	.390E+07	3683.4	1.72	17.66
15.44	233.49	.472E+07	4824.1	1.86	16.30
16.88	234.93	.557E+07	6092.4	1.99	15.23
18.32	236.37	.646E+07	7503.3	2.11	14.35
19.76	237.81	.739E+07	9061.2	2.23	13.59
21.20	239.25	.836E+07	10767.6	2.34	12.94
22.64	240.69	.937E+07	12624.8	2.45	12.37
24.07	242.12	.104E+08	14634.9	2.56	11.87
25.51	243.56	.115E+08	16762.8	2.65	11.45
26.95	245.00	.127E+08	18699.9	2.69	11.28

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8162)	*****	176.67	9.25	38.56	4.60	1.20
OUTFLOW: ID= 1 (6228)	*****	173.98	10.00	38.56	4.57	1.21

CALIB
STANDHYD (2287)
ID= 1 DT=15.0 min

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	30.01	100.48
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	932.74	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	116.82	56.46
over (min)	15.00	30.00
Storage Coeff. (min)	11.28 (ii)	24.73 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04

PEAK FLOW (cms) = 3.44 7.20 *TOTALS*
TIME TO PEAK (hrs) = 6.00 6.25 8.539 (iii)
RUNOFF VOLUME (mm) = 86.50 31.79 6.25
TOTAL RAINFALL (mm) = 88.50 88.50 37.26 88.50

RUNOFF COEFFICIENT = 0.98 0.36 0.42

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 61.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9275)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.5000	3.5000
0.8000	0.1000	6.0000	4.0000
1.5000	2.0000	6.5000	4.5000
2.5000	3.0000	7.0000	5.5000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2287)	130.500	8.539	6.25	37.26
OUTFLOW: ID= 1 (9275)	130.500	1.803	7.50	37.26

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 21.11
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha.m.) = 2.3069

RESERVOIR (5228)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	6.7260	2.6184
0.3410	1.2286	8.4460	2.9937
1.9480	1.4214	9.8320	3.3609
3.6280	1.8348	10.2320	3.3709
5.0330	2.1904	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9275)	130.500	1.803	7.50	37.26
OUTFLOW: ID= 1 (5228)	130.500	1.592	8.75	37.26

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 88.30
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha.m.) = 1.3791

CALIB NASHYD (2282)
 ID= 1 DT=15.0 min

Area (ha) = 180.03 Curve Number (CN) = 61.9
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp (hrs) = 1.12

Unit Hyd Opeak (cms) = 2.754
 PEAK FLOW (cms) = 2.227 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 28.818
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.326

- (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9228)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2282):	180.03	2.227	7.50	28.82
+ ID2= 2 (5228):	130.50	1.592	8.75	37.25
ID = 3 (9228):	310.53	3.655	8.50	32.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8170)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6228):	17353.76	173.976	10.00	38.56
+ ID2= 2 (9228):	310.53	3.655	8.50	32.36
ID = 3 (8170):	17664.29	177.098	10.00	38.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2302)
 ID= 1 DT=15.0 min

Area (ha) = 30.47
 Total Imp (%) = 29.00 Dir. Conn. (%) = 11.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 8.84 21.63
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 450.70 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr) = 116.82 73.75
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 7.29 (ii) 19.38 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.10 0.05

PEAK FLOW (cms) = 0.99 2.28 *TOTALS*
 TIME TO PEAK (hrs) = 6.00 6.25 2.534 (iii)
 RUNOFF VOLUME (mm) = 86.50 38.37 6.25
 TOTAL RAINFALL (mm) = 88.50 88.50 43.67
 RUNOFF COEFFICIENT = 0.98 0.43 88.50
 0.49

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2307)
 ID= 1 DT=15.0 min

Area (ha) = 175.00
 Total Imp (%) = 25.00 Dir. Conn. (%) = 9.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 43.75 131.25
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 1080.12 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr) = 116.82 86.78
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 12.32 (ii) 23.65 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.08 0.04

PEAK FLOW (cms) = 4.02 14.94 *TOTALS*
 TIME TO PEAK (hrs) = 6.00 6.25 16.624 (iii)
 RUNOFF VOLUME (mm) = 86.50 46.18 6.25
 TOTAL RAINFALL (mm) = 88.50 88.50 49.81
 RUNOFF COEFFICIENT = 0.98 0.52 88.50
 0.56

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 75.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9280)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	8.7500	3.0604
0.5900	1.4722	11.7100	3.8736
3.3900	1.6096	14.7000	4.6556
6.3200	2.4308	17.1100	5.4201

INFLOW : ID= 2 (2307)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
175.000	175.000	16.624	6.25	49.81
OUTFLOW: ID= 1 (9280)	175.000	9.018	6.75	49.81

PEAK FLOW REDUCTION [Qout/Qin] (%) = 54.25
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 3.1869

RESERVOIR (5230)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	8.6830	2.7782
0.4700	1.3193	10.7480	3.1570
2.5870	1.5374	12.3570	3.5304
5.1590	1.9534	12.7570	3.5404
6.7830	2.3333	0.0000	0.0000

INFLOW : ID= 2 (9280)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
175.000	175.000	9.018	6.75	49.81
OUTFLOW: ID= 1 (5230)	175.000	6.511	7.50	49.81

PEAK FLOW REDUCTION [Qout/Qin] (%) = 72.20
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 2.2872

ADD HYD (8172)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2302):	30.47	2.534	6.25	43.67
+ ID2= 2 (5230):	175.00	6.511	7.50	49.81
-----	-----	-----	-----	-----
ID = 3 (8172):	205.47	6.947	7.25	48.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (2329)
ID= 1 DT=15.0 min

Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1.80	5.00	0.22	49.81

Uni t Hyd Qpeak (cms) = 0.143

PEAK FLOW (cms) = 0.093 (i)
TIME TO PEAK (hrs) = 6.000
RUNOFF VOLUME (mm) = 31.896
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.360

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9269)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.0900	0.0300
0.0100	0.0136	0.1200	0.0386
0.0400	0.0146	0.1500	0.0471
0.0700	0.0234	0.1800	0.0554

INFLOW : ID= 2 (2329)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1.800	1.800	0.093	6.00	31.90
OUTFLOW: ID= 1 (9269)	1.800	0.054	6.75	31.72

PEAK FLOW REDUCTION [Qout/Qin] (%) = 57.61
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.0187

CALIB STANDHYD (2327)
ID= 1 DT=15.0 min

Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
184.00	24.00	8.00	49.81

Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
44.16		139.84
Dep. Storage (mm) = 2.00		5.00
Average Slope (%) = 0.50		0.50
Length (m) = 1107.55		40.00
Mannings n = 0.013		0.250
Max. Eff. Inten. (mm/hr) = 116.82		68.57
over (min) = 15.00		30.00
Storage Coeff. (min) = 12.51 (ii)		24.95 (ii)
Uni t Hyd. Tpeak (min) = 15.00		30.00
Uni t Hyd. peak (cms) = 0.08		0.04

TOTALS
13.748 (iii)

PEAK FLOW (cms)	TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
3.74	6.25	36.99	88.50	0.42
6.00	6.25	40.95	88.50	0.46
86.50	6.25	40.95	88.50	0.46
88.50	6.25	40.95	88.50	0.46
0.98	6.25	40.95	88.50	0.46

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 66.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9270)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.0000	4.0000
1.1000	0.3000	3.5000	4.5000
1.8000	1.8000	4.5000	6.0000
2.0000	2.5000	5.5000	8.0000

INFLOW : ID= 2 (2327)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
184.000	184.000	13.748	6.25	40.95
OUTFLOW: ID= 1 (9270)	184.000	2.806	7.50	40.95

PEAK FLOW REDUCTION [Qout/Qin] (%) = 20.41
TIME SHIFT OF PEAK FLOW (min) = 75.00
MAXIMUM STORAGE USED (ha. m.) = 3.7126

ADD HYD (9249)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (9269):	1.80	0.054	6.75	31.72
+ ID2= 2 (9270):	184.00	2.806	7.50	40.95
-----	-----	-----	-----	-----
ID = 3 (9249):	185.80	2.849	7.50	40.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5232)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	11.1130	4.3262
0.5630	0.0299	13.9540	4.9462
3.2190	2.3484	16.2450	5.5530
5.9950	3.0315	16.6450	5.5630
8.3150	3.6189	0.0000	0.0000

INFLOW : ID= 2 (9249) AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 185.800 2.849 7.50 40.86
 OUTFLOW: ID= 1 (5232) 185.800 2.493 9.00 40.85

PEAK FLOW REDUCTION [Qout/Qin] (%) = 87.51
 TIME SHIFT OF PEAK FLOW (min) = 90.00
 MAXIMUM STORAGE USED (ha. m.) = 2.2619

CALIB NASHYD (2322) | Area (ha) = 129.00 Curve Number (CN) = 58.1
 ID= 1 DT=15.0 min | Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.00

Unit Hyd Qpeak (cms) = 2.209

PEAK FLOW (cms) = 1.558 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 25.877
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.292

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9232) | AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 1 + 2 = 3 |
 ID1= 1 (2322): 129.00 1.558 7.25 25.88
 + ID2= 2 (5232): 185.80 2.493 9.00 40.85
 ID = 3 (9232): 314.80 3.784 9.00 34.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8173) | AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 1 + 2 = 3 |
 ID1= 1 (8172): 205.47 6.947 7.25 48.90
 + ID2= 2 (9232): 314.80 3.784 9.00 34.72
 ID = 3 (8173): 520.27 8.907 7.50 40.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8174) | AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 1 + 2 = 3 |
 ID1= 1 (8170): 17664.29 177.098 10.00 38.45
 + ID2= 2 (8173): 520.27 8.907 7.50 40.32
 ID = 3 (8174): 18184.56 181.932 10.00 38.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6234) | Routing time step (min)' = 15.00
 IN= 2--> OUT= 1

<----- DATA FOR SECTION (2341.0) ----->

Distance	Elevation	Manning
0.00	228.00	0.0550
42.86	223.21	0.0550
85.72	219.56	0.0550
107.15	219.42	0.0550
128.58	219.39	0.0550
214.30	219.30	0.0550
225.02	219.26	0.0550
235.73	219.24	0.0550
255.16	219.20	0.0550 / 0.0350
257.16	217.20	0.0350
303.16	217.20	0.0350

Main Channel
Main Channel
Main Channel

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	217.70	887E+05	7.2	0.31	206.54
1.00	218.20	179E+06	22.7	0.48	131.74
1.50	218.70	272E+06	44.5	0.62	101.75
2.00	219.20	366E+06	71.8	0.75	84.97
2.59	219.79	993E+06	136.1	0.52	121.59
3.17	220.37	189E+07	258.6	0.52	121.80
3.76	220.96	294E+07	434.7	0.56	112.82
4.35	221.55	415E+07	665.9	0.61	103.88
4.93	222.13	551E+07	955.0	0.66	96.22
5.52	222.72	703E+07	1305.1	0.71	89.80
6.11	223.31	868E+07	1744.9	0.77	82.91
6.69	223.89	104E+08	2261.4	0.83	76.46
7.28	224.48	121E+08	2838.0	0.89	71.12
7.87	225.07	139E+08	3472.9	0.95	66.63
8.45	225.65	157E+08	4165.0	1.01	62.82
9.04	226.24	176E+08	4913.5	1.07	59.54
9.63	226.83	194E+08	5717.7	1.12	56.68
10.21	227.41	214E+08	6577.1	1.17	54.18
10.80	228.00	235E+08	6919.5	1.12	56.61

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	217.70	887E+05	7.2	0.31	206.54
1.00	218.20	179E+06	22.7	0.48	131.74
1.50	218.70	272E+06	44.5	0.62	101.75
2.00	219.20	366E+06	71.8	0.75	84.97
2.59	219.79	993E+06	136.1	0.52	121.59
3.17	220.37	189E+07	258.6	0.52	121.80
3.76	220.96	294E+07	434.7	0.56	112.82
4.35	221.55	415E+07	665.9	0.61	103.88
4.93	222.13	551E+07	955.0	0.66	96.22
5.52	222.72	703E+07	1305.1	0.71	89.80
6.11	223.31	868E+07	1744.9	0.77	82.91
6.69	223.89	104E+08	2261.4	0.83	76.46
7.28	224.48	121E+08	2838.0	0.89	71.12
7.87	225.07	139E+08	3472.9	0.95	66.63
8.45	225.65	157E+08	4165.0	1.01	62.82
9.04	226.24	176E+08	4913.5	1.07	59.54
9.63	226.83	194E+08	5717.7	1.12	56.68
10.21	227.41	214E+08	6577.1	1.17	54.18
10.80	228.00	235E+08	6919.5	1.12	56.61

<---- hydrograph ----> <- pipe / channel ->
 AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm) MAX DEPTH (m) MAX VEL (m/s)
 INFLOW : ID= 2 (8174) ***** 181.93 10.00 38.50 2.81 0.52
 OUTFLOW: ID= 1 (6234) ***** 153.06 11.75 38.50 2.67 0.52

CALIB NASHYD (2347) | Area (ha) = 266.10 Curve Number (CN) = 56.0
 ID= 1 DT=15.0 min | Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 2.00

Unit Hyd Qpeak (cms) = 2.276

PEAK FLOW (cms) = 1.772 (i)
 TIME TO PEAK (hrs) = 9.000
 RUNOFF VOLUME (mm) = 24.541
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.277

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9281) | IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	13.3100	4.0635
0.9000	1.7152	17.8000	5.3537
5.1600	1.7685	22.3500	6.6446
9.6100	3.0976	26.0300	7.9060

INFLOW : ID= 2 (2347)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
266.100	266.100	1.772	9.00	24.54
OUTFLOW: ID= 1 (9281)	266.100	1.987	10.50	24.54

PEAK FLOW REDUCTION [Qout/Qin] (%) = 112.14
 TIME SHIFT OF PEAK FLOW (min) = 90.00
 MAXIMUM STORAGE USED (ha. m.) = 1.7597

**** WARNING : HYDROGRAPH PEAK WAS NOT REDUCED.
 CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.

RESERVOIR (5234) | IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW	STORAGE	OUTFLOW	STORAGE
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(cms)	(ha. m.)	(cms)	(ha. m.)
0.0000	0.0000	0.5630	0.1745
0.1300	0.0819	0.6550	0.1995
0.2420	0.0947	0.7760	0.2240
0.3350	0.1223	1.0550	0.2340
0.4480	0.1460	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9281)	266.100	1.987	10.50	24.54
OUTFLOW: ID= 1 (5234)	266.100	1.763	10.75	24.54

PEAK FLOW REDUCTION [Qout/Qin] (%) = 88.69
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 0.2703

CALIB STANDHYD (2342) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	1.06 22.00	Di r. Conn. (%) =	8.00
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	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	0.23	0.83
Dep. Storage (mm)=	1.00	5.00
Average Slope (%)=	1.00	2.00
Length (m)=	84.06	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	116.82	29.74
over (min)	15.00	15.00
Storage Coeff. (min)=	2.16 (ii)	13.63 (ii)
Unit Hyd. Tpeak (min)=	15.00	15.00
Unit Hyd. peak (cms)=	0.11	0.08

TOTALS
 PEAK FLOW (cms) = 0.03
 TIME TO PEAK (hrs) = 6.00
 RUNOFF VOLUME (mm) = 87.50
 TOTAL RAINFALL (mm) = 88.50
 RUNOFF COEFFICIENT = 0.99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 39.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9234) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2342):	1.06	0.077	6.00	22.71
+ ID2= 2 (5234):	266.10	1.763	10.75	24.54
ID = 3 (9234):	267.16	1.765	10.75	24.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8176) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6234):	18184.56	153.062	11.75	38.50
+ ID2= 2 (9234):	267.16	1.765	10.75	24.53
ID = 3 (8176):	18451.72	154.642	11.75	38.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2369) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	441.90 23.00	Di r. Conn. (%) =	8.00
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IMPERVIOUS PERVIOUS (i)

Surface Area (ha)=	101.64	340.26
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1716.39	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	116.82	72.62
over (min)	15.00	30.00
Storage Coeff. (min)=	16.27 (ii)	28.43 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.07	0.04

TOTALS
 PEAK FLOW (cms) = 8.06
 TIME TO PEAK (hrs) = 6.00
 RUNOFF VOLUME (mm) = 86.50
 TOTAL RAINFALL (mm) = 88.50
 RUNOFF COEFFICIENT = 0.98

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9266) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	2.3000	8.0000
	0.2000	2.0000	3.5000	10.0000
	1.9000	7.0000	4.5000	13.0000
	2.0000	7.5000	5.5000	14.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2369)	441.900	33.514	6.25	43.29
OUTFLOW: ID= 1 (9266)	441.900	4.330	8.50	43.28

PEAK FLOW REDUCTION [Qout/Qin] (%) = 12.92
 TIME SHIFT OF PEAK FLOW (min) = 135.00
 MAXIMUM STORAGE USED (ha. m.) = 12.5100

RESERVOIR (5236) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	3.1380	1.5897
	1.0220	0.7542	3.7950	2.0204
	1.7530	0.8200	4.4970	2.4373
	2.3290	1.2567	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9266)	441.900	4.330	8.50	43.28
OUTFLOW: ID= 1 (5236)	441.900	3.907	11.00	43.28

PEAK FLOW REDUCTION [Qout/Qin] (%) = 90.22
 TIME SHIFT OF PEAK FLOW (min) = 150.00
 MAXIMUM STORAGE USED (ha. m.) = 2.0868

CALIB NASHYD (2362) ID= 1 DT=15.0 min	Area Ia	(ha)= (mm)=	52.59 5.00	Curve Number (CN) =	58.7
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Unit Hyd Opeak (cms) = 0.726

PEAK FLOW (cms) =	0.549 (i)
TIME TO PEAK (hrs) =	8.000
RUNOFF VOLUME (mm) =	26.392
TOTAL RAINFALL (mm) =	88.500
RUNOFF COEFFICIENT =	0.298

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9236)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (2362):	52.59	0.549	8.00	26.39
+ ID2= 2 (5236):	441.90	3.907	11.00	43.28
ID = 3 (9236):	494.49	4.297	10.50	41.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6238)
IN= 2---> OUT= 1 Routing time step (min)' = 15.00

Distance	Elevation	Manning	
0.00	241.54	0.0380	
602.55	227.00	0.0380	
1702.00	224.50	0.0380	
1721.25	223.00	0.0380	
1725.10	222.60	0.0380	
1780.94	222.50	0.0380	
1782.87	222.45	0.0380	
1784.79	222.40	0.0380 / 0.0300	Main Channel
1786.57	221.75	0.0300	Main Channel
1787.07	220.75	0.0300	Main Channel
1787.57	220.75	0.0300	Main Channel
1791.57	220.75	0.0300	Main Channel
1794.07	221.75	0.0300	Main Channel
1798.27	222.00	0.0300	Main Channel
1800.19	222.35	0.0300 / 0.0380	Main Channel
1802.12	222.47	0.0380	
1840.63	223.00	0.0380	
1900.00	225.00	0.0380	
1950.00	226.00	0.0380	
2242.61	240.00	0.0380	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.80	221.55	.101E+05	5.2	1.14	32.36
1.60	222.35	.296E+05	18.7	1.40	26.36
2.64	223.39	.249E+06	151.8	1.35	27.33
3.68	224.43	.613E+06	512.2	1.85	19.94
4.71	225.46	.152E+07	1066.8	1.55	23.80
5.75	226.50	.359E+07	2824.9	1.75	21.16
6.79	227.54	.663E+07	6528.0	2.18	16.93
7.83	228.58	.994E+07	12158.6	2.71	13.62
8.87	229.62	.134E+08	19268.8	3.19	11.58
9.91	230.66	.170E+08	27785.3	3.63	10.19
10.94	231.69	.207E+08	37667.1	4.03	9.18
11.98	232.73	.246E+08	48891.7	4.40	8.40
13.02	233.77	.287E+08	61448.0	4.75	7.78
14.06	234.81	.329E+08	75332.7	5.08	7.28
15.10	235.85	.372E+08	90547.8	5.39	6.85
16.14	236.89	.417E+08	*****	5.69	6.49
17.17	237.92	.464E+08	*****	5.98	6.18
18.21	238.96	.512E+08	*****	6.25	5.91
19.25	240.00	.561E+08	*****	6.52	5.67

INFLOW : ID= 2 (9236)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW: ID= 1 (6238)	494.49	4.30	10.50	41.49	0.66	1.14
	494.49	4.26	11.25	41.49	0.65	1.14

CALIB STANDHYD (2389)	Area (ha)	Imp (%)	Di r. Conn. (%)
ID= 1 DT=15.0 mi n	4.70	21.00	7.00
IMPERVIOUS	0.99		
PERVIOUS (i)	3.71		
Surface Area (ha)	0.99		
Dep. Storage (mm)	2.00		
Average Slope (%)	0.50		
Length (m)	177.01		
Mannings n	0.013		
	0.250		

Max. Eff. Inten. (mm/hr)=	116.82	70.90
Storage over (mi n)	15.00	30.00
Storage Coeff. (mi n)=	4.16 (ii)	16.44 (ii)
Unit Hyd. Tpeak (mi n)=	15.00	30.00
Unit Hyd. peak (cms)=	0.11	0.05

PEAK FLOW (cms)=	0.10	0.40	*TOTALS* (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.25
RUNOFF VOLUME (mm)=	86.50	39.18	42.49
TOTAL RAINFALL (mm)=	88.50	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.44	0.48

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9267)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2---> OUT= 1 DT= 15.0 mi n	0.0000	0.0000	0.2300	0.0785
	0.0200	0.0364	0.3100	0.1005
	0.0900	0.0393	0.3900	0.1220
	0.1700	0.0616	0.4600	0.1429

INFLOW : ID= 2 (2389)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (9267)	4.700	0.418	6.25	42.49
	4.700	0.218	6.50	42.40

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 52.19
TIME SHI FT OF PEAK FLOW (mi n) = 15.00
MAXIMUM STORAGE USED (ha. m.) = 0.0765

CALIB NASHYD (2387)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 mi n	90.20	54.0
	5.00	# of Li near Res. (N) = 1.50
	U. H. Tp (hrs) = 0.86	

Unit Hyd Opeak (cms)=	1.791
PEAK FLOW (cms)=	1.082 (i)
TIME TO PEAK (hrs)=	7.000
RUNOFF VOLUME (mm)=	22.954
TOTAL RAINFALL (mm)=	88.500
RUNOFF COEFFICIENT =	0.259

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9268)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2---> OUT= 1 DT= 15.0 mi n	0.0000	0.0000	4.5100	1.3564
	0.3100	0.5624	6.0400	1.7959
	1.7500	0.5748	7.5800	2.2374
	3.2600	1.0285	8.8300	2.6688

INFLOW : ID= 2 (2387)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (9268)	90.200	1.082	7.00	22.95
	90.200	1.234	8.00	22.95

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 114.03
TIME SHI FT OF PEAK FLOW (mi n) = 60.00
MAXIMUM STORAGE USED (ha. m.) = 0.5945

**** WARNING : HYDROGRAPH PEAK WAS NOT REDUCED.
CHECK OUTFLOW/STORAGE TABLE OR REDUCE DT.

ADD HYD	(9247)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (9267):	4.70	0.218	6.50	42.40	
+ ID2= 2 (9268):	90.20	1.234	8.00	22.95	
ID = 3 (9247):	94.90	1.330	8.00	23.91	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5238)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	3.8780	1.5095
0.1960	0.7083	4.8600	1.7258
1.1230	0.8194	5.6680	1.9375
2.0920	1.0578	6.0680	1.9475
2.9010	1.2627	0.0000	0.0000

INFLOW : ID= 2 (9247)	AREA	OPEAK	TPEAK	R. V.
OUTFLOW: ID= 1 (5238)	(ha)	(cms)	(hrs)	(mm)
	94.900	1.330	8.00	23.91
	94.900	0.651	10.25	23.90

PEAK FLOW REDUCTION [Qout/Qin] (%) = 48.96
TIME SHIFT OF PEAK FLOW (min) = 135.00
MAXIMUM STORAGE USED (ha.m.) = 0.7634

CALIB
NASHYD (2382)
ID= 1 DT=15.0 min

Area (ha) = 216.80
Curve Number (CN) = 56.8
U.H. Tp(hrs) = 1.56
of Linear Res. (N) = 1.50

Unit Hyd Opeak (cms) = 2.368

PEAK FLOW (cms) = 1.792 (i)
TIME TO PEAK (hrs) = 8.250
RUNOFF VOLUME (mm) = 25.067
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.283

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9238)
1 + 2 = 3

ADD HYD	(9238)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (2382):	216.80	1.792	8.25	25.07	
+ ID2= 2 (5238):	94.90	0.651	10.25	23.90	
ID = 3 (9238):	311.70	2.274	10.00	24.71	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8180)
1 + 2 = 3

ADD HYD	(8180)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (6238):	494.49	4.264	11.25	41.49	
+ ID2= 2 (9238):	311.70	2.274	10.00	24.71	
ID = 3 (8180):	806.19	6.427	10.25	35.00	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8178)
1 + 2 = 3

ADD HYD	(8178)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (8176):	18451.72	154.642	11.75	38.30	
+ ID2= 2 (8180):	806.19	6.427	10.25	35.00	
ID = 3 (8178):	19257.91	160.700	11.75	38.16	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6240)
IN= 2---> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (2401.0) ----->

Distance	Elevation	Manning	
0.00	222.00	0.0450	
11.46	221.00	0.0450	
208.98	221.00	0.0450	
404.04	220.97	0.0450	
808.08	220.83	0.0450	
905.60	220.17	0.0450	
919.53	219.43	0.0450	
933.47	219.22	0.0450	
945.26	219.21	0.0450 / 0.0300	Mai n Channel
946.26	217.81	0.0300	Mai n Channel
975.26	217.81	0.0300	Mai n Channel
1003.26	217.81	0.0300	Mai n Channel
1005.26	219.21	0.0300 / 0.0450	Mai n Channel
1017.06	219.28	0.0450	
1030.99	219.26	0.0450	
1044.92	219.23	0.0450	
1058.86	219.23	0.0450	
1253.91	219.22	0.0450	
1323.57	221.05	0.0450	
1379.30	222.00	0.0450	

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(min)
0.20	218.01	.307E+05	2.2	0.20	227.52
0.40	218.21	.617E+05	7.1	0.31	143.95
0.60	218.41	.928E+05	14.0	0.41	110.33
0.80	218.61	.124E+06	22.6	0.49	91.46
1.00	218.81	.156E+06	32.8	0.57	79.15
1.20	219.01	.188E+06	44.5	0.64	70.38
1.40	219.21	.220E+06	57.5	0.70	63.77
1.63	219.44	.413E+06	82.5	0.54	83.52
1.87	219.68	.632E+06	120.9	0.51	87.03
2.10	219.91	.858E+06	170.0	0.53	84.11
2.33	220.14	.109E+07	228.7	0.56	79.62
2.56	220.37	.134E+07	294.4	0.59	76.03
2.80	220.61	.162E+07	371.1	0.61	72.75
3.03	220.84	.192E+07	456.8	0.64	70.18
3.26	221.07	.251E+07	552.1	0.59	75.74
3.49	221.30	.333E+07	711.8	0.57	78.07
3.73	221.54	.417E+07	907.0	0.58	76.62
3.96	221.77	.502E+07	1134.1	0.61	73.70
4.19	222.00	.587E+07	1390.9	0.64	70.34

<---- hydrograph ---->

AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8178) *****	160.70	11.75	38.16	2.05	0.53
OUTFLOW: ID= 1 (6240) *****	148.59	13.25	38.16	2.00	0.52

CALIB
NASHYD (0240)
ID= 1 DT=15.0 min

Area (ha) = 345.47
Curve Number (CN) = 46.0
U.H. Tp(hrs) = 3.60
of Linear Res. (N) = 1.50

Unit Hyd Opeak (cms) = 1.637

PEAK FLOW (cms) = 1.080 (i)
TIME TO PEAK (hrs) = 12.000
RUNOFF VOLUME (mm) = 18.240
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.206

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (2407)
ID= 1 DT=15.0 min

Area (ha) = 88.90
Total Imp(%) = 20.00
Dir. Conn. (%) = 7.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 17.78 71.12
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 769.85 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.82 62.41
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 10.06 (ii) 22.98 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.04

PEAK FLOW (cms)= 1.70 5.85
 TIME TO PEAK (hrs)= 6.00 6.25
 RUNOFF VOLUME (mm)= 86.50 35.12
 TOTAL RAINFALL (mm)= 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.40

TOTALS
 6.450 (iii)
 6.25
 38.72
 88.50
 0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9274)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.4500	5.4723
0.3000	4.6746	5.9500	5.8930
1.7300	4.7225	7.4700	6.3047
3.2100	5.1511	8.7000	6.7071

INFLOW : ID= 2 (2407)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
88.900	88.900	6.450	6.25	38.72
OUTFLOW: ID= 1 (9273)	88.900	0.194	12.25	38.67

PEAK FLOW REDUCTION [Qout/Qin] (%) = 3.01
 TIME SHIFT OF PEAK FLOW (min) = 360.00
 MAXIMUM STORAGE USED (ha.m.) = 3.0279

ADD HYD (9252)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0240):	345.47	1.080	12.00	18.24
+ ID2= 2 (9273):	88.90	0.194	12.25	38.67

ID = 3 (9252):	434.37	1.273	12.00	22.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8182)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6240):	19257.91	148.587	13.25	38.16
+ ID2= 2 (9252):	434.37	1.273	12.00	22.42

ID = 3 (8182):	19692.28	149.820	13.00	37.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (2427)
 ID= 1 DT=15.0 min

Area (ha)= 5.40 Curve Number (CN)= 74.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp (hrs)= 0.37

Unit Hyd Opeak (cms)= 0.249

PEAK FLOW (cms)= 0.218 (i)
 TIME TO PEAK (hrs)= 6.250
 RUNOFF VOLUME (mm)= 38.583
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.436

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9274)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.2700	0.0857
0.0200	0.0379	0.3600	0.1115
0.1000	0.0399	0.4500	0.1369
0.1900	0.0663	0.5300	0.1618

INFLOW : ID= 2 (2427)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
5.400	5.400	0.218	6.25	38.58
OUTFLOW: ID= 1 (9274)	5.400	0.159	7.00	38.50

PEAK FLOW REDUCTION [Qout/Qin] (%) = 73.00
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0577

RESERVOIR (5242)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0480	1.0185
0.0020	1.0087	0.0600	1.0212
0.0140	1.0101	0.0700	1.0238
0.0260	1.0130	0.4700	1.0338
0.0360	1.0155	0.0000	0.0000

INFLOW : ID= 2 (9274)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
5.400	5.400	0.159	7.00	38.50
OUTFLOW: ID= 1 (5242)	5.400	0.000	33.75	11.36

PEAK FLOW REDUCTION [Qout/Qin] (%) = 0.25
 TIME SHIFT OF PEAK FLOW (min) = *****
 MAXIMUM STORAGE USED (ha.m.) = 0.2041

CALIB NASHYD (2422)
 ID= 1 DT=15.0 min

Area (ha)= 652.48 Curve Number (CN)= 51.6
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp (hrs)= 5.37

Unit Hyd Opeak (cms)= 2.073

PEAK FLOW (cms)= 1.713 (i)
 TIME TO PEAK (hrs)= 12.750
 RUNOFF VOLUME (mm)= 21.652
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.245

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9242)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2422):	652.48	1.713	12.75	21.65
+ ID2= 2 (5242):	5.40	0.000	33.75	11.36

ID = 3 (9242):	657.88	1.713	12.75	21.57

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8184)
 1 + 2 = 3

AREA OPEAK TPEAK R. V.

	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8182):	19692.28	149.820	13.00	37.81
+ ID2= 2 (9242):	657.88	1.713	12.75	21.57

ID = 3 (8184):	20350.16	151.532	13.00	37.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6244)
IN= 2---> OUT= 1

Routing time step (min)= 15.00

<----- DATA FOR SECTION (2441.0) ----->

Distance	Elevation	Manning	
0.00	225.00	0.0450	
2.50	220.10	0.0450	
3.00	220.06	0.0450	
3.64	220.07	0.0450	
7.28	220.08	0.0450	
10.91	220.09	0.0450	
14.55	219.81	0.0450	
18.19	219.29	0.0450	
24.13	219.21	0.0450 / 0.0300	Main Channel
24.63	217.81	0.0300	Main Channel
69.13	217.81	0.0300	Main Channel
114.13	217.81	0.0300	Main Channel
115.13	219.21	0.0300 / 0.0450	Main Channel
120.06	219.25	0.0450	
123.70	219.32	0.0450	
127.34	219.77	0.0450	
130.98	219.88	0.0450	
134.61	219.99	0.0450	
350.00	220.18	0.0450	
360.18	225.00	0.0450	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.35	218.16	.108E+06	9.0	0.29	200.52
0.70	218.51	.216E+06	28.4	0.45	126.89
1.05	218.86	.325E+06	55.7	0.59	97.27
1.40	219.21	.434E+06	89.7	0.71	80.65
1.79	219.60	.574E+06	135.9	0.81	70.36
2.17	219.98	.726E+06	190.8	0.90	63.38
2.56	220.37	.111E+07	264.3	0.82	69.82
2.94	220.75	.157E+07	371.7	0.81	70.36
3.33	221.14	.203E+07	503.4	0.85	67.30
3.72	221.53	.250E+07	656.6	0.90	63.40
4.10	221.91	.296E+07	829.6	0.96	59.54
4.49	222.30	.343E+07	1021.1	1.02	56.00
4.87	222.68	.390E+07	1230.2	1.08	52.83
5.26	223.07	.437E+07	1456.2	1.14	50.01
5.65	223.46	.484E+07	1698.4	1.20	47.51
6.03	223.84	.531E+07	1956.3	1.26	45.28
6.42	224.23	.579E+07	2229.3	1.32	43.28
6.80	224.61	.626E+07	2517.0	1.38	41.48
7.19	225.00	.674E+07	2819.2	1.44	39.85

<----- hydrograph ----->				<-pi pe / channel-->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8184) *****	151.53	13.00	37.29	1.90	0.84
OUTFLOW: ID= 1 (6244) *****	145.89	14.25	37.29	1.85	0.83

CALIB STANDHYD (2441)
ID= 1 DT=15.0 mi n

Area (ha)= 91.52
Total Imp(%)= 45.00 Di r. Conn.(%)= 15.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	41.18	50.34
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	781.11	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (mi n)	116.82	89.89
Storage Coeff. (mi n)	10.15 (ii)	21.31 (ii)
Unit Hyd. Tpeak (mi n)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.05

	(cms)	(hrs)	(mm)	*TOTALS*
PEAK FLOW	3.74	6.20	7.728	(iii)
TIME TO PEAK	6.00	6.25	6.00	
RUNOFF VOLUME	86.50	38.11	45.36	
TOTAL RAI NFALL	88.50	88.50	88.50	
RUNOFF COEFFICIENT	0.98	0.43	0.51	

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 60.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2449)
ID= 1 DT=15.0 mi n

Area (ha)= 143.20
Total Imp(%)= 34.00 Di r. Conn.(%)= 17.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	48.69	94.51
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	977.07	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (mi n)	116.82	78.85
Storage Coeff. (mi n)	11.60 (ii)	23.37 (ii)
Unit Hyd. Tpeak (mi n)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04

	(cms)	(hrs)	(mm)	*TOTALS*
PEAK FLOW	6.36	9.79	12.639	(iii)
TIME TO PEAK	6.00	6.25	6.00	
RUNOFF VOLUME	86.50	40.76	48.54	
TOTAL RAI NFALL	88.50	88.50	88.50	
RUNOFF COEFFICIENT	0.98	0.46	0.55	

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9265)
IN= 2---> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	7.1600	7.7857
0.4900	6.4537	9.5800	8.4259
2.7800	6.6401	12.0300	9.0179
5.1700	7.2743	14.0100	9.5968

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2449)	143.200	12.639	6.00	48.54
OUTFLOW: ID= 1 (9265)	143.200	0.453	12.25	48.51

PEAK FLOW REDUCTION [Qout/Qin](%)= 3.58
TIME SHIFT OF PEAK FLOW (mi n)=375.00
MAXIMUM STORAGE USED (ha. m.)= 5.9648

ADD HYD (9245)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2441):	91.52	7.728	6.00	45.36
+ ID2= 2 (9265):	143.20	0.453	12.25	48.51
=====				
ID = 3 (9245):	234.72	7.849	6.00	47.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5244)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	15.7000	6.1120
0.7950	2.8678	19.7140	6.9879
4.5470	3.3178	22.9500	7.8451
8.4690	4.2829	23.3500	7.8551
11.7470	5.1128	0.0000	0.0000

INFLOW : ID= 2 (9245)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
234.720	7.849	6.00	47.29	
OUTFLOW: ID= 1 (5244)	234.720	1.415	8.00	47.28

PEAK FLOW REDUCTION [Qout/Qin] (%) = 18.02
TIME SHIFT OF PEAK FLOW (min) = 120.00
MAXIMUM STORAGE USED (ha.m.) = 2.9429

CALLIB NASHYD (2442)
ID= 1 DT=15.0 min
Area (ha)=1674.00
Curve Number (CN)= 62.1
La (mm)= 5.00
of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 2.20

Unit Hyd Opeak (cms)= 12.991

PEAK FLOW (cms)= 12.326 (i)
TIME TO PEAK (hrs)= 9.500
RUNOFF VOLUME (mm)= 29.139
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.329

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9244)
1 + 2 = 3
ID1= 1 (2442): 1674.00 12.326 9.50 29.14
+ ID2= 2 (5244): 234.72 1.415 8.00 47.28
ID = 3 (9244): 1908.72 13.380 9.00 31.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8186)
1 + 2 = 3
ID1= 1 (6244): 20350.16 145.888 14.25 37.29
+ ID2= 2 (9244): 1908.72 13.380 9.00 31.37
ID = 3 (8186): 22258.88 154.944 14.00 36.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ HYD (8190)
DT=15.0 min
AREA (ha)=*****
TPEAK (hrs)= 11.50
VOLUME (mm)= 33.75

Filename: S:\2010 Projects\D-EE (E10)\WR\W10-487 East Gwillimbury SWM Master Plan\Calcs\Hydrologic Model s\Ex Comments:

HOWEVER THE TABLE BELOW AND HYDROGRAPH FILES DO.

TIME hrs	FLOW cms	TIME hrs	FLOW cms	TIME hrs	FLOW cms	TIME hrs	FLOW cms	TIME hrs	FLOW cms
0.00	0.000	66.00	2.532	132.00	0.380	198.00	0.054	264.00	0.007
0.25	0.000	66.25	2.513	132.25	0.377	198.25	0.054	264.25	0.007
0.50	0.000	66.50	2.494	132.50	0.375	198.50	0.053	264.50	0.007
0.75	0.000	66.75	2.476	132.75	0.372	198.75	0.053	264.75	0.007
1.00	0.159	67.00	2.458	133.00	0.369	199.00	0.052	265.00	0.007
1.25	0.498	67.25	2.439	133.25	0.367	199.25	0.052	265.25	0.007
1.50	0.689	67.50	2.422	133.50	0.364	199.50	0.052	265.50	0.007

1.75	0.825	67.75	2.404	133.75	0.361	199.75	0.051	265.75	0.007
2.00	0.935	68.00	2.386	134.00	0.359	200.00	0.051	266.00	0.007
2.25	1.115	68.25	2.368	134.25	0.356	200.25	0.050	266.25	0.007
2.50	1.251	68.50	2.351	134.50	0.354	200.50	0.050	266.50	0.007
2.75	1.376	68.75	2.334	134.75	0.351	200.75	0.050	266.75	0.007
3.00	1.503	69.00	2.317	135.00	0.348	201.00	0.049	267.00	0.007
3.25	1.811	69.25	2.300	135.25	0.346	201.25	0.049	267.25	0.007
3.50	2.067	69.50	2.283	135.50	0.343	201.50	0.049	267.50	0.007
3.75	2.327	69.75	2.266	135.75	0.341	201.75	0.048	267.75	0.007
4.00	2.607	70.00	2.250	136.00	0.338	202.00	0.048	268.00	0.006
4.25	3.283	70.25	2.234	136.25	0.336	202.25	0.048	268.25	0.006
4.50	3.889	70.50	2.218	136.50	0.334	202.50	0.047	268.50	0.006
4.75	4.914	70.75	2.201	136.75	0.331	202.75	0.047	268.75	0.006
5.00	5.918	71.00	2.186	137.00	0.329	203.00	0.046	269.00	0.006
5.25	7.814	71.25	2.170	137.25	0.326	203.25	0.046	269.25	0.006
5.50	9.720	71.50	2.154	137.50	0.324	203.50	0.046	269.50	0.006
5.75	19.726	71.75	2.139	137.75	0.322	203.75	0.045	269.75	0.006
6.00	52.942	72.00	2.123	138.00	0.319	204.00	0.045	270.00	0.006
6.25	67.988	72.25	2.108	138.25	0.317	204.25	0.045	270.25	0.006
6.50	85.386	72.50	2.093	138.50	0.315	204.50	0.044	270.50	0.006
6.75	96.686	72.75	2.078	138.75	0.312	204.75	0.044	270.75	0.006
7.00	107.297	73.00	2.063	139.00	0.310	205.00	0.044	271.00	0.006
7.25	117.169	73.25	2.048	139.25	0.308	205.25	0.043	271.25	0.006
7.50	126.892	73.50	2.033	139.50	0.306	205.50	0.043	271.50	0.006
7.75	136.427	73.75	2.019	139.75	0.303	205.75	0.043	271.75	0.006
8.00	144.516	74.00	2.004	140.00	0.301	206.00	0.042	272.00	0.006
8.25	150.308	74.25	1.990	140.25	0.299	206.25	0.042	272.25	0.006
8.50	156.065	74.50	1.976	140.50	0.297	206.50	0.042	272.50	0.006
8.75	161.697	74.75	1.962	140.75	0.295	206.75	0.042	272.75	0.006
9.00	167.230	75.00	1.948	141.00	0.293	207.00	0.041	273.00	0.006
9.25	172.551	75.25	1.934	141.25	0.291	207.25	0.041	273.25	0.006
9.50	177.536	75.50	1.920	141.50	0.288	207.50	0.041	273.50	0.005
9.75	182.010	75.75	1.906	141.75	0.286	207.75	0.040	273.75	0.005
10.00	185.865	76.00	1.893	142.00	0.284	208.00	0.040	274.00	0.005
10.25	188.136	76.25	1.879	142.25	0.282	208.25	0.040	274.25	0.005
10.50	189.142	76.50	1.866	142.50	0.280	208.50	0.039	274.50	0.005
10.75	189.861	76.75	1.852	142.75	0.278	208.75	0.039	274.75	0.005
11.00	190.369	77.00	1.839	143.00	0.276	209.00	0.039	275.00	0.005
11.25	190.687	77.25	1.826	143.25	0.274	209.25	0.039	275.25	0.005
11.50	190.831	77.50	1.813	143.50	0.272	209.50	0.038	275.50	0.005
11.75	190.809	77.75	1.800	143.75	0.270	209.75	0.038	275.75	0.005
12.00	190.638	78.00	1.787	144.00	0.268	210.00	0.038	276.00	0.005
12.25	189.753	78.25	1.775	144.25	0.266	210.25	0.037	276.25	0.005
12.50	188.672	78.50	1.762	144.50	0.264	210.50	0.037	276.50	0.005
12.75	187.484	78.75	1.750	144.75	0.262	210.75	0.037	276.75	0.005
13.00	186.158	79.00	1.737	145.00	0.260	211.00	0.037	277.00	0.005
13.25	184.692	79.25	1.725	145.25	0.259	211.25	0.036	277.25	0.005
13.50	183.096	79.50	1.713	145.50	0.257	211.50	0.036	277.50	0.005
13.75	181.381	79.75	1.700	145.75	0.255	211.75	0.036	277.75	0.005
14.00	179.560	80.00	1.688	146.00	0.253	212.00	0.035	278.00	0.005
14.25	177.651	80.25	1.676	146.25	0.251	212.25	0.035	278.25	0.005
14.50	175.660	80.50	1.664	146.50	0.249	212.50	0.035	278.50	0.005
14.75	173.596	80.75	1.653	146.75	0.247	212.75	0.035	278.75	0.005
15.00	171.469	81.00	1.641	147.00	0.246	213.00	0.034	279.00	0.005
15.25	169.280	81.25	1.629	147.25	0.244	213.25	0.034	279.25	0.005
15.50	166.973	81.50	1.617	147.50	0.242	213.50	0.034	279.50	0.005
15.75	164.500	81.75	1.606	147.75	0.240	213.75	0.034	279.75	0.005
16.00	161.880	82.00	1.594	148.00	0.239	214.00	0.033	280.00	0.004
16.25	159.133	82.25	1.583	148.25	0.237	214.25	0.033	280.25	0.004
16.50	156.268	82.50	1.572	148.50	0.235	214.50	0.033	280.50	0.004
16.75	153.303	82.75	1.560	148.75	0.233	214.75	0.033	280.75	0.004
17.00	149.826	83.00	1.549	149.00	0.232	215.00	0.032	281.00	0.004
17.25	145.711	83.25	1.538	149.25	0.230	215.25	0.032	281.25	0.004
17.50	141.690	83.50	1.527	149.50	0.228	215.50	0.032	281.50	0.004
17.75	137.768	83.75	1.517	149.75	0.227	215.75	0.032	281.75	0.004
18.00	133.949	84.00	1.506	150.00	0.225	216.00	0.031	282.00	0.004
18.25	130.235	84.25	1.495	150.25	0.223	216.25	0.031	282.25	0.004
18.50	126.613	84.50	1.485	150.50	0.222	216.50	0.031	282.50	0.004
18.75	123.061	84.75	1.474	150.75	0.220	216.75	0.031	282.75	0.004
19.00	119.574	85.00	1.464	151.00	0.218	217.00	0.030	283.00	0.004
19.25	116.168	85.25	1.453	151.25	0.217	217.25	0.030	283.25	0.004
19.50	112.861	85.50	1.443	151.50	0.215	217.50	0.030	283.50	0.004
19.75	109.660	85.75	1.433	151.75	0.214	217.75	0.030	283.75	0.004
20.00	106.566	86.00	1.423	152.00	0.212	218.00	0.030	284.00	0.004
20.25	103.580	86.25	1.413	152.25	0.211	218.25	0.029	284.25	0.004
20.50	100.701	86.50	1.403	152.50	0.209	218.50	0.029	284.50	0.004
20.75	97.924	86.75	1.393	152.75	0.208	218.75	0.029	284.75	0.004
21.00	95.245	87.00	1.383	153.00	0.206	219.00	0.029	285.00	0.004
21.25	92.665	87.25	1.373	153.25	0.205	219.25	0.028	285.25	0.004
21.50	90.179	87.50	1.363	153.50	0.203	219.50	0.028	285.50	0.004
21.75	87.777	87.75	1.354	153.75	0.202	219.75	0.028	285.75	0.004
22.00	85.431	88.00	1.344	154.00	0.200	220.00	0.028	286.00	0.004
22.25	80.967	88.25	1.334	154.25	0.199	220.25	0.028	286.25	0.004
22.50	78.331	88.50	1.325	154.50	0.197	220.50	0.027	286.50	0.004

22.75	75.941	88.75	1.316	154.75	0.196	220.75	0.027	286.75	0.004
23.00	73.744	89.00	1.306	155.00	0.194	221.00	0.027	287.00	0.004
23.25	71.703	89.25	1.297	155.25	0.193	221.25	0.027	287.25	0.004
23.50	69.790	89.50	1.288	155.50	0.191	221.50	0.027	287.50	0.004
23.75	67.982	89.75	1.279	155.75	0.190	221.75	0.026	287.75	0.004
24.00	66.264	90.00	1.270	156.00	0.189	222.00	0.026	288.00	0.004
24.25	64.486	90.25	1.261	156.25	0.187	222.25	0.026	288.25	0.003
24.50	62.514	90.50	1.252	156.50	0.186	222.50	0.026	288.50	0.003
24.75	60.401	90.75	1.243	156.75	0.185	222.75	0.026	288.75	0.003
25.00	58.220	91.00	1.234	157.00	0.183	223.00	0.025	289.00	0.003
25.25	56.022	91.25	1.225	157.25	0.182	223.25	0.025	289.25	0.003
25.50	53.843	91.50	1.217	157.50	0.181	223.50	0.025	289.50	0.003
25.75	51.705	91.75	1.208	157.75	0.179	223.75	0.025	289.75	0.003
26.00	49.626	92.00	1.200	158.00	0.178	224.00	0.025	290.00	0.003
26.25	47.614	92.25	1.191	158.25	0.176	224.25	0.025	290.25	0.003
26.50	45.675	92.50	1.183	158.50	0.175	224.50	0.024	290.50	0.003
26.75	43.814	92.75	1.174	158.75	0.174	224.75	0.024	290.75	0.003
27.00	42.031	93.00	1.166	159.00	0.172	225.00	0.024	291.00	0.003
27.25	40.329	93.25	1.158	159.25	0.171	225.25	0.024	291.25	0.003
27.50	38.706	93.50	1.149	159.50	0.170	225.50	0.024	291.50	0.003
27.75	37.160	93.75	1.141	159.75	0.168	225.75	0.023	291.75	0.003
28.00	35.685	94.00	1.133	160.00	0.167	226.00	0.023	292.00	0.003
28.25	34.280	94.25	1.125	160.25	0.166	226.25	0.023	292.25	0.003
28.50	32.940	94.50	1.117	160.50	0.165	226.50	0.023	292.50	0.003
28.75	31.663	94.75	1.109	160.75	0.164	226.75	0.023	292.75	0.003
29.00	30.445	95.00	1.101	161.00	0.162	227.00	0.023	293.00	0.003
29.25	29.285	95.25	1.094	161.25	0.161	227.25	0.022	293.25	0.003
29.50	28.178	95.50	1.086	161.50	0.160	227.50	0.022	293.50	0.003
29.75	27.122	95.75	1.078	161.75	0.159	227.75	0.022	293.75	0.003
30.00	26.115	96.00	1.071	162.00	0.158	228.00	0.022	294.00	0.003
30.25	25.155	96.25	1.063	162.25	0.156	228.25	0.022	294.25	0.003
30.50	24.240	96.50	1.055	162.50	0.155	228.50	0.022	294.50	0.003
30.75	23.366	96.75	1.048	162.75	0.154	228.75	0.021	294.75	0.003
31.00	22.533	97.00	1.040	163.00	0.153	229.00	0.021	295.00	0.003
31.25	21.738	97.25	1.033	163.25	0.152	229.25	0.021	295.25	0.003
31.50	20.979	97.50	1.026	163.50	0.151	229.50	0.021	295.50	0.003
31.75	20.255	97.75	1.018	163.75	0.150	229.75	0.021	295.75	0.003
32.00	19.563	98.00	1.011	164.00	0.149	230.00	0.021	296.00	0.003
32.25	18.903	98.25	1.004	164.25	0.147	230.25	0.020	296.25	0.003
32.50	18.273	98.50	0.997	164.50	0.146	230.50	0.020	296.50	0.003
32.75	17.671	98.75	0.990	164.75	0.145	230.75	0.020	296.75	0.003
33.00	17.095	99.00	0.983	165.00	0.144	231.00	0.020	297.00	0.003
33.25	16.545	99.25	0.976	165.25	0.143	231.25	0.020	297.25	0.003
33.50	16.020	99.50	0.969	165.50	0.142	231.50	0.020	297.50	0.003
33.75	15.518	99.75	0.962	165.75	0.141	231.75	0.020	297.75	0.003
34.00	15.037	100.00	0.955	166.00	0.140	232.00	0.019	298.00	0.003
34.25	14.578	100.25	0.948	166.25	0.139	232.25	0.019	298.25	0.003
34.50	14.139	100.50	0.942	166.50	0.138	232.50	0.019	298.50	0.003
34.75	13.719	100.75	0.935	166.75	0.137	232.75	0.019	298.75	0.003
35.00	13.317	101.00	0.928	167.00	0.136	233.00	0.019	299.00	0.002
35.25	12.933	101.25	0.922	167.25	0.135	233.25	0.019	299.25	0.002
35.50	12.565	101.50	0.915	167.50	0.134	233.50	0.019	299.50	0.002
35.75	12.213	101.75	0.909	167.75	0.133	233.75	0.018	299.75	0.002
36.00	11.876	102.00	0.902	168.00	0.132	234.00	0.018	300.00	0.002
36.25	11.554	102.25	0.896	168.25	0.131	234.25	0.018	300.25	0.002
36.50	11.245	102.50	0.889	168.50	0.130	234.50	0.018	300.50	0.002
36.75	10.950	102.75	0.883	168.75	0.129	234.75	0.018	300.75	0.002
37.00	10.667	103.00	0.877	169.00	0.128	235.00	0.018	301.00	0.002
37.25	10.396	103.25	0.870	169.25	0.127	235.25	0.018	301.25	0.002
37.50	10.137	103.50	0.864	169.50	0.126	235.50	0.018	301.50	0.002
37.75	9.888	103.75	0.858	169.75	0.125	235.75	0.017	301.75	0.002
38.00	9.650	104.00	0.852	170.00	0.124	236.00	0.017	302.00	0.002
38.25	9.421	104.25	0.846	170.25	0.124	236.25	0.017	302.25	0.002
38.50	9.202	104.50	0.840	170.50	0.123	236.50	0.017	302.50	0.002
38.75	8.992	104.75	0.834	170.75	0.122	236.75	0.017	302.75	0.002
39.00	8.790	105.00	0.828	171.00	0.121	237.00	0.017	303.00	0.002
39.25	8.596	105.25	0.822	171.25	0.120	237.25	0.017	303.25	0.002
39.50	8.410	105.50	0.816	171.50	0.119	237.50	0.016	303.50	0.002
39.75	8.231	105.75	0.810	171.75	0.118	237.75	0.016	303.75	0.002
40.00	8.059	106.00	0.804	172.00	0.117	238.00	0.016	304.00	0.002
40.25	7.894	106.25	0.798	172.25	0.116	238.25	0.016	304.25	0.002
40.50	7.735	106.50	0.793	172.50	0.116	238.50	0.016	304.50	0.002
40.75	7.581	106.75	0.787	172.75	0.115	238.75	0.016	304.75	0.002
41.00	7.434	107.00	0.781	173.00	0.114	239.00	0.016	305.00	0.002
41.25	7.292	107.25	0.776	173.25	0.113	239.25	0.016	305.25	0.002
41.50	7.155	107.50	0.770	173.50	0.112	239.50	0.015	305.50	0.002
41.75	7.023	107.75	0.765	173.75	0.111	239.75	0.015	305.75	0.002
42.00	6.896	108.00	0.759	174.00	0.111	240.00	0.015	306.00	0.002
42.25	6.773	108.25	0.754	174.25	0.110	240.25	0.015	306.25	0.002
42.50	6.654	108.50	0.749	174.50	0.109	240.50	0.015	306.50	0.002
42.75	6.540	108.75	0.743	174.75	0.108	240.75	0.015	306.75	0.002
43.00	6.429	109.00	0.738	175.00	0.107	241.00	0.015	307.00	0.002
43.25	6.323	109.25	0.733	175.25	0.107	241.25	0.015	307.25	0.002
43.50	6.219	109.50	0.727	175.50	0.106	241.50	0.015	307.50	0.002

43.75	6.119	109.75	0.722	175.75	0.105	241.75	0.014	307.75	0.002
44.00	6.023	110.00	0.717	176.00	0.104	242.00	0.014	308.00	0.002
44.25	5.929	110.25	0.712	176.25	0.103	242.25	0.014	308.25	0.002
44.50	5.839	110.50	0.707	176.50	0.103	242.50	0.014	308.50	0.002
44.75	5.751	110.75	0.702	176.75	0.102	242.75	0.014	308.75	0.002
45.00	5.666	111.00	0.697	177.00	0.101	243.00	0.014	309.00	0.002
45.25	5.584	111.25	0.692	177.25	0.100	243.25	0.014	309.25	0.002
45.50	5.504	111.50	0.687	177.50	0.099	243.50	0.014	309.50	0.002
45.75	5.427	111.75	0.682	177.75	0.099	243.75	0.014	309.75	0.002
46.00	5.352	112.00	0.677	178.00	0.098	244.00	0.013	310.00	0.002
46.25	5.279	112.25	0.672	178.25	0.097	244.25	0.013	310.25	0.002
46.50	5.208	112.50	0.667	178.50	0.096	244.50	0.013	310.50	0.002
46.75	5.139	112.75	0.663	178.75	0.096	244.75	0.013	310.75	0.002
47.00	5.072	113.00	0.658	179.00	0.095	245.00	0.013	311.00	0.002
47.25	5.007	113.25	0.653	179.25	0.094	245.25	0.013	311.25	0.002
47.50	4.944	113.50	0.649	179.50	0.094	245.50	0.013	311.50	0.002
47.75	4.883	113.75	0.644	179.75	0.093	245.75	0.013	311.75	0.002
48.00	4.823	114.00	0.639	180.00	0.092	246.00	0.013	312.00	0.002
48.25	4.764	114.25	0.635	180.25	0.092	246.25	0.013	312.25	0.002
48.50	4.707	114.50	0.630	180.50	0.091	246.50	0.012	312.50	0.002
48.75	4.652	114.75	0.626	180.75	0.090	246.75	0.012	312.75	0.002
49.00	4.598	115.00	0.621	181.00	0.090	247.00	0.012	313.00	0.002
49.25	4.545	115.25	0.617	181.25	0.089	247.25	0.012	313.25	0.002
49.50	4.494	115.50	0.612	181.50	0.088	247.50	0.012	313.50	0.002
49.75	4.444	115.75	0.608	181.75	0.088	247.75	0.012	313.75	0.002
50.00	4.395	116.00	0.604	182.00	0.087	248.00	0.012	314.00	0.002
50.25	4.347	116.25	0.599	182.25	0.086	248.25	0.012	314.25	0.002
50.50	4.300	116.50	0.595	182.50	0.086	248.50	0.012	314.50	0.002
50.75	4.254	116.75	0.591	182.75	0.085	248.75			

64.75	2.629	130.75	0.394	196.75	0.056	262.75	0.008	328.75	0.001
65.00	2.609	131.00	0.391	197.00	0.056	263.00	0.008		
65.25	2.589	131.25	0.388	197.25	0.055	263.25	0.007		
65.50	2.570	131.50	0.386	197.50	0.055	263.50	0.007		
65.75	2.551	131.75	0.383	197.75	0.054	263.75	0.007		

ADD HYD (8188)				
1 + 2 = 3				
	AREA	OPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8186):	22258.88	154.944	14.00	36.78
+ ID2= 2 (8190):	20350.16	145.888	14.25	37.29

ID = 3 (8188):	42609.04	300.772	14.25	37.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (4462)			
ID= 1 DT=15.0 min	Area (ha)=1238.89	Curve Number (CN)= 60.9	# of Linear Res. (N)= 1.50
	Ia (mm)= 5.00		
	U.H. Tp(hrs)= 4.02		

Unit Hyd Qpeak (cms)=	5.256
PEAK FLOW (cms)=	5.487 (i)
TIME TO PEAK (hrs)=	12.000
RUNOFF VOLUME (mm)=	28.240
TOTAL RAINFALL (mm)=	88.500
RUNOFF COEFFICIENT =	0.319

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (4461)			
ID= 1 DT=15.0 min	Area (ha)= 236.12	Dir. Conn. (%)= 17.00	
	Total Imp(%)= 47.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	110.98	125.14
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1254.64	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	116.82	91.84
over (min)=	15.00	30.00
Storage Coeff. (min)=	13.48 (ii)	24.55 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04
TOTALS		
PEAK FLOW (cms)=	9.91	14.74
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	86.50	38.41
TOTAL RAINFALL (mm)=	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 60.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5446)				
IN= 2----> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	10.1800	6.2621
	0.8000	2.9957	12.5050	7.1218
	3.0350	3.4813	14.4090	7.9633
	5.6510	4.4353	14.8090	7.9733
	7.7470	5.2683	0.0000	0.0000
	AREA	OPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)

INFLOW : ID= 2 (4461)	236.120	19.333	6.00	46.59
OUTFLOW: ID= 1 (5446)	236.120	7.563	7.00	46.59

PEAK FLOW REDUCTION [Qout/Qin] (%) = 39.12
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 5.2556

ADD HYD (8192)				
1 + 2 = 3				
	AREA	OPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (4462):	1238.89	5.487	12.00	28.24
+ ID2= 2 (5446):	236.12	7.563	7.00	46.59

ID = 3 (8192):	1475.01	10.736	7.00	31.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8194)				
1 + 2 = 3				
	AREA	OPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8188):	42609.04	300.772	14.25	37.02
+ ID2= 2 (8192):	1475.01	10.736	7.00	31.18

ID = 3 (8194):	44084.05	306.544	14.25	36.83

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0442)			
ID= 1 DT=15.0 min	Area (ha)= 117.26	Curve Number (CN)= 43.0	# of Linear Res. (N)= 1.50
	Ia (mm)= 5.00		
	U.H. Tp(hrs)= 1.17		

Unit Hyd Qpeak (cms)=	1.705
PEAK FLOW (cms)=	0.783 (i)
TIME TO PEAK (hrs)=	7.750
RUNOFF VOLUME (mm)=	16.459
TOTAL RAINFALL (mm)=	88.500
RUNOFF COEFFICIENT =	0.186

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0440)			
ID= 1 DT=15.0 min	Area (ha)= 226.35	Curve Number (CN)= 60.0	# of Linear Res. (N)= 1.50
	Ia (mm)= 5.00		
	U.H. Tp(hrs)= 1.11		

Unit Hyd Qpeak (cms)=	3.481
PEAK FLOW (cms)=	2.661 (i)
TIME TO PEAK (hrs)=	7.500
RUNOFF VOLUME (mm)=	27.335
TOTAL RAINFALL (mm)=	88.500
RUNOFF COEFFICIENT =	0.309

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0438)			
ID= 1 DT=15.0 min	Area (ha)= 130.70	Curve Number (CN)= 51.0	# of Linear Res. (N)= 1.50
	Ia (mm)= 5.00		
	U.H. Tp(hrs)= 0.60		

Unit Hyd Qpeak (cms)=	3.738
PEAK FLOW (cms)=	1.886 (i)
TIME TO PEAK (hrs)=	6.750
RUNOFF VOLUME (mm)=	20.821
TOTAL RAINFALL (mm)=	88.500
RUNOFF COEFFICIENT =	0.235

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB (0436) Area (ha)= 187.51 Curve Number (CN)= 55.0
 NASHYD (0436) Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 ID= 1 DT=15.0 min U. H. Tp(hrs)= 0.73

Unit Hyd Qpeak (cms)= 4.391

PEAK FLOW (cms)= 2.626 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 23.543
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.266

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB (0434) Area (ha)= 56.64 Curve Number (CN)= 46.0
 NASHYD (0434) Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 ID= 1 DT=15.0 min U. H. Tp(hrs)= 0.52

Unit Hyd Qpeak (cms)= 1.867

PEAK FLOW (cms)= 0.782 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 17.775
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.201

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8206) 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 ID1= 1 (0434): 56.64 0.782 6.50 17.78
 + ID2= 2 (0436): 187.51 2.626 7.00 23.54

 ID = 3 (8206): 244.15 3.390 6.75 22.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6438) IN= 2---> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (4381.0) ----->

Distance	Elevation	Manning	
0.00	256.00	0.0600	
9.23	255.50	0.0600	
27.69	255.00	0.0600	
50.77	254.90	0.0600	
64.62	254.85	0.0600	
78.47	252.26	0.0600	
96.93	249.44	0.0600	
129.24	245.94	0.0600	
145.40	245.76	0.0600	
146.20	245.80	0.0600 / 0.0350	Main Channel
146.70	245.30	0.0350	Main Channel
147.70	245.30	0.0350	Main Channel
148.20	245.80	0.0350 / 0.0600	Main Channel
150.01	245.78	0.0600	
152.32	245.79	0.0600	
163.86	246.72	0.0600	
175.40	249.07	0.0600	
186.94	253.15	0.0600	
196.17	255.38	0.0600	
228.48	255.55	0.0600	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	245.80	.221E+04	0.9	1.02	40.44
1.01	246.31	.352E+05	14.0	0.99	41.77
1.53	246.83	.841E+05	47.3	1.39	29.61
2.04	247.34	.144E+06	102.0	1.76	23.49
2.55	247.85	.213E+06	177.3	2.07	19.99
3.07	248.37	.291E+06	274.4	2.34	17.66

3.58	248.88	.378E+06	394.6	2.58	15.97
4.09	249.39	.474E+06	541.4	2.83	14.60
4.61	249.91	.578E+06	719.9	3.08	13.38
5.12	250.42	.688E+06	923.8	3.33	12.40
5.63	250.93	.803E+06	1153.3	3.56	11.61
6.14	251.44	.925E+06	1408.6	3.77	10.95
6.66	251.96	.105E+07	1690.6	3.98	10.38
7.17	252.47	.119E+07	2002.3	4.18	9.88
7.68	252.98	.133E+07	2345.4	4.38	9.43
8.20	253.50	.147E+07	2710.1	4.56	9.05
8.71	254.01	.162E+07	3101.0	4.73	8.72
9.22	254.52	.178E+07	3521.8	4.90	8.43
9.74	255.04	.195E+07	3467.6	4.39	9.39

<---- hydrograph ----> <-pi pe / channel ->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8206) 244.15 3.39 6.75 22.21 0.60 1.01
 OUTFLOW: ID= 1 (6438) 244.15 2.96 7.75 22.21 0.58 1.02

 ADD HYD (8208) 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 ID1= 1 (0438): 130.70 1.886 6.75 20.82
 + ID2= 2 (6438): 244.15 2.963 7.75 22.21

 ID = 3 (8208): 374.85 4.587 7.25 21.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8210) 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 ID1= 1 (0440): 226.35 2.661 7.50 27.34
 + ID2= 2 (8208): 374.85 4.587 7.25 21.72

 ID = 3 (8210): 601.20 7.236 7.50 23.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB (0432) Area (ha)= 114.21 Curve Number (CN)= 53.0
 NASHYD (0432) Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 ID= 1 DT=15.0 min U. H. Tp(hrs)= 1.21

Unit Hyd Qpeak (cms)= 1.617

PEAK FLOW (cms)= 1.026 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 22.408
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.253

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB (0430) Area (ha)= 111.64 Curve Number (CN)= 55.0
 NASHYD (0430) Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 ID= 1 DT=15.0 min U. H. Tp(hrs)= 0.52

Unit Hyd Qpeak (cms)= 3.687

PEAK FLOW (cms)= 2.049 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 23.287
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.263

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB

NASHYD (0428) Area (ha)= 50.53 Curve Number (CN)= 51.0
 ID= 1 DT=15.0 min I a (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.50

Unit Hyd Qpeak (cms)= 1.725

PEAK FLOW (cms)= 0.841 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 20.682
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.234

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0426) Area (ha)= 247.17 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min I a (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.98

Unit Hyd Qpeak (cms)= 4.315

PEAK FLOW (cms)= 3.300 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 28.056
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.317

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0424) Area (ha)= 49.57 Curve Number (CN)= 59.0
 ID= 1 DT=15.0 min I a (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.37

Unit Hyd Qpeak (cms)= 2.293

PEAK FLOW (cms)= 1.282 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 25.630
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.290

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	(8198)	AREA	OPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0424):	49.57	1.282	6.50	25.63	
+ ID2= 2 (0426):	247.17	3.300	7.25	28.06	
ID = 3 (8198):	296.74	4.328	6.75	27.65	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6428) Routing time step (min)'= 15.00
 IN= 2--> OUT= 1

<----- DATA FOR SECTION (4281.0) ----->			
Distance	Elevation	Manning	
0.00	246.54	0.0900	
8.80	246.29	0.0900	
30.78	246.03	0.0900	
41.78	243.97	0.0900	
63.77	237.16	0.0900	
85.76	232.53	0.0900	
96.75	230.16	0.0900	
109.94	228.40	0.0900	
112.14	228.28	0.0900 / 0.0450	Main Channel
112.84	228.00	0.0450	Main Channel
113.34	227.50	0.0450	Main Channel
114.34	227.50	0.0450	Main Channel
114.84	228.00	0.0450	Main Channel
116.54	228.29	0.0450 / 0.0900	Main Channel
118.74	228.40	0.0900	
120.94	228.52	0.0900	

149.52	229.51	0.0900
169.31	230.69	0.0900
186.00	234.27	0.0900
217.69	243.56	0.0900

<----- TRAVEL TIME TABLE ----->						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME	
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)	
0.78	228.28	205E+04	1.6	0.97	21.60	
1.58	229.08	202E+05	16.6	1.02	20.37	
2.39	229.89	665E+05	64.8	1.22	17.11	
3.19	230.69	133E+06	158.1	1.49	14.05	
4.00	231.50	213E+06	308.6	1.82	11.48	
4.80	232.30	299E+06	504.9	2.11	9.89	
5.61	233.11	394E+06	746.6	2.37	8.79	
6.41	233.91	496E+06	1034.5	2.61	7.99	
7.21	234.71	605E+06	1373.3	2.84	7.35	
8.02	235.52	722E+06	1763.3	3.06	6.82	
8.82	236.32	844E+06	2202.0	3.27	6.39	
9.63	237.13	974E+06	2690.5	3.46	6.03	
10.43	237.93	111E+07	3239.5	3.66	5.71	
11.23	238.73	125E+07	3839.0	3.84	5.43	
12.04	239.54	140E+07	4489.1	4.03	5.18	
12.84	240.34	155E+07	5190.5	4.20	4.97	
13.65	241.15	170E+07	5943.8	4.36	4.78	
14.45	241.95	187E+07	6749.8	4.53	4.61	
15.26	242.76	204E+07	7609.4	4.68	4.46	

<----- hydrograph ----->							<- pi pe / channel ->	
	AREA	OPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL		
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW: ID= 2 (8198)	296.74	4.33	6.75	27.65	0.93	0.98		
OUTFLOW: ID= 1 (6428)	296.74	4.16	7.25	27.65	0.92	0.98		

ADD HYD	(8200)	AREA	OPEAK	TPEAK	R.V.
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0428):		50.53	0.841	6.50	20.68
+ ID2= 2 (6428):		296.74	4.156	7.25	27.65
ID = 3 (8200):		347.27	4.864	7.25	26.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8202)	AREA	OPEAK	TPEAK	R.V.
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0430):		111.64	2.049	6.50	23.29
+ ID2= 2 (8200):		347.27	4.864	7.25	26.64
ID = 3 (8202):		458.91	6.730	7.00	25.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6432) Routing time step (min)'= 15.00
 IN= 2--> OUT= 1

<----- DATA FOR SECTION (4321.0) ----->			
Distance	Elevation	Manning	
0.00	221.00	0.0600	
15.29	219.85	0.0600	
45.86	219.86	0.0600	
76.44	219.96	0.0600	
110.84	220.23	0.0600	
112.00	220.15	0.0600	
114.66	220.09	0.0600	
118.48	219.73	0.0600	
120.80	219.70	0.0600 / 0.0350	Main Channel
121.30	219.20	0.0350	Main Channel
122.30	219.20	0.0350	Main Channel
122.80	219.70	0.0350 / 0.0600	Main Channel
122.95	220.11	0.0600	
129.95	220.11	0.0600	
175.81	220.24	0.0600	
214.03	220.55	0.0600	
252.25	220.61	0.0600	

290.47	220.70	0.0600
328.69	221.04	0.0600
347.80	221.08	0.0600
378.37	221.04	0.0600

TRAVEL TIME TABLE						
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)	
0.08	219.28	.185E+03	0.0	0.23	149.38	
0.17	219.37	.399E+03	0.1	0.34	100.62	
0.25	219.45	.641E+03	0.1	0.42	80.98	
0.33	219.53	.911E+03	0.2	0.49	69.79	
0.42	219.62	.121E+04	0.3	0.55	62.35	
0.50	219.70	.154E+04	0.5	0.60	56.91	
0.61	219.81	.267E+04	0.7	0.56	61.28	
0.71	219.91	.901E+04	1.3	0.30	112.96	
0.82	220.02	.249E+05	3.4	0.28	122.71	
0.92	220.12	.452E+05	6.8	0.31	109.98	
1.03	220.23	.747E+05	12.2	0.33	102.05	
1.14	220.34	.112E+06	21.3	0.39	87.59	
1.24	220.44	.153E+06	33.0	0.44	77.01	
1.35	220.55	.196E+06	47.2	0.49	69.22	
1.46	220.66	.248E+06	62.3	0.51	66.48	
1.56	220.76	.310E+06	83.4	0.55	62.06	
1.67	220.87	.376E+06	108.8	0.59	57.56	
1.77	220.97	.444E+06	137.7	0.64	53.77	
1.88	221.08	.517E+06	163.6	0.65	52.70	

<---- hydrograph ---->							<- pi pe / channel ->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)		
INFLOW: ID= 2 (8202)	458.91	6.73	7.00	25.82	0.92	0.31		
OUTFLOW: ID= 1 (6432)	458.91	4.41	9.00	25.82	0.85	0.29		

ADD HYD (8204)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0432):	114.21	1.026	7.75	22.41
+ ID2= 2 (6432):	458.91	4.414	9.00	25.82
ID = 3 (8204):	573.12	5.371	8.75	25.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8212)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8204):	573.12	5.371	8.75	25.14
+ ID2= 2 (8210):	601.20	7.236	7.50	23.84
ID = 3 (8212):	1174.32	12.072	8.00	24.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6442)
IN= 2--> OUT= 1

<----- DATA FOR SECTION (4421.0) ----->			
Distance	Elevation	Manning	
0.00	221.00	0.0350	
26.73	220.80	0.0350	
53.47	220.60	0.0350	
66.83	220.40	0.0350	
80.20	220.20	0.0350	
347.52	220.00	0.0350	
354.21	219.59	0.0350	
360.89	219.50	0.0350	
367.57	219.08	0.0350	
368.76	219.00	0.0350 / 0.0300	Main Channel
369.26	218.50	0.0300	Main Channel
374.26	218.50	0.0300	Main Channel
374.76	219.00	0.0300 / 0.0350	Main Channel
380.94	220.33	0.0350	
387.62	219.62	0.0350	
394.31	219.23	0.0350	

454.45	219.07	0.0350
514.60	219.09	0.0350
660.00	219.19	0.0350
661.63	221.00	0.0350

TRAVEL TIME TABLE						
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)	
0.12	218.62	.100E+04	0.2	0.28	92.89	
0.25	218.75	.205E+04	0.6	0.43	60.09	
0.37	218.87	.315E+04	1.1	0.55	46.99	
0.50	219.00	.430E+04	1.8	0.66	39.67	
0.63	219.13	.141E+05	3.4	0.37	69.99	
0.77	219.27	.652E+05	13.7	0.33	79.14	
0.90	219.40	.124E+06	35.5	0.45	58.02	
1.03	219.53	.183E+06	65.7	0.56	46.45	
1.17	219.67	.245E+06	103.8	0.66	39.30	
1.30	219.80	.307E+06	149.3	0.76	34.30	
1.43	219.93	.371E+06	201.6	0.85	30.65	
1.57	220.07	.440E+06	256.5	0.91	28.56	
1.70	220.20	.541E+06	325.7	0.94	27.70	
1.83	220.33	.663E+06	415.2	0.98	26.62	
1.97	220.47	.787E+06	520.3	1.03	25.21	
2.10	220.60	.913E+06	638.5	1.09	23.82	
2.23	220.73	.104E+07	767.2	1.15	22.62	
2.37	220.87	.117E+07	908.0	1.21	21.54	
2.50	221.00	.131E+07	1060.7	1.27	20.57	

<---- hydrograph ---->							<- pi pe / channel ->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)		
INFLOW: ID= 2 (8212)	1174.32	12.07	8.00	24.47	0.75	0.34		
OUTFLOW: ID= 1 (6442)	1174.32	10.36	9.50	24.47	0.72	0.34		

ADD HYD (8214)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0442):	117.26	0.783	7.75	16.46
+ ID2= 2 (6442):	1174.32	10.360	9.50	24.47
ID = 3 (8214):	1291.58	11.029	9.50	23.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8196)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8194):	44084.05	306.544	14.25	36.83
+ ID2= 2 (8214):	1291.58	11.029	9.50	23.75
ID = 3 (8196):	45375.62	312.400	13.75	36.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHYD (0444)
ID= 1 DT=15.0 min

Area (ha)= 221.65
Ia (mm)= 5.00
U. H. Tp(hrs)= 1.03

Curve Number (CN)= 56.0
of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 3.656

PEAK FLOW (cms)= 2.441 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 24.392
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.276

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8216)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)

ID1= 1 (0444):	221.65	2.441	7.50	24.39
+ ID2= 2 (8196):	45375.62	312.400	13.75	36.45
=====				
ID = 3 (8216):	45597.28	313.039	13.75	36.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH
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V V I SSSS U U A L
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***** DETAILED OUTPUT *****

Input filename: C:\Program Files (x86)\Visual Otthymo 2.4\VO2\vo1n.dat
 Output filename: C:\Users\aman1\cu\AppData\Local\Temp\ab70bd54-a452-450c-b03c-83cdc4826a98\Scenario.out
 Summary filename: C:\Users\aman1\cu\AppData\Local\Temp\ab70bd54-a452-450c-b03c-83cdc4826a98\Scenario.sum

DATE: 08/29/2012 TIME: 01:53:59

USER:

COMMENTS: _____

 ** SIMULATION NUMBER: 1 **

READ STORM File name: C:\Users\aman1\cu\AppData\Local\Temp\ab70bd54-a452-450c-b03c-83cdc4826a98\477750dd
 Ptotal = 42.00 mm Comments: 2-Year 12-Hour SCS II Design Storm

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.05	3.25	1.68	6.25	7.56	9.25	1.47
0.50	1.05	3.50	1.68	6.50	7.56	9.50	1.47
0.75	1.05	3.75	1.68	6.75	3.36	9.75	1.47
1.00	1.05	4.00	1.68	7.00	3.36	10.00	1.47
1.25	1.05	4.25	2.52	7.25	2.52	10.25	0.84
1.50	1.05	4.50	2.52	7.50	2.52	10.50	0.84
1.75	1.05	4.75	3.36	7.75	2.52	10.75	0.84
2.00	1.05	5.00	3.36	8.00	2.52	11.00	0.84
2.25	1.26	5.25	5.04	8.25	1.47	11.25	0.84
2.50	1.26	5.50	5.04	8.50	1.47	11.50	0.84
2.75	1.26	5.75	20.16	8.75	1.47	11.75	0.84
3.00	1.26	6.00	55.44	9.00	1.47	12.00	0.84

CALIB NASHYD (0356) Area (ha)=1015.22 Curve Number (CN)= 46.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 1.37

Unit Hyd Qpeak (cms)= 12.651

PEAK FLOW (cms)= 1.461 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 4.058
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.097

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0352) Area (ha)= 381.43 Curve Number (CN)= 54.0

ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.82

Unit Hyd Qpeak (cms)= 7.980

PEAK FLOW (cms)= 1.066 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 5.329
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.127

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0350) Area (ha)= 366.84 Curve Number (CN)= 48.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 5.831

PEAK FLOW (cms)= 0.676 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 4.345
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.103

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0348) Area (ha)= 590.18 Curve Number (CN)= 48.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 1.91

Unit Hyd Qpeak (cms)= 5.267

PEAK FLOW (cms)= 0.711 (i)
 TIME TO PEAK (hrs)= 9.250
 RUNOFF VOLUME (mm)= 4.368
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.104

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6350) Routing time step (min)'= 15.00
 IN= 2---> OUT= 1

<----- DATA FOR SECTION (3501.0) ----->

Distance	Elevation	Manning	
0.00	287.00	0.0500	
23.66	283.72	0.0500	
70.98	280.94	0.0500	
112.38	280.23	0.0500	
171.52	276.80	0.0500	
260.24	274.46	0.0500	
266.16	274.26	0.0500	
272.07	274.12	0.0500	
276.49	274.12	0.0500 / 0.0300	Main Channel
276.99	273.82	0.0300	Main Channel
277.99	273.82	0.0300	Main Channel
279.49	273.82	0.0300	Main Channel
280.49	274.27	0.0300 / 0.0500	Main Channel
283.90	274.27	0.0500	
289.81	274.57	0.0500	
325.30	275.75	0.0500	
396.28	278.98	0.0500	
449.51	280.97	0.0500	
496.83	283.90	0.0500	
585.54	287.92	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.30	274.12	.273E+04	0.9	0.99	49.81
0.98	274.80	.583E+05	20.8	1.05	46.71
1.66	275.48	.203E+06	95.2	1.38	35.53
2.33	276.15	.438E+06	256.0	1.73	28.50
3.01	276.83	.755E+06	524.8	2.05	23.97
3.69	277.51	.114E+07	935.8	2.43	20.26

4.37	278.19	.157E+07	1472.0	2.76	17.82
5.05	278.87	.206E+07	2140.7	3.06	16.06
5.72	279.54	.261E+07	2938.6	3.33	14.79
6.40	280.22	.321E+07	3887.6	3.57	13.77
7.08	280.90	.390E+07	4795.4	3.63	13.56
7.76	281.58	.468E+07	6196.2	3.91	12.58
8.43	282.25	.550E+07	7803.9	4.19	11.74
9.11	282.93	.637E+07	9601.9	4.45	11.05
9.79	283.61	.728E+07	11594.8	4.70	10.46
10.47	284.29	.823E+07	13845.3	4.96	9.91
11.15	284.97	.922E+07	16293.4	5.21	9.43
11.82	285.64	.103E+08	18952.0	5.45	9.02
12.50	286.32	.113E+08	21824.3	5.69	8.65

----- hydrograph -----							-<- pi pe / channel ->	
	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL		
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW : ID= 2 (0348)	590.18	0.71	9.25	4.37	0.23	0.99		
OUTFLOW: ID= 1 (6350)	590.18	0.70	10.25	4.37	0.23	0.99		

ADD HYD (8220)					
1 + 2 = 3					
ID1= 1 (0350):	366.84	0.676	7.75	4.35	
+ ID2= 2 (6350):	590.18	0.695	10.25	4.37	

ID = 3 (8220):	957.02	1.262	8.75	4.36	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (3461)			
ID= 1 DT=15.0 min			
Area	(ha)= 33.62		
Total Imp(%)	= 61.00	Dir. Conn.(%)	= 38.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)=	20.51		13.11
Dep. Storage (mm)=	2.00		5.00
Average Slope (%)=	0.50		0.50
Length (m)=	473.43		40.00
Mannings n =	0.013		0.250

Max. Eff. Inten. (mm/hr)=	55.44	20.73
over (min)	15.00	45.00
Storage Coeff. (min)=	10.12 (ii)	30.20 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.09	0.03

TOTALS		
PEAK FLOW (cms)=	1.66	0.46
TIME TO PEAK (hrs)=	6.00	6.00
RUNOFF VOLUME (mm)=	40.00	13.98
TOTAL RAINFALL (mm)=	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.33
		1.848 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5346)			
IN= 2----> OUT= 1			
DT= 15.0 min			
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.0090	0.6158
0.2450	0.2524	2.6260	0.6610
0.6230	0.3644	3.2370	0.7019
0.9920	0.4690	3.6370	0.7119
1.2510	0.5428	0.0000	0.0000
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3461)	33.620	1.848	23.87
OUTFLOW: ID= 1 (5346)	33.620	0.559	6.75

PEAK FLOW REDUCTION [Qout/Qin] (%) = 30.27
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha.m.) = 0.3470

CALIB NASHYD (3462)			
ID= 1 DT=15.0 min			
Area (ha)=	317.31	Curve Number (CN)=	71.0
Ia (mm)=	5.00	# of Linear Res. (N)=	1.50
U. H. Tp (hrs)=	0.96		

Unit Hyd Qpeak (cms) = 5.625

PEAK FLOW (cms)=	1.434 (i)
TIME TO PEAK (hrs)=	7.250
RUNOFF VOLUME (mm)=	9.622
TOTAL RAINFALL (mm)=	42.000
RUNOFF COEFFICIENT =	0.229

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9346)				
1 + 2 = 3				
ID1= 1 (3462):	317.31	1.434	7.25	9.62
+ ID2= 2 (5346):	33.62	0.559	6.75	23.86

ID = 3 (9346):	350.93	1.952	7.00	10.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8222)				
1 + 2 = 3				
ID1= 1 (8220):	957.02	1.262	8.75	4.36
+ ID2= 2 (9346):	350.93	1.952	7.00	10.99

ID = 3 (8222):	1307.95	2.990	7.50	6.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6352)	
IN= 2----> OUT= 1	
Routing time step (min)' = 15.00	

<----- DATA FOR SECTION (3521.0) ----->		
Distance	Elevation	Manning
0.00	257.95	0.0500
7.83	257.34	0.0500
15.66	256.19	0.0500
21.53	254.01	0.0500
41.11	244.06	0.0500
76.35	241.38	0.0500
111.58	239.74	0.0500
113.75	239.61	0.0500
113.85	238.81	0.0500 / 0.0300
115.50	238.81	0.0300
116.15	238.81	0.0300
116.25	239.41	0.0300
117.46	239.52	0.0500
119.41	239.72	0.0500
121.37	240.04	0.0500
131.16	241.84	0.0500
156.61	247.03	0.0500
176.19	251.46	0.0500
193.80	258.79	0.0500

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.60	239.41	.224E+04	2.2	1.52	17.12
1.58	240.39	.216E+05	22.3	1.61	16.16
2.55	241.36	.804E+05	101.0	1.96	13.27
3.53	242.34	.173E+06	275.6	2.48	10.46
4.50	243.31	.293E+06	553.9	2.95	8.80
5.48	244.29	.438E+06	961.8	3.42	7.60

6.45	245.26	.599E+06	1534.4	3.99	6.51
7.43	246.24	.770E+06	2228.4	4.51	5.76
8.41	247.22	.951E+06	3043.6	4.98	5.21
9.38	248.19	.114E+07	3982.1	5.43	4.78
10.36	249.17	.134E+07	5040.1	5.85	4.44
11.33	250.14	.155E+07	6218.5	6.24	4.16
12.31	251.12	.177E+07	7518.6	6.61	3.93
13.29	252.10	.200E+07	8969.0	6.98	3.72
14.26	253.07	.224E+07	10554.1	7.35	3.53
15.24	254.05	.248E+07	12257.1	7.71	3.37
16.21	255.02	.272E+07	14045.1	8.03	3.23
17.19	256.00	.298E+07	15954.8	8.34	3.11
18.16	256.97	.325E+07	17780.4	8.53	3.04

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8222)	1307.95	2.99	7.50	6.14	0.64	1.52
OUTFLOW: ID= 1 (6352)	1307.95	2.96	8.00	6.14	0.64	1.52

ADD HYD (8224)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0352):	381.43	1.066	7.00	5.33
+ ID2= 2 (6352):	1307.95	2.963	8.00	6.14
ID = 3 (8224):	1689.38	3.984	7.75	5.95

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6354)	Routing time step (min) = 15.00
IN= 2---> OUT= 1	

<----- DATA FOR SECTION (3541.0) ----->

Distance	Elevation	Manning	
0.00	253.92	0.0500	
7.95	251.83	0.0500	
15.89	249.97	0.0500	
19.87	249.05	0.0500	
47.68	242.00	0.0500	
67.55	237.10	0.0500	
83.44	231.11	0.0500	
85.43	230.75	0.0500	
85.66	230.61	0.0500 / 0.0300	Main Channel
85.76	230.01	0.0300	Main Channel
87.41	230.01	0.0300	Main Channel
88.06	230.01	0.0300	Main Channel
88.16	230.61	0.0300 / 0.0500	Main Channel
89.40	230.72	0.0500	
91.39	230.88	0.0500	
93.37	231.03	0.0500	
133.11	233.96	0.0500	
150.99	235.39	0.0500	
172.84	239.03	0.0500	
196.68	250.00	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.60	230.61	.451E+04	1.5	1.06	49.40
1.62	231.63	.394E+05	14.6	1.16	45.13
2.64	232.65	.128E+06	58.8	1.44	36.30
3.66	233.67	.269E+06	149.7	1.74	30.00
4.68	234.69	.463E+06	301.5	2.04	25.58
5.70	235.71	.705E+06	530.5	2.36	22.14
6.72	236.73	.980E+06	848.4	2.71	19.25
7.74	237.75	.128E+07	1237.9	3.02	17.29
8.76	238.77	.162E+07	1706.7	3.30	15.83
9.78	239.79	.199E+07	2291.9	3.61	14.45
10.81	240.82	.237E+07	2973.7	3.92	13.30
11.83	241.84	.278E+07	3742.2	4.21	12.38
12.85	242.86	.321E+07	4599.1	4.49	11.62
13.87	243.88	.365E+07	5544.6	4.75	10.98
14.89	244.90	.412E+07	6579.7	5.00	10.44
15.91	245.92	.461E+07	7705.9	5.23	9.97
16.93	246.94	.512E+07	8924.6	5.46	9.55
17.95	247.96	.564E+07	10237.5	5.68	9.19
18.97	248.98	.619E+07	11646.2	5.89	8.86

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8224)	1689.38	3.98	7.75	5.95	0.79	1.07
OUTFLOW: ID= 1 (6354)	1689.38	3.66	8.75	5.95	0.77	1.07

CALIB STANDHYD (3541)	Area (ha)	Imp(%)	Di r. Conn. (%)
ID= 1 DT=15.0 min	8.01	75.00	70.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	6.01	2.00
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	231.08	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	55.44	2.31
Storage Coeff. (min)	15.00	60.00
Unit Hyd. Tpeak (min)	6.58 (ii)	54.87 (ii)
Unit Hyd. peak (cms)	0.10	0.02
PEAK FLOW (cms)	0.80	0.01
TIME TO PEAK (hrs)	6.00	6.75
RUNOFF VOLUME (mm)	40.00	3.38
TOTAL RAINFALL (mm)	42.00	42.00
RUNOFF COEFFICIENT	0.95	0.08

TOTALS
0.807 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 35.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5354)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2---> OUT= 1 DT= 15.0 min	0.0000	0.0000	0.5360	0.2546
	0.0270	0.1427	0.6730	0.2758
	0.1550	0.1714	0.7830	0.2966
	0.2890	0.1975	1.1830	0.3066
	0.4010	0.2251	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3541)	8.010	0.807	6.00	29.01
OUTFLOW: ID= 1 (5354)	8.010	0.088	6.75	28.84

PEAK FLOW REDUCTION [Qout/Qin] (%) = 10.87
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.1572

CALIB NASHYD (3542)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	254.67	36.7
U. H. Tp(hrs)=	5.00	# of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)	PEAK FLOW (cms)	TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
3.174	0.258 (i)	8.250	2.863	42.000	0.068

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9354)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3				

	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (3542):	254.67	0.258	8.25	2.86
+ ID2= 2 (5354):	8.01	0.088	6.75	28.84

ID = 3 (9354):	262.68	0.311	7.25	3.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8226)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6354):	1689.38	3.661	8.75	5.95
+ ID2= 2 (9354):	262.68	0.311	7.25	3.66

ID = 3 (8226):	1952.06	3.950	8.75	5.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (3402)				
ID= 1 DT=15.0 min				
	Area Total (ha)	Imp(%)	Dir. Conn.(%)	
	138.83	46.00	28.00	

	IMPERVIOUS (ha)	PERVIOUS (i)		
Surface Area	63.86	74.97		
Dep. Storage	2.00	5.00		
Average Slope	0.50	0.50		
Length	962.05	40.00		
Mannings n	0.013	0.250		
Max. Eff. Inten. (mm/hr)=	55.44	5.30		
over (min)	15.00	60.00		
Storage Coeff. (min)=	15.49 (ii)	50.12 (ii)		
Unit Hyd. Tpeak (min)=	15.00	60.00		
Unit Hyd. peak (cms)=	0.07	0.02		
PEAK FLOW (cms)=	4.30	0.64	*TOTALS*	(iii)
TIME TO PEAK (hrs)=	6.00	6.75		
RUNOFF VOLUME (mm)=	40.00	5.88		
TOTAL RAINFALL (mm)=	42.00	42.00		
RUNOFF COEFFICIENT =	0.95	0.14		

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 47.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3401)				
ID= 1 DT=15.0 min				
	Area Total (ha)	Imp(%)	Dir. Conn.(%)	
	146.96	58.00	35.00	

	IMPERVIOUS (ha)	PERVIOUS (i)		
Surface Area	85.24	61.72		
Dep. Storage	2.00	5.00		
Average Slope	0.50	0.50		
Length	989.81	40.00		
Mannings n	0.013	0.250		
Max. Eff. Inten. (mm/hr)=	55.44	9.35		
over (min)	15.00	45.00		
Storage Coeff. (min)=	15.76 (ii)	43.37 (ii)		
Unit Hyd. Tpeak (min)=	15.00	45.00		
Unit Hyd. peak (cms)=	0.07	0.03		
PEAK FLOW (cms)=	5.65	0.82	*TOTALS*	(iii)
TIME TO PEAK (hrs)=	6.00	6.50		
RUNOFF VOLUME (mm)=	40.00	6.83		
TOTAL RAINFALL (mm)=	42.00	42.00		
RUNOFF COEFFICIENT =	0.95	0.16		

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 47.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5340)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	11.4780	4.3560
	0.3860	2.0347	14.9940	4.9991
	3.2310	2.5818	18.1680	5.6243
	5.1770	3.1417	18.5680	5.6343
	7.7590	3.6318	0.0000	0.0000
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (3401)	146.960	5.973	6.00	18.44
OUTFLOW: ID= 1 (5340)	146.960	0.374	9.25	18.43
PEAK FLOW REDUCTION [Qout/Qin] (%) =	6.26			
TIME SHIFT OF PEAK FLOW (min) =	195.00			
MAXIMUM STORAGE USED (ha.m.) =	1.9715			

ADD HYD (8232)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3402):	138.83	4.487	6.00	15.43
+ ID2= 2 (5340):	146.96	0.374	9.25	18.43

ID = 3 (8232):	285.79	4.643	6.00	16.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0410)				
ID= 1 DT=15.0 min				
	Area Total (ha)	Imp(%)	Dir. Conn.(%)	Curve Number (CN)
	572.01	5.00		48.0
	Ia (mm)	5.00		# of Linear Res. (N) = 1.50
	U. H. Tp (hrs)	1.46		
Unit Hyd. Tpeak (cms)=	6.698			
PEAK FLOW (cms)=	0.844 (i)			
TIME TO PEAK (hrs)=	8.250			
RUNOFF VOLUME (mm)=	4.360			
TOTAL RAINFALL (mm)=	42.000			
RUNOFF COEFFICIENT =	0.104			

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0408)				
ID= 1 DT=15.0 min				
	Area Total (ha)	Imp(%)	Dir. Conn.(%)	Curve Number (CN)
	231.62	5.00		58.0
	Ia (mm)	5.00		# of Linear Res. (N) = 1.50
	U. H. Tp (hrs)	0.64		
Unit Hyd. Tpeak (cms)=	6.198			
PEAK FLOW (cms)=	0.897 (i)			
TIME TO PEAK (hrs)=	6.750			
RUNOFF VOLUME (mm)=	6.074			
TOTAL RAINFALL (mm)=	42.000			
RUNOFF COEFFICIENT =	0.145			

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0376)				
ID= 1 DT=15.0 min				
	Area Total (ha)	Imp(%)	Dir. Conn.(%)	Curve Number (CN)
	463.85	5.00		74.0
	Ia (mm)	5.00		# of Linear Res. (N) = 1.50
	U. H. Tp (hrs)	1.07		
Unit Hyd. Tpeak (cms)=	7.380			
PEAK FLOW (cms)=	2.165 (i)			
TIME TO PEAK (hrs)=	7.500			
RUNOFF VOLUME (mm)=	10.744			
TOTAL RAINFALL (mm)=	42.000			
RUNOFF COEFFICIENT =	0.256			

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0374)
ID= 1 DT=15.0 min

Area (ha)	= 545.70	Curve Number (CN)	= 61.0
La (mm)	= 5.00	# of Linear Res. (N)	= 1.50
U. H. Tp (hrs)	= 1.51		

Unit Hyd Qpeak (cms) = 6.158

PEAK FLOW (cms) = 1.234 (i)
TIME TO PEAK (hrs) = 8.250
RUNOFF VOLUME (mm) = 6.828
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.163

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0372)
ID= 1 DT=15.0 min

Area (ha)	= 110.42	Curve Number (CN)	= 37.0
La (mm)	= 5.00	# of Linear Res. (N)	= 1.50
U. H. Tp (hrs)	= 0.96		

Unit Hyd Qpeak (cms) = 1.954

PEAK FLOW (cms) = 0.145 (i)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 2.885
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.069

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0370)
ID= 1 DT=15.0 min

Area (ha)	= 191.85	Curve Number (CN)	= 63.0
La (mm)	= 5.00	# of Linear Res. (N)	= 1.50
U. H. Tp (hrs)	= 0.67		

Unit Hyd Qpeak (cms) = 4.860

PEAK FLOW (cms) = 0.850 (i)
TIME TO PEAK (hrs) = 6.750
RUNOFF VOLUME (mm) = 7.219
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.172

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0366)
ID= 1 DT=15.0 min

Area (ha)	= 462.62	Curve Number (CN)	= 62.0
La (mm)	= 5.00	# of Linear Res. (N)	= 1.50
U. H. Tp (hrs)	= 1.06		

Unit Hyd Qpeak (cms) = 7.451

PEAK FLOW (cms) = 1.407 (i)
TIME TO PEAK (hrs) = 7.750
RUNOFF VOLUME (mm) = 7.039
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.168

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3641)
ID= 1 DT=15.0 min

Area Total	(ha) = 7.85	Dir. Conn. (%)	= 16.00
Imp (%)	= 45.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 3.53	4.32
Dep. Storage (mm)	= 2.00	5.00
Average Slope (%)	= 0.50	0.50
Length (m)	= 228.76	40.00
Mannings n	= 0.013	0.250
Max. Eff. Inten. (mm/hr)	= 55.44	7.73

over (min)	= 15.00	45.00
Storage Coeff. (min)	= 6.54 (ii)	36.33 (ii)
Unit Hyd. Tpeak (min)	= 15.00	45.00
Unit Hyd. peak (cms)	= 0.10	0.03

TOTALS
0.201 (iii)
6.00
11.24
42.00
0.27

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
**** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 42.9 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5364)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.5250	0.2044
0.0270	0.0959	0.6590	0.2337
0.1520	0.1110	0.7670	0.2624
0.2830	0.1432	1.1670	0.2724
0.3930	0.1710	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3641)	7.850	0.201	6.00	11.24
OUTFLOW: ID= 1 (5364)	7.850	0.016	8.75	11.13

PEAK FLOW REDUCTION [Qout/Qin] (%) = 8.16
TIME SHIFT OF PEAK FLOW (min) = 165.00
MAXIMUM STORAGE USED (ha.m.) = 0.0583

CALIB STANDHYD (3642)
ID= 1 DT=15.0 min

Area Total	(ha) = 147.42	Dir. Conn. (%)	= 7.00
Imp (%)	= 21.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 30.96	116.46
Dep. Storage (mm)	= 2.00	5.00
Average Slope (%)	= 0.50	0.50
Length (m)	= 991.36	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr) = 55.44 3.49

over (min) = 15.00 60.00

Storage Coeff. (min) = 15.77 (ii) 56.73 (ii)

Unit Hyd. Tpeak (min) = 15.00 60.00

Unit Hyd. peak (cms) = 0.07 0.02

PEAK FLOW (cms) = 1.13 0.61 *TOTALS*
TIME TO PEAK (hrs) = 6.00 6.75 1.303 (iii)
RUNOFF VOLUME (mm) = 40.00 4.39 6.00
TOTAL RAINFALL (mm) = 42.00 42.00 6.88
RUNOFF COEFFICIENT = 0.95 0.10 0.16

**** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 42.9 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9364)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3642):	147.42	1.303	6.00	6.88
+ ID2= 2 (5364):	7.85	0.016	8.75	11.13

=====

ID = 3 (9364):	155.27	1.310	6.00	7.09
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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	(8302)				
	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0366):	462.62	1.407	7.75	7.04	
+ ID2= 2 (9364):	155.27	1.310	6.00	7.09	
ID = 3 (8302):	617.89	2.105	6.75	7.05	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	(0358)	Area (ha) = 429.87	Curve Number (CN) = 35.0
		Ia (mm) = 5.00	# of Linear Res. (N) = 1.50
		U. H. Tp (hrs) = 1.03	

Unit Hyd Qpeak (cms) = 7.091

PEAK FLOW (cms) = 0.496 (i)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 2.665
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.063

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	(0360)	Area (ha) = 138.37	Curve Number (CN) = 46.0
		Ia (mm) = 5.00	# of Linear Res. (N) = 1.50
		U. H. Tp (hrs) = 0.60	

Unit Hyd Qpeak (cms) = 3.957

PEAK FLOW (cms) = 0.367 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 3.995
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.095

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	(8306)				
	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0358):	429.87	0.496	7.75	2.66	
+ ID2= 2 (0360):	138.37	0.367	6.75	4.00	
ID = 3 (8306):	568.24	0.839	7.00	2.99	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6362)	
IN= 2--> OUT= 1	Routing time step (min) = 15.00

<----- DATA FOR SECTION (3621.0) ----->			
Distance	Elevation	Manning	
0.00	261.46	0.0550	
27.86	254.23	0.0550	
51.07	251.96	0.0550	
74.29	250.77	0.0550	
97.50	249.91	0.0550	
125.36	249.40	0.0550	
150.93	247.40	0.0550	
155.93	247.33	0.0550 / 0.0350	Main Channel
157.93	246.85	0.0350	Main Channel
159.18	246.65	0.0350	Main Channel
160.18	246.63	0.0350	Main Channel
160.93	246.85	0.0350	Main Channel

161.93	247.18	0.0350 / 0.0550	Main Channel
163.18	248.03	0.0550	
168.18	248.58	0.0550	
183.18	250.18	0.0550	
201.97	252.59	0.0550	
213.57	256.02	0.0550	
225.18	260.31	0.0550	
229.82	261.00	0.0550	

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)
0.55	247.18	556E+04	1.9	1.03	49.95
1.28	247.91	336E+05	16.7	1.53	33.61
2.00	248.63	913E+05	53.6	1.81	28.40
2.73	249.36	185E+06	122.3	2.05	25.18
3.46	250.09	344E+06	224.6	2.02	25.52
4.19	250.82	574E+06	417.9	2.25	22.91
4.91	251.54	855E+06	699.1	2.53	20.38
5.64	252.27	118E+07	1075.3	2.82	18.26
6.37	253.00	153E+07	1559.9	3.15	16.38
7.10	253.73	191E+07	2137.6	3.46	14.89
7.82	254.45	231E+07	2815.2	3.77	13.67
8.55	255.18	272E+07	3608.5	4.10	12.58
9.28	255.91	315E+07	4488.2	4.41	11.69
10.01	256.64	358E+07	5458.0	4.71	10.95
10.73	257.36	403E+07	6512.4	4.99	10.32
11.46	258.09	449E+07	7649.8	5.27	9.78
12.19	258.82	496E+07	8869.5	5.53	9.32
12.92	259.55	544E+07	10171.2	5.78	8.91
13.64	260.27	593E+07	11554.8	6.02	8.55

<---- hydrograph ---->						<- pi pe / channel ->	
	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL	
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW: ID= 2 (8306)	568.24	0.84	7.00	2.99	0.25	1.03	
OUTFLOW: ID= 1 (6362)	568.24	0.75	8.25	2.99	0.22	1.03	

CALIB STANDHYD ID= 1 DT=15.0 min	(3621)	Area Total (ha) = 11.87	Imp(%) = 45.00	Dir. Conn. (%) = 17.00
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Surface Area	(ha) = 5.34	PERVIOUS (i)	6.53
Dep. Storage	(mm) = 2.00		5.00
Average Slope	(%) = 0.50		0.50
Length	(m) = 281.31		40.00
Mannings n	= 0.013		0.250
Max. Eff. Inten. (mm/hr) =	55.44		8.33
over (min) =	15.00		45.00
Storage Coeff. (min) =	7.41 (ii)		36.31 (ii)
Unit Hyd. Tpeak (min) =	15.00		45.00
Unit Hyd. peak (cms) =	0.10		0.03

PEAK FLOW (cms) =	0.28	0.09	0.317 (iii)
TIME TO PEAK (hrs) =	6.00	6.50	6.00
RUNOFF VOLUME (mm) =	40.00	6.27	12.00
TOTAL RAINFALL (mm) =	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.15	0.29

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 45.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5362)			
IN= 2--> OUT= 1			
DT= 15.0 min			
OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.7940	0.3091
0.0400	0.1451	0.9970	0.3534
0.2300	0.1678	1.1610	0.3968
0.4280	0.2166	1.5610	0.4068

0.5940 0.2586 | 0.0000 0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3621)	11.870	0.317	6.00	12.00
OUTFLOW: ID= 1 (5362)	11.870	0.026	8.75	11.93

PEAK FLOW REDUCTION [Qout/Qin] (%) = 8.23
 TIME SHIFT OF PEAK FLOW (min) = 165.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0947

CALIB STANDHYD (3622) ID= 1 DT=15.0 min	Area (ha)	Total Imp(%)	Di r. Conn. (%)
	106.91	24.00	9.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	25.66	81.25
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	844.24	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	55.44	4.00
over (min)	15.00	60.00
Storage Coeff. (min)	14.32 (ii)	53.10 (ii)
Unit Hyd. Tpeak (min)	15.00	60.00
Unit Hyd. peak (cms)	0.07	0.02

TOTALS
 PEAK FLOW (cms) = 1.10 0.50 1.243 (iii)
 TIME TO PEAK (hrs) = 6.00 6.75 6.00
 RUNOFF VOLUME (mm) = 40.00 4.94 8.09
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.12 0.19

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 45.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9362) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3622):	106.91	1.243	6.00	8.09
+ ID2= 2 (5362):	11.87	0.026	8.75	11.93
ID = 3 (9362):	118.78	1.254	6.00	8.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8304) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6362):	568.24	0.746	8.25	2.99
+ ID2= 2 (9362):	118.78	1.254	6.00	8.47
ID = 3 (8304):	687.02	1.352	6.00	3.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8300) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8302):	617.89	2.105	6.75	7.05
+ ID2= 2 (8304):	687.02	1.352	6.00	3.94
ID = 3 (8300):	1304.91	3.392	6.00	5.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6368)
 IN= 2--> OUT= 1 Routing time step (min) = 15.00

<----- DATA FOR SECTION (3681.0) ----->

Distance	Elevation	Manning
0.00	230.00	0.0370
18.48	223.26	0.0370
36.96	223.05	0.0370
64.67	222.94	0.0370
110.87	222.86	0.0370
133.96	222.74	0.0370
147.82	222.65	0.0370
170.92	222.31	0.0370
174.79	222.26	0.0370 / 0.0300
174.89	221.86	0.0300
175.54	221.86	0.0300
176.19	221.86	0.0300
176.29	222.26	0.0300 / 0.0370
180.16	222.25	0.0370
184.78	222.28	0.0370
189.40	222.31	0.0370
332.60	222.37	0.0370
450.00	230.00	0.0370

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.40	222.26	.771E+03	0.3	0.52	41.74
0.81	222.67	.784E+05	30.3	0.51	43.17
1.21	223.07	.210E+06	123.2	0.77	28.42
1.62	223.48	.384E+06	301.0	1.03	21.25
2.03	223.89	.563E+06	550.2	1.28	17.06
2.44	224.30	.747E+06	860.7	1.51	14.46
2.84	224.70	.934E+06	1228.1	1.73	12.68
3.25	225.11	.113E+07	1649.3	1.92	11.37
3.66	225.52	.132E+07	2122.3	2.11	10.37
4.07	225.93	.152E+07	2645.5	2.29	9.58
4.47	226.33	.172E+07	3217.8	2.45	8.92
4.88	226.74	.193E+07	3838.3	2.61	8.38
5.29	227.15	.214E+07	4506.3	2.76	7.92
5.70	227.56	.236E+07	5221.3	2.91	7.52
6.10	227.96	.258E+07	5983.0	3.05	7.17
6.51	228.37	.280E+07	6790.9	3.19	6.87
6.92	228.78	.303E+07	7645.0	3.32	6.60
7.33	229.19	.326E+07	8545.1	3.45	6.35
7.73	229.59	.349E+07	9491.0	3.57	6.13

<---- hydrograph ----> <- pipe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8300)	1304.91	3.39	6.00	5.41	0.44	0.52
OUTFLOW: ID= 1 (6368)	1304.91	2.93	8.00	5.41	0.44	0.52

CALIB STANDHYD (3681) ID= 1 DT=15.0 min	Area (ha)	Total Imp(%)	Di r. Conn. (%)
	2.95	45.00	15.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	1.33	1.62
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	140.24	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	55.44	5.60
over (min)	15.00	45.00
Storage Coeff. (min)	4.88 (ii)	38.76 (ii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.11	0.03

TOTALS
 PEAK FLOW (cms) = 0.07 0.02 0.073 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 40.00 5.38 10.57
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.13 0.25

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%

YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 40.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5368)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.1970	0.0767
0.0100	0.0360	0.2470	0.0877
0.0570	0.0416	0.2880	0.0984
0.1060	0.0537	0.6880	0.1084
0.1470	0.0642	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (3681)	2.950	0.073	6.00	10.57
OUTFLOW: ID= 1 (5368)	2.950	0.006	8.75	10.26

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 7.82
TIME SHIFT OF PEAK FLOW (min) = 165.00
MAXIMUM STORAGE USED (ha.m.) = 0.0206

CALIB NASHYD (3682)
ID= 1 DT=15.0 min

Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
156.53	0.289	7.50	4.03
5.00	0.006	8.75	10.26

Unit Hyd Opeak (cms) = 2.778

PEAK FLOW (cms) = 0.289 (i)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 4.026
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.096

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9368)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3682):	156.53	0.289	7.50	4.03
+ ID2= 2 (5368):	2.95	0.006	8.75	10.26
ID = 3 (9368):	159.48	0.295	7.50	4.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8298)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6368):	1304.91	2.935	8.00	5.41
+ ID2= 2 (9368):	159.48	0.295	7.50	4.14
ID = 3 (8298):	1464.39	3.225	8.00	5.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8296)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0370):	191.85	0.850	6.75	7.22
+ ID2= 2 (8298):	1464.39	3.225	8.00	5.27
ID = 3 (8296):	1656.24	3.980	7.75	5.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6372)
IN= 2--> OUT= 1

Routing time step (min) = 15.00

DATA FOR SECTION (3721.0)

Distance	Elevation	Manning	
0.00	225.00	0.0390	
30.80	219.38	0.0390	
61.61	219.30	0.0390	
77.01	219.27	0.0390	
469.76	219.14	0.0390	
477.46	219.13	0.0390	
485.16	219.10	0.0390	
492.86	219.09	0.0390	
495.56	219.09	0.0390 / 0.0310	Main Channel
495.66	218.51	0.0310	Main Channel
500.56	218.51	0.0310	Main Channel
505.46	218.51	0.0310	Main Channel
505.56	219.09	0.0310 / 0.0390	Main Channel
508.26	219.09	0.0390	
515.96	219.10	0.0390	
523.67	219.21	0.0390	
562.17	219.32	0.0390	
654.58	219.43	0.0390	
731.59	219.46	0.0390	
762.39	225.00	0.0390	

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.29	218.80	.431E+04	1.7	0.58	43.48
0.58	219.09	.867E+04	5.1	0.89	28.32
0.93	219.44	.200E+06	58.2	0.44	57.35
1.28	219.79	.568E+06	278.6	0.74	33.99
1.62	220.13	.939E+06	630.2	1.01	24.84
1.97	220.48	.131E+07	1090.4	1.25	20.05
2.32	220.83	.169E+07	1647.7	1.47	17.06
2.67	221.18	.206E+07	2294.6	1.68	14.99
3.01	221.52	.244E+07	3025.6	1.87	13.46
3.36	221.87	.282E+07	3836.5	2.05	12.27
3.71	222.22	.321E+07	4723.7	2.22	11.31
4.06	222.57	.359E+07	5684.6	2.39	10.53
4.40	222.91	.398E+07	6716.6	2.55	9.87
4.75	223.26	.437E+07	7817.7	2.70	9.31
5.10	223.61	.476E+07	8986.2	2.85	8.83
5.45	223.96	.515E+07	10220.4	2.99	8.40
5.79	224.30	.555E+07	11519.1	3.13	8.03
6.14	224.65	.594E+07	12880.8	3.27	7.69
6.49	225.00	.634E+07	14304.5	3.40	7.39

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8296)	1656.24	3.98	7.75	5.50	0.49	0.76
OUTFLOW: ID= 1 (6372)	1656.24	3.87	8.25	5.50	0.47	0.74

ADD HYD (8294)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0372):	110.42	0.145	7.50	2.88
+ ID2= 2 (6372):	1656.24	3.867	8.25	5.50
ID = 3 (8294):	1766.66	4.006	8.25	5.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8292)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0374):	545.70	1.234	8.25	6.83
+ ID2= 2 (8294):	1766.66	4.006	8.25	5.34
ID = 3 (8292):	2312.36	5.240	8.25	5.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5505)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	65.1290	345.3754
25.4850	24.6697	84.9510	456.3890
31.1490	98.6787	*****	838.7689
39.6440	*****	*****	838.7789
48.1390	*****	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (8292)	2312.360	5.240	8.25	5.69
OUTFLOW: ID= 1 (5505)	2312.360	3.724	11.00	5.69

PEAK FLOW REDUCTION [Qout/Qin] (%) = 71.06
 TIME SHIFT OF PEAK FLOW (min) = 165.00
 MAXIMUM STORAGE USED (ha.m.) = 3.6074

ADD HYD (8272)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0376):	463.85	2.165	7.50	10.74
+ ID2= 2 (5505):	2312.36	3.724	11.00	5.69
=====				
ID = 3 (8272):	2776.21	5.215	10.00	6.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0396)
 ID= 1 DT=15.0 min

Area (ha) = 305.21 Curve Number (CN) = 69.0
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.08

Unit Hyd Opeak (cms) = 4.811
 PEAK FLOW (cms) = 1.174 (i)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 8.977
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.214

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0394)
 ID= 1 DT=15.0 min

Area (ha) = 325.45 Curve Number (CN) = 53.0
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 0.92

Unit Hyd Opeak (cms) = 6.013
 PEAK FLOW (cms) = 0.799 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 5.160
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.123

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0390)
 ID= 1 DT=15.0 min

Area (ha) = 420.00 Curve Number (CN) = 55.0
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.07

Unit Hyd Opeak (cms) = 6.683
 PEAK FLOW (cms) = 0.992 (i)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 5.541
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.132

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0388)
 ID= 1 DT=15.0 min

Area (ha) = 220.77 Curve Number (CN) = 58.0
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 0.99

Unit Hyd Opeak (cms) = 3.819
 PEAK FLOW (cms) = 0.615 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 6.132
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.146

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0386)
 ID= 1 DT=15.0 min

Area (ha) = 241.27 Curve Number (CN) = 61.0
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 0.90

Unit Hyd Opeak (cms) = 4.562
 PEAK FLOW (cms) = 0.799 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 6.784
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.162

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8286)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0386):	241.27	0.799	7.25	6.78
+ ID2= 2 (0388):	220.77	0.615	7.50	6.13
=====				
ID = 3 (8286):	462.04	1.411	7.25	6.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0384)
 ID= 1 DT=15.0 min

Area (ha) = 199.07 Curve Number (CN) = 44.0
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 0.96

Unit Hyd Opeak (cms) = 3.537
 PEAK FLOW (cms) = 0.343 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 3.759
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.089

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0380)
 ID= 1 DT=15.0 min

Area (ha) = 182.01 Curve Number (CN) = 40.0
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 0.55

Unit Hyd Opeak (cms) = 5.609
 PEAK FLOW (cms) = 0.407 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 3.195
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.076

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0382) Area (ha) = 216.59 Curve Number (CN) = 53.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U.H. Tp(hrs) = 0.64

Unit Hyd Qpeak (cms) = 5.733

PEAK FLOW (cms) = 0.697 (i)
TIME TO PEAK (hrs) = 6.750
RUNOFF VOLUME (mm) = 5.118
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.122

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8290)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0380):	182.01	0.407	6.75	3.20
+ ID2= 2 (0382):	216.59	0.697	6.75	5.12
ID = 3 (8290):	398.60	1.104	6.75	4.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6384)
IN= 2--> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3841.0) ----->

Distance	Elevation	Manning	
0.00	294.40	0.0380	
10.59	291.93	0.0380	
21.17	289.19	0.0380	
26.46	287.99	0.0380	
31.76	286.79	0.0380	
71.45	279.97	0.0380	
74.10	279.79	0.0380	
76.74	279.71	0.0380	
78.99	279.64	0.0380 / 0.0300	Main Channel
79.14	279.30	0.0300	Main Channel
79.39	279.30	0.0300	Main Channel
79.64	279.30	0.0300	Main Channel
79.79	279.64	0.0300 / 0.0380	Main Channel
89.98	279.78	0.0380	
119.09	281.86	0.0380	
145.55	282.87	0.0380	
198.48	284.85	0.0380	
211.71	286.31	0.0380	
230.23	287.59	0.0380	
261.99	294.00	0.0380	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.34	279.64	.715E+03	0.2	0.85	63.42
1.10	280.40	.507E+05	21.9	1.40	38.54
1.85	281.15	.141E+06	91.8	2.10	25.69
2.61	281.91	.269E+06	218.9	2.63	20.46
3.36	282.66	.445E+06	407.1	2.96	18.23
4.12	283.42	.681E+06	698.3	3.31	16.26
4.87	284.17	.977E+06	1111.8	3.68	14.65
5.63	284.93	.133E+07	1674.3	4.06	13.27
6.39	285.69	.173E+07	2457.8	4.59	11.73
7.14	286.44	.215E+07	3378.8	5.07	10.63
7.90	287.20	.261E+07	4418.4	5.47	9.85
8.65	287.95	.310E+07	5676.3	5.92	9.11
9.41	288.71	.361E+07	7150.5	6.40	8.42
10.17	289.47	.414E+07	8783.1	6.86	7.86
10.92	290.22	.469E+07	10575.1	7.30	7.38
11.68	290.98	.525E+07	12520.4	7.72	6.98
12.43	291.73	.582E+07	14619.0	8.12	6.64
13.19	292.49	.642E+07	16863.2	8.50	6.34
13.94	293.24	.703E+07	19259.8	8.86	6.08

<---- hydrograph ----> <- pi pe / channel ->

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8290)	398.60	1.10	6.75	4.24	0.37

OUTFLOW: ID= 1 (6384) 398.60 0.87 8.00 4.24 0.36 0.86

ADD HYD (8288)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0384):	199.07	0.343	7.50	3.76
+ ID2= 2 (6384):	398.60	0.866	8.00	4.24
ID = 3 (8288):	597.67	1.204	8.00	4.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8284)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8286):	462.04	1.411	7.25	6.47
+ ID2= 2 (8288):	597.67	1.204	8.00	4.08
ID = 3 (8284):	1059.71	2.590	7.75	5.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8280)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0390):	420.00	0.992	7.75	5.54
+ ID2= 2 (8284):	1059.71	2.590	7.75	5.12
ID = 3 (8280):	1479.71	3.582	7.75	5.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHYD (0392) Area (ha) = 167.22 Curve Number (CN) = 62.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U.H. Tp(hrs) = 0.74

Unit Hyd Qpeak (cms) = 3.837

PEAK FLOW (cms) = 0.666 (i)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 6.993
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.166

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0378) Area (ha) = 606.72 Curve Number (CN) = 55.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U.H. Tp(hrs) = 1.18

Unit Hyd Qpeak (cms) = 8.771

PEAK FLOW (cms) = 1.341 (i)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 5.547
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.132

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8282)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0378):	606.72	1.341	8.00	5.55
+ ID2= 2 (0392):	167.22	0.666	7.00	6.99

ID = 3 (8282): 773.94 1.958 7.50 5.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8278)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8280):	1479.71	3.582	7.75	5.24
+ ID2= 2 (8282):	773.94	1.958	7.50	5.86

ID = 3 (8278):	2253.65	5.533	7.75	5.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6394)
IN= 2----> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3941.0) ----->

Distance	Elevation	Manning	
0.00	283.00	0.0380	
13.48	282.87	0.0380	
53.92	280.08	0.0380	
74.13	276.62	0.0380	
97.72	265.45	0.0380	
114.57	256.93	0.0380	
131.42	253.04	0.0380	
134.79	252.58	0.0380	
138.53	251.74	0.0380 / 0.0300	Main Channel
139.03	251.20	0.0300	Main Channel
141.53	251.20	0.0300	Main Channel
144.03	251.20	0.0300	Main Channel
144.53	251.74	0.0300 / 0.0380	Main Channel
148.27	252.69	0.0380	
151.64	252.97	0.0380	
185.34	255.08	0.0380	
219.03	257.54	0.0380	
262.84	259.43	0.0380	
310.02	262.80	0.0380	
333.60	283.00	0.0380	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.54	251.74	.827E+04	4.5	1.51	30.71
2.19	253.39	.758E+05	73.2	2.69	17.27
3.83	255.03	.282E+06	331.8	3.27	14.17
5.48	256.68	.633E+06	917.3	4.03	11.51
7.12	258.32	.112E+07	1889.5	4.69	9.90
8.77	259.97	.179E+07	3410.2	5.31	8.73
10.41	261.61	.259E+07	5627.6	6.05	7.66
12.06	263.26	.351E+07	8605.1	6.83	6.79
13.70	264.90	.447E+07	12544.9	7.81	5.94
15.35	266.55	.546E+07	17101.4	8.71	5.32
16.99	268.19	.648E+07	22251.7	9.56	4.85
18.64	269.84	.752E+07	27979.0	10.36	4.48
20.28	271.48	.858E+07	34269.0	11.11	4.17
21.93	273.13	.967E+07	41110.8	11.83	3.92
23.57	274.77	.108E+08	48495.9	12.51	3.71
25.22	276.42	.119E+08	56418.1	13.17	3.52
26.86	278.06	.131E+08	64410.6	13.69	3.39
28.51	279.71	.143E+08	73002.1	14.18	3.27
30.15	281.35	.156E+08	81379.3	14.50	3.20

<----- hydrograph -----> <-pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8278)	2253.65	5.53	7.75	5.45	0.57	1.52
OUTFLOW: ID= 1 (6394)	2253.65	5.37	8.25	5.45	0.56	1.52

ADD HYD (8276)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0394):	325.45	0.799	7.25	5.16
+ ID2= 2 (6394):	2253.65	5.366	8.25	5.45

ID = 3 (8276): 2579.10 6.118 8.25 5.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6396)
IN= 2----> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3961.0) ----->

Distance	Elevation	Manning	
0.00	263.00	0.0410	
11.75	257.14	0.0410	
23.50	253.97	0.0410	
41.13	247.83	0.0410	
76.38	232.09	0.0410	
135.13	229.07	0.0410	
149.82	228.97	0.0410	
152.75	228.96	0.0410 / 0.0300	Main Channel
154.19	228.73	0.0300	Main Channel
154.69	228.20	0.0300	Main Channel
155.69	228.20	0.0300	Main Channel
156.69	228.20	0.0300	Main Channel
157.19	228.73	0.0300	Main Channel
158.63	228.95	0.0300 / 0.0410	Main Channel
161.57	228.96	0.0410	
164.51	229.71	0.0410	
196.82	241.70	0.0410	
223.26	249.21	0.0410	
246.76	255.13	0.0410	
290.82	263.51	0.0410	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.75	228.95	.780E+04	2.2	0.98	58.06
2.54	230.74	.277E+06	145.1	1.78	31.76
4.33	232.53	.786E+06	609.1	2.63	21.50
6.13	234.33	.140E+07	1481.5	3.59	15.77
7.92	236.12	.207E+07	2677.3	4.39	12.90
9.71	237.91	.280E+07	4184.8	5.09	11.14
11.50	239.70	.357E+07	6001.4	5.71	9.93
13.29	241.49	.441E+07	8129.1	6.27	9.03
15.09	243.29	.530E+07	10549.4	6.77	8.37
16.88	245.08	.625E+07	13300.6	7.24	7.83
18.67	246.87	.726E+07	16396.2	7.68	7.38
20.46	248.66	.834E+07	19809.8	8.07	7.02
22.26	250.46	.949E+07	23532.5	8.43	6.72
24.05	252.25	.107E+08	27641.2	8.77	6.46
25.84	254.04	.120E+08	32155.2	9.10	6.22
27.63	255.83	.134E+08	36901.0	9.37	6.05
29.42	257.62	.149E+08	42147.7	9.64	5.88
31.22	259.42	.164E+08	48149.1	9.97	5.68
33.01	261.21	.181E+08	54644.8	10.29	5.51

<----- hydrograph -----> <-pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8276)	2579.10	6.12	8.25	5.42	0.80	0.99
OUTFLOW: ID= 1 (6396)	2579.10	5.54	9.25	5.42	0.79	0.99

ADD HYD (8274)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0396):	305.21	1.174	7.75	8.98
+ ID2= 2 (6396):	2579.10	5.542	9.25	5.42

ID = 3 (8274):	2884.31	6.553	9.25	5.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8270)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8272):	2776.21	5.215	10.00	6.53
+ ID2= 2 (8274):	2884.31	6.553	9.25	5.79

 ID = 3 (8270): 5660.52 11.652 9.50 6.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5506)				
IN= 2----> OUT= 1				
DT= 15.0 min				
OUTFLOW	STORAGE	OUTFLOW	STORAGE	
(cms)	(ha. m.)	(cms)	(ha. m.)	
0.0000	0.0000	60.8810	135.6832	
31.1490	24.6697	96.2770	900.4431	
36.8120	37.0045	96.6770	900.4531	
45.3070	86.3439	0.0000	0.0000	
AREA	OPEAK	TPEAK	R. V.	
(ha)	(cms)	(hrs)	(mm)	
INFLOW : ID= 2 (8270) 5660.520	11.652	9.50	6.16	
OUTFLOW: ID= 1 (5506) 5660.520	9.673	11.75	6.16	

PEAK FLOW REDUCTION [Oout/Oi n] (%) = 83.02
 TIME SHIFT OF PEAK FLOW (min) = 135.00
 MAXIMUM STORAGE USED (ha. m.) = 7.6665

CALIB NASHYD (0406)			
ID= 1 DT=15.0 min			
Area (ha)	= 142.65	Curve Number (CN)	= 66.0
Ia (mm)	= 5.00	# of Linear Res. (N)	= 1.50
U. H. Tp(hrs)	= 0.59		

Unit Hyd Qpeak (cms) = 4.135

PEAK FLOW (cms) = 0.781 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 7.974
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.190

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0404)			
ID= 1 DT=15.0 min			
Area (ha)	= 246.46	Curve Number (CN)	= 47.0
Ia (mm)	= 5.00	# of Linear Res. (N)	= 1.50
U. H. Tp(hrs)	= 0.98		

Unit Hyd Qpeak (cms) = 4.280

PEAK FLOW (cms) = 0.467 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 4.188
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.100

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0402)			
ID= 1 DT=15.0 min			
Area (ha)	= 244.00	Curve Number (CN)	= 61.0
Ia (mm)	= 5.00	# of Linear Res. (N)	= 1.50
U. H. Tp(hrs)	= 1.07		

Unit Hyd Qpeak (cms) = 3.879

PEAK FLOW (cms) = 0.710 (i)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 6.803
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.162

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0400)			
ID= 1 DT=15.0 min			
Area (ha)	= 93.97	Curve Number (CN)	= 52.0
Ia (mm)	= 5.00	# of Linear Res. (N)	= 1.50
U. H. Tp(hrs)	= 0.44		

Unit Hyd Qpeak (cms) = 3.630

PEAK FLOW (cms) = 0.390 (i)
 TIME TO PEAK (hrs) = 6.500
 RUNOFF VOLUME (mm) = 4.872
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.116

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0398)			
ID= 1 DT=15.0 min			
Area (ha)	= 328.19	Curve Number (CN)	= 55.0
Ia (mm)	= 5.00	# of Linear Res. (N)	= 1.50
U. H. Tp(hrs)	= 0.83		

Unit Hyd Qpeak (cms) = 6.759

PEAK FLOW (cms) = 0.938 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 5.517
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.131

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8268)				
1 + 2 = 3				
ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0398)	328.19	0.938	7.00	5.52
+ ID2= 2 (0400)	93.97	0.390	6.50	4.87

ID = 3 (8268)	422.16	1.286	7.00	5.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6402)		Routing time step (min) = 15.00	
IN= 2----> OUT= 1			

<----- DATA FOR SECTION (4021.0) ----->			
Di stance	Elevation	Manning	
0.00	238.50	0.0360	
11.50	238.00	0.0360	
23.00	237.93	0.0360	
34.49	236.39	0.0360	
63.24	233.98	0.0360	
97.73	228.15	0.0360	
123.60	227.08	0.0360	
126.48	226.61	0.0360	
127.60	226.47	0.0360 / 0.0330	Mai n Channel
127.85	225.25	0.0330	Mai n Channel
129.35	225.25	0.0330	Mai n Channel
130.85	225.25	0.0330	Mai n Channel
131.10	226.47	0.0330 / 0.0360	Mai n Channel
132.22	226.59	0.0360	
143.72	227.42	0.0360	
169.59	227.88	0.0360	
192.59	231.19	0.0360	
218.46	233.02	0.0360	
241.45	235.50	0.0360	
284.57	236.43	0.0360	

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.41	225.66	.391E+04	1.7	1.36	38.31
0.81	226.06	.804E+04	5.0	1.93	27.02
1.22	226.47	.124E+05	9.1	2.30	22.61
1.84	227.09	.308E+05	23.3	2.36	22.03
2.46	227.71	.904E+05	62.0	2.14	24.31
3.09	228.34	.223E+06	188.3	2.63	19.75
3.71	228.96	.379E+06	410.2	3.38	15.39
4.33	229.58	.550E+06	711.5	4.04	12.89
4.95	230.20	.737E+06	1091.3	4.62	11.25
5.58	230.83	.939E+06	1550.6	5.15	10.10
6.20	231.45	.116E+07	2072.4	5.59	9.31
6.82	232.07	.140E+07	2659.2	5.93	8.77
7.44	232.69	.167E+07	3345.8	6.27	8.30
8.07	233.32	.195E+07	4159.0	6.64	7.83

8.69	233.94	.226E+07	5100.8	7.03	7.39
9.31	234.56	.259E+07	6081.3	7.32	7.11
9.93	235.18	.295E+07	7183.1	7.60	6.84
10.56	235.81	.334E+07	8145.2	7.62	6.83
11.18	236.43	.379E+07	9116.0	7.51	6.92

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8268)	422.16	1.29	7.00	5.37	0.31	1.36
OUTFLOW: ID= 1 (6402)	422.16	1.15	7.75	5.37	0.28	1.36

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0402)	244.00	0.710	7.75	6.80
+ ID2= 2 (6402)	422.16	1.155	7.75	5.37
ID = 3 (8266)	666.16	1.865	7.75	5.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0404)	246.46	0.467	7.50	4.19
+ ID2= 2 (8266)	666.16	1.865	7.75	5.90
ID = 3 (8264)	912.62	2.329	7.75	5.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0406)	142.65	0.781	6.75	7.97
+ ID2= 2 (8264)	912.62	2.329	7.75	5.44
ID = 3 (8262)	1055.27	3.000	7.50	5.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (5506)	5660.52	9.673	11.75	6.16
+ ID2= 2 (8262)	1055.27	3.000	7.50	5.78
ID = 3 (8260)	6715.79	11.084	11.25	6.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0408)	231.62	0.897	6.75	6.07
+ ID2= 2 (8260)	6715.79	11.084	11.25	6.10
ID = 3 (8258)	6947.41	11.368	11.25	6.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8256)	231.62	0.897	6.75	6.07
+ ID2= 2 (8260)	6715.79	11.084	11.25	6.10
ID = 3 (8256)	6947.41	11.368	11.25	6.10

ID1= 1 (0410):	572.01	0.844	8.25	4.36
+ ID2= 2 (8258):	6947.41	11.368	11.25	6.10
ID = 3 (8256):	7519.42	12.064	11.00	5.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5507)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2--> OUT= 1	0.0000	0.0000	90.6140	67.8416
DT= 15.0 min	39.6440	12.3348	*****	160.3529
	48.1390	18.5023	*****	160.3629
	67.9600	37.0045	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (8256)	7519.421	12.064	11.00	5.96
OUTFLOW: ID= 1 (5507)	7519.421	11.816	12.00	5.96

PEAK FLOW REDUCTION [Qout/Qin] (%) = 97.95
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 3.6767

CALIB NASHYD (0420)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	175.82	53.0
	U. H. (mm) = 5.00	# of Li near Res. (N) = 1.50
	U. H. Tp(hrs) = 0.81	

Unit Hyd Opeak (cms) = 3.692

PEAK FLOW (cms) = 0.476 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 5.148
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.123

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (4141)	Area (ha)	Dir. Conn. (%)
ID= 1 DT=15.0 min	156.72	21.00
	Total Imp(%) = 52.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	81.49	75.23
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1022.15	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	55.44	20.02
over (min)	15.00	45.00
Storage Coeff. (min)	16.06 (ii)	36.42 (ii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.07	0.03

TOTALS
 PEAK FLOW (cms) = 3.58 2.35 4.547 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 40.00 13.12 18.77
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.31 0.45

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.8 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (4144)	Area (ha)	Dir. Conn. (%)
ID= 1 DT=15.0 min	2.10	10.00
	Total Imp(%) = 30.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	0.63	1.47

Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 118.32 40.00
 Mannings n = 0.013 0.250

 Max. Eff. Inten. (mm/hr)= 55.44 19.55
 over (min) 15.00 30.00
 Storage Coeff. (mi n)= 4.41 (ii) 24.96 (ii)
 Unit Hyd. Tpeak (mi n)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.11 0.04

 PEAK FLOW (cms)= 0.03 0.06 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.25 0.068 (iii)
 RUNOFF VOLUME (mm)= 40.00 16.03 6.00
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.38 0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9429)
 IN= 2--> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.1000	0.0386
0.0100	0.0195	0.1400	0.0480
0.0400	0.0218	0.1700	0.0569
0.0700	0.0312	0.2000	0.0655

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (4144)	2.100	0.068	6.00	18.42
OUTFLOW: ID= 1 (9429)	2.100	0.023	7.00	18.20

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 33.26
 TIME SHIFT OF PEAK FLOW (mi n) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 0.0205

ADD HYD (9424)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4141):	156.72	4.547	6.00	18.77
+ ID2= 2 (9429):	2.10	0.023	7.00	18.20

ID = 3 (9424):	158.82	4.550	6.00	18.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5414)
 IN= 2--> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.7060	4.4896
0.1870	1.6651	5.4940	5.4773
2.0210	2.0447	6.0800	6.4738
2.9590	2.4658	6.4800	6.4838
3.7490	3.5575	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9424)	158.820	4.550	6.00	18.76
OUTFLOW: ID= 1 (5414)	158.820	1.102	7.75	18.75

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 24.21
 TIME SHIFT OF PEAK FLOW (mi n) = 105.00
 MAXIMUM STORAGE USED (ha. m.) = 1.8569

CALIB

STANDHYD (4142) Area (ha)= 29.14
 ID= 1 DT=15.0 mi n Total Imp(%)= 52.00 Di r. Conn. (%)= 21.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	15.15	13.99
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	440.76	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 55.44 20.02
 over (mi n) 15.00 45.00
 Storage Coeff. (mi n)= 9.70 (ii) 30.05 (ii)
 Unit Hyd. Tpeak (mi n)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.09 0.03

PEAK FLOW (cms)= 0.80 0.48 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 40.00 13.12 18.77
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.31 0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 66.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8254)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4142):	29.14	1.001	6.00	18.77
+ ID2= 2 (5414):	158.82	1.102	7.75	18.75

ID = 3 (8254):	187.96	1.286	7.50	18.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (4121)
 ID= 1 DT=15.0 mi n

Area (ha)= 202.00
 Total Imp(%)= 45.00 Di r. Conn. (%)= 16.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	90.90	111.10
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1160.46	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 55.44 10.44
 over (mi n) 15.00 45.00
 Storage Coeff. (mi n)= 17.33 (ii) 43.75 (ii)
 Unit Hyd. Tpeak (mi n)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.07 0.03

PEAK FLOW (cms)= 3.40 1.64 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 40.00 7.68 12.85
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.18 0.31

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 51.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5412)
 IN= 2--> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	6.5000	1.0500

0.3860	0.5085	7.9000	1.1500
1.8460	0.6103	9.3000	1.2500
3.5050	0.7566	9.7000	1.2600
4.7890	0.8929	0.0000	0.0000

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (4121)	202.000	4.048	6.00	12.85
OUTFLOW: ID= 1 (5412)	202.000	2.756	6.50	12.85

PEAK FLOW REDUCTION [Qout/Qin] (%) = 68.09
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 0.6928

CALIB STANDHYD (4122)
 ID= 1 DT=15.0 min
 Area (ha) = 36.70
 Imp(%) = 63.00
 Dir. Conn. (%) = 22.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) =	23.12	13.58
Dep. Storage (mm) =	2.00	5.00
Average Slope (%) =	0.50	0.50
Length (m) =	494.64	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr) =	55.44	19.63
over (min) =	15.00	45.00
Storage Coeff. (min) =	10.39 (ii)	30.91 (ii)
Unit Hyd. Tpeak (min) =	15.00	45.00
Unit Hyd. peak (cms) =	0.09	0.03

TOTALS
 PEAK FLOW (cms) = 1.04
 TIME TO PEAK (hrs) = 6.00
 RUNOFF VOLUME (mm) = 40.00
 TOTAL RAINFALL (mm) = 42.00
 RUNOFF COEFFICIENT = 0.95

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 51.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9412) 1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (4122):	36.70	1.224	6.00	16.80
+ ID2= 2 (5412):	202.00	2.756	6.50	12.85

ID = 3 (9412):	238.70	3.427	6.50	13.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8252) 1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8254):	187.96	1.286	7.50	18.75
+ ID2= 2 (9412):	238.70	3.427	6.50	13.45

ID = 3 (8252):	426.66	4.210	6.50	15.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6416)
 IN= 2----> OUT= 1
 Routing time step (min) = 15.00

<----- DATA FOR SECTION (4161.0) ----->

Distance	Elevation	Manning
0.00	270.07	0.0340
20.67	267.91	0.0340
62.01	264.33	0.0340
113.69	259.75	0.0340

165.37	253.30	0.0340
227.38	246.29	0.0340
232.55	246.03	0.0340
237.72	246.16	0.0340
241.39	246.02	0.0340 / 0.0300
241.64	245.75	0.0300
242.89	245.75	0.0300
244.14	245.75	0.0300
244.39	246.02	0.0300 / 0.0340
248.06	246.20	0.0340
253.22	246.28	0.0340
258.39	246.63	0.0340
346.25	252.57	0.0340
413.43	257.77	0.0340
465.11	261.78	0.0340
511.62	270.00	0.0340

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)
0.27	246.02	.295E+04	0.6	0.75	88.26
1.53	247.28	.168E+06	69.7	1.65	40.13
2.79	248.54	.490E+06	299.5	2.42	27.28
4.06	249.81	.962E+06	740.1	3.05	21.67
5.32	251.07	.158E+07	1440.0	3.61	18.33
6.58	252.33	.235E+07	2443.8	4.12	16.06
7.84	253.59	.327E+07	3811.2	4.63	14.30
9.10	254.85	.432E+07	5565.1	5.11	12.94
10.37	256.12	.550E+07	7721.3	5.57	11.88
11.63	257.38	.682E+07	10309.9	6.00	11.02
12.89	258.64	.827E+07	13360.6	6.42	10.31
14.15	259.90	.985E+07	16884.5	6.81	9.72
15.42	261.17	.116E+08	20818.4	7.14	9.26
16.68	262.43	.134E+08	25510.4	7.53	8.78
17.94	263.69	.154E+08	30958.0	7.97	8.30
19.20	264.95	.175E+08	36988.3	8.38	7.89
20.46	266.21	.197E+08	43617.1	8.78	7.53
21.73	267.48	.220E+08	50868.1	9.17	7.22
22.99	268.74	.244E+08	58854.0	9.56	6.92

<---- hydrograph ----> <- pipe / channel ->

	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW: ID= 2 (8252)	426.66	4.21	6.50	15.79	0.34	0.77
OUTFLOW: ID= 1 (6416)	426.66	2.42	7.75	15.79	0.30	0.76

CALIB NASHYD (4164)
 ID= 1 DT=15.0 min
 Area (ha) = 89.30
 Ia (mm) = 5.00
 U.H. Tp(hrs) = 1.46
 Curve Number (CN) = 76.0
 # of Linear Res. (N) = 3.00

Unit Hyd Opeak (cms) =	2.344
PEAK FLOW (cms) =	0.714 (i)
TIME TO PEAK (hrs) =	7.500
RUNOFF VOLUME (mm) =	11.679
TOTAL RAINFALL (mm) =	42.000
RUNOFF COEFFICIENT =	0.278

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9427) IN= 2----> OUT= 1 DT= 15.0 min	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha.m.)	(cms)	(ha.m.)
	0.0000	0.0000	4.4700	2.5000
	0.9000	1.0000	5.9800	3.0000
	1.7300	1.5000	7.5000	3.5000
	3.2200	2.0000	8.7400	4.0000

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (4164)	89.300	0.714	7.50	11.68
OUTFLOW: ID= 1 (9427)	89.300	0.379	9.75	11.68

PEAK FLOW REDUCTION [Qout/Qin] (%) = 53.04
 TIME SHIFT OF PEAK FLOW (min) = 135.00
 MAXIMUM STORAGE USED (ha.m.) = 0.4213

RESERVOIR (5416)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	1.5980	0.7565
0.0330	0.3532	1.8570	0.8137
0.7450	0.4481	2.0990	0.8707
1.0270	0.5154	2.4990	0.8807
1.3050	0.6350	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9427)	89.300	0.379	9.75	11.68
OUTFLOW: ID= 1 (5416)	89.300	0.335	11.25	11.65

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 88.54
TIME SHIFT OF PEAK FLOW (min) = 90.00
MAXIMUM STORAGE USED (ha. m.) = 0.3936

CALIB NASHYD (4162)
ID= 1 DT=15.0 min

Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
349.99	0.907	8.00	6.89
U. H. Tp (hrs) = 1.29			

Unit Hyd Opeak (cms) = 4.646

PEAK FLOW (cms) = 0.907 (i)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 6.888
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.164

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9416)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4162):	349.99	0.907	8.00	6.89
+ ID2= 2 (5416):	89.30	0.335	11.25	11.65
-----	-----	-----	-----	-----
ID = 3 (9416):	439.29	1.005	11.00	7.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8250)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6416):	426.66	2.418	7.75	15.79
+ ID2= 2 (9416):	439.29	1.005	11.00	7.86
-----	-----	-----	-----	-----
ID = 3 (8250):	865.95	3.332	8.00	11.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0418)
ID= 1 DT=15.0 min

Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
174.09	0.571	7.50	7.54
U. H. Tp (hrs) = 1.05			

Unit Hyd Opeak (cms) = 2.825

PEAK FLOW (cms) = 0.571 (i)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 7.539
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.179

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

STANDHYD (4184)
ID= 1 DT=15.0 min

Area Total (ha) = 8.70
Imp (%) = 30.00
Dir. Conn. (%) = 10.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	2.61	6.09
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	240.83	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	55.44	15.57
over (min)	15.00	30.00
Storage Coeff. (min)	6.75 (ii)	29.26 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.10	0.04

	PEAK FLOW (cms)	TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
	0.12	6.00	40.00	42.00	0.95
	0.18	6.25	13.06	42.00	0.31
	0.233 (iii)	6.00	15.76	42.00	0.38

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 73.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9428)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.4300	0.1618
0.0300	0.0818	0.5800	0.2012
0.1700	0.0912	0.7300	0.2384
0.3100	0.1306	0.8500	0.2747

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (4184)	8.700	0.233	6.00	15.76
OUTFLOW: ID= 1 (9428)	8.700	0.047	7.75	15.68

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 19.96
TIME SHIFT OF PEAK FLOW (min) = 105.00
MAXIMUM STORAGE USED (ha. m.) = 0.0829

ADD HYD (9423)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0418):	174.09	0.571	7.50	7.54
+ ID2= 2 (9428):	8.70	0.047	7.75	15.68
-----	-----	-----	-----	-----
ID = 3 (9423):	182.79	0.618	7.75	7.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8248)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8250):	865.95	3.332	8.00	11.76
+ ID2= 2 (9423):	182.79	0.618	7.75	7.93
-----	-----	-----	-----	-----
ID = 3 (8248):	1048.74	3.944	8.00	11.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8246)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)

ID1= 1 (0420):	175.82	0.476	7.00	5.15
+ ID2= 2 (8248):	1048.74	3.944	8.00	11.09

ID = 3 (8246):	1224.56	4.396	7.75	10.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8244) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (5507):	7519.42	11.816	12.00	5.96
+ ID2= 2 (8246):	1224.56	4.396	7.75	10.24

ID = 3 (8244):	8743.98	14.432	11.25	6.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (4227) ID= 1 DT=15.0 min	Area Total	(ha)= 66.80	Dir. Conn. (%)=	15.00
	Imp(%)=	29.00		
	IMPERVIOUS	PERVIOUS (i)		
Surface Area	(ha)=	19.37		47.43 (i)
Dep. Storage	(mm)=	2.00		5.00
Average Slope	(%)=	0.50		0.50
Length	(m)=	667.33		40.00
Mannings n	=	0.013		0.250
Max. Eff. Inten. (mm/hr)=	55.44	5.33		
over (min)	15.00	60.00		
Storage Coeff. (min)=	12.44 (ii)	47.00 (ii)		
Unit Hyd. Tpeak (min)=	15.00	60.00		
Unit Hyd. peak (cms)=	0.08	0.02		
			TOTALS	
PEAK FLOW (cms)=	1.21	0.42		1.332 (iii)
TIME TO PEAK (hrs)=	6.00	6.75		6.00
RUNOFF VOLUME (mm)=	40.00	6.55		11.56
TOTAL RAINFALL (mm)=	42.00	42.00		42.00
RUNOFF COEFFICIENT =	0.95	0.16		0.28

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 54.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9426) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	3.3400	1.2303
	0.2300	0.6171	4.4700	1.5350
	1.3000	0.6860	5.6100	1.8228
	2.4100	0.9909	6.5300	2.1041
		AREA (ha)	OPEAK (cms)	TPEAK (hrs)
INFLOW: ID= 2 (4227)	66.800	1.332	6.00	11.56
OUTFLOW: ID= 1 (9426)	66.800	0.173	8.75	11.55

PEAK FLOW REDUCTION [Qout/Qin] (%) = 12.98
TIME SHIFT OF PEAK FLOW (min) = 165.00
MAXIMUM STORAGE USED (ha. m.) = 0.4642

RESERVOIR (5422) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	4.3110	2.6782
	0.2180	1.7874	5.4130	2.9186
	1.2490	1.9110	6.3010	3.1540

	2.3250	2.1760	6.7010	3.1640
	3.2250	2.4038	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9426)	66.800	0.173	8.75	11.55
OUTFLOW: ID= 1 (5422)	66.800	0.054	20.00	11.52

PEAK FLOW REDUCTION [Qout/Qin] (%) = 31.22
TIME SHIFT OF PEAK FLOW (min) = 675.00
MAXIMUM STORAGE USED (ha. m.) = 0.4429

CALIB NASHYD (4222) ID= 1 DT=15.0 min	Area Ia	(ha)= (mm)=	713.41 5.00	Curve Number # of Linear Res. (N)=	(CN)= 54.0 1.50
	U. H.	TP(hrs)=	1.95		

Unit Hyd Opeak (cms) = 6.252

PEAK FLOW (cms) = 1.046 (i)
TIME TO PEAK (hrs) = 9.500
RUNOFF VOLUME (mm) = 5.383
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.128

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9422) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4222):	713.41	1.046	9.50	5.38
+ ID2= 2 (5422):	66.80	0.054	20.00	11.52

ID = 3 (9422):	780.21	1.070	9.50	5.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8242) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8244):	8743.98	14.432	11.25	6.56
+ ID2= 2 (9422):	780.21	1.070	9.50	5.91

ID = 3 (8242):	9524.19	15.441	11.00	6.51

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5508) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	*****	197.3574
	76.4550	30.8371	*****	394.7148
	*****	61.6742	*****	394.7248
	*****	*****	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (8242)	9524.190	15.441	11.00	6.51
OUTFLOW: ID= 1 (5508)	9524.190	14.957	12.25	6.51

PEAK FLOW REDUCTION [Qout/Qin] (%) = 96.86
TIME SHIFT OF PEAK FLOW (min) = 75.00
MAXIMUM STORAGE USED (ha. m.) = 6.0355

CALIB NASHYD (0336) ID= 1 DT=15.0 min	Area Ia	(ha)= (mm)=	2785.00 5.00	Curve Number # of Linear Res. (N)=	(CN)= 72.0 1.50
	U. H.	TP(hrs)=	15.39		

Unit Hyd Opeak (cms) = 3.090

PEAK FLOW (cms)= 1.223 (i)
 TIME TO PEAK (hrs)= 22.500
 RUNOFF VOLUME (mm)= 10.068
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.240

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (3382)
 ID= 1 DT=15.0 min

Area (ha)= 433.29	Di r. Conn. (%)= 20.00
Total Imp(%)= 39.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	168.98	264.31
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1699.59	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	37.80	12.12
over (min)	30.00	60.00
Storage Coeff. (min)=	25.40 (ii)	50.28 (ii)
Unit Hyd. Tpeak (min)=	30.00	60.00
Unit Hyd. peak (cms)=	0.04	0.02

TOTALS

PEAK FLOW (cms)=	6.45	3.96	8.562 (iii)
TIME TO PEAK (hrs)=	6.25	6.75	6.25
RUNOFF VOLUME (mm)=	40.00	10.20	16.16
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.24	0.38

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (3381)
 ID= 1 DT=15.0 min

Area (ha)= 165.62	Di r. Conn. (%)= 27.00
Total Imp(%)= 53.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	87.78	77.84
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1050.78	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	55.44	16.86
over (min)	15.00	45.00
Storage Coeff. (min)=	16.33 (ii)	38.14 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.07	0.03

TOTALS

PEAK FLOW (cms)=	4.83	2.00	5.643 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	40.00	11.82	19.43
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.28	0.46

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5338)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	9.5300	3.8529
0.6370	1.6350	12.1350	4.2725
2.7300	2.1009	14.3850	4.6808
5.1010	2.7445	14.7850	4.6908

7.0460 3.2936 | 0.0000 0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3381)	165.620	5.643	6.00	19.43
OUTFLOW: ID= 1 (5338)	165.620	1.359	7.50	19.43

PEAK FLOW REDUCTION [Qout/Qi n] (%)= 24.08
 TIME SHIFT OF PEAK FLOW (min)= 90.00
 MAXIMUM STORAGE USED (ha. m.)= 1.8017

ADD HYD (8310)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3382):	433.29	8.562	6.25	16.16
+ ID2= 2 (5338):	165.62	1.359	7.50	19.43
-----	-----	-----	-----	-----
ID = 3 (8310):	598.91	8.998	6.25	17.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (3342)
 ID= 1 DT=15.0 min

Area (ha)= 586.87	Curve Number (CN)= 56.0
Ia (mm)= 5.00	# of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 8.20	

Unit Hyd Opeak (cms)= 1.221

PEAK FLOW (cms)= 0.275 (i)
 TIME TO PEAK (hrs)= 15.500
 RUNOFF VOLUME (mm)= 5.784
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.138

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (3341)
 ID= 1 DT=15.0 min

Area (ha)= 33.23	Di r. Conn. (%)= 21.00
Total Imp(%)= 51.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	16.95	16.28
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	470.67	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	55.44	11.79
over (min)	15.00	45.00
Storage Coeff. (min)=	10.09 (ii)	35.24 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.09	0.03

TOTALS

PEAK FLOW (cms)=	0.91	0.31	1.028 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	40.00	8.18	14.86
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.19	0.35

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 51.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5334)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.6690	0.4644
0.0400	0.1927	0.8520	0.5318
0.1900	0.2643	1.0360	0.5973
0.3830	0.3576	1.4360	0.6073

0.4460 0.3863 | 0.0000 0.0000

INFLOW : ID= 2 (3341) 33.230 1.028 6.00 14.86
 OUTFLOW: ID= 1 (5334) 33.230 0.210 7.25 14.83

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 20.40
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha. m.) = 0.2739

ADD HYD (8314)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3342):	586.87	0.275	15.50	5.78
+ ID2= 2 (5334):	33.23	0.210	7.25	14.83
ID = 3 (8314):	620.10	0.341	8.00	6.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0306)
 ID= 1 DT=15.0 min

Area (ha)	= 283.97	Curve Number (CN)	= 52.0
Ia (mm)	= 5.00	# of Linear Res. (N)	= 1.50
U. H. Tp (hrs)	= 6.44		

Unit Hyd Qpeak (cms) = 0.753

PEAK FLOW (cms) = 0.146 (i)
 TIME TO PEAK (hrs) = 14.000
 RUNOFF VOLUME (mm) = 5.040
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.120

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0282)
 ID= 1 DT=15.0 min

Area (ha)	= 449.38	Curve Number (CN)	= 77.0
Ia (mm)	= 5.00	# of Linear Res. (N)	= 1.50
U. H. Tp (hrs)	= 1.47		

Unit Hyd Qpeak (cms) = 5.226

PEAK FLOW (cms) = 1.868 (i)
 TIME TO PEAK (hrs) = 8.250
 RUNOFF VOLUME (mm) = 12.059
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.287

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0284)
 ID= 1 DT=15.0 min

Area (ha)	= 78.93	Curve Number (CN)	= 84.0
Ia (mm)	= 5.00	# of Linear Res. (N)	= 1.50
U. H. Tp (hrs)	= 0.57		

Unit Hyd Qpeak (cms) = 2.344

PEAK FLOW (cms) = 0.907 (i)
 TIME TO PEAK (hrs) = 6.500
 RUNOFF VOLUME (mm) = 15.663
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.373

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8388)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0282):	449.38	1.868	8.25	12.06
+ ID2= 2 (0284):	78.93	0.907	6.50	15.66

ID = 3 (8388): 528.31 2.560 7.50 12.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6286)
 IN= 2----> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (2861.0) ----->

Distance	Elevation	Manning
0.00	233.00	0.0450
20.58	228.51	0.0450
41.17	227.74	0.0450
51.46	227.41	0.0450
97.77	225.96	0.0450
149.23	223.94	0.0450
200.69	220.84	0.0450
226.42	220.66	0.0450
238.85	220.22	0.0450
241.35	220.01	0.0450 / 0.0350
241.85	219.70	0.0350
245.85	219.72	0.0350
246.35	220.06	0.0350
248.85	220.23	0.0350
303.60	221.64	0.0350
380.79	224.98	0.0450
432.25	229.54	0.0450
457.98	233.33	0.0450
483.71	234.27	0.0450
509.44	233.81	0.0450

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.31	220.01	382E+04	0.8	0.57	82.89
1.06	220.76	525E+05	12.1	0.66	72.51
1.81	221.51	240E+06	88.6	1.05	45.21
2.56	222.26	510E+06	261.7	1.46	32.47
3.31	223.01	843E+06	533.8	1.81	26.34
4.06	223.76	124E+07	913.6	2.10	22.64
4.81	224.51	171E+07	1399.0	2.34	20.33
5.56	225.26	225E+07	2051.0	2.60	18.27
6.31	226.01	285E+07	2907.0	2.91	16.35
7.06	226.76	352E+07	3902.5	3.16	15.04
7.82	227.52	426E+07	5069.9	3.39	14.01
8.57	228.27	507E+07	6433.0	3.62	13.13
9.32	229.02	593E+07	8069.2	3.89	12.24
10.07	229.77	681E+07	9926.7	4.16	11.44
10.82	230.52	772E+07	11962.8	4.42	10.75
11.57	231.27	864E+07	14167.1	4.68	10.17
12.32	232.02	959E+07	16537.1	4.92	9.66
13.07	232.77	105E+08	19070.7	5.16	9.22
13.82	233.52	115E+08	21790.4	5.39	8.82

<---- hydrograph ----> <- pipe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8388)	528.31	2.56	7.50	12.60	0.43	0.59
OUTFLOW: ID= 1 (6286)	528.31	2.18	9.25	12.60	0.40	0.58

CALIB STANDHYD (2861)
 ID= 1 DT=15.0 min

Area (ha)	= 54.86	Dir. Conn. (%)	= 19.00
Total Imp (%)	= 45.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	= 24.69	30.17
Dep. Storage (mm)	= 2.00	5.00
Average Slope (%)	= 0.50	0.50
Length (m)	= 604.76	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr)	= 55.44	24.91
over (min)	= 15.00	45.00
Storage Coeff. (min)	= 11.72 (ii)	30.37 (ii)
Unit Hyd. Tpeak (min)	= 15.00	45.00
Unit Hyd. peak (cms)	= 0.08	0.03

TOTALS
 PEAK FLOW (cms) = 1.29 1.27 1.824 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 40.00 17.65 21.89

TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.42 0.52

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5286)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.6700	1.4285
0.1860	0.6703	4.6080	1.6332
1.0630	0.7754	5.3640	1.8336
1.9790	1.0010	5.7640	1.8436
2.7460	1.1950	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (2861)	54.860	1.824	6.00	21.89
OUTFLOW: ID= 1 (5286)	54.860	0.537	7.50	21.88

PEAK FLOW REDUCTION [Out/Oin] (%) = 29.46
 TIME SHIFT OF PEAK FLOW (min) = 90.00
 MAXIMUM STORAGE USED (ha. m.) = 0.7139

CALIB
 STANDHYD (2862)
 ID= 1 DT=15.0 min

Area (ha) = 205.65
 Total Imp (%) = 22.00
 Dir. Conn. (%) = 10.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	45.24	160.41
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1170.90	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 55.44 15.99
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 17.43 (ii) 39.70 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.07 0.03

PEAK FLOW (cms) = 2.16 3.81 *TOTALS*
 TIME TO PEAK (hrs) = 6.00 6.50 4.545 (iii)
 RUNOFF VOLUME (mm) = 40.00 14.75 17.28
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.35 0.41

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9286)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (2862):	205.65	4.545	6.50	17.28
+ ID2= 2 (5286):	54.86	0.537	7.50	21.88
-----	-----	-----	-----	-----
ID = 3 (9286):	260.51	4.681	6.50	18.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ADD HYD (8386) 1 + 2 = 3				
ID1= 1 (6286):	528.31	2.176	9.25	12.60
+ ID2= 2 (9286):	260.51	4.681	6.50	18.24
-----	-----	-----	-----	-----
ID = 3 (8386):	788.82	5.423	6.50	14.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0302)
 ID= 1 DT=15.0 min

Area (ha) = 473.90
 Curve Number (CN) = 58.0
 Ia (mm) = 5.00
 # of Linear Res. (N) = 1.50
 U.H. Tp (hrs) = 1.66

Unit Hyd Opeak (cms) = 4.874
 PEAK FLOW (cms) = 0.899 (i)
 TIME TO PEAK (hrs) = 8.750
 RUNOFF VOLUME (mm) = 6.167
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.147

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0298)
 ID= 1 DT=15.0 min

Area (ha) = 330.51
 Curve Number (CN) = 45.0
 Ia (mm) = 5.00
 # of Linear Res. (N) = 1.50
 U.H. Tp (hrs) = 1.26

Unit Hyd Opeak (cms) = 4.461
 PEAK FLOW (cms) = 0.488 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 3.912
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.093

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (3001)
 ID= 1 DT=15.0 min

Area (ha) = 0.09
 Total Imp (%) = 45.00
 Dir. Conn. (%) = 15.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	0.04	0.05
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	24.49	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 55.44 9.13
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 1.71 (ii) 29.58 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.11 0.04

PEAK FLOW (cms) = 0.00 0.00 *TOTALS*
 TIME TO PEAK (hrs) = 6.00 6.25 0.003 (iii)
 RUNOFF VOLUME (mm) = 40.00 6.68 6.00
 TOTAL RAINFALL (mm) = 42.00 42.00 11.01
 RUNOFF COEFFICIENT = 0.95 0.16 0.26

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 46.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5300)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW	STORAGE	OUTFLOW	STORAGE

(cms)	(ha. m.)	(cms)	(ha. m.)
0.0000	0.0000	0.0060	0.0023
0.0020	0.0012	0.0070	0.0026
0.0030	0.0016	0.0090	0.0029
0.0040	0.0019	0.4090	0.0129

AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (3001) 0.090 0.003 6.00 11.01
 OUTFLOW: ID= 1 (5300) 0.090 0.001 6.75 9.72

PEAK FLOW REDUCTION [Qout/Qi n](%) = 26.16
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 0.0004

CALIB NASHYD (3002) Area (ha) = 258.84 Curve Number (CN) = 51.8
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 1.03

Unit Hyd Qpeak (cms) = 4.290

PEAK FLOW (cms) = 0.562 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 4.959
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.118

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9300)
 1 + 2 = 3
 ID1= 1 (3002): 258.84 0.562 7.50 4.96
 + ID2= 2 (5300): 0.09 0.001 6.75 9.72
 ID = 3 (9300): 258.93 0.563 7.50 4.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8395)
 1 + 2 = 3
 ID1= 1 (0298): 330.51 0.488 8.00 3.91
 + ID2= 2 (9300): 258.93 0.563 7.50 4.96
 ID = 3 (8395): 589.44 1.047 8.00 4.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0288) Area (ha) = 340.83 Curve Number (CN) = 78.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 2.21

Unit Hyd Qpeak (cms) = 2.629

PEAK FLOW (cms) = 1.075 (i)
 TIME TO PEAK (hrs) = 9.750
 RUNOFF VOLUME (mm) = 12.562
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.299

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0290) Area (ha) = 269.18 Curve Number (CN) = 78.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 1.07

Unit Hyd Qpeak (cms) = 4.279

PEAK FLOW (cms) = 1.470 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 12.485
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.297

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8397)
 1 + 2 = 3
 ID1= 1 (0288): 340.83 1.075 9.75 12.56
 + ID2= 2 (0290): 269.18 1.470 7.50 12.49
 ID = 3 (8397): 610.01 2.456 8.00 12.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0296) Area (ha) = 293.65 Curve Number (CN) = 76.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 1.13

Unit Hyd Qpeak (cms) = 4.437

PEAK FLOW (cms) = 1.427 (i)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 11.580
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.276

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0292) Area (ha) = 738.49 Curve Number (CN) = 68.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 1.52

Unit Hyd Qpeak (cms) = 8.289

PEAK FLOW (cms) = 2.131 (i)
 TIME TO PEAK (hrs) = 8.250
 RUNOFF VOLUME (mm) = 8.698
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.207

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0294) Area (ha) = 274.15 Curve Number (CN) = 57.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 0.87

Unit Hyd Qpeak (cms) = 5.367

PEAK FLOW (cms) = 0.809 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 5.913
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.141

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8398)
 1 + 2 = 3
 ID1= 1 (0292): 738.49 2.131 8.25 8.70
 + ID2= 2 (0294): 274.15 0.809 7.25 5.91
 ID = 3 (8398): 1012.64 2.892 8.00 7.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6296)
IN= 2----> OUT= 1

Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2961.0) ----->

Distance	Elevation	Manning	
0.00	243.98	0.0400	
42.59	243.18	0.0400	
85.17	241.81	0.0400	
120.66	240.50	0.0400	
156.15	239.56	0.0400	
198.74	236.15	0.0400	
237.78	234.01	0.0400	
241.33	233.82	0.0400	
248.77	233.12	0.0400 / 0.0400	Main Channel
249.87	232.32	0.0400	Main Channel
250.37	231.80	0.0400	Main Channel
250.87	232.23	0.0400	Main Channel
251.97	233.10	0.0400 / 0.0400	Main Channel
255.37	233.22	0.0400	
259.07	233.87	0.0400	
262.62	234.12	0.0400	
266.17	234.23	0.0400	
283.91	234.73	0.0400	
337.15	241.75	0.0400	
351.34	244.00	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.43	232.23	.510E+03	0.1	0.53	81.07
0.87	232.67	.213E+04	0.7	0.83	51.49
1.30	233.10	.501E+04	2.2	1.10	38.75
1.98	233.78	.242E+05	12.6	1.34	31.93
2.66	234.46	.738E+05	42.1	1.46	29.21
3.34	235.14	.177E+06	127.9	1.84	23.13
4.02	235.82	.314E+06	281.4	2.29	18.61
4.70	236.50	.481E+06	509.4	2.71	15.73
5.38	237.18	.672E+06	815.6	3.11	13.73
6.06	237.86	.886E+06	1200.7	3.46	12.30
6.74	238.54	.112E+07	1669.4	3.80	11.23
7.42	239.22	.139E+07	2226.9	4.11	10.38
8.10	239.90	.169E+07	2808.4	4.28	9.95
8.78	240.58	.202E+07	3470.4	4.40	9.68
9.46	241.26	.240E+07	4331.7	4.61	9.24
10.14	241.94	.283E+07	5335.2	4.83	8.84
10.82	242.62	.330E+07	6482.6	5.03	8.48
11.50	243.30	.381E+07	7764.1	5.21	8.18
12.18	243.98	.439E+07	9101.2	5.31	8.04

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8398)	1012.64	2.89	8.00	7.94	1.35	1.11
OUTFLOW: ID= 1 (6296)	1012.64	2.79	8.75	7.94	1.34	1.11

ADD HYD (8396)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0296):	293.65	1.427	7.75	11.58
+ ID2= 2 (6296):	1012.64	2.793	8.75	7.94
ID = 3 (8396):	1306.29	4.146	8.50	8.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8394)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8396):	1306.29	4.146	8.50	8.76
+ ID2= 2 (8397):	610.01	2.456	8.00	12.53
ID = 3 (8394):	1916.30	6.591	8.25	9.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8392)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8394):	1916.30	6.591	8.25	9.96
+ ID2= 2 (8395):	589.44	1.047	8.00	4.37
ID = 3 (8392):	2505.74	7.623	8.25	8.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6302)
IN= 2----> OUT= 1

Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3021.0) ----->

Distance	Elevation	Manning	
0.00	228.10	0.0400	
18.47	227.12	0.0400	
36.95	226.12	0.0400	
46.18	225.84	0.0400	
55.42	225.58	0.0400	
272.47	222.88	0.0400	
277.09	222.76	0.0400	
281.71	222.58	0.0400	
288.54	222.18	0.0400 / 0.0300	Main Channel
288.64	221.00	0.0300	Main Channel
290.94	221.00	0.0300	Main Channel
291.04	221.00	0.0300	Main Channel
291.54	222.75	0.0300 / 0.0400	Main Channel
300.18	222.83	0.0400	
304.80	223.04	0.0400	
309.42	223.25	0.0400	
318.65	223.69	0.0400	
360.22	225.57	0.0400	
397.16	227.60	0.0400	
457.20	228.35	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.29	221.29	.228E+04	0.2	0.32	165.63
0.59	221.59	.465E+04	0.7	0.45	116.15
0.88	221.88	.713E+04	1.2	0.54	96.87
1.18	222.18	.971E+04	1.9	0.61	86.20
1.57	222.57	.175E+05	3.3	0.59	88.86
1.97	222.97	.427E+05	6.4	0.47	111.39
2.36	223.36	.115E+06	16.1	0.44	119.14
2.76	223.76	.237E+06	37.1	0.49	106.49
3.15	224.15	.409E+06	72.9	0.56	93.40
3.55	224.55	.631E+06	127.0	0.63	82.76
3.94	224.94	.903E+06	202.4	0.70	74.37
4.34	225.34	.123E+07	301.8	0.77	67.68
4.73	225.73	.160E+07	433.5	0.85	61.37
5.13	226.13	.200E+07	602.2	0.95	55.29
5.52	226.52	.242E+07	804.7	1.05	50.13
5.92	226.92	.286E+07	1034.2	1.14	46.11
6.31	227.31	.332E+07	1290.5	1.22	42.88
6.71	227.71	.380E+07	1560.9	1.29	40.55
7.10	228.10	.432E+07	1835.6	1.34	39.19

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8392)	2505.74	7.62	8.25	8.65	2.02	0.47
OUTFLOW: ID= 1 (6302)	2505.74	6.16	10.50	8.65	1.94	0.48

ADD HYD (8390)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0302):	473.90	0.899	8.75	6.17
+ ID2= 2 (6302):	2505.74	6.159	10.50	8.65
ID = 3 (8390):	2979.64	6.993	10.50	8.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8386):	788.82	5.423	6.50	14.46
+ ID2= 2 (8390):	2979.64	6.993	10.50	8.25
ID = 3 (8348):	3768.46	9.856	10.00	9.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area Ia U. H.	(ha) (mm) Tp(hrs)=	Curve Number # of Linear Res. (N)=	(CN)= 1.50
(0304)		292.37 5.00 2.78	(CN)= 63.0	

Unit Hyd Opeak (cms) = 1.793

PEAK FLOW (cms) = 0.450 (i)
 TIME TO PEAK (hrs) = 10.500
 RUNOFF VOLUME (mm) = 7.337
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.175

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area Ia U. H.	(ha) (mm) Tp(hrs)=	Curve Number # of Linear Res. (N)=	(CN)= 1.50
(0280)		299.86 5.00 0.85	(CN)= 82.0	

Unit Hyd Opeak (cms) = 6.009

PEAK FLOW (cms) = 2.320 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 14.568
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.347

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area Ia U. H.	(ha) (mm) Tp(hrs)=	Curve Number # of Linear Res. (N)=	(CN)= 1.50
(0278)		485.49 5.00 1.52	(CN)= 82.0	

Unit Hyd Opeak (cms) = 5.453

PEAK FLOW (cms) = 2.409 (i)
 TIME TO PEAK (hrs) = 8.250
 RUNOFF VOLUME (mm) = 14.678
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.349

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area Ia U. H.	(ha) (mm) Tp(hrs)=	Curve Number # of Linear Res. (N)=	(CN)= 1.50
(0276)		90.89 5.00 0.67	(CN)= 79.0	

Unit Hyd Opeak (cms) = 2.302

PEAK FLOW (cms) = 0.742 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 12.858
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.306

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area Ia U. H.	(ha) (mm) Tp(hrs)=	Curve Number # of Linear Res. (N)=	(CN)= 1.50
(0274)		392.49 5.00 1.08	(CN)= 75.0	

Unit Hyd Opeak (cms) = 6.182

PEAK FLOW (cms) = 1.889 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 11.150
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.265

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0274):	392.49	1.889	7.50	11.15
+ ID2= 2 (0276):	90.89	0.742	6.75	12.86
ID = 3 (8360):	483.38	2.585	7.25	11.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0278):	485.49	2.409	8.25	14.68
+ ID2= 2 (8360):	483.38	2.585	7.25	11.47
ID = 3 (8358):	968.87	4.901	7.75	13.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6280) IN= 2----> OUT= 1	Routing time step (min) = 15.00
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<----- DATA FOR SECTION (2801.0) ----->		
Distance	Elevation	Manning
0.00	241.14	0.0500
13.32	240.80	0.0500
39.95	240.07	0.0500
96.54	236.21	0.0500
113.19	234.15	0.0500
123.18	232.35	0.0500
143.15	225.80	0.0500
149.81	225.62	0.0500
153.14	225.40	0.0500
157.30	224.76	0.0500 / 0.0300
159.80	224.26	0.0300
162.30	224.85	0.0300
162.55	224.97	0.0300 / 0.0500
167.80	225.05	0.0500
186.43	229.14	0.0500
213.06	234.75	0.0500
236.37	237.09	0.0500
266.33	237.31	0.0500
292.96	237.83	0.0500
329.58	241.50	0.0500

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	224.76	.263E+04	0.8	0.65	58.31
1.36	225.62	.278E+05	13.4	1.10	34.54
2.22	226.48	.855E+05	52.6	1.40	27.09
3.09	227.35	.157E+06	120.4	1.74	21.78
3.95	228.21	.242E+06	216.3	2.04	18.64
4.81	229.07	.340E+06	341.8	2.29	16.56
5.67	229.93	.450E+06	498.3	2.52	15.06
6.53	230.79	.574E+06	688.3	2.73	13.90
7.40	231.66	.711E+06	913.8	2.93	12.97
8.26	232.52	.861E+06	1173.4	3.11	12.23
9.12	233.38	.103E+07	1461.3	3.24	11.72
9.98	234.24	.121E+07	1793.3	3.37	11.26

10.85	235.11	.142E+07	2141.0	3.45	11.02
11.71	235.97	.165E+07	2530.4	3.50	10.86
12.57	236.83	.192E+07	2954.0	3.51	10.81
13.43	237.69	.226E+07	3160.3	3.18	11.93
14.29	238.55	.271E+07	3869.4	3.26	11.67
15.16	239.42	.320E+07	4740.3	3.38	11.24
16.02	240.28	.373E+07	5688.4	3.48	10.92

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8358)	968.87	4.90	7.75	13.08	0.78	0.75
OUTFLOW: ID= 1 (6280)	968.87	4.56	8.75	13.08	0.76	0.74

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0280):	299.86	2.320	7.00	14.57
+ ID2= 2 (6280):	968.87	4.564	8.75	13.08
ID = 3 (8354):	1268.73	6.519	8.25	13.43

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0270) ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)
	243.61	81.0
	U.H. Tp(hrs)= 0.87	# of Linear Res. (N)= 1.30

Unit Hyd Qpeak (cms)=	3.429
PEAK FLOW (cms)=	1.320 (i)
TIME TO PEAK (hrs)=	7.500
RUNOFF VOLUME (mm)=	13.984
TOTAL RAINFALL (mm)=	42.000
RUNOFF COEFFICIENT =	0.333

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0268) ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)
	215.76	75.0
	U.H. Tp(hrs)= 0.69	# of Linear Res. (N)= 1.30

Unit Hyd Qpeak (cms)=	3.807
PEAK FLOW (cms)=	1.091 (i)
TIME TO PEAK (hrs)=	7.000
RUNOFF VOLUME (mm)=	11.050
TOTAL RAINFALL (mm)=	42.000
RUNOFF COEFFICIENT =	0.263

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0266) ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)
	508.09	64.0
	U.H. Tp(hrs)= 1.63	# of Linear Res. (N)= 1.30

Unit Hyd Qpeak (cms)=	3.786
PEAK FLOW (cms)=	0.901 (i)
TIME TO PEAK (hrs)=	9.750
RUNOFF VOLUME (mm)=	7.566
TOTAL RAINFALL (mm)=	42.000
RUNOFF COEFFICIENT =	0.180

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2641) ID= 1 DT=15.0 min	Area (ha)	Total Imp(%)	Dir. Conn. (%)
	17.58	75.00	70.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	13.18	4.39
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	342.34	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	55.44	10.89
over (min)	15.00	45.00
Storage Coeff. (min)=	8.33 (ii)	34.30 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.09	0.03
PEAK FLOW (cms)=	1.68	0.08
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	40.00	10.04
TOTAL RAINFALL (mm)=	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.24

TOTALS
1.712 (iii)
6.00
31.01
42.00
0.74

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 66.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5264) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	1.1760	0.5589
	0.0600	0.3133	1.4770	0.6054
	0.3410	0.3763	1.7190	0.6511
	0.6340	0.4336	2.1190	0.6611
	0.8800	0.4941	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (2641)	17.580	1.712	6.00	31.01
OUTFLOW: ID= 1 (5264)	17.580	0.236	6.75	30.94

PEAK FLOW REDUCTION [Qout/Oin] (%) = 13.80
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha.m.) = 0.3541

CALIB NASHYD (2642) ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)
	336.37	69.0
	U.H. Tp(hrs)= 1.28	# of Linear Res. (N)= 1.30

Unit Hyd Qpeak (cms)=	3.192
PEAK FLOW (cms)=	0.853 (i)
TIME TO PEAK (hrs)=	8.250
RUNOFF VOLUME (mm)=	8.986
TOTAL RAINFALL (mm)=	42.000
RUNOFF COEFFICIENT =	0.214

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (2642):	336.37	0.853	8.25	8.99
+ ID2= 2 (5264):	17.58	0.236	6.75	30.94
ID = 3 (9264):	353.95	0.988	8.00	10.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA	OPEAK	TPEAK	R.V.
----------------------	------	-------	-------	------

	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0266):	508.09	0.901	9.75	7.57
+ ID2= 2 (9264):	353.95	0.988	8.00	10.08
ID = 3 (8380):	862.04	1.852	8.25	8.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6268)
IN= 2--> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (2681.0) ----->

Distance	Elevation	Manning		
0.00	274.03	0.0500		
10.55	272.73	0.0500		
21.10	271.28	0.0500		
50.12	266.83	0.0500		
87.04	260.36	0.0500		
92.32	260.06	0.0500		
94.95	259.93	0.0500		
101.00	256.87	0.0500		
103.00	256.58	0.0500 / 0.0350	Main Channel	
104.00	256.32	0.0350	Main Channel	
106.00	256.05	0.0350	Main Channel	
108.00	256.25	0.0350	Main Channel	
110.78	256.65	0.0350 / 0.0500	Main Channel	
113.42	256.81	0.0500		
116.06	257.00	0.0500		
155.62	260.24	0.0500		
187.27	263.35	0.0500		
211.01	267.22	0.0500		
224.20	269.60	0.0500		
261.13	272.68	0.0500		

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.53	256.58	.697E+04	2.1	0.97	55.64
1.38	257.43	.477E+05	25.9	1.76	30.66
2.22	258.27	.123E+06	84.3	2.22	24.26
3.07	259.12	.231E+06	185.4	2.60	20.72
3.92	259.97	.371E+06	335.8	2.92	18.42
4.77	260.82	.560E+06	547.6	3.15	17.06
5.61	261.66	.790E+06	845.6	3.46	15.57
6.46	262.51	.106E+07	1225.0	3.75	14.37
7.31	263.36	.136E+07	1693.3	4.02	13.37
8.16	264.21	.169E+07	2283.2	4.35	12.36
9.00	265.05	.206E+07	2969.1	4.66	11.54
9.85	265.90	.245E+07	3754.4	4.96	10.86
10.70	266.75	.286E+07	4643.0	5.24	10.28
11.55	267.60	.331E+07	5636.1	5.50	9.78
12.39	268.44	.378E+07	6746.2	5.76	9.34
13.24	269.29	.428E+07	7972.2	6.01	8.96
14.09	270.14	.482E+07	9218.9	6.18	8.71
14.94	270.99	.539E+07	10550.5	6.32	8.52
15.78	271.83	.601E+07	12024.7	6.46	8.33

<----- hydrograph -----> <-pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8380)	862.04	1.85	8.25	8.60	0.47	0.97
OUTFLOW: ID= 1 (6268)	862.04	1.78	10.00	8.60	0.45	0.97

ADD HYD (8382)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0268):	215.76	1.091	7.00	11.05
+ ID2= 2 (6268):	862.04	1.779	10.00	8.60
ID = 3 (8382):	1077.80	2.636	8.75	9.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6270)
IN= 2--> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (2701.0) ----->

Distance	Elevation	Manning		
0.00	245.98	0.0500		
8.14	245.66	0.0500		
16.28	245.16	0.0500		
20.35	244.84	0.0500		
38.66	242.98	0.0500		
48.83	240.65	0.0500		
63.07	235.91	0.0500		
65.11	235.18	0.0500		
68.43	234.34	0.0500 / 0.0300	Main Channel	
68.68	233.95	0.0300	Main Channel	
69.18	233.89	0.0300	Main Channel	
69.68	233.95	0.0300	Main Channel	
71.21	234.48	0.0300 / 0.0500	Main Channel	
81.38	236.44	0.0500		
95.63	236.66	0.0500		
120.04	237.00	0.0500		
148.53	241.77	0.0500		
158.70	242.34	0.0500		
187.18	244.03	0.0500		
201.42	244.36	0.0500		

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.45	234.34	.217E+04	0.7	0.99	52.83
0.98	234.87	.966E+04	4.6	1.49	35.20
1.50	235.39	.250E+05	13.7	1.73	30.41
2.03	235.92	.475E+05	29.5	1.95	26.87
2.56	236.45	.772E+05	52.5	2.14	24.50
3.09	236.98	.142E+06	82.4	1.83	28.73
3.61	237.50	.246E+06	158.1	2.03	25.91
4.14	238.03	.357E+06	262.0	2.31	22.73
4.67	238.56	.477E+06	392.1	2.59	20.27
5.20	239.09	.604E+06	547.8	2.86	18.38
5.72	239.61	.739E+06	729.2	3.11	16.90
6.25	240.14	.882E+06	936.5	3.34	15.70
6.78	240.67	.103E+07	1169.7	3.57	14.72
7.31	241.20	.119E+07	1425.9	3.77	13.94
7.83	241.72	.136E+07	1710.4	3.96	13.26
8.36	242.25	.154E+07	1967.9	4.02	13.07
8.89	242.78	.174E+07	2264.9	4.09	12.83
9.42	243.31	.196E+07	2589.3	4.15	12.65
9.94	243.83	.221E+07	2953.5	4.22	12.46

<----- hydrograph -----> <-pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8382)	1077.80	2.64	8.75	9.09	0.71	1.19
OUTFLOW: ID= 1 (6270)	1077.80	2.57	9.50	9.09	0.70	1.19

ADD HYD (8384)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0270):	243.61	1.320	7.50	13.98
+ ID2= 2 (6270):	1077.80	2.574	9.50	9.09
ID = 3 (8384):	1321.41	3.729	9.00	9.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHVD (0262)
ID= 1 DT=15.0 min

Area (ha)	(ha)	Curve Number (CN)
341.31	341.31	82.0
5.00	5.00	# of Li near Res. (N) = 1.30
1.01	1.01	

Unit Hyd Opeak (cms) = 4.094

PEAK FLOW (cms) = 1.720 (i)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 14.597
TOTAL RAINFALL (mm) = 42.000
RUNOFF COEFFICIENT = 0.348

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0258) Area (ha) = 181.99 Curve Number (CN) = 79.0
 ID= 1 DT=15.0 min I a (mm) = 5.00 # of Linear Res. (N) = 1.30
 U. H. Tp(hrs) = 1.18

Unit Hyd Qpeak (cms) = 1.881

PEAK FLOW (cms) = 0.722 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 12.980
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.309

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0256) Area (ha) = 145.79 Curve Number (CN) = 67.0
 ID= 1 DT=15.0 min I a (mm) = 5.00 # of Linear Res. (N) = 1.30
 U. H. Tp(hrs) = 1.04

Unit Hyd Qpeak (cms) = 1.707

PEAK FLOW (cms) = 0.405 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 8.355
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.199

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0254) Area (ha) = 403.00 Curve Number (CN) = 55.0
 ID= 1 DT=15.0 min I a (mm) = 5.00 # of Linear Res. (N) = 1.30
 U. H. Tp(hrs) = 1.22

Unit Hyd Qpeak (cms) = 4.028

PEAK FLOW (cms) = 0.650 (i)
 TIME TO PEAK (hrs) = 8.250
 RUNOFF VOLUME (mm) = 5.543
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.132

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6256) Routing time step (min) = 15.00
 IN= 2--- OUT= 1

Distance	Elevation	Manning	
0.00	276.07	0.0400	
11.68	273.71	0.0400	
23.36	271.35	0.0400	
29.19	270.30	0.0400	
35.03	269.44	0.0400	
55.47	267.90	0.0400	
78.82	266.24	0.0400	
90.50	265.63	0.0400	
102.18	265.40	0.0400	
105.10	264.95	0.0400 / 0.0350	Main Channel
108.02	264.39	0.0350	Main Channel
110.94	264.72	0.0350	Main Channel
113.86	265.19	0.0350 / 0.0400	Main Channel
116.78	265.49	0.0400	
143.05	268.24	0.0400	
172.25	270.53	0.0400	
207.28	271.95	0.0400	
233.55	273.72	0.0400	
256.91	274.98	0.0400	
289.02	275.97	0.0400	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.56	264.95	.723E+04	1.9	0.90	62.51
1.14	265.53	.307E+05	13.5	1.49	37.92
1.72	266.11	.951E+05	48.6	1.73	32.62

2.30	266.69	.191E+06	117.8	2.09	27.03
2.88	267.27	.314E+06	225.2	2.43	23.24
3.46	267.85	.464E+06	375.5	2.74	20.59
4.04	268.43	.641E+06	573.2	3.03	18.62
4.62	269.01	.846E+06	821.9	3.29	17.15
5.20	269.59	1.08E+07	1135.0	3.56	15.87
5.78	270.17	1.34E+07	1525.1	3.86	14.64
6.36	270.75	1.62E+07	1971.1	4.12	13.71
6.94	271.33	1.94E+07	2474.0	4.33	13.04
7.52	271.91	2.28E+07	3065.0	4.55	12.42
8.10	272.49	2.66E+07	3773.4	4.80	11.75
8.68	273.07	3.06E+07	4566.3	5.05	11.17
9.26	273.65	3.48E+07	5443.7	5.30	10.66
9.84	274.23	3.93E+07	6389.6	5.51	10.25
10.42	274.81	4.40E+07	7426.2	5.72	9.88
11.00	275.39	4.90E+07	8490.1	5.86	9.63

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (0254)	403.00	0.65	8.25	5.54	0.19	0.90
OUTFLOW: ID= 1 (6256)	403.00	0.62	10.00	5.54	0.18	0.90

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ADD HYD (8370)				
1 + 2 = 3				
ID1= 1 (0256):	145.79	0.405	8.00	8.36
+ ID2= 2 (6256):	403.00	0.617	10.00	5.54
ID = 3 (8370):	548.79	0.986	9.50	6.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0250) Area (ha) = 192.88 Curve Number (CN) = 70.0
 ID= 1 DT=15.0 min I a (mm) = 5.00 # of Linear Res. (N) = 1.30
 U. H. Tp(hrs) = 1.22

Unit Hyd Qpeak (cms) = 1.930

PEAK FLOW (cms) = 0.529 (i)
 TIME TO PEAK (hrs) = 8.250
 RUNOFF VOLUME (mm) = 9.304
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.222

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0246) Area (ha) = 759.61 Curve Number (CN) = 55.0
 ID= 1 DT=15.0 min I a (mm) = 5.00 # of Linear Res. (N) = 1.30
 U. H. Tp(hrs) = 1.81

Unit Hyd Qpeak (cms) = 5.121

PEAK FLOW (cms) = 0.922 (i)
 TIME TO PEAK (hrs) = 10.000
 RUNOFF VOLUME (mm) = 5.563
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.132

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0248) Area (ha) = 146.04 Curve Number (CN) = 64.0
 ID= 1 DT=15.0 min I a (mm) = 5.00 # of Linear Res. (N) = 1.30
 U. H. Tp(hrs) = 0.78

Unit Hyd Qpeak (cms) = 2.271

PEAK FLOW (cms) = 0.446 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 7.494
 TOTAL RAINFALL (mm) = 42.000

RUNOFF COEFFICIENT = 0.178

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0246):	759.61	0.922	10.00	5.56
+ ID2= 2 (0248):	146.04	0.446	7.50	7.49
ID = 3 (8364):	905.65	1.303	8.50	5.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6250)	Routing time step (mi.n)' = 15.00
IN= 2---> OUT= 1	

<----- DATA FOR SECTION (2501.0) ----->

Distance	Elevation	Manning	
0.00	269.96	0.0500	
8.56	268.55	0.0500	
17.13	266.91	0.0500	
21.41	266.13	0.0500	
40.68	263.15	0.0500	
62.09	260.75	0.0500	
85.64	258.02	0.0500	
88.20	257.69	0.0500	
93.20	257.05	0.0500 / 0.0350	Main Channel
93.45	256.88	0.0350	Main Channel
94.20	256.56	0.0350	Main Channel
94.95	256.83	0.0350	Main Channel
95.20	257.08	0.0350 / 0.0500	Main Channel
100.62	257.45	0.0500	
115.61	258.57	0.0500	
139.16	260.43	0.0500	
152.01	261.95	0.0500	
171.27	264.63	0.0500	
188.40	267.90	0.0500	
211.95	274.18	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.49	257.05	.205E+04	0.4	0.68	89.65
1.17	257.73	.248E+05	5.7	0.85	72.16
1.85	258.41	.841E+05	25.2	1.10	55.60
2.53	259.09	.180E+06	66.5	1.36	45.14
3.21	259.77	.312E+06	135.7	1.60	38.36
3.89	260.45	.481E+06	238.3	1.82	33.61
4.57	261.13	.681E+06	383.5	2.07	29.60
5.25	261.81	.911E+06	568.9	2.29	26.70
5.93	262.49	.117E+07	800.5	2.51	24.36
6.61	263.17	.146E+07	1079.0	2.72	22.49
7.28	263.84	.177E+07	1413.4	2.94	20.85
7.96	264.52	.210E+07	1797.0	3.14	19.50
8.64	265.20	.246E+07	2241.6	3.35	18.28
9.32	265.88	.283E+07	2739.9	3.55	17.25
10.00	266.56	.323E+07	3295.7	3.75	16.34
10.68	267.24	.364E+07	3908.9	3.94	15.54
11.36	267.92	.408E+07	4578.5	4.12	14.84
12.04	268.60	.453E+07	5320.1	4.32	14.18
12.72	269.28	.499E+07	6109.1	4.50	13.61

<----- hydrograph ----->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8364)	905.65	1.30	8.50	5.87	0.61	0.71
OUTFLOW: ID= 1 (6250)	905.65	1.21	10.75	5.87	0.59	0.70

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0250):	192.88	0.529	8.25	9.30
+ ID2= 2 (6250):	905.65	1.210	10.75	5.87

ID = 3 (8366): 1098.53 1.691 10.25 6.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8366):	1098.53	1.691	10.25	6.48
+ ID2= 2 (8370):	548.79	0.986	9.50	6.29
ID = 3 (8368):	1647.32	2.669	10.00	6.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2521)	Area Total	(ha)= Imp(%)=	Di r. Conn. (%)=
ID= 1 DT=15.0 mi n	32.51	75.00	70.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	24.38	8.13
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	465.55	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	55.44	12.46
over (min)	15.00	45.00
Storage Coeff. (mi n)=	10.02 (ii)	34.63 (ii)
Unit Hyd. Tpeak (mi n)=	15.00	45.00
Unit Hyd. peak (cms)=	0.09	0.03
PEAK FLOW (cms)=	2.96	0.16
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	40.00	11.36
TOTAL RAINFALL (mm)=	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.27

TOTALS (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5252)
IN= 2---> OUT= 1
DT= 15.0 mi n

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	2.1750	1.0334
	0.1100	0.5793	2.7310	1.1195
	0.6300	0.6958	3.1790	1.2039
	1.1730	0.8017	3.5790	1.2139
	1.6270	0.9137	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2521)	32.510	3.022	6.00	31.41
OUTFLOW: ID= 1 (5252)	32.510	0.456	6.75	31.37

PEAK FLOW REDUCTION [Out/Oi n] (%) = 15.08
TIME SHIFT OF PEAK FLOW (mi n) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.6578

CALIB NASHYD (2522)	Area Ia	(ha)= (mm)=	Curve Number (CN)=
ID= 1 DT=15.0 mi n	287.47	5.00	71.5
	U. H. Tp (hrs)=	1.01	# of Linear Res. (N)= 1.30

Unit Hyd Opeak (cms)=	3.448
PEAK FLOW (cms)=	0.958 (i)
TIME TO PEAK (hrs)=	8.000
RUNOFF VOLUME (mm)=	9.794
TOTAL RAINFALL (mm)=	42.000

RUNOFF COEFFICIENT = 0.233

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2522):	287.47	0.958	8.00	9.79
+ ID2= 2 (5252):	32.51	0.456	6.75	31.37
ID = 3 (9252):	319.98	1.334	7.00	11.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8372):	1647.32	2.669	10.00	6.41
+ ID2= 2 (9252):	319.98	1.334	7.00	11.99
ID = 3 (8372):	1967.30	3.668	9.00	7.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6258)
IN= 2---> OUT= 1 Routing time step (min)' = 15.00

Distance	Elevation	Manning	
0.00	252.88	0.0500	
15.47	251.89	0.0500	
46.41	248.45	0.0500	
73.48	245.81	0.0500	
88.95	243.75	0.0500	
112.15	242.00	0.0500	
135.35	240.23	0.0500	
162.42	239.76	0.0500	
170.97	239.52	0.0500 / 0.0350	Main Channel
171.58	239.03	0.0350	Main Channel
174.02	239.03	0.0350	Main Channel
176.46	239.03	0.0350	Main Channel
177.07	239.52	0.0350 / 0.0500	Main Channel
185.63	239.67	0.0500	
208.83	239.87	0.0500	
239.77	240.14	0.0500	
274.57	244.93	0.0500	
336.45	249.51	0.0500	
363.52	249.77	0.0500	
382.85	251.78	0.0500	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.49	239.52	.135E+05	2.5	0.92	91.32
1.14	240.17	.175E+06	25.6	0.73	114.22
1.78	240.81	.532E+06	123.5	1.17	71.85
2.43	241.46	.932E+06	285.3	1.54	54.48
3.07	242.10	.138E+07	506.5	1.85	45.25
3.72	242.75	.186E+07	786.8	2.12	39.42
4.36	243.39	.239E+07	1127.3	2.37	35.33
5.01	244.04	.296E+07	1535.8	2.61	32.12
5.65	244.68	.356E+07	2014.3	2.84	29.48
6.30	245.33	.420E+07	2535.9	3.03	27.60
6.94	245.97	.488E+07	3108.4	3.20	26.16
7.59	246.62	.561E+07	3740.6	3.35	24.98
8.23	247.26	.638E+07	4446.7	3.50	23.93
8.88	247.91	.721E+07	5229.7	3.64	22.98
9.52	248.55	.809E+07	6094.3	3.78	22.12
10.17	249.20	.901E+07	7050.4	3.93	21.31
10.81	249.84	.100E+08	7773.0	3.90	21.46
11.46	250.49	.111E+08	8998.8	4.07	20.56
12.10	251.13	.122E+08	10318.2	4.24	19.75

hydrograph <---> <--- pi pe / channel --->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)

INFLOW : ID= 2 (8372) 1967.30 3.67 9.00 7.32 0.52 0.90
 OUTFLOW: ID= 1 (6258) 1967.30 3.40 11.25 7.32 0.52 0.91

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0258):	181.99	0.722	8.00	12.98
+ ID2= 2 (6258):	1967.30	3.396	11.25	7.32
ID = 3 (8374):	2149.29	3.999	10.75	7.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2601)
 ID= 1 DT=15.0 min Area Total (ha)= 56.22
 Imp(%)= 65.00 Dir. Conn.(%)= 48.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	36.54	19.68
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	612.21	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	55.44	44.84
over (min)=	15.00	30.00
Storage Coeff. (min)=	11.81 (ii)	26.55 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. Tpeak (cms)=	0.08	0.04
PEAK FLOW (cms)=	3.32	1.08
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	40.00	19.27
TOTAL RAINFALL (mm)=	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.46

TOTALS
 4.004 (iii)
 6.00
 29.22
 42.00
 0.70

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 81.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5260)
 IN= 2---> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000		3.7610	1.6799
0.1900	0.8974		4.7220	1.8499
1.0890	1.0678		5.4970	2.0146
2.0290	1.2667		5.8970	2.0246
2.8140	1.4621		0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2601)	56.220	4.004	6.00	29.22
OUTFLOW: ID= 1 (5260)	56.220	0.782	7.00	29.20

PEAK FLOW REDUCTION [Out/Oi n] (%) = 19.53
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 1.0129

CALIB NASHYD (2602)
 ID= 1 DT=15.0 min Area (ha)= 420.02
 Ia (mm)= 5.00 Curve Number (CN)= 83.3
 U. H. Tp(hrs)= 1.30 # of Linear Res. (N)= 1.30

Unit Hyd Opeak (cms)=	3.946
PEAK FLOW (cms)=	1.850 (i)
TIME TO PEAK (hrs)=	8.250
RUNOFF VOLUME (mm)=	15.446
TOTAL RAINFALL (mm)=	42.000

RUNOFF COEFFICIENT = 0.368

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2602):	420.02	1.850	8.25	15.45
+ ID2= 2 (5260):	56.22	0.782	7.00	29.20
ID = 3 (9260):	476.24	2.460	7.00	17.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8374):	2149.29	3.999	10.75	7.80
+ ID2= 2 (9260):	476.24	2.460	7.00	17.07
ID = 3 (8376):	2625.53	5.889	10.00	9.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0262):	341.31	1.720	8.00	14.60
+ ID2= 2 (8376):	2625.53	5.889	10.00	9.48
ID = 3 (8378):	2966.84	7.378	9.75	10.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8378):	2966.84	7.378	9.75	10.07
+ ID2= 2 (8384):	1321.41	3.729	9.00	9.99
ID = 3 (8362):	4288.25	11.073	9.25	10.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6272)
IN= 2--> OUT= 1
Routing time step (mi n)' = 15.00

Distance	Elevation	Manning	
0.00	231.01	0.0450	
23.01	223.65	0.0450	
34.51	222.46	0.0450	
51.77	222.11	0.0450	
69.02	221.87	0.0450	
161.06	221.92	0.0450	
166.81	221.91	0.0450	
172.56	221.89	0.0450	
180.57	221.40	0.0450 / 0.0300	Main Channel
181.57	220.60	0.0300	Main Channel
184.07	220.16	0.0300	Main Channel
195.57	221.85	0.0300 / 0.0450	Main Channel
201.32	221.82	0.0450	
207.07	221.77	0.0450	
212.83	221.72	0.0450	
253.09	222.52	0.0450	
408.40	222.65	0.0450	
460.17	223.20	0.0450	
517.69	224.84	0.0450	
569.46	232.57	0.0450	

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.41	220.57	.189E+04	0.5	0.49	60.18
0.83	220.99	.697E+04	3.2	0.82	35.79
1.24	221.40	.145E+05	8.8	1.07	27.45
1.84	222.00	.643E+05	29.3	0.81	36.61
2.44	222.60	.282E+06	132.9	0.83	35.41
3.04	223.20	.707E+06	413.7	1.04	28.50
3.64	223.80	.118E+07	876.8	1.31	22.46
4.24	224.40	.168E+07	1494.8	1.57	18.75
4.84	225.00	.221E+07	2264.1	1.82	16.23
5.44	225.60	.274E+07	3196.3	2.06	14.29
6.04	226.20	.328E+07	4258.6	2.30	12.84
6.65	226.81	.383E+07	5444.7	2.52	11.72
7.25	227.41	.438E+07	6749.7	2.73	10.82
7.85	228.01	.494E+07	8169.4	2.93	10.08
8.45	228.61	.551E+07	9700.7	3.12	9.46
9.05	229.21	.608E+07	11340.8	3.30	8.94
9.65	229.81	.666E+07	13087.4	3.48	8.48
10.25	230.41	.724E+07	14938.5	3.65	8.08
10.85	231.01	.783E+07	16892.4	3.82	7.73

INFLOW	ID= 2 (8362)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW	ID= 2 (8362)	4288.25	11.07	9.25	10.05	1.31	1.04
OUTFLOW	ID= 1 (6272)	4288.25	10.99	10.00	10.05	1.30	1.04

CALIB
STANDHYD (2721)
ID= 1 DT=15.0 mi n
Area (ha)= 2.21
Total Imp(%)= 49.00
Di r. Conn.(%)= 19.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	1.08	1.13
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	121.38	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	55.44	23.13
Storage Coeff. (mi n)=	15.00	30.00
Unit Hyd. Tpeak (mi n)=	4.47 (ii)	23.69 (ii)
Unit Hyd. peak (cms)=	15.00	30.00
	0.11	0.04
PEAK FLOW (cms)=	0.06	0.05
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	40.00	15.45
TOTAL RAI NFALL (mm)=	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.37

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5272)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.1480	0.0589
0.0070	0.0284	0.1850	0.0669
0.0430	0.0331	0.2160	0.0746
0.0800	0.0419	0.6160	0.0846
0.1100	0.0497	0.0000	0.0000

INFLOW	ID= 2 (2721)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW	ID= 2 (2721)	2.210	0.097	6.00	20.11
OUTFLOW	ID= 1 (5272)	2.210	0.013	7.75	19.64

PEAK FLOW REDUCTION [Qout/Qi n](%)= 13.16
TIME SHIF T OF PEAK FLOW (mi n)=105.00

MAXIMUM STORAGE USED (ha. m.) = 0.0292

CALIB NASHYD (2722)
 ID= 1 DT=15.0 min
 Area (ha) = 155.17
 Curve Number (CN) = 75.3
 U.H. Tp(hrs) = 1.09
 # of Linear Res. (N) = 1.50

Unit Hyd Opeak (cms) = 2.433
 PEAK FLOW (cms) = 0.753 (i)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 11.276
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.268

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9272)
 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (2722): 155.17 0.753 7.75 11.28
 + ID2= 2 (5272): 2.21 0.013 7.75 19.64
 ID = 3 (9272): 157.38 0.766 7.75 11.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8356)
 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (6272): 4288.25 10.989 10.00 10.05
 + ID2= 2 (9272): 157.38 0.766 7.75 11.39
 ID = 3 (8356): 4445.63 11.586 9.75 10.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8352)
 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (8354): 1268.73 6.519 8.25 13.43
 + ID2= 2 (8356): 4445.63 11.586 9.75 10.09
 ID = 3 (8352): 5714.36 17.689 9.00 10.83

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6304)
 IN= 2--> OUT= 1
 Routing time step (min) = 15.00

<----- DATA FOR SECTION (3041.0) ----->

Distance	Elevation	Manning	
0.00	232.08	0.0500	
19.00	231.87	0.0500	
38.00	231.33	0.0500	
66.51	230.44	0.0500	
104.51	228.25	0.0500	
118.76	225.17	0.0500	
128.26	219.86	0.0500	
175.77	219.17	0.0500	
185.27	218.90	0.0500 / 0.0300	Main Channel
185.52	218.65	0.0300	Main Channel
190.02	218.37	0.0300	Main Channel
194.52	218.60	0.0300	Main Channel
194.77	218.85	0.0300 / 0.0500	Main Channel
204.27	219.60	0.0500	
299.28	220.91	0.0500	
327.78	222.36	0.0500	
375.28	225.71	0.0500	
403.79	229.37	0.0500	
432.29	230.43	0.0500	

470.29 232.00 0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.48	218.85	.121E+05	1.2	0.36	173.94
1.17	219.54	.839E+05	9.8	0.44	142.60
1.86	220.23	.323E+06	41.0	0.47	131.43
2.56	220.93	.708E+06	109.5	0.58	107.67
3.25	221.62	.118E+07	224.3	0.71	87.36
3.94	222.31	.168E+07	376.5	0.84	74.48
4.63	223.00	.222E+07	567.9	0.96	65.26
5.32	223.69	.279E+07	794.7	1.07	58.58
6.02	224.39	.339E+07	1056.2	1.17	53.52
6.71	225.08	.402E+07	1352.4	1.26	49.52
7.40	225.77	.468E+07	1677.3	1.34	46.46
8.09	226.46	.536E+07	2050.0	1.43	43.58
8.79	227.16	.607E+07	2457.9	1.52	41.14
9.48	227.85	.680E+07	2901.0	1.60	39.05
10.17	228.54	.755E+07	3352.7	1.66	37.53
10.86	229.23	.834E+07	3812.5	1.71	36.48
11.55	229.92	.919E+07	4241.8	1.73	36.13
12.25	230.62	.101E+08	4697.0	1.74	35.93
12.94	231.31	.111E+08	5186.3	1.74	35.83

<---- hydrograph ----> <- pipe / channel ->
 AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW: ID= 2 (8352) 5714.36 17.69 9.00 10.83
 OUTFLOW: ID= 1 (6304) 5714.36 14.52 11.75 10.83 1.28 0.44

ADD HYD (8350)
 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (0304): 292.37 0.450 10.50 7.34
 + ID2= 2 (6304): 5714.36 14.520 11.75 10.83
 ID = 3 (8350): 6006.73 14.962 11.75 10.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8346)
 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (8348): 3768.46 9.856 10.00 9.55
 + ID2= 2 (8350): 6006.73 14.962 11.75 10.66
 ID = 3 (8346): 9775.19 24.252 11.25 10.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8344)
 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 ID1= 1 (0306): 283.97 0.146 14.00 5.04
 + ID2= 2 (8346): 9775.19 24.252 11.25 10.24
 ID = 3 (8344): 10059.16 24.388 11.25 10.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5509)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	*****	370.0451
41.0590	56.7403	*****	863.4386
48.1390	86.3439	*****	*****
56.6340	*****	*****	*****
67.9600	*****	0.0000	0.0000

AREA OPEAK TPEAK R.V.

INFLOW : ID= 2 (8344) (ha) (cms) (hrs) (mm)
 ***** 24.388 11.25 10.09
 OUTFLOW: ID= 1 (5509) ***** 19.104 14.50 10.09

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 78.33
 TIME SHIF T OF PEAK FLOW (mi n)=195.00
 MAXI MUM STORAGE USED (ha. m.) = 26.4011

CALIB NASHYD (0332) Area (ha)= 393.44 Curve Number (CN)= 75.0
 ID= 1 DT=15.0 mi n Ia (mm)= 5.00 # of Li near Res. (N)= 1.50
 U.H. Tp(hrs)= 2.32

Uni t Hyd Qpeak (cms)= 2.894

PEAK FLOW (cms)= 1.069 (i)
 TIME TO PEAK (hrs)= 10.000
 RUNOFF VOLUME (mm)= 11.219
 TOTAL RAI NFALL (mm)= 42.000
 RUNOFF COEFFI CI ENT = 0.267

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0330) Area (ha)= 468.30 Curve Number (CN)= 80.0
 ID= 1 DT=15.0 mi n Ia (mm)= 5.00 # of Li near Res. (N)= 1.50
 U.H. Tp(hrs)= 1.09

Uni t Hyd Qpeak (cms)= 7.335

PEAK FLOW (cms)= 2.745 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 13.500
 TOTAL RAI NFALL (mm)= 42.000
 RUNOFF COEFFI CI ENT = 0.321

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6332) Routing time step (mi n)'= 15.00
 IN= 2--> OUT= 1

<----- DATA FOR SECTION (3321.0) ----->

Di stance	Elevation	Manning	
0.00	234.00	0.0380	
25.85	227.20	0.0380	
96.94	226.44	0.0380	
168.03	227.38	0.0380	
219.73	225.62	0.0380	
342.52	221.57	0.0380	
368.37	221.42	0.0380	
374.83	221.23	0.0380	
379.79	220.98	0.0380 /0.0300	Main Channel
380.29	220.47	0.0300	Main Channel
381.29	220.47	0.0300	Main Channel
382.79	220.47	0.0300	Main Channel
383.29	220.98	0.0300 /0.0380	Main Channel
394.22	221.22	0.0380	
400.68	221.33	0.0380	
407.14	221.44	0.0380	
491.16	225.70	0.0380	
568.71	227.55	0.0380	
607.49	230.14	0.0380	
639.80	234.08	0.0380	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.51	220.98	.554E+04	0.9	0.56	107.27
1.20	221.67	.885E+05	12.1	0.49	122.32
1.88	222.35	.309E+06	64.7	0.76	79.70
2.57	223.04	.615E+06	165.4	0.97	61.99
3.25	223.72	.101E+07	322.1	1.16	52.06
3.94	224.41	.148E+07	542.9	1.33	45.51
4.62	225.09	.204E+07	835.4	1.48	40.77
5.31	225.78	.269E+07	1202.8	1.62	37.27
5.99	226.46	.344E+07	1618.0	1.70	35.46
6.68	227.15	.447E+07	1932.0	1.56	38.56
7.36	227.83	.580E+07	2763.9	1.72	34.96

8.05 228.52 .718E+07 3861.9 1.95 30.97
 8.73 229.20 .859E+07 5115.4 2.16 27.98
 9.42 229.89 .100E+08 6519.2 2.35 25.64
 10.10 230.57 .115E+08 8092.3 2.55 23.69
 10.79 231.26 .130E+08 9821.5 2.73 22.05
 11.47 231.94 .145E+08 11690.0 2.92 20.68
 12.16 232.63 .160E+08 13694.9 3.09 19.52
 12.84 233.31 .176E+08 15833.9 3.26 18.52

<---- hydrograph ----> <-pi pe / channel ->

AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0330) 468.30 2.74 7.50 13.50 0.63 0.55
 OUTFLOW: ID= 1 (6332) 468.30 2.16 9.75 13.50 0.59 0.55

ADD HYD (8322)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0332): 393.44 1.069 10.00 11.22
 + ID2= 2 (6332): 468.30 2.156 9.75 13.50
 ID = 3 (8322): 861.74 3.223 9.75 12.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0328) Area (ha)= 492.92 Curve Number (CN)= 77.0
 ID= 1 DT=15.0 mi n Ia (mm)= 5.00 # of Li near Res. (N)= 1.50
 U.H. Tp(hrs)= 1.91

Uni t Hyd Qpeak (cms)= 4.411

PEAK FLOW (cms)= 1.670 (i)
 TIME TO PEAK (hrs)= 9.000
 RUNOFF VOLUME (mm)= 12.082
 TOTAL RAI NFALL (mm)= 42.000
 RUNOFF COEFFI CI ENT = 0.288

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (0326) Area (ha)= 678.91 Curve Number (CN)= 80.0
 ID= 1 DT=15.0 mi n Ia (mm)= 5.00 # of Li near Res. (N)= 1.50
 U.H. Tp(hrs)= 1.95

Uni t Hyd Qpeak (cms)= 5.941

PEAK FLOW (cms)= 2.547 (i)
 TIME TO PEAK (hrs)= 9.000
 RUNOFF VOLUME (mm)= 13.570
 TOTAL RAI NFALL (mm)= 42.000
 RUNOFF COEFFI CI ENT = 0.323

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3181) Area (ha)= 357.30 Di r. Conn. (%) = 32.00
 ID= 1 DT=15.0 mi n Total Imp(%)= 56.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 200.09 157.21
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1543.37 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 37.80 21.86
 over (mi n) 30.00 45.00
 Storage Coeff. (mi n)= 23.97 (ii) 43.63 (ii)
 Uni t Hyd. Tpeak (mi n)= 30.00 45.00
 Uni t Hyd. peak (cms)= 0.04 0.03

TOTALS
 PEAK FLOW (cms)= 8.72 4.83 12.259 (iii)
 TIME TO PEAK (hrs)= 6.25 6.50 6.25

RUNOFF VOLUME (mm)= 40.00 15.05 23.03
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.36 0.55

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5318)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	22.8580	8.7280
0.8190	2.5190	27.5380	9.6853
4.2860	5.8947	32.5800	10.8345
11.0600	6.7336	32.9800	10.8445
16.3830	7.4416	0.0000	0.0000

INFLOW : ID= 2 (3181)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5318)	357.300	12.259	6.25	23.03
	357.300	2.969	7.75	23.03

PEAK FLOW REDUCTION [Qout/Oin] (%) = 24.22
 TIME SHIFT OF PEAK FLOW (min) = 90.00
 MAXIMUM STORAGE USED (ha.m.) = 4.6144

CALIB STANDHYD (3182)
 ID= 1 DT=15.0 min

Area (ha) = 126.13
 Total Imp (%) = 21.00
 Dir. Conn. (%) = 12.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	26.49	99.64
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	916.99	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 55.44 11.72
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 15.05 (ii) 40.27 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.07 0.03

TOTALS
 PEAK FLOW (cms) = 1.70 1.73 2.374 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 40.00 11.51 14.93
 TOTAL RAINFALL (mm) = 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.27 0.36

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8334)
 1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3182):	126.13	2.374	6.00	14.93
+ ID2= 2 (5318):	357.30	2.969	7.75	23.03
ID = 3 (8334):	483.43	3.978	7.00	20.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6320)
 IN= 2--> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (3201.0) ----->

Distance	Elevation	Manning
0.00	249.00	0.0380
22.92	245.86	0.0380
45.83	244.87	0.0380
91.66	243.11	0.0380
126.03	239.53	0.0380
160.41	237.17	0.0380
166.14	237.06	0.0380
177.59	237.13	0.0380
183.32	237.20	0.0380
189.05	236.70	0.0380 / 0.0350
193.05	235.89	0.0350
197.05	236.64	0.0350 / 0.0380
200.51	236.74	0.0380
206.24	237.03	0.0380
246.34	238.82	0.0380
263.53	243.87	0.0380
389.56	247.64	0.0380
452.58	247.74	0.0380
498.41	248.60	0.0380
567.16	249.84	0.0380

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.37	236.26	270E+04	0.4	0.58	107.96
0.75	236.64	108E+05	2.6	0.92	68.01
1.48	237.37	877E+05	27.4	1.17	53.35
2.20	238.09	277E+06	122.1	1.65	37.83
2.93	238.82	540E+06	299.7	2.08	30.01
3.66	239.55	857E+06	587.8	2.57	24.29
4.39	240.28	120E+07	977.8	3.04	20.52
5.11	241.00	158E+07	1458.1	3.46	18.02
5.84	241.73	198E+07	2028.6	3.84	16.23
6.57	242.46	240E+07	2690.5	4.19	14.86
7.29	243.18	285E+07	3431.8	4.50	13.84
8.02	243.91	335E+07	4149.1	4.64	13.44
8.75	244.64	393E+07	4746.6	4.51	13.80
9.47	245.36	463E+07	5618.6	4.53	13.75
10.20	246.09	545E+07	6781.8	4.66	13.39
10.93	246.82	635E+07	8274.5	4.87	12.79
11.66	247.55	733E+07	9988.0	5.10	12.23
12.38	248.27	853E+07	11422.0	5.01	12.45
13.11	249.00	989E+07	13749.7	5.20	11.98

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8334)	483.43	3.98	7.00	20.92	0.79	0.93
OUTFLOW: ID= 1 (6320)	483.43	3.36	8.50	20.92	0.77	0.92

CALIB NASHYD (3202)
 ID= 1 DT=15.0 min

Area (ha) = 84.56
 Ia (mm) = 5.00
 U. H. Tp (hrs) = 0.70
 Curve Number (CN) = 80.9
 # of Linear Res. (N) = 1.50

Unit Hyd Qpeak (cms) = 2.054
 PEAK FLOW (cms) = 0.723 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 13.875
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.330

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3201)
 ID= 1 DT=15.0 min

Area (ha) = 194.18
 Total Imp (%) = 45.00
 Dir. Conn. (%) = 16.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	87.38	106.80
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1137.78	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 55.44 25.79

over (min) 15.00 45.00
 Storage Coeff. (min)= 17.13 (ii) 35.53 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.07 0.03

TOTALS
 5.421 (iii)
 6.50
 21.21
 42.00
 0.50

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 78.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5320)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	7.4940	4.4477
0.2490	1.8139	9.8910	4.7762
1.2830	3.1517	12.4550	5.0763
3.7490	3.7610	12.8550	5.0863
5.0040	4.0198	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (3201)	194.180	5.421	6.50	21.21
OUTFLOW: ID= 1 (5320)	194.180	0.961	8.50	21.20

PEAK FLOW REDUCTION [Qout/Qin] (%) = 17.73
 TIME SHIFT OF PEAK FLOW (min) = 120.00
 MAXIMUM STORAGE USED (ha.m.) = 2.7368

ADD HYD (9320)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3202):	84.56	0.723	6.75	13.88
+ ID2= 2 (5320):	194.18	0.961	8.50	21.20
=====				
ID = 3 (9320):	278.74	1.542	8.00	18.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8332)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6320):	483.43	3.359	8.50	20.92
+ ID2= 2 (9320):	278.74	1.542	8.00	18.97
=====				
ID = 3 (8332):	762.17	4.874	8.25	20.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (3142)
 ID= 1 DT=15.0 min

Area (ha) = 122.75 Curve Number (CN) = 78.0
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp (hrs) = 0.53

Unit Hyd Opeak (cms) = 3.924

PEAK FLOW (cms) = 1.153 (i)
 TIME TO PEAK (hrs) = 6.500
 RUNOFF VOLUME (mm) = 12.276
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.292

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (3141)
 ID= 1 DT=15.0 min

Area Total Imp (%) = 42.46 70.00
 Dir. Conn. (%) = 46.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	29.72	12.74
Dep. Storage	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	532.04	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	55.44	46.55
over (min)	15.00	30.00
Storage Coeff. (min)	10.86 (ii)	25.38 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04

PEAK FLOW (cms) = 2.48 0.74
 TIME TO PEAK (hrs) = 6.00 6.25
 RUNOFF VOLUME (mm) = 40.00 16.78
 TOTAL RAINFALL (mm) = 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.40

TOTALS
 2.941 (iii)
 6.00
 27.46
 42.00
 0.65

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5314)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.8400	1.3064
0.1440	0.7144	3.5660	1.4269
0.8230	0.8540	4.1520	1.5451
1.5320	0.9987	4.5520	1.5551
2.1250	1.1456	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (3141)	42.460	2.941	6.00	27.46
OUTFLOW: ID= 1 (5314)	42.460	0.400	7.25	27.43

PEAK FLOW REDUCTION [Qout/Qin] (%) = 13.61
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha.m.) = 0.7689

ADD HYD (9314)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3142):	122.75	1.153	6.50	12.28
+ ID2= 2 (5314):	42.46	0.400	7.25	27.43
=====				
ID = 3 (9314):	165.21	1.478	7.00	16.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6316)
 IN= 2---> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (3161.0) ----->

Distance	Elevation	Manning
0.00	248.54	0.0350
27.93	248.34	0.0350
41.89	246.61	0.0350
62.83	243.09	0.0350
132.65	239.00	0.0350
188.50	236.74	0.0350
195.48	236.54	0.0350
202.46	236.32	0.0350
205.48	236.14	0.0350 / 0.0310
205.98	235.61	0.0310

Main Channel
 Main Channel

207.98	235.25	0.0310	Main Channel
209.98	235.53	0.0310	Main Channel
210.48	236.00	0.0310 / 0.0350	Main Channel
216.42	236.73	0.0350	
258.31	239.09	0.0350	
328.12	239.84	0.0350	
439.83	241.47	0.0350	
530.58	242.08	0.0350	
586.43	242.93	0.0350	
691.16	248.00	0.0350	

TRAVEL TIME TABLE					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.37	235.62	.215E+04	0.7	0.77	53.88
0.75	236.00	.634E+04	3.5	1.36	30.45
1.46	236.71	.297E+05	19.3	1.62	25.66
2.16	237.41	.103E+06	75.8	1.84	22.60
2.87	238.12	.229E+06	202.8	2.21	18.79
3.57	238.82	.407E+06	423.3	2.59	16.03
4.28	239.53	.654E+06	727.7	2.77	14.97
4.99	240.24	.103E+07	1242.0	3.01	13.76
5.69	240.94	.151E+07	2002.3	3.31	12.54
6.40	241.65	.210E+07	2987.9	3.55	11.70
7.10	242.35	.286E+07	4302.8	3.75	11.07
7.81	243.06	.374E+07	6165.3	4.10	10.11
8.51	243.76	.468E+07	8642.4	4.60	9.03
9.22	244.47	.568E+07	11496.3	5.06	8.20
9.93	245.18	.666E+07	14714.1	5.50	7.54
10.63	245.88	.770E+07	18290.3	5.91	7.02
11.34	246.59	.877E+07	22221.9	6.30	6.58
12.04	247.29	.988E+07	26464.4	6.67	6.22
12.75	248.00	.110E+08	31061.3	7.01	5.91

hydrograph					pi pe / channel	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (9314)	165.21	1.48	7.00	16.17	0.48	0.88
OUTFLOW: ID= 1 (6316)	165.21	1.21	7.75	16.17	0.45	0.84

CALIB NASHYD (3162)	Area (ha)= 196.72	Curve Number (CN)= 83.0
ID= 1 DT=15.0 mi n	Ia (mm)= 5.00	# of Li near Res. (N)= 1.50
	U. H. Tp(hrs)= 0.88	

Unit Hyd Qpeak (cms)= 3.838

PEAK FLOW (cms)= 1.556 (i)

TIME TO PEAK (hrs)= 7.000

RUNOFF VOLUME (mm)= 15.187

TOTAL RAINFALL (mm)= 42.000

RUNOFF COEFFICIENT = 0.362

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3161)	Area (ha)= 35.62	Dir. Conn. (%)= 44.00
ID= 1 DT=15.0 mi n	Total Imp(%)= 63.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	22.44	13.18
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	487.31	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 55.44 46.22

over (mi n) 15.00 30.00

Storage Coeff. (mi n)= 10.30 (ii) 24.87 (ii)

Unit Hyd. Tpeak (mi n)= 15.00 30.00

Unit Hyd. peak (cms)= 0.09 0.04

TOTALS

PEAK FLOW (cms)= 2.02 0.77 2.508 (iii)

TIME TO PEAK (hrs)= 6.00 6.25 6.00

RUNOFF VOLUME (mm)= 40.00 19.50 28.52

TOTAL RAINFALL (mm)= 42.00 42.00 42.00

RUNOFF COEFFICIENT = 0.95 0.46 0.68

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 81.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5316)				
IN= 2--> OUT= 1				
DT= 15.0 mi n				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	2.3160	0.9307
	0.0930	0.5020	2.9060	1.0021
	0.5400	0.6637	3.5360	1.0731
	1.1940	0.7495	3.9360	1.0831
	1.6920	0.8359	0.0000	0.0000
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3161)	35.620	2.508	6.00	28.52
OUTFLOW: ID= 1 (5316)	35.620	0.439	7.00	28.48
PEAK FLOW REDUCTION [Qout/Qi n] (%) = 17.52				
TIME SHI FT OF PEAK FLOW (mi n) = 60.00				
MAXI MUM STORAGE USED (ha. m.) = 0.6274				

ADD HYD (9316)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3162):	196.72	1.556	7.00	15.19
+ ID2= 2 (5316):	35.62	0.439	7.00	28.48
=====				
ID = 3 (9316):	232.34	1.996	7.00	17.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8338)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6316):	165.21	1.206	7.75	16.17
+ ID2= 2 (9316):	232.34	1.996	7.00	17.22
=====				
ID = 3 (8338):	397.55	3.121	7.50	16.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0312)	Area (ha)= 359.44	Curve Number (CN)= 80.0
ID= 1 DT=15.0 mi n	Ia (mm)= 5.00	# of Li near Res. (N)= 1.50
	U. H. Tp(hrs)= 1.36	

Unit Hyd Qpeak (cms)= 4.529

PEAK FLOW (cms)= 1.796 (i)

TIME TO PEAK (hrs)= 8.000

RUNOFF VOLUME (mm)= 13.533

TOTAL RAINFALL (mm)= 42.000

RUNOFF COEFFICIENT = 0.322

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0308)	Area (ha)= 529.30	Curve Number (CN)= 62.0
ID= 1 DT=15.0 mi n	Ia (mm)= 5.00	# of Li near Res. (N)= 1.50
	U. H. Tp(hrs)= 1.62	

Unit Hyd Qpeak (cms)= 5.575

PEAK FLOW (cms)= 1.176 (i)

TIME TO PEAK (hrs)= 8.500

RUNOFF VOLUME (mm)= 7.070

TOTAL RAINFALL (mm)= 42.000

RUNOFF COEFFICIENT = 0.168

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALLIB (0310) Area (ha) = 138.28 Curve Number (CN) = 65.0
 NASHYD (0310) Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 ID= 1 DT=15.0 min U. H. Tp(hrs) = 0.76

Unit Hyd Opeak (cms) = 3.102

PEAK FLOW (cms) = 0.603 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 7.758
 TOTAL RAINFALL (mm) = 42.000
 RUNOFF COEFFICIENT = 0.185

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8342) | AREA OPEAK TPEAK R. V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0308): 529.30 1.176 8.50 7.07
 + ID2= 2 (0310): 138.28 0.603 7.00 7.76

 ID = 3 (8342): 667.58 1.702 8.00 7.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6312) | Routing time step (min)' = 15.00
 IN= 2----> OUT= 1

<----- DATA FOR SECTION (3121.0) ----->

Distance	Elevation	Manning	
0.00	265.94	0.0360	
38.07	265.43	0.0360	
59.82	263.98	0.0360	
103.32	254.59	0.0360	
157.70	252.16	0.0360	
217.52	250.45	0.0360	
233.84	247.69	0.0360	
234.71	247.27	0.0360	
239.71	246.38	0.0360	
244.71	246.12	0.0360 / 0.0330	Main Channel
247.71	245.17	0.0330	Main Channel
249.71	245.19	0.0330	Main Channel
251.71	245.64	0.0330 / 0.0360	Main Channel
259.71	246.67	0.0360	
282.78	247.12	0.0360	
315.41	251.60	0.0360	
424.17	256.13	0.0360	
478.55	257.04	0.0360	
516.62	259.37	0.0360	
538.37	266.00	0.0360	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.47	245.64	.111E+04	3.1	1.78	6.07
1.54	246.71	.103E+05	53.9	3.39	3.19
2.61	247.78	.414E+05	281.3	4.40	2.45
3.68	248.85	.839E+05	749.1	5.79	1.87
4.74	249.91	.136E+06	1463.2	6.97	1.55
5.81	250.98	.201E+06	2354.8	7.61	1.42
6.88	252.05	.295E+06	3533.0	7.76	1.39
7.95	253.12	.427E+06	5303.9	8.05	1.34
9.02	254.19	.594E+06	7856.5	8.58	1.26
10.09	255.26	.792E+06	11449.2	9.37	1.15
11.15	256.32	.101E+07	15862.4	10.15	1.06
12.22	257.39	.127E+07	21074.5	10.75	1.01
13.29	258.46	.155E+07	28184.1	11.78	0.92
14.36	259.53	.185E+07	36386.2	12.77	0.85
15.43	260.60	.215E+07	46110.6	13.89	0.78
16.50	261.67	.246E+07	56822.5	14.95	0.72
17.56	262.73	.278E+07	68498.1	15.97	0.68
18.63	263.80	.310E+07	81118.8	16.94	0.64
19.70	264.87	.344E+07	93603.2	17.66	0.61

<---- hydrograph ----> <-pi pe / channel ->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8342) 667.58 1.70 8.00 7.21 0.26 1.78
 OUTFLOW: ID= 1 (6312) 667.58 1.70 8.00 7.21 0.26 1.78

 ADD HYD (8340) | AREA OPEAK TPEAK R. V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0312): 359.44 1.796 8.00 13.53
 + ID2= 2 (6312): 667.58 1.699 8.00 7.21

 ID = 3 (8340): 1027.02 3.494 8.00 9.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8336) | AREA OPEAK TPEAK R. V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (8338): 397.55 3.121 7.50 16.79
 + ID2= 2 (8340): 1027.02 3.494 8.00 9.42

 ID = 3 (8336): 1424.57 6.538 7.50 11.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8330) | AREA OPEAK TPEAK R. V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (8332): 762.17 4.874 8.25 20.21
 + ID2= 2 (8336): 1424.57 6.538 7.50 11.48

 ID = 3 (8330): 2186.74 11.309 8.00 14.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6324) | Routing time step (min)' = 15.00
 IN= 2----> OUT= 1

<----- DATA FOR SECTION (3241.0) ----->

Distance	Elevation	Manning	
0.00	234.38	0.0360	
33.57	232.48	0.0360	
67.14	230.14	0.0360	
83.93	228.80	0.0360	
134.29	227.62	0.0360	
209.82	225.10	0.0360	
218.21	224.86	0.0360	
226.61	224.47	0.0360	
234.00	223.86	0.0360 / 0.0300	Main Channel
234.10	223.66	0.0300	Main Channel
235.00	223.66	0.0300	Main Channel
235.90	223.66	0.0300	Main Channel
236.00	223.86	0.0300 / 0.0360	Main Channel
243.39	224.92	0.0360	
251.78	224.89	0.0360	
335.71	225.64	0.0360	
562.32	226.53	0.0360	
637.85	228.36	0.0360	
705.00	229.80	0.0360	
830.89	234.00	0.0360	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.20	223.86	.145E+04	0.2	0.43	147.27
0.73	224.39	.159E+05	2.8	0.68	93.11
1.27	224.93	.561E+05	11.0	0.75	85.19
1.80	225.46	.217E+06	45.6	0.80	79.20
2.33	225.99	.566E+06	134.4	0.91	70.20
2.87	226.53	.122E+07	338.1	1.06	59.97

3.40	227.06	.206E+07	744.3	1.38	46.16
3.94	227.60	.298E+07	1295.9	1.66	38.38
4.47	228.13	.399E+07	1979.9	1.90	33.59
5.00	228.66	.509E+07	2807.5	2.11	30.21
5.54	229.20	.627E+07	3815.1	2.32	27.41
6.07	229.73	.753E+07	4984.1	2.53	25.17
6.60	230.26	.884E+07	6346.3	2.74	23.20
7.14	230.80	.102E+08	7861.2	2.95	21.61
7.67	231.33	.116E+08	9526.4	3.14	20.29
8.21	231.87	.131E+08	11342.2	3.32	19.18
8.74	232.40	.146E+08	13309.6	3.49	18.22
9.27	232.93	.161E+08	15414.0	3.66	17.41
9.81	233.47	.177E+08	17671.0	3.81	16.70

<---- hydrograph ---->					
<- pi pe / channel ->					
	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH
	(ha)	(cms)	(hrs)	(mm)	(m)
INFLOW : ID= 2 (8330)	2186.74	11.31	8.00	14.52	1.27
OUTFLOW: ID= 1 (6324)	2186.74	9.41	9.50	14.52	1.16

CALIB NASHYD (3242)	Area (ha)= 602.18	Curve Number (CN)= 78.8
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50
	U. H. Tp(hrs)= 1.80	

Unit Hyd Qpeak (cms)= 5.708
 PEAK FLOW (cms)= 2.290 (i)
 TIME TO PEAK (hrs)= 8.750
 RUNOFF VOLUME (mm)= 12.941
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.308

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3241)	Area (ha)= 13.46	Dir. Conn. (%)= 15.00
ID= 1 DT=15.0 min	Total Imp(%)= 45.00	

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)=	6.06		7.40
Dep. Storage (mm)=	2.00		5.00
Average Slope (%)=	0.50		0.50
Length (m)=	299.56		40.00
Mannings n =	0.013		0.250
Max. Eff. Inten. (mm/hr)=	55.44		26.32
over (min)=	15.00		30.00
Storage Coeff. (min)=	7.69 (ii)		25.94 (ii)
Unit Hyd. Tpeak (min)=	15.00		30.00
Unit Hyd. peak (cms)=	0.10		0.04
TOTALS			
PEAK FLOW (cms)=	0.28	0.39	0.526 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	40.00	17.77	21.10
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.42	0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 78.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5324)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2----> OUT= 1	(cms)	(ha. m.)	(cms)	(ha. m.)
DT= 15.0 min	0.0000	0.0000	0.9000	0.3505
	0.0460	0.1645	1.1310	0.4008
	0.2610	0.1903	1.3160	0.4499
	0.4860	0.2456	1.7160	0.4599
	0.6740	0.2932	0.0000	0.0000

INFLOW : ID= 2 (3241)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5324)	13.460	0.526	6.00	21.10
	13.460	0.114	7.25	21.03

PEAK FLOW REDUCTION [Qout/Qin] (%) = 21.64
 TIME SHIF OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha. m.) = 0.1732

ADD HYD (9324)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (3242):	602.18	2.290	8.75	12.94
+ ID2= 2 (5324):	13.46	0.114	7.25	21.03
ID = 3 (9324):	615.64	2.354	8.50	13.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8328)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (6324):	2186.74	9.411	9.50	14.52
+ ID2= 2 (9324):	615.64	2.354	8.50	13.12
ID = 3 (8328):	2802.38	11.718	9.50	14.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0322)	Area (ha)= 513.13	Curve Number (CN)= 80.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50
	U. H. Tp(hrs)= 1.75	

Unit Hyd Qpeak (cms)= 5.020
 PEAK FLOW (cms)= 2.099 (i)
 TIME TO PEAK (hrs)= 8.500
 RUNOFF VOLUME (mm)= 13.561
 TOTAL RAINFALL (mm)= 42.000
 RUNOFF COEFFICIENT = 0.323

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8326)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0322):	513.13	2.099	8.50	13.56
+ ID2= 2 (8328):	2802.38	11.718	9.50	14.21
ID = 3 (8326):	3315.51	13.778	9.50	14.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8324)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0326):	678.91	2.547	9.00	13.57
+ ID2= 2 (8326):	3315.51	13.778	9.50	14.11
ID = 3 (8324):	3994.42	16.316	9.50	14.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6328)	Routing time step (min) = 15.00
IN= 2----> OUT= 1	

<----- DATA FOR SECTION (3281.0) ----->

Distance	Elevation	Manning	
0.00	228.00	0.0380	
18.58	224.97	0.0380	
78.98	223.52	0.0380	
125.44	223.28	0.0380	
171.90	221.71	0.0380	
213.72	219.65	0.0380	
218.36	219.40	0.0380	
223.01	219.19	0.0380	
225.95	219.14	0.0380 / 0.0320	Main Channel
226.45	218.14	0.0320	Main Channel
236.95	218.14	0.0320	Main Channel
245.85	218.14	0.0320	Main Channel
245.95	219.14	0.0320 / 0.0380	Main Channel
246.24	219.16	0.0380	
250.88	219.24	0.0380	
255.53	219.39	0.0380	
325.22	221.47	0.0380	
367.03	223.14	0.0380	
404.20	225.17	0.0380	
459.95	228.04	0.0380	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.50	218.64	.373E+05	3.2	0.33	192.37
1.00	219.14	.752E+05	10.1	0.51	124.71
1.52	219.66	.149E+06	22.2	0.57	112.06
2.04	220.18	.279E+06	43.4	0.59	107.06
2.56	220.70	.464E+06	77.0	0.63	100.52
3.08	221.22	.705E+06	125.7	0.68	93.56
3.61	221.75	.100E+07	192.7	0.73	86.62
4.13	222.27	.135E+07	278.1	0.79	80.92
4.65	222.79	.176E+07	385.9	0.84	75.86
5.17	223.31	.222E+07	514.9	0.89	71.82
5.69	223.83	.280E+07	662.7	0.90	70.40
6.21	224.35	.345E+07	867.7	0.96	66.35
6.73	224.87	.417E+07	1107.0	1.01	62.81
7.25	225.39	.494E+07	1400.0	1.08	58.80
7.78	225.92	.573E+07	1732.2	1.15	55.17
8.30	226.44	.655E+07	2099.9	1.22	52.02
8.82	226.96	.740E+07	2502.8	1.29	49.29
9.34	227.48	.828E+07	2940.9	1.36	46.90
9.86	228.00	.918E+07	3414.3	1.42	44.79

<---- hydrograph ---->					<- pi pe / channel ->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)	
INFLOW: ID= 2 (8324)	3994.42	16.32	9.50	14.02	1.27	
OUTFLOW: ID= 1 (6328)	3994.42	13.67	11.25	14.02	0.53	

ADD HYD (8320)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0328):	492.92	1.670	9.00	12.08
+ ID2= 2 (6328):	3994.42	13.669	11.25	14.02
ID = 3 (8320):	4487.34	15.203	11.00	13.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8318)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8320):	4487.34	15.203	11.00	13.81
+ ID2= 2 (8322):	861.74	3.223	9.75	12.46
ID = 3 (8318):	5349.08	18.289	10.75	13.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8316)				
1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.

	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (5509):	10059.16	19.104	14.50	10.09
+ ID2= 2 (8318):	5349.08	18.289	10.75	13.59
ID = 3 (8316):	15408.24	34.763	12.75	11.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8312)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8314):	620.10	0.341	8.00	6.27
+ ID2= 2 (8316):	15408.24	34.763	12.75	11.30
ID = 3 (8312):	16028.34	35.069	12.75	11.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8308)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8310):	598.91	8.998	6.25	17.06
+ ID2= 2 (8312):	16028.34	35.069	12.75	11.11
ID = 3 (8308):	16627.25	36.062	12.25	11.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5510)			
IN= 2----> OUT= 1			
DT= 15.0 mi n			
OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	*****	74.0090
66.5450	18.5023	*****	*****
98.5430	37.0045	*****	*****

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (8308)	*****	36.062	12.25	11.32
OUTFLOW: ID= 1 (5510)	*****	35.558	13.25	11.32

PEAK FLOW REDUCTION [Out/Oi n] (%) = 98.60
 TIME SHIFT OF PEAK FLOW (mi n) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 9.8897

ADD HYD (8240)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0336):	2785.00	1.223	22.50	10.07
+ ID2= 2 (5510):	16627.25	35.558	13.25	11.32
ID = 3 (8240):	19412.25	36.601	13.25	11.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8238)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (5508):	9524.19	14.957	12.25	6.51
+ ID2= 2 (8240):	19412.25	36.601	13.25	11.15
ID = 3 (8238):	28936.44	51.353	13.00	9.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (3427)			
ID= 1 DT=15.0 mi n	Area (ha)	Total Imp (%)	Di r. Conn. (%)
	249.40	45.00	33.00

Surface Area (ha)=	112.23	PERVIOUS (i)	137.17
Dep. Storage (mm)=	2.00		5.00
Average Slope (%)=	0.50		0.50
Length (m)=	1289.44		40.00
Mannings n =	0.013		0.250

Max. Eff. Inten. (mm/hr)=	55.44	11.27
over (min)	15.00	45.00
Storage Coeff. (min)=	18.47 (ii)	44.08 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.06	0.03

TOTALS

PEAK FLOW (cms)=	8.40	2.18	9.251 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	40.00	10.21	20.04
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.24	0.48

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 67.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9425)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	12.4700	5.4211
0.8500	3.0375	16.6800	6.4838
4.8400	3.5126	20.9500	7.4161
9.0000	4.5390	24.3900	8.3284

INFLOW : ID= 2 (3427)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (9425)	249.400	9.251	6.00	20.04
	249.400	1.618	7.75	20.04

PEAK FLOW REDUCTION [Qout/Qin] (%) = 17.49
TIME SHIFT OF PEAK FLOW (min) = 105.00
MAXIMUM STORAGE USED (ha.m.) = 3.1322

RESERVOIR (5342)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.6600	12.0067
1.2080	11.1244	4.4200	12.4001
2.0570	11.3002	5.2300	12.7452
2.7240	11.3602	0.0000	0.0000

INFLOW : ID= 2 (9425)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5342)	249.400	1.618	7.75	20.04
	249.400	0.296	21.75	20.03

PEAK FLOW REDUCTION [Qout/Qin] (%) = 18.32
TIME SHIFT OF PEAK FLOW (min) = 840.00
MAXIMUM STORAGE USED (ha.m.) = 2.7302

CALIB NASHYD (3422)
ID= 1 DT=15.0 min

Area (ha) = 755.17 Curve Number (CN) = 54.5
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U.H. Tp (hrs) = 2.57

Unit Hyd Opeak (cms) = 5.026

PEAK FLOW (cms)=	0.924 (i)
TIME TO PEAK (hrs)=	10.250
RUNOFF VOLUME (mm)=	5.483
TOTAL RAINFALL (mm)=	42.000
RUNOFF COEFFICIENT =	0.131

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9342)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3422):	755.17	0.924	10.25	5.48
+ ID2= 2 (5342):	249.40	0.296	21.75	20.03
=====				
ID = 3 (9342):	1004.57	1.100	11.25	9.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8236)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8238):	28936.44	51.353	13.00	9.62
+ ID2= 2 (9342):	1004.57	1.100	11.25	9.09
=====				
ID = 3 (8236):	29941.01	52.403	13.00	9.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8234)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8232):	285.79	4.643	6.00	16.97
+ ID2= 2 (8236):	29941.01	52.403	13.00	9.60
=====				
ID = 3 (8234):	30226.80	52.798	12.75	9.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (3442)
ID= 1 DT=15.0 min

Area (ha) = 188.84
Total Imp (%) = 32.00
Dir. Conn. (%) = 24.00

Surface Area (ha)=	60.43	PERVIOUS (i)	128.41
Dep. Storage (mm)=	2.00		5.00
Average Slope (%)=	0.50		0.50
Length (m)=	1122.02		40.00
Mannings n =	0.013		0.250

Max. Eff. Inten. (mm/hr)=	55.44	4.48
over (min)	15.00	60.00
Storage Coeff. (min)=	16.99 (ii)	54.05 (ii)
Unit Hyd. Tpeak (min)=	15.00	60.00
Unit Hyd. peak (cms)=	0.07	0.02

TOTALS

PEAK FLOW (cms)=	4.81	0.88	5.059 (iii)
TIME TO PEAK (hrs)=	6.00	6.75	6.00
RUNOFF VOLUME (mm)=	40.00	5.89	14.08
TOTAL RAINFALL (mm)=	42.00	42.00	42.00
RUNOFF COEFFICIENT =	0.95	0.14	0.34

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 53.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3441)
ID= 1 DT=15.0 min

Area (ha) = 155.15
Total Imp (%) = 39.00
Dir. Conn. (%) = 29.00

Surface Area (ha)=	60.51	PERVIOUS (i)	94.64
Dep. Storage (mm)=	2.00		5.00
Average Slope (%)=	0.50		0.50
Length (m)=	1017.02		40.00
Mannings n =	0.013		0.250

Max. Eff. Inten. (mm/hr)= 55.44 4.86
 over (min) 15.00 60.00
 Storage Coeff. (min)= 16.01 (ii) 51.87 (ii)
 Unit Hyd. Tpeak (min)= 15.00 60.00
 Unit Hyd. peak (cms)= 0.07 0.02

PEAK FLOW (cms)= 4.91 0.73 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.75 5.112 (iii)
 RUNOFF VOLUME (mm)= 40.00 6.15 6.00
 TOTAL RAINFALL (mm)= 42.00 42.00 42.00
 RUNOFF COEFFICIENT = 0.95 0.15 0.38

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 53.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5344)				
IN= 2---> OUT= 1				
DT= 15.0 min				
OUTFLOW	STORAGE	OUTFLOW	STORAGE	
(cms)	(ha.m.)	(cms)	(ha.m.)	
0.0000	0.0000	2.6700	6.1252	
0.3000	1.6125	3.3150	7.0314	
0.8100	3.0036	3.8990	7.9700	
1.4170	4.2559	4.2990	7.9800	
1.8540	5.0526	0.0000	0.0000	
AREA	OPEAK	TPEAK	R. V.	
(ha)	(cms)	(hrs)	(mm)	
INFLOW: ID= 2 (3441)	155.150	5.112	6.00	15.97
OUTFLOW: ID= 1 (5344)	155.150	0.359	9.50	15.96

PEAK FLOW REDUCTION [Qout/Qin] (%) = 7.03
 TIME SHIFT OF PEAK FLOW (min) = 210.00
 MAXIMUM STORAGE USED (ha.m.) = 1.7739

ADD HYD (9344)				
1 + 2 = 3				
AREA	OPEAK	TPEAK	R. V.	
(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (3442):	188.84	5.059	6.00	14.08
+ ID2= 2 (5344):	155.15	0.359	9.50	15.96
=====				
ID = 3 (9344):	343.99	5.190	6.00	14.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8230)				
1 + 2 = 3				
AREA	OPEAK	TPEAK	R. V.	
(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (8234):	30226.80	52.798	12.75	9.67
+ ID2= 2 (9344):	343.99	5.190	6.00	14.92
=====				
ID = 3 (8230):	30570.79	53.191	12.75	9.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8228)				
1 + 2 = 3				
AREA	OPEAK	TPEAK	R. V.	
(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (8226):	1952.06	3.950	8.75	5.65
+ ID2= 2 (8230):	30570.79	53.191	12.75	9.73
=====				
ID = 3 (8228):	32522.85	55.690	12.50	9.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8190)				
1 + 2 = 3				
AREA	OPEAK	TPEAK	R. V.	

(ha) (cms) (hrs) (mm)
 ID1= 1 (0356): 1015.22 1.461 8.25 4.06
 + ID2= 2 (8228): 32522.85 55.690 12.50 9.49
 =====
 ID = 3 (8190): 33538.07 56.608 12.50 9.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

** SIMULATION NUMBER: 2 **

READ STORM
 Ptotal = 54.40 mm
 Filename: C:\Users\aman\lucu\AppData\Local\Temp\ab70bd54-a452-450c-b03c-83cdc4826a98\10083009
 Comments: 5-Year 12-Hour SCS II Design Storm

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.36	3.25	2.18	6.25	9.79	9.25	1.90
0.50	1.36	3.50	2.18	6.50	9.79	9.50	1.90
0.75	1.36	3.75	2.18	6.75	4.35	9.75	1.90
1.00	1.36	4.00	2.18	7.00	4.35	10.00	1.90
1.25	1.36	4.25	3.26	7.25	3.26	10.25	1.09
1.50	1.36	4.50	3.26	7.50	3.26	10.50	1.09
1.75	1.36	4.75	4.35	7.75	3.26	10.75	1.09
2.00	1.36	5.00	4.35	8.00	3.26	11.00	1.09
2.25	1.63	5.25	6.53	8.25	1.90	11.25	1.09
2.50	1.63	5.50	6.53	8.50	1.90	11.50	1.09
2.75	1.63	5.75	26.11	8.75	1.90	11.75	1.09
3.00	1.63	6.00	71.81	9.00	1.90	12.00	1.09

CALIB NASHYD (0356)
 ID= 1 DT=15.0 min
 Area (ha)=1015.22
 Ia (mm)= 5.00
 U. H. Tp(hrs)= 1.37
 Curve Number (CN)= 46.0
 # of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 12.651
 PEAK FLOW (cms)= 2.534 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 6.976
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.128

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0352)
 ID= 1 DT=15.0 min
 Area (ha)= 381.43
 Ia (mm)= 5.00
 U. H. Tp(hrs)= 0.82
 Curve Number (CN)= 54.0
 # of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 7.980
 PEAK FLOW (cms)= 1.840 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 9.056
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.166

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0350)
 ID= 1 DT=15.0 min
 Area (ha)= 366.84
 Ia (mm)= 5.00
 U. H. Tp(hrs)= 1.07
 Curve Number (CN)= 48.0
 # of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 5.831
 PEAK FLOW (cms)= 1.170 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 7.450
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.137

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0348)
 ID= 1 DT=15.0 min
 Area (ha)= 590.18 Curve Number (CN)= 48.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 1.91

Unit Hyd Qpeak (cms)= 5.267
 PEAK FLOW (cms)= 1.224 (i)
 TIME TO PEAK (hrs)= 9.250
 RUNOFF VOLUME (mm)= 7.490
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.138

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6350)
 IN= 2--> OUT= 1
 Routing time step (min)' = 15.00

DATA FOR SECTION (3501.0) ----->

Distance	Elevation	Manning	
0.00	287.00	0.0500	
23.66	283.72	0.0500	
70.98	280.94	0.0500	
112.38	280.23	0.0500	
171.52	276.80	0.0500	
260.24	274.46	0.0500	
266.16	274.26	0.0500	
272.07	274.12	0.0500	
276.49	274.12	0.0500 / 0.0300	Main Channel
276.99	273.82	0.0300	Main Channel
277.99	273.82	0.0300	Main Channel
279.49	273.82	0.0300	Main Channel
280.49	274.27	0.0300 / 0.0500	Main Channel
283.90	274.27	0.0500	
289.81	274.57	0.0500	
325.30	275.75	0.0500	
396.28	278.98	0.0500	
449.51	280.97	0.0500	
496.83	283.90	0.0500	
585.54	287.92	0.0500	

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.30	274.12	.273E+04	0.9	0.99	49.81
0.98	274.80	.583E+05	20.8	1.05	46.71
1.66	275.48	.203E+06	95.2	1.38	35.53
2.33	276.15	.438E+06	256.0	1.73	28.50
3.01	276.83	.755E+06	524.8	2.05	23.97
3.69	277.51	.114E+07	935.8	2.43	20.26
4.37	278.19	.157E+07	1472.0	2.76	17.82
5.05	278.87	.206E+07	2140.7	3.06	16.06
5.72	279.54	.261E+07	2938.6	3.33	14.79
6.40	280.22	.321E+07	3887.6	3.57	13.77
7.08	280.90	.390E+07	4795.4	3.63	13.56
7.76	281.58	.468E+07	6196.2	3.91	12.58
8.43	282.25	.550E+07	7803.9	4.19	11.74
9.11	282.93	.637E+07	9601.9	4.45	11.05
9.79	283.61	.728E+07	11594.8	4.70	10.46
10.47	284.29	.823E+07	13845.3	4.96	9.91
11.15	284.97	.922E+07	16293.4	5.21	9.43
11.82	285.64	.103E+08	18952.0	5.45	9.02
12.50	286.32	.113E+08	21824.3	5.69	8.65

hydrograph -----> <-- pi pe / channel -->
 AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0348) 590.18 1.22 9.25 7.49 0.31 0.99
 OUTFLOW: ID= 1 (6350) 590.18 1.20 10.25 7.49 0.31 0.99

ADD HYD (8220)
 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)

ID1= 1 (0350): 366.84 1.170 7.75 7.45
 + ID2= 2 (6350): 590.18 1.197 10.25 7.49

 ID = 3 (8220): 957.02 2.179 8.75 7.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (3461)
 ID= 1 DT=15.0 min
 Area (ha)= 33.62
 Total Imp(%)= 61.00 Di r. Conn. (%)= 38.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 20.51 13.11
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 473.43 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 71.81 53.28
 over (min) 15.00 30.00
 Storage Coeff. (min)= 9.13 (ii) 22.89 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.04

PEAK FLOW (cms)= 2.21 0.92 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.25 2.791 (iii)
 RUNOFF VOLUME (mm)= 52.40 21.82 33.44
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.40 0.61

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5346)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.0090	0.6158
0.2450	0.2524	2.6260	0.6610
0.6230	0.3644	3.2370	0.7019
0.9920	0.4690	3.6370	0.7119
1.2510	0.5428	0.0000	0.0000

INFLOW : ID= 2 (3461) 33.620 2.791 6.00 33.44
 OUTFLOW: ID= 1 (5346) 33.620 0.994 6.50 33.43

PEAK FLOW REDUCTION [Qout/Qin] (%) = 35.62
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 0.4723

CALIB NASHYD (3462)
 ID= 1 DT=15.0 min
 Area (ha)= 317.31 Curve Number (CN)= 71.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 5.625
 PEAK FLOW (cms)= 2.390 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 15.763
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.290

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9346)
 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)

ID1= 1 (3462):	317.31	2.390	7.25	15.76
+ ID2= 2 (5346):	33.62	0.994	6.50	33.43

ID = 3 (9346):	350.93	3.244	6.75	17.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8222)				
1 + 2 = 3				
	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8220):	957.02	2.179	8.75	7.47
+ ID2= 2 (9346):	350.93	3.244	6.75	17.46

ID = 3 (8222):	1307.95	4.977	7.25	10.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6352)
IN= 2--> OUT= 1

Routing time step (min)' = 15.00

DATA FOR SECTION (3521.0) ----->

Distance	Elevation	Manning		
0.00	257.95	0.0500		
7.83	257.34	0.0500		
15.66	256.19	0.0500		
21.53	254.01	0.0500		
41.11	244.06	0.0500		
76.35	241.38	0.0500		
111.58	239.74	0.0500		
113.54	239.61	0.0500		
113.75	239.41	0.0500 / 0.0300	Main Channel	
113.85	238.81	0.0300	Main Channel	
115.50	238.81	0.0300	Main Channel	
116.15	238.81	0.0300	Main Channel	
116.25	239.41	0.0300	Main Channel	
117.46	239.52	0.0500		
119.41	239.72	0.0500		
121.37	240.04	0.0500		
131.16	241.84	0.0500		
156.61	247.03	0.0500		
176.19	251.46	0.0500		
193.80	258.79	0.0500		

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.60	239.41	.224E+04	2.2	1.52	17.12
1.58	240.39	.216E+05	22.3	1.61	16.16
2.55	241.36	.804E+05	101.0	1.96	13.27
3.53	242.34	.173E+06	275.6	2.48	10.46
4.50	243.31	.293E+06	553.9	2.95	8.80
5.48	244.29	.438E+06	961.8	3.42	7.60
6.45	245.26	.599E+06	1534.4	3.99	6.51
7.43	246.24	.770E+06	2228.4	4.51	5.76
8.41	247.22	.951E+06	3043.6	4.98	5.21
9.38	248.19	.114E+07	3982.1	5.43	4.78
10.36	249.17	.134E+07	5040.1	5.85	4.44
11.33	250.14	.155E+07	6218.5	6.24	4.16
12.31	251.12	.177E+07	7518.6	6.61	3.93
13.29	252.10	.200E+07	8969.0	6.98	3.72
14.26	253.07	.224E+07	10554.1	7.35	3.53
15.24	254.05	.248E+07	12257.1	7.71	3.37
16.21	255.02	.272E+07	14045.1	8.03	3.23
17.19	256.00	.298E+07	15954.8	8.34	3.11
18.16	256.97	.325E+07	17780.4	8.53	3.04

<---- hydrograph ---->				<- pi pe / channel ->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8222)	1307.95	4.98	7.25	10.15	0.74
OUTFLOW: ID= 1 (6352)	1307.95	4.93	7.75	10.15	0.73

ADD HYD (8224)				
1 + 2 = 3				
	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)

ID1= 1 (0352):	381.43	1.840	7.00	9.06
+ ID2= 2 (6352):	1307.95	4.929	7.75	10.15

ID = 3 (8224):	1689.38	6.703	7.50	9.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6354)
IN= 2--> OUT= 1

Routing time step (min)' = 15.00

DATA FOR SECTION (3541.0) ----->

Distance	Elevation	Manning		
0.00	253.92	0.0500		
7.95	251.83	0.0500		
15.89	249.97	0.0500		
19.87	249.05	0.0500		
47.68	242.00	0.0500		
67.55	237.10	0.0500		
83.44	231.11	0.0500		
85.43	230.75	0.0500		
85.66	230.61	0.0500 / 0.0300	Main Channel	
85.76	230.01	0.0300	Main Channel	
87.41	230.01	0.0300	Main Channel	
88.06	230.01	0.0300	Main Channel	
88.16	230.61	0.0300 / 0.0500	Main Channel	
89.40	230.72	0.0500		
91.39	230.88	0.0500		
93.37	231.03	0.0500		
133.11	233.96	0.0500		
150.99	235.39	0.0500		
172.84	239.03	0.0500		
196.68	250.00	0.0500		

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.60	230.61	.451E+04	1.5	1.06	49.40
1.62	231.63	.394E+05	14.6	1.16	45.13
2.64	232.65	.128E+06	58.8	1.44	36.30
3.66	233.67	.269E+06	149.7	1.74	30.00
4.68	234.69	.463E+06	301.5	2.04	25.58
5.70	235.71	.705E+06	530.5	2.36	22.14
6.72	236.73	.980E+06	848.4	2.71	19.25
7.74	237.75	.128E+07	1237.9	3.02	17.29
8.76	238.77	.162E+07	1706.7	3.30	15.83
9.78	239.79	.199E+07	2291.9	3.61	14.45
10.81	240.82	.237E+07	2973.7	3.92	13.30
11.83	241.84	.278E+07	3742.2	4.21	12.38
12.85	242.86	.321E+07	4599.1	4.49	11.62
13.87	243.88	.365E+07	5544.6	4.75	10.98
14.89	244.90	.412E+07	6579.7	5.00	10.44
15.91	245.92	.461E+07	7705.9	5.23	9.97
16.93	246.94	.512E+07	8924.6	5.46	9.55
17.95	247.96	.564E+07	10237.5	5.68	9.19
18.97	248.98	.619E+07	11646.2	5.89	8.86

<---- hydrograph ---->				<- pi pe / channel ->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8224)	1689.38	6.70	7.50	9.90	1.01
OUTFLOW: ID= 1 (6354)	1689.38	6.15	8.75	9.90	0.96

CALIB
STANDHYD (3541)
ID= 1 DT=15.0 min

Area (ha)= 8.01
Total Imp(%)= 75.00
Di r. Conn.(%)= 70.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)	6.01		2.00
Dep. Storage (mm)	2.00		5.00
Average Slope (%)	0.50		0.50
Length (m)	231.08		40.00
Mannings n	0.013		0.250
Max. Eff. Inten. (mm/hr)	71.81		4.66
Storage over (min)	15.00		45.00
Storage Coeff. (min)	5.94	(ii)	42.42 (ii)
Unit Hyd. Tpeak (min)	15.00		45.00
Unit Hyd. peak (cms)	0.10		0.03

TOTALS

PEAK FLOW (cms)= 1.06 0.02 1.066 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 52.40 5.78 38.41
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.11 0.71

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 35.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5354)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.5360	0.2546
0.0270	0.1427	0.6730	0.2758
0.1550	0.1714	0.7830	0.2966
0.2890	0.1975	1.1830	0.3066
0.4010	0.2251	0.0000	0.0000

AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 INFLOW : ID= 2 (3541) 8.010 1.066 6.00 38.41
 OUTFLOW: ID= 1 (5354) 8.010 0.230 6.25 38.25

PEAK FLOW REDUCTION [Qout/Qin] (%) = 21.54
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha.m.) = 0.1863

CALIB NASHYD (3542)
 ID= 1 DT=15.0 min

Area (ha) = 254.67 Curve Number (CN) = 36.7
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp (hrs) = 1.37

Unit Hyd Qpeak (cms) = 3.174

PEAK FLOW (cms) = 0.451 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 4.974
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.091

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9354)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (3542): 254.67	0.451	8.00	4.97
+ ID2= 2 (5354): 8.01	0.230	6.25	38.25
ID = 3 (9354): 262.68	0.563	6.75	5.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8226)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6354): 1689.38	6.147	8.75	9.90
+ ID2= 2 (9354): 262.68	0.563	6.75	5.99
ID = 3 (8226): 1952.06	6.641	8.50	9.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (3402)
 ID= 1 DT=15.0 min

Area (ha) = 138.83 Total Imp(%) = 46.00
 Di r. Conn. (%) = 28.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 63.86 74.97
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 962.05 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 71.81 11.66
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 13.97 (ii) 39.24 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.07 0.03

PEAK FLOW (cms) = 5.82 1.32 *TOTALS*
 TIME TO PEAK (hrs) = 6.00 6.50 6.344 (iii)
 RUNOFF VOLUME (mm) = 52.40 9.82 21.74
 TOTAL RAINFALL (mm) = 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.18 0.40

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 47.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3401)
 ID= 1 DT=15.0 min

Area (ha) = 146.96
 Total Imp(%) = 58.00 Di r. Conn. (%) = 35.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 85.24 61.72
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 989.81 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 71.81 15.65
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 14.21 (ii) 36.67 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.07 0.03

PEAK FLOW (cms) = 7.64 1.51 *TOTALS*
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 52.40 11.26 25.66
 TOTAL RAINFALL (mm) = 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.21 0.47

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 47.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5340)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	11.4780	4.3560
0.3860	2.0347	14.9940	4.9991
3.2310	2.5818	18.1680	5.6243
5.1770	3.1417	18.5680	5.6343
7.7590	3.6318	0.0000	0.0000

AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 INFLOW : ID= 2 (3401) 146.960 8.256 6.00 25.66
 OUTFLOW: ID= 1 (5340) 146.960 1.670 7.00 25.65

PEAK FLOW REDUCTION [Qout/Qin] (%) = 20.22
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha.m.) = 2.2884

ADD HYD	(8232)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)
ID= 1	(3402):	138.83	6.344	6.00	21.74
+ ID= 2	(5340):	146.96	1.670	7.00	25.65

ID = 3	(8232):	285.79	6.561	6.00	23.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	(0410)	Area	(ha)	Curve Number	(CN)= 48.0
NASHYD	(0410)	Ia	(mm)= 5.00	# of Linear Res.	(N)= 1.50
ID= 1	DT=15.0 min	U. H.	Tp(hrs)= 1.46		

Unit Hyd Opeak (cms)= 6.698

PEAK FLOW (cms)= 1.460 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 7.475
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.137

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	(0408)	Area	(ha)	Curve Number	(CN)= 58.0
NASHYD	(0408)	Ia	(mm)= 5.00	# of Linear Res.	(N)= 1.50
ID= 1	DT=15.0 min	U. H.	Tp(hrs)= 0.64		

Unit Hyd Opeak (cms)= 6.198

PEAK FLOW (cms)= 1.542 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 10.251
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.188

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	(0376)	Area	(ha)	Curve Number	(CN)= 74.0
NASHYD	(0376)	Ia	(mm)= 5.00	# of Linear Res.	(N)= 1.50
ID= 1	DT=15.0 min	U. H.	Tp(hrs)= 1.07		

Unit Hyd Opeak (cms)= 7.380

PEAK FLOW (cms)= 3.568 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 17.440
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.321

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	(0374)	Area	(ha)	Curve Number	(CN)= 61.0
NASHYD	(0374)	Ia	(mm)= 5.00	# of Linear Res.	(N)= 1.50
ID= 1	DT=15.0 min	U. H.	Tp(hrs)= 1.51		

Unit Hyd Opeak (cms)= 6.158

PEAK FLOW (cms)= 2.092 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 11.459
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.211

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	(0372)	Area	(ha)	Curve Number	(CN)= 37.0
NASHYD	(0372)	Ia	(mm)= 5.00	# of Linear Res.	(N)= 1.50
ID= 1	DT=15.0 min	U. H.	Tp(hrs)= 0.96		

Unit Hyd Opeak (cms)= 1.954

PEAK FLOW (cms)= 0.255 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 5.010
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.092

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	(0370)	Area	(ha)	Curve Number	(CN)= 63.0
NASHYD	(0370)	Ia	(mm)= 5.00	# of Linear Res.	(N)= 1.50
ID= 1	DT=15.0 min	U. H.	Tp(hrs)= 0.67		

Unit Hyd Opeak (cms)= 4.860

PEAK FLOW (cms)= 1.448 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 12.065
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.222

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	(0366)	Area	(ha)	Curve Number	(CN)= 62.0
NASHYD	(0366)	Ia	(mm)= 5.00	# of Linear Res.	(N)= 1.50
ID= 1	DT=15.0 min	U. H.	Tp(hrs)= 1.06		

Unit Hyd Opeak (cms)= 7.451

PEAK FLOW (cms)= 2.387 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 11.788
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.217

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	(3641)	Area	(ha)	Curve Number	(CN)= 74.0
STANDHYD	(3641)	Total Imp(%)	= 45.00	Dir. Conn. (%)	= 16.00
ID= 1	DT=15.0 min				

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	3.53	4.32
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	228.76	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 71.81
 over (min)= 15.00
 Storage Coeff. (min)= 5.90 (ii)
 Unit Hyd. Tpeak (min)= 15.00
 Unit Hyd. peak (cms)= 0.10 0.03

PEAK FLOW (cms)= 0.24 0.10
 TIME TO PEAK (hrs)= 6.00 6.50
 RUNOFF VOLUME (mm)= 52.40 9.59
 TOTAL RAINFALL (mm)= 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.18

TOTALS
 0.277 (iii)
 6.00
 16.44
 54.40
 0.30

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 42.9 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR	(5364)
ID= 2	OUT= 1

DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	0.5250	0.2044
	0.0270	0.0959	0.6590	0.2337
	0.1520	0.1110	0.7670	0.2624
	0.2830	0.1432	1.1670	0.2724
	0.3930	0.1710	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (3641)	7.850	0.277	6.00	16.44
OUTFLOW: ID= 1 (5364)	7.850	0.024	8.50	16.33

PEAK FLOW REDUCTION [Qout/Qin](%)= 8.70
 TIME SHIFT OF PEAK FLOW (min)=150.00
 MAXIMUM STORAGE USED (ha.m.)= 0.0857

CALIB STANDHYD (3642) ID= 1 DT=15.0 min	Area (ha)= 147.42	Total Imp(%)= 21.00	Dir. Conn.(%)= 7.00
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	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	30.96	116.46
Dep. Storage	2.00	5.00
Average Slope	0.50	5.00
Length	991.36	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	71.81	7.70
over (min)	15.00	45.00
Storage Coeff. (min)	14.22 (ii)	44.05 (ii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.07	0.03

		TOTALS	(iii)
PEAK FLOW (cms)	1.53	1.27	2.031
TIME TO PEAK (hrs)	6.00	6.50	6.00
RUNOFF VOLUME (mm)	52.40	7.46	10.60
TOTAL RAINFALL (mm)	54.40	54.40	54.40
RUNOFF COEFFICIENT	0.96	0.14	0.19

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 42.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9364) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (3642):	147.42	2.031	6.00	10.60
+ ID2= 2 (5364):	7.85	0.024	8.50	16.33
ID = 3 (9364):	155.27	2.040	6.00	10.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8302) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0366):	462.62	2.387	7.50	11.79
+ ID2= 2 (9364):	155.27	2.040	6.00	10.89
ID = 3 (8302):	617.89	3.765	6.50	11.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0358) ID= 1 DT=15.0 min	Area (ha)= 429.87	Curve Number (CN)= 35.0
	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50
	U.H. Tp(hrs)= 1.03	

Unit Hyd Opeak (cms)= 7.091

PEAK FLOW (cms)	= 0.870 (i)
TIME TO PEAK (hrs)	= 7.500
RUNOFF VOLUME (mm)	= 4.638
TOTAL RAINFALL (mm)	= 54.400
RUNOFF COEFFICIENT	= 0.085

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0360) ID= 1 DT=15.0 min	Area (ha)= 138.37	Curve Number (CN)= 46.0
	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50
	U.H. Tp(hrs)= 0.60	

Unit Hyd Opeak (cms)= 3.957

PEAK FLOW (cms)	= 0.641 (i)
TIME TO PEAK (hrs)	= 6.750
RUNOFF VOLUME (mm)	= 6.867
TOTAL RAINFALL (mm)	= 54.400
RUNOFF COEFFICIENT	= 0.126

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8306) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0358):	429.87	0.870	7.50	4.64
+ ID2= 2 (0360):	138.37	0.641	6.75	6.87
ID = 3 (8306):	568.24	1.473	7.00	5.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6362) IN= 2---> OUT= 1	Routing time step (min)'= 15.00
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<----- DATA FOR SECTION (3621.0) ----->		
Distance	Elevation	Manning
0.00	261.46	0.0550
27.86	254.23	0.0550
51.07	251.96	0.0550
74.29	250.77	0.0550
97.50	249.91	0.0550
125.36	249.40	0.0550
150.93	247.40	0.0550
155.93	247.33	0.0550 / 0.0350
157.93	246.85	0.0350
159.18	246.65	0.0350
160.18	246.63	0.0350
160.93	246.85	0.0350
161.93	247.18	0.0350 / 0.0550
163.18	248.03	0.0550
168.18	248.58	0.0550
183.18	250.18	0.0550
201.97	252.59	0.0550
213.57	256.02	0.0550
225.18	260.31	0.0550
229.82	261.00	0.0550

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.55	247.18	.556E+04	1.9	1.03	49.95
1.28	247.91	.336E+05	16.7	1.53	33.61
2.00	248.63	.913E+05	53.6	1.81	28.40
2.73	249.36	.185E+06	122.3	2.05	25.18
3.46	250.09	.344E+06	224.6	2.02	25.52
4.19	250.82	.574E+06	417.9	2.25	22.91
4.91	251.54	.855E+06	699.1	2.53	20.38
5.64	252.27	.118E+07	1075.3	2.82	18.26
6.37	253.00	.153E+07	1559.9	3.15	16.38
7.10	253.73	.191E+07	2137.6	3.46	14.89
7.82	254.45	.231E+07	2815.2	3.77	13.67
8.55	255.18	.272E+07	3608.5	4.10	12.58

9.28	255.91	.315E+07	4488.2	4.41	11.69
10.01	256.64	.358E+07	5458.0	4.71	10.95
10.73	257.36	.403E+07	6512.4	4.99	10.32
11.46	258.09	.449E+07	7649.8	5.27	9.78
12.19	258.82	.496E+07	8869.5	5.53	9.32
12.92	259.55	.544E+07	10171.2	5.78	8.91
13.64	260.27	.593E+07	11554.8	6.02	8.55

<---- hydrograph ----> <- pi pe / channel ->

	AREA	OPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW: ID= 2 (8306)	568.24	1.47	7.00	5.18	0.44	1.03
OUTFLOW: ID= 1 (6362)	568.24	1.30	8.25	5.18	0.38	1.03

CALIB STANDHYD (3621) ID= 1 DT=15.0 min	Area (ha)= 11.87 Total Imp(%)= 45.00	Dir. Conn.(%)= 17.00
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	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	5.34	6.53
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	281.31	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	71.81	14.02
over (min)=	15.00	45.00
Storage Coeff. (min)=	6.68 (ii)	30.15 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.10	0.03
PEAK FLOW (cms)=	0.37	0.16
TIME TO PEAK (hrs)=	6.00	6.00 (iii)
RUNOFF VOLUME (mm)=	52.40	10.39
TOTAL RAINFALL (mm)=	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.19

TOTALS

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 45.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5362) IN= 2---> OUT= 1 DT= 15.0 min
--

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.7940	0.3091
0.0400	0.1451	0.9970	0.3534
0.2300	0.1678	1.1610	0.3968
0.4280	0.2166	1.5610	0.4068
0.5940	0.2586	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (3621)	11.870	0.438	6.00	17.53
OUTFLOW: ID= 1 (5362)	11.870	0.038	8.50	17.46

PEAK FLOW REDUCTION [Oout/Oi n](%)= 8.73
TIME SHIFT OF PEAK FLOW (min)=150.00
MAXIMUM STORAGE USED (ha.m.)= 0.1389

CALIB STANDHYD (3622) ID= 1 DT=15.0 min	Area (ha)= 106.91 Total Imp(%)= 24.00	Dir. Conn.(%)= 9.00
---	--	---------------------

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	25.66	81.25
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	844.24	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	71.81	8.81
over (min)=	15.00	45.00
Storage Coeff. (min)=	12.91 (ii)	41.18 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03
PEAK FLOW (cms)=	1.49	1.05
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	52.40	8.34
TOTAL RAINFALL (mm)=	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.15

TOTALS
1.900 (iii)
6.00
12.31
54.40
0.23

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 45.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9362) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (3622):	106.91	1.900	6.00	12.31
+ ID2= 2 (5362):	11.87	0.038	8.50	17.46
ID = 3 (9362):	118.78	1.915	6.00	12.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8304) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6362):	568.24	1.303	8.25	5.18
+ ID2= 2 (9362):	118.78	1.915	6.00	12.82
ID = 3 (8304):	687.02	2.105	6.50	6.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8300) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8302):	617.89	3.765	6.50	11.56
+ ID2= 2 (8304):	687.02	2.105	6.50	6.50
ID = 3 (8300):	1304.91	5.871	6.50	8.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6368) IN= 2---> OUT= 1	Routing time step (min)= 15.00
--------------------------------------	--------------------------------

<----- DATA FOR SECTION (3681.0) ----->

Distance	Elevation	Manning
0.00	230.00	0.0370
18.48	223.26	0.0370
36.96	223.05	0.0370
64.67	222.94	0.0370
110.87	222.86	0.0370
133.96	222.74	0.0370
147.82	222.65	0.0370
170.92	222.31	0.0370
174.79	222.26	0.0370 / 0.0300
174.89	221.86	0.0300
175.54	221.86	0.0300
176.19	221.86	0.0300
176.29	222.26	0.0300 / 0.0370
180.16	222.25	0.0370
184.78	222.28	0.0370
189.40	222.31	0.0370

332.60 222.37 0.0370
 450.00 230.00 0.0370

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.40	222.26	.771E+03	0.3	0.52	41.74
0.81	222.67	.784E+05	30.3	0.51	43.17
1.21	223.07	.210E+06	123.2	0.77	28.42
1.62	223.48	.384E+06	301.0	1.03	21.25
2.03	223.89	.563E+06	550.2	1.28	17.06
2.44	224.30	.747E+06	860.7	1.51	14.46
2.84	224.70	.934E+06	1228.1	1.73	12.68
3.25	225.11	.113E+07	1649.3	1.92	11.37
3.66	225.52	.132E+07	2122.3	2.11	10.37
4.07	225.93	.152E+07	2645.5	2.29	9.58
4.47	226.33	.172E+07	3217.8	2.45	8.92
4.88	226.74	.193E+07	3838.3	2.61	8.38
5.29	227.15	.214E+07	4506.3	2.76	7.92
5.70	227.56	.236E+07	5221.3	2.91	7.52
6.10	227.96	.258E+07	5983.0	3.05	7.17
6.51	228.37	.280E+07	6790.9	3.19	6.87
6.92	228.78	.303E+07	7645.0	3.32	6.60
7.33	229.19	.326E+07	8545.1	3.45	6.35
7.73	229.59	.349E+07	9491.0	3.57	6.13

<---- hydrograph ----> <- pipe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8300)	1304.91	5.87	6.50	8.90	0.48	0.52
OUTFLOW: ID= 1 (6368)	1304.91	4.88	7.75	8.90	0.46	0.52

CALIB STANDHYD (3681)
 ID= 1 DT=15.0 mi n

Area (ha)= 2.95
 Total Imp(%)= 45.00
 Dir. Conn.(%)= 15.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	1.33	1.62
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	140.24	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 71.81
 over (mi n)= 15.00

Storage Coeff. (mi n)= 4.40 (ii)
 Unit Hyd. Tpeak (mi n)= 15.00
 Unit Hyd. peak (cms)= 0.11

TOTALS
 PEAK FLOW (cms)= 0.09
 TIME TO PEAK (hrs)= 6.00
 RUNOFF VOLUME (mm)= 52.40
 TOTAL RAINFALL (mm)= 54.40
 RUNOFF COEFFICIENT = 0.96

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 40.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5368)
 IN= 2--> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.1970	0.0767
0.0100	0.0360	0.2470	0.0877
0.0570	0.0416	0.2880	0.0984
0.1060	0.0537	0.6880	0.1084
0.1470	0.0642	0.0000	0.0000

AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 INFLOW : ID= 2 (3681) 2.950 0.109 6.00 15.48
 OUTFLOW: ID= 1 (5368) 2.950 0.008 8.50 15.18

PEAK FLOW REDUCTION [Qout/Qin](%)= 7.76
 TIME SHIFT OF PEAK FLOW (mi n)=150.00
 MAXIMUM STORAGE USED (ha. m.)= 0.0306

CALIB NASHYD (3682)
 ID= 1 DT=15.0 mi n

Area (ha)= 156.53
 Ia (mm)= 5.00
 U.H. Tp(hrs)= 0.96

Curve Number (CN)= 45.9
 # of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 2.778

PEAK FLOW (cms)= 0.503 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 6.921
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.127

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9368)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (3682):	156.53	0.503	7.25	6.92
+ ID2= 2 (5368):	2.95	0.008	8.50	15.18
ID = 3 (9368):	159.48	0.511	7.25	7.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8298)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6368):	1304.91	4.883	7.75	8.90
+ ID2= 2 (9368):	159.48	0.511	7.25	7.07
ID = 3 (8298):	1464.39	5.390	7.75	8.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8296)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0370):	191.85	1.448	6.75	12.06
+ ID2= 2 (8298):	1464.39	5.390	7.75	8.70
ID = 3 (8296):	1656.24	6.710	7.50	9.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6372)
 IN= 2--> OUT= 1

Routing time step (mi n)= 15.00

<----- DATA FOR SECTION (3721.0) ----->

Distance	Elevation	Manning
0.00	225.00	0.0390
30.80	219.38	0.0390
61.61	219.30	0.0390
77.01	219.27	0.0390
469.76	219.14	0.0390
477.46	219.13	0.0390
485.16	219.10	0.0390
492.86	219.09	0.0390
495.56	219.09	0.0390 / 0.0310
495.66	218.51	0.0310
500.56	218.51	0.0310
505.46	218.51	0.0310
505.56	219.09	0.0310 / 0.0390
508.26	219.09	0.0390
515.96	219.10	0.0390
523.67	219.21	0.0390

562.17	219.32	0.0390
654.58	219.43	0.0390
731.59	219.46	0.0390
762.39	225.00	0.0390

TRAVEL TIME TABLE						
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)	
0.29	218.80	.431E+04	1.7	0.58	43.48	
0.58	219.09	.867E+04	5.1	0.89	28.32	
0.93	219.44	.200E+06	58.2	0.44	57.35	
1.28	219.79	.568E+06	278.6	0.74	33.99	
1.62	220.13	.939E+06	630.2	1.01	24.84	
1.97	220.48	.131E+07	1090.4	1.25	20.05	
2.32	220.83	.169E+07	1647.7	1.47	17.06	
2.67	221.18	.206E+07	2294.6	1.68	14.99	
3.01	221.52	.244E+07	3025.6	1.87	13.46	
3.36	221.87	.282E+07	3836.5	2.05	12.27	
3.71	222.22	.321E+07	4723.7	2.22	11.31	
4.06	222.57	.359E+07	5684.6	2.39	10.53	
4.40	222.91	.398E+07	6716.6	2.55	9.87	
4.75	223.26	.437E+07	7817.7	2.70	9.31	
5.10	223.61	.476E+07	8986.2	2.85	8.83	
5.45	223.96	.515E+07	10220.4	2.99	8.40	
5.79	224.30	.555E+07	11519.1	3.13	8.03	
6.14	224.65	.594E+07	12880.8	3.27	7.69	
6.49	225.00	.634E+07	14304.5	3.40	7.39	

hydrograph							pi pe / channel -->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)		
INFLOW: ID= 2 (8296)	1656.24	6.71	7.50	9.09	0.59	0.86		
OUTFLOW: ID= 1 (6372)	1656.24	6.48	8.25	9.09	0.59	0.87		

ADD HYD (8294)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (0372):	110.42	0.255	7.50	5.01	
+ ID2= 2 (6372):	1656.24	6.484	8.25	9.09	
ID = 3 (8294):	1766.66	6.731	8.00	8.83	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8292)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (0374):	545.70	2.092	8.25	11.46	
+ ID2= 2 (8294):	1766.66	6.731	8.00	8.83	
ID = 3 (8292):	2312.36	8.818	8.25	9.45	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5505)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	65.1290	345.3754
	25.4850	24.6697	84.9510	456.3890
	31.1490	98.6787	*****	838.7689
	39.6440	*****	*****	838.7789
	48.1390	*****	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (8292)	2312.360	8.818	8.25	9.45
OUTFLOW: ID= 1 (5505)	2312.360	6.272	10.75	9.45

PEAK FLOW REDUCTION [Qout/Qin] (%) = 71.13
 TIME SHIF OF PEAK FLOW (min) = 150.00
 MAXIMUM STORAGE USED (ha. m.) = 6.0742

ADD HYD (8272)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0376):	463.85	3.568	7.50	17.44
+ ID2= 2 (5505):	2312.36	6.272	10.75	9.45
ID = 3 (8272):	2776.21	8.743	10.00	10.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0396)			
ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
	305.21	69.0	1.50
	Ia (mm) = 5.00		
	U. H. Tp(hrs) = 1.08		

Unit Hyd Opeak (cms) = 4.811
 PEAK FLOW (cms) = 1.961 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 14.789
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.272

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0394)			
ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
	325.45	53.0	1.50
	Ia (mm) = 5.00		
	U. H. Tp(hrs) = 0.92		

Unit Hyd Opeak (cms) = 6.013
 PEAK FLOW (cms) = 1.379 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 8.784
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.161

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0390)			
ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
	420.00	55.0	1.50
	Ia (mm) = 5.00		
	U. H. Tp(hrs) = 1.07		

Unit Hyd Opeak (cms) = 6.683
 PEAK FLOW (cms) = 1.700 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 9.400
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.173

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0388)			
ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
	220.77	58.0	1.50
	Ia (mm) = 5.00		
	U. H. Tp(hrs) = 0.99		

Unit Hyd Opeak (cms) = 3.819
 PEAK FLOW (cms) = 1.051 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 10.350
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.190

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0386)			
ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
	241.27	61.0	1.50
	Ia (mm) = 5.00		

U. H. Tp(hrs)= 0.90

Unit Hyd Opeak (cms)= 4.562

PEAK FLOW (cms)= 1.360 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 11.386
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.209

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8286)
1 + 2 = 3

AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
ID1= 1 (0386): 241.27 1.360 7.25 11.39
+ ID2= 2 (0388): 220.77 1.051 7.50 10.35
ID = 3 (8286): 462.04 2.411 7.25 10.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0384)
ID= 1 DT=15.0 min

Area (ha)= 199.07 Curve Number (CN)= 44.0
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 0.96

Unit Hyd Opeak (cms)= 3.537

PEAK FLOW (cms)= 0.598 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 6.477
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.119

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0380)
ID= 1 DT=15.0 min

Area (ha)= 182.01 Curve Number (CN)= 40.0
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 0.55

Unit Hyd Opeak (cms)= 5.609

PEAK FLOW (cms)= 0.715 (i)
TIME TO PEAK (hrs)= 6.500
RUNOFF VOLUME (mm)= 5.531
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.102

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0382)
ID= 1 DT=15.0 min

Area (ha)= 216.59 Curve Number (CN)= 53.0
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 0.64

Unit Hyd Opeak (cms)= 5.733

PEAK FLOW (cms)= 1.207 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 8.712
TOTAL RAINFALL (mm)= 54.400
RUNOFF COEFFICIENT = 0.160

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8290)
1 + 2 = 3

AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
ID1= 1 (0380): 182.01 0.715 6.50 5.53
+ ID2= 2 (0382): 216.59 1.207 6.75 8.71

ID = 3 (8290): 398.60 1.920 6.75 7.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6384)
IN= 2 -> OUT= 1

Routing time step (min)= 15.00

DATA FOR SECTION (3841.0)

Table with columns: Distance, Elevation, Manning, and Channel Name. Rows list various distance and elevation points with corresponding Manning coefficients and channel names like 'Main Channel'.

TRAVEL TIME TABLE

Table with columns: DEPTH (m), ELEV (m), VOLUME (cu. m.), FLOW RATE (cms), VELOCITY (m/s), TRAV. TIME (min). Rows show data for depths from 0.34 to 13.94 meters.

hydrograph table with columns: AREA (ha), OPEAK (cms), TPEAK (hrs), R. V. (mm), MAX DEPTH (m), MAX VEL (m/s). Rows for INFLOW and OUTFLOW at ID=2 and ID=1.

ADD HYD (8288)
1 + 2 = 3

AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
ID1= 1 (0384): 199.07 0.598 7.25 6.48
+ ID2= 2 (6384): 398.60 1.505 8.00 7.26
ID = 3 (8288): 597.67 2.089 8.00 7.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8284)
1 + 2 = 3

AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
ID1= 1 (8286): 462.04 2.411 7.25 10.89
+ ID2= 2 (8288): 597.67 2.089 8.00 7.00

=====
 ID = 3 (8284): 1059.71 4.450 7.75 8.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8280)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0390): 420.00 1.700 7.50 9.40
 + ID2= 2 (8284): 1059.71 4.450 7.75 8.70
 =====
 ID = 3 (8280): 1479.71 6.150 7.75 8.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB NASHYD (0392)
 ID= 1 DT=15.0 min
 Area (ha)= 167.22 Curve Number (CN)= 62.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.74

Unit Hyd Qpeak (cms)= 3.837

PEAK FLOW (cms)= 1.133 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 11.712
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.215

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB NASHYD (0378)
 ID= 1 DT=15.0 min
 Area (ha)= 606.72 Curve Number (CN)= 55.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 1.18

Unit Hyd Qpeak (cms)= 8.771

PEAK FLOW (cms)= 2.295 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 9.412
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.173

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8282)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0378): 606.72 2.295 8.00 9.41
 + ID2= 2 (0392): 167.22 1.133 7.00 11.71
 =====
 ID = 3 (8282): 773.94 3.350 7.50 9.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8278)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8280): 1479.71 6.150 7.75 8.90
 + ID2= 2 (8282): 773.94 3.350 7.50 9.91
 =====
 ID = 3 (8278): 2253.65 9.491 7.50 9.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6394)
 IN= 2--> OUT= 1
 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3941.0) ----->

Distance	Elevation	Manning
0.00	283.00	0.0380
13.48	282.87	0.0380
53.92	280.08	0.0380
74.13	276.62	0.0380
97.72	265.45	0.0380
114.57	256.93	0.0380
131.42	253.04	0.0380
134.79	252.58	0.0380
138.53	251.74	0.0380 / 0.0300
139.03	251.20	0.0300
141.53	251.20	0.0300
144.03	251.20	0.0300
144.53	251.74	0.0300 / 0.0380
148.27	252.69	0.0380
151.64	252.97	0.0380
185.34	255.08	0.0380
219.03	257.54	0.0380
262.84	259.43	0.0380
310.02	262.80	0.0380
333.60	283.00	0.0380

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.54	251.74	827E+04	4.5	1.51	30.71
2.19	253.39	758E+05	73.2	2.69	17.27
3.83	255.03	282E+06	331.8	3.27	14.17
5.48	256.68	633E+06	917.3	4.03	11.51
7.12	258.32	112E+07	1889.5	4.69	9.90
8.77	259.97	179E+07	3410.2	5.31	8.73
10.41	261.61	259E+07	5627.6	6.05	7.66
12.06	263.26	351E+07	8605.1	6.83	6.79
13.70	264.90	447E+07	12544.9	7.81	5.94
15.35	266.55	546E+07	17101.4	8.71	5.32
16.99	268.19	648E+07	22251.7	9.56	4.85
18.64	269.84	752E+07	27979.0	10.36	4.48
20.28	271.48	858E+07	34269.0	11.11	4.17
21.93	273.13	967E+07	41110.8	11.83	3.92
23.57	274.77	108E+08	48495.9	12.51	3.71
25.22	276.42	119E+08	56418.1	13.17	3.52
26.86	278.06	131E+08	64410.6	13.69	3.39
28.51	279.71	143E+08	73002.1	14.18	3.27
30.15	281.35	156E+08	81379.3	14.50	3.20

<---- hydrograph ---->						
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8278)	2253.65	9.49	7.50	9.24	0.66	1.56
OUTFLOW: ID= 1 (6394)	2253.65	9.22	8.25	9.24	0.65	1.56

 ADD HYD (8276)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0394): 325.45 1.379 7.25 8.78
 + ID2= 2 (6394): 2253.65 9.215 8.25 9.24
 =====
 ID = 3 (8276): 2579.10 10.506 8.00 9.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6396)
 IN= 2--> OUT= 1
 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3961.0) ----->			
Distance	Elevation	Manning	
0.00	263.00	0.0410	
11.75	257.14	0.0410	
23.50	253.97	0.0410	
41.13	247.83	0.0410	
76.38	232.09	0.0410	
135.13	229.07	0.0410	
149.82	228.97	0.0410	
152.75	228.96	0.0410 / 0.0300	Main Channel
154.19	228.73	0.0300	Main Channel
154.69	228.20	0.0300	Main Channel
155.69	228.20	0.0300	Main Channel
156.69	228.20	0.0300	Main Channel

157.19	228.73	0.0300	Main Channel
158.63	228.95	0.0300 / 0.0410	Main Channel
161.57	228.96	0.0410	
164.51	229.71	0.0410	
196.82	241.70	0.0410	
223.26	249.21	0.0410	
246.76	255.13	0.0410	
290.82	263.51	0.0410	

TRAVEL TIME TABLE						
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)	
0.75	228.95	.780E+04	2.2	0.98	58.06	
2.54	230.74	.277E+06	145.1	1.78	31.76	
4.33	232.53	.786E+06	609.1	2.63	21.50	
6.13	234.33	.140E+07	1481.5	3.59	15.77	
7.92	236.12	.207E+07	2677.3	4.39	12.90	
9.71	237.91	.280E+07	4184.8	5.09	11.14	
11.50	239.70	.357E+07	6001.4	5.71	9.93	
13.29	241.49	.441E+07	8129.1	6.27	9.03	
15.09	243.29	.530E+07	10549.4	6.77	8.37	
16.88	245.08	.625E+07	13300.6	7.24	7.83	
18.67	246.87	.726E+07	16396.2	7.68	7.38	
20.46	248.66	.834E+07	19809.8	8.07	7.02	
22.26	250.46	.949E+07	23532.5	8.43	6.72	
24.05	252.25	.107E+08	27641.2	8.77	6.46	
25.84	254.04	.120E+08	32155.2	9.10	6.22	
27.63	255.83	.134E+08	36901.0	9.37	6.05	
29.42	257.62	.149E+08	42147.7	9.64	5.88	
31.22	259.42	.164E+08	48149.1	9.97	5.68	
33.01	261.21	.181E+08	54644.8	10.29	5.51	

<---- hydrograph ---->						
		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)
INFLOW :	ID= 2 (8276)	2579.10	10.51	8.00	9.19	0.85
OUTFLOW:	ID= 1 (6396)	2579.10	9.52	9.25	9.19	0.84

ADD HYD (8274)					
1 + 2 = 3		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0396):		305.21	1.961	7.50	14.79
+ ID2= 2 (6396):		2579.10	9.515	9.25	9.19
ID = 3 (8274):		2884.31	11.197	9.00	9.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8270)					
1 + 2 = 3		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8272):		2776.21	8.743	10.00	10.79
+ ID2= 2 (8274):		2884.31	11.197	9.00	9.78
ID = 3 (8270):		5660.52	19.715	9.25	10.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5506)				
IN= 2----> OUT= 1				
DT= 15.0 mi n				
OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)	
0.0000	0.0000	60.8810	135.6832	
31.1490	24.6697	96.2770	900.4431	
36.8120	37.0045	96.6770	900.4531	
45.3070	86.3439	0.0000	0.0000	

INFLOW :	ID= 2 (8270)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)
OUTFLOW:	ID= 1 (5506)	5660.520	19.715	9.25
		5660.520	16.265	11.75

PEAK FLOW REDUCTION [Qout/Qin] (%) = 82.50
 TIME SHIFT OF PEAK FLOW (mi n) = 150.00
 MAXIMUM STORAGE USED (ha. m.) = 12.8857

CALIB NASHYD (0406)			
ID= 1 DT=15.0 mi n	Area (ha)	Curve Number (CN)	# of Li near Res. (N)
	142.65	66.0	1.50
	Ia (mm) = 5.00		
	U. H. Tp(hrs) = 0.59		

Uni t Hyd Qpeak (cms) = 4.135

PEAK FLOW (cms) = 1.319 (i)
TIME TO PEAK (hrs) = 6.750
RUNOFF VOLUME (mm) = 13.237
TOTAL RAINFALL (mm) = 54.400
RUNOFF COEFFICIENT = 0.243

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0404)			
ID= 1 DT=15.0 mi n	Area (ha)	Curve Number (CN)	# of Li near Res. (N)
	246.46	47.0	1.50
	Ia (mm) = 5.00		
	U. H. Tp(hrs) = 0.98		

Uni t Hyd Qpeak (cms) = 4.280

PEAK FLOW (cms) = 0.810 (i)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 7.191
TOTAL RAINFALL (mm) = 54.400
RUNOFF COEFFICIENT = 0.132

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0402)			
ID= 1 DT=15.0 mi n	Area (ha)	Curve Number (CN)	# of Li near Res. (N)
	244.00	61.0	1.50
	Ia (mm) = 5.00		
	U. H. Tp(hrs) = 1.07		

Uni t Hyd Qpeak (cms) = 3.879

PEAK FLOW (cms) = 1.206 (i)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 11.417
TOTAL RAINFALL (mm) = 54.400
RUNOFF COEFFICIENT = 0.210

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0400)			
ID= 1 DT=15.0 mi n	Area (ha)	Curve Number (CN)	# of Li near Res. (N)
	93.97	52.0	1.50
	Ia (mm) = 5.00		
	U. H. Tp(hrs) = 0.44		

Uni t Hyd Qpeak (cms) = 3.630

PEAK FLOW (cms) = 0.675 (i)
TIME TO PEAK (hrs) = 6.500
RUNOFF VOLUME (mm) = 8.305
TOTAL RAINFALL (mm) = 54.400
RUNOFF COEFFICIENT = 0.153

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0398)			
ID= 1 DT=15.0 mi n	Area (ha)	Curve Number (CN)	# of Li near Res. (N)
	328.19	55.0	1.50
	Ia (mm) = 5.00		
	U. H. Tp(hrs) = 0.83		

Uni t Hyd Qpeak (cms) = 6.759

PEAK FLOW (cms) = 1.617 (i)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 9.360
TOTAL RAINFALL (mm) = 54.400
RUNOFF COEFFICIENT = 0.172

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0398):	328.19	1.617	7.00	9.36
+ ID2= 2 (0400):	93.97	0.675	6.50	8.30
ID = 3 (8268):	422.16	2.224	6.75	9.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6402)	Routing time step (min) = 15.00
IN= 2---> OUT= 1	

Distance	Elevation	Manning	
0.00	238.50	0.0360	
11.50	238.00	0.0360	
23.00	237.93	0.0360	
34.49	236.39	0.0360	
63.24	233.98	0.0360	
97.73	228.15	0.0360	
123.60	227.08	0.0360	
126.48	226.61	0.0360	
127.60	226.47	0.0360 / 0.0330	Main Channel
127.85	225.25	0.0330	Main Channel
129.35	225.25	0.0330	Main Channel
130.85	225.25	0.0330	Main Channel
131.10	226.47	0.0330 / 0.0360	Main Channel
132.22	226.59	0.0360	
143.72	227.42	0.0360	
169.59	227.88	0.0360	
192.59	231.19	0.0360	
218.46	233.02	0.0360	
241.45	235.50	0.0360	
284.57	236.43	0.0360	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.41	225.66	391E+04	1.7	1.36	38.31
0.81	226.06	804E+04	5.0	1.93	27.02
1.22	226.47	124E+05	9.1	2.30	22.61
1.84	227.09	308E+05	23.3	2.36	22.03
2.46	227.71	904E+05	62.0	2.14	24.31
3.09	228.34	223E+06	188.3	2.63	19.75
3.71	228.96	379E+06	410.2	3.38	15.39
4.33	229.58	550E+06	711.5	4.04	12.89
4.95	230.20	737E+06	1091.3	4.62	11.25
5.58	230.83	939E+06	1550.6	5.15	10.10
6.20	231.45	116E+07	2072.4	5.59	9.31
6.82	232.07	140E+07	2659.2	5.93	8.77
7.44	232.69	167E+07	3345.8	6.27	8.30
8.07	233.32	195E+07	4159.0	6.64	7.83
8.69	233.94	226E+07	5100.8	7.03	7.39
9.31	234.56	259E+07	6081.3	7.32	7.11
9.93	235.18	295E+07	7183.1	7.60	6.84
10.56	235.81	334E+07	8145.2	7.62	6.83
11.18	236.43	379E+07	9116.0	7.51	6.92

INFLOW : ID= 2 (8268)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW : ID= 1 (6402)	422.16	2.22	6.75	9.12	0.47	1.43
	422.16	2.00	7.75	9.12	0.44	1.39

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0402):	244.00	1.206	7.50	11.42
+ ID2= 2 (6402):	422.16	1.996	7.75	9.12
ID = 3 (8266):	666.16	3.200	7.75	9.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0404):	246.46	0.810	7.50	7.19
+ ID2= 2 (8266):	666.16	3.200	7.75	9.96
ID = 3 (8264):	912.62	4.005	7.75	9.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0406):	142.65	1.319	6.75	13.24
+ ID2= 2 (8264):	912.62	4.005	7.75	9.22
ID = 3 (8262):	1055.27	5.146	7.25	9.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (5506):	5660.52	16.265	11.75	10.27
+ ID2= 2 (8262):	1055.27	5.146	7.25	9.76
ID = 3 (8260):	6715.79	18.663	11.25	10.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0408):	231.62	1.542	6.75	10.25
+ ID2= 2 (8260):	6715.79	18.663	11.25	10.19
ID = 3 (8258):	6947.41	19.154	11.00	10.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0410):	572.01	1.460	8.25	7.48
+ ID2= 2 (8258):	6947.41	19.154	11.00	10.19
ID = 3 (8256):	7519.42	20.346	10.75	9.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5507)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2---> OUT= 1	0.0000	0.0000	90.6140	67.8416
DT= 15.0 min	39.6440	12.3348	*****	160.3529
	48.1390	18.5023	*****	160.3629
	67.9600	37.0045	0.0000	0.0000

INFLOW : ID= 2 (8256)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW : ID= 1 (5507)	7519.421	20.346	10.75	9.99
	7519.421	19.905	11.75	9.99

PEAK FLOW REDUCTION [Qout/Qin] (%) = 97.84

TIME SHIFT OF PEAK FLOW (min)= 60.00
 MAXIMUM STORAGE USED (ha. m.) = 6.1970

CALIB NASHYD (0420) Area (ha)= 175.82 Curve Number (CN)= 53.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.81

Unit Hyd Qpeak (cms)= 3.692
 PEAK FLOW (cms)= 0.822 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 8.762
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.161

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (4141) Area (ha)= 156.72 Dir. Conn. (%) = 21.00
 ID= 1 DT=15.0 min Total Imp(%)= 52.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 81.49 75.23
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1022.15 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 71.81 51.84
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 14.48 (ii) 28.40 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.07 0.04

TOTALS
 PEAK FLOW (cms)= 4.85 4.61 7.731 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 52.40 20.60 27.28
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.38 0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (4144) Area (ha)= 2.10 Dir. Conn. (%) = 10.00
 ID= 1 DT=15.0 min Total Imp(%)= 30.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.63 1.47
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 118.32 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 71.81 49.61
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 3.97 (ii) 18.13 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.11 0.05

TOTALS
 PEAK FLOW (cms)= 0.04 0.11 0.113 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.25
 RUNOFF VOLUME (mm)= 52.40 24.76 27.52
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.46 0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.0 Ia = Dep. Storage (Above)

- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9429) IN= 2---> OUT= 1 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.1000	0.0386
0.0100	0.0195	0.1400	0.0480
0.0400	0.0218	0.1700	0.0569
0.0700	0.0312	0.2000	0.0655

INFLOW : ID= 2 (4144)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
2.100	2.100	0.113	6.25	27.52
OUTFLOW: ID= 1 (9429)	2.100	0.053	6.75	27.30

PEAK FLOW REDUCTION [Qout/Qin] (%) = 46.76
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 0.0262

ADD HYD (9424) 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4141):	156.72	7.731	6.00	27.28
+ ID2= 2 (9429):	2.10	0.053	6.75	27.30
-----	-----	-----	-----	-----
ID = 3 (9424):	158.82	7.737	6.00	27.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5414) IN= 2---> OUT= 1 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.7060	4.4896
0.1870	1.6651	5.4940	5.4773
2.0210	2.0447	6.0800	6.4738
2.9590	2.4658	6.4800	6.4838
3.7490	3.5575	0.0000	0.0000

INFLOW : ID= 2 (9424)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
158.820	158.820	7.737	6.00	27.28
OUTFLOW: ID= 1 (5414)	158.820	2.545	7.00	27.26

PEAK FLOW REDUCTION [Qout/Qin] (%) = 32.89
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 2.2825

CALIB STANDHYD (4142) Area (ha)= 29.14 Dir. Conn. (%) = 21.00
 ID= 1 DT=15.0 min Total Imp(%)= 52.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 15.15 13.99
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 440.76 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 71.81 51.84
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 8.74 (ii) 22.66 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.04

TOTALS
 PEAK FLOW (cms)= 1.07 0.96 1.675 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 52.40 20.60 27.28
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.38 0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 66.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8254)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4142):	29.14	1.675	6.00	27.28
+ ID2= 2 (5414):	158.82	2.545	7.00	27.26

ID = 3 (8254):	187.96	3.020	6.75	27.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (4121)			
ID= 1 DT=15.0 min			
Area Total (ha)	Imp(%)	Dir. Conn. (%)	
202.00	45.00	16.00	

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area (mm)	90.90	111.10
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1160.46	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	71.81	17.38
over (min)	15.00	45.00
Storage Coeff. (min)	15.63 (ii)	37.17 (ii)
Unit Hyd. Tpeak (min)	15.00	45.00
Unit Hyd. peak (cms)	0.07	0.03

TOTALS
PEAK FLOW (cms) = 4.61 2.99 5.830 (iii)
TIME TO PEAK (hrs) = 6.00 6.50 6.00
RUNOFF VOLUME (mm) = 52.40 12.58 18.95
TOTAL RAINFALL (mm) = 54.40 54.40 54.40
RUNOFF COEFFICIENT = 0.96 0.23 0.35

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 51.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5412)				
IN= 2--> OUT= 1				
DT= 15.0 min				
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)	
0.0000	0.0000	6.5000	1.0500	
0.3860	0.5085	7.9000	1.1500	
1.8460	0.6103	9.3000	1.2500	
3.5050	0.7566	9.7000	1.2600	
4.7890	0.8929	0.0000	0.0000	

INFLOW: ID= 2 (4121)	202.000	5.830	6.00	18.95
OUTFLOW: ID= 1 (5412)	202.000	4.284	6.50	18.95

PEAK FLOW REDUCTION [Qout/Qin] (%) = 73.48
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha.m.) = 0.8453

CALIB STANDHYD (4122)			
ID= 1 DT=15.0 min			
Area Total (ha)	Imp(%)	Dir. Conn. (%)	
36.70	63.00	22.00	

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area (mm)	23.12	13.58
Dep. Storage (mm)	2.00	5.00

Average Slope (%)	=	0.50	0.50
Length (m)	=	494.64	40.00
Mannings n	=	0.013	0.250
Max. Eff. Inten. (mm/hr)	=	71.81	51.82
over (min)	=	15.00	30.00
Storage Coeff. (min)	=	9.37 (ii)	23.29 (ii)
Unit Hyd. Tpeak (min)	=	15.00	30.00
Unit Hyd. peak (cms)	=	0.09	0.04

PEAK FLOW (cms)	=	1.39	0.92	*TOTALS* 1.959 (iii)
TIME TO PEAK (hrs)	=	6.00	6.25	6.00
RUNOFF VOLUME (mm)	=	52.40	16.36	24.29
TOTAL RAINFALL (mm)	=	54.40	54.40	54.40
RUNOFF COEFFICIENT	=	0.96	0.30	0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 51.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9412)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4122):	36.70	1.959	6.00	24.29
+ ID2= 2 (5412):	202.00	4.284	6.50	18.95

ID = 3 (9412):	238.70	5.407	6.25	19.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8252)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8254):	187.96	3.020	6.75	27.27
+ ID2= 2 (9412):	238.70	5.407	6.25	19.77

ID = 3 (8252):	426.66	8.117	6.50	23.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6416)		Routing time step (min) = 15.00	
IN= 2--> OUT= 1			

<----- DATA FOR SECTION (4161.0) ----->			
Distance (m)	Elevation (m)	Manning	
0.00	270.07	0.0340	
20.67	267.91	0.0340	
62.01	264.33	0.0340	
113.69	259.75	0.0340	
165.37	253.30	0.0340	
227.38	246.29	0.0340	
232.55	246.03	0.0340	
237.72	246.16	0.0340	
241.39	246.02	0.0340 / 0.0300	Main Channel
241.64	245.75	0.0300	Main Channel
242.89	245.75	0.0300	Main Channel
244.14	245.75	0.0300	Main Channel
244.39	246.02	0.0300 / 0.0340	Main Channel
248.06	246.20	0.0340	
253.22	246.28	0.0340	
258.39	246.63	0.0340	
346.25	252.57	0.0340	
413.43	257.77	0.0340	
465.11	261.78	0.0340	
511.62	270.00	0.0340	

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.27	246.02	295E+04	0.6	0.75	88.26
1.53	247.28	.168E+06	69.7	1.65	40.13
2.79	248.54	.490E+06	299.5	2.42	27.28

4.06	249.81	.962E+06	740.1	3.05	21.67
5.32	251.07	.158E+07	1440.0	3.61	18.33
6.58	252.33	.235E+07	2443.8	4.12	16.06
7.84	253.59	.327E+07	3811.2	4.63	14.30
9.10	254.85	.432E+07	5565.1	5.11	12.94
10.37	256.12	.550E+07	7721.3	5.57	11.88
11.63	257.38	.682E+07	10309.9	6.00	11.02
12.89	258.64	.827E+07	13360.6	6.42	10.31
14.15	259.90	.985E+07	16884.5	6.81	9.72
15.42	261.17	.116E+08	20818.4	7.14	9.26
16.68	262.43	.134E+08	25510.4	7.53	8.78
17.94	263.69	.154E+08	30958.0	7.97	8.30
19.20	264.95	.175E+08	36988.3	8.38	7.89
20.46	266.21	.197E+08	43617.1	8.78	7.53
21.73	267.48	.220E+08	50868.1	9.17	7.22
22.99	268.74	.244E+08	58854.0	9.56	6.92

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8252)	426.66	8.12	6.50	23.07	0.41	0.80
OUTFLOW: ID= 1 (6416)	426.66	4.60	7.50	23.07	0.34	0.77

CALIB NASHYD (4164) Area (ha)= 89.30 Curve Number (CN)= 76.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 3.00
 U. H. Tp(hrs)= 1.46

Unit Hyd Qpeak (cms)= 2.344
 PEAK FLOW (cms)= 1.169 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 18.827
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.346

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9427)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2--> OUT= 1 DT= 15.0 min	0.0000	0.0000	4.4700	2.5000
	0.9000	1.0000	5.9800	3.0000
	1.7300	1.5000	7.5000	3.5000
	3.2200	2.0000	8.7400	4.0000

INFLOW : ID= 2 (4164)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (9427)	89.300	1.169	7.50	18.83
	89.300	0.616	9.50	18.82

PEAK FLOW REDUCTION [Qout/Qin] (%) = 52.71
 TIME SHIFT OF PEAK FLOW (min) = 120.00
 MAXIMUM STORAGE USED (ha. m.) = 0.6857

RESERVOIR (5416)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2--> OUT= 1 DT= 15.0 min	0.0000	0.0000	1.5980	0.7565
	0.0330	0.3532	1.8570	0.8137
	0.7450	0.4481	2.0990	0.8707
	1.0270	0.5154	2.4990	0.8807
	1.3050	0.6350	0.0000	0.0000

INFLOW : ID= 2 (9427)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5416)	89.300	0.616	9.50	18.82
	89.300	0.593	10.50	18.79

PEAK FLOW REDUCTION [Qout/Qin] (%) = 96.28
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 0.4281

CALIB NASHYD (4162) Area (ha)= 349.99 Curve Number (CN)= 61.3
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.29

Unit Hyd Qpeak (cms)= 4.646
 PEAK FLOW (cms)= 1.536 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 11.553
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.212

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9416)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (4162):	349.99	1.536	8.00	11.55
+ ID2= 2 (5416):	89.30	0.593	10.50	18.79
ID= 3 (9416):	439.29	1.882	10.00	13.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8250)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (6416):	426.66	4.601	7.50	23.07
+ ID2= 2 (9416):	439.29	1.882	10.00	13.02
ID= 3 (8250):	865.95	6.117	7.75	17.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0418) Area (ha)= 174.09 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.05

Unit Hyd Qpeak (cms)= 2.825
 PEAK FLOW (cms)= 0.966 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 12.572
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.231

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (4184) Area Total (ha)= 8.70 Imp(%)= 30.00 Di r. Conn. (%)= 10.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 2.61 6.09
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 240.83 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 71.81 25.10
 over (min) 15.00 30.00
 Storage Coeff. (min)= 6.08 (ii) 24.68 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.10 0.04

TOTALS
 PEAK FLOW (cms)= 0.16 0.32 0.361 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 52.40 20.65 23.82
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.38 0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9428)
IN= 2----> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.4300	0.1618
0.0300	0.0818	0.5800	0.2012
0.1700	0.0912	0.7300	0.2384
0.3100	0.1306	0.8500	0.2747

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (4184)	8.700	0.361	6.00	23.82
OUTFLOW: ID= 1 (9428)	8.700	0.179	6.75	23.74

PEAK FLOW REDUCTION [Qout/Qin] (%) = 49.69
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.0939

ADD HYD (9423)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0418):	174.09	0.966	7.50	12.57
+ ID2= 2 (9428):	8.70	0.179	6.75	23.74
=====				
ID = 3 (9423):	182.79	1.114	7.00	13.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8248)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8250):	865.95	6.117	7.75	17.98
+ ID2= 2 (9423):	182.79	1.114	7.00	13.10
=====				
ID = 3 (8248):	1048.74	7.172	7.50	17.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8246)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0420):	175.82	0.822	7.00	8.76
+ ID2= 2 (8248):	1048.74	7.172	7.50	17.13
=====				
ID = 3 (8246):	1224.56	7.973	7.50	15.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8244)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (5507):	7519.42	19.905	11.75	9.99
+ ID2= 2 (8246):	1224.56	7.973	7.50	15.93
=====				
ID = 3 (8244):	8743.98	24.107	11.00	10.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB

STANDHYD (4227) | Area (ha) = 66.80
ID= 1 DT=15.0 min | Total Imp(%) = 29.00 Dir. Conn. (%) = 15.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	19.37	47.43
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	667.33	40.00
Mannings n	0.013	0.250

			TOTALS
Max. Eff. Inten. (mm/hr)=	71.81	11.70	1.974 (iii)
over (min)	15.00	45.00	6.00
Storage Coeff. (min)=	11.21 (ii)	36.45 (ii)	17.14
Unit Hyd. Tpeak (min)=	15.00	45.00	54.40
Unit Hyd. peak (cms)=	0.08	0.03	0.32
PEAK FLOW (cms)=	1.63	0.87	
TIME TO PEAK (hrs)=	6.00	6.50	
RUNOFF VOLUME (mm)=	52.40	10.92	
TOTAL RAINFALL (mm)=	54.40	54.40	
RUNOFF COEFFICIENT =	0.96	0.20	

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 54.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9426)
IN= 2----> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.3400	1.2303
0.2300	0.6171	4.4700	1.5350
1.3000	0.6860	5.6100	1.8228
2.4100	0.9909	6.5300	2.1041

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (4227)	66.800	1.974	6.00	17.14
OUTFLOW: ID= 1 (9426)	66.800	0.512	7.50	17.13

PEAK FLOW REDUCTION [Qout/Qin] (%) = 25.93
TIME SHIFT OF PEAK FLOW (min) = 90.00
MAXIMUM STORAGE USED (ha. m.) = 0.6373

RESERVOIR (5422)
IN= 2----> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.3110	2.6782
0.2180	1.7874	5.4130	2.9186
1.2490	1.9110	6.3010	3.1540
2.3250	2.1760	6.7010	3.1640
3.2250	2.4038	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9426)	66.800	0.512	7.50	17.13
OUTFLOW: ID= 1 (5422)	66.800	0.080	19.00	17.10

PEAK FLOW REDUCTION [Qout/Qin] (%) = 15.70
TIME SHIFT OF PEAK FLOW (min) = 690.00
MAXIMUM STORAGE USED (ha. m.) = 0.6589

CALIB NASHYD (4222)
ID= 1 DT=15.0 min

Area (ha) = 713.41 Curve Number (CN) = 54.0
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 1.95

Unit Hyd Opeak (cms) = 6.252

PEAK FLOW (cms) = 1.787 (i)
TIME TO PEAK (hrs) = 9.250

RUNOFF VOLUME (mm) = 9.147
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.168

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9422) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4222):	713.41	1.787	9.25	9.15
+ ID2= 2 (5422):	66.80	0.080	19.00	17.10
ID = 3 (9422):	780.21	1.829	9.25	9.83

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8242) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8244):	8743.98	24.107	11.00	10.82
+ ID2= 2 (9422):	780.21	1.829	9.25	9.83
ID = 3 (8242):	9524.19	25.855	10.75	10.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5508) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	*****	197.3574
	76.4550	30.8371	*****	394.7148
	*****	61.6742	*****	394.7248
	*****	*****	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (8242)	9524.190	25.855	10.75	10.74
OUTFLOW: ID= 1 (5508)	9524.190	25.053	12.00	10.74

PEAK FLOW REDUCTION [Oout/Oi n] (%) = 96.90
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha. m.) = 10.1112

CALIB NASHYD (0336) ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)
	2785.00	72.0
	La (mm) = 5.00	# of Linear Res. (N) = 1.50
	U. H. Tp (hrs) = 15.39	

Unit Hyd Opeak (cms) = 3.090
 PEAK FLOW (cms) = 1.997 (i)
 TIME TO PEAK (hrs) = 22.250
 RUNOFF VOLUME (mm) = 16.446
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.302

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3382) ID= 1 DT=15.0 min	Area (ha)	Dir. Conn. (%)
	433.29	20.00
	Total Imp (%) = 39.00	

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 168.98 264.31
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 1699.59 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr) = 48.96 19.96
 over (min) = 30.00 45.00

Storage Coeff. (min) = 22.90 (ii) 43.28 (ii)
 Unit Hyd. Tpeak (min) = 30.00 45.00
 Unit Hyd. peak (cms) = 0.04 0.03

TOTALS
 14.179 (iii)
 6.25
 23.66
 54.40
 0.43

PEAK FLOW (cms) = 8.72 7.47
 TIME TO PEAK (hrs) = 6.25 6.50
 RUNOFF VOLUME (mm) = 52.40 16.47
 TOTAL RAINFALL (mm) = 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.30

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 64.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3381) ID= 1 DT=15.0 min	Area (ha)	Dir. Conn. (%)
	165.62	27.00
	Total Imp (%) = 53.00	

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 87.78 77.84
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 1050.78 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 71.81 44.37
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 14.73 (ii) 29.53 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.07 0.04

PEAK FLOW (cms) = 6.55 3.98
 TIME TO PEAK (hrs) = 6.00 6.25
 RUNOFF VOLUME (mm) = 52.40 18.76
 TOTAL RAINFALL (mm) = 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.34

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 64.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5338) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	9.5300	3.8529
	0.6370	1.6350	12.1350	4.2725
	2.7300	2.1009	14.3850	4.6808
	5.1010	2.7445	14.7850	4.6908
	7.0460	3.2936	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3381)	165.620	9.015	6.00	27.84
OUTFLOW: ID= 1 (5338)	165.620	3.192	6.75	27.84

PEAK FLOW REDUCTION [Oout/Oi n] (%) = 35.41
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 2.2355

ADD HYD (8310) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3382):	433.29	14.179	6.25	23.66
+ ID2= 2 (5338):	165.62	3.192	6.75	27.84
ID = 3 (8310):	598.91	15.937	6.50	24.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (3342) Area (ha)= 586.87 Curve Number (CN)= 56.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 8.20

Unit Hyd Opeak (cms)= 1.221
 PEAK FLOW (cms)= 0.466 (i)
 TIME TO PEAK (hrs)= 15.500
 RUNOFF VOLUME (mm)= 9.797
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.180

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3341) Area (ha)= 33.23
 ID= 1 DT=15.0 min Total Imp(%)= 51.00 Dir. Conn.(%)= 21.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 16.95 16.28
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 470.67 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 71.81 19.53
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 9.10 (ii) 29.66 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.04

TOTALS
 PEAK FLOW (cms)= 1.21 0.60 1.573 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 52.40 13.34 21.54
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.25 0.40

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 51.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5334)
 IN= 2----> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.6690	0.4664
0.0400	0.1927	0.8520	0.5318
0.1900	0.2643	1.0360	0.5973
0.3830	0.3576	1.4360	0.6073
0.4460	0.3863	0.0000	0.0000

INFLOW: ID= 2 (3341)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
	33.230	1.573	6.00	21.54
OUTFLOW: ID= 1 (5334)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
	33.230	0.398	7.00	21.50

PEAK FLOW REDUCTION [Qout/Qin](%)= 25.31
 TIME SHIFT OF PEAK FLOW (min)= 60.00
 MAXIMUM STORAGE USED (ha. m.)= 0.3664

ADD HYD (8314)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3342):	586.87	0.466	15.50	9.80
+ ID2= 2 (5334):	33.23	0.398	7.00	21.50
ID = 3 (8314):	620.10	0.587	7.25	10.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0306) Area (ha)= 283.97 Curve Number (CN)= 52.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 6.44

Unit Hyd Opeak (cms)= 0.753
 PEAK FLOW (cms)= 0.250 (i)
 TIME TO PEAK (hrs)= 13.750
 RUNOFF VOLUME (mm)= 8.592
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.158

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0282) Area (ha)= 449.38 Curve Number (CN)= 77.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.47

Unit Hyd Opeak (cms)= 5.226
 PEAK FLOW (cms)= 3.035 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 19.368
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.356

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0284) Area (ha)= 78.93 Curve Number (CN)= 84.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.57

Unit Hyd Opeak (cms)= 2.344
 PEAK FLOW (cms)= 1.445 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 24.380
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.448

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8388)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0282):	449.38	3.035	8.00	19.37
+ ID2= 2 (0284):	78.93	1.445	6.50	24.38
ID = 3 (8388):	528.31	4.143	7.25	20.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6286)
 IN= 2----> OUT= 1

Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2861.0) ----->

Distance	Elevation	Manning
0.00	233.00	0.0450
20.58	228.51	0.0450
41.17	227.74	0.0450
51.46	227.41	0.0450
97.77	225.96	0.0450
149.23	223.94	0.0450
200.69	220.64	0.0450
226.42	220.66	0.0450
238.85	220.22	0.0450
241.35	220.01	0.0450 / 0.0350
241.85	219.70	0.0350

Main Channel
 Main Channel

245.85	219.72	0.0350	Main Channel
246.35	220.06	0.0350	Main Channel
248.85	220.23	0.0350	Main Channel
303.60	221.64	0.0350	Main Channel
380.79	224.98	0.0450	
432.25	229.54	0.0450	
457.98	233.33	0.0450	
483.71	234.27	0.0450	
509.44	233.81	0.0450	

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	TRAVEL TIME (mins)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.31	220.01	.382E+04	0.8	0.57	82.89
1.06	220.76	.525E+05	12.1	0.66	72.51
1.81	221.51	.240E+06	88.6	1.05	45.21
2.56	222.26	.510E+06	261.7	1.46	32.47
3.31	223.01	.843E+06	533.8	1.81	26.34
4.06	223.76	.124E+07	913.6	2.10	22.64
4.81	224.51	.171E+07	1399.0	2.34	20.33
5.56	225.26	.225E+07	2051.0	2.60	18.27
6.31	226.01	.285E+07	2907.0	2.91	16.35
7.06	226.76	.352E+07	3902.5	3.16	15.04
7.82	227.52	.426E+07	5069.9	3.39	14.01
8.57	228.27	.507E+07	6433.0	3.62	13.13
9.32	229.02	.593E+07	8069.2	3.89	12.24
10.07	229.77	.681E+07	9926.7	4.16	11.44
10.82	230.52	.772E+07	11962.8	4.42	10.75
11.57	231.27	.864E+07	14167.1	4.68	10.17
12.32	232.02	.959E+07	16537.1	4.92	9.66
13.07	232.77	.105E+08	19070.7	5.16	9.22
13.82	233.52	.115E+08	21790.4	5.39	8.82

<---- hydrograph ---->				<- pi pe / channel ->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8388)	528.31	4.14	7.25	20.12	0.53
OUTFLOW: ID= 1 (6286)	528.31	3.51	9.25	20.12	0.49

CALIB STANDHYD (2861)	Area (ha)= 54.86	Di r. Conn. (%)= 19.00
ID= 1 DT=15.0 mi n	Total Imp(%)= 45.00	

Surface Area (ha)=	24.69	PERVIOUS (i)	30.17
Dep. Storage (mm)=	2.00		5.00
Average Slope (%)=	0.50		0.50
Length (m)=	604.76		40.00
Mannings n =	0.013		0.250
Max. Eff. Inten. (mm/hr)=	71.81	61.65	
over (mi n)	15.00	30.00	
Storage Coeff. (mi n)=	10.57 (ii)	23.55 (ii)	
Unit Hyd. Tpeak (mi n)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.09	0.04	

PEAK FLOW (cms)=	1.73	2.43	*TOTALS*	3.288 (iii)
TIME TO PEAK (hrs)=	6.00	6.25		6.00
RUNOFF VOLUME (mm)=	52.40	26.86		31.71
TOTAL RAINFALL (mm)=	54.40	54.40		54.40
RUNOFF COEFFICIENT =	0.96	0.49		0.58

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5286)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2--> OUT= 1	0.0000	0.0000	3.6700	1.4285
DT= 15.0 mi n	0.1860	0.6703	4.6080	1.6332
	1.0630	0.7754	5.3640	1.8336

1.9790	1.0010	5.7640	1.8436
2.7460	1.1950	0.0000	0.0000

INFLOW : ID= 2 (2861)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5286)	54.860	3.288	6.00	31.71
	54.860	1.389	6.75	31.70

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 42.23
 TIME SHIF T OF PEAK FLOW (mi n) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 0.8558

CALIB STANDHYD (2862)	Area (ha)= 205.65	Di r. Conn. (%)= 10.00
ID= 1 DT=15.0 mi n	Total Imp(%)= 22.00	

Surface Area (ha)=	45.24	PERVIOUS (i)	160.41
Dep. Storage (mm)=	2.00		5.00
Average Slope (%)=	0.50		0.50
Length (m)=	1170.90		40.00
Mannings n =	0.013		0.250

Max. Eff. Inten. (mm/hr)=	71.81	25.56	
over (mi n)	15.00	45.00	
Storage Coeff. (mi n)=	15.71 (ii)	34.18 (ii)	
Unit Hyd. Tpeak (mi n)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.07	0.03	

PEAK FLOW (cms)=	2.93	6.55	*TOTALS*	7.457 (iii)
TIME TO PEAK (hrs)=	6.00	6.50		6.50
RUNOFF VOLUME (mm)=	52.40	23.09		26.02
TOTAL RAINFALL (mm)=	54.40	54.40		54.40
RUNOFF COEFFICIENT =	0.96	0.42		0.48

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9286)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (2862):	205.65	7.457	6.50	26.02
+ ID2= 2 (5286):	54.86	1.389	6.75	31.70
ID = 3 (9286):	260.51	8.699	6.50	27.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8386)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (6286):	528.31	3.506	9.25	20.12
+ ID2= 2 (9286):	260.51	8.699	6.50	27.21
ID = 3 (8386):	788.82	9.699	6.50	22.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0302)	Area (ha)= 473.90	Curve Number (CN)= 58.0
ID= 1 DT=15.0 mi n	Ia (mm)= 5.00	# of Li near Res. (N)= 1.50
	U. H. Tp(hrs)= 1.66	

Unit Hyd Opeak (cms)=	4.874	
PEAK FLOW (cms)=	1.530 (i)	
TIME TO PEAK (hrs)=	8.500	
RUNOFF VOLUME (mm)=	10.409	

TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.191

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0298) Area (ha) = 330.51 Curve Number (CN) = 45.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 1.26

Unit Hyd Opeak (cms) = 4.461

PEAK FLOW (cms) = 0.846 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 6.733
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.124

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3001) Area (ha) = 0.09 Dir. Conn. (%) = 15.00
 ID= 1 DT=15.0 min Total Imp(%) = 45.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	0.04	0.05
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	24.49	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 71.81 15.30
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 1.54 (ii) 24.21 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.11 0.04

TOTALS
 PEAK FLOW (cms) = 0.00 0.00 0.004 (iii)
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 52.40 11.03 17.08
 TOTAL RAINFALL (mm) = 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.20 0.31

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 46.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5300) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	0.0060	0.0023
	0.0020	0.0012	0.0070	0.0026
	0.0030	0.0016	0.0090	0.0029
	0.0040	0.0019	0.4090	0.0129

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (3001)	0.090	0.004	6.00	17.08
OUTFLOW: ID= 1 (5300)	0.090	0.001	6.75	15.86

PEAK FLOW REDUCTION [Out/In] (%) = 27.91
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0006

CALIB NASHYD (3002) Area (ha) = 258.84 Curve Number (CN) = 51.8
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 1.03

Unit Hyd Opeak (cms) = 4.290

PEAK FLOW (cms) = 0.970 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 8.457
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.155

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9300) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3002):	258.84	0.970	7.50	8.46
+ ID2= 2 (5300):	0.09	0.001	6.75	15.86
ID= 3 (9300):	258.93	0.971	7.50	8.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8395) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0298):	330.51	0.846	8.00	6.73
+ ID2= 2 (9300):	258.93	0.971	7.50	8.46
ID= 3 (8395):	589.44	1.807	7.75	7.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0288) Area (ha) = 340.83 Curve Number (CN) = 78.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 2.21

Unit Hyd Opeak (cms) = 2.629

PEAK FLOW (cms) = 1.727 (i)
 TIME TO PEAK (hrs) = 9.500
 RUNOFF VOLUME (mm) = 20.098
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.369

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0290) Area (ha) = 269.18 Curve Number (CN) = 78.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 1.07

Unit Hyd Opeak (cms) = 4.279

PEAK FLOW (cms) = 2.388 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 19.976
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.367

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8397) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0288):	340.83	1.727	9.50	20.10
+ ID2= 2 (0290):	269.18	2.388	7.50	19.98
ID= 3 (8397):	610.01	3.977	8.00	20.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0296) Area (ha) = 293.65 Curve Number (CN) = 76.0
 ID= 1 DT=15.0 min I a (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 1.13

Unit Hyd Qpeak (cms) = 4.437

PEAK FLOW (cms) = 2.330 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 18.668
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.343

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0292) Area (ha) = 738.49 Curve Number (CN) = 68.0
 ID= 1 DT=15.0 min I a (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 1.52

Unit Hyd Qpeak (cms) = 8.289

PEAK FLOW (cms) = 3.557 (i)
 TIME TO PEAK (hrs) = 8.250
 RUNOFF VOLUME (mm) = 14.367
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.264

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0294) Area (ha) = 274.15 Curve Number (CN) = 57.0
 ID= 1 DT=15.0 min I a (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 0.87

Unit Hyd Qpeak (cms) = 5.367

PEAK FLOW (cms) = 1.387 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 9.999
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.184

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	(8398)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0292):	738.49	3.557	8.25	14.37	
+ ID2= 2 (0294):	274.15	1.387	7.25	10.00	
ID = 3 (8398):	1012.64	4.858	8.00	13.18	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6296)
 IN= 2--> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (2961.0) ----->			
Distance	Elevation	Manning	
0.00	243.98	0.0400	
42.59	243.18	0.0400	
85.17	241.81	0.0400	
120.66	240.50	0.0400	
156.15	239.56	0.0400	
198.74	236.15	0.0400	
237.78	234.01	0.0400	
241.33	233.82	0.0400	
248.77	233.12	0.0400 / 0.0400	Main Channel
249.87	232.32	0.0400	Main Channel
250.37	231.80	0.0400	Main Channel
250.87	232.23	0.0400	Main Channel
251.97	233.10	0.0400 / 0.0400	Main Channel
255.37	233.22	0.0400	

259.07	233.87	0.0400
262.62	234.12	0.0400
266.17	234.23	0.0400
283.91	234.73	0.0400
337.15	241.75	0.0400
351.34	244.00	0.0400

<----- TRAVEL TIME TABLE ----->						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME	
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)	
0.43	232.23	510E+03	0.1	0.53	81.07	
0.87	232.67	213E+04	0.7	0.83	51.49	
1.30	233.10	501E+04	2.2	1.10	38.75	
1.98	233.78	242E+05	12.6	1.34	31.93	
2.66	234.46	738E+05	42.1	1.46	29.21	
3.34	235.14	177E+06	127.9	1.84	23.13	
4.02	235.82	314E+06	281.4	2.29	18.61	
4.70	236.50	481E+06	509.4	2.71	15.73	
5.38	237.18	672E+06	815.6	3.11	13.73	
6.06	237.86	886E+06	1200.7	3.46	12.30	
6.74	238.54	112E+07	1669.4	3.80	11.23	
7.42	239.22	139E+07	2226.9	4.11	10.38	
8.10	239.90	168E+07	2808.4	4.28	9.95	
8.78	240.58	202E+07	3470.4	4.40	9.68	
9.46	241.26	240E+07	4331.7	4.61	9.24	
10.14	241.94	283E+07	5335.2	4.83	8.84	
10.82	242.62	330E+07	6482.6	5.03	8.48	
11.50	243.30	381E+07	7764.1	5.21	8.18	
12.18	243.98	439E+07	9101.2	5.31	8.04	

<---- hydrograph ---->						<- pi pe / channel ->	
INFLOW :	ID=	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
OUTFLOW :	ID=	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
ID= 2 (8398)	1012.64	4.86	8.00	13.18	1.48	1.15	
ID= 1 (6296)	1012.64	4.69	8.75	13.18	1.46	1.15	

ADD HYD	(8396)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0296):	293.65	2.330	7.50	18.67	
+ ID2= 2 (6296):	1012.64	4.688	8.75	13.18	
ID = 3 (8396):	1306.29	6.905	8.25	14.42	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8394)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (8396):	1306.29	6.905	8.25	14.42	
+ ID2= 2 (8397):	610.01	3.977	8.00	20.04	
ID = 3 (8394):	1916.30	10.857	8.25	16.21	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8392)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (8394):	1916.30	10.857	8.25	16.21	
+ ID2= 2 (8395):	589.44	1.807	7.75	7.49	
ID = 3 (8392):	2505.74	12.633	8.25	14.16	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6302)
 IN= 2--> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3021.0) ----->		
Distance	Elevation	Manning
0.00	228.10	0.0400

18.47	227.12	0.0400	
36.95	226.12	0.0400	
46.18	225.84	0.0400	
55.42	225.58	0.0400	
272.47	222.88	0.0400	
277.09	222.76	0.0400	
281.71	222.58	0.0400	
288.54	222.18	0.0400 / 0.0300	Main Channel
288.64	221.00	0.0300	Main Channel
290.94	221.00	0.0300	Main Channel
291.04	221.00	0.0300	Main Channel
291.54	222.75	0.0300 / 0.0400	Main Channel
300.18	222.83	0.0400	
304.80	223.04	0.0400	
309.42	223.25	0.0400	
318.65	223.69	0.0400	
360.22	225.57	0.0400	
397.16	227.60	0.0400	
457.20	228.35	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.29	221.29	.228E+04	0.2	0.32	165.63
0.59	221.59	.465E+04	0.7	0.45	116.15
0.88	221.88	.713E+04	1.2	0.54	96.87
1.18	222.18	.971E+04	1.9	0.61	86.20
1.57	222.57	.175E+05	3.3	0.59	88.86
1.97	222.97	.427E+05	6.4	0.47	111.39
2.36	223.36	.115E+06	16.1	0.44	119.14
2.76	223.76	.237E+06	37.1	0.49	106.49
3.15	224.15	.409E+06	72.9	0.56	93.40
3.55	224.55	.631E+06	127.0	0.63	82.76
3.94	224.94	.903E+06	202.4	0.70	74.37
4.34	225.34	.123E+07	301.8	0.77	67.68
4.73	225.73	.160E+07	433.5	0.85	61.37
5.13	226.13	.200E+07	602.2	0.95	55.29
5.52	226.52	.242E+07	804.7	1.05	50.13
5.92	226.92	.286E+07	1034.2	1.14	46.11
6.31	227.31	.332E+07	1290.5	1.22	42.88
6.71	227.71	.380E+07	1560.9	1.29	40.55
7.10	228.10	.432E+07	1835.6	1.34	39.19

<---- hydrograph ----> <-- pipe / channel -->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8392)	2505.74	12.63	8.25	14.16	2.22	0.45
OUTFLOW: ID= 1 (6302)	2505.74	10.17	10.50	14.16	2.12	0.46

ADD HYD (8390)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0302):	473.90	1.530	8.50	10.41
+ ID2= 2 (6302):	2505.74	10.168	10.50	14.16
ID = 3 (8390):	2979.64	11.571	10.25	13.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8348)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (8386):	788.82	9.969	6.50	22.46
+ ID2= 2 (8390):	2979.64	11.571	10.25	13.56
ID = 3 (8348):	3768.46	16.007	10.00	15.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0304)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	292.37	63.0
	U. H. Tp(hrs)= 5.00	# of Linear Res. (N)= 1.50
	U. H. Tp(hrs)= 2.78	

Unit Hyd Opeak (cms)= 1.793

PEAK FLOW (cms)=	0.753 (i)
TIME TO PEAK (hrs)=	10.500
RUNOFF VOLUME (mm)=	12.262
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.225

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0280)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	299.86	82.0
	U. H. Tp(hrs)= 5.00	# of Linear Res. (N)= 1.50
	U. H. Tp(hrs)= 0.85	

Unit Hyd Opeak (cms)= 6.009

PEAK FLOW (cms)=	3.720 (i)
TIME TO PEAK (hrs)=	7.000
RUNOFF VOLUME (mm)=	22.907
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.421

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0278)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	485.49	82.0
	U. H. Tp(hrs)= 5.00	# of Linear Res. (N)= 1.50
	U. H. Tp(hrs)= 1.52	

Unit Hyd Opeak (cms)= 5.453

PEAK FLOW (cms)=	3.831 (i)
TIME TO PEAK (hrs)=	8.000
RUNOFF VOLUME (mm)=	23.080
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.424

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0276)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	90.89	79.0
	U. H. Tp(hrs)= 5.00	# of Linear Res. (N)= 1.50
	U. H. Tp(hrs)= 0.67	

Unit Hyd Opeak (cms)= 2.302

PEAK FLOW (cms)=	1.207 (i)
TIME TO PEAK (hrs)=	6.750
RUNOFF VOLUME (mm)=	20.490
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.377

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0274)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	392.49	75.0
	U. H. Tp(hrs)= 5.00	# of Linear Res. (N)= 1.50
	U. H. Tp(hrs)= 1.08	

Unit Hyd Opeak (cms)= 6.182

PEAK FLOW (cms)=	3.104 (i)
TIME TO PEAK (hrs)=	7.500
RUNOFF VOLUME (mm)=	18.038
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.332

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8360)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0274):	392.49	3.104	7.50	18.04

+ ID2= 2 (0276): 90.89 1.207 6.75 20.49

 ID = 3 (8360): 483.38 4.235 7.25 18.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8358)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0278):	485.49	3.831	8.00	23.08
+ ID2= 2 (8360):	483.38	4.235	7.25	18.50

ID = 3 (8358):	968.87	7.908	7.75	20.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6280)
 IN= 2--> OUT= 1 Routing time step (min)' = 15.00

----- DATA FOR SECTION (2801.0) ----->

Distance	Elevation	Manning	
0.00	241.14	0.0500	
13.32	240.80	0.0500	
39.95	240.07	0.0500	
96.54	236.21	0.0500	
113.19	234.15	0.0500	
123.18	232.35	0.0500	
143.15	225.80	0.0500	
149.81	225.62	0.0500	
153.14	225.40	0.0500	
157.30	224.76	0.0500 / 0.0300	Main Channel
159.80	224.26	0.0300	Main Channel
162.30	224.85	0.0300	Main Channel
162.55	224.97	0.0300 / 0.0500	Main Channel
167.80	225.05	0.0500	
186.43	229.14	0.0500	
213.06	234.75	0.0500	
236.37	237.09	0.0500	
266.33	237.31	0.0500	
292.96	237.83	0.0500	
329.58	241.50	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	224.76	.263E+04	0.8	0.65	58.31
1.36	225.62	.278E+05	13.4	1.10	34.54
2.22	226.48	.855E+05	52.6	1.40	27.09
3.09	227.35	.157E+06	120.4	1.74	21.78
3.95	228.21	.242E+06	216.3	2.04	18.64
4.81	229.07	.340E+06	341.8	2.29	16.56
5.67	229.93	.450E+06	498.3	2.52	15.06
6.53	230.79	.574E+06	688.3	2.73	13.90
7.40	231.66	.711E+06	913.8	2.93	12.97
8.26	232.52	.861E+06	1173.4	3.11	12.23
9.12	233.38	.103E+07	1461.3	3.24	11.72
9.98	234.24	.121E+07	1793.3	3.37	11.26
10.85	235.11	.142E+07	2141.0	3.45	11.02
11.71	235.97	.165E+07	2530.4	3.50	10.86
12.57	236.83	.192E+07	2954.0	3.51	10.81
13.43	237.69	.226E+07	3160.3	3.18	11.93
14.29	238.55	.271E+07	3869.4	3.26	11.67
15.16	239.42	.320E+07	4740.3	3.38	11.24
16.02	240.28	.373E+07	5688.4	3.48	10.92

<----- hydrograph -----> <--pi pe / channel -->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8358)	968.87	7.91	7.75	20.79	0.99	0.85
OUTFLOW : ID= 1 (6280)	968.87	7.48	8.50	20.79	0.95	0.83

ADD HYD (8354)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0280):	299.86	3.720	7.00	22.91

+ ID2= 2 (6280): 968.87 7.477 8.50 20.79

 ID = 3 (8354): 1268.73 10.681 8.00 21.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0270)
 ID= 1 DT=15.0 min

Area (ha)	Curve Number (CN)
243.61	81.0
Imp (mm)= 5.00	# of Linear Res. (N)= 1.30
U. H. Tp(hrs)= 0.87	

Unit Hyd Opeak (cms)= 3.429

PEAK FLOW (cms)=	2.117 (i)
TIME TO PEAK (hrs)=	7.500
RUNOFF VOLUME (mm)=	22.091
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.406

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0268)
 ID= 1 DT=15.0 min

Area (ha)	Curve Number (CN)
215.76	75.0
Imp (mm)= 5.00	# of Linear Res. (N)= 1.30
U. H. Tp(hrs)= 0.69	

Unit Hyd Opeak (cms)= 3.807

PEAK FLOW (cms)=	1.800 (i)
TIME TO PEAK (hrs)=	7.000
RUNOFF VOLUME (mm)=	17.875
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.329

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (0266)
 ID= 1 DT=15.0 min

Area Total	Imp(%)	Curve Number (CN)	Di r. Conn. (%)
508.09	75.00	64.0	70.00
Imp (mm)= 5.00		# of Linear Res. (N)= 1.30	
U. H. Tp(hrs)= 1.63			

Unit Hyd Opeak (cms)= 3.786

PEAK FLOW (cms)=	1.507 (i)
TIME TO PEAK (hrs)=	9.500
RUNOFF VOLUME (mm)=	12.617
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.232

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2641)
 ID= 1 DT=15.0 min

Area Total	Imp(%)	Curve Number (CN)	Di r. Conn. (%)
17.58	75.00	70.00	
Imp (mm)= 5.00			
U. H. Tp(hrs)= 1.63			

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	13.18	4.39
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	342.34	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	71.81	18.04
over (min)	15.00	30.00
Storage Coeff. (min)=	7.51 (ii)	28.74 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.10	0.04

		TOTALS
PEAK FLOW (cms)=	2.23	0.15
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	52.40	16.28
TOTAL RAINFALL (mm)=	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.30
		0.76

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 66.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5264)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.1760	0.5589
0.0600	0.3133	1.4770	0.6054
0.3410	0.3763	1.7190	0.6511
0.6340	0.4336	2.1190	0.6611
0.8800	0.4941	0.0000	0.0000

INFLOW : ID= 2 (2641) AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
OUTFLOW: ID= 1 (5264) 17.580 2.323 6.00 41.56
17.580 0.563 6.50 41.49

PEAK FLOW REDUCTION [Qout/Qin] (%) = 24.26
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha.m.) = 0.4233

CALIB NASHYD (2642)
ID= 1 DT=15.0 min

Area (ha) = 336.37 Curve Number (CN) = 69.0
Ia (mm) = 5.00 # of Linear Res. (N) = 1.30
U.H. Tp(hrs) = 1.28

Unit Hyd Qpeak (cms) = 3.192

PEAK FLOW (cms) = 1.419 (i)
TIME TO PEAK (hrs) = 8.250
RUNOFF VOLUME (mm) = 14.803
TOTAL RAINFALL (mm) = 54.400
RUNOFF COEFFICIENT = 0.272

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9264)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (2642):	336.37	1.419	8.25	14.80
+ ID2= 2 (5264):	17.58	0.563	6.50	41.49

ID = 3 (9264):	353.95	1.707	6.75	16.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8380)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0268):	508.09	1.507	9.50	12.62
+ ID2= 2 (9264):	353.95	1.707	6.75	16.13

ID = 3 (8380):	862.04	3.076	8.00	14.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6268)
IN= 2--> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (2681.0) ----->

Distance	Elevation	Manning
0.00	274.03	0.0500
10.55	272.73	0.0500
21.10	271.28	0.0500
50.12	266.83	0.0500
87.04	260.36	0.0500
92.32	260.06	0.0500
94.95	259.93	0.0500

101.00	256.87	0.0500		
103.00	256.58	0.0500 / 0.0350	Main	Channel
104.00	256.32	0.0350	Main	Channel
106.00	256.05	0.0350	Main	Channel
108.00	256.25	0.0350	Main	Channel
110.78	256.65	0.0350 / 0.0500	Main	Channel
113.42	256.81	0.0500		
116.06	257.00	0.0500		
155.62	260.24	0.0500		
187.27	263.35	0.0500		
211.01	267.22	0.0500		
224.20	269.60	0.0500		
261.13	272.68	0.0500		

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.53	256.58	.697E+04	2.1	0.97	55.64
1.38	257.43	.477E+05	25.9	1.76	30.66
2.22	258.27	.123E+06	84.3	2.22	24.26
3.07	259.12	.231E+06	185.4	2.60	20.72
3.92	259.97	.371E+06	335.8	2.92	18.42
4.77	260.82	.560E+06	547.6	3.15	17.06
5.61	261.66	.790E+06	845.6	3.46	15.57
6.46	262.51	.106E+07	1225.0	3.75	14.37
7.31	263.36	.136E+07	1693.3	4.02	13.37
8.16	264.21	.169E+07	2283.2	4.35	12.36
9.00	265.05	.206E+07	2969.1	4.66	11.54
9.85	265.90	.245E+07	3754.4	4.96	10.86
10.70	266.75	.286E+07	4643.0	5.24	10.28
11.55	267.60	.331E+07	5636.1	5.50	9.78
12.39	268.44	.378E+07	6746.2	5.76	9.34
13.24	269.29	.428E+07	7972.2	6.01	8.96
14.09	270.14	.482E+07	9218.9	6.18	8.71
14.94	270.99	.539E+07	10550.5	6.32	8.52
15.78	271.83	.601E+07	12024.7	6.46	8.33

<----- hydrograph -----> <-- pi pe / channel -->
AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm) MAX DEPTH (m) MAX VEL (m/s)
INFLOW : ID= 2 (8380) 862.04 3.08 8.00 14.06 0.57 0.99
OUTFLOW: ID= 1 (6268) 862.04 2.95 9.75 14.06 0.56 0.98

ADD HYD (8382)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0268):	215.76	1.800	7.00	17.88
+ ID2= 2 (6268):	862.04	2.953	9.75	14.06

ID = 3 (8382):	1077.80	4.392	8.50	14.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6270)
IN= 2--> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (2701.0) ----->

Distance	Elevation	Manning
0.00	245.98	0.0500
8.14	245.66	0.0500
16.28	245.16	0.0500
20.35	244.84	0.0500
38.66	242.98	0.0500
48.83	240.65	0.0500
63.07	235.91	0.0500
65.11	235.18	0.0500
68.43	234.34	0.0500 / 0.0300
68.68	233.95	0.0300
69.18	233.89	0.0300
69.68	233.95	0.0300
71.21	234.48	0.0300 / 0.0500
81.38	236.44	0.0500
95.63	236.66	0.0500
121.04	237.00	0.0500
148.53	241.77	0.0500
158.70	242.34	0.0500
187.18	244.03	0.0500
201.42	244.36	0.0500

TRAVEL TIME TABLE						
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)	
0.45	234.34	.217E+04	0.7	0.99	52.83	
0.98	234.87	.966E+04	4.6	1.49	35.20	
1.50	235.39	.250E+05	13.7	1.73	30.41	
2.03	235.92	.475E+05	29.5	1.95	26.87	
2.56	236.45	.772E+05	52.5	2.14	24.50	
3.09	236.98	.142E+06	82.4	1.83	28.73	
3.61	237.50	.246E+06	158.1	2.03	25.91	
4.14	238.03	.357E+06	262.0	2.31	22.73	
4.67	238.56	.477E+06	392.1	2.59	20.27	
5.20	239.09	.604E+06	547.8	2.86	18.38	
5.72	239.61	.739E+06	729.2	3.11	16.90	
6.25	240.14	.882E+06	936.5	3.34	15.70	
6.78	240.67	.103E+07	1169.7	3.57	14.72	
7.31	241.20	.119E+07	1425.9	3.77	13.94	
7.83	241.72	.136E+07	1710.4	3.96	13.26	
8.36	242.25	.154E+07	1967.9	4.02	13.07	
8.89	242.78	.174E+07	2264.9	4.09	12.83	
9.42	243.31	.196E+07	2589.3	4.15	12.65	
9.94	243.83	.221E+07	2953.5	4.22	12.46	

<---- hydrograph ---->						
		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	<-pi pe / channel -> MAX DEPTH (m) MAX VEL (m/s)
INFLOW :	ID= 2 (8382)	1077.80	4.39	8.50	14.82	0.95 1.46
OUTFLOW:	ID= 1 (6270)	1077.80	4.35	9.00	14.82	0.94 1.45

ADD HYD (8384)						
1 + 2 = 3		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1=	1 (0270):	243.61	2.117	7.50	22.09	
+ ID2=	2 (6270):	1077.80	4.348	9.00	14.82	
ID = 3 (8384):		1321.41	6.275	8.50	16.16	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0262)						
ID=	1 DT=15.0 min	Area (ha)	(ha)=	Curve Number (CN)=	82.0	# of Linear Res. (N)= 1.30
		341.31				
		5.00				
		1.01				

Unit Hyd Opeak (cms)= 4.094

PEAK FLOW (cms)= 2.732 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 22.953
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.422

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0258)						
ID=	1 DT=15.0 min	Area (ha)	(ha)=	Curve Number (CN)=	79.0	# of Linear Res. (N)= 1.30
		181.99				
		5.00				
		1.18				

Unit Hyd Opeak (cms)= 1.881

PEAK FLOW (cms)= 1.165 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 20.683
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.380

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0256)						
ID=	1 DT=15.0 min	Area (ha)	(ha)=	Curve Number (CN)=	67.0	# of Linear Res. (N)= 1.30
		145.79				
		5.00				
		1.04				

Unit Hyd Opeak (cms)= 1.707

PEAK FLOW (cms)= 0.678 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 13.836
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.254

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0254)						
ID=	1 DT=15.0 min	Area (ha)	(ha)=	Curve Number (CN)=	55.0	# of Linear Res. (N)= 1.30
		403.00				
		5.00				
		1.22				

Unit Hyd Opeak (cms)= 4.028

PEAK FLOW (cms)= 1.113 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 9.405
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.173

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6256)						
IN= 2--> OUT= 1		Routing time step (min)'= 15.00				

<----- DATA FOR SECTION (2561.0) ----->						
Distance	Elevation	Manning				
0.00	276.07	0.0400				
11.68	273.71	0.0400				
23.36	271.35	0.0400				
29.19	270.30	0.0400				
35.03	269.44	0.0400				
55.47	267.90	0.0400				
78.82	266.24	0.0400				
90.50	265.63	0.0400				
102.18	265.40	0.0400				
105.10	264.95	0.0400 / 0.0350	Main Channel			
108.02	264.39	0.0350	Main Channel			
110.94	264.72	0.0350	Main Channel			
113.86	265.19	0.0350 / 0.0400	Main Channel			
116.78	265.49	0.0400				
143.05	268.24	0.0400				
172.25	270.53	0.0400				
207.28	271.95	0.0400				
233.55	273.72	0.0400				
256.91	274.98	0.0400				
289.02	275.97	0.0400				

TRAVEL TIME TABLE						
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)	
0.56	264.95	.723E+04	1.9	0.90	62.51	
1.14	265.53	.307E+05	13.5	1.49	37.92	
1.72	266.11	.951E+05	48.6	1.73	32.62	
2.30	266.69	.191E+06	117.8	2.09	27.03	
2.88	267.27	.314E+06	225.2	2.43	23.24	
3.46	267.85	.464E+06	375.5	2.74	20.59	
4.04	268.43	.641E+06	573.2	3.03	18.62	
4.62	269.01	.846E+06	821.9	3.29	17.15	
5.20	269.59	.108E+07	1135.0	3.56	15.87	
5.78	270.17	.134E+07	1525.1	3.86	14.64	
6.36	270.75	.162E+07	1971.1	4.12	13.71	
6.94	271.33	.194E+07	2474.0	4.33	13.04	
7.52	271.91	.228E+07	3065.0	4.55	12.42	
8.10	272.49	.266E+07	3773.4	4.80	11.75	
8.68	273.07	.306E+07	4566.3	5.05	11.17	
9.26	273.65	.348E+07	5443.7	5.30	10.66	
9.84	274.23	.393E+07	6389.6	5.51	10.25	
10.42	274.81	.440E+07	7426.2	5.72	9.88	
11.00	275.39	.490E+07	8490.1	5.86	9.63	

<---- hydrograph ---->						
		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	<-pi pe / channel -> MAX DEPTH (m) MAX VEL (m/s)
INFLOW :	ID= 2 (0254)	403.00	1.11	8.25	9.41	0.32 0.90
OUTFLOW:	ID= 1 (6256)	403.00	1.05	10.00	9.41	0.31 0.90

ADD HYD (8370)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0256):	145.79	0.678	8.00	13.84
+ ID2= 2 (6256):	403.00	1.052	10.00	9.41
ID = 3 (8370):	548.79	1.667	9.25	10.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0250)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	192.88	70.0
	U. H. Tp (hrs) = 5.00	# of Linear Res. (N) = 1.30
		U. H. Tp (hrs) = 1.22

Unit Hyd Opeak (cms) = 1.930

PEAK FLOW (cms) = 0.878 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 15.287
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.281

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0246)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	759.61	55.0
	U. H. Tp (hrs) = 5.00	# of Linear Res. (N) = 1.30
		U. H. Tp (hrs) = 1.81

Unit Hyd Opeak (cms) = 5.121

PEAK FLOW (cms) = 1.566 (i)
 TIME TO PEAK (hrs) = 10.000
 RUNOFF VOLUME (mm) = 9.438
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.173

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0248)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	146.04	64.0
	U. H. Tp (hrs) = 5.00	# of Linear Res. (N) = 1.30
		U. H. Tp (hrs) = 0.78

Unit Hyd Opeak (cms) = 2.271

PEAK FLOW (cms) = 0.754 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 12.498
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.230

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8364)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0246):	759.61	1.566	10.00	9.44
+ ID2= 2 (0248):	146.04	0.754	7.25	12.50
ID = 3 (8364):	905.65	2.217	8.25	9.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6250)	Routing time step (min)
IN= 2--> OUT= 1	15.00

<----- DATA FOR SECTION (2501.0) ----->

Distance	Elevation	Manning
0.00	269.96	0.0500
8.56	268.55	0.0500
17.13	266.91	0.0500
21.41	266.13	0.0500
40.68	263.15	0.0500
62.09	260.75	0.0500
85.64	258.02	0.0500
88.20	257.69	0.0500
93.20	257.05	0.0500 / 0.0350
93.45	256.88	0.0350
94.20	256.56	0.0350
94.95	256.83	0.0350
95.20	257.08	0.0350 / 0.0500
100.62	257.45	0.0500
115.61	258.57	0.0500
139.16	260.43	0.0500
152.01	261.95	0.0500
171.27	264.63	0.0500
188.40	267.90	0.0500
211.95	274.18	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.49	257.05	205E+04	0.4	0.68	89.65
1.17	257.73	248E+05	5.7	0.85	72.16
1.85	258.41	841E+05	25.2	1.10	55.60
2.53	259.09	180E+06	66.5	1.36	45.14
3.21	259.77	312E+06	135.7	1.60	38.36
3.89	260.45	481E+06	238.3	1.82	33.61
4.57	261.13	681E+06	383.5	2.07	29.60
5.25	261.81	911E+06	568.9	2.29	26.70
5.93	262.49	117E+07	800.5	2.51	24.36
6.61	263.17	146E+07	1079.0	2.72	22.49
7.28	263.84	177E+07	1413.4	2.94	20.85
7.96	264.52	210E+07	1797.0	3.14	19.50
8.64	265.20	246E+07	2241.6	3.35	18.28
9.32	265.88	283E+07	2739.9	3.55	17.25
10.00	266.56	323E+07	3295.7	3.75	16.34
10.68	267.24	364E+07	3908.9	3.94	15.54
11.36	267.92	408E+07	4578.5	4.12	14.84
12.04	268.60	453E+07	5320.1	4.32	14.18
12.72	269.28	499E+07	6109.1	4.50	13.61

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8364)	905.65	2.22	8.25	9.93	0.72	0.73
OUTFLOW: ID= 1 (6250)	905.65	2.06	10.50	9.93	0.70	0.73

ADD HYD (8366)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0250):	192.88	0.878	8.00	15.29
+ ID2= 2 (6250):	905.65	2.056	10.50	9.93
ID = 3 (8366):	1098.53	2.855	10.00	10.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8368)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (8366):	1098.53	2.855	10.00	10.87
+ ID2= 2 (8370):	548.79	1.667	9.25	10.58
ID = 3 (8368):	1647.32	4.503	10.00	10.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2521)	Area (ha)	Total Imp (%)	Di r. Conn. (%)
ID= 1 DT=15.0 min	32.51	75.00	70.00

 IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 24.38 8.13
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 465.55 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 71.81 20.43
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 9.04 (ii) 29.23 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.04

TOTALS
 PEAK FLOW (cms)= 3.95 0.31 4.138 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 52.40 18.24 42.15
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.34 0.77

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 70.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5252)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.1750	1.0334
0.1100	0.5793	2.7310	1.1195
0.6300	0.6958	3.1790	1.2039
1.1730	0.8017	3.5790	1.2139
1.6270	0.9137	0.0000	0.0000

INFLOW : ID= 2 (2521)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
32.510	4.138	6.00	42.15	42.15
OUTFLOW: ID= 1 (5252)	32.510	1.061	6.50	42.11

PEAK FLOW REDUCTION [Qout/Qin] (%) = 25.64
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 0.7835

CALIB NASHYD (2522)
 ID= 1 DT=15.0 min

Area (ha)= 287.47 Curve Number (CN)= 71.5
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.30
 U.H. Tp(hrs)= 1.01

Unit Hyd Opeak (cms)= 3.448
 PEAK FLOW (cms)= 1.583 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 16.022
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.295

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9252)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2522):	287.47	1.583	8.00	16.02
+ ID2= 2 (5252):	32.51	1.061	6.50	42.11
-----	-----	-----	-----	-----
ID = 3 (9252):	319.98	2.417	6.50	18.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8372)
 1 + 2 = 3

AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)

 (ha) (cms) (hrs) (mm)
 ID1= 1 (8368): 1647.32 4.503 10.00 10.78
 + ID2= 2 (9252): 319.98 2.417 6.50 18.67

 ID = 3 (8372): 1967.30 6.123 9.00 12.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6258)
 IN= 2--> OUT= 1

Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2581.0) ----->

Distance	Elevation	Manning
0.00	252.88	0.0500
15.47	251.89	0.0500
46.41	248.45	0.0500
73.48	245.81	0.0500
88.95	243.75	0.0500
112.15	242.00	0.0500
135.35	240.23	0.0500
162.42	239.76	0.0500
170.97	239.52	0.0500 / 0.0350
171.58	239.03	0.0350
174.02	239.03	0.0350
176.46	239.03	0.0350
177.07	239.52	0.0350 / 0.0500
185.63	239.67	0.0500
208.83	239.87	0.0500
239.77	240.14	0.0500
274.57	244.93	0.0500
336.45	249.51	0.0500
363.52	249.77	0.0500
382.85	251.78	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.49	239.52	.135E+05	2.5	0.92	91.32
1.14	240.17	.175E+06	25.6	0.73	114.22
1.78	240.81	.532E+06	123.5	1.17	71.85
2.43	241.46	.932E+06	285.3	1.54	54.48
3.07	242.10	.138E+07	506.5	1.85	45.25
3.72	242.75	.186E+07	786.8	2.12	39.42
4.36	243.39	.239E+07	1127.3	2.37	35.33
5.01	244.04	.296E+07	1535.8	2.61	32.12
5.65	244.68	.356E+07	2014.3	2.84	29.48
6.30	245.33	.420E+07	2535.9	3.03	27.60
6.94	245.97	.488E+07	3108.4	3.20	26.16
7.59	246.62	.561E+07	3740.6	3.35	24.98
8.23	247.26	.638E+07	4446.7	3.50	23.93
8.88	247.91	.721E+07	5229.7	3.64	22.98
9.52	248.55	.809E+07	6094.3	3.78	22.12
10.17	249.20	.901E+07	7050.4	3.93	21.31
10.81	249.84	.100E+08	7773.0	3.90	21.46
11.46	250.49	.111E+08	8998.8	4.07	20.56
12.10	251.13	.122E+08	10318.2	4.24	19.75

INFLOW : ID= 2 (8372)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
1967.30	6.12	9.00	12.06	0.59	0.88	0.88
OUTFLOW: ID= 1 (6258)	1967.30	5.63	11.00	12.06	0.58	0.89

ADD HYD (8374)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0258):	181.99	1.165	8.00	20.68
+ ID2= 2 (6258):	1967.30	5.632	11.00	12.06
-----	-----	-----	-----	-----
ID = 3 (8374):	2149.29	6.595	10.75	12.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2601)
 ID= 1 DT=15.0 min

Area (ha)= 56.22
 Total Imp(%)= 65.00 Di r. Conn. (%)= 48.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	36.54	19.68	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	612.21	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	71.81	67.36	
over (min)	15.00	30.00	
Storage Coeff. (min)=	10.65 (ii)	23.18 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.09	0.04	
TOTALS			
PEAK FLOW (cms)=	4.46	1.75	5.594 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	52.40	28.99	40.23
TOTAL RAINFALL (mm)=	54.40	54.40	54.40
RUNOFF COEFFICIENT =	0.96	0.53	0.74

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 81.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5260)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2---> OUT= 1	0.0000	0.0000	3.7610	1.6799
DT= 15.0 min	0.1900	0.8974	4.7220	1.8499
	1.0890	1.0678	5.4970	2.0146
	2.0290	1.2667	5.8970	2.0246
	2.8140	1.4621	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2601)	56.220	5.594	6.00	40.23
OUTFLOW: ID= 1 (5260)	56.220	1.696	6.75	40.21
PEAK FLOW REDUCTION [Qout/Qin] (%)=	30.32			
TIME SHIFT OF PEAK FLOW (min)=	45.00			
MAXIMUM STORAGE USED (ha.m.)=	1.2129			

CALIB NASHYD (2602)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	420.02	83.3
	Ia (mm)= 5.00	# of Linear Res. (N)= 1.30
	U.H. Tp (hrs)= 1.30	

Unit Hyd Opeak (cms)=	3.946
PEAK FLOW (cms)=	2.923 (i)
TIME TO PEAK (hrs)=	8.000
RUNOFF VOLUME (mm)=	24.131
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.444

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9260)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (2602):	420.02	2.923	8.00	24.13
+ ID2= 2 (5260):	56.22	1.696	6.75	40.21
ID = 3 (9260):	476.24	4.252	6.75	26.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8376)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3				

	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8374):	2149.29	6.595	10.75	12.79
+ ID2= 2 (9260):	476.24	4.252	6.75	26.03
ID = 3 (8376):	2625.53	9.506	10.00	15.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8378)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0262):	341.31	2.732	8.00	22.95
+ ID2= 2 (8376):	2625.53	9.506	10.00	15.19
ID = 3 (8378):	2966.84	11.844	9.50	16.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8362)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (8378):	2966.84	11.844	9.50	16.08
+ ID2= 2 (8384):	1321.41	6.275	8.50	16.16
ID = 3 (8362):	4288.25	18.011	9.00	16.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6272)	Routing time step (min)
IN= 2---> OUT= 1	15.00

Distance	Elevation	Manning
0.00	231.01	0.0450
23.01	223.65	0.0450
34.51	222.46	0.0450
51.77	222.11	0.0450
69.02	221.87	0.0450
161.06	221.92	0.0450
166.81	221.91	0.0450
172.56	221.89	0.0450
180.57	221.40	0.0450 / 0.0300
181.57	220.60	0.0300
184.07	220.16	0.0300
195.57	221.85	0.0300 / 0.0450
201.32	221.82	0.0450
207.07	221.77	0.0450
212.83	221.72	0.0450
253.09	222.52	0.0450
408.40	222.65	0.0450
460.17	223.20	0.0450
517.69	224.84	0.0450
569.46	232.57	0.0450

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.41	220.57	.189E+04	0.5	0.49	60.18
0.83	220.99	.697E+04	3.2	0.82	35.79
1.24	221.40	.145E+05	8.8	1.07	27.45
1.84	222.00	.643E+05	29.3	0.81	36.61
2.44	222.60	.282E+06	132.9	0.83	35.41
3.04	223.20	.707E+06	413.7	1.04	28.50
3.64	223.80	.118E+07	876.8	1.31	22.46
4.24	224.40	.168E+07	1494.8	1.57	18.75
4.84	225.00	.221E+07	2264.1	1.82	16.23
5.44	225.60	.274E+07	3196.3	2.06	14.29
6.04	226.20	.328E+07	4258.6	2.30	12.84
6.65	226.81	.383E+07	5444.7	2.52	11.72
7.25	227.41	.438E+07	6749.7	2.73	10.82
7.85	228.01	.494E+07	8169.4	2.93	10.08
8.45	228.61	.551E+07	9700.7	3.12	9.46
9.05	229.21	.608E+07	11340.8	3.30	8.94
9.65	229.81	.666E+07	13087.4	3.48	8.48
10.25	230.41	.724E+07	14938.5	3.65	8.08
10.85	231.01	.783E+07	16892.4	3.82	7.73

<---- hydrograph ----> <--pi pe / channel -->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8362)	4288.25	18.01	9.00	16.11	1.51	0.93
OUTFLOW: ID= 1 (6272)	4288.25	17.76	10.00	16.11	1.50	0.94

CALIB STANDHYD (2721)
ID= 1 DT=15.0 min

Area (ha)=	2.21
Total Imp(%)=	49.00
Di r. Conn.(%)=	19.00

	IMPERVIOUS (ha)	PERVIOUS (i) (i)
Surface Area	1.08	1.13
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	121.38	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	71.81	58.51
over (min)	15.00	30.00
Storage Coeff. (min)=	4.03 (ii)	17.29 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.11	0.05

PEAK FLOW (cms)=	0.08	0.10	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	0.146 (iii)
RUNOFF VOLUME (mm)=	52.40	23.86	6.00
TOTAL RAINFALL (mm)=	54.40	54.40	29.27
RUNOFF COEFFICIENT =	0.96	0.44	54.40
			0.54

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5272)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.1480	0.0589
0.0070	0.0284	0.1850	0.0669
0.0430	0.0331	0.2160	0.0746
0.0800	0.0419	0.6160	0.0846
0.1100	0.0497	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2721)	2.210	0.146	6.00	29.27
OUTFLOW: ID= 1 (5272)	2.210	0.048	6.75	28.82

PEAK FLOW REDUCTION [Qout/Qin] (%)=	33.22
TIME SHIFT OF PEAK FLOW (min)=	45.00
MAXIMUM STORAGE USED (ha. m.)=	0.0346

CALIB NASHYD (2722)
ID= 1 DT=15.0 min

Area (ha)=	155.17	Curve Number (CN)=	75.3
Ia (mm)=	5.00	# of Linear Res. (N)=	1.50
U. H. Tp(hrs)=	1.09		

Unit Hyd Qpeak (cms)=	2.433
PEAK FLOW (cms)=	1.236 (i)
TIME TO PEAK (hrs)=	7.500
RUNOFF VOLUME (mm)=	18.222
TOTAL RAINFALL (mm)=	54.400
RUNOFF COEFFICIENT =	0.335

- (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9272)
1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2722):	155.17	1.236	7.50	18.22
+ ID2= 2 (5272):	2.21	0.048	6.75	28.82
ID = 3 (9272):	157.38	1.265	7.25	18.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8356)
1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6272):	4288.25	17.765	10.00	16.11
+ ID2= 2 (9272):	157.38	1.265	7.25	18.37
ID = 3 (8356):	4445.63	18.744	9.50	16.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8352)
1 + 2 = 3

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8354):	1268.73	10.681	8.00	21.29
+ ID2= 2 (8356):	4445.63	18.744	9.50	16.19
ID = 3 (8352):	5714.36	28.625	8.75	17.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6304)
IN= 2--> OUT= 1 Routing time step (min)= 15.00

<----- DATA FOR SECTION (3041.0) ----->

Di stance	Elevation	Manning	
0.00	232.08	0.0500	
19.00	231.87	0.0500	
38.00	231.33	0.0500	
66.51	230.44	0.0500	
104.51	228.25	0.0500	
118.76	225.17	0.0500	
128.26	219.86	0.0500	
175.77	219.17	0.0500	
185.27	218.90	0.0500 / 0.0300	Main Channel
185.52	218.65	0.0300	Main Channel
190.02	218.37	0.0300	Main Channel
194.52	218.60	0.0300	Main Channel
194.77	218.85	0.0300 / 0.0500	Main Channel
204.27	219.60	0.0500	
299.28	220.91	0.0500	
327.78	222.36	0.0500	
375.28	225.71	0.0500	
403.79	229.37	0.0500	
432.29	230.43	0.0500	
470.29	232.00	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.48	218.85	.121E+05	1.2	0.36	173.94
1.17	219.54	.839E+05	9.8	0.44	142.60
1.86	220.23	.323E+06	41.0	0.47	131.43
2.56	220.93	.708E+06	109.5	0.58	107.67
3.25	221.62	.118E+07	224.3	0.71	87.36
3.94	222.31	.168E+07	376.5	0.84	74.48
4.63	223.00	.222E+07	567.9	0.96	65.26
5.32	223.69	.279E+07	794.7	1.07	58.58
6.02	224.39	.339E+07	1056.2	1.17	53.52
6.71	225.08	.402E+07	1352.4	1.26	49.52
7.40	225.77	.468E+07	1677.3	1.34	46.46
8.09	226.46	.536E+07	2050.0	1.43	43.58
8.79	227.16	.607E+07	2457.9	1.52	41.14
9.48	227.85	.680E+07	2901.0	1.60	39.05
10.17	228.54	.755E+07	3352.7	1.66	37.53
10.86	229.23	.834E+07	3812.5	1.71	36.48
11.55	229.92	.919E+07	4241.8	1.73	36.13

12.25 230.62 .101E+08 4697.0 1.74 35.93
 12.94 231.31 .111E+08 5186.3 1.74 35.83

<---- hydrograph ----> <--pi pe / channel -->
 AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8352) 5714.36 28.63 8.75 17.32 1.59 0.46
 OUTFLOW: ID= 1 (6304) 5714.36 23.30 11.50 17.32 1.47 0.45

ADD HYD (8350)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0304): 292.37 0.753 10.50 12.26
 + ID2= 2 (6304): 5714.36 23.300 11.50 17.32
 ID= 3 (8350): 6006.73 24.043 11.50 17.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8346)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8348): 3768.46 16.007 10.00 15.42
 + ID2= 2 (8350): 6006.73 24.043 11.50 17.08
 ID= 3 (8346): 9775.19 39.262 10.75 16.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8344)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0306): 283.97 0.250 13.75 8.59
 + ID2= 2 (8346): 9775.19 39.262 10.75 16.44
 ID= 3 (8344): 10059.16 39.486 10.75 16.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5509)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	*****	370.0451
41.0590	56.7403	*****	863.4386
48.1390	86.3439	*****	*****
56.6340	*****	*****	*****
67.9600	*****	0.0000	0.0000

AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (8344) ***** 39.486 10.75 16.22
 OUTFLOW: ID= 1 (5509) ***** 30.704 14.50 16.22

PEAK FLOW REDUCTION [Qout/Qin](%) = 77.76
 TIME SHIFT OF PEAK FLOW (min) = 225.00
 MAXIMUM STORAGE USED (ha.m.) = 42.4484

CALIB NASHYD (0332)
 ID= 1 DT=15.0 min

Area (ha) = 393.44 Curve Number (CN) = 75.0
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 2.32

Unit Hyd Opeak (cms) = 2.894
 PEAK FLOW (cms) = 1.734 (i)
 TIME TO PEAK (hrs) = 10.000
 RUNOFF VOLUME (mm) = 18.150
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.334

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0330)
 ID= 1 DT=15.0 min

Area (ha) = 468.30 Curve Number (CN) = 80.0
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.09

Unit Hyd Opeak (cms) = 7.335
 PEAK FLOW (cms) = 4.424 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 21.421
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.394

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6332)
 IN= 2--> OUT= 1

Routing time step (min) = 15.00

DATA FOR SECTION (3321.0) ----->

Distance	Elevation	Manning	Main Channel
0.00	234.00	0.0380	
25.85	227.20	0.0380	
96.94	226.44	0.0380	
168.03	227.38	0.0380	
219.73	225.62	0.0380	
342.52	221.57	0.0380	
368.37	221.42	0.0380	
374.83	221.23	0.0380	
379.79	220.98	0.0380 / 0.0300	Main Channel
380.29	220.47	0.0300	Main Channel
381.29	220.47	0.0300	Main Channel
382.79	220.47	0.0300	Main Channel
383.29	220.98	0.0300 / 0.0380	Main Channel
394.22	221.22	0.0380	
400.68	221.33	0.0380	
407.14	221.44	0.0380	
491.16	225.70	0.0380	
568.71	227.55	0.0380	
607.49	230.14	0.0380	
639.80	234.08	0.0380	

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.51	220.98	.554E+04	0.9	0.56	107.27
1.20	221.67	.885E+05	12.1	0.49	122.32
1.88	222.35	.309E+06	64.7	0.76	79.70
2.57	223.04	.615E+06	165.4	0.97	61.99
3.25	223.72	.101E+07	322.1	1.16	52.06
3.94	224.41	.148E+07	542.9	1.33	45.51
4.62	225.09	.204E+07	835.4	1.48	40.77
5.31	225.78	.269E+07	1202.8	1.62	37.27
5.99	226.46	.344E+07	1618.0	1.70	35.46
6.68	227.15	.447E+07	1932.0	1.56	38.56
7.36	227.83	.580E+07	2763.9	1.72	34.96
8.05	228.52	.718E+07	3861.9	1.95	30.97
8.73	229.20	.859E+07	5115.4	2.16	27.98
9.42	229.89	.100E+08	6519.2	2.35	25.64
10.10	230.57	.115E+08	8092.3	2.55	23.69
10.79	231.26	.130E+08	9821.5	2.73	22.05
11.47	231.94	.145E+08	11690.0	2.92	20.68
12.16	232.63	.160E+08	13694.9	3.09	19.52
12.84	233.31	.176E+08	15833.9	3.26	18.52

<---- hydrograph ----> <--pi pe / channel -->
 AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0330) 468.30 4.42 7.50 21.42 0.73 0.54
 OUTFLOW: ID= 1 (6332) 468.30 3.43 9.75 21.42 0.67 0.54

ADD HYD (8322)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)

ID1= 1 (0332): 393.44 1.734 10.00 18.15
 + ID2= 2 (6332): 468.30 3.427 9.75 21.42

 ID = 3 (8322): 861.74 5.160 9.75 19.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0328) Area (ha) = 492.92 Curve Number (CN) = 77.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.91

Unit Hyd Qpeak (cms) = 4.411

PEAK FLOW (cms) = 2.703 (i)
 TIME TO PEAK (hrs) = 8.750
 RUNOFF VOLUME (mm) = 19.405
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.357

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0326) Area (ha) = 678.91 Curve Number (CN) = 80.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.95

Unit Hyd Qpeak (cms) = 5.941

PEAK FLOW (cms) = 4.073 (i)
 TIME TO PEAK (hrs) = 9.000
 RUNOFF VOLUME (mm) = 21.534
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.396

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (3181) Area (ha) = 357.30 Dir. Conn. (%) = 32.00
 ID= 1 DT=15.0 min Total Imp(%) = 56.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	200.09	157.21
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1543.37	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 71.81 over (min) = 15.00
 Storage Coeff. (min) = 18.55 (ii) 32.07 (ii)
 Unit Hyd. Tpeak (min) = 15.00
 Unit Hyd. peak (cms) = 0.06 0.03

TOTALS
 18.910 (iii)

PEAK FLOW (cms) = 15.08 8.92
 TIME TO PEAK (hrs) = 6.00 6.50
 RUNOFF VOLUME (mm) = 52.40 32.62
 TOTAL RAINFALL (mm) = 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.43 0.60

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5318)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1				
DT= 15.0 min				
	0.0000	0.0000	22.8580	8.7280
	0.8190	2.5190	27.5380	9.6853
	4.2860	5.8947	32.5800	10.8345
	11.0600	6.7336	32.9800	10.8445
	16.3830	7.4416	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (3181)	357.300	18.910	6.00	32.62
OUTFLOW: ID= 1 (5318)	357.300	6.323	7.25	32.62

PEAK FLOW REDUCTION [Qout/Qin] (%) = 33.44
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha.m.) = 6.1801

CALIB
 STANDHYD (3182) Area (ha) = 126.13 Dir. Conn. (%) = 12.00
 ID= 1 DT=15.0 min Total Imp(%) = 21.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	26.49	99.64
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	916.99	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 71.81 over (min) = 15.00
 Storage Coeff. (min) = 13.57 (ii) 34.24 (ii)
 Unit Hyd. Tpeak (min) = 15.00
 Unit Hyd. peak (cms) = 0.08 0.03

TOTALS
 3.702 (iii)

PEAK FLOW (cms) = 2.29 3.08
 TIME TO PEAK (hrs) = 6.00 6.50
 RUNOFF VOLUME (mm) = 52.40 18.50
 TOTAL RAINFALL (mm) = 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.34 0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ADD HYD (8334)				
1 + 2 = 3				
ID1= 1 (3182):	126.13	3.702	6.50	22.57
+ ID2= 2 (5318):	357.30	6.323	7.25	32.62

ID = 3 (8334):	483.43	8.226	7.00	30.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6320)
 IN= 2--> OUT= 1 Routing time step (min) = 15.00

Distance	Elevation	Manning
0.00	249.00	0.0380
22.92	245.86	0.0380
45.83	244.87	0.0380
91.66	243.11	0.0380
126.03	239.53	0.0380
160.41	237.17	0.0380
166.14	237.06	0.0380
177.59	237.13	0.0380
183.32	237.20	0.0380
189.05	236.70	0.0380 / 0.0350
193.05	235.89	0.0350
197.05	236.64	0.0350 / 0.0380
200.51	236.74	0.0380
206.24	237.03	0.0380
246.34	238.82	0.0380
263.53	243.87	0.0380
389.56	247.64	0.0380
452.58	247.74	0.0380
498.41	248.60	0.0380

567.16 249.84 0.0380

TRAVEL TIME TABLE					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.37	236.26	.270E+04	0.4	0.58	107.96
0.75	236.64	.108E+05	2.6	0.92	68.01
1.48	237.37	.877E+05	27.4	1.17	53.35
2.20	238.09	.277E+06	122.1	1.65	37.83
2.93	238.82	.540E+06	299.7	2.08	30.01
3.66	239.55	.857E+06	587.8	2.57	24.29
4.39	240.28	.120E+07	977.8	3.04	20.52
5.11	241.00	.158E+07	1458.1	3.46	18.02
5.84	241.73	.198E+07	2028.6	3.84	16.23
6.57	242.46	.240E+07	2690.5	4.19	14.86
7.29	243.18	.285E+07	3431.8	4.50	13.84
8.02	243.91	.335E+07	4149.1	4.64	13.44
8.75	244.64	.393E+07	4746.6	4.51	13.80
9.47	245.36	.463E+07	5618.6	4.53	13.75
10.20	246.09	.545E+07	6781.8	4.66	13.39
10.93	246.82	.635E+07	8274.5	4.87	12.79
11.66	247.55	.733E+07	9988.0	5.10	12.23
12.38	248.27	.853E+07	11422.0	5.01	12.45
13.11	249.00	.989E+07	13749.7	5.20	11.98

hydrograph						pe / channel	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)		
INFLOW: ID= 2 (8334)	483.43	8.23	7.00	30.00	0.91	0.96	
OUTFLOW: ID= 1 (6320)	483.43	5.89	7.75	30.00	0.85	0.94	

CALIB NASHYD (3202) ID= 1 DT=15.0 min	Area (ha)= 84.56 Ia (mm)= 5.00 U.H. Tp(hrs)= 0.70	Curve Number (CN)= 80.9 # of Linear Res. (N)= 1.50
--	---	---

Unit Hyd Opeak (cms)= 2.054
 PEAK FLOW (cms)= 1.167 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 21.930
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.403

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3201) ID= 1 DT=15.0 min	Area (ha)= 194.18 Total Imp(%)= 45.00	Dir. Conn. (%)= 16.00
--	--	-----------------------

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	87.38	106.80
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1137.78	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 71.81 over (min)= 15.00
 Storage Coeff. (min)= 15.45 (ii)
 Unit Hyd. Tpeak (min)= 15.00
 Unit Hyd. peak (cms)= 0.07

TOTALS
 PEAK FLOW (cms)= 4.46 8.12 6.25 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.25
 RUNOFF VOLUME (mm)= 52.40 26.81 30.91
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.49 0.57

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 78.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5320)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	7.4940	4.4477
0.2490	1.8139	9.8910	4.7762
1.2830	3.1517	12.4550	5.0763
3.7490	3.7610	12.8550	5.0863
5.0040	4.0198	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (3201)	194.180	10.338	6.25	30.91
OUTFLOW: ID= 1 (5320)	194.180	2.752	7.25	30.90

PEAK FLOW REDUCTION [Qout/Qin] (%) = 26.63
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 3.5174

ADD HYD (9320)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (3202):	84.56	1.167	6.75	21.93
+ ID2= 2 (5320):	194.18	2.752	7.25	30.90
ID = 3 (9320):	278.74	3.873	7.25	28.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8332)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6320):	483.43	5.886	7.75	30.00
+ ID2= 2 (9320):	278.74	3.873	7.25	28.18
ID = 3 (8332):	762.17	9.528	7.50	29.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (3142)
 ID= 1 DT=15.0 min

Area (ha)= 122.75
 Ia (mm)= 5.00
 U.H. Tp(hrs)= 0.53
 Curve Number (CN)= 78.0
 # of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 3.924
 PEAK FLOW (cms)= 1.888 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 19.642
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.361

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3141)
 ID= 1 DT=15.0 min

Area (ha)= 42.46
 Total Imp(%)= 70.00
 Dir. Conn. (%)= 46.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	29.72	12.74
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	532.04	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 71.81 over (min)= 15.00
 Storage Coeff. (min)= 9.79 (ii)
 Unit Hyd. Tpeak (min)= 15.00
 Unit Hyd. peak (cms)= 0.09

TOTALS
 PEAK FLOW (cms)= 3.31 1.22 4.098 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00

RUNOFF VOLUME (mm)= 52.40 25.60 37.93
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.47 0.70

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5314)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.8400	1.3064
0.1440	0.7144	3.5660	1.4269
0.8230	0.8540	4.1520	1.5451
1.5320	0.9987	4.5520	1.5551
2.1250	1.1456	0.0000	0.0000

INFLOW : ID= 2 (3141) AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 42.460 4.098 6.00 37.93
 OUTFLOW: ID= 1 (5314) 42.460 1.076 6.75 37.90

PEAK FLOW REDUCTION [Oout/Oi n](%) = 26.25
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 0.9133

ADD HYD (9314)
 1 + 2 = 3

ID	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (3142):	122.75	1.888	6.50	19.64
+ ID2= 2 (5314):	42.46	1.076	6.75	37.90
ID = 3 (9314):	165.21	2.926	6.75	24.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6316)
 IN= 2---> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3161.0) ----->

Distance	Elevation	Manning	
0.00	248.54	0.0350	
27.93	248.34	0.0350	
41.89	246.61	0.0350	
62.83	243.09	0.0350	
132.65	239.00	0.0350	
188.50	236.74	0.0350	
195.48	236.54	0.0350	
202.46	236.32	0.0350	
205.48	236.14	0.0350 /0.0310	Main Channel
205.98	235.61	0.0310	Main Channel
207.98	235.25	0.0310	Main Channel
209.98	235.53	0.0310	Main Channel
210.48	236.00	0.0310 /0.0350	Main Channel
216.42	236.73	0.0350	
258.31	239.09	0.0350	
328.12	239.84	0.0350	
439.83	241.47	0.0350	
530.58	242.08	0.0350	
586.43	242.93	0.0350	
691.16	248.00	0.0350	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.37	235.62	.215E+04	0.7	0.77	53.88
0.75	236.00	.634E+04	3.5	1.36	30.45
1.46	236.71	.297E+05	19.3	1.62	25.66
2.16	237.41	.103E+06	75.8	1.84	22.60
2.87	238.12	.229E+06	202.8	2.21	18.79
3.57	238.82	.407E+06	423.3	2.59	16.03
4.28	239.53	.654E+06	727.7	2.77	14.97

4.99	240.24	.103E+07	1242.0	3.01	13.76
5.69	240.94	.151E+07	2002.3	3.31	12.54
6.40	241.65	.210E+07	2987.9	3.55	11.70
7.10	242.35	.286E+07	4302.8	3.75	11.07
7.81	243.06	.374E+07	6165.3	4.10	10.11
8.51	243.76	.468E+07	8642.4	4.60	9.03
9.22	244.47	.565E+07	11496.3	5.06	8.20
9.93	245.18	.666E+07	14714.1	5.50	7.54
10.63	245.88	.770E+07	18290.3	5.91	7.02
11.34	246.59	.877E+07	22221.9	6.30	6.58
12.04	247.29	.988E+07	26464.4	6.67	6.22
12.75	248.00	.110E+08	31061.3	7.01	5.91

<---- hydrograph ----> <-pi pe / channel ->

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (9314)	165.21	2.93	6.75	24.33	0.68	1.18
OUTFLOW: ID= 1 (6316)	165.21	2.39	7.25	24.33	0.60	1.05

CALIB NASHYD (3162) Area (ha)= 196.72 Curve Number (CN)= 83.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.88

Unit Hyd Qpeak (cms)= 3.838

PEAK FLOW (cms)= 2.484 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 23.762
 TOTAL RAINFALL (mm)= 54.400
 RUNOFF COEFFICIENT = 0.437

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3161) Area Total (ha)= 35.62 Imp(%)= 63.00 Dir. Conn.(%)= 44.00
 ID= 1 DT=15.0 min

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	22.44	13.18
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	487.31	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	71.81	69.27
over (min)	15.00	30.00
Storage Coeff. (min)=	9.29 (ii)	21.68 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.05

TOTALS
 PEAK FLOW (cms)= 2.70 1.25 3.509 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 52.40 29.27 39.45
 TOTAL RAINFALL (mm)= 54.40 54.40 54.40
 RUNOFF COEFFICIENT = 0.96 0.54 0.73

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 81.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5316)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.3160	0.9307
0.0930	0.5020	2.9060	1.0021
0.5400	0.6637	3.5360	1.0731
1.1940	0.7495	3.9360	1.0831
1.6920	0.8359	0.0000	0.0000

AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)

INFLOW : ID= 2 (3161) 35.620 3.509 6.00 39.45
 OUTFLOW: ID= 1 (5316) 35.620 1.165 6.50 39.41

PEAK FLOW REDUCTION [Qout/Qin] (%) = 33.21
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 0.7536

ADD HYD (9316)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (3162):	196.72	2.484	7.00	23.76
+ ID2= 2 (5316):	35.62	1.165	6.50	39.41
ID = 3 (9316):	232.34	3.560	6.75	26.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8338)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6316):	165.21	2.393	7.25	24.33
+ ID2= 2 (9316):	232.34	3.560	6.75	26.16
ID = 3 (8338):	397.55	5.707	7.00	25.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0312)
 ID= 1 DT=15.0 min

Area (ha) = 359.44 Curve Number (CN) = 80.0
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.36

Unit Hyd Opeak (cms) = 4.529

PEAK FLOW (cms) = 2.884 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 21.475
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.395

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0308)
 ID= 1 DT=15.0 min

Area (ha) = 529.30 Curve Number (CN) = 62.0
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.62

Unit Hyd Opeak (cms) = 5.575

PEAK FLOW (cms) = 1.987 (i)
 TIME TO PEAK (hrs) = 8.500
 RUNOFF VOLUME (mm) = 11.840
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.218

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0310)
 ID= 1 DT=15.0 min

Area (ha) = 138.28 Curve Number (CN) = 65.0
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 0.76

Unit Hyd Opeak (cms) = 3.102

PEAK FLOW (cms) = 1.019 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 12.908
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.237

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8342)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0308):	529.30	1.987	8.50	11.84
+ ID2= 2 (0310):	138.28	1.019	7.00	12.91
ID = 3 (8342):	667.58	2.871	8.00	12.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6312)
 IN= 2--> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3121.0) ----->

Distance	Elevation	Manning	
0.00	265.94	0.0360	
38.07	265.43	0.0360	
59.82	263.98	0.0360	
103.32	254.59	0.0360	
157.70	252.16	0.0360	
217.52	250.45	0.0360	
233.84	247.69	0.0360	
234.71	247.27	0.0360	
239.71	246.38	0.0360	
244.71	246.12	0.0360 / 0.0330	Main Channel
247.71	245.17	0.0330	Main Channel
249.71	245.19	0.0330	Main Channel
251.71	245.64	0.0330 / 0.0360	Main Channel
259.71	246.67	0.0360	
282.78	247.12	0.0360	
315.41	251.60	0.0360	
424.17	256.13	0.0360	
478.55	257.04	0.0360	
516.62	259.37	0.0360	
538.37	266.00	0.0360	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.47	245.64	.111E+04	3.1	1.78	6.07
1.54	246.71	.103E+05	53.9	3.39	3.19
2.61	247.78	.414E+05	281.3	4.40	2.45
3.68	248.85	.839E+05	749.1	5.79	1.87
4.74	249.91	.136E+06	1463.2	6.97	1.55
5.81	250.98	.201E+06	2354.8	7.61	1.42
6.88	252.05	.295E+06	3533.0	7.76	1.39
7.95	253.12	.427E+06	5303.9	8.05	1.34
9.02	254.19	.594E+06	7856.5	8.58	1.26
10.09	255.26	.792E+06	11449.2	9.37	1.15
11.15	256.32	.101E+07	15862.4	10.15	1.06
12.22	257.39	.127E+07	21074.5	10.75	1.01
13.29	258.46	.155E+07	28184.1	11.78	0.92
14.36	259.53	.185E+07	36386.2	12.77	0.85
15.43	260.60	.215E+07	46110.6	13.89	0.78
16.50	261.67	.246E+07	56822.5	14.95	0.72
17.56	262.73	.278E+07	68498.1	15.97	0.68
18.63	263.80	.310E+07	81118.8	16.94	0.64
19.70	264.87	.344E+07	93603.2	17.66	0.61

<----- hydrograph -----> <- pipe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8342)	667.58	2.87	8.00	12.06	0.44	1.78
OUTFLOW: ID= 1 (6312)	667.58	2.87	8.00	12.06	0.44	1.78

ADD HYD (8340)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0312):	359.44	2.884	8.00	21.47
+ ID2= 2 (6312):	667.58	2.869	8.00	12.06
ID = 3 (8340):	1027.02	5.753	8.00	15.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8336)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8338):	397.55	5.707	7.00	25.40
+ ID2= 2 (8340):	1027.02	5.753	8.00	15.36
=====				
ID = 3 (8336):	1424.57	11.106	7.25	18.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8330)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8332):	762.17	9.528	7.50	29.33
+ ID2= 2 (8336):	1424.57	11.106	7.25	18.16
=====				
ID = 3 (8330):	2186.74	20.460	7.50	22.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6324)
IN= 2--> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3241.0) ----->				
Distance	Elevation	Manning		
0.00	234.38	0.0360		
33.57	232.48	0.0360		
67.14	230.14	0.0360		
83.93	228.80	0.0360		
134.29	227.62	0.0360		
209.82	225.10	0.0360		
218.21	224.86	0.0360		
226.61	224.47	0.0360		
234.00	223.86	0.0360 / 0.0300	Main Channel	
234.10	223.66	0.0300	Main Channel	
235.00	223.66	0.0300	Main Channel	
235.90	223.66	0.0300	Main Channel	
236.00	223.86	0.0300 / 0.0360	Main Channel	
243.39	224.92	0.0360		
251.78	224.89	0.0360		
335.71	225.64	0.0360		
562.32	226.53	0.0360		
637.85	228.36	0.0360		
705.00	229.80	0.0360		
830.89	234.00	0.0360		

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.20	223.86	.145E+04	0.2	0.43	147.27
0.73	224.39	.159E+05	2.8	0.68	93.11
1.27	224.93	.561E+05	11.0	0.75	85.19
1.80	225.46	.217E+06	45.6	0.80	79.20
2.33	225.99	.566E+06	134.4	0.91	70.20
2.87	226.53	.122E+07	338.1	1.06	59.97
3.40	227.06	.206E+07	744.3	1.38	46.16
3.94	227.60	.298E+07	1295.9	1.66	38.38
4.47	228.13	.399E+07	1979.9	1.90	33.59
5.00	228.66	.509E+07	2807.5	2.11	30.21
5.54	229.20	.627E+07	3815.1	2.32	27.41
6.07	229.73	.753E+07	4984.1	2.53	25.17
6.60	230.26	.884E+07	6346.3	2.74	23.20
7.14	230.80	.102E+08	7861.2	2.95	21.61
7.67	231.33	.116E+08	9526.4	3.14	20.29
8.21	231.87	.131E+08	11342.2	3.32	19.18
8.74	232.40	.146E+08	13309.6	3.49	18.22
9.27	232.93	.161E+08	15414.0	3.66	17.41
9.81	233.47	.177E+08	17671.0	3.81	16.70

<----- hydrograph ----->					
<--pi pe / channel-->					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)
INFLOW: ID= 2 (8330)	2186.74	20.46	7.50	22.05	1.41
OUTFLOW: ID= 1 (6324)	2186.74	15.90	9.00	22.05	1.34

CALIB NASHYD (3242)			
ID= 1 DT=15.0 mi n			
Area (ha)	Curve Number (CN)	Imp (%)	Dir. Conn. (%)
602.18	78.8	45.00	15.00
U. H. Tp (hrs) = 1.80			

Unit Hyd Opeak (cms) = 5.708

PEAK FLOW (cms) = 3.685 (i)
 TIME TO PEAK (hrs) = 8.500
 RUNOFF VOLUME (mm) = 20.639
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.379

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3241)			
ID= 1 DT=15.0 mi n			
Area Total	Imp (%)	Dir. Conn. (%)	
13.46	45.00	15.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	6.06	7.40
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	299.56	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	71.81	64.92
over (min)	15.00	30.00
Storage Coeff. (min)	6.94 (ii)	19.65 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.10	0.05

		TOTALS
PEAK FLOW (cms)	0.37	0.68
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	52.40	26.99
TOTAL RAINFALL (mm)	54.40	54.40
RUNOFF COEFFICIENT	0.96	0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 78.2 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5324)					
ID= 2----> OUT= 1					
DT= 15.0 mi n					
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)	
	0.0000	0.0000	0.9000	0.3505	
	0.0460	0.1645	1.1310	0.4008	
	0.2610	0.1903	1.3160	0.4499	
	0.4860	0.2456	1.7160	0.4599	
	0.6740	0.2932	0.0000	0.0000	

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (3241)	13.460	0.815	6.00	30.80
OUTFLOW: ID= 1 (5324)	13.460	0.334	6.75	30.74

PEAK FLOW REDUCTION [Qout/Qin] (%) = 41.03
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 0.2088

ADD HYD (9324)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3242):	602.18	3.685	8.50	20.64
+ ID2= 2 (5324):	13.46	0.334	6.75	30.74
=====				
ID = 3 (9324):	615.64	3.778	8.25	20.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6324):	2186.74	15.897	9.00	22.05
+ ID2= 2 (9324):	615.64	3.778	8.25	20.86
=====				
ID = 3 (8328):	2802.38	19.637	9.00	21.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)
	513.13	80.0
	U. H. Tp (hrs) = 1.75	# of Linear Res. (N) = 1.50

Unit Hyd Opeak (cms) = 5.020
 PEAK FLOW (cms) = 3.366 (i)
 TIME TO PEAK (hrs) = 8.502
 RUNOFF VOLUME (mm) = 21.519
 TOTAL RAINFALL (mm) = 54.400
 RUNOFF COEFFICIENT = 0.396

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0322):	513.13	3.366	8.50	21.52
+ ID2= 2 (8328):	2802.38	19.637	9.00	21.79
=====				
ID = 3 (8326):	3315.51	22.976	9.00	21.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0326):	678.91	4.073	9.00	21.53
+ ID2= 2 (8326):	3315.51	22.976	9.00	21.75
=====				
ID = 3 (8324):	3994.42	27.049	9.00	21.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6328)
 IN= 2----> OUT= 1

Routing time step (min) = 15.00

Distance	Elevation	Manning	Main Channel
0.00	228.00	0.0380	
18.58	224.97	0.0380	
78.98	223.52	0.0380	
125.44	223.28	0.0380	
171.90	221.71	0.0380	
213.72	219.65	0.0380	
218.36	219.40	0.0380	
223.01	219.19	0.0380	
225.95	219.14	0.0380 / 0.0320	Main Channel
226.45	218.14	0.0320	Main Channel
236.95	218.14	0.0320	Main Channel
245.85	218.14	0.0320	Main Channel
245.95	219.14	0.0320 / 0.0380	Main Channel
246.24	219.16	0.0380	
250.88	219.24	0.0380	
255.53	219.39	0.0380	
325.22	221.47	0.0380	
367.03	223.14	0.0380	
404.20	225.17	0.0380	

459.95 228.04 0.0380

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	218.64	373E+05	3.2	0.33	192.37
1.00	219.14	752E+05	10.1	0.51	124.71
1.52	219.66	149E+06	22.2	0.57	112.06
2.04	220.18	279E+06	43.4	0.59	107.06
2.56	220.70	464E+06	77.0	0.63	100.52
3.08	221.22	705E+06	125.7	0.68	93.56
3.61	221.75	100E+07	192.7	0.73	86.62
4.13	222.27	135E+07	278.1	0.79	80.92
4.65	222.79	176E+07	385.9	0.84	75.86
5.17	223.31	222E+07	514.9	0.89	71.82
5.69	223.83	280E+07	662.7	0.90	70.40
6.21	224.35	345E+07	867.7	0.96	66.35
6.73	224.87	417E+07	1107.0	1.01	62.81
7.25	225.39	494E+07	1400.0	1.08	58.80
7.78	225.92	573E+07	1732.2	1.15	55.17
8.30	226.44	655E+07	2099.9	1.22	52.02
8.82	226.96	740E+07	2502.8	1.29	49.29
9.34	227.48	828E+07	2940.9	1.36	46.90
9.86	228.00	918E+07	3414.3	1.42	44.79

INFLOW	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
ID= 2 (8324)	3994.42	27.05	9.00	21.71	1.64	0.57
OUTFLOW: ID= 1 (6328)	3994.42	22.44	11.00	21.71	1.53	0.57

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0328):	492.92	2.703	8.75	19.40
+ ID2= 2 (6328):	3994.42	22.441	11.00	21.71
=====				
ID = 3 (8320):	4487.34	24.947	10.75	21.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8320):	4487.34	24.947	10.75	21.46
+ ID2= 2 (8322):	861.74	5.160	9.75	19.93
=====				
ID = 3 (8318):	5349.08	29.930	10.50	21.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (5509):	10059.16	30.704	14.50	16.22
+ ID2= 2 (8318):	5349.08	29.930	10.50	21.21
=====				
ID = 3 (8316):	15408.24	55.846	12.25	17.95

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8314):	620.10	0.587	7.25	10.42
+ ID2= 2 (8316):	15408.24	55.846	12.25	17.95
=====				
ID = 3 (8312):	16028.34	56.363	12.25	17.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8308)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8310):	598.91	15.937	6.50	24.82
+ ID2= 2 (8312):	16028.34	56.363	12.25	17.66
=====				
ID = 3 (8308):	16627.25	57.753	12.00	17.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5510)				
IN= 2--> OUT= 1				
DT= 15.0 mi n				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	*****	74.0090
	66.5450	18.5023	*****	*****
	98.5430	37.0045	*****	*****

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (8308)	*****	57.753	12.00	17.92
OUTFLOW: ID= 1 (5510)	*****	56.851	12.75	17.92

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 98.44
 TIME SHIFT OF PEAK FLOW (mi n) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 15.8138

ADD HYD (8240)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0336):	2785.00	1.997	22.25	16.45
+ ID2= 2 (5510):	16627.25	56.851	12.75	17.92
=====				
ID = 3 (8240):	19412.25	58.518	13.00	17.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8238)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (5508):	9524.19	25.053	12.00	10.74
+ ID2= 2 (8240):	19412.25	58.518	13.00	17.71
=====				
ID = 3 (8238):	28936.44	83.335	12.75	15.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (3427)			
ID= 1 DT=15.0 mi n	Area (ha)	Imp (%)	Dir. Conn. (%)
	249.40	45.00	33.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	112.23	137.17
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1289.44	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	71.81	18.62
over (mi n)	15.00	45.00
Storage Coeff. (mi n)=	16.65 (ii)	37.61 (ii)
Unit Hyd. Tpeak (mi n)=	15.00	45.00
Unit Hyd. peak (cms)=	0.07	0.03

	PEAK FLOW (cms)	TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
	11.42	3.91	52.40	54.40	0.96
	6.00	6.50	16.53	54.40	0.30
	28.37	13.01	28.37	54.40	0.52
	13.018 (iii)	6.00	28.37	54.40	0.52

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 67.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9425)				
IN= 2--> OUT= 1				
DT= 15.0 mi n				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	12.4700	5.4211
	0.8500	3.0375	16.6800	6.4838
	4.8400	3.5126	20.9500	7.4161
	9.0000	4.5390	24.3900	8.3284

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3427)	249.400	13.018	6.00	28.37
OUTFLOW: ID= 1 (9425)	249.400	4.823	6.75	28.37

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 37.05
 TIME SHIFT OF PEAK FLOW (mi n) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 3.5334

RESERVOIR (5342)				
IN= 2--> OUT= 1				
DT= 15.0 mi n				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	3.6600	12.0067
	1.2080	11.1244	4.4200	12.4001
	2.0570	11.3002	5.2300	12.7452
	2.7240	11.3602	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9425)	249.400	4.823	6.75	28.37
OUTFLOW: ID= 1 (5342)	249.400	0.430	18.75	28.36

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 8.92
 TIME SHIFT OF PEAK FLOW (mi n) = 720.00
 MAXIMUM STORAGE USED (ha. m.) = 3.9605

CALIB NASHYD (3422)			
ID= 1 DT=15.0 mi n	Area (ha)	Curve Number (CN)	# of Li near Res. (N)
	755.17	54.5	1.50
	5.00		
	U. H. Tp (hrs) = 2.57		

Unit Hyd Opeak (cms) = 5.026

PEAK FLOW (cms) =	1.572 (i)
TIME TO PEAK (hrs) =	10.250
RUNOFF VOLUME (mm) =	9.310
TOTAL RAINFALL (mm) =	54.400
RUNOFF COEFFICIENT =	0.171

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9342)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3422):	755.17	1.572	10.25	9.31
+ ID2= 2 (5342):	249.40	0.430	18.75	28.36
=====				
ID = 3 (9342):	1004.57	1.919	10.50	14.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8236)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8238):	28936.44	83.335	12.75	15.41
+ ID2= 2 (9342):	1004.57	1.919	10.50	14.04

 ID = 3 (8236): 29941.01 85.162 12.50 15.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8232):	285.79	6.561	6.00	23.75
+ ID2= 2 (8236):	29941.01	85.162	12.50	15.37

ID = 3 (8234):	30226.80	85.629	12.50	15.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (3442) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	188.84 32.00	Dir. Conn. (%)=	24.00
		IMPERVIOUS	PERVIOUS (i)		
Surface Area (ha)=	60.43		128.41		
Dep. Storage (mm)=	2.00		5.00		
Average Slope (%)=	0.50		0.50		
Length (m)=	1122.02		40.00		
Mannings n =	0.013		0.250		
Max. Eff. Inten. (mm/hr)=	71.81		9.85		
over (min)=	15.00		45.00		
Storage Coeff. (min)=	15.32 (ii)		42.35 (ii)		
Unit Hyd. Tpeak (min)=	15.00		45.00		
Unit Hyd. peak (cms)=	0.07		0.03		
				TOTALS	
PEAK FLOW (cms)=	6.53		1.83		7.246 (iii)
TIME TO PEAK (hrs)=	6.00		6.50		6.00
RUNOFF VOLUME (mm)=	52.40		9.91		20.11
TOTAL RAINFALL (mm)=	54.40		54.40		54.40
RUNOFF COEFFICIENT =	0.96		0.18		0.37

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 53.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3441) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	155.15 39.00	Dir. Conn. (%)=	29.00
		IMPERVIOUS	PERVIOUS (i)		
Surface Area (ha)=	60.51		94.64		
Dep. Storage (mm)=	2.00		5.00		
Average Slope (%)=	0.50		0.50		
Length (m)=	1017.02		40.00		
Mannings n =	0.013		0.250		
Max. Eff. Inten. (mm/hr)=	71.81		10.70		
over (min)=	15.00		45.00		
Storage Coeff. (min)=	14.44 (ii)		40.60 (ii)		
Unit Hyd. Tpeak (min)=	15.00		45.00		
Unit Hyd. peak (cms)=	0.07		0.03		
				TOTALS	
PEAK FLOW (cms)=	6.64		1.50		7.236 (iii)
TIME TO PEAK (hrs)=	6.00		6.50		6.00
RUNOFF VOLUME (mm)=	52.40		10.30		22.51
TOTAL RAINFALL (mm)=	54.40		54.40		54.40
RUNOFF COEFFICIENT =	0.96		0.19		0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 53.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 RESERVOIR (5344)
 IN= 2----> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.6700	6.1252
0.3000	1.6125	3.3150	7.0314
0.8100	3.0036	3.8990	7.9700
1.4170	4.2559	4.2990	7.9800
1.8540	5.0526	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3441)	155.150	7.236	6.00	22.51
OUTFLOW: ID= 1 (5344)	155.150	0.601	8.75	22.50

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 8.30
 TIME SHIFT OF PEAK FLOW (min) = 165.00
 MAXIMUM STORAGE USED (ha. m.) = 2.4341

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3442):	188.84	7.246	6.00	20.11
+ ID2= 2 (5344):	155.15	0.601	8.75	22.50

ID = 3 (9344):	343.99	7.431	6.00	21.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8234):	30226.80	85.629	12.50	15.45
+ ID2= 2 (9344):	343.99	7.431	6.00	21.19

ID = 3 (8230):	30570.79	86.272	12.50	15.51

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8226):	1952.06	6.641	8.50	9.38
+ ID2= 2 (8230):	30570.79	86.272	12.50	15.51

ID = 3 (8228):	32522.85	90.545	12.25	15.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0356):	1015.22	2.534	8.00	6.98
+ ID2= 2 (8228):	32522.85	90.545	12.25	15.14

ID = 3 (8190):	33538.07	92.185	12.25	14.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ** SIMULATION NUMBER: 3 **

 READ STORM
 Ptotal = 62.70 mm
 File name: C:\Users\aman\cu\AppData
 Local\Temp
 ab70bd54-a452-450c-b03c-83cdc4826a98\dae664a4
 Comments: 10-Year 12-Hour SCS II Design Storm

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.25	1.57	3.25	2.51	6.25	11.29	9.25	2.19
0.50	1.57	3.50	2.51	6.50	11.29	9.50	2.19
0.75	1.57	3.75	2.51	6.75	5.02	9.75	2.19
1.00	1.57	4.00	2.51	7.00	5.02	10.00	2.19
1.25	1.57	4.25	3.76	7.25	3.76	10.25	1.25
1.50	1.57	4.50	3.76	7.50	3.76	10.50	1.25
1.75	1.57	4.75	5.02	7.75	3.76	10.75	1.25
2.00	1.57	5.00	5.02	8.00	3.76	11.00	1.25
2.25	1.88	5.25	7.52	8.25	2.19	11.25	1.25
2.50	1.88	5.50	7.52	8.50	2.19	11.50	1.25
2.75	1.88	5.75	30.10	8.75	2.19	11.75	1.25
3.00	1.88	6.00	82.76	9.00	2.19	12.00	1.25

CALIB NASHYD (0356) Area (ha)=1015.22 Curve Number (CN)= 46.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U.H. Tp(hrs)= 1.37

Unit Hyd Opeak (cms)= 12.651

PEAK FLOW (cms)= 3.392 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 9.295
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.148

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0352) Area (ha)= 381.43 Curve Number (CN)= 54.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U.H. Tp(hrs)= 0.82

Unit Hyd Opeak (cms)= 7.980

PEAK FLOW (cms)= 2.453 (i)
TIME TO PEAK (hrs)= 7.090
RUNOFF VOLUME (mm)= 11.980
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.191

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0350) Area (ha)= 366.84 Curve Number (CN)= 48.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U.H. Tp(hrs)= 1.07

Unit Hyd Opeak (cms)= 5.831

PEAK FLOW (cms)= 1.564 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 9.910
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.158

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0348) Area (ha)= 590.18 Curve Number (CN)= 48.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U.H. Tp(hrs)= 1.91

Unit Hyd Opeak (cms)= 5.267

PEAK FLOW (cms)= 1.632 (i)
TIME TO PEAK (hrs)= 9.000
RUNOFF VOLUME (mm)= 9.963
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.159

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6350) Routing time step (min)'= 15.00
IN= 2--> OUT= 1

<----- DATA FOR SECTION (3501.0) ----->

Distance	Elevation	Manning
0.00	287.00	0.0500
23.66	283.72	0.0500
70.98	280.94	0.0500
112.38	280.23	0.0500
171.52	276.80	0.0500
260.24	274.46	0.0500
266.16	274.26	0.0500
272.07	274.12	0.0500
276.49	274.12	0.0500 / 0.0300
276.99	273.82	0.0300
277.99	273.82	0.0300
279.49	273.82	0.0300
280.49	274.27	0.0300 / 0.0500
283.90	274.27	0.0500
289.81	274.57	0.0500
325.30	275.75	0.0500
396.28	278.98	0.0500
449.51	280.97	0.0500
496.83	283.90	0.0500
585.54	287.92	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.30	274.12	273E+04	0.9	0.99	49.81
0.98	274.80	583E+05	20.8	1.05	46.71
1.66	275.48	203E+06	95.2	1.38	35.53
2.33	276.15	438E+06	256.0	1.73	28.50
3.01	276.83	755E+06	524.8	2.05	23.97
3.69	277.51	114E+07	935.8	2.43	20.26
4.37	278.19	157E+07	1472.0	2.76	17.82
5.05	278.87	206E+07	2140.7	3.06	16.06
5.72	279.54	261E+07	2938.6	3.33	14.79
6.40	280.22	321E+07	3887.6	3.57	13.77
7.08	280.90	390E+07	4795.4	3.63	13.56
7.76	281.58	468E+07	6196.2	3.91	12.58
8.43	282.25	550E+07	7803.9	4.19	11.74
9.11	282.93	637E+07	9601.9	4.45	11.05
9.79	283.61	728E+07	11594.8	4.70	10.46
10.47	284.29	823E+07	13845.3	4.96	9.91
11.15	284.97	922E+07	16293.4	5.21	9.43
11.82	285.64	103E+08	18952.0	5.45	9.02
12.50	286.32	113E+08	21824.3	5.69	8.65

<---- hydrograph ----> <-pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0348)	590.18	1.63	9.00	9.96	0.32	0.99
OUTFLOW: ID= 1 (6350)	590.18	1.59	10.25	9.96	0.32	0.99

ADD HYD (8220)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0350):	366.84	1.564	7.50	9.91
+ ID2= 2 (6350):	590.18	1.595	10.25	9.96
ID = 3 (8220):	957.02	2.909	8.75	9.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (3461) Area (ha)= 33.62 Dir. Conn. (%)= 38.00
ID= 1 DT=15.0 min Total Imp(%)= 61.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	20.51	13.11
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	473.43	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 82.76 67.41
 over (min) 15.00 30.00
 Storage Coeff. (min)= 8.62 (ii) 21.15 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.05

PEAK FLOW (cms)= 2.58 1.21 *TOTALS* 3.362 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 60.70 27.56 40.15
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.44 0.64

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5346)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.0090	0.6158
0.2450	0.2524	2.6260	0.6610
0.6230	0.3644	3.2370	0.7019
0.9920	0.4690	3.6370	0.7119
1.2510	0.5428	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
33.620	3.362	6.00	40.15
33.620	1.291	6.50	40.15

INFLOW : ID= 2 (3461)
 OUTFLOW: ID= 1 (5346)

PEAK FLOW REDUCTION [Oout/Oin] (%) = 38.41
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 0.5476

CALIB NASHYD (3462)
 ID= 1 DT=15.0 min

Area (ha)= 317.31 Curve Number (CN)= 71.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.96

Unit Hyd Opeak (cms)= 5.625
 PEAK FLOW (cms)= 3.117 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 20.399
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.325

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9346)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (3462):	317.31	3.117	7.25	20.40
+ ID2= 2 (5346):	33.62	1.291	6.50	40.15
ID = 3 (9346):	350.93	4.193	6.75	22.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8222)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8220):	957.02	2.909	8.75	9.94
+ ID2= 2 (9346):	350.93	4.193	6.75	22.29
ID = 3 (8222):	1307.95	6.510	7.25	13.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6352)
 IN= 2--> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3521.0) ----->

Distance	Elevation	Manning
0.00	257.95	0.0500
7.83	257.34	0.0500
15.66	256.19	0.0500
21.53	254.01	0.0500
41.11	244.06	0.0500
76.35	241.38	0.0500
111.58	239.74	0.0500
113.54	239.61	0.0500
113.75	239.41	0.0500 / 0.0300
113.85	238.81	0.0300
115.50	238.81	0.0300
116.15	238.81	0.0300
116.25	239.41	0.0300
117.46	239.52	0.0500
119.41	239.72	0.0500
121.37	240.04	0.0500
131.16	241.84	0.0500
156.61	247.03	0.0500
176.19	251.46	0.0500
193.80	258.79	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.60	239.41	224E+04	2.2	1.52	17.12
1.58	240.39	216E+05	22.3	1.61	16.16
2.55	241.36	804E+05	101.0	1.96	13.27
3.53	242.34	173E+06	275.6	2.48	10.46
4.50	243.31	293E+06	553.9	2.95	8.80
5.48	244.29	438E+06	961.8	3.42	7.60
6.45	245.26	599E+06	1534.4	3.99	6.51
7.43	246.24	770E+06	2228.4	4.51	5.76
8.41	247.22	951E+06	3043.6	4.98	5.21
9.38	248.19	114E+07	3982.1	5.43	4.78
10.36	249.17	134E+07	5040.1	5.85	4.44
11.33	250.14	155E+07	6218.5	6.24	4.16
12.31	251.12	177E+07	7518.6	6.61	3.93
13.29	252.10	200E+07	8969.0	6.98	3.72
14.26	253.07	224E+07	10554.1	7.35	3.53
15.24	254.05	248E+07	12257.1	7.71	3.37
16.21	255.02	272E+07	14045.1	8.03	3.23
17.19	256.00	298E+07	15954.8	8.34	3.11
18.16	256.97	325E+07	17780.4	8.53	3.04

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8222)	1307.95	6.51	7.25	13.26	0.81
OUTFLOW: ID= 1 (6352)	1307.95	6.45	7.75	13.26	1.53

ADD HYD (8224)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0352):	381.43	2.453	7.00	11.98
+ ID2= 2 (6352):	1307.95	6.446	7.75	13.26
ID = 3 (8224):	1689.38	8.810	7.50	12.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6354)
 IN= 2--> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3541.0) ----->

Distance	Elevation	Manning
0.00	253.92	0.0500
7.95	251.83	0.0500
15.89	249.97	0.0500
19.87	249.05	0.0500
47.68	242.00	0.0500
67.55	237.10	0.0500

83.44	231.11	0.0500		
85.43	230.75	0.0500		
85.66	230.61	0.0500 / 0.0300	Main	Channel
85.76	230.01	0.0300	Main	Channel
87.41	230.01	0.0300	Main	Channel
88.06	230.01	0.0300	Main	Channel
88.16	230.61	0.0300 / 0.0500	Main	Channel
89.40	230.72	0.0500		
91.39	230.88	0.0500		
93.37	231.03	0.0500		
133.11	233.96	0.0500		
150.99	235.39	0.0500		
172.84	239.03	0.0500		
196.68	250.00	0.0500		

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.60	230.61	.451E+04	1.5	1.06	49.40
1.62	231.63	.394E+05	14.6	1.16	45.13
2.64	232.65	.128E+06	58.8	1.44	36.30
3.66	233.67	.269E+06	149.7	1.74	30.00
4.68	234.69	.463E+06	301.5	2.04	25.58
5.70	235.71	.705E+06	530.5	2.36	22.14
6.72	236.73	.980E+06	848.4	2.71	19.25
7.74	237.75	.128E+07	1237.9	3.02	17.29
8.76	238.77	.162E+07	1706.7	3.30	15.83
9.78	239.79	.199E+07	2291.9	3.61	14.45
10.81	240.82	.237E+07	2973.7	3.92	13.30
11.83	241.84	.278E+07	3742.2	4.21	12.38
12.85	242.86	.321E+07	4599.1	4.49	11.62
13.87	243.88	.365E+07	5544.6	4.75	10.98
14.89	244.90	.412E+07	6579.7	5.00	10.44
15.91	245.92	.461E+07	7705.9	5.23	9.97
16.93	246.94	.512E+07	8924.6	5.46	9.55
17.95	247.96	.564E+07	10237.5	5.68	9.19
18.97	248.98	.619E+07	11646.2	5.89	8.86

<---- hydrograph ---->				<- pi pe / channel ->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8224)	1689.38	8.81	7.50	12.97	1.17
OUTFLOW: ID= 1 (6354)	1689.38	8.08	8.50	12.97	1.10

CALIB STANDBY (3541) ID= 1 DT=15.0 mi n	Area (ha)= 8.01 Total Imp(%)= 75.00	Di r. Conn. (%)= 70.00
--	--	------------------------

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	6.01	2.00
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	231.08	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	82.76	8.09
over (mi n)	15.00	45.00
Storage Coeff. (mi n)=	5.61 (ii)	34.85 (ii)
Uni t Hyd. Tpeak (mi n)=	15.00	45.00
Uni t Hyd. peak (cms)=	0.11	0.03

TOTALS		
PEAK FLOW (cms)=	1.23	0.03
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	60.70	7.70
TOTAL RAINFALL (mm)=	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.12

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 35.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5354) IN= 2--> OUT= 1 DT= 15.0 mi n	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
--	---------------	------------------	---------------	------------------

0.0000	0.0000	0.5360	0.2546
0.0270	0.1427	0.6730	0.2758
0.1550	0.1714	0.7830	0.2966
0.2890	0.1975	1.1830	0.3066
0.4010	0.2251	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3541)	8.010	1.241	6.00	44.80
OUTFLOW: ID= 1 (5354)	8.010	0.330	6.25	44.63

PEAK FLOW REDUCTION [Out/Oi n] (%) = 26.63
TIME SHIF T OF PEAK FLOW (mi n) = 15.00
MAXIMUM STORAGE USED (ha. m.) = 0.2106

CALIB NASHYD (3542) ID= 1 DT=15.0 mi n	Area (ha)= 254.67 Ia (mm)= 5.00 U. H. Tp (hrs)= 1.37	Curve Number (CN)= 36.7 # of Li near Res. (N)= 1.50
---	--	--

Uni t Hyd Opeak (cms)=	3.174
PEAK FLOW (cms)=	0.608 (i)
TIME TO PEAK (hrs)=	8.000
RUNOFF VOLUME (mm)=	6.672
TOTAL RAINFALL (mm)=	62.700
RUNOFF COEFFICIENT =	0.106

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9354) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3542):	254.67	0.608	8.00	6.67
+ ID2= 2 (5354):	8.01	0.330	6.25	44.63
ID = 3 (9354):	262.68	0.760	6.50	7.83

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8226) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6354):	1689.38	8.081	8.50	12.97
+ ID2= 2 (9354):	262.68	0.760	6.50	7.83
ID = 3 (8226):	1952.06	8.750	8.50	12.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDBY (3402) ID= 1 DT=15.0 mi n	Area (ha)= 138.83 Total Imp(%)= 46.00	Di r. Conn. (%)= 28.00
--	--	------------------------

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	63.86	74.97
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	962.05	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	82.76	15.43
over (mi n)	15.00	45.00
Storage Coeff. (mi n)=	13.20 (ii)	35.78 (ii)
Uni t Hyd. Tpeak (mi n)=	15.00	45.00
Uni t Hyd. peak (cms)=	0.08	0.03

TOTALS		
PEAK FLOW (cms)=	6.86	1.83
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	60.70	12.89
TOTAL RAINFALL (mm)=	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.21

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 47.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3401) ID= 1 DT=15.0 min	Area (ha)= 146.96 Total Imp(%)= 58.00	Di r. Conn. (%)= 35.00
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	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	(ha)= 85.24	61.72
Dep. Storage	(mm)= 2.00	5.00
Average Slope	(%)= 0.50	0.50
Length	(m)= 989.81	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr)=	82.76	20.57
over (min)	15.00	45.00
Storage Coeff. (min)=	13.42 (ii)	33.56 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03

TOTALS
 PEAK FLOW (cms)= 9.01
 TIME TO PEAK (hrs)= 6.00
 RUNOFF VOLUME (mm)= 60.70
 TOTAL RAINFALL (mm)= 62.70
 RUNOFF COEFFICIENT = 0.97

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 47.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5340) IN= 2 --> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	11.4780	4.3560
	0.3860	2.0347	14.9940	4.9991
	3.2310	2.5818	18.1680	5.6243
	5.1770	3.1417	18.5680	5.6343
	7.7590	3.6318	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (3401)	146.960	9.866	6.00	30.79
OUTFLOW: ID= 1 (5340)	146.960	2.714	6.75	30.78

PEAK FLOW REDUCTION [Oout/Oin] (%) = 27.51
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 2.4917

ADD HYD (8232) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3402):	138.83	7.600	6.00	26.28
+ ID2= 2 (5340):	146.96	2.714	6.75	30.78
ID = 3 (8232):	285.79	7.859	6.00	28.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0410) ID= 1 DT=15.0 min	Area (ha)= 572.01 Ia (mm)= 5.00 U. H. Tp(hrs)= 1.46	Curve Number (CN)= 48.0 # of Linear Res. (N)= 1.50
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Unit Hyd Opeak (cms)= 6.698

PEAK FLOW (cms)= 1.949 (i)

TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 9.944
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.159

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0408) ID= 1 DT=15.0 min	Area (ha)= 231.62 Ia (mm)= 5.00 U. H. Tp(hrs)= 0.64	Curve Number (CN)= 58.0 # of Linear Res. (N)= 1.50
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Unit Hyd Opeak (cms)= 6.198

PEAK FLOW (cms)= 2.048 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 13.505
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.215

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0376) ID= 1 DT=15.0 min	Area (ha)= 463.85 Ia (mm)= 5.00 U. H. Tp(hrs)= 1.07	Curve Number (CN)= 74.0 # of Linear Res. (N)= 1.50
---	---	---

Unit Hyd Opeak (cms)= 7.380

PEAK FLOW (cms)= 4.625 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 22.449
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.358

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0374) ID= 1 DT=15.0 min	Area (ha)= 545.70 Ia (mm)= 5.00 U. H. Tp(hrs)= 1.51	Curve Number (CN)= 61.0 # of Linear Res. (N)= 1.50
---	---	---

Unit Hyd Opeak (cms)= 6.158

PEAK FLOW (cms)= 2.759 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 15.043
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.240

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0372) ID= 1 DT=15.0 min	Area (ha)= 110.42 Ia (mm)= 5.00 U. H. Tp(hrs)= 0.96	Curve Number (CN)= 37.0 # of Linear Res. (N)= 1.50
---	---	---

Unit Hyd Opeak (cms)= 1.954

PEAK FLOW (cms)= 0.343 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 6.719
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.107

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0370) ID= 1 DT=15.0 min	Area (ha)= 191.85 Ia (mm)= 5.00 U. H. Tp(hrs)= 0.67	Curve Number (CN)= 63.0 # of Linear Res. (N)= 1.50
---	---	---

Unit Hyd Opeak (cms)= 4.860

PEAK FLOW (cms)= 1.913 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 15.799
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.252

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0366) Area (ha)= 462.62 Curve Number (CN)= 62.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 1.06

Unit Hyd Qpeak (cms)= 7.451

PEAK FLOW (cms)= 3.151 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 15.457
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.247

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3641) Area (ha)= 7.85 Di r. Conn. (%)= 16.00
 ID= 1 DT=15.0 min Total Imp(%)= 45.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	3.53	4.32
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	228.76	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 82.76 17.23
 over (min) 15.00 30.00
 Storage Coeff. (min)= 5.57 (ii) 27.19 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.11 0.04

PEAK FLOW (cms)= 0.28 0.15 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.25 0.366 (iii)
 RUNOFF VOLUME (mm)= 60.70 12.58 6.00
 TOTAL RAINFALL (mm)= 62.70 62.70 20.27
 RUNOFF COEFFICIENT = 0.97 0.20 0.32

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 42.9 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5364) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	0.5250	0.2044
	0.0270	0.0959	0.6590	0.2337
	0.1520	0.1110	0.7670	0.2624
	0.2830	0.1432	1.1670	0.2724
	0.3930	0.1710	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (3641)	7.850	0.366	6.00	20.27
OUTFLOW: ID= 1 (5364)	7.850	0.054	7.50	20.16

PEAK FLOW REDUCTION [Oout/Oin] (%) = 14.74
 TIME SHIFT OF PEAK FLOW (min) = 90.00
 MAXIMUM STORAGE USED (ha. m.) = 0.0993

CALIB

STANDHYD (3642) Area (ha)= 147.42 Di r. Conn. (%)= 7.00
 ID= 1 DT=15.0 min Total Imp(%)= 21.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	30.96	116.46
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	991.36	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 82.76 10.29
 over (min) 15.00 45.00
 Storage Coeff. (min)= 13.44 (ii) 40.00 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03

PEAK FLOW (cms)= 1.81 1.79 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.50 2.520 (iii)
 RUNOFF VOLUME (mm)= 60.70 9.89 6.00
 TOTAL RAINFALL (mm)= 62.70 62.70 13.44
 RUNOFF COEFFICIENT = 0.97 0.16 62.70
 0.21

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 42.9 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9364) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3642):	147.42	2.520	6.00	13.44
+ ID2= 2 (5364):	7.85	0.054	7.50	20.16
ID = 3 (9364):	155.27	2.533	6.00	13.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8302) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0366):	462.62	3.151	7.50	15.46
+ ID2= 2 (9364):	155.27	2.533	6.00	13.78
ID = 3 (8302):	617.89	5.018	6.50	15.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0358) Area (ha)= 429.87 Curve Number (CN)= 35.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 1.03

Unit Hyd Qpeak (cms)= 7.091

PEAK FLOW (cms)= 1.174 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 6.228
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.099

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0360) Area (ha)= 138.37 Curve Number (CN)= 46.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.60

Unit Hyd Qpeak (cms)= 3.957

PEAK FLOW (cms)= 0.860 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 9.151
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.146

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8306)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0358):	429.87	1.174	7.50	6.23
+ ID2= 2 (0360):	138.37	0.860	6.75	9.15
ID = 3 (8306):	568.24	1.984	7.00	6.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6362)
 IN= 2---> OUT= 1
 Routing time step (min)' = 15.00

----- DATA FOR SECTION (3621.0) -----

Distance	Elevation	Manning	
0.00	261.46	0.0550	
27.86	254.23	0.0550	
51.07	251.96	0.0550	
74.29	250.77	0.0550	
97.50	249.91	0.0550	
125.36	249.40	0.0550	
150.93	247.40	0.0550	
155.93	247.33	0.0550 / 0.0350	Main Channel
157.93	246.85	0.0350	Main Channel
159.18	246.65	0.0350	Main Channel
160.18	246.63	0.0350	Main Channel
160.93	246.85	0.0350	Main Channel
161.93	247.18	0.0350 / 0.0550	Main Channel
163.18	248.03	0.0550	
168.18	248.58	0.0550	
183.18	250.18	0.0550	
201.97	252.59	0.0550	
213.57	256.02	0.0550	
225.18	260.31	0.0550	
229.82	261.00	0.0550	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.55	247.18	.556E+04	1.9	1.03	49.95
1.28	247.91	.336E+05	16.7	1.53	33.61
2.00	248.63	.913E+05	53.6	1.81	28.40
2.73	249.36	.185E+06	122.3	2.05	25.18
3.46	250.09	.344E+06	224.6	2.02	25.52
4.19	250.82	.574E+06	417.9	2.25	22.91
4.91	251.54	.855E+06	699.1	2.53	20.38
5.64	252.27	.118E+07	1075.3	2.82	18.26
6.37	253.00	.153E+07	1559.9	3.15	16.38
7.10	253.73	.191E+07	2137.6	3.46	14.89
7.82	254.45	.231E+07	2815.2	3.77	13.67
8.55	255.18	.272E+07	3608.5	4.10	12.58
9.28	255.91	.315E+07	4488.2	4.41	11.69
10.01	256.64	.358E+07	5458.0	4.71	10.95
10.73	257.36	.403E+07	6512.4	4.99	10.32
11.46	258.09	.449E+07	7649.8	5.27	9.78
12.19	258.82	.496E+07	8869.5	5.53	9.32
12.92	259.55	.544E+07	10171.2	5.78	8.91
13.64	260.27	.593E+07	11554.8	6.02	8.55

----- hydrograph -----

INFLOW : ID= 2 (8306)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW: ID= 1 (6362)	568.24	1.98	7.00	6.94	0.56	1.03
	568.24	1.75	8.25	6.94	0.52	1.03

CALIB STANDHYD (3621)
 ID= 1 DT=15.0 mi n
 Area (ha)= 11.87
 Total Imp(%)= 45.00
 Di r. Conn.(%)= 17.00

----- IMPERVIOUS PERVIOUS (i)

Surface Area (ha)=	5.34	6.53
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	281.31	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	82.76	18.49
over (mi n)	15.00	30.00
Storage Coeff. (mi n)=	6.31 (ii)	27.33 (ii)
Unit Hyd. Tpeak (mi n)=	15.00	30.00
Unit Hyd. peak (cms)=	0.10	0.04
PEAK FLOW (cms)=	0.44	0.24
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	60.70	13.60
TOTAL RAINFALL (mm)=	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.22

TOTALS
 0.581 (iii)
 6.00
 21.60
 62.70
 0.34

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 45.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- RESERVOIR (5362) -----
 IN= 2---> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.7940	0.3091
0.0400	0.1451	0.9970	0.3534
0.2300	0.1678	1.1610	0.3968
0.4280	0.2166	1.5610	0.4068
0.5940	0.2586	0.0000	0.0000

INFLOW : ID= 2 (3621)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5362)	11.870	0.581	6.00	21.60
	11.870	0.109	7.25	21.53

PEAK FLOW REDUCTION [Out/Oi n](%)= 18.72
 TIME SHIFT OF PEAK FLOW (mi n)= 75.00
 MAXIMUM STORAGE USED (ha. m.)= 0.1538

CALIB STANDHYD (3622)
 ID= 1 DT=15.0 mi n
 Area (ha)= 106.91
 Total Imp(%)= 24.00
 Di r. Conn.(%)= 9.00

----- IMPERVIOUS PERVIOUS (i)

Surface Area (ha)=	25.66	81.25
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	844.24	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	82.76	11.74
over (mi n)	15.00	45.00
Storage Coeff. (mi n)=	12.20 (ii)	37.40 (ii)
Unit Hyd. Tpeak (mi n)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03
PEAK FLOW (cms)=	1.75	1.48
TIME TO PEAK (hrs)=	6.00	6.50
RUNOFF VOLUME (mm)=	60.70	11.03
TOTAL RAINFALL (mm)=	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.18

TOTALS
 2.340 (iii)
 6.00
 15.50
 62.70
 0.25

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 45.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9362)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (3622):	106.91	2.340	6.00	15.50	
+ ID2= 2 (5362):	11.87	0.109	7.25	21.53	

ID = 3 (9362):	118.78	2.359	6.00	16.10	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8304)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (6362):	568.24	1.750	8.25	6.94	
+ ID2= 2 (9362):	118.78	2.359	6.00	16.10	

ID = 3 (8304):	687.02	2.818	6.50	8.52	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8300)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (8302):	617.89	5.018	6.50	15.04	
+ ID2= 2 (8304):	687.02	2.818	6.50	8.52	

ID = 3 (8300):	1304.91	7.836	6.50	11.61	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6368)
IN= 2----> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3681.0) ----->

Distance	Elevation	Manning			
0.00	230.00	0.0370			
18.48	223.26	0.0370			
36.96	223.05	0.0370			
64.67	222.94	0.0370			
110.87	222.86	0.0370			
133.96	222.74	0.0370			
147.82	222.65	0.0370			
170.92	222.31	0.0370			
174.79	222.26	0.0370 / 0.0300	Main	Channel	
174.89	221.86	0.0300	Main	Channel	
175.54	221.86	0.0300	Main	Channel	
176.19	221.86	0.0300	Main	Channel	
176.29	222.26	0.0300 / 0.0370	Main	Channel	
180.16	222.25	0.0370			
184.78	222.28	0.0370			
189.40	222.31	0.0370			
332.60	222.37	0.0370			
450.00	230.00	0.0370			

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.40	222.26	.771E+03	0.3	0.52	41.74
0.81	222.67	.784E+05	30.3	0.51	43.17
1.21	223.07	.210E+06	123.2	0.77	28.42
1.62	223.48	.384E+06	301.0	1.03	21.25
2.03	223.89	.563E+06	550.2	1.28	17.06
2.44	224.30	.747E+06	860.7	1.51	14.46
2.84	224.70	.934E+06	1228.1	1.73	12.68
3.25	225.11	.113E+07	1649.3	1.92	11.37
3.66	225.52	.132E+07	2122.3	2.11	10.37
4.07	225.93	.152E+07	2645.5	2.29	9.58
4.47	226.33	.172E+07	3217.8	2.45	8.92
4.88	226.74	.193E+07	3838.3	2.61	8.38
5.29	227.15	.214E+07	4506.3	2.76	7.92
5.70	227.56	.236E+07	5221.3	2.91	7.52

6.10	227.96	.258E+07	5983.0	3.05	7.17
6.51	228.37	.280E+07	6790.9	3.19	6.87
6.92	228.78	.303E+07	7645.0	3.32	6.60
7.33	229.19	.326E+07	8545.1	3.45	6.35
7.73	229.59	.349E+07	9491.0	3.57	6.13

<---- hydrograph ---->						<-pi pe / channel ->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)		MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8300)	1304.91	7.84	6.50	11.61		0.50	0.52
OUTFLOW: ID= 1 (6368)	1304.91	6.47	7.75	11.61		0.48	0.52

CALIB
STANDHYD (3681)
ID= 1 DT=15.0 min

Area (ha)= 2.95
Total Imp(%)= 45.00
Di r. Conn. (%)= 15.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	1.33	1.62
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	140.24	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	82.76	16.28
over (min)	15.00	30.00
Storage Coeff. (min)	4.16 (ii)	26.27 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.11	0.04
PEAK FLOW (cms)	0.10	0.05
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	60.70	11.79
TOTAL RAINFALL (mm)	62.70	62.70
RUNOFF COEFFICIENT	0.97	0.19

TOTALS
0.132 (iii)
6.00
19.12
62.70
0.30

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 40.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5368)
IN= 2----> OUT= 1
DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	0.1970	0.0767
	0.0100	0.0360	0.2470	0.0877
	0.0570	0.0416	0.2880	0.0984
	0.1060	0.0537	0.6880	0.1084
	0.1470	0.0642	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3681)	2.950	0.132	6.00	19.12
OUTFLOW: ID= 1 (5368)	2.950	0.015	7.75	18.82

PEAK FLOW REDUCTION [Qout/Qin] (%) = 11.44
TIME SHIFT OF PEAK FLOW (min) = 105.00
MAXIMUM STORAGE USED (ha. m.) = 0.0366

CALIB
NASHYD (3682)
ID= 1 DT=15.0 min

Area (ha)= 156.53
Ia (mm)= 5.00
U. H. Tp (hrs)= 0.96
Curve Number (CN)= 45.9
of Linear Res. (N)= 1.50

Unit Hyd Qpeak (cms)= 2.778

PEAK FLOW (cms)	0.674 (i)
TIME TO PEAK (hrs)	7.250
RUNOFF VOLUME (mm)	9.223
TOTAL RAINFALL (mm)	62.700
RUNOFF COEFFICIENT	0.147

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9368)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (3682):	156.53	0.674	7.25	9.22	
+ ID2= 2 (5368):	2.95	0.015	7.75	18.82	
=====					
ID = 3 (9368):	159.48	0.687	7.50	9.40	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8298)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (6368):	1304.91	6.469	7.75	11.61	
+ ID2= 2 (9368):	159.48	0.687	7.50	9.40	
=====					
ID = 3 (8298):	1464.39	7.150	7.75	11.37	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8296)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (0370):	191.85	1.913	6.75	15.80	
+ ID2= 2 (8298):	1464.39	7.150	7.75	11.37	
=====					
ID = 3 (8296):	1656.24	8.888	7.50	11.88	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6372)
IN= 2----> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3721.0) ----->

Di stance	Elevation	Manning			
0.00	225.00	0.0390			
30.80	219.38	0.0390			
61.61	219.30	0.0390			
77.01	219.27	0.0390			
469.76	219.14	0.0390			
477.46	219.13	0.0390			
485.16	219.10	0.0390			
492.86	219.09	0.0390			
495.56	219.09	0.0390 / 0.0310	Main	Channel	
495.66	218.51	0.0310	Main	Channel	
500.56	218.51	0.0310	Main	Channel	
505.46	218.51	0.0310	Main	Channel	
505.56	219.09	0.0310 / 0.0390	Main	Channel	
508.26	219.09	0.0390			
515.96	219.10	0.0390			
523.67	219.21	0.0390			
562.17	219.32	0.0390			
654.58	219.43	0.0390			
731.59	219.46	0.0390			
762.39	225.00	0.0390			

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.29	218.80	.431E+04	1.7	0.58	43.48
0.58	219.09	.867E+04	5.1	0.89	28.32
0.93	219.44	.200E+06	58.2	0.44	57.35
1.28	219.79	.568E+06	278.6	0.74	33.99
1.62	220.13	.939E+06	630.2	1.01	24.84
1.97	220.48	.131E+07	1090.4	1.25	20.05
2.32	220.83	.169E+07	1647.7	1.47	17.06
2.67	221.18	.206E+07	2294.6	1.68	14.99
3.01	221.52	.244E+07	3025.6	1.87	13.46
3.36	221.87	.282E+07	3836.5	2.05	12.27
3.71	222.22	.321E+07	4723.7	2.22	11.31
4.06	222.57	.359E+07	5684.6	2.39	10.53

4.40	222.91	.398E+07	6716.6	2.55	9.87
4.75	223.26	.437E+07	7817.7	2.70	9.31
5.10	223.61	.476E+07	8986.2	2.85	8.83
5.45	223.96	.515E+07	10220.4	2.99	8.40
5.79	224.30	.555E+07	11519.1	3.13	8.03
6.14	224.65	.594E+07	12880.8	3.27	7.69
6.49	225.00	.634E+07	14304.5	3.40	7.39

		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	<-pi pe / channel ->	
						MAX DEPTH (m)	MAX VEL (m/s)
INFLOW :	ID= 2 (8296)	1656.24	8.89	7.50	11.88	0.60	0.83
OUTFLOW:	ID= 1 (6372)	1656.24	8.52	8.25	11.88	0.60	0.83

ADD HYD (8294)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (0372):	110.42	0.343	7.25	6.72	
+ ID2= 2 (6372):	1656.24	8.519	8.25	11.88	
=====					
ID = 3 (8294):	1766.66	8.844	8.25	11.56	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8292)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (0374):	545.70	2.759	8.25	15.04	
+ ID2= 2 (8294):	1766.66	8.844	8.25	11.56	
=====					
ID = 3 (8292):	2312.36	11.603	8.25	12.38	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5505)
IN= 2----> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	65.1290	345.3754
25.4850	24.6697	84.9510	456.3890
31.1490	98.6787	*****	838.7689
39.6440	*****	*****	838.7789
48.1390	*****	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW :	ID= 2 (8292)	2312.360	11.603	8.25
OUTFLOW:	ID= 1 (5505)	2312.360	8.313	11.00

PEAK FLOW REDUCTION [Qout/Qi n](%) = 71.65
TIME SHI FT OF PEAK FLOW (mi n) = 165.00
MAXI MUM STORAGE USED (ha. m.) = 8.0502

ADD HYD (8272)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (0376):	463.85	4.625	7.50	22.45	
+ ID2= 2 (5505):	2312.36	8.313	11.00	12.38	
=====					
ID = 3 (8272):	2776.21	11.409	10.00	14.06	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD > (0396)
ID= 1 DT=15.0 mi n

Area (ha)	= 305.21	Curve Number (CN)	= 69.0
Ia (mm)	= 5.00	# of Li near Res. (N)	= 1.50
U. H. Tp(hrs)	= 1.08		

Uni t Hyd Opeak (cms)= 4.811

PEAK FLOW (cms)= 2.564 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 19.201
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.306

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0394) Area (ha)= 325.45 Curve Number (CN)= 53.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.92

Unit Hyd Qpeak (cms)= 6.013

PEAK FLOW (cms)= 1.838 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 11.632
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.186

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0390) Area (ha)= 420.00 Curve Number (CN)= 55.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 6.683

PEAK FLOW (cms)= 2.260 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 12.424
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.198

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0388) Area (ha)= 220.77 Curve Number (CN)= 58.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.99

Unit Hyd Qpeak (cms)= 3.819

PEAK FLOW (cms)= 1.394 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 13.635
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.217

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0386) Area (ha)= 241.27 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.90

Unit Hyd Qpeak (cms)= 4.562

PEAK FLOW (cms)= 1.798 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 14.947
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.238

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8286)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0386):	241.27	1.798	7.25	14.95
+ ID2= 2 (0388):	220.77	1.394	7.25	13.64

 ID = 3 (8286): 462.04 3.192 7.25 14.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0384) Area (ha)= 199.07 Curve Number (CN)= 44.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 3.537

PEAK FLOW (cms)= 0.803 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 8.644
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.138

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0380) Area (ha)= 182.01 Curve Number (CN)= 40.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.55

Unit Hyd Qpeak (cms)= 5.609

PEAK FLOW (cms)= 0.964 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 7.404
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.118

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0382) Area (ha)= 216.59 Curve Number (CN)= 53.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.64

Unit Hyd Qpeak (cms)= 5.733

PEAK FLOW (cms)= 1.611 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 11.537
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.184

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8290)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0380):	182.01	0.964	6.50	7.40
+ ID2= 2 (0382):	216.59	1.611	6.75	11.54

ID = 3 (8290):	398.60	2.571	6.75	9.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6384)
 IN= 2----> OUT= 1

<----- DATA FOR SECTION (3841.0) ----->

Distance	Elevation	Manning
0.00	294.40	0.0380
10.59	291.93	0.0380
21.17	289.19	0.0380
26.46	287.99	0.0380
31.76	286.79	0.0380
71.45	279.97	0.0380
74.10	279.79	0.0380
76.74	279.71	0.0380

78.99	279.64	0.0380	/0.0300	Main Channel
79.14	279.30	0.0300		Main Channel
79.39	279.30	0.0300		Main Channel
79.64	279.30	0.0300		Main Channel
79.79	279.64	0.0300	/0.0380	Main Channel
89.98	279.78	0.0380		
119.09	281.86	0.0380		
145.55	282.87	0.0380		
198.48	284.85	0.0380		
211.71	286.31	0.0380		
230.23	287.59	0.0380		
261.99	294.00	0.0380		

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.34	279.64	.715E+03	0.2	0.85	63.42
1.10	280.40	.507E+05	21.9	1.40	38.54
1.85	281.15	.141E+06	91.8	2.10	25.69
2.61	281.91	.269E+06	218.9	2.63	20.46
3.36	282.66	.445E+06	407.1	2.96	18.23
4.12	283.42	.681E+06	698.3	3.31	16.26
4.87	284.17	.977E+06	1111.8	3.68	14.65
5.63	284.93	.133E+07	1674.3	4.06	13.27
6.39	285.69	.173E+07	2457.8	4.59	11.73
7.14	286.44	.215E+07	3378.8	5.07	10.63
7.90	287.20	.261E+07	4418.4	5.47	9.85
8.65	287.95	.310E+07	5676.3	5.92	9.11
9.41	288.71	.361E+07	7150.5	6.40	8.42
10.17	289.47	.414E+07	8783.1	6.86	7.86
10.92	290.22	.469E+07	10575.1	7.30	7.38
11.68	290.98	.525E+07	12520.4	7.72	6.98
12.43	291.73	.582E+07	14619.0	8.12	6.64
13.19	292.49	.642E+07	16863.2	8.50	6.34
13.94	293.24	.703E+07	19259.8	8.86	6.08

<---- hydrograph ---->				<-pi pe / channel -->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8290)	398.60	2.57	6.75	9.65	0.42
OUTFLOW: ID= 1 (6384)	398.60	2.01	8.00	9.65	0.89

ADD HYD (8288)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0384):	199.07	0.803	7.25	8.64
+ ID2= 2 (6384):	398.60	2.015	8.00	9.65
ID = 3 (8288):	597.67	2.797	8.00	9.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8284)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8286):	462.04	3.192	7.25	14.32
+ ID2= 2 (8288):	597.67	2.797	8.00	9.31
ID = 3 (8284):	1059.71	5.920	7.75	11.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8280)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0390):	420.00	2.260	7.50	12.42
+ ID2= 2 (8284):	1059.71	5.920	7.75	11.50
ID = 3 (8280):	1479.71	8.177	7.50	11.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0392)			
ID= 1 DT=15.0 mi n	Area (ha)	Curve Number (CN)	# of Li near Res. (N)
	167.22	62.0	1.50
	U. H. Tp (hrs)= 5.00		

Unit Hyd Qpeak (cms) = 3.837

PEAK FLOW (cms)	= 1.497 (i)
TIME TO PEAK (hrs)	= 7.000
RUNOFF VOLUME (mm)	= 15.356
TOTAL RAINFALL (mm)	= 62.700
RUNOFF COEFFICIENT	= 0.245

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0378)			
ID= 1 DT=15.0 mi n	Area (ha)	Curve Number (CN)	# of Li near Res. (N)
	606.72	55.0	1.50
	U. H. Tp (hrs)= 5.00		

Unit Hyd Qpeak (cms) = 8.771

PEAK FLOW (cms)	= 3.046 (i)
TIME TO PEAK (hrs)	= 8.000
RUNOFF VOLUME (mm)	= 12.439
TOTAL RAINFALL (mm)	= 62.700
RUNOFF COEFFICIENT	= 0.198

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8282)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0378):	606.72	3.046	8.00	12.44
+ ID2= 2 (0392):	167.22	1.497	7.00	15.36
ID = 3 (8282):	773.94	4.443	7.25	13.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8278)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8280):	1479.71	8.177	7.50	11.76
+ ID2= 2 (8282):	773.94	4.443	7.25	13.07
ID = 3 (8278):	2253.65	12.619	7.50	12.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6394)		Routing time step (mi n) = 15.00
IN= 2	OUT= 1	

<----- DATA FOR SECTION (3941.0) ----->		
Di stance	El evation	Manni ng
0.00	283.00	0.0380
13.48	282.87	0.0380
53.92	280.08	0.0380
74.13	276.62	0.0380
97.72	265.45	0.0380
114.57	256.93	0.0380
131.42	253.04	0.0380
134.79	252.58	0.0380
138.53	251.74	0.0380 /0.0300
139.03	251.20	0.0300
141.53	251.20	0.0300
144.03	251.20	0.0300
144.53	251.74	0.0300 /0.0380
148.27	252.69	0.0380
151.64	252.97	0.0380
185.34	255.08	0.0380
219.03	257.54	0.0380
262.84	259.43	0.0380
310.02	262.80	0.0380

333.60 283.00 0.0380

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.54	251.74	.827E+04	4.5	1.51	30.71
2.19	253.39	.758E+05	73.2	2.69	17.27
3.83	255.03	.282E+06	331.8	3.27	14.17
5.48	256.68	.633E+06	917.3	4.03	11.51
7.12	258.32	.112E+07	1889.5	4.69	9.90
8.77	259.97	.179E+07	3410.2	5.31	8.73
10.41	261.61	.259E+07	5627.6	6.05	7.66
12.06	263.26	.351E+07	8605.1	6.83	6.79
13.70	264.90	.447E+07	12544.9	7.81	5.94
15.35	266.55	.546E+07	17101.4	8.71	5.32
16.99	268.19	.648E+07	22251.7	9.56	4.85
18.64	269.84	.752E+07	27979.0	10.36	4.48
20.28	271.48	.858E+07	34269.0	11.11	4.17
21.93	273.13	.967E+07	41110.8	11.83	3.92
23.57	274.77	.108E+08	48495.9	12.51	3.71
25.22	276.42	.119E+08	56418.1	13.17	3.52
26.86	278.06	.131E+08	64410.6	13.69	3.39
28.51	279.71	.143E+08	73002.1	14.18	3.27
30.15	281.35	.156E+08	81379.3	14.50	3.20

<---- hydrograph ----> <-pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8278)	2253.65	12.62	7.50	12.21	0.73	1.59
OUTFLOW: ID= 1 (6394)	2253.65	12.25	8.25	12.21	0.73	1.59

ADD HYD (8276)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0394):	325.45	1.838	7.25	11.63
+ ID2= 2 (6394):	2253.65	12.251	8.25	12.21
ID = 3 (8276):	2579.10	13.996	8.00	12.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6396)
IN= 2---> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3961.0) ----->

Distance	Elevation	Manning	
0.00	263.00	0.0410	
11.75	257.14	0.0410	
23.50	253.97	0.0410	
41.13	247.83	0.0410	
76.38	232.09	0.0410	
135.13	229.07	0.0410	
149.82	228.97	0.0410	
152.75	228.96	0.0410 / 0.0300	Main Channel
154.19	228.73	0.0300	Main Channel
154.69	228.20	0.0300	Main Channel
155.69	228.20	0.0300	Main Channel
156.69	228.20	0.0300	Main Channel
157.19	228.73	0.0300	Main Channel
158.63	228.95	0.0300 / 0.0410	Main Channel
161.57	228.96	0.0410	
164.51	229.71	0.0410	
196.82	241.70	0.0410	
223.26	249.21	0.0410	
246.76	255.13	0.0410	
290.82	263.51	0.0410	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.75	228.95	.780E+04	2.2	0.98	58.06
2.54	230.74	.277E+06	145.1	1.78	31.76
4.33	232.53	.786E+06	609.1	2.63	21.50
6.13	234.33	.140E+07	1481.5	3.59	15.77
7.92	236.12	.207E+07	2677.3	4.39	12.90
9.71	237.91	.280E+07	4184.8	5.09	11.14
11.50	239.70	.357E+07	6001.4	5.71	9.93
13.29	241.49	.441E+07	8129.1	6.27	9.03

15.09	243.29	.530E+07	10549.4	6.77	8.37
16.88	245.08	.625E+07	13300.6	7.24	7.83
18.67	246.87	.726E+07	16396.2	7.68	7.38
20.46	248.66	.834E+07	19809.8	8.07	7.02
22.26	250.46	.949E+07	23532.5	8.43	6.72
24.05	252.25	.107E+08	27641.2	8.77	6.46
25.84	254.04	.120E+08	32155.2	9.10	6.22
27.63	255.83	.134E+08	36901.0	9.37	6.05
29.42	257.62	.149E+08	42147.7	9.64	5.88
31.22	259.42	.164E+08	48149.1	9.97	5.68
33.01	261.21	.181E+08	54644.8	10.29	5.51

<---- hydrograph ----> <-pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8276)	2579.10	14.00	8.00	12.14	0.90	1.01
OUTFLOW: ID= 1 (6396)	2579.10	12.65	9.25	12.14	0.88	1.01

ADD HYD (8274)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0396):	305.21	2.564	7.50	19.20
+ ID2= 2 (6396):	2579.10	12.651	9.25	12.14
ID = 3 (8274):	2884.31	14.864	9.00	12.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8270)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8272):	2776.21	11.409	10.00	14.06
+ ID2= 2 (8274):	2884.31	14.864	9.00	12.88
ID = 3 (8270):	5660.52	25.939	9.25	13.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5506)
IN= 2---> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	60.8810	135.6832
31.1490	24.6697	96.2770	900.4431
36.8120	37.0045	96.6770	900.4531
45.3070	86.3439	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (8270)	5660.520	25.939	9.25	13.46
OUTFLOW: ID= 1 (5506)	5660.520	21.378	11.75	13.46

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 82.42
TIME SHIFT OF PEAK FLOW (mi n) = 150.00
MAXIMUM STORAGE USED (ha. m.) = 16.9394

CALIB NASHYD (0406)
ID= 1 DT=15.0 mi n

Area (ha) = 142.65	Curve Number (CN) = 66.0
Ia (mm) = 5.00	# of Linear Res. (N) = 1.50
U. H. Tp (hrs) = 0.59	

Unit Hyd Opeak (cms) = 4.135

PEAK FLOW (cms) = 1.735 (i)
TIME TO PEAK (hrs) = 6.750
RUNOFF VOLUME (mm) = 17.263
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.275

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0404)
 NASHYD (0404)
 ID= 1 DT=15.0 min
 Area (ha)= 246.46
 Curve Number (CN)= 47.0
 I a (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.98

Unit Hyd Qpeak (cms)= 4.280
 PEAK FLOW (cms)= 1.084 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 9.573
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.153

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0402)
 NASHYD (0402)
 ID= 1 DT=15.0 min
 Area (ha)= 244.00
 Curve Number (CN)= 61.0
 I a (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 3.879
 PEAK FLOW (cms)= 1.593 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 14.988
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.239

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0400)
 NASHYD (0400)
 ID= 1 DT=15.0 min
 Area (ha)= 93.97
 Curve Number (CN)= 52.0
 I a (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.44

Unit Hyd Qpeak (cms)= 3.630
 PEAK FLOW (cms)= 0.902 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 11.008
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.176

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0398)
 NASHYD (0398)
 ID= 1 DT=15.0 min
 Area (ha)= 328.19
 Curve Number (CN)= 55.0
 I a (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.83

Unit Hyd Qpeak (cms)= 6.759
 PEAK FLOW (cms)= 2.154 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 12.370
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.197

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0398):	328.19	2.154	7.00	12.37
+ ID2= 2 (0400):	93.97	0.902	6.50	11.01
=====				
ID = 3 (8268):	422.16	2.966	6.75	12.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6402)
 IN= 2--> OUT= 1
 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (4021.0) ----->

Distance	Elevation	Manning
0.00	238.50	0.0360
11.50	238.00	0.0360
23.00	237.93	0.0360
34.49	236.39	0.0360
63.24	233.98	0.0360
97.73	228.15	0.0360
123.60	227.08	0.0360
126.48	226.61	0.0360
127.60	226.47	0.0360 /0.0330
127.85	225.25	0.0330
129.35	225.25	0.0330
130.85	225.25	0.0330
131.10	226.47	0.0330 /0.0360
132.22	226.59	0.0360
143.72	227.42	0.0360
169.59	227.88	0.0360
192.59	231.19	0.0360
218.46	233.02	0.0360
241.45	235.50	0.0360
284.57	236.43	0.0360

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.41	225.66	391E+04	1.7	1.36	38.31
0.81	226.06	804E+04	5.0	1.93	27.02
1.22	226.47	124E+05	9.1	2.30	22.61
1.84	227.09	308E+05	23.3	2.36	22.03
2.46	227.71	904E+05	62.0	2.14	24.31
3.09	228.34	223E+06	188.3	2.63	19.75
3.71	228.96	379E+06	410.2	3.38	15.39
4.33	229.58	550E+06	711.5	4.04	12.89
4.95	230.20	737E+06	1091.3	4.62	11.25
5.58	230.83	939E+06	1550.6	5.15	10.10
6.20	231.45	116E+07	2072.4	5.59	9.31
6.82	232.07	140E+07	2659.2	5.93	8.77
7.44	232.69	167E+07	3345.8	6.27	8.30
8.07	233.32	195E+07	4159.0	6.64	7.83
8.69	233.94	226E+07	5100.8	7.03	7.39
9.31	234.56	259E+07	6081.3	7.32	7.11
9.93	235.18	295E+07	7183.1	7.60	6.84
10.56	235.81	334E+07	8145.2	7.62	6.83
11.18	236.43	379E+07	9116.0	7.51	6.92

INFLOW: ID= 2 (8268)	AREA (ha)	<---- hydrograph ---->			<- pipe / channel ->	
		OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
422.16	2.97	6.75	12.07	0.56	1.53	
OUTFLOW: ID= 1 (6402)	422.16	2.70	7.50	12.07	0.53	1.49

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0402):	244.00	1.593	7.50	14.99
+ ID2= 2 (6402):	422.16	2.701	7.50	12.07
=====				
ID = 3 (8266):	666.16	4.294	7.50	13.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0404):	246.46	1.084	7.50	9.57
+ ID2= 2 (8266):	666.16	4.294	7.50	13.14
=====				
ID = 3 (8264):	912.62	5.378	7.50	12.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3				

	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0406):	142.65	1.735	6.75	17.26
+ ID2= 2 (8264):	912.62	5.378	7.50	12.17

ID = 3 (8262):	1055.27	6.905	7.25	12.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8260)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (5506):	5660.52	21.378	11.75	13.46
+ ID2= 2 (8262):	1055.27	6.905	7.25	12.86

ID = 3 (8260):	6715.79	24.511	11.25	13.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8258)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0408):	231.62	2.048	6.75	13.51
+ ID2= 2 (8260):	6715.79	24.511	11.25	13.37

ID = 3 (8258):	6947.41	25.154	11.00	13.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8256)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0410):	572.01	1.949	8.25	9.94
+ ID2= 2 (8258):	6947.41	25.154	11.00	13.37

ID = 3 (8256):	7519.42	26.740	10.75	13.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5507)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2---> OUT= 1 DT= 15.0 min	0.0000	0.0000	90.6140	67.8416
	39.6440	12.3348	*****	160.3529
	48.1390	18.5023	*****	160.3629
	67.9600	37.0045	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (8256)	7519.421	26.740	10.75	13.11
OUTFLOW: ID= 1 (5507)	7519.421	26.163	11.75	13.11

PEAK FLOW REDUCTION [Qout/Qin] (%) = 97.84
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 8.1438

CALIB NASHYD (0420)	Area (ha)	(ha) = 175.82	Curve Number (CN) = 53.0
ID= 1 DT=15.0 min	Ia (mm) = 5.00		# of Linear Res. (N) = 1.50
	U. H. Tp (hrs) = 0.81		

Unit Hyd Opeak (cms) = 3.692
 PEAK FLOW (cms) = 1.097 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 11.603
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.185

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (4141)	Area Total (ha)	(ha) = 156.72	Dir. Conn. (%) = 21.00
ID= 1 DT=15.0 min	Imp (%) = 52.00		

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area (ha) =	81.49	75.23
Dep. Storage (mm) =	2.00	5.00
Average Slope (%) =	0.50	0.50
Length (m) =	1022.15	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr) =	82.76	65.84
over (min) =	15.00	30.00
Storage Coeff. (min) =	13.68 (ii)	26.33 (ii)
Unit Hyd. Tpeak (min) =	15.00	30.00
Unit Hyd. peak (cms) =	0.08	0.04

	(cms)	(hrs)	(mm)	(mm)
PEAK FLOW (cms) =	5.72	6.11	9.590	*TOTALS*
TIME TO PEAK (hrs) =	6.00	6.25	6.00	
RUNOFF VOLUME (mm) =	60.70	26.10	33.37	
TOTAL RAINFALL (mm) =	62.70	62.70	62.70	
RUNOFF COEFFICIENT =	0.97	0.42	0.53	

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 66.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (4144)	Area Total (ha)	(ha) = 2.10	Dir. Conn. (%) = 10.00
ID= 1 DT=15.0 min	Imp (%) = 30.00		

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area (ha) =	0.63	1.47
Dep. Storage (mm) =	2.00	5.00
Average Slope (%) =	0.50	0.50
Length (m) =	118.32	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr) =	82.76	62.25
over (min) =	15.00	30.00
Storage Coeff. (min) =	3.75 (ii)	16.68 (ii)
Unit Hyd. Tpeak (min) =	15.00	30.00
Unit Hyd. peak (cms) =	0.11	0.05

	(cms)	(hrs)	(mm)	(mm)
PEAK FLOW (cms) =	0.05	0.14	0.146	*TOTALS*
TIME TO PEAK (hrs) =	6.00	6.25	6.25	
RUNOFF VOLUME (mm) =	60.70	31.07	34.03	
TOTAL RAINFALL (mm) =	62.70	62.70	62.70	
RUNOFF COEFFICIENT =	0.97	0.50	0.54	

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9429)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2---> OUT= 1 DT= 15.0 min	0.0000	0.0000	0.1000	0.0386
	0.0100	0.0195	0.1400	0.0480
	0.0400	0.0218	0.1700	0.0569
	0.0700	0.0312	0.2000	0.0655

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (4144)	2.100	0.146	6.25	34.03
OUTFLOW: ID= 1 (9429)	2.100	0.067	6.75	33.81

PEAK FLOW REDUCTION [Qout/Qin] (%) = 45.71
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 0.0307

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4141):	156.72	9.590	6.00	33.37
+ ID2= 2 (9429):	2.10	0.067	6.75	33.81
=====				
ID = 3 (9424):	158.82	9.598	6.00	33.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5414) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	4.7060	4.4896
	0.1870	1.6651	5.4940	5.4773
	2.0210	2.0447	6.0800	6.4738
	2.9590	2.4658	6.4800	6.4838
	3.7490	3.5575	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9424)	158.820	9.598	6.00	33.37
OUTFLOW: ID= 1 (5414)	158.820	3.108	7.00	33.36

PEAK FLOW REDUCTION [Qout/Qin] (%) = 32.38
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 2.6777

CALIB STANDHYD (4142) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	Di r. Conn. (%)=
	29.14	52.00	21.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	15.15	13.99
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	440.76	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	82.76	65.84
over (min)	15.00	30.00
Storage Coeff. (min)=	8.26 (ii)	20.91 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.05

PEAK FLOW	(cms)= 1.25	1.27	*TOTALS*
TIME TO PEAK	(hrs)= 6.00	6.25	2.062 (iii)
RUNOFF VOLUME	(mm)= 60.70	26.10	33.37
TOTAL RAINFALL	(mm)= 62.70	62.70	62.70
RUNOFF COEFFICIENT	= 0.97	0.42	0.53

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 66.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4142):	29.14	2.062	6.00	33.37
+ ID2= 2 (5414):	158.82	3.108	7.00	33.36
=====				
ID = 3 (8254):	187.96	3.944	6.50	33.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (4121) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	Di r. Conn. (%)=
	202.00	45.00	16.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	90.90	111.10
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1160.46	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	82.76	22.77
over (min)	15.00	45.00
Storage Coeff. (min)=	14.77 (ii)	34.10 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.07	0.03

PEAK FLOW	(cms)= 5.45	4.09	*TOTALS*
TIME TO PEAK	(hrs)= 6.00	6.50	7.137 (iii)
RUNOFF VOLUME	(mm)= 60.70	16.35	23.44
TOTAL RAINFALL	(mm)= 62.70	62.70	62.70
RUNOFF COEFFICIENT	= 0.97	0.26	0.37

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 51.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5412) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	6.5000	1.0500
	0.3860	0.5085	7.9000	1.1500
	1.8460	0.6103	9.3000	1.2500
	3.5050	0.7566	9.7000	1.2600
	4.7890	0.8929	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (4121)	202.000	7.137	6.00	23.44
OUTFLOW: ID= 1 (5412)	202.000	5.475	6.50	23.44

PEAK FLOW REDUCTION [Qout/Qin] (%) = 76.71
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 0.9652

CALIB STANDHYD (4122) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	Di r. Conn. (%)=
	36.70	63.00	22.00

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	23.12	13.58
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	494.64	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	82.76	66.55
over (min)	15.00	30.00
Storage Coeff. (min)=	8.85 (ii)	21.44 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.05

PEAK FLOW	(cms)= 1.62	1.23	*TOTALS*
TIME TO PEAK	(hrs)= 6.00	6.25	2.398 (iii)
RUNOFF VOLUME	(mm)= 60.70	20.94	29.69
TOTAL RAINFALL	(mm)= 62.70	62.70	62.70
RUNOFF COEFFICIENT	= 0.97	0.33	0.47

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

- (ii) CN* = 51.5 Ia = Dep. Storage (Above)
TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4122):	36.70	2.398	6.00	29.69
+ ID2= 2 (5412):	202.00	5.475	6.50	23.44
=====				
ID = 3 (9412):	238.70	6.862	6.25	24.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8254):	187.96	3.944	6.50	33.36
+ ID2= 2 (9412):	238.70	6.862	6.25	24.40
=====				
ID = 3 (8252):	426.66	10.546	6.50	28.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6416)
IN= 2---> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (4161.0) ----->

Distance	Elevation	Manning	
0.00	270.07	0.0340	
20.67	267.91	0.0340	
62.01	264.33	0.0340	
113.69	259.75	0.0340	
165.37	253.30	0.0340	
227.38	246.29	0.0340	
232.55	246.03	0.0340	
237.72	246.16	0.0340	
241.39	246.02	0.0340 / 0.0300	Main Channel
241.64	245.75	0.0300	Main Channel
242.89	245.75	0.0300	Main Channel
244.14	245.75	0.0300	Main Channel
244.39	246.02	0.0300 / 0.0340	Main Channel
248.06	246.20	0.0340	
253.22	246.28	0.0340	
258.39	246.63	0.0340	
346.25	252.57	0.0340	
413.43	257.77	0.0340	
465.11	261.78	0.0340	
511.62	270.00	0.0340	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.27	246.02	.295E+04	0.6	0.75	88.26
1.53	247.28	.168E+06	69.7	1.65	40.13
2.79	248.54	.490E+06	299.5	2.42	27.28
4.06	249.81	.962E+06	740.1	3.05	21.67
5.32	251.07	.158E+07	1440.0	3.61	18.33
6.58	252.33	.235E+07	2443.8	4.12	16.06
7.84	253.59	.327E+07	3811.2	4.63	14.30
9.10	254.85	.432E+07	5565.1	5.11	12.94
10.37	256.12	.550E+07	7721.3	5.57	11.88
11.63	257.38	.682E+07	10309.9	6.00	11.02
12.89	258.64	.827E+07	13360.6	6.42	10.31
14.15	259.90	.985E+07	16884.5	6.81	9.72
15.42	261.17	.116E+08	20818.4	7.14	9.26
16.68	262.43	.134E+08	25510.4	7.53	8.78
17.94	263.69	.154E+08	30958.0	7.97	8.30
19.20	264.95	.175E+08	36988.3	8.38	7.89
20.46	266.21	.197E+08	43617.1	8.78	7.53
21.73	267.48	.220E+08	50868.1	9.17	7.22
22.99	268.74	.244E+08	58854.0	9.56	6.92

<----- hydrograph -----> <-pi pe / channel ->
AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL

	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8252)	426.66	10.55	6.50	28.35	0.45	0.81
OUTFLOW: ID= 1 (6416)	426.66	5.97	7.50	28.35	0.37	0.78

CALIB NASHYD (4164) ID= 1 DT=15.0 mi n	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
	89.30	76.0	3.00
	Ia (mm)= 5.00		
	U. H. Tp(hrs)= 1.46		

Unit Hyd Opeak (cms)= 2.344

PEAK FLOW (cms)= 1.510 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 24.139
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.385

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9427) IN= 2---> OUT= 1 DT= 15.0 mi n	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	4.4700	2.5000
	0.9000	1.0000	5.9800	3.0000
	1.7300	1.5000	7.5000	3.5000
	3.2200	2.0000	8.7400	4.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (4164)	89.300	1.510	7.50	24.14
OUTFLOW: ID= 1 (9427)	89.300	0.794	9.50	24.14

PEAK FLOW REDUCTION [Qout/Qi n](%) = 52.60
TIME SHIFT OF PEAK FLOW (mi n)=120.00
MAXIMUM STORAGE USED (ha. m.) = 0.8830

RESERVOIR (5416) IN= 2---> OUT= 1 DT= 15.0 mi n	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	1.5980	0.7565
	0.0330	0.3532	1.8570	0.8137
	0.7450	0.4481	2.0990	0.8707
	1.0270	0.5154	2.4990	0.8807
	1.3050	0.6350	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9427)	89.300	0.794	9.50	24.14
OUTFLOW: ID= 1 (5416)	89.300	0.769	10.25	24.11

PEAK FLOW REDUCTION [Qout/Qi n](%) = 96.77
TIME SHIFT OF PEAK FLOW (mi n)= 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.4539

CALIB NASHYD (4162) ID= 1 DT=15.0 mi n	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
	349.99	61.3	1.50
	Ia (mm)= 5.00		
	U. H. Tp(hrs)= 1.29		

Unit Hyd Opeak (cms)= 4.646

PEAK FLOW (cms)= 2.025 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 15.161
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.242

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9416)

1 + 2 = 3		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4162):		349.99	2.025	8.00	15.16
+ ID2= 2 (5416):		89.30	0.769	10.25	24.11

ID = 3 (9416):		439.29	2.523	9.50	16.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD. (8250) 1 + 2 = 3		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6416):		426.66	5.973	7.50	28.35
+ ID2= 2 (9416):		439.29	2.523	9.50	16.98

ID = 3 (8250):		865.95	7.974	7.50	22.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0418) ID= 1 DT=15.0 min		Area (ha)=	Curve Number (CN)=
		174.09	64.0
		5.00	# of Linear Res. (N)= 1.50
		1.05	

Unit Hyd Opeak (cms)= 2.825

PEAK FLOW (cms)= 1.272 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 16.442
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.262

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (4184) ID= 1 DT=15.0 min		Area (ha)=	Dir. Conn. (%) =
		8.70	10.00
		30.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	2.61	6.09
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	240.83	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 82.76 over (min)= 15.00
 Storage Coeff. (min)= 5.75 (ii)
 Unit Hyd. Tpeak (min)= 15.00
 Unit Hyd. peak (cms)= 0.10

TOTALS
 PEAK FLOW (cms)= 0.19
 TIME TO PEAK (hrs)= 6.00
 RUNOFF VOLUME (mm)= 60.70
 TOTAL RAINFALL (mm)= 62.70
 RUNOFF COEFFICIENT = 0.97

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 73.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9428) IN= 2--> OUT= 1 DT= 15.0 min		OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
		0.0000	0.0000	0.4300	0.1618
		0.0300	0.0818	0.5800	0.2012
		0.1700	0.0912	0.7300	0.2384
		0.3100	0.1306	0.8500	0.2747

INFLOW : ID= 2 (4184) OUTFLOW: ID= 1 (9428)		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
		8.700	0.488	6.25	29.67
		8.700	0.239	6.75	29.60

PEAK FLOW REDUCTION [Qout/Qin] (%) = 49.04
 TIME SHIF OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 0.1119

ADD HYD. (9423) 1 + 2 = 3		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0418):		174.09	1.272	7.50	16.44
+ ID2= 2 (9428):		8.70	0.239	6.75	29.60

ID = 3 (9423):		182.79	1.467	7.00	17.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD. (8248) 1 + 2 = 3		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8250):		865.95	7.974	7.50	22.58
+ ID2= 2 (9423):		182.79	1.467	7.00	17.07

ID = 3 (8248):		1048.74	9.403	7.50	21.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD. (8246) 1 + 2 = 3		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0420):		175.82	1.097	7.00	11.60
+ ID2= 2 (8248):		1048.74	9.403	7.50	21.62

ID = 3 (8246):		1224.56	10.469	7.50	20.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD. (8244) 1 + 2 = 3		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (5507):		7519.42	26.163	11.75	13.11
+ ID2= 2 (8246):		1224.56	10.469	7.50	20.18

ID = 3 (8244):		8743.98	31.588	10.75	14.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (4227) ID= 1 DT=15.0 min		Area (ha)=	Dir. Conn. (%) =
		66.80	15.00
		29.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	19.37	47.43
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	667.33	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)= 82.76 over (min)= 15.48
 Storage Coeff. (min)= 10.60 (ii)
 Unit Hyd. Tpeak (min)= 15.00
 Unit Hyd. peak (cms)= 0.09

TOTALS
 PEAK FLOW (cms)= 1.91
 TIME TO PEAK (hrs)= 6.00
 RUNOFF VOLUME (mm)= 60.70

TOTAL RAINFALL (mm) = 62.70
 RUNOFF COEFFICIENT = 0.97

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 54.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9426)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.3400	1.2303
0.2300	0.6171	4.4700	1.5350
1.3000	0.6860	5.6100	1.8228
2.4100	0.9909	6.5300	2.1041

INFLOW : ID= 2 (4227) 66.800
 OUTFLOW: ID= 1 (9426) 66.800

PEAK FLOW REDUCTION [Oout/Oin] (%) = 42.73
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 0.6729

RESERVOIR (5422)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.3110	2.6782
0.2180	1.7874	5.4130	2.9186
1.2490	1.9110	6.3010	3.1540
2.3250	2.1760	6.7010	3.1640
3.2250	2.4038	0.0000	0.0000

INFLOW : ID= 2 (9426) 66.800
 OUTFLOW: ID= 1 (5422) 66.800

PEAK FLOW REDUCTION [Oout/Oin] (%) = 9.79
 TIME SHIFT OF PEAK FLOW (min) = 660.00
 MAXIMUM STORAGE USED (ha. m.) = 0.8236

CALIB NASHYD (4222)
 ID= 1 DT=15.0 min

Area (ha) = 713.41
 Ia (mm) = 5.00
 U. H. Tp (hrs) = 1.95
 Curve Number (CN) = 54.0
 # of Linear Res. (N) = 1.50

Unit Hyd Qpeak (cms) = 6.252

PEAK FLOW (cms) = 2.370 (i)
 TIME TO PEAK (hrs) = 9.250
 RUNOFF VOLUME (mm) = 12.102
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.193

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9422)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4222): 713.41	2.370	9.25	12.10
+ ID2= 2 (5422): 66.80	0.100	18.00	21.23
ID = 3 (9422): 780.21	2.438	9.25	12.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8242)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8244): 8743.98	31.588	10.75	14.10
+ ID2= 2 (9422): 780.21	2.438	9.25	12.88
ID = 3 (8242): 9524.19	33.916	10.75	14.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5508)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	*****	197.3574
76.4550	30.8371	*****	394.7148
*****	61.6742	*****	394.7248
*****	*****	0.0000	0.0000

INFLOW : ID= 2 (8242) 9524.190
 OUTFLOW: ID= 1 (5508) 9524.190

PEAK FLOW REDUCTION [Oout/Oin] (%) = 96.96
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha. m.) = 13.2662

CALIB NASHYD (0336)
 ID= 1 DT=15.0 min

Area (ha) = 2785.00
 Ia (mm) = 5.00
 U. H. Tp (hrs) = 15.39
 Curve Number (CN) = 72.0
 # of Linear Res. (N) = 1.50

Unit Hyd Qpeak (cms) = 3.090

PEAK FLOW (cms) = 2.580 (i)
 TIME TO PEAK (hrs) = 22.250
 RUNOFF VOLUME (mm) = 21.246
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.339

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3382)
 ID= 1 DT=15.0 min

Area (ha) = 433.29
 Total Imp (%) = 39.00
 Di r. Conn. (%) = 20.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 168.98 264.31
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 1699.59 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 82.76 25.93
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 18.57 (ii) 36.92 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.06 0.03

TOTALS

PEAK FLOW (cms) = 13.16 10.57 17.576 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 60.70 21.19 29.09
 TOTAL RAINFALL (mm) = 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.34 0.46

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 64.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (3381)
ID= 1 DT=15.0 min

Area (ha)= 165.62
Total Imp(%)= 53.00 Dir. Conn.(%)= 27.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	87.78	77.84
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1050.78	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	82.76	56.72
over (min)	15.00	30.00
Storage Coeff. (min)=	13.91 (ii)	27.33 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.07	0.04

	TOTALS	
PEAK FLOW (cms)=	7.73	5.33
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	60.70	23.92
TOTAL RAINFALL (mm)=	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.38

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5338)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	9.5300	3.8529
0.6370	1.6350	12.1350	4.2725
2.7300	2.1009	14.3850	4.6808
5.1010	2.7445	14.7850	4.6908
7.0460	3.2936	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (3381)	165.620	11.067	6.00	33.85
OUTFLOW: ID= 1 (5338)	165.620	4.369	6.75	33.84

PEAK FLOW REDUCTION [Qout/Qin] (%) = 39.48
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 2.5572

ADD HYD (8310)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3382):	433.29	17.576	6.00	29.09
+ ID2= 2 (5338):	165.62	4.369	6.75	33.84
=====				
ID = 3 (8310):	598.91	19.443	6.50	30.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHYD (3342)
ID= 1 DT=15.0 min

Area (ha)= 586.87
Ia (mm)= 5.00
U. H. Tp(hrs)= 8.20

Curve Number (CN)= 56.0
of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 1.221

PEAK FLOW (cms)=	0.615 (i)
TIME TO PEAK (hrs)=	15.500
RUNOFF VOLUME (mm)=	12.935
TOTAL RAINFALL (mm)=	62.700
RUNOFF COEFFICIENT =	0.206

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (3341)
ID= 1 DT=15.0 min

Area (ha)= 33.23
Total Imp(%)= 51.00 Dir. Conn.(%)= 21.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	16.95	16.28
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	470.67	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	82.76	25.50
over (min)	15.00	30.00
Storage Coeff. (min)=	8.59 (ii)	27.07 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.04

	TOTALS	
PEAK FLOW (cms)=	1.41	0.82
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	60.70	17.28
TOTAL RAINFALL (mm)=	62.70	62.70
RUNOFF COEFFICIENT =	0.97	0.28

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 51.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5334)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.6690	0.4664
0.0400	0.1927	0.8520	0.5318
0.1900	0.2643	1.0360	0.5973
0.3830	0.3576	1.4360	0.6073
0.4460	0.3863	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (3341)	33.230	1.919	6.00	26.39
OUTFLOW: ID= 1 (5334)	33.230	0.558	6.75	26.36

PEAK FLOW REDUCTION [Qout/Qin] (%) = 29.08
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.4280

ADD HYD (8314)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3342):	586.87	0.615	15.50	12.94
+ ID2= 2 (5334):	33.23	0.558	6.75	26.36
=====				
ID = 3 (8314):	620.10	0.793	7.00	13.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHYD (0306)
ID= 1 DT=15.0 min

Area (ha)= 283.97
Ia (mm)= 5.00
U. H. Tp(hrs)= 6.44

Curve Number (CN)= 52.0
of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 0.753

PEAK FLOW (cms)=	0.331 (i)
TIME TO PEAK (hrs)=	13.750
RUNOFF VOLUME (mm)=	11.388
TOTAL RAINFALL (mm)=	62.700
RUNOFF COEFFICIENT =	0.182

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0282)
 ID= 1 DT=15.0 min
 Area (ha)= 449.38
 Curve Number (CN)= 77.0
 U. H. Tp(hrs)= 1.47
 # of Linear Res. (N)= 1.50

Unit Hyd Qpeak (cms)= 5.226
 PEAK FLOW (cms)= 3.907 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 24.782
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.395

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0284)
 ID= 1 DT=15.0 min
 Area (ha)= 78.93
 Curve Number (CN)= 84.0
 U. H. Tp(hrs)= 0.57
 # of Linear Res. (N)= 1.50

Unit Hyd Qpeak (cms)= 2.344
 PEAK FLOW (cms)= 1.836 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 30.659
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.489

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8388)
 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 ID1= 1 (0282): 449.38 3.907 8.00 24.78
 + ID2= 2 (0284): 78.93 1.836 6.50 30.66
 ID = 3 (8388): 528.31 5.316 7.25 25.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6286)
 IN= 2---> OUT= 1
 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2861.0) ----->

Distance	Elevation	Manning	
0.00	233.00	0.0450	
20.58	228.51	0.0450	
41.17	227.74	0.0450	
51.46	227.41	0.0450	
97.77	225.96	0.0450	
149.23	223.94	0.0450	
200.69	220.84	0.0450	
226.42	220.66	0.0450	
238.85	220.22	0.0450	
241.35	220.01	0.0450 / 0.0350	Main Channel
241.85	219.70	0.0350	Main Channel
245.85	219.72	0.0350	Main Channel
246.35	220.06	0.0350	Main Channel
248.85	220.23	0.0350	Main Channel
303.60	221.64	0.0350	Main Channel
380.79	224.98	0.0450	
432.25	229.54	0.0450	
457.98	233.33	0.0450	
483.71	234.27	0.0450	
509.44	233.81	0.0450	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.31	220.01	.382E+04	0.8	0.57	82.89
1.06	220.76	.525E+05	12.1	0.66	72.51
1.81	221.51	.240E+06	88.6	1.05	45.21
2.56	222.26	.510E+06	261.7	1.46	32.47
3.31	223.01	.843E+06	533.8	1.81	26.34
4.06	223.76	.124E+07	913.6	2.10	22.64

4.81	224.51	.171E+07	1399.0	2.34	20.33
5.56	225.26	.225E+07	2051.0	2.60	18.27
6.31	226.01	.285E+07	2907.0	2.91	16.35
7.06	226.76	.352E+07	3902.5	3.16	15.04
7.82	227.52	.426E+07	5069.9	3.39	14.01
8.57	228.27	.507E+07	6433.0	3.62	13.13
9.32	229.02	.593E+07	8069.2	3.89	12.24
10.07	229.77	.681E+07	9926.7	4.16	11.44
10.82	230.52	.772E+07	11962.8	4.42	10.75
11.57	231.27	.864E+07	14167.1	4.68	10.17
12.32	232.02	.959E+07	16537.1	4.92	9.66
13.07	232.77	.105E+08	19070.7	5.16	9.22
13.82	233.52	.115E+08	21790.4	5.39	8.82

<---- hydrograph ----> <-pi pe / channel -->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8388) 528.31 5.32 7.25 25.66 0.61 0.60
 OUTFLOW: ID= 1 (6286) 528.31 4.49 9.00 25.66 0.56 0.60

CALIB STANDHYD (2861)
 ID= 1 DT=15.0 min
 Area Total (ha)= 54.86
 Imp(%)= 45.00
 Di r. Conn. (%)= 19.00

IMPERVIOUS PERVERIOUS (i)
 Surface Area (ha)= 24.69 30.17
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 604.76 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 82.76 76.69
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 9.99 (ii) 21.88 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.05

TOTALS
 PEAK FLOW (cms)= 2.02 3.15 4.075 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 60.70 33.45 38.63
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.53 0.62

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVERIOUS LOSSES:
 CN* = 79.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5286)
 IN= 2---> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.6700	1.4285
0.1860	0.6703	4.6080	1.6332
1.0630	0.7754	5.3640	1.8336
1.9790	1.0010	5.7640	1.8436
2.7460	1.1950	0.0000	0.0000

AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (2861) 54.860 4.075 6.00 38.63
 OUTFLOW: ID= 1 (5286) 54.860 1.834 6.75 38.61

PEAK FLOW REDUCTION [Qout/Qin] (%)= 45.02
 TIME SHIFT OF PEAK FLOW (mi n)= 45.00
 MAXIMUM STORAGE USED (ha. m.) = 0.9733

CALIB STANDHYD (2862)
 ID= 1 DT=15.0 min
 Area Total (ha)= 205.65
 Imp(%)= 22.00
 Di r. Conn. (%)= 10.00

IMPERVIOUS PERVERIOUS (i)

Surface Area (ha)= 45.24 160.41
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1170.90 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 82.76 52.41
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 14.85 (ii) 28.70 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.07 0.04

TOTALS
 PEAK FLOW (cms)= 3.46 9.91 11.580 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.25
 RUNOFF VOLUME (mm)= 60.70 29.15 32.30
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.46 0.52

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD. (9286)					
1 + 2 = 3					
	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (2862):	205.65	11.580	6.25	32.30	
+ ID2= 2 (5286):	54.86	1.834	6.75	38.61	
=====					
ID = 3 (9286):	260.51	12.654	6.25	33.63	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD. (8386)					
1 + 2 = 3					
	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (6286):	528.31	4.493	9.00	25.66	
+ ID2= 2 (9286):	260.51	12.654	6.25	33.63	
=====					
ID = 3 (8386):	788.82	13.743	6.25	28.29	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0302) Area (ha)= 473.90 Curve Number (CN)= 58.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.66

Unit Hyd Qpeak (cms)= 4.874
 PEAK FLOW (cms)= 2.024 (i)
 TIME TO PEAK (hrs)= 8.500
 RUNOFF VOLUME (mm)= 13.712
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.219

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0298) Area (ha)= 330.51 Curve Number (CN)= 45.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.26

Unit Hyd Qpeak (cms)= 4.461
 PEAK FLOW (cms)= 1.133 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 8.978
 TOTAL RAINFALL (mm)= 62.700

RUNOFF COEFFICIENT = 0.143

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (3001) Area (ha)= 0.09
 ID= 1 DT=15.0 min Total Imp(%)= 45.00 Dir. Conn.(%)= 15.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.04 0.05
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 24.49 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 82.76 20.12
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 1.46 (ii) 21.77 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.11 0.05

TOTALS
 PEAK FLOW (cms)= 0.00 0.00 0.004 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 60.70 14.40 21.16
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.23 0.34

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 46.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5300)					
IN= 2----> OUT= 1					
DT= 15.0 min					
	OUTFLOW	STORAGE	OUTFLOW	STORAGE	
	(cms)	(ha.m.)	(cms)	(ha.m.)	
	0.0000	0.0000	0.0060	0.0023	
	0.0020	0.0012	0.0070	0.0026	
	0.0030	0.0016	0.0090	0.0029	
	0.0040	0.0019	0.0090	0.0129	
=====					
	0.0040	0.0019	0.0090	0.0129	
	0.0090	0.004	6.00	21.16	
INFLOW : ID= 2 (3001)	0.090	0.004	6.00	21.16	
OUTFLOW: ID= 1 (5300)	0.090	0.001	6.75	19.66	

PEAK FLOW REDUCTION [Qout/Oin](%)= 28.78
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 0.0008

CALIB NASHYD (3002) Area (ha)= 258.84 Curve Number (CN)= 51.8
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.03

Unit Hyd Qpeak (cms)= 4.290
 PEAK FLOW (cms)= 1.293 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 11.212
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.179

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD. (9300)					
1 + 2 = 3					
	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (3002):	258.84	1.293	7.50	11.21	
+ ID2= 2 (5300):	0.09	0.001	6.75	19.66	

 ID = 3 (9300): 258.93 1.294 7.50 11.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8395)
 1 + 2 = 3

 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0298): 330.51 1.133 8.00 8.98
 + ID2= 2 (9300): 258.93 1.294 7.50 11.21

 ID = 3 (8395): 589.44 2.414 7.75 9.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB NASHYD (0288) Area (ha)= 340.83 Curve Number (CN)= 78.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 2.21

Unit Hyd Qpeak (cms) = 2.629

PEAK FLOW (cms) = 2.210 (i)
 TIME TO PEAK (hrs) = 9.500
 RUNOFF VOLUME (mm) = 25.660
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.409

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB NASHYD (0290) Area (ha)= 269.18 Curve Number (CN)= 78.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms) = 4.279

PEAK FLOW (cms) = 3.070 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 25.504
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.407

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8397)
 1 + 2 = 3

 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0288): 340.83 2.210 9.50 25.66
 + ID2= 2 (0290): 269.18 3.070 7.50 25.50

 ID = 3 (8397): 610.01 5.105 8.00 25.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB NASHYD (0296) Area (ha)= 293.65 Curve Number (CN)= 76.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 1.13

Unit Hyd Qpeak (cms) = 4.437

PEAK FLOW (cms) = 3.009 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 23.935
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.382

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB NASHYD (0292) Area (ha)= 738.49 Curve Number (CN)= 68.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 1.52

Unit Hyd Qpeak (cms) = 8.289

PEAK FLOW (cms) = 4.648 (i)
 TIME TO PEAK (hrs) = 8.250
 RUNOFF VOLUME (mm) = 18.682
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.298

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB NASHYD (0294) Area (ha)= 274.15 Curve Number (CN)= 57.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.87

Unit Hyd Qpeak (cms) = 5.367

PEAK FLOW (cms) = 1.843 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 13.187
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.210

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8398)
 1 + 2 = 3

 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0292): 738.49 4.648 8.25 18.68
 + ID2= 2 (0294): 274.15 1.843 7.00 13.19

 ID = 3 (8398): 1012.64 6.373 8.00 17.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6296) | Routing time step (min)' = 15.00
 IN= 2---> OUT= 1

<----- DATA FOR SECTION (2961.0) ----->

Distance	Elevation	Manning	
0.00	243.98	0.0400	
42.59	243.18	0.0400	
85.17	241.81	0.0400	
120.66	240.50	0.0400	
156.15	239.56	0.0400	
198.74	236.15	0.0400	
237.78	234.01	0.0400	
241.33	233.82	0.0400	
248.77	233.12	0.0400 / 0.0400	Main Channel
249.87	232.32	0.0400	Main Channel
250.37	231.80	0.0400	Main Channel
250.87	232.23	0.0400	Main Channel
251.97	233.10	0.0400 / 0.0400	Main Channel
255.37	233.22	0.0400	
259.07	233.87	0.0400	
262.62	234.12	0.0400	
266.17	234.23	0.0400	
283.91	234.73	0.0400	
337.15	241.75	0.0400	
351.34	244.00	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.43	232.23	.510E+03	0.1	0.53	81.07
0.87	232.67	.213E+04	0.7	0.83	51.49
1.30	233.10	.501E+04	2.2	1.10	38.75
1.98	233.78	.242E+05	12.6	1.34	31.93
2.66	234.46	.738E+05	42.1	1.46	29.21
3.34	235.14	.177E+06	127.9	1.84	23.13
4.02	235.82	.314E+06	281.4	2.29	18.61
4.70	236.50	.481E+06	509.4	2.71	15.73

5.38	237.18	.672E+06	815.6	3.11	13.73
6.06	237.86	.886E+06	1200.7	3.46	12.30
6.74	238.54	.112E+07	1669.4	3.80	11.23
7.42	239.22	.139E+07	2226.9	4.11	10.38
8.10	239.90	.168E+07	2808.4	4.28	9.95
8.78	240.58	.202E+07	3470.4	4.40	9.68
9.46	241.26	.240E+07	4331.7	4.61	9.24
10.14	241.94	.283E+07	5335.2	4.83	8.84
10.82	242.62	.330E+07	6482.6	5.03	8.48
11.50	243.30	.381E+07	7764.1	5.21	8.18
12.18	243.98	.439E+07	9101.2	5.31	8.04

	<--- hydrograph --->				<-pi pe / channel-->	
	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8398)	1012.64	6.37	8.00	17.19	1.57	1.18
OUTFLOW: ID= 1 (6296)	1012.64	6.16	8.50	17.19	1.56	1.18

ADD HYD (8396)					
1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0296):	293.65	3.009	7.50	23.94	
+ ID2= 2 (6296):	1012.64	6.164	8.50	17.19	
=====					
ID = 3 (8396):	1306.29	9.032	8.25	18.71	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8394)					
1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (8396):	1306.29	9.032	8.25	18.71	
+ ID2= 2 (8397):	610.01	5.105	8.00	25.59	
=====					
ID = 3 (8394):	1916.30	14.100	8.25	20.90	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8392)					
1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (8394):	1916.30	14.100	8.25	20.90	
+ ID2= 2 (8395):	589.44	2.414	7.75	9.96	
=====					
ID = 3 (8392):	2505.74	16.488	8.00	18.33	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6302)
IN= 2--> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3021.0) ----->					
Distance	Elevation	Manning			
0.00	228.10	0.0400			
18.47	227.12	0.0400			
36.95	226.12	0.0400			
46.18	225.84	0.0400			
55.42	225.58	0.0400			
272.47	222.88	0.0400			
277.09	222.76	0.0400			
281.71	222.58	0.0400			
288.54	222.18	0.0400 / 0.0300	Main	Channel	
288.64	221.00	0.0300	Main	Channel	
290.94	221.00	0.0300	Main	Channel	
291.04	221.00	0.0300	Main	Channel	
291.54	222.75	0.0300 / 0.0400	Main	Channel	
300.18	222.83	0.0400			
304.80	223.04	0.0400			
309.42	223.25	0.0400			
318.65	223.69	0.0400			
360.22	225.57	0.0400			
397.16	227.60	0.0400			

457.20 228.35 0.0400

<----- TRAVEL TIME TABLE ----->						
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME	
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)	
0.29	221.29	.228E+04	0.2	0.32	165.63	
0.59	221.59	.465E+04	0.7	0.45	116.15	
0.88	221.88	.713E+04	1.2	0.54	96.87	
1.18	222.18	.971E+04	1.9	0.61	86.20	
1.57	222.57	.175E+05	3.3	0.59	88.86	
1.97	222.97	.427E+05	6.4	0.47	111.39	
2.36	223.36	.115E+06	16.1	0.44	119.14	
2.76	223.76	.237E+06	37.1	0.49	106.49	
3.15	224.15	.409E+06	72.9	0.56	93.40	
3.55	224.55	.631E+06	127.0	0.63	82.76	
3.94	224.94	.903E+06	202.4	0.70	74.37	
4.34	225.34	.123E+07	301.8	0.77	67.68	
4.73	225.73	.160E+07	433.5	0.85	61.37	
5.13	226.13	.200E+07	602.2	0.95	55.29	
5.52	226.52	.242E+07	804.7	1.05	50.13	
5.92	226.92	.286E+07	1034.2	1.14	46.11	
6.31	227.31	.332E+07	1290.5	1.22	42.88	
6.71	227.71	.380E+07	1560.9	1.29	40.55	
7.10	228.10	.432E+07	1835.6	1.34	39.19	

	<--- hydrograph --->				<-pi pe / channel-->	
	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8392)	2505.74	16.49	8.00	18.33	2.37	0.44
OUTFLOW: ID= 1 (6302)	2505.74	13.11	10.50	18.33	2.24	0.45

ADD HYD (8390)					
1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0302):	473.90	2.024	8.50	13.71	
+ ID2= 2 (6302):	2505.74	13.114	10.50	18.33	
=====					
ID = 3 (8390):	2979.64	14.975	10.25	17.59	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8348)					
1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (8386):	788.82	13.743	6.25	28.29	
+ ID2= 2 (8390):	2979.64	14.975	10.25	17.59	
=====					
ID = 3 (8348):	3768.46	20.547	10.00	19.83	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0304)
ID= 1 DT=15.0 min

Area (ha)= 292.37
Curve Number (CN)= 63.0
U. H. Tp(hrs)= 2.78
of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 1.793

PEAK FLOW (cms)= 0.987 (i)
TIME TO PEAK (hrs)= 10.500
RUNOFF VOLUME (mm)= 16.057
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.256

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0280)
ID= 1 DT=15.0 min

Area (ha)= 299.86
Curve Number (CN)= 82.0
U. H. Tp(hrs)= 0.85
of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 6.009

PEAK FLOW (cms)= 4.745 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 28.965
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.462

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0278) | Area (ha)= 485.49 Curve Number (CN)= 82.0
 NASHYD | Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.52

Unit Hyd Qpeak (cms)= 5.453

PEAK FLOW (cms)= 4.873 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 29.184
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.465

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0276) | Area (ha)= 90.89 Curve Number (CN)= 79.0
 NASHYD | Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.67

Unit Hyd Qpeak (cms)= 2.302

PEAK FLOW (cms)= 1.552 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 26.101
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.416

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0274) | Area (ha)= 392.49 Curve Number (CN)= 75.0
 NASHYD | Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 1.08

Unit Hyd Qpeak (cms)= 6.182

PEAK FLOW (cms)= 4.016 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 23.174
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.370

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3 (8360)				
ID1= 1 (0274):	392.49	4.016	7.50	23.17
+ ID2= 2 (0276):	90.89	1.552	6.75	26.10
ID = 3 (8360):	483.38	5.470	7.25	23.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3 (8358)				
ID1= 1 (0278):	485.49	4.873	8.00	29.18
+ ID2= 2 (8360):	483.38	5.470	7.25	23.72
ID = 3 (8358):	968.87	10.143	7.50	26.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6280) | Routing time step (min)'= 15.00
 IN= 2--> OUT= 1

<----- DATA FOR SECTION (2801.0) ----->

Distance	Elevation	Manning
0.00	241.14	0.0500
13.32	240.80	0.0500
39.95	240.07	0.0500
96.54	236.21	0.0500
113.19	234.15	0.0500
123.18	232.35	0.0500
143.15	225.80	0.0500
149.81	225.62	0.0500
153.14	225.40	0.0500
157.30	224.76	0.0500 / 0.0300
159.80	224.26	0.0300
162.30	224.85	0.0300
162.55	224.97	0.0300 / 0.0500
167.80	225.05	0.0500
186.43	229.14	0.0500
213.06	234.75	0.0500
236.37	237.09	0.0500
266.33	237.31	0.0500
292.96	237.83	0.0500
329.58	241.50	0.0500

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	224.76	.263E+04	0.8	0.65	58.31
1.36	225.62	.278E+05	13.4	1.10	34.54
2.22	226.48	.855E+05	52.6	1.40	27.09
3.09	227.35	.157E+06	120.4	1.74	21.78
3.95	228.21	.242E+06	216.3	2.04	18.64
4.81	229.07	.340E+06	341.8	2.29	16.56
5.67	229.93	.450E+06	498.3	2.52	15.06
6.53	230.79	.574E+06	688.3	2.73	13.90
7.40	231.66	.711E+06	913.8	2.93	12.97
8.26	232.52	.861E+06	1173.4	3.11	12.23
9.12	233.38	.103E+07	1461.3	3.24	11.72
9.98	234.24	.121E+07	1793.3	3.37	11.26
10.85	235.11	.142E+07	2141.0	3.45	11.02
11.71	235.97	.165E+07	2530.4	3.50	10.86
12.57	236.83	.192E+07	2954.0	3.51	10.81
13.43	237.69	.226E+07	3160.3	3.18	11.93
14.29	238.55	.271E+07	3869.4	3.26	11.67
15.16	239.42	.320E+07	4740.3	3.38	11.24
16.02	240.28	.373E+07	5688.4	3.48	10.92

<---- hydrograph ----> <- pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8358)	968.87	10.14	7.50	26.46	1.14	0.93
OUTFLOW: ID= 1 (6280)	968.87	9.72	8.25	26.46	1.11	0.92

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3 (8354)				
ID1= 1 (0280):	299.86	4.745	7.00	28.96
+ ID2= 2 (6280):	968.87	9.721	8.25	26.46
ID = 3 (8354):	1268.73	13.898	8.00	27.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB (0270) | Area (ha)= 243.61 Curve Number (CN)= 81.0
 NASHYD | Ia (mm)= 5.00 # of Linear Res. (N)= 1.30
 ID= 1 DT=15.0 min | U.H. Tp(hrs)= 0.87

Unit Hyd Qpeak (cms)= 3.429

PEAK FLOW (cms)= 2.706 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 28.005

TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.447

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0268) Area (ha)= 215.76 Curve Number (CN)= 75.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.30
 U. H. Tp(hrs)= 0.69

Unit Hyd Opeak (cms)= 3.807

PEAK FLOW (cms)= 2.333 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 22.965
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.366

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0266) Area (ha)= 508.09 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.30
 U. H. Tp(hrs)= 1.63

Unit Hyd Opeak (cms)= 3.786

PEAK FLOW (cms)= 1.975 (i)
 TIME TO PEAK (hrs)= 9.250
 RUNOFF VOLUME (mm)= 16.500
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.263

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2641) Area (ha)= 17.58 Dir. Conn. (%)= 70.00
 ID= 1 DT=15.0 min Total Imp(%)= 75.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	13.18	4.39
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	342.34	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 82.76 over (min) 15.00 23.48 30.00
 Storage Coeff. (min)= 7.10 (ii) 26.20 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.10 0.04

TOTALS
 PEAK FLOW (cms)= 2.60 0.21 2.728 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 60.70 20.99 48.78
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.33 0.78

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5264) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	1.1760	0.5589
	0.0600	0.3133	1.4770	0.6054
	0.3410	0.3763	1.7190	0.6511
	0.6340	0.4336	2.1190	0.6611
	0.8800	0.4941	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2641)	17.580	2.728	6.00	48.78
OUTFLOW: ID= 1 (5264)	17.580	0.768	6.25	48.71

PEAK FLOW REDUCTION [Qout/Qin] (%) = 28.13
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 0.4706

CALIB NASHYD (2642) Area (ha)= 336.37 Curve Number (CN)= 69.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.30
 U. H. Tp(hrs)= 1.28

Unit Hyd Opeak (cms)= 3.192

PEAK FLOW (cms)= 1.852 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 19.220
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.307

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9264) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2642):	336.37	1.852	8.25	19.22
+ ID2= 2 (5264):	17.58	0.768	6.25	48.71
=====				
ID = 3 (9264):	353.95	2.253	6.75	20.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8380) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0266):	508.09	1.975	9.25	16.50
+ ID2= 2 (9264):	353.95	2.253	6.75	20.68
=====				
ID = 3 (8380):	862.04	4.011	8.00	18.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6268) Routing time step (min)'= 15.00
 IN= 2--> OUT= 1

<----- DATA FOR SECTION (2681.0) ----->			
Distance	Elevation	Manning	
0.00	274.03	0.0500	
10.55	272.73	0.0500	
21.10	271.28	0.0500	
50.12	266.83	0.0500	
87.04	260.36	0.0500	
92.32	260.06	0.0500	
94.95	259.93	0.0500	
101.00	256.87	0.0500	
103.00	256.58	0.0500 / 0.0350	Main Channel
104.00	256.32	0.0350	Main Channel
106.00	256.05	0.0350	Main Channel
108.00	256.25	0.0350	Main Channel
110.78	256.65	0.0350 / 0.0500	Main Channel
113.42	256.81	0.0500	
116.06	257.00	0.0500	
155.62	260.24	0.0500	
187.27	263.35	0.0500	
211.01	267.22	0.0500	
224.20	269.60	0.0500	
261.13	272.68	0.0500	

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.53	256.58	697E+04	2.1	0.97	55.64

1.38	257.43	.477E+05	25.9	1.76	30.66
2.22	258.27	.123E+06	84.3	2.22	24.26
3.07	259.12	.231E+06	185.4	2.60	20.72
3.92	259.97	.371E+06	335.8	2.92	18.42
4.77	260.82	.560E+06	547.6	3.15	17.06
5.61	261.66	.790E+06	845.6	3.46	15.57
6.46	262.51	.106E+07	1225.0	3.75	14.37
7.31	263.36	.136E+07	1693.3	4.02	13.37
8.16	264.21	.169E+07	2283.2	4.35	12.36
9.00	265.05	.206E+07	2969.1	4.66	11.54
9.85	265.90	.245E+07	3754.4	4.96	10.86
10.70	266.75	.286E+07	4643.0	5.24	10.28
11.55	267.60	.331E+07	5636.1	5.50	9.78
12.39	268.44	.378E+07	6746.2	5.76	9.34
13.24	269.29	.428E+07	7972.2	6.01	8.96
14.09	270.14	.482E+07	9218.9	6.18	8.71
14.94	270.99	.539E+07	10550.5	6.32	8.52
15.78	271.83	.601E+07	12024.7	6.46	8.33

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8380)	862.04	4.01	8.00	18.22	0.60	1.00
OUTFLOW: ID= 1 (6268)	862.04	3.85	9.50	18.22	0.59	1.00

ADD HYD (8382) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0268):	215.76	2.333	7.00	22.96
+ ID2= 2 (6268):	862.04	3.853	9.50	18.22
ID = 3 (8382):	1077.80	5.729	8.50	19.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6270)
IN= 2----> OUT= 1

Routing time step (mi n)' = 15.00

Distance	Elevation	Manning	
0.00	245.98	0.0500	
8.14	245.66	0.0500	
16.28	245.16	0.0500	
20.35	244.84	0.0500	
38.66	242.98	0.0500	
48.83	240.65	0.0500	
63.07	235.91	0.0500	
65.11	235.18	0.0500	
68.43	234.34	0.0500 / 0.0300	Main Channel
68.68	233.95	0.0300	Main Channel
69.18	233.89	0.0300	Main Channel
69.68	233.95	0.0300	Main Channel
71.21	234.48	0.0300 / 0.0500	Main Channel
81.38	236.44	0.0500	
95.63	236.66	0.0500	
120.04	237.00	0.0500	
148.53	241.77	0.0500	
158.70	242.34	0.0500	
187.18	244.03	0.0500	
201.42	244.36	0.0500	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.45	234.34	.217E+04	0.7	0.99	52.83
0.98	234.87	.966E+04	4.6	1.49	35.20
1.50	235.39	.250E+05	13.7	1.73	30.41
2.03	235.92	.475E+05	29.5	1.95	26.87
2.56	236.45	.772E+05	52.5	2.14	24.50
3.09	236.98	.142E+06	82.4	1.83	28.73
3.61	237.50	.246E+06	158.1	2.03	25.91
4.14	238.03	.357E+06	262.0	2.31	22.73
4.67	238.56	.477E+06	392.1	2.59	20.27
5.20	239.09	.604E+06	547.8	2.86	18.38
5.72	239.61	.739E+06	729.2	3.11	16.90
6.25	240.14	.882E+06	936.5	3.34	15.70
6.78	240.67	.103E+07	1169.7	3.57	14.72
7.31	241.20	.119E+07	1425.9	3.77	13.94

7.83	241.72	.136E+07	1710.4	3.96	13.26
8.36	242.25	.154E+07	1967.9	4.02	13.07
8.89	242.78	.174E+07	2264.9	4.09	12.83
9.42	243.31	.196E+07	2589.3	4.15	12.65
9.94	243.83	.221E+07	2953.5	4.22	12.46

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8382)	1077.80	5.73	8.50	19.17	1.04	1.52
OUTFLOW: ID= 1 (6270)	1077.80	5.63	9.00	19.17	1.04	1.52

ADD HYD (8384) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0270):	243.61	2.706	7.25	28.01
+ ID2= 2 (6270):	1077.80	5.629	9.00	19.17
ID = 3 (8384):	1321.41	8.059	8.50	20.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0262) ID= 1 DT=15.0 mi n	Area (ha)	Curve Number (CN)
	341.31	82.0
	5.00	# of Li near Res. (N)= 1.30
	1.01	

Unit Hyd Opeak	(cms)= 4.094
PEAK FLOW	(cms)= 3.472 (i)
TIME TO PEAK	(hrs)= 7.750
RUNOFF VOLUME	(mm)= 29.023
TOTAL RAI NFALL	(mm)= 62.700
RUNOFF COEFFI CIENT	= 0.463

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0258) ID= 1 DT=15.0 mi n	Area (ha)	Curve Number (CN)
	181.99	79.0
	5.00	# of Li near Res. (N)= 1.30
	1.18	

Unit Hyd Opeak	(cms)= 1.881
PEAK FLOW	(cms)= 1.492 (i)
TIME TO PEAK	(hrs)= 8.000
RUNOFF VOLUME	(mm)= 26.347
TOTAL RAI NFALL	(mm)= 62.700
RUNOFF COEFFI CIENT	= 0.420

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0256) ID= 1 DT=15.0 mi n	Area (ha)	Curve Number (CN)
	145.79	67.0
	5.00	# of Li near Res. (N)= 1.30
	1.04	

Unit Hyd Opeak	(cms)= 1.707
PEAK FLOW	(cms)= 0.887 (i)
TIME TO PEAK	(hrs)= 8.000
RUNOFF VOLUME	(mm)= 18.019
TOTAL RAI NFALL	(mm)= 62.700
RUNOFF COEFFI CIENT	= 0.287

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0254) ID= 1 DT=15.0 mi n	Area (ha)	Curve Number (CN)
	403.00	55.0
	5.00	# of Li near Res. (N)= 1.30
	1.22	

Unit Hyd Qpeak (cms)= 4.028

PEAK FLOW (cms)= 1.477 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 12.430
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.198

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6256)
IN= 2--> OUT= 1
Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2561.0) ----->

Table with columns: Di stance, El evation, Mann ing, and channel descriptions (Main Channel).

<----- TRAVEL TIME TABLE ----->

Table with columns: DEPTH (m), ELEV (m), VOLUME (cu. m.), FLOW RATE (cms), VELOCITY (m/s), TRAV. TIME (min).

hydrograph table with columns: AREA (ha), OPEAK (cms), TPEAK (hrs), R. V. (mm), MAX DEPTH (m), MAX VEL (m/s).

ADD HYD (8370) table with columns: AREA (ha), OPEAK (cms), TPEAK (hrs), R. V. (mm).

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB

NASHYD (0250) Area (ha)= 192.88 Curve Number (CN)= 70.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.30
U. H. Tp(hrs)= 1.22

Unit Hyd Qpeak (cms)= 1.930

PEAK FLOW (cms)= 1.144 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 19.816
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.316

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0246) Area (ha)= 759.61 Curve Number (CN)= 55.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.30
U. H. Tp(hrs)= 1.81

Unit Hyd Qpeak (cms)= 5.121

PEAK FLOW (cms)= 2.071 (i)
TIME TO PEAK (hrs)= 10.000
RUNOFF VOLUME (mm)= 12.474
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.199

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0248) Area (ha)= 146.04 Curve Number (CN)= 64.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.30
U. H. Tp(hrs)= 0.78

Unit Hyd Qpeak (cms)= 2.271

PEAK FLOW (cms)= 0.994 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 16.345
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.261

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8364) table with columns: AREA (ha), OPEAK (cms), TPEAK (hrs), R. V. (mm).

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6250)
IN= 2--> OUT= 1
Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2501.0) ----->

Table with columns: Di stance, El evation, Mann ing, and channel descriptions (Main Channel).

152.01	261.95	0.0500
171.27	264.63	0.0500
188.40	267.90	0.0500
211.95	274.18	0.0500

TRAVEL TIME TABLE						
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)	
0.49	257.05	205E+04	0.4	0.68	89.65	
1.17	257.73	248E+05	5.7	0.85	72.16	
1.85	258.41	841E+05	25.2	1.10	55.60	
2.53	259.09	180E+06	66.5	1.36	45.14	
3.21	259.77	312E+06	135.7	1.60	38.36	
3.89	260.45	481E+06	238.3	1.82	33.61	
4.57	261.13	681E+06	383.5	2.07	29.60	
5.25	261.81	911E+06	568.9	2.29	26.70	
5.93	262.49	117E+07	800.5	2.51	24.36	
6.61	263.17	146E+07	1079.0	2.72	22.49	
7.28	263.84	177E+07	1413.4	2.94	20.85	
7.96	264.52	210E+07	1797.0	3.14	19.50	
8.64	265.20	246E+07	2241.6	3.35	18.28	
9.32	265.88	283E+07	2739.9	3.55	17.25	
10.00	266.56	323E+07	3295.7	3.75	16.34	
10.68	267.24	364E+07	3908.9	3.94	15.54	
11.36	267.92	408E+07	4578.5	4.12	14.84	
12.04	268.60	453E+07	5320.1	4.32	14.18	
12.72	269.28	499E+07	6109.1	4.50	13.61	

<---- hydrograph ---->							<- pi pe / channel ->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)		
INFLOW: ID= 2 (8364)	905.65	2.93	8.25	13.10	0.81	0.75		
OUTFLOW: ID= 1 (6250)	905.65	2.72	10.50	13.10	0.79	0.75		

ADD HYD (8366)					
ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
1 + 2 = 3					
ID1= 1 (0250):	192.88	1.144	8.00	19.82	
+ ID2= 2 (6250):	905.65	2.724	10.50	13.10	
ID = 3 (8366):	1098.53	3.765	10.00	14.28	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8368)					
ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
1 + 2 = 3					
ID1= 1 (8366):	1098.53	3.765	10.00	14.28	
+ ID2= 2 (8370):	548.79	2.197	9.25	13.91	
ID = 3 (8368):	1647.32	5.937	9.75	14.16	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2521)			
ID= 1 DT=15.0 min	Area (ha)	Imp(%)	Di r. Conn.(%)
	32.51	75.00	70.00

IMPERVIOUS			PERVIOUS (i)		
Surface Area (ha)	24.38		8.13		
Dep. Storage (mm)	2.00		5.00		
Average Slope (%)	0.50		0.50		
Length (m)	465.55		40.00		
Mannings n	0.013		0.250		

Max. Eff. Inten. (mm/hr)	82.76	43.01
over (mi n)	15.00	30.00
Storage Coeff. (mi n)	8.54 (ii)	23.53 (ii)
Uni t Hyd. Tpeak (mi n)	15.00	30.00
Uni t Hyd. tpeak (cms)	0.09	0.04

TOTALS			
PEAK FLOW (cms)	4.61	0.45	4.899 (iii)
TIME TO PEAK (hrs)	6.00	6.25	
RUNOFF VOLUME (mm)	60.70	23.36	49.50
TOTAL RAINFALL (mm)	62.70	62.70	62.70

RUNOFF COEFFICIENT = 0.97 0.37 0.79

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.6 Ia = Dep Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5252)					
IN= 2---> OUT= 1	DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
		0.0000	0.0000	2.1750	1.0334
		0.1100	0.5793	2.7310	1.1195
		0.6300	0.6958	3.1790	1.2039
		1.1730	0.8017	3.5790	1.2139
		1.6270	0.9137	0.0000	0.0000

INFLOW: ID= 2 (2521)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5252)	32.510	4.899	6.00	49.50
	32.510	1.433	6.50	49.46

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 29.25
TIME SHIFT OF PEAK FLOW (mi n) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 0.8786

CALIB NASHYD (2522)			
ID= 1 DT=15.0 min	Area (ha)	Imp(%)	Di r. Conn.(%)
	287.47	5.00	71.5
	5.00		1.30
	U. H. Tp (hrs)	1.01	

Uni t Hyd Opeak (cms) = 3.448

PEAK FLOW (cms)	2.056 (i)
TIME TO PEAK (hrs)	8.000
RUNOFF VOLUME (mm)	20.717
TOTAL RAINFALL (mm)	62.700
RUNOFF COEFFICIENT	0.330

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9252)					
ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
1 + 2 = 3					
ID1= 1 (2522):	287.47	2.056	8.00	20.72	
+ ID2= 2 (5252):	32.51	1.433	6.50	49.46	
ID = 3 (9252):	319.98	3.213	6.50	23.64	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8372)					
ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
1 + 2 = 3					
ID1= 1 (8368):	1647.32	5.937	9.75	14.16	
+ ID2= 2 (9252):	319.98	3.213	6.50	23.64	
ID = 3 (8372):	1967.30	8.024	9.00	15.70	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6258)		
IN= 2---> OUT= 1	Routing time step (mi n)	
	15.00	

<---- DATA FOR SECTION (2581.0) ---->		
Di stance	Ei evation	Manning
0.00	252.88	0.0500
15.47	251.89	0.0500
46.41	248.45	0.0500

73.48	245.81	0.0500	
88.95	243.75	0.0500	
112.15	242.00	0.0500	
135.35	240.23	0.0500	
162.42	239.76	0.0500	
170.97	239.52	0.0500 / 0.0350	Main Channel
171.58	239.03	0.0350	Main Channel
174.02	239.03	0.0350	Main Channel
176.46	239.03	0.0350	Main Channel
177.07	239.52	0.0350 / 0.0500	Main Channel
185.63	239.67	0.0500	
208.83	239.87	0.0500	
239.77	240.14	0.0500	
274.57	244.93	0.0500	
336.45	249.51	0.0500	
363.52	249.77	0.0500	
382.85	251.78	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.49	239.52	.135E+05	2.5	0.92	91.32
1.14	240.17	.175E+06	25.6	0.73	114.22
1.78	240.81	.532E+06	123.5	1.17	71.85
2.43	241.46	.932E+06	285.3	1.54	54.48
3.07	242.10	.138E+07	506.5	1.85	45.25
3.72	242.75	.186E+07	786.8	2.12	39.42
4.36	243.39	.239E+07	1127.3	2.37	35.33
5.01	244.04	.296E+07	1535.8	2.61	32.12
5.65	244.68	.356E+07	2014.3	2.84	29.48
6.30	245.33	.420E+07	2535.9	3.03	27.60
6.94	245.97	.488E+07	3108.4	3.20	26.16
7.59	246.62	.561E+07	3740.6	3.35	24.98
8.23	247.26	.638E+07	4446.7	3.50	23.93
8.88	247.91	.721E+07	5229.7	3.64	22.98
9.52	248.55	.809E+07	6094.3	3.78	22.12
10.17	249.20	.901E+07	7050.4	3.93	21.31
10.81	249.84	.100E+08	7773.0	3.90	21.46
11.46	250.49	.111E+08	8998.8	4.07	20.56
12.10	251.13	.122E+08	10318.2	4.24	19.75

<----- hydrograph -----> <-- pi pe / channel -->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8372)	1967.30	8.02	9.00	15.70	0.65	0.86
OUTFLOW: ID= 1 (6258)	1967.30	7.35	11.00	15.70	0.62	0.87

ADD HYD (8374)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0258):	181.99	1.492	8.00	26.35
+ ID2= 2 (6258):	1967.30	7.350	11.00	15.70
ID = 3 (8374):	2149.29	8.571	10.75	16.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2601)	Area (ha)	Imp(%)	Di r. Conn. (%)
ID= 1 DT=15.0 mi n	56.22	65.00	48.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	36.54	19.68
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	612.21	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	82.76	83.16
over (mi n)	15.00	30.00
Storage Coeff. (mi n)=	10.06 (ii)	21.58 (ii)
Uni t Hyd. Tpeak (mi n)=	15.00	30.00
Uni t Hyd. tpeak (cms)=	0.09	0.05

	(cms)	(hrs)	(mm)	(mm)
PEAK FLOW	5.23	2.25	6.708 (iii)	
TIME TO PEAK	6.00	6.25	6.00	
RUNOFF VOLUME	60.70	35.87	47.79	
TOTAL RAINFALL	62.70	62.70	62.70	

RUNOFF COEFFICIENT = 0.97 0.57 0.76

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 81.5 Ia = Dep Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5260)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2---> OUT= 1				
DT= 15.0 mi n				
	0.0000	0.0000	3.7610	1.6799
	0.1900	0.8974	4.7220	1.8499
	1.0890	1.0678	5.4970	2.0146
	2.0290	1.2667	5.8970	2.0246
	2.8140	1.4621	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2601)	56.220	6.708	6.00	47.79
OUTFLOW: ID= 1 (5260)	56.220	2.381	6.50	47.77

PEAK FLOW REDUCTION [Qout/Oi n] (%)	TIME SHIFT OF PEAK FLOW (mi n)	MAXIMUM STORAGE USED (ha. m.)
35.50	30.00	1.3617

CALIB NASHYD (2602)	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID= 1 DT=15.0 mi n	420.02	5.00	83.3	1.30
	U. H. Tp (hrs)=	1.30		

Uni t Hyd Opeak (cms)= 3.946

PEAK FLOW	(cms)= 3.705 (i)
TIME TO PEAK	(hrs)= 8.000
RUNOFF VOLUME	(mm)= 30.405
TOTAL RAINFALL	(mm)= 62.700
RUNOFF COEFFICIENT	= 0.485

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9260)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (2602):	420.02	3.705	8.00	30.41
+ ID2= 2 (5260):	56.22	2.381	6.50	47.77
ID = 3 (9260):	476.24	5.546	6.75	32.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8376)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (8374):	2149.29	8.571	10.75	16.60
+ ID2= 2 (9260):	476.24	5.546	6.75	32.46
ID = 3 (8376):	2625.53	12.206	10.00	19.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8378)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0262):	341.31	3.472	7.75	29.02
+ ID2= 2 (8376):	2625.53	12.206	10.00	19.48
ID = 3 (8378):	2966.84	15.157	9.50	20.57

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	(8362)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8378):	2966.84	15.157	9.50	20.57	
+ ID2= 2 (8384):	1321.41	8.059	8.50	20.80	

ID = 3 (8362):	4288.25	23.095	9.00	20.64	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6272)
IN= 2----> OUT= 1 Routing time step (mi n)' = 15.00

<----- DATA FOR SECTION (2721.0) ----->

Distance	Elevation	Manning		
0.00	231.01	0.0450		
23.01	223.65	0.0450		
34.51	222.46	0.0450		
51.77	222.11	0.0450		
69.02	221.87	0.0450		
161.06	221.92	0.0450		
166.81	221.91	0.0450		
172.56	221.89	0.0450		
180.57	221.40	0.0450 / 0.0300	Main Channel	
181.57	220.60	0.0300	Main Channel	
184.07	220.16	0.0300	Main Channel	
195.57	221.85	0.0300 / 0.0450	Main Channel	
201.32	221.82	0.0450		
207.07	221.77	0.0450		
212.83	221.72	0.0450		
253.09	222.52	0.0450		
408.40	222.65	0.0450		
460.17	223.20	0.0450		
517.69	224.84	0.0450		
569.46	232.57	0.0450		

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	TRAVEL TIME FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.41	220.57	.189E+04	0.5	0.49	60.18
0.83	220.99	.697E+04	3.2	0.82	35.79
1.24	221.40	.145E+05	8.8	1.07	27.45
1.84	222.00	.643E+05	29.3	0.81	36.61
2.44	222.60	.282E+06	132.9	0.83	35.41
3.04	223.20	.707E+06	413.7	1.04	28.50
3.64	223.80	.118E+07	876.8	1.31	22.46
4.24	224.40	.168E+07	1494.8	1.57	18.75
4.84	225.00	.221E+07	2264.1	1.82	16.23
5.44	225.60	.274E+07	3196.3	2.06	14.29
6.04	226.20	.328E+07	4258.6	2.30	12.84
6.65	226.81	.383E+07	5444.7	2.52	11.72
7.25	227.41	.438E+07	6749.7	2.73	10.82
7.85	228.01	.494E+07	8169.4	2.93	10.08
8.45	228.61	.551E+07	9700.7	3.12	9.46
9.05	229.21	.608E+07	11340.8	3.30	8.94
9.65	229.81	.666E+07	13087.4	3.48	8.48
10.25	230.41	.724E+07	14938.5	3.65	8.08
10.85	231.01	.783E+07	16892.4	3.82	7.73

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8362)	4288.25	23.10	9.00	20.64	1.66	0.87
OUTFLOW: ID= 1 (6272)	4288.25	22.75	9.75	20.64	1.65	0.88

CALIB STANDHYD (2721)
ID= 1 DT=15.0 mi n

Area (ha)= 2.21
Total Imp(%)= 49.00
Dir. Conn.(%)= 19.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	1.08	1.13
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50

Length (m)=	121.38	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	82.76	73.51
over (mi n)	15.00	30.00
Storage Coeff. (mi n)=	3.81 (ii)	15.91 (ii)
Uni t Hyd. Tpeak (mi n)=	15.00	30.00
Uni t Hyd. peak (cms)=	0.11	0.05

TOTALS
0.179 (iii)
6.00
35.79
62.70
0.57

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5272)
IN= 2----> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.1480	0.0589
0.0070	0.0284	0.1850	0.0669
0.0430	0.0331	0.2160	0.0746
0.0800	0.0419	0.6160	0.0846
0.1100	0.0497	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2721)	2.210	0.179	6.00	35.79
OUTFLOW: ID= 1 (5272)	2.210	0.066	6.75	35.33

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 37.02
TIME SHIFT OF PEAK FLOW (mi n) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.0393

CALIB NASHYD (2722)
ID= 1 DT=15.0 mi n

Area (ha)= 155.17
U. H. Tp(hrs)= 1.09
Curve Number (CN)= 75.3
of Linear Res. (N)= 1.50

Uni t Hyd Opeak (cms)= 2.433

PEAK FLOW (cms)=	1.598 (i)
TIME TO PEAK (hrs)=	7.500
RUNOFF VOLUME (mm)=	23.397
TOTAL RAI NFALL (mm)=	62.700
RUNOFF COEFFICIENT =	0.373

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9272)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2722):	155.17	1.598	7.50	23.40
+ ID2= 2 (5272):	2.21	0.066	6.75	35.33

ID = 3 (9272):	157.38	1.642	7.25	23.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8356)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6272):	4288.25	22.749	9.75	20.64
+ ID2= 2 (9272):	157.38	1.642	7.25	23.56

=====
 ID = 3 (8356): 4445.63 23.987 9.75 20.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8352)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8354): 1268.73 13.898 8.00 27.05
 + ID2= 2 (8356): 4445.63 23.987 9.75 20.75

 ID = 3 (8352): 5714.36 36.561 8.75 22.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6304)
 IN= 2--> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3041.0) ----->
 Distance Elevation Manning
 0.00 232.08 0.0500
 19.00 231.87 0.0500
 38.00 231.33 0.0500
 66.51 230.44 0.0500
 104.51 228.25 0.0500
 118.76 225.17 0.0500
 128.26 219.86 0.0500
 175.77 219.17 0.0500
 185.27 218.90 0.0500 / 0.0300 Main Channel
 185.52 218.65 0.0300 Main Channel
 190.02 218.37 0.0300 Main Channel
 194.52 218.60 0.0300 Main Channel
 194.77 218.85 0.0300 / 0.0500 Main Channel
 204.27 219.60 0.0500
 299.28 220.91 0.0500
 327.78 222.36 0.0500
 375.28 225.71 0.0500
 403.79 229.37 0.0500
 432.29 230.43 0.0500
 470.29 232.00 0.0500

<----- TRAVEL TIME TABLE ----->
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV. TIME
 (m) (m) (cu. m.) (cms) (m/s) (mi n)
 0.48 218.85 .121E+05 1.2 0.36 173.94
 1.17 219.54 .839E+05 9.8 0.44 142.60
 1.86 220.23 .323E+06 41.0 0.47 131.43
 2.56 220.93 .708E+06 109.5 0.58 107.67
 3.25 221.62 .118E+07 224.3 0.71 87.36
 3.94 222.31 .168E+07 376.5 0.84 74.48
 4.63 223.00 .222E+07 567.9 0.96 65.26
 5.32 223.69 .279E+07 794.7 1.07 58.58
 6.02 224.39 .339E+07 1056.2 1.17 53.52
 6.71 225.08 .402E+07 1352.4 1.26 49.52
 7.40 225.77 .468E+07 1677.3 1.34 46.46
 8.09 226.46 .536E+07 2050.0 1.43 43.58
 8.79 227.16 .607E+07 2457.9 1.52 41.14
 9.48 227.85 .680E+07 2901.0 1.60 39.05
 10.17 228.54 .755E+07 3352.7 1.66 37.53
 10.86 229.23 .834E+07 3812.5 1.71 36.48
 11.55 229.92 .919E+07 4241.8 1.73 36.13
 12.25 230.62 .101E+08 4697.0 1.74 35.93
 12.94 231.31 .111E+08 5186.3 1.74 35.83

<----- hydrograph -----> <- pi pe / channel ->
 AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8352) 5714.36 36.56 8.75 22.15 1.77 0.47
 OUTFLOW: ID= 1 (6304) 5714.36 29.80 11.50 22.15 1.62 0.46

 ADD HYD (8350)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0304): 292.37 0.987 10.50 16.06
 + ID2= 2 (6304): 5714.36 29.801 11.50 22.15

=====
 ID = 3 (8350): 6006.73 30.772 11.50 21.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8346)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8348): 3768.46 20.547 10.00 19.83
 + ID2= 2 (8350): 6006.73 30.772 11.50 21.85

 ID = 3 (8346): 9775.19 50.360 10.75 21.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8344)
 1 + 2 = 3
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0306): 283.97 0.331 13.75 11.39
 + ID2= 2 (8346): 9775.19 50.360 10.75 21.07

 ID = 3 (8344): 10059.16 50.658 10.75 20.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 RESERVOIR (5509)
 IN= 2--> OUT= 1
 DT= 15.0 min
 OUTFLOW STORAGE OUTFLOW STORAGE
 (cms) (ha. m.) (cms) (ha. m.)
 0.0000 0.0000 ***** 370.0451
 41.0590 56.7403 ***** 863.4386
 48.1390 86.3439 *****
 56.6340 *****
 67.9600 ***** 0.0000 0.0000
 AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (8344) ***** 50.658 10.75 20.80
 OUTFLOW: ID= 1 (5509) ***** 39.339 14.25 20.80

PEAK FLOW REDUCTION [Out/Oi n] (%) = 77.66
 TIME SHIF T OF PEAK FLOW (mi n) = 210.00
 MAXIMUM STORAGE USED (ha. m.) = 54.3802

 CALIB NASHYD (0332)
 ID= 1 DT=15.0 mi n
 Area (ha) = 393.44 Curve Number (CN) = 75.0
 Ia (mm) = 5.00 # of Li near Res. (N) = 1.50
 U. H. Tp (hrs) = 2.32

Unit Hyd Opeak (cms) = 2.894
 PEAK FLOW (cms) = 2.232 (i)
 TIME TO PEAK (hrs) = 9.750
 RUNOFF VOLUME (mm) = 23.317
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.372

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB NASHYD (0330)
 ID= 1 DT=15.0 mi n
 Area (ha) = 468.30 Curve Number (CN) = 80.0
 Ia (mm) = 5.00 # of Li near Res. (N) = 1.50
 U. H. Tp (hrs) = 1.09

Unit Hyd Opeak (cms) = 7.335
 PEAK FLOW (cms) = 5.661 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 27.223
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.434

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6332)
IN= 2---> OUT= 1 Routing time step (min)' = 15.00

----- DATA FOR SECTION (3321.0) -----

Distance	Elevation	Manning	
0.00	234.00	0.0380	
25.85	227.20	0.0380	
96.94	226.44	0.0380	
168.03	227.38	0.0380	
219.73	225.62	0.0380	
342.52	221.57	0.0380	
368.37	221.42	0.0380	
374.83	221.23	0.0380	
379.79	220.98	0.0380 / 0.0300	Main Channel
380.29	220.47	0.0300	Main Channel
381.29	220.47	0.0300	Main Channel
382.79	220.47	0.0300	Main Channel
383.29	220.98	0.0300 / 0.0380	Main Channel
394.22	221.22	0.0380	
400.68	221.33	0.0380	
407.14	221.44	0.0380	
491.16	225.70	0.0380	
568.71	227.55	0.0380	
607.49	230.14	0.0380	
639.80	234.08	0.0380	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.51	220.98	.554E+04	0.9	0.56	107.27
1.20	221.67	.885E+05	12.1	0.49	122.32
1.88	222.35	.309E+06	64.7	0.76	79.70
2.57	223.04	.615E+06	165.4	0.97	61.99
3.25	223.72	.101E+07	322.1	1.16	52.06
3.94	224.41	.148E+07	542.9	1.33	45.51
4.62	225.09	.204E+07	835.4	1.48	40.77
5.31	225.78	.269E+07	1202.8	1.62	37.27
5.99	226.46	.344E+07	1618.0	1.70	35.46
6.68	227.15	.447E+07	1932.0	1.56	38.56
7.36	227.83	.580E+07	2763.9	1.72	34.96
8.05	228.52	.718E+07	3861.9	1.95	30.97
8.73	229.20	.859E+07	5115.4	2.16	27.98
9.42	229.89	.100E+08	6519.2	2.35	25.64
10.10	230.57	.115E+08	8092.3	2.55	23.69
10.79	231.26	.130E+08	9821.5	2.73	22.05
11.47	231.94	.145E+08	11690.0	2.92	20.68
12.16	232.63	.160E+08	13694.9	3.09	19.52
12.84	233.31	.176E+08	15833.9	3.26	18.52

----- hydrograph -----

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (0330)	468.30	5.66	7.50	27.22	0.80
OUTFLOW: ID= 1 (6332)	468.30	4.35	9.75	27.22	0.54

----- ADD HYD (8322) -----

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0332):	393.44	2.232	9.75
+ ID2= 2 (6332):	468.30	4.351	9.75
ID = 3 (8322):	861.74	6.582	9.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

----- CALIB NASHYD (0328) -----

Area (ha)=	492.92	Curve Number (CN)=	77.0
U. H. Tp (hrs)=	1.91	# of Linear Res. (N)=	1.50

Unit Hyd Opeak (cms)= 4.411
PEAK FLOW (cms)= 3.474 (i)
TIME TO PEAK (hrs)= 8.750

RUNOFF VOLUME (mm)= 24.828
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.396

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB NASHYD (0326) -----

Area (ha)=	678.91	Curve Number (CN)=	80.0
U. H. Tp (hrs)=	1.95	# of Linear Res. (N)=	1.50

Unit Hyd Opeak (cms)= 5.941
PEAK FLOW (cms)= 5.199 (i)
TIME TO PEAK (hrs)= 8.750
RUNOFF VOLUME (mm)= 27.366
TOTAL RAINFALL (mm)= 62.700
RUNOFF COEFFICIENT = 0.436

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- CALIB STANDHYD (3181) -----

Area (ha)=	357.30	Dir. Conn. (%)=	32.00
Total Imp (%)=	56.00		

IMPERVIOUS (ha)= 200.09
PERVIOUS (i) (ha)= 157.21
Dep. Storage (mm)= 2.00
Average Slope (%)= 0.50
Length (m)= 1543.37
Mannings n = 0.013

Max. Eff. Inten. (mm/hr)= 82.76
over (min)= 15.00
Storage Coeff. (min)= 17.52 (ii)
Unit Hyd. Tpeak (min)= 15.00
Unit Hyd. peak (cms)= 0.07

PEAK FLOW (cms)= 17.86
TIME TO PEAK (hrs)= 6.00
RUNOFF VOLUME (mm)= 60.70
TOTAL RAINFALL (mm)= 62.70
RUNOFF COEFFICIENT = 0.97

TOTALS
25.957 (iii)
6.25
29.33
62.70
0.47

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.9 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

----- RESERVOIR (5318) -----

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	22.8580	8.7280
0.8190	2.5190	27.5380	9.6853
4.2860	5.8947	32.5800	10.8345
11.0600	6.7336	32.9800	10.8445
16.3830	7.4416	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (3181)	357.300	25.957	6.00
OUTFLOW: ID= 1 (5318)	357.300	11.106	6.75

PEAK FLOW REDUCTION [Qout/Qin] (%) = 42.78
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 6.7468

----- CALIB STANDHYD (3182) -----

Area (ha)=	126.13	Dir. Conn. (%)=	12.00
Total Imp (%)=	21.00		

Surface Area (ha)= IMPERVIOUS (ha)= 26.49
PERVIOUS (i) (ha)= 99.64

Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 916.99 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 82.76 24.95
 over (min) 15.00 45.00
 Storage Coeff. (min)= 12.82 (ii) 31.46 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03
 PEAK FLOW (cms)= 2.70 4.15
 TIME TO PEAK (hrs)= 6.00 6.50
 RUNOFF VOLUME (mm)= 60.70 23.70
 TOTAL RAINFALL (mm)= 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.38

TOTALS
 4.839 (iii)
 6.50
 28.14
 62.70
 0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8334)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (3182):	126.13	4.839	6.50	28.14
+ ID2= 2 (5318):	357.30	11.106	6.75	39.37
=====				
ID = 3 (8334):	483.43	14.677	6.75	36.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6320)	Routing time step (min)'
IN= 2--> OUT= 1	15.00

DATA FOR SECTION (3201.0) -----				
Distance	Elevation	Manning		
0.00	249.00	0.0380		
22.92	245.86	0.0380		
45.83	244.87	0.0380		
91.66	243.11	0.0380		
126.03	239.53	0.0380		
160.41	237.17	0.0380		
166.14	237.06	0.0380		
177.59	237.13	0.0380		
183.32	237.20	0.0380		
189.05	236.70	0.0380 / 0.0350	Main	Channel
193.05	235.89	0.0350	Main	Channel
197.05	236.64	0.0350 / 0.0380	Main	Channel
200.51	236.74	0.0380		
206.24	237.03	0.0380		
246.34	238.82	0.0380		
263.53	243.87	0.0380		
389.56	247.64	0.0380		
452.58	247.74	0.0380		
498.41	248.60	0.0380		
567.16	249.84	0.0380		

TRAVEL TIME TABLE -----					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.37	236.26	.270E+04	0.4	0.58	107.96
0.75	236.64	.108E+05	2.6	0.92	68.01
1.48	237.37	.877E+05	27.4	1.17	53.35
2.20	238.09	.277E+06	122.1	1.65	37.83
2.93	238.82	.540E+06	299.7	2.08	30.01
3.66	239.55	.857E+06	587.8	2.57	24.29
4.39	240.28	.120E+07	977.8	3.04	20.52
5.11	241.00	.158E+07	1458.1	3.46	18.02
5.84	241.73	.198E+07	2028.6	3.84	16.23
6.57	242.46	.240E+07	2690.5	4.19	14.86
7.29	243.18	.285E+07	3431.8	4.50	13.84
8.02	243.91	.335E+07	4149.1	4.64	13.44
8.75	244.64	.393E+07	4746.6	4.51	13.80

9.47	245.36	.463E+07	5618.6	4.53	13.75
10.20	246.09	.545E+07	6781.8	4.66	13.39
10.93	246.82	.635E+07	8274.5	4.87	12.79
11.66	247.55	.733E+07	9988.0	5.10	12.23
12.38	248.27	.853E+07	11422.0	5.01	12.45
13.11	249.00	.989E+07	13749.7	5.20	11.98

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8334)	483.43	14.68	6.75	36.44	1.10	1.02
OUTFLOW: ID= 1 (6320)	483.43	8.87	7.50	36.44	0.93	0.97

CALIB NASHYD (3202)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	84.56	80.9
	Imp (mm)= 5.00	# of Linear Res. (N)= 1.50
	U. H. Tp (hrs)= 0.70	

Unit Hyd Qpeak (cms)= 2.054

PEAK FLOW (cms)= 1.495 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 27.807
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.444

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3201)	Area (ha)	Di r. Conn. (%)
ID= 1 DT=15.0 min	194.18	16.00
	Total Imp (%)= 45.00	

	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	87.38	106.80
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1137.78	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 82.76 79.28
 over (min) 15.00 30.00
 Storage Coeff. (min)= 14.59 (ii) 26.33 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.07 0.04

TOTALS
 PEAK FLOW (cms)= 5.26 10.54 13.044 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.25
 RUNOFF VOLUME (mm)= 60.70 33.38 37.75
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.53 0.60

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 78.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5320)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2--> OUT= 1 DT= 15.0 min	0.0000	0.0000	7.4940	4.4477
	0.2490	1.8139	9.8910	4.7762
	1.2830	3.1517	12.4550	5.0763
	3.7490	3.7610	12.8550	5.0863
	5.0040	4.0198	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3201)	194.180	13.044	6.25	37.75
OUTFLOW: ID= 1 (5320)	194.180	4.563	7.00	37.74

PEAK FLOW REDUCTION [Qout/Qin] (%) = 34.98

TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 3.9331

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
(9320)				
1 + 2 = 3				
ID1= 1 (3202):	84.56	1.495	6.75	27.81
+ ID2= 2 (5320):	194.18	4.563	7.00	37.74
ID = 3 (9320):	278.74	6.043	7.00	34.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
(8332)				
1 + 2 = 3				
ID1= 1 (6320):	483.43	8.875	7.50	36.44
+ ID2= 2 (9320):	278.74	6.043	7.00	34.73
ID = 3 (8332):	762.17	14.603	7.25	35.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD	Area (ha)	Curve Number (CN)
(3142)	122.75	78.0
ID= 1 DT=15.0 min	5.00	# of Linear Res. (N) = 1.50
	0.53	

Unit Hyd Opeak (cms) = 3.924

PEAK FLOW (cms) = 2.435 (i)
 TIME TO PEAK (hrs) = 6.500
 RUNOFF VOLUME (mm) = 25.077
 TOTAL RAINFALL (mm) = 62.700
 RUNOFF COEFFICIENT = 0.400

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD	Area (ha)	Di r. Conn. (%)
(3141)	42.46	46.00
ID= 1 DT=15.0 min	70.00	

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 29.72 12.74
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 532.04 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 82.76 88.86
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 9.25 (ii) 20.46 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.09 0.05

TOTALS
 4.912 (iii)
 6.00
 45.18
 62.70
 0.72

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5314)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1				
DT= 15.0 min				

	(cms)	(ha. m.)	(cms)	(ha. m.)
	0.0000	0.0000	2.8400	1.3064
	0.1440	0.7144	3.5660	1.4269
	0.8230	0.8540	4.1520	1.5451
	1.5320	0.9987	4.5520	1.5551
	2.1250	1.1456	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3141)	42.460	4.912	6.00	45.18
OUTFLOW: ID= 1 (5314)	42.460	1.562	6.50	45.15

PEAK FLOW REDUCTION [Out/Oi n] (%) = 31.79
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 1.0151

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
(9314)				
1 + 2 = 3				
ID1= 1 (3142):	122.75	2.435	6.50	25.08
+ ID2= 2 (5314):	42.46	1.562	6.50	45.15
ID = 3 (9314):	165.21	3.997	6.50	30.23

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6316)	Routing time step (min)'
IN= 2--> OUT= 1	15.00

Distance	Elevation	Manning
0.00	248.54	0.0350
27.93	248.34	0.0350
41.89	246.61	0.0350
62.83	243.09	0.0350
132.65	239.00	0.0350
188.50	236.74	0.0350
195.48	236.54	0.0350
202.46	236.32	0.0350
205.48	236.14	0.0350
205.98	235.61	0.0310 /0.0310
207.98	235.25	0.0310
209.98	235.53	0.0310
210.48	236.00	0.0310 /0.0350
216.42	236.73	0.0350
258.31	239.09	0.0350
328.12	239.84	0.0350
439.83	241.47	0.0350
530.58	242.08	0.0350
586.43	242.93	0.0350
691.16	248.00	0.0350

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.37	235.62	.215E+04	0.7	0.77	53.88
0.75	236.00	.634E+04	3.5	1.36	30.45
1.46	236.71	.297E+05	19.3	1.62	25.66
2.16	237.41	.103E+06	75.8	1.84	22.60
2.87	238.12	.229E+06	202.8	2.21	18.79
3.57	238.82	.407E+06	423.3	2.59	16.03
4.28	239.53	.654E+06	727.7	2.77	14.97
4.99	240.24	.103E+07	1242.0	3.01	13.76
5.69	240.94	.151E+07	2002.3	3.31	12.54
6.40	241.65	.210E+07	2987.9	3.55	11.70
7.10	242.35	.286E+07	4302.8	3.75	11.07
7.81	243.06	.374E+07	6165.3	4.10	10.11
8.51	243.76	.468E+07	8642.4	4.60	9.03
9.22	244.47	.565E+07	11496.3	5.06	8.20
9.93	245.18	.666E+07	14714.1	5.50	7.54
10.63	245.88	.770E+07	18290.3	5.91	7.02
11.34	246.59	.877E+07	22221.9	6.30	6.58
12.04	247.29	.988E+07	26464.4	6.67	6.22
12.75	248.00	.110E+08	31061.3	7.01	5.91

INFLOW : ID= 2 (9314)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
	165.21	4.00	6.50	30.23	0.77	1.37

OUTFLOW: ID= 1 (6316) 165.21 3.50 7.00 30.23 0.75 1.36

CALIB NASHYD (3162) Area (ha)= 196.72 Curve Number (CN)= 83.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.88

Unit Hyd Qpeak (cms)= 3.838
 PEAK FLOW (cms)= 3.160 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 29.965
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.478

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3161) Area (ha)= 35.62 Dir. Conn. (%)= 44.00
 ID= 1 DT=15.0 min Total Imp(%)= 63.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 22.44 13.18
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 487.31 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 82.76 85.42
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 8.77 (ii) 20.17 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.05

TOTALS

PEAK FLOW (cms)= 3.16 1.59 4.209 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 60.70 36.19 46.97
 TOTAL RAINFALL (mm)= 62.70 62.70 62.70
 RUNOFF COEFFICIENT = 0.97 0.58 0.75

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 81.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5316) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	2.3160	0.9307
	0.0930	0.5020	2.9060	1.0021
	0.5400	0.6637	3.5360	1.0731
	1.1940	0.7495	3.9360	1.0831
	1.6920	0.8359	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (3161)	35.620	4.209	6.00	46.97
OUTFLOW: ID= 1 (5316)	35.620	1.629	6.50	46.93

PEAK FLOW REDUCTION [Qout/Qin] (%) = 38.69
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 0.8266

ADD HYD (9316)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3162):	196.72	3.160	7.00	29.96
+ ID2= 2 (5316):	35.62	1.629	6.50	46.93
ID = 3 (9316):	232.34	4.593	6.75	32.57

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8338)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6316):	165.21	3.495	7.00	30.23
+ ID2= 2 (9316):	232.34	4.593	6.75	32.57
ID = 3 (8338):	397.55	7.866	7.00	31.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0312) Area (ha)= 359.44 Curve Number (CN)= 80.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.36

Unit Hyd Qpeak (cms)= 4.529
 PEAK FLOW (cms)= 3.684 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 27.291
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.435

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0308) Area (ha)= 529.30 Curve Number (CN)= 62.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.62

Unit Hyd Qpeak (cms)= 5.575
 PEAK FLOW (cms)= 2.615 (i)
 TIME TO PEAK (hrs)= 8.500
 RUNOFF VOLUME (mm)= 15.525
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.248

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0310) Area (ha)= 138.28 Curve Number (CN)= 65.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.76

Unit Hyd Qpeak (cms)= 3.102
 PEAK FLOW (cms)= 1.342 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 16.858
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.269

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8342)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0308):	529.30	2.615	8.50	15.53
+ ID2= 2 (0310):	138.28	1.342	7.00	16.86
ID = 3 (8342):	667.58	3.777	8.00	15.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6312)
 IN= 2---> OUT= 1 Routing time step (min) = 15.00

<----- DATA FOR SECTION (3121.0) ----->

Distance	Elevation	Manning	
0.00	265.94	0.0360	
38.07	265.43	0.0360	
59.82	263.98	0.0360	
103.32	254.59	0.0360	
157.70	252.16	0.0360	
217.52	250.45	0.0360	
233.84	247.69	0.0360	
234.71	247.27	0.0360	
239.71	246.38	0.0360	
244.71	246.12	0.0360 / 0.0330	Main Channel
247.71	245.17	0.0330	Main Channel
249.71	245.19	0.0330	Main Channel
251.71	245.64	0.0330 / 0.0360	Main Channel
259.71	246.67	0.0360	
282.78	247.12	0.0360	
315.41	251.60	0.0360	
424.17	256.13	0.0360	
478.55	257.04	0.0360	
516.62	259.37	0.0360	
538.37	266.00	0.0360	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.47	245.64	.111E+04	3.1	1.78	6.07
1.54	246.71	.103E+05	53.9	3.39	3.19
2.61	247.78	.414E+05	281.3	4.40	2.45
3.68	248.85	.839E+05	749.1	5.79	1.87
4.74	249.91	.136E+06	1463.2	6.97	1.55
5.81	250.98	.201E+06	2354.8	7.61	1.42
6.88	252.05	.295E+06	3533.0	7.76	1.39
7.95	253.12	.427E+06	5303.9	8.05	1.34
9.02	254.19	.594E+06	7856.5	8.58	1.26
10.09	255.26	.792E+06	11449.2	9.37	1.15
11.15	256.32	.101E+07	15862.4	10.15	1.06
12.22	257.39	.127E+07	21074.5	10.75	1.01
13.29	258.46	.155E+07	28184.1	11.78	0.92
14.36	259.53	.185E+07	36386.2	12.77	0.85
15.43	260.60	.215E+07	46110.6	13.89	0.78
16.50	261.67	.246E+07	56822.5	14.95	0.72
17.56	262.73	.278E+07	68498.1	15.97	0.68
18.63	263.80	.310E+07	81118.8	16.94	0.64
19.70	264.87	.344E+07	93603.2	17.66	0.61

<---- hydrograph ---->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8342)	667.58	3.78	8.00	15.80	0.49	1.79
OUTFLOW: ID= 1 (6312)	667.58	3.78	8.00	15.80	0.49	1.79

ADD HYD (8340) | 1 + 2 = 3 |

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0312):	359.44	3.684	8.00	27.29
+ ID2= 2 (6312):	667.58	3.776	8.00	15.80
ID = 3 (8340):	1027.02	7.460	8.00	19.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8336) | 1 + 2 = 3 |

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8338):	397.55	7.866	7.00	31.60
+ ID2= 2 (8340):	1027.02	7.460	8.00	19.82
ID = 3 (8336):	1424.57	14.842	7.00	23.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8330) |

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8332):	762.17	14.603	7.25	35.81
+ ID2= 2 (8336):	1424.57	14.842	7.00	23.11
ID = 3 (8330):	2186.74	29.308	7.25	27.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6324) | IN= 2--> OUT= 1 |

Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3241.0) ----->

Distance	Elevation	Manning	
0.00	234.38	0.0360	
33.57	232.48	0.0360	
67.14	230.14	0.0360	
83.93	228.80	0.0360	
134.29	227.62	0.0360	
209.82	225.10	0.0360	
218.21	224.86	0.0360	
226.61	224.47	0.0360	
234.00	223.86	0.0360 / 0.0300	Main Channel
234.10	223.66	0.0300	Main Channel
235.00	223.66	0.0300	Main Channel
235.90	223.66	0.0300	Main Channel
236.00	223.86	0.0300 / 0.0360	Main Channel
243.39	224.92	0.0360	
251.78	224.89	0.0360	
335.71	225.64	0.0360	
562.32	226.53	0.0360	
637.85	228.36	0.0360	
705.00	229.80	0.0360	
830.89	234.00	0.0360	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.20	223.86	.145E+04	0.2	0.43	147.27
0.73	224.39	.159E+05	2.8	0.68	93.11
1.27	224.93	.561E+05	11.0	0.75	85.19
1.80	225.46	.217E+06	45.6	0.80	79.20
2.33	225.99	.566E+06	134.4	0.91	70.20
2.87	226.53	.122E+07	338.1	1.06	59.97
3.40	227.06	.206E+07	744.3	1.38	46.16
3.94	227.60	.298E+07	1295.9	1.66	38.38
4.47	228.13	.399E+07	1979.9	1.90	33.59
5.00	228.66	.509E+07	2807.5	2.11	30.21
5.54	229.20	.627E+07	3815.1	2.32	27.41
6.07	229.73	.753E+07	4984.1	2.53	25.17
6.60	230.26	.884E+07	6346.3	2.74	23.20
7.14	230.80	.102E+08	7861.2	2.95	21.61
7.67	231.33	.116E+08	9526.4	3.14	20.29
8.21	231.87	.131E+08	11342.2	3.32	19.18
8.74	232.40	.146E+08	13309.6	3.49	18.22
9.27	232.93	.161E+08	15414.0	3.66	17.41
9.81	233.47	.177E+08	17671.0	3.81	16.70

<---- hydrograph ---->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8330)	2186.74	29.31	7.25	27.54	1.55	0.78
OUTFLOW: ID= 1 (6324)	2186.74	21.18	8.75	27.54	1.42	0.76

CALIB NASHYD (3242) |

Area (ha)= 602.18
 Curve Number (CN)= 78.8
 U.H. Tp(hrs)= 1.80
 # of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 5.708
 PEAK FLOW (cms)= 4.720 (i)
 TIME TO PEAK (hrs)= 8.500
 RUNOFF VOLUME (mm)= 26.303
 TOTAL RAINFALL (mm)= 62.700
 RUNOFF COEFFICIENT = 0.420

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (3241)
ID= 1 DT=15.0 min

Area Total	(ha)= 13.46	Dir. Conn. (%)= 15.00
Imp (%)	= 45.00	
IMPERVIOUS PERVIOUS (i)		
Surface Area (ha)	= 6.06	7.40
Dep. Storage (mm)	= 2.00	5.00
Average Slope (%)	= 0.50	0.50
Length (m)	= 299.56	40.00
Mannings n	= 0.013	0.250
Max. Eff. Inten. (mm/hr)	= 82.76	80.70
over (min)	= 15.00	30.00
Storage Coeff. (min)	= 6.55 (ii)	18.21 (ii)
Unit Hyd. Tpeak (min)	= 15.00	30.00
Unit Hyd. peak (cms)	= 0.10	0.05

PEAK FLOW (cms)	= 0.43	0.88	*TOTALS*
TIME TO PEAK (hrs)	= 6.00	6.25	1.012 (iii)
RUNOFF VOLUME (mm)	= 60.70	33.59	6.00
TOTAL RAINFALL (mm)	= 62.70	62.70	37.65
RUNOFF COEFFICIENT	= 0.97	0.54	62.70
			0.60

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 78.2 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5324)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.9000	0.3505
0.0460	0.1645	1.1310	0.4008
0.2610	0.1903	1.3160	0.4499
0.4860	0.2456	1.7160	0.4599
0.6740	0.2932	0.0000	0.0000

INFLOW: ID= 2 (3241)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5324)	13.460	1.012	6.00	37.65
	13.460	0.445	6.75	37.59

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 44.03
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha.m.) = 0.2384

ADD HYD (9324) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3242):	602.18	4.720	8.50	26.30
+ ID2= 2 (5324):	13.46	0.445	6.75	37.59
ID = 3 (9324):	615.64	4.842	8.25	26.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8328) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6324):	2186.74	21.184	8.75	27.54
+ ID2= 2 (9324):	615.64	4.842	8.25	26.55
ID = 3 (8328):	2802.38	25.992	8.75	27.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0322) ID= 1 DT=15.0 min	Area (ha)= 513.13 Ia (mm)= 5.00 U. H. Tp (hrs)= 1.75	Curve Number (CN)= 80.0 # of Linear Res. (N)= 1.50
--	--	---

Unit Hyd Opeak (cms) = 5.020
PEAK FLOW (cms) = 4.298 (i)
TIME TO PEAK (hrs) = 8.500
RUNOFF VOLUME (mm) = 27.347
TOTAL RAINFALL (mm) = 62.700
RUNOFF COEFFICIENT = 0.436

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8326) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0322):	513.13	4.298	8.50	27.35
+ ID2= 2 (8328):	2802.38	25.992	8.75	27.32
ID = 3 (8326):	3315.51	30.276	8.75	27.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8324) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0326):	678.91	5.199	8.75	27.37
+ ID2= 2 (8326):	3315.51	30.276	8.75	27.32
ID = 3 (8324):	3994.42	35.474	8.75	27.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6328)
IN= 2---> OUT= 1
Routing time step (min) = 15.00

<----- DATA FOR SECTION (3281.0) ----->

Distance	Elevation	Manning
0.00	228.00	0.0380
18.58	224.97	0.0380
78.98	223.52	0.0380
125.44	223.28	0.0380
171.90	221.71	0.0380
213.72	219.65	0.0380
218.36	219.40	0.0380
223.01	219.19	0.0380
225.95	219.14	0.0380 / 0.0320
226.45	218.14	0.0320
236.95	218.14	0.0320
245.85	218.14	0.0320
245.95	219.14	0.0320 / 0.0380
246.24	219.16	0.0380
250.88	219.24	0.0380
255.53	219.39	0.0380
325.22	221.47	0.0380
367.03	223.14	0.0380
404.20	225.17	0.0380
459.95	228.04	0.0380

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	218.64	.373E+05	3.2	0.33	192.37
1.00	219.14	.752E+05	10.1	0.51	124.71
1.52	219.66	.149E+06	22.2	0.57	112.06
2.04	220.18	.279E+06	43.4	0.59	107.06
2.56	220.70	.464E+06	77.0	0.63	100.52
3.08	221.22	.705E+06	125.7	0.68	93.56
3.61	221.75	.100E+07	192.7	0.73	86.62
4.13	222.27	.135E+07	278.1	0.79	80.92
4.65	222.79	.176E+07	385.9	0.84	75.86
5.17	223.31	.222E+07	514.9	0.89	71.82
5.69	223.83	.280E+07	662.7	0.90	70.40
6.21	224.35	.345E+07	867.7	0.96	66.35

6.73	224.87	.417E+07	1107.0	1.01	62.81
7.25	225.39	.494E+07	1400.0	1.08	58.80
7.78	225.92	.573E+07	1732.2	1.15	55.17
8.30	226.44	.655E+07	2099.9	1.22	52.02
8.82	226.96	.740E+07	2502.8	1.29	49.29
9.34	227.48	.828E+07	2940.9	1.36	46.90
9.86	228.00	.918E+07	3414.3	1.42	44.79

<---- hydrograph ---->						<- pi pe / channel ->	
AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL		
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW: ID= 2 (8324)	3994.42	35.47	8.75	27.33	1.85	0.58	
OUTFLOW: ID= 1 (6328)	3994.42	28.86	10.50	27.33	1.69	0.58	

ADD HYD (8320)					
1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0328):	492.92	3.474	8.75	24.83	
+ ID2= 2 (6328):	3994.42	28.865	10.50	27.33	
=====					
ID = 3 (8320):	4487.34	32.131	10.50	27.06	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8318)					
1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (8320):	4487.34	32.131	10.50	27.06	
+ ID2= 2 (8322):	861.74	6.582	9.75	25.44	
=====					
ID = 3 (8318):	5349.08	38.605	10.25	26.80	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8316)					
1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (5509):	10059.16	39.339	14.25	20.80	
+ ID2= 2 (8318):	5349.08	38.605	10.25	26.80	
=====					
ID = 3 (8316):	15408.24	71.535	12.25	22.88	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8312)					
1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (8314):	620.10	0.793	7.00	13.65	
+ ID2= 2 (8316):	15408.24	71.535	12.25	22.88	
=====					
ID = 3 (8312):	16028.34	72.210	12.25	22.52	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8308)					
1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (8310):	598.91	19.443	6.50	30.41	
+ ID2= 2 (8312):	16028.34	72.210	12.25	22.52	
=====					
ID = 3 (8308):	16627.25	73.756	12.00	22.81	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5510)
IN= 2---> OUT= 1

DT= 15.0 mi n			
OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha. m.)	(cms)	(ha. m.)
0.0000	0.0000	*****	74.0090
66.5450	18.5023	*****	*****
98.5430	37.0045	*****	*****

AREA	OPEAK	TPEAK	R. V.
(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (8308)	*****	73.756	12.00
OUTFLOW: ID= 1 (5510)	*****	71.367	13.25

PEAK FLOW REDUCTION [Out/Oi n] (%) = 96.76
TIME SHIF T OF PEAK FLOW (mi n) = 75.00
MAXIMUM STORAGE USED (ha. m.) = 21.2924

ADD HYD (8240)					
1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0336):	2785.00	2.580	22.25	21.25	
+ ID2= 2 (5510):	16627.25	71.367	13.25	22.81	
=====					
ID = 3 (8240):	19412.25	73.582	13.25	22.59	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8238)					
1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.	
	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (5508):	9524.19	32.883	12.00	14.00	
+ ID2= 2 (8240):	19412.25	73.582	13.25	22.59	
=====					
ID = 3 (8238):	28936.44	105.762	12.75	19.76	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB			
STANDHYD (3427)	Area	(ha)= 249.40	
ID= 1 DT=15.0 mi n	Total	Imp(%)= 45.00	Di r. Conn. (%)= 33.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)= 112.23	137.17
Dep. Storage	(mm)= 2.00	5.00
Average Slope	(%)= 0.50	0.50
Length	(m)= 1289.44	40.00
Mannings n	= 0.013	0.250

Max. Eff. Inten. (mm/hr)=	82.76	24.21
over (mi n)	15.00	45.00
Storage Coeff. (mi n)=	15.73 (ii)	34.60 (ii)
Uni t Hyd. Tpeak (mi n)=	15.00	45.00
Uni t Hyd. peak (cms)=	0.07	0.03

PEAK FLOW	(cms)= 13.50	5.30	*TOTALS*
TIME TO PEAK	(hrs)= 6.00	6.50	15.709 (iii)
RUNOFF VOLUME	(mm)= 60.70	21.29	6.00
TOTAL RAINFALL	(mm)= 62.70	62.70	34.29
RUNOFF COEFFICIENT	= 0.97	0.34	62.70
			0.55

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 67.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9425)				
IN= 2---> OUT= 1	OUTFLOW	STORAGE	OUTFLOW	STORAGE
DT= 15.0 mi n	(cms)	(ha. m.)	(cms)	(ha. m.)
	0.0000	0.0000	12.4700	5.4211
	0.8500	3.0375	16.6800	6.4838
	4.8400	3.5126	20.9500	7.4161
	9.0000	4.5390	24.3900	8.3284

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (3427)	249.400	15.709	6.00	34.29
OUTFLOW: ID= 1 (9425)	249.400	6.470	6.75	34.29

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 41.19
 TIME SHIF T OF PEAK FLOW (mi n) = 45.00
 MAXI MUM STORAGE USED (ha. m.) = 3.9189

RESERVOIR (5342)
 IN= 2---> OUT= 1
 DT= 15.0 mi n

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha. m.)	(cms)	(ha. m.)
0.0000	0.0000	3.6600	12.0067
1.2080	11.1244	4.4200	12.4001
2.0570	11.3002	5.2300	12.7452
2.7240	11.3602	0.0000	0.0000

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (9425)	249.400	6.470	6.75	34.29
OUTFLOW: ID= 1 (5342)	249.400	0.534	17.00	34.28

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 8.25
 TIME SHIF T OF PEAK FLOW (mi n) = 615.00
 MAXI MUM STORAGE USED (ha. m.) = 4.9183

CALIB NASHDY (3422)
 ID= 1 DT=15.0 mi n

Area (ha) = 755.17 Curve Number (CN) = 54.5
 Ia (mm) = 5.00 # of Li near Res. (N) = 1.50
 U. H. Tp (hrs) = 2.57

Uni t Hyd Qpeak (cms) = 5.026

PEAK FLOW (cms) = 2.081 (i)
 TIME TO PEAK (hrs) = 10.250
 RUNOFF VOLUME (mm) = 12.311
 TOTAL RAI NFALL (mm) = 62.700
 RUNOFF COEFFI CI ENT = 0.196

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9342)
 1 + 2 = 3

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (3422):	755.17	2.081	10.25	12.31
+ ID2= 2 (5342):	249.40	0.534	17.00	34.28
=====				
ID = 3 (9342):	1004.57	2.558	10.50	17.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8236)
 1 + 2 = 3

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8238):	28936.44	105.762	12.75	19.76
+ ID2= 2 (9342):	1004.57	2.558	10.50	17.77
=====				
ID = 3 (8236):	29941.01	108.149	12.75	19.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8234)
 1 + 2 = 3

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8232):	285.79	7.859	6.00	28.59
+ ID2= 2 (8236):	29941.01	108.149	12.75	19.69
=====				
ID = 3 (8234):	30226.80	108.594	12.75	19.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (3442)
 ID= 1 DT=15.0 mi n

Area (ha) = 188.84
 Total Imp (%) = 32.00 Di r. Conn. (%) = 24.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	60.43	128.41
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1122.02	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	82.76	13.09
over (mi n)	15.00	45.00
Storage Coeff. (mi n)	14.47 (ii)	38.60 (ii)
Uni t Hyd. Tpeak (mi n)	15.00	45.00
Uni t Hyd. peak (cms)	0.07	0.03

TOTALS
 PEAK FLOW (cms) = 7.70 2.56 8.730 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 60.70 13.05 24.49
 TOTAL RAI NFALL (mm) = 62.70 62.70 62.70
 RUNOFF COEFFI CI ENT = 0.97 0.21 0.39

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 53.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFI CI ENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3441)
 ID= 1 DT=15.0 mi n

Area (ha) = 155.15
 Total Imp (%) = 39.00 Di r. Conn. (%) = 29.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	60.51	94.64
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1017.02	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	82.76	14.18
over (mi n)	15.00	45.00
Storage Coeff. (mi n)	13.64 (ii)	37.01 (ii)
Uni t Hyd. Tpeak (mi n)	15.00	45.00
Uni t Hyd. peak (cms)	0.08	0.03

TOTALS
 PEAK FLOW (cms) = 7.83 2.09 8.676 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 60.70 13.54 27.22
 TOTAL RAI NFALL (mm) = 62.70 62.70 62.70
 RUNOFF COEFFI CI ENT = 0.97 0.22 0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 53.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFI CI ENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5344)
 IN= 2---> OUT= 1
 DT= 15.0 mi n

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha. m.)	(cms)	(ha. m.)
0.0000	0.0000	2.6700	6.1252
0.3000	1.6125	3.3150	7.0314
0.8100	3.0036	3.8990	7.9700
1.4170	4.2559	4.2990	7.9800
1.8540	5.0526	0.0000	0.0000

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (3441)	155.150	8.676	6.00	27.22
OUTFLOW: ID= 1 (5344)	155.150	0.773	8.50	27.21

PEAK FLOW REDUCTION [Qout/ Qin] (%) = 8.91
 TIME SHIFT OF PEAK FLOW (min) = 150.00
 MAXIMUM STORAGE USED (ha.m.) = 2.9027

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3442):	188.84	8.730	6.00	24.49
+ ID2= 2 (5344):	155.15	0.773	8.50	27.21
-----	-----	-----	-----	-----
ID = 3 (9344):	343.99	8.952	6.00	25.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8234):	30226.80	108.594	12.75	19.78
+ ID2= 2 (9344):	343.99	8.952	6.00	25.71
-----	-----	-----	-----	-----
ID = 3 (8230):	30570.79	109.344	12.50	19.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8226):	1952.06	8.750	8.50	12.28
+ ID2= 2 (8230):	30570.79	109.344	12.50	19.85
-----	-----	-----	-----	-----
ID = 3 (8228):	32522.85	114.868	12.25	19.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0356):	1015.22	3.392	8.00	9.30
+ ID2= 2 (8228):	32522.85	114.868	12.25	19.39
-----	-----	-----	-----	-----
ID = 3 (8190):	33538.07	117.084	12.00	19.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ** SIMULATION NUMBER: 4 **

READ STORM
 Ptotal = 73.10 mm
 File name: C:\Users\aman\ucv\AppData\Local\Temp\ab70bd54-a452-450c-b03c-83cdc4826a98\12f62f64
 Comments: 25-Year 12-Hour SCS II Design Storm

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.25	1.83	3.25	2.92	6.25	13.16	9.25	2.56
0.50	1.83	3.50	2.92	6.50	13.16	9.50	2.56
0.75	1.83	3.75	2.92	6.75	5.85	9.75	2.56
1.00	1.83	4.00	2.92	7.00	5.85	10.00	2.56
1.25	1.83	4.25	4.39	7.25	4.39	10.25	1.46
1.50	1.83	4.50	4.39	7.50	4.39	10.50	1.46
1.75	1.83	4.75	5.85	7.75	4.39	10.75	1.46
2.00	1.83	5.00	5.85	8.00	4.39	11.00	1.46
2.25	2.19	5.25	8.77	8.25	2.56	11.25	1.46
2.50	2.19	5.50	8.77	8.50	2.56	11.50	1.46
2.75	2.19	5.75	35.09	8.75	2.56	11.75	1.46
3.00	2.19	6.00	96.49	9.00	2.56	12.00	1.46

CALIB NASHYD (0356)
 ID= 1 DT=15.0 min
 Area (ha)=1015.22
 Ia (mm)= 5.00
 U. H. Tp(hrs)= 1.37
 Curve Number (CN)= 46.0
 # of Linear Res. (N)= 1.50

Unit Hyd Qpeak (cms) = 12.651
 PEAK FLOW (cms) = 4.610 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 12.581
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.172

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0352)
 ID= 1 DT=15.0 min
 Area (ha)= 381.43
 Ia (mm)= 5.00
 U. H. Tp(hrs)= 0.82
 Curve Number (CN)= 54.0
 # of Linear Res. (N)= 1.50

Unit Hyd Qpeak (cms) = 7.980
 PEAK FLOW (cms) = 3.316 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 16.078
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.220

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0350)
 ID= 1 DT=15.0 min
 Area (ha)= 366.84
 Ia (mm)= 5.00
 U. H. Tp(hrs)= 1.07
 Curve Number (CN)= 48.0
 # of Linear Res. (N)= 1.50

Unit Hyd Qpeak (cms) = 5.831
 PEAK FLOW (cms) = 2.124 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 13.386
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.183

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0348)
 ID= 1 DT=15.0 min
 Area (ha)= 590.18
 Ia (mm)= 5.00
 U. H. Tp(hrs)= 1.91
 Curve Number (CN)= 48.0
 # of Linear Res. (N)= 1.50

Unit Hyd Qpeak (cms) = 5.267
 PEAK FLOW (cms) = 2.211 (i)
 TIME TO PEAK (hrs) = 9.000
 RUNOFF VOLUME (mm) = 13.458
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.184

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6350)
 IN= 2----> OUT= 1
 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3501.0) ----->

Distance	Elevation	Manning
0.00	287.00	0.0500
23.66	283.72	0.0500
70.98	280.94	0.0500
112.38	280.23	0.0500
171.52	276.80	0.0500
260.24	274.46	0.0500
266.16	274.26	0.0500
272.07	274.12	0.0500
276.49	274.12	0.0500 / 0.0300

Main Channel

276.99	273.82	0.0300	Main Channel
277.99	273.82	0.0300	Main Channel
279.49	273.82	0.0300	Main Channel
280.49	274.27	0.0300 / 0.0500	Main Channel
283.90	274.27	0.0500	
289.81	274.57	0.0500	
325.30	275.75	0.0500	
396.28	278.98	0.0500	
449.51	280.97	0.0500	
496.83	283.90	0.0500	
585.54	287.92	0.0500	

TRAVEL TIME TABLE					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.30	274.12	.273E+04	0.9	0.99	49.81
0.98	274.80	.583E+05	20.8	1.05	46.71
1.66	275.48	.203E+06	95.2	1.38	35.53
2.33	276.15	.438E+06	256.0	1.73	28.50
3.01	276.83	.755E+06	524.8	2.05	23.97
3.69	277.51	.114E+07	935.8	2.43	20.26
4.37	278.19	.157E+07	1472.0	2.76	17.82
5.05	278.87	.206E+07	2140.7	3.06	16.06
5.72	279.54	.261E+07	2938.6	3.33	14.79
6.40	280.22	.321E+07	3887.6	3.57	13.77
7.08	280.90	.390E+07	4795.4	3.63	13.56
7.76	281.58	.468E+07	6196.2	3.91	12.58
8.43	282.25	.550E+07	7803.9	4.19	11.74
9.11	282.93	.637E+07	9601.9	4.45	11.05
9.79	283.61	.728E+07	11594.8	4.70	10.46
10.47	284.29	.823E+07	13845.3	4.96	9.91
11.15	284.97	.922E+07	16293.4	5.21	9.43
11.82	285.64	.103E+08	18952.0	5.45	9.02
12.50	286.32	.113E+08	21824.3	5.69	8.65

hydrograph					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)
INFLOW : ID= 2 (0348)	590.18	2.21	9.00	13.46	0.34
OUTFLOW: ID= 1 (6350)	590.18	2.16	10.25	13.46	0.34

ADD HYD (8220)					
ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (0350):	366.84	2.124	7.50	13.39	
+ ID2= 2 (6350):	590.18	2.158	10.25	13.46	
ID = 3 (8220):	957.02	3.943	8.75	13.43	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (3461)			
ID	Area Total (ha)	Imp (%)	Dir. Conn. (%)
ID= 1 DT=15.0 min	33.62	61.00	38.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)	20.51		13.11
Dep. Storage (mm)	2.00		5.00
Average Slope (%)	0.50		0.50
Length (m)	473.43		40.00
Mannings n	0.013		0.250
Max. Eff. Inten. (mm/hr) over (min)	96.49 / 15.00		86.14 / 30.00
Storage Coeff. (min)	8.11 (ii)		19.47 (ii)
Unit Hyd. Tpeak (min)	15.00		30.00
Unit Hyd. peak (cms)	0.10		0.05

TOTALS			
PEAK FLOW (cms)	3.06	1.61	4.107 (iii)
TIME TO PEAK (hrs)	6.00	6.25	6.00
RUNOFF VOLUME (mm)	71.10	35.19	48.83
TOTAL RAINFALL (mm)	73.10	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.48	0.67

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 69.8 Ia = Dep. Storage (Above)

- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5346)			
IN= 2	OUT= 1	DT= 15.0 min	
0.0000	0.0000	2.0090	0.6158
0.2450	0.2524	2.6260	0.6610
0.6230	0.3644	3.2370	0.7019
0.9920	0.4690	3.6370	0.7119
1.2510	0.5428	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3461)	33.620	4.107	6.00	48.83
OUTFLOW: ID= 1 (5346)	33.620	1.934	6.50	48.83

PEAK FLOW REDUCTION [Qout/Qin] (%) = 47.09
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 0.6258

CALIB NASHYD (3462)			
ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
	317.31	71.0	1.50
	Ia (mm) = 5.00		
	U. H. Tp (hrs) = 0.96		

Unit Hyd Opeak (cms) = 5.625

PEAK FLOW (cms)	4.111 (i)
TIME TO PEAK (hrs)	7.250
RUNOFF VOLUME (mm)	26.696
TOTAL RAINFALL (mm)	73.100
RUNOFF COEFFICIENT	0.365

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9346)					
ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (3462):	317.31	4.111	7.25	26.70	
+ ID2= 2 (5346):	33.62	1.934	6.50	48.83	
ID = 3 (9346):	350.93	5.662	6.50	28.82	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8222)					
ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (8220):	957.02	3.943	8.75	13.43	
+ ID2= 2 (9346):	350.93	5.662	6.50	28.82	
ID = 3 (8222):	1307.95	8.520	7.25	17.56	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6352)	
IN= 2	OUT= 1

<----- DATA FOR SECTION (3521.0) ----->		
Distance	Elevation	Manning
0.00	257.95	0.0500
7.83	257.34	0.0500
15.66	256.19	0.0500
21.53	254.01	0.0500
41.11	244.06	0.0500
76.35	241.38	0.0500
111.58	239.74	0.0500
113.54	239.61	0.0500
113.75	239.41	0.0500 / 0.0300

Main Channel

113.85	238.81	0.0300	Main Channel
115.50	238.81	0.0300	Main Channel
116.15	238.81	0.0300	Main Channel
116.25	239.41	0.0300	Main Channel
117.46	239.52	0.0500	
119.41	239.72	0.0500	
121.37	240.04	0.0500	
131.16	241.84	0.0500	
156.61	247.03	0.0500	
176.19	251.46	0.0500	
193.80	258.79	0.0500	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.60	239.41	.224E+04	2.2	1.52	17.12
1.58	240.39	.216E+05	22.3	1.61	16.16
2.55	241.36	.804E+05	101.0	1.96	13.27
3.53	242.34	.173E+06	275.6	2.48	10.46
4.50	243.31	.293E+06	553.9	2.95	8.80
5.48	244.29	.438E+06	961.8	3.42	7.60
6.45	245.26	.599E+06	1534.4	3.99	6.51
7.43	246.24	.770E+06	2228.4	4.51	5.76
8.41	247.22	.951E+06	3043.6	4.98	5.21
9.38	248.19	.114E+07	3982.1	5.43	4.78
10.36	249.17	.134E+07	5040.1	5.85	4.44
11.33	250.14	.155E+07	6218.5	6.24	4.16
12.31	251.12	.177E+07	7518.6	6.61	3.93
13.29	252.10	.200E+07	8969.0	6.98	3.72
14.26	253.07	.224E+07	10554.1	7.35	3.53
15.24	254.05	.248E+07	12257.1	7.71	3.37
16.21	255.02	.272E+07	14045.1	8.03	3.23
17.19	256.00	.298E+07	15954.8	8.34	3.11
18.16	256.97	.325E+07	17780.4	8.53	3.04

<--- hydrograph --->				<-pi pe / channel -->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8222)	1307.95	8.52	7.25	17.56	0.91
OUTFLOW: ID= 1 (6352)	1307.95	8.47	7.75	17.56	0.90

ADD HYD (8224)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0352):	381.43	3.316	7.00	16.08
+ ID2= 2 (6352):	1307.95	8.472	7.75	17.56
=====				
ID = 3 (8224):	1689.38	11.650	7.50	17.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6354)
IN= 2---> OUT= 1

Routing time step (mi n)' = 15.00

<----- DATA FOR SECTION (3541.0) ----->

Distance	Elevation	Manning	
0.00	253.92	0.0500	
7.95	251.83	0.0500	
15.89	249.97	0.0500	
19.87	249.05	0.0500	
47.68	242.00	0.0500	
67.55	237.10	0.0500	
83.44	231.11	0.0500	
85.43	230.75	0.0500	
85.66	230.61	0.0500 / 0.0300	Main Channel
85.76	230.01	0.0300	Main Channel
87.41	230.01	0.0300	Main Channel
88.06	230.01	0.0300	Main Channel
88.16	230.61	0.0300 / 0.0500	Main Channel
89.40	230.72	0.0500	
91.39	230.88	0.0500	
93.37	231.03	0.0500	
133.11	233.96	0.0500	
154.99	235.39	0.0500	
172.84	239.03	0.0500	
196.68	250.00	0.0500	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.60	230.61	.451E+04	1.5	1.06	49.40
1.62	231.63	.394E+05	14.6	1.16	45.13
2.64	232.65	.128E+06	58.8	1.44	36.30
3.66	233.67	.269E+06	149.7	1.74	30.00
4.68	234.69	.463E+06	301.5	2.04	25.58
5.70	235.71	.708E+06	530.5	2.36	22.14
6.72	236.73	.980E+06	848.4	2.71	19.25
7.74	237.75	.128E+07	1237.9	3.02	17.29
8.76	238.77	.162E+07	1706.7	3.30	15.83
9.78	239.79	.199E+07	2291.9	3.61	14.45
10.81	240.82	.237E+07	2973.7	3.92	13.30
11.83	241.84	.278E+07	3742.2	4.21	12.38
12.85	242.86	.321E+07	4599.1	4.49	11.62
13.87	243.88	.365E+07	5544.6	4.75	10.98
14.89	244.90	.412E+07	6579.7	5.00	10.44
15.91	245.92	.461E+07	7705.9	5.23	9.97
16.93	246.94	.512E+07	8924.6	5.46	9.55
17.95	247.96	.564E+07	10237.5	5.68	9.19
18.97	248.98	.619E+07	11646.2	5.89	8.86

<--- hydrograph --->				<-pi pe / channel -->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8224)	1689.38	11.65	7.50	17.22	1.39
OUTFLOW: ID= 1 (6354)	1689.38	10.76	8.50	17.22	1.32

CALIB
STANDHYD (3541)
ID= 1 DT=15.0 mi n

Area (ha)= 8.01
Total Imp(%)= 75.00
Di r. Conn.(%)= 70.00

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)	6.01		2.00
Dep. Storage (mm)	2.00		5.00
Average Slope (%)	0.50		0.50
Length (m)	231.08		40.00
Mannings n	0.013		0.250

Max. Eff. Inten. (mm/hr)	96.49	11.04
Storage over (mi n)	15.00	45.00
Storage Coeff. (mi n)	5.27 (ii)	31.10 (ii)
Unit Hyd. Tpeak (mi n)	15.00	45.00
Unit Hyd. Tpeak (cms)	0.11	0.03

PEAK FLOW (cms)	1.45	0.04	1.461 (iii)
TIME TO PEAK (hrs)	6.00	6.50	6.00
RUNOFF VOLUME (mm)	71.10	10.44	52.90
TOTAL RAINFALL (mm)	73.10	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.14	0.72

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 35.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5354)
IN= 2---> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.5360	0.2546
0.0270	0.1427	0.6730	0.2758
0.1550	0.1714	0.7830	0.2966
0.2890	0.1975	1.1830	0.3066
0.4010	0.2251	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3541)	8.010	1.461	6.00	52.90
OUTFLOW: ID= 1 (5354)	8.010	0.440	6.25	52.73

PEAK FLOW REDUCTION [Qout/Qi n](%) = 30.12
TIME SHIFT OF PEAK FLOW (mi n) = 15.00
MAXIMUM STORAGE USED (ha. m.) = 0.2397

CALIB NASHYD (3542) Area (ha) = 254.67 Curve Number (CN) = 36.7
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 1.37

Unit Hyd Qpeak (cms) = 3.174

PEAK FLOW (cms) = 0.832 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 9.103
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.125

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9354)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3542):	254.67	0.832	8.00	9.10
+ ID2= 2 (5354):	8.01	0.440	6.25	52.73
=====				
ID = 3 (9354):	262.68	1.025	6.75	10.43

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8226)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6354):	1689.38	10.758	8.50	17.22
+ ID2= 2 (9354):	262.68	1.025	6.75	10.43
=====				
ID = 3 (8226):	1952.06	11.661	8.50	16.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (3402) Area (ha) = 138.83
 ID= 1 DT=15.0 min Total Imp(%) = 46.00 Dir. Conn. (%) = 28.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	63.86	74.97
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	962.05	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 96.49 over (min) = 15.00
 Storage Coeff. (min) = 12.41 (ii) 32.48 (ii)
 Unit Hyd. Tpeak (min) = 15.00
 Unit Hyd. peak (cms) = 0.08

PEAK FLOW (cms) = 8.18 2.58 *TOTALS*
 TIME TO PEAK (hrs) = 6.00 6.50 9.245 (iii)
 RUNOFF VOLUME (mm) = 71.10 17.18 32.28
 TOTAL RAINFALL (mm) = 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.24 0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 47.5 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3401) Area (ha) = 146.96
 ID= 1 DT=15.0 min Total Imp(%) = 58.00 Dir. Conn. (%) = 35.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	85.24	61.72
Dep. Storage	2.00	5.00

Average Slope (%) = 0.50
 Length (m) = 989.81
 Mannings n = 0.013

Max. Eff. Inten. (mm/hr) = 96.49 over (min) = 15.00
 Storage Coeff. (min) = 12.62 (ii) 27.36 (ii)
 Unit Hyd. Tpeak (min) = 15.00
 Unit Hyd. peak (cms) = 0.08

PEAK FLOW (cms) = 10.76 3.33 *TOTALS*
 TIME TO PEAK (hrs) = 6.00 6.25 6.00
 RUNOFF VOLUME (mm) = 71.10 19.43 37.51
 TOTAL RAINFALL (mm) = 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.27 0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 47.5 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5340)
 IN= 2---> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	11.4780	4.3560
	0.3860	2.0347	14.9940	4.9991
	3.2310	2.5818	18.1680	5.6243
	5.1770	3.1417	18.5680	5.6343
	7.7590	3.6318	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (3401)	146.960	12.814	6.00	37.51
OUTFLOW: ID= 1 (5340)	146.960	4.037	6.75	37.50

PEAK FLOW REDUCTION [Qout/Qin] (%) = 31.50
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 2.8465

ADD HYD (8232)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3402):	138.83	9.245	6.00	32.28
+ ID2= 2 (5340):	146.96	4.037	6.75	37.50
=====				
ID = 3 (8232):	285.79	9.571	6.00	34.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0410) Area (ha) = 572.01 Curve Number (CN) = 48.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 1.46

Unit Hyd Qpeak (cms) = 6.698

PEAK FLOW (cms) = 2.643 (i)
 TIME TO PEAK (hrs) = 8.250
 RUNOFF VOLUME (mm) = 13.432
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.184

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0408) Area (ha) = 231.62 Curve Number (CN) = 58.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 0.64

Unit Hyd Qpeak (cms) = 6.198

PEAK FLOW (cms)= 2.757 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 18.036
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.247

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0376) Area (ha)= 463.85 Curve Number (CN)= 74.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 7.380

PEAK FLOW (cms)= 6.058 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 29.204
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.400

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0374) Area (ha)= 545.70 Curve Number (CN)= 61.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 1.51

Unit Hyd Qpeak (cms)= 6.158

PEAK FLOW (cms)= 3.687 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 20.009
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.274

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0372) Area (ha)= 110.42 Curve Number (CN)= 37.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 1.954

PEAK FLOW (cms)= 0.471 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 9.165
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.125

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0370) Area (ha)= 191.85 Curve Number (CN)= 63.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.67

Unit Hyd Qpeak (cms)= 4.860

PEAK FLOW (cms)= 2.560 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 20.954
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.287

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0366) Area (ha)= 462.62 Curve Number (CN)= 62.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 1.06

Unit Hyd Qpeak (cms)= 7.451

PEAK FLOW (cms)= 4.212 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 20.530
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.281

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3641) Area (ha)= 7.85
 ID= 1 DT=15.0 min Total Imp(%)= 45.00 Di r. Conn.(%)= 16.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	3.53	4.32
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	228.76	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	23.12
over (min)	15.00	30.00
Storage Coeff. (min)=	5.24 (ii)	24.46 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.11	0.04

		TOTALS
PEAK FLOW (cms)=	0.32	0.21
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	71.10	16.74
TOTAL RAINFALL (mm)=	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.23
		0.452 (iii)
		6.00
		25.44
		73.10
		0.35

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 42.9 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5364) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	0.5250	0.2044
	0.0270	0.0959	0.6590	0.2337
	0.1520	0.1110	0.7670	0.2624
	0.2830	0.1432	1.1670	0.2724
	0.3930	0.1710	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3641)	7.850	0.452	6.00	25.44
OUTFLOW: ID= 1 (5364)	7.850	0.121	6.75	25.33

PEAK FLOW REDUCTION [Qout/Qin](%)= 26.83
 TIME SHIFT OF PEAK FLOW (min)= 45.00
 MAXIMUM STORAGE USED (ha.m.)= 0.1078

CALIB STANDHYD (3642) Area (ha)= 147.42
 ID= 1 DT=15.0 min Total Imp(%)= 21.00 Di r. Conn.(%)= 7.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	30.96	116.46
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	991.36	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	13.98
over (min)	15.00	45.00
Storage Coeff. (min)=	12.64 (ii)	36.14 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03

TOTALS

PEAK FLOW (cms)= 2.16 2.57 3.194 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 71.10 13.32 17.36
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.18 0.24

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 42.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9364) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3642):	147.42	3.194	6.00	17.36
+ ID2= 2 (5364):	7.85	0.121	6.75	25.33
ID = 3 (9364):	155.27	3.209	6.00	17.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8302) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0366):	462.62	4.212	7.50	20.53
+ ID2= 2 (9364):	155.27	3.209	6.00	17.76
ID = 3 (8302):	617.89	6.845	6.50	19.83

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0358) Area (ha)= 429.87 Curve Number (CN)= 35.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.03

Unit Hyd Opeak (cms)= 7.091
 PEAK FLOW (cms)= 1.611 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 8.508
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.116

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0360) Area (ha)= 138.37 Curve Number (CN)= 46.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.60

Unit Hyd Opeak (cms)= 3.957
 PEAK FLOW (cms)= 1.171 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 12.385
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.169

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8306) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0358):	429.87	1.611	7.50	8.51
+ ID2= 2 (0360):	138.37	1.171	6.75	12.38

===== ID = 3 (8306): 568.24 2.716 7.00 9.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6362) Routing time step (min)= 15.00
 IN= 2----> OUT= 1

<----- DATA FOR SECTION (3621.0) ----->

Distance	Elevation	Manning
0.00	261.46	0.0550
27.86	254.23	0.0550
51.07	251.96	0.0550
74.29	250.77	0.0550
97.50	249.91	0.0550
125.36	249.40	0.0550
150.93	247.40	0.0550
155.93	247.33	0.0550 / 0.0350
157.93	246.85	0.0350
159.18	246.65	0.0350
160.18	246.63	0.0350
160.93	246.85	0.0350
161.93	247.18	0.0350 / 0.0550
163.18	248.03	0.0550
168.18	248.58	0.0550
183.18	250.18	0.0550
201.97	252.59	0.0550
213.57	256.02	0.0550
225.18	260.31	0.0550
229.82	261.00	0.0550

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.55	247.18	.556E+04	1.9	1.03	49.95
1.28	247.91	.336E+05	16.7	1.53	33.61
2.00	248.63	.913E+05	53.6	1.81	28.40
2.73	249.36	.185E+06	122.3	2.05	25.18
3.46	250.09	.344E+06	224.6	2.02	25.52
4.19	250.82	.574E+06	417.9	2.25	22.91
4.91	251.54	.855E+06	699.1	2.53	20.38
5.64	252.27	.118E+07	1075.3	2.82	18.26
6.37	253.00	.153E+07	1559.9	3.15	16.38
7.10	253.73	.191E+07	2137.6	3.46	14.89
7.82	254.45	.231E+07	2815.2	3.77	13.67
8.55	255.18	.272E+07	3608.5	4.10	12.58
9.28	255.91	.315E+07	4488.2	4.41	11.69
10.01	256.64	.358E+07	5458.0	4.71	10.95
10.73	257.36	.403E+07	6512.4	4.99	10.32
11.46	258.09	.449E+07	7649.8	5.27	9.78
12.19	258.82	.496E+07	8869.5	5.53	9.32
12.92	259.55	.544E+07	10171.2	5.78	8.91
13.64	260.27	.593E+07	11554.8	6.02	8.55

<---- hydrograph ----> <-- pipe / channel -->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8306)	568.24	2.72	7.00	9.45	0.59	1.05
OUTFLOW: ID= 1 (6362)	568.24	2.40	8.25	9.45	0.58	1.04

CALIB STANDHYD (3621) Area (ha)= 11.87
 ID= 1 DT=15.0 min Total Imp(%)= 45.00 Di r. Conn. (%)= 17.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	5.34	6.53
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	281.31	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	96.49	24.75
over (min)=	15.00	30.00
Storage Coeff. (min)=	5.93 (ii)	24.64 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.10	0.04

PEAK FLOW (cms)= 0.51 0.34 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.25 0.720 (iii)
 6.00

RUNOFF VOLUME (mm) = 71.10 18.05 27.07
 TOTAL RAINFALL (mm) = 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.25 0.37

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 45.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5362)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.7940	0.3091
0.0400	0.1451	0.9970	0.3534
0.2300	0.1678	1.1610	0.3968
0.4280	0.2166	1.5610	0.4068
0.5940	0.2586	0.0000	0.0000

INFLOW : ID= 2 (3621) AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
 11.870 0.720 6.00 27.07
 OUTFLOW: ID= 1 (5362) 11.870 0.222 6.75 26.99

PEAK FLOW REDUCTION [Out/On] (%) = 30.81
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 0.1671

CALIB STANDHYD (3622)
 ID= 1 DT=15.0 min

Area (ha) = 106.91
 Total Imp(%) = 24.00 Dir. Conn. (%) = 9.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 25.66 81.25
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 844.24 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 96.49 15.90
 over (min) = 15.00 45.00
 Storage Coeff. (min) = 11.47 (ii) 33.80 (ii)
 Unit Hyd. Tpeak (min) = 15.00 45.00
 Unit Hyd. peak (cms) = 0.08 0.03

PEAK FLOW (cms) = 2.08 2.11 2.939 (iii)
 TIME TO PEAK (hrs) = 6.00 6.50 6.00
 RUNOFF VOLUME (mm) = 71.10 14.80 19.87
 TOTAL RAINFALL (mm) = 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.20 0.27

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 45.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9362)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (3622):	106.91	2.939	6.00 19.87
+ ID2= 2 (5362):	11.87	0.222	6.75 26.99
ID = 3 (9362):	118.78	2.962	6.00 20.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8304)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6362):	568.24	2.400	8.25 9.45
+ ID2= 2 (9362):	118.78	2.962	6.00 20.58
ID = 3 (8304):	687.02	3.973	6.50 11.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8300)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8302):	617.89	6.845	6.50 19.83
+ ID2= 2 (8304):	687.02	3.973	6.50 11.38
ID = 3 (8300):	1304.91	10.818	6.50 15.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6368)
 IN= 2--> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (3681.0) ----->

Distance	Elevation	Manning	
0.00	230.00	0.0370	
18.48	223.26	0.0370	
36.96	223.05	0.0370	
64.67	222.94	0.0370	
110.87	222.86	0.0370	
133.96	222.74	0.0370	
147.82	222.65	0.0370	
170.92	222.31	0.0370	
174.79	222.26	0.0370 / 0.0300	Main Channel
174.89	221.86	0.0300	Main Channel
175.54	221.86	0.0300	Main Channel
176.19	221.86	0.0300	Main Channel
176.29	222.26	0.0300 / 0.0370	Main Channel
180.16	222.25	0.0370	
184.78	222.28	0.0370	
189.40	222.31	0.0370	
332.60	222.37	0.0370	
450.00	230.00	0.0370	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.40	222.26	.771E+03	0.3	0.52	41.74
0.81	222.67	.784E+05	30.3	0.51	43.17
1.21	223.07	.210E+06	123.2	0.77	28.42
1.62	223.48	.384E+06	301.0	1.03	21.25
2.03	223.89	.563E+06	550.2	1.28	17.06
2.44	224.30	.747E+06	860.7	1.51	14.46
2.84	224.70	.934E+06	1228.1	1.73	12.68
3.25	225.11	.113E+07	1649.3	1.92	11.37
3.66	225.52	.132E+07	2122.3	2.11	10.37
4.07	225.93	.152E+07	2645.5	2.29	9.58
4.47	226.33	.172E+07	3217.8	2.45	8.92
4.88	226.74	.193E+07	3838.3	2.61	8.38
5.29	227.15	.214E+07	4506.3	2.76	7.92
5.70	227.56	.236E+07	5221.3	2.91	7.52
6.10	227.96	.258E+07	5983.0	3.05	7.17
6.51	228.37	.280E+07	6790.9	3.19	6.87
6.92	228.78	.303E+07	7645.0	3.32	6.60
7.33	229.19	.326E+07	8545.1	3.45	6.35
7.73	229.59	.349E+07	9491.0	3.57	6.13

<---- hydrograph ----> <- pipe / channel ->
 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm) MAX DEPTH (m) MAX VEL (m/s)
 INFLOW : ID= 2 (8300) 1304.91 10.82 6.50 15.38 0.54 0.52
 OUTFLOW: ID= 1 (6368) 1304.91 8.71 7.50 15.38 0.51 0.52

CALIB

STANDHYD (3681) | Area (ha)= 2.95
 ID= 1 DT=15.0 min | Total Imp(%)= 45.00 Dir. Conn.(%)= 15.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 1.33 1.62
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 140.24 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 96.49 21.89
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 3.91 (ii) 23.55 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.11 0.04

TOTALS

PEAK FLOW (cms)= 0.12 0.08 0.163 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 71.10 15.73 24.03
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.22 0.33

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 40.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5368)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.1970	0.0767
0.0100	0.0360	0.2470	0.0877
0.0570	0.0416	0.2880	0.0984
0.1060	0.0537	0.6880	0.1084
0.1470	0.0642	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (3681)	2.950	0.163	6.00	24.03
OUTFLOW: ID= 1 (5368)	2.950	0.040	7.00	23.73

PEAK FLOW REDUCTION [Out/Oin](%)= 24.30
 TIME SHIFT OF PEAK FLOW (min)= 60.00
 MAXIMUM STORAGE USED (ha.m.)= 0.0398

CALIB NASHYD (3682)
 ID= 1 DT=15.0 min

Area (ha)= 156.53 Curve Number (CN)= 45.9
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 2.778
 PEAK FLOW (cms)= 0.918 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 12.484
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.171

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9368)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3682):	156.53	0.918	7.25	12.48
+ ID2= 2 (5368):	2.95	0.040	7.00	23.73
=====				
ID = 3 (9368):	159.48	0.953	7.25	12.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8298)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6368):	1304.91	8.705	7.50	15.38
+ ID2= 2 (9368):	159.48	0.953	7.25	12.69
=====				
ID = 3 (8298):	1464.39	9.649	7.50	15.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8296)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0370):	191.85	2.560	6.75	20.95
+ ID2= 2 (8298):	1464.39	9.649	7.50	15.09
=====				
ID = 3 (8296):	1656.24	11.977	7.50	15.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6372)
 IN= 2--> OUT= 1

Routing time step (min)= 15.00

<----- DATA FOR SECTION (3721.0) ----->

Distance	Elevation	Manning	
0.00	225.00	0.0390	
30.80	219.38	0.0390	
61.61	219.30	0.0390	
77.01	219.27	0.0390	
469.76	219.14	0.0390	
477.46	219.13	0.0390	
485.16	219.10	0.0390	
492.86	219.09	0.0390	
495.56	219.09	0.0390 / 0.0310	Main Channel
495.66	218.51	0.0310	Main Channel
500.56	218.51	0.0310	Main Channel
505.46	218.51	0.0310	Main Channel
505.56	219.09	0.0310 / 0.0390	Main Channel
508.26	219.09	0.0390	
515.96	219.10	0.0390	
523.67	219.21	0.0390	
562.17	219.32	0.0390	
654.58	219.43	0.0390	
731.59	219.46	0.0390	
762.39	225.00	0.0390	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.29	218.80	.431E+04	1.7	0.58	43.48
0.58	219.09	.867E+04	5.1	0.89	28.32
0.93	219.44	.200E+06	58.2	0.44	57.35
1.28	219.79	.568E+06	278.6	0.74	33.99
1.62	220.13	.939E+06	630.2	1.01	24.84
1.97	220.48	.131E+07	1090.4	1.25	20.05
2.32	220.83	.169E+07	1647.7	1.47	17.06
2.67	221.18	.206E+07	2294.6	1.68	14.99
3.01	221.52	.244E+07	3025.6	1.87	13.46
3.36	221.87	.282E+07	3836.5	2.05	12.27
3.71	222.22	.321E+07	4723.7	2.22	11.31
4.06	222.57	.359E+07	5684.6	2.39	10.53
4.40	222.91	.398E+07	6716.6	2.55	9.87
4.75	223.26	.437E+07	7817.7	2.70	9.31
5.10	223.61	.476E+07	8986.2	2.85	8.83
5.45	223.96	.515E+07	10220.4	2.99	8.40
5.79	224.30	.555E+07	11519.1	3.13	8.03
6.14	224.65	.594E+07	12880.8	3.27	7.69
6.49	225.00	.634E+07	14304.5	3.40	7.39

<---- hydrograph ---->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8296)	1656.24	11.98	7.50	15.77	0.63	0.78
OUTFLOW: ID= 1 (6372)	1656.24	11.36	8.25	15.77	0.62	0.79

ADD HYD	(8294)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0372):		110.42	0.471	7.25	9.16
+ ID2= 2 (6372):		1656.24	11.364	8.25	15.77
=====					
ID = 3 (8294):		1766.66	11.808	8.25	15.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8292)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0374):		545.70	3.687	8.25	20.01
+ ID2= 2 (8294):		1766.66	11.808	8.25	15.35
=====					
ID = 3 (8292):		2312.36	15.495	8.25	16.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5505)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 15.0 min				
	0.0000	0.0000	65.1290	345.3754
	25.4850	24.6697	84.9510	456.3890
	31.1490	98.6787	*****	838.7689
	39.6440	*****	*****	838.7789
	48.1390	*****	0.0000	0.0000

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8292)	2312.360	15.495	8.25	16.45
OUTFLOW: ID= 1 (5505)	2312.360	11.092	11.00	16.45

PEAK FLOW REDUCTION [Qout/Qin] (%) = 71.59
 TIME SHIF OF PEAK FLOW (min) = 165.00
 MAXIMUM STORAGE USED (ha.m.) = 10.7426

ADD HYD	(8272)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0376):		463.85	6.058	7.50	29.20
+ ID2= 2 (5505):		2312.36	11.092	11.00	16.45
=====					
ID = 3 (8272):		2776.21	15.089	10.00	18.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	NASHYD	Area	(ha)	Curve Number	(CN)
ID= 1 DT=15.0 min		Ia	(mm)	# of Linear Res.	(N)
		U. H. Tp(hrs)			
	(0396)	305.21	5.00	69.0	1.50
			1.08		

Unit Hyd Opeak (cms) = 4.811

PEAK FLOW (cms) = 3.390 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 25.220
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.345

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD	Area	(ha)	Curve Number	(CN)
ID= 1 DT=15.0 min		Ia	(mm)	# of Linear Res.	(N)
		U. H. Tp(hrs)			
	(0394)	325.45	5.00	53.0	1.50
			0.92		

Unit Hyd Opeak (cms) = 6.013

PEAK FLOW (cms) = 2.485 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 15.628
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.214

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD	Area	(ha)	Curve Number	(CN)
ID= 1 DT=15.0 min		Ia	(mm)	# of Linear Res.	(N)
		U. H. Tp(hrs)			
	(0390)	420.00	5.00	55.0	1.50
			1.07		

Unit Hyd Opeak (cms) = 6.683

PEAK FLOW (cms) = 3.048 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 16.653
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.228

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD	Area	(ha)	Curve Number	(CN)
ID= 1 DT=15.0 min		Ia	(mm)	# of Linear Res.	(N)
		U. H. Tp(hrs)			
	(0388)	220.77	5.00	58.0	1.50
			0.99		

Unit Hyd Opeak (cms) = 3.819

PEAK FLOW (cms) = 1.875 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 18.209
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.249

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	NASHYD	Area	(ha)	Curve Number	(CN)
ID= 1 DT=15.0 min		Ia	(mm)	# of Linear Res.	(N)
		U. H. Tp(hrs)			
	(0386)	241.27	5.00	61.0	1.50
			0.90		

Unit Hyd Opeak (cms) = 4.562

PEAK FLOW (cms) = 2.408 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 19.881
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.272

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	(8286)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0386):		241.27	2.408	7.25	19.88
+ ID2= 2 (0388):		220.77	1.875	7.25	18.21
=====					
ID = 3 (8286):		462.04	4.284	7.25	19.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	NASHYD	Area	(ha)	Curve Number	(CN)
ID= 1 DT=15.0 min		Ia	(mm)	# of Linear Res.	(N)
		U. H. Tp(hrs)			
	(0384)	199.07	5.00	44.0	1.50
			0.96		

Unit Hyd Opeak (cms) = 3.537

PEAK FLOW (cms) = 1.095 (i)
 TIME TO PEAK (hrs) = 7.250

RUNOFF VOLUME (mm)= 11.721
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.160

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0380) Area (ha)= 182.01 Curve Number (CN)= 40.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.55

Unit Hyd Opeak (cms)= 5.609

PEAK FLOW (cms)= 1.322 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 10.074
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.138

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0382) Area (ha)= 216.59 Curve Number (CN)= 53.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.64

Unit Hyd Opeak (cms)= 5.733

PEAK FLOW (cms)= 2.181 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 15.501
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.212

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0380):	182.01	1.322	6.50	10.07
+ ID2= 2 (0382):	216.59	2.181	6.75	15.50

ID = 3 (8290):	398.60	3.495	6.75	13.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6384)
 IN= 2--> OUT= 1 Routing time step (min)' = 15.00

Distance	Elevation	Manning	
0.00	294.40	0.0380	
10.59	291.93	0.0380	
21.17	289.19	0.0380	
26.46	287.99	0.0380	
31.76	286.79	0.0380	
71.45	279.97	0.0380	
74.10	279.79	0.0380	
76.74	279.71	0.0380	
78.99	279.64	0.0380 / 0.0300	Main Channel
79.14	279.30	0.0300	Main Channel
79.39	279.30	0.0300	Main Channel
79.64	279.30	0.0300	Main Channel
79.79	279.64	0.0300 / 0.0380	Main Channel
89.98	279.78	0.0380	
119.09	281.86	0.0380	
145.55	282.87	0.0380	
198.48	284.85	0.0380	
211.71	286.31	0.0380	
230.23	287.59	0.0380	
261.99	294.00	0.0380	

TRAVEL TIME TABLE
 DEPTH (m) ELEV (m) VOLUME (cu. m.) FLOW RATE (cms) VELOCITY (m/s) TRAV. TIME (min)

0.34	279.64	.715E+03	0.2	0.85	63.42
1.10	280.40	.507E+05	21.9	1.40	38.54
1.85	281.15	.141E+06	91.8	2.10	25.69
2.61	281.91	.269E+06	218.9	2.63	20.46
3.36	282.66	.445E+06	407.1	2.96	18.23
4.12	283.42	.681E+06	698.3	3.31	16.26
4.87	284.17	.977E+06	1111.8	3.68	14.65
5.63	284.93	.133E+07	1674.3	4.06	13.27
6.39	285.69	.173E+07	2457.8	4.59	11.73
7.14	286.44	.215E+07	3378.8	5.07	10.63
7.90	287.20	.261E+07	4418.4	5.47	9.85
8.65	287.95	.310E+07	5676.3	5.92	9.11
9.41	288.71	.361E+07	7150.5	6.40	8.42
10.17	289.47	.414E+07	8783.1	6.86	7.86
10.92	290.22	.469E+07	10575.1	7.30	7.38
11.68	290.98	.525E+07	12520.4	7.72	6.98
12.43	291.73	.582E+07	14619.0	8.12	6.64
13.19	292.49	.642E+07	16863.2	8.50	6.34
13.94	293.24	.703E+07	19259.8	8.86	6.08

<--- hydrograph --->
 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm) MAX DEPTH (m) MAX VEL (m/s)
 INFLOW: ID= 2 (8290) 398.60 3.50 6.75 13.02 0.45 0.90
 OUTFLOW: ID= 1 (6384) 398.60 2.74 8.00 13.02 0.43 0.89

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0384):	199.07	1.095	7.25	11.72
+ ID2= 2 (6384):	398.60	2.742	8.00	13.02

ID = 3 (8288):	597.67	3.810	7.75	12.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (8286):	462.04	4.284	7.25	19.08
+ ID2= 2 (8288):	597.67	3.810	7.75	12.59

ID = 3 (8284):	1059.71	8.003	7.50	15.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0390):	420.00	3.048	7.50	16.65
+ ID2= 2 (8284):	1059.71	8.003	7.50	15.42

ID = 3 (8280):	1479.71	11.051	7.50	15.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0392) Area (ha)= 167.22 Curve Number (CN)= 62.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.74

Unit Hyd Opeak (cms)= 3.837

PEAK FLOW (cms)= 2.002 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 20.396
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.279

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0378) Area (ha) = 606.72 Curve Number (CN) = 55.0
 ID= 1 DT=15.0 min I a (mm) = 5.00 # of Li near Res. (N) = 1.50
 U. H. Tp(hrs) = 1.18

Unit Hyd Qpeak (cms) = 8.771

PEAK FLOW (cms) = 4.102 (i)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 16.674
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.228

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8282)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0378):	606.72	4.102	7.75	16.67
+ ID2= 2 (0392):	167.22	2.002	7.00	20.40
ID = 3 (8282):	773.94	5.981	7.25	17.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8278)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8280):	1479.71	11.051	7.50	15.77
+ ID2= 2 (8282):	773.94	5.981	7.25	17.48
ID = 3 (8278):	2253.65	17.022	7.50	16.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6394)
 IN= 2--> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3941.0) ----->

Di stance	Elevation	Manning	
0.00	283.00	0.0380	
13.48	282.87	0.0380	
53.92	280.08	0.0380	
74.13	276.62	0.0380	
97.72	265.45	0.0380	
114.57	256.93	0.0380	
131.42	253.04	0.0380	
134.79	252.58	0.0380	
138.53	251.74	0.0380 / 0.0300	Main Channel
139.03	251.20	0.0300	Main Channel
141.53	251.20	0.0300	Main Channel
144.03	251.20	0.0300	Main Channel
144.53	251.74	0.0300 / 0.0380	Main Channel
148.27	252.69	0.0380	
151.64	252.97	0.0380	
185.34	255.08	0.0380	
219.03	257.54	0.0380	
262.84	259.43	0.0380	
310.02	262.80	0.0380	
333.60	283.00	0.0380	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.54	251.74	.827E+04	4.5	1.51	30.71
2.19	253.39	.758E+05	73.2	2.69	17.27
3.83	255.03	.282E+06	331.8	3.27	14.17
5.48	256.68	.633E+06	917.3	4.03	11.51
7.12	258.32	.112E+07	1889.5	4.69	9.90
8.77	259.97	.179E+07	3410.2	5.31	8.73
10.41	261.61	.259E+07	5627.6	6.05	7.66
12.06	263.26	.351E+07	8605.1	6.83	6.79
13.70	264.90	.447E+07	12544.9	7.81	5.94
15.35	266.55	.546E+07	17101.4	8.71	5.32
16.99	268.19	.648E+07	22251.7	9.56	4.85

18.64	269.84	.752E+07	27979.0	10.36	4.48
20.28	271.48	.858E+07	34269.0	11.11	4.17
21.93	273.13	.967E+07	41110.8	11.83	3.92
23.57	274.77	.108E+08	48495.9	12.51	3.71
25.22	276.42	.119E+08	56418.1	13.17	3.52
26.86	278.06	.131E+08	64410.6	13.69	3.39
28.51	279.71	.143E+08	73002.1	14.18	3.27
30.15	281.35	.156E+08	81379.3	14.50	3.20

<---- hydrograph ----> <-pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8278)	2253.65	17.02	7.50	16.36	0.84	1.64
OUTFLOW: ID= 1 (6394)	2253.65	16.55	8.00	16.36	0.83	1.64

ADD HYD (8276)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0394):	325.45	2.485	7.25	15.63
+ ID2= 2 (6394):	2253.65	16.553	8.00	16.36
ID = 3 (8276):	2579.10	18.926	8.00	16.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6396)
 IN= 2--> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3961.0) ----->

Di stance	Elevation	Manning	
0.00	263.00	0.0410	
11.75	257.14	0.0410	
23.50	253.97	0.0410	
41.13	247.83	0.0410	
76.38	232.09	0.0410	
135.13	229.07	0.0410	
149.82	228.97	0.0410	
152.75	228.96	0.0410 / 0.0300	Main Channel
154.19	228.73	0.0300	Main Channel
154.69	228.20	0.0300	Main Channel
155.69	228.20	0.0300	Main Channel
156.69	228.20	0.0300	Main Channel
157.19	228.73	0.0300	Main Channel
158.63	228.95	0.0300 / 0.0410	Main Channel
161.57	228.96	0.0410	
164.51	229.71	0.0410	
196.82	241.70	0.0410	
223.26	249.21	0.0410	
246.76	255.13	0.0410	
290.82	263.51	0.0410	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.75	228.95	.780E+04	2.2	0.98	58.06
2.54	230.74	.277E+06	145.1	1.78	31.76
4.33	232.53	.786E+06	609.1	2.63	21.50
6.13	234.33	.140E+07	1481.5	3.59	15.77
7.92	236.12	.207E+07	2677.3	4.39	12.90
9.71	237.91	.280E+07	4184.8	5.09	11.14
11.50	239.70	.357E+07	6001.4	5.71	9.93
13.29	241.49	.441E+07	8129.1	6.27	9.03
15.09	243.29	.530E+07	10549.4	6.77	8.37
16.88	245.08	.625E+07	13300.6	7.24	7.83
18.67	246.87	.726E+07	16396.2	7.68	7.38
20.46	248.66	.834E+07	19809.8	8.07	7.02
22.26	250.46	.949E+07	23532.5	8.43	6.72
24.05	252.25	.107E+08	27641.2	8.77	6.46
25.84	254.04	.120E+08	32155.2	9.10	6.22
27.63	255.83	.134E+08	36901.0	9.37	6.05
29.42	257.62	.149E+08	42147.7	9.64	5.88
31.22	259.42	.164E+08	48149.1	9.97	5.68
33.01	261.21	.181E+08	54644.8	10.29	5.51

<---- hydrograph ----> <-pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8276)	2579.10	18.93	8.00	16.26	0.96	1.03

OUTFLOW: ID= 1 (6396) 2579.10 17.09 9.00 16.26 0.93 1.02

ADD HYD	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0396):	305.21	3.390	7.50	25.22
+ ID2= 2 (6396):	2579.10	17.092	9.00	16.26

ID = 3 (8274):	2884.31	20.021	9.00	17.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8272):	2776.21	15.089	10.00	18.58
+ ID2= 2 (8274):	2884.31	20.021	9.00	17.21

ID = 3 (8270):	5660.52	34.607	9.25	17.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5506)
IN= 2---> OUT= 1
DT= 15.0 mi n

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha. m.)	(cms)	(ha. m.)
0.0000	0.0000	60.8810	135.6832
31.1490	24.6697	96.2770	900.4431
36.8120	37.0045	96.6770	900.4531
45.3070	86.3439	0.0000	0.0000

INFLOW	AREA	OPEAK	TPEAK	R. V.
ID= 2 (8270)	(ha)	(cms)	(hrs)	(mm)
OUTFLOW: ID= 1 (5506)	5660.520	34.607	9.25	17.88
	5660.520	28.433	11.75	17.88

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 82.16
TIME SHIFT OF PEAK FLOW (mi n) = 150.00
MAXIMUM STORAGE USED (ha. m.) = 22.5372

CALIB
NASHYD (0406)
ID= 1 DT=15.0 mi n

Area (ha) = 142.65 Curve Number (CN) = 66.0
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 0.59

Uni t Hyd Qpeak (cms) = 4.135
PEAK FLOW (cms) = 2.315 (i)
TIME TO PEAK (hrs) = 6.500
RUNOFF VOLUME (mm) = 22.790
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.312

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0404)
ID= 1 DT=15.0 mi n

Area (ha) = 246.46 Curve Number (CN) = 47.0
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 0.98

Uni t Hyd Qpeak (cms) = 4.280
PEAK FLOW (cms) = 1.475 (i)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 12.944
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.177

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0402)
ID= 1 DT=15.0 mi n

Area (ha) = 244.00 Curve Number (CN) = 61.0
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 1.07

Uni t Hyd Qpeak (cms) = 3.879
PEAK FLOW (cms) = 2.133 (i)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 19.936
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.273

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0400)
ID= 1 DT=15.0 mi n

Area (ha) = 93.97 Curve Number (CN) = 52.0
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 0.44

Uni t Hyd Qpeak (cms) = 3.630
PEAK FLOW (cms) = 1.222 (i)
TIME TO PEAK (hrs) = 6.500
RUNOFF VOLUME (mm) = 14.807
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.203

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0398)
ID= 1 DT=15.0 mi n

Area (ha) = 328.19 Curve Number (CN) = 55.0
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 0.83

Uni t Hyd Qpeak (cms) = 6.759
PEAK FLOW (cms) = 2.909 (i)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 16.581
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.227

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8268)
1 + 2 = 3

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0398):	328.19	2.909	7.00	16.58
+ ID2= 2 (0400):	93.97	1.222	6.50	14.81

ID = 3 (8268):	422.16	4.012	6.75	16.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6402)
IN= 2---> OUT= 1

Routing time step (mi n)' = 15.00

<----- DATA FOR SECTION (4021.0) ----->		
Di stance	El evation	Manni ng
0.00	238.50	0.0360
11.50	238.00	0.0360
23.00	237.93	0.0360
34.49	236.39	0.0360
63.24	233.98	0.0360
97.73	228.15	0.0360
123.60	227.08	0.0360
126.48	226.61	0.0360
127.60	226.47	0.0360 / 0.0330
127.85	225.25	0.0330
129.35	225.25	0.0330
130.85	225.25	0.0330
131.10	226.47	0.0330 / 0.0360
132.22	226.59	0.0360

143.72	227.42	0.0360
169.59	227.88	0.0360
192.59	231.19	0.0360
218.46	233.02	0.0360
241.45	235.50	0.0360
284.57	236.43	0.0360

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	TRAVEL TIME (mins)	VELOCITY (m/s)	TRAV. TIME (min)
0.41	225.66	391E+04	1.7	1.36	38.31
0.81	226.06	804E+04	5.0	1.93	27.02
1.22	226.47	124E+05	9.1	2.30	22.61
1.84	227.09	308E+05	23.3	2.36	22.03
2.46	227.71	904E+05	62.0	2.14	24.31
3.09	228.34	223E+06	188.3	2.63	19.75
3.71	228.96	379E+06	410.2	3.38	15.39
4.33	229.58	550E+06	711.5	4.04	12.89
4.95	230.20	737E+06	1091.3	4.62	11.25
5.58	230.83	939E+06	1550.6	5.15	10.10
6.20	231.45	116E+07	2072.4	5.59	9.31
6.82	232.07	140E+07	2659.2	5.93	8.77
7.44	232.69	167E+07	3345.8	6.27	8.30
8.07	233.32	195E+07	4159.0	6.64	7.83
8.69	233.94	226E+07	5100.8	7.03	7.39
9.31	234.56	259E+07	6081.3	7.32	7.11
9.93	235.18	295E+07	7183.1	7.60	6.84
10.56	235.81	334E+07	8145.2	7.62	6.83
11.18	236.43	379E+07	9116.0	7.51	6.92

<--- hydrograph --->					<--- pipe / channel --->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8268)	422.16	4.01	6.75	16.19	0.70	1.72
OUTFLOW: ID= 1 (6402)	422.16	3.72	7.50	16.19	0.66	1.66

ADD HYD (8266)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
ID1= 1 (0402):	244.00	2.133	7.50	19.94	
+ ID2= 2 (6402):	422.16	3.724	7.50	16.19	
ID = 3 (8266):	666.16	5.857	7.50	17.56	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8264)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
ID1= 1 (0404):	246.46	1.475	7.25	12.94	
+ ID2= 2 (8266):	666.16	5.857	7.50	17.56	
ID = 3 (8264):	912.62	7.330	7.50	16.31	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8262)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
ID1= 1 (0406):	142.65	2.315	6.50	22.79	
+ ID2= 2 (8264):	912.62	7.330	7.50	16.31	
ID = 3 (8262):	1055.27	9.418	7.25	17.19	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8260)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
ID1= 1 (5506):	5660.52	28.433	11.75	17.88	
+ ID2= 2 (8262):	1055.27	9.418	7.25	17.19	

=====
ID = 3 (8260): 6715.79 32.587 11.00 17.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8258)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
ID1= 1 (0408):	231.62	2.757	6.75	18.04	
+ ID2= 2 (8260):	6715.79	32.587	11.00	17.78	
ID = 3 (8258):	6947.41	33.435	11.00	17.78	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8256)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
ID1= 1 (0410):	572.01	2.643	8.25	13.43	
+ ID2= 2 (8258):	6947.41	33.435	11.00	17.78	
ID = 3 (8256):	7519.42	35.591	10.75	17.45	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5507)					
IN= 2---> OUT= 1					
DT= 15.0 min					
	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)	
	0.0000	0.0000	90.6140	67.8416	
	39.6440	12.3348	*****	160.3529	
	48.1390	18.5023	*****	160.3629	
	67.9600	37.0045	0.0000	0.0000	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	
INFLOW: ID= 2 (8256)	7519.421	35.591	10.75	17.45	
OUTFLOW: ID= 1 (5507)	7519.421	34.824	11.75	17.45	

PEAK FLOW REDUCTION [Out/Oi n] (%) = 97.85
TIME SHIFT OF PEAK FLOW (min) = 60.00
MAXIMUM STORAGE USED (ha.m.) = 10.8364

CALIB NASHYD (0420)					
ID= 1 DT=15.0 min					
	Area (ha)	Imp (%)	Curve Number	Di r. Conn. (%)	
	175.82	5.00	53.0		
	U.H. Tp (hrs) =	0.81	# of Linear Res. (N) =	1.50	

Unit Hyd Opeak (cms) = 3.692
PEAK FLOW (cms) = 1.485 (i)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 15.590
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.213

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (4141)					
ID= 1 DT=15.0 min					
	Area (ha)	Imp (%)	Di r. Conn. (%)		
	156.72	52.00	21.00		
	IMPERVIOUS	PERVIOUS (i)			
Surface Area	(ha) = 81.49	75.23			
Dep. Storage	(mm) = 2.00	5.00			
Average Slope	(%) = 0.50	0.50			
Length	(m) = 1022.15	40.00			
Mannings n	= 0.013	0.250			
Max. Eff. Inten. (mm/hr) =	96.49	84.49			
over (min) =	15.00	30.00			
Storage Coeff. (min) =	12.87 (ii)	24.31 (ii)			

Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

PEAK FLOW (cms)= 6.83 8.20
 TIME TO PEAK (hrs)= 6.00 6.25
 RUNOFF VOLUME (mm)= 71.10 33.45
 TOTAL RAINFALL (mm)= 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.46

TOTALS
 12.089 (iii)
 6.00
 41.35
 73.10
 0.57

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (4144)
 ID= 1 DT=15.0 min

Area (ha)= 2.10
 Total Imp(%)= 30.00 Dir. Conn.(%)= 10.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 0.63 1.47
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 118.32 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 96.49 78.76
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 3.53 (ii) 15.30 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.11 0.05

TOTALS
 0.190 (iii)
 6.25
 42.52
 73.10
 0.58

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9429)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.1000	0.0386
0.0100	0.0195	0.1400	0.0480
0.0400	0.0218	0.1700	0.0569
0.0700	0.0312	0.2000	0.0655

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (4144)	2.100	0.190	6.25	42.52
OUTFLOW: ID= 1 (9429)	2.100	0.094	6.50	42.30

PEAK FLOW REDUCTION [Qout/Qin](%)= 49.43
 TIME SHIFT OF PEAK FLOW (min)= 15.00
 MAXIMUM STORAGE USED (ha.m.)= 0.0377

ADD HYD (9424)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4141):	156.72	12.089	6.00	41.35
+ ID2= 2 (9429):	2.10	0.094	6.50	42.30
=====				
ID = 3 (9424):	158.82	12.098	6.00	41.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5414)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	4.7060	4.4896
0.1870	1.6651	5.4940	5.4773
2.0210	2.0447	6.0800	6.4738
2.9590	2.4658	6.4800	6.4838
3.7490	3.5575	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9424)	158.820	12.098	6.00	41.37
OUTFLOW: ID= 1 (5414)	158.820	3.602	7.00	41.35

PEAK FLOW REDUCTION [Qout/Qin](%)= 29.78
 TIME SHIFT OF PEAK FLOW (min)= 60.00
 MAXIMUM STORAGE USED (ha.m.)= 3.3552

CALIB STANDHYD (4142)
 ID= 1 DT=15.0 min

Area (ha)= 29.14
 Total Imp(%)= 52.00 Dir. Conn.(%)= 21.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 15.15 13.99
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 440.76 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 96.49 84.49
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 7.77 (ii) 19.21 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.10 0.05

TOTALS
 1.70 (iii)
 6.25
 41.35
 73.10
 0.57

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8254)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4142):	29.14	2.578	6.00	41.35
+ ID2= 2 (5414):	158.82	3.602	7.00	41.35
=====				
ID = 3 (8254):	187.96	5.037	6.25	41.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (4121)
 ID= 1 DT=15.0 min

Area (ha)= 202.00
 Total Imp(%)= 45.00 Dir. Conn.(%)= 16.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 90.90 111.10
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1160.46 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 96.49 49.39
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 13.89 (ii) 28.07 (ii)

Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.07 0.04

PEAK FLOW (cms)= 6.51 6.51
 TIME TO PEAK (hrs)= 6.00 6.25
 RUNOFF VOLUME (mm)= 71.10 21.53
 TOTAL RAINFALL (mm)= 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.29

TOTALS
 10.542 (iii)
 6.00
 29.46
 73.10
 0.40

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 51.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5412)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	6.5000	1.0500
0.3860	0.5085	7.9000	1.1500
1.8460	0.6103	9.3000	1.2500
3.5050	0.7566	9.7000	1.2600
4.7890	0.8929	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (4121)	202.000	10.542	6.00	29.46
OUTFLOW: ID= 1 (5412)	202.000	8.602	6.25	29.46

PEAK FLOW REDUCTION [Qout/Qin] (%) = 81.59
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha.m.) = 1.2404

CALIB STANDHYD (4122)
 ID= 1 DT=15.0 min

Area (ha) = 36.70
 Total Imp (%) = 63.00
 Dir. Conn. (%) = 22.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 23.12 13.58
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 494.64 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 96.49 86.58
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 8.33 (ii) 19.66 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.09 0.05

PEAK FLOW (cms) = 1.92 1.67
 TIME TO PEAK (hrs) = 6.00 6.25
 RUNOFF VOLUME (mm) = 71.10 27.16
 TOTAL RAINFALL (mm) = 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.37

TOTALS
 2.985 (iii)
 6.00
 36.83
 73.10
 0.50

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 51.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9412)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (4122):	36.70	2.985	6.00	36.83
+ ID2= 2 (5412):	202.00	8.602	6.25	29.46
ID = 3 (9412):	238.70	10.831	6.25	30.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8252)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (8254):	187.96	5.037	6.25	41.35
+ ID2= 2 (9412):	238.70	10.831	6.25	30.59
ID = 3 (8252):	426.66	15.868	6.25	35.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6416)
 IN= 2--> OUT= 1

Routing time step (min)' = 15.00

DATA FOR SECTION (4161.0) ----->

Distance	Elevation	Manning	
0.00	270.07	0.0340	
20.67	267.91	0.0340	
62.01	264.33	0.0340	
113.69	259.75	0.0340	
165.37	253.30	0.0340	
227.38	246.29	0.0340	
232.55	246.03	0.0340	
237.72	246.16	0.0340	
241.39	246.02	0.0340 / 0.0300	Main Channel
241.64	245.75	0.0300	Main Channel
242.89	245.75	0.0300	Main Channel
244.14	245.75	0.0300	Main Channel
244.39	246.02	0.0300 / 0.0340	Main Channel
248.06	246.20	0.0340	
253.22	246.28	0.0340	
258.39	246.63	0.0340	
346.25	252.57	0.0340	
413.43	257.77	0.0340	
465.11	261.78	0.0340	
511.62	270.00	0.0340	

TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.27	246.02	.295E+04	0.6	0.75	88.26
1.53	247.28	.168E+06	69.7	1.65	40.13
2.79	248.54	.490E+06	299.5	2.42	27.28
4.06	249.81	.962E+06	740.1	3.05	21.67
5.32	251.07	.158E+07	1440.0	3.61	18.33
6.58	252.33	.235E+07	2443.8	4.12	16.06
7.84	253.59	.327E+07	3811.2	4.63	14.30
9.10	254.85	.432E+07	5565.1	5.11	12.94
10.37	256.12	.550E+07	7721.3	5.57	11.88
11.63	257.38	.682E+07	10309.9	6.00	11.02
12.89	258.64	.827E+07	13360.6	6.42	10.31
14.15	259.90	.985E+07	16884.5	6.81	9.72
15.42	261.17	.116E+08	20818.4	7.14	9.26
16.68	262.43	.134E+08	25510.4	7.53	8.78
17.94	263.69	.154E+08	30958.0	7.97	8.30
19.20	264.95	.175E+08	36988.3	8.38	7.89
20.46	266.21	.197E+08	43617.1	8.78	7.53
21.73	267.48	.220E+08	50868.1	9.17	7.22
22.99	268.74	.244E+08	58854.0	9.56	6.92

hydrograph -----> <- pipe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8252)	426.66	15.87	6.25	35.33	0.55	0.85
OUTFLOW: ID= 1 (6416)	426.66	7.64	7.25	35.33	0.40	0.79

CALIB NASHYD (4164)
 ID= 1 DT=15.0 min

Area (ha) = 89.30
 Ia (mm) = 5.00
 U.H. Tp (hrs) = 1.46
 Curve Number (CN) = 76.0
 # of Linear Res. (N) = 3.00

Unit Hyd Opeak (cms) = 2.344

PEAK FLOW (cms) = 1.968 (i)
 TIME TO PEAK (hrs) = 7.500

RUNOFF VOLUME (mm) = 31.268
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.428

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9427)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.4700	2.5000
0.9000	1.0000	5.9800	3.0000
1.7300	1.5000	7.5000	3.5000
3.2200	2.0000	8.7400	4.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (4164)	89.300	1.968	7.50	31.27
OUTFLOW: ID= 1 (9427)	89.300	1.106	9.25	31.26

PEAK FLOW REDUCTION [Qout/Qin] (%) = 56.20
 TIME SHIFT OF PEAK FLOW (min) = 105.00
 MAXIMUM STORAGE USED (ha. m.) = 1.1262

RESERVOIR (5416)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	1.5980	0.7565
0.0330	0.3532	1.8570	0.8137
0.7450	0.4481	2.0990	0.8707
1.0270	0.5154	2.4990	0.8807
1.3050	0.6350	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9427)	89.300	1.106	9.25	31.26
OUTFLOW: ID= 1 (5416)	89.300	1.035	10.25	31.23

PEAK FLOW REDUCTION [Qout/Qin] (%) = 93.53
 TIME SHIFT OF PEAK FLOW (min) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 0.5192

CALIB NASHYD (4162)
 ID= 1 DT=15.0 min

Area (ha) = 349.99 Curve Number (CN) = 61.3
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp (hrs) = 1.29

Unit Hyd Opeak (cms) = 4.646
 PEAK FLOW (cms) = 2.705 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 20.157
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.276

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9416)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4162):	349.99	2.705	8.00	20.16
+ ID2= 2 (5416):	89.30	1.035	10.25	31.23
-----	-----	-----	-----	-----
ID = 3 (9416):	439.29	3.388	9.25	22.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8250)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6416):	426.66	7.635	7.25	35.33

+ ID2= 2 (9416): 439.29 3.388 9.25 22.41

 ID = 3 (8250): 865.95 10.264 7.50 28.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0418)
 ID= 1 DT=15.0 min

Area (ha) = 174.09 Curve Number (CN) = 64.0
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp (hrs) = 1.05

Unit Hyd Opeak (cms) = 2.825
 PEAK FLOW (cms) = 1.695 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 21.774
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.298

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (4184)
 ID= 1 DT=15.0 min

Area Total (ha) = 8.70
 Imp (%) = 30.00 Di r. Conn. (%) = 10.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 2.61 6.09
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 240.83 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 96.49 66.87
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 5.41 (ii) 17.97 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.11 0.05

PEAK FLOW (cms) = 0.22 0.60 *TOTALS*
 TIME TO PEAK (hrs) = 6.00 6.25 0.643 (iii)
 RUNOFF VOLUME (mm) = 71.10 33.67 6.25
 TOTAL RAINFALL (mm) = 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.46 0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 73.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9428)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.4300	0.1618
0.0300	0.0818	0.5800	0.2012
0.1700	0.0912	0.7300	0.2384
0.3100	0.1306	0.8500	0.2747

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (4184)	8.700	0.643	6.25	37.41
OUTFLOW: ID= 1 (9428)	8.700	0.318	6.75	37.33

PEAK FLOW REDUCTION [Qout/Qin] (%) = 49.48
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 0.1355

ADD HYD (9423)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6416):	426.66	7.635	7.25	35.33

ID1= 1 (0418):	174.09	1.695	7.50	21.77
+ ID2= 2 (9428):	8.70	0.318	6.75	37.33

ID = 3 (9423):	182.79	1.954	7.00	22.51

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8248)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8250):	865.95	10.264	7.50	28.78
+ ID2= 2 (9423):	182.79	1.954	7.00	22.51

ID = 3 (8248):	1048.74	12.200	7.25	27.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8246)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0420):	175.82	1.485	7.00	15.59
+ ID2= 2 (8248):	1048.74	12.200	7.25	27.68

ID = 3 (8246):	1224.56	13.670	7.25	25.95

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8244)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (5507):	7519.42	34.824	11.75	17.45
+ ID2= 2 (8246):	1224.56	13.670	7.25	25.95

ID = 3 (8244):	8743.98	42.082	10.75	18.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (4227)	Area (ha) = 66.80	Dir. Conn. (%) = 15.00
ID= 1 DT=15.0 min	Total Imp(%) = 29.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) =	19.37	47.43
Dep. Storage (mm) =	2.00	5.00
Average Slope (%) =	0.50	0.50
Length (m) =	667.33	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr) =	96.49	20.78
over (min) =	15.00	45.00
Storage Coeff. (min) =	9.96 (ii)	30.02 (ii)
Unit Hyd. Tpeak (min) =	15.00	45.00
Unit Hyd. peak (cms) =	0.09	0.03

TOTALS
2.970 (iii)
6.00
26.84
73.10
0.37

PEAK FLOW (cms) =	2.27	1.70
TIME TO PEAK (hrs) =	6.00	6.50
RUNOFF VOLUME (mm) =	71.10	19.03
TOTAL RAINFALL (mm) =	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.26

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 54.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| RESERVOIR (9426) |

IN= 2---> OUT= 1				
DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	3.3400	1.2303
	0.2300	0.6171	4.4700	1.5350
	1.3000	0.6860	5.6100	1.8228
	2.4100	0.9909	6.5300	2.1041

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (4227)	66.800	2.970	6.00	26.84
OUTFLOW: ID= 1 (9426)	66.800	1.478	6.75	26.83

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 49.74
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.7373

RESERVOIR (5422)				
IN= 2---> OUT= 1				
DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	4.3110	2.6782
	0.2180	1.7874	5.4130	2.9186
	1.2490	1.9110	6.3010	3.1540
	2.3250	2.1760	6.7010	3.1640
	3.2250	2.4038	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (9426)	66.800	1.478	6.75	26.83
OUTFLOW: ID= 1 (5422)	66.800	0.129	16.50	26.80

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 8.73
TIME SHIFT OF PEAK FLOW (min) = 585.00
MAXIMUM STORAGE USED (ha. m.) = 1.0580

CALIB NASHYD (4222)	Area (ha) = 713.41	Curve Number (CN) = 54.0
ID= 1 DT=15.0 min	Ia (mm) = 5.00	# of Li near Res. (N) = 1.50
	U. H. Tp (hrs) = 1.95	

Unit Hyd Opeak (cms) = 6.252

PEAK FLOW (cms) =	3.189 (i)
TIME TO PEAK (hrs) =	9.000
RUNOFF VOLUME (mm) =	16.241
TOTAL RAINFALL (mm) =	73.100
RUNOFF COEFFICIENT =	0.222

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9422)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4222):	713.41	3.189	9.00	16.24
+ ID2= 2 (5422):	66.80	0.129	16.50	26.80

ID = 3 (9422):	780.21	3.292	9.25	17.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8242)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8244):	8743.98	42.082	10.75	18.64
+ ID2= 2 (9422):	780.21	3.292	9.25	17.14

ID = 3 (8242):	9524.19	45.212	10.50	18.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

| RESERVOIR (5508) |

IN= 2---> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	*****	197.3574
76.4550	30.8371	*****	394.7148
*****	61.6742	*****	394.7248
*****	*****	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8242)	9524.190	45.212	10.50	18.52
OUTFLOW: ID= 1 (5508)	9524.190	43.777	12.00	18.52

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 96.83
TIME SHIFT OF PEAK FLOW (mi n) = 90.00
MAXIMUM STORAGE USED (ha. m.) = 17.6601

CALIB NASHYD (0336)
ID= 1 DT=15.0 mi n

Area (ha)=2785.00 Curve Number (CN)= 72.0
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U.H. Tp(hrs)= 15.39

Unit Hyd Qpeak (cms)= 3.090
PEAK FLOW (cms)= 3.370 (i)
TIME TO PEAK (hrs)= 22.250
RUNOFF VOLUME (mm)= 27.751
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.380

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3382)
ID= 1 DT=15.0 mi n

Area (ha)= 433.29
Total Imp(%)= 39.00 Di r. Conn.(%)= 20.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	168.98	264.31
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1699.59	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 96.49 over (mi n)= 15.00
Storage Coeff. (mi n)= 17.46 (ii) 31.03 (ii)
Unit Hyd. Tpeak (mi n)= 15.00
Unit Hyd. peak (cms)= 0.07

TOTALS
22.247 (iii)
6.00
36.29
73.10
0.38

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.9 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3381)
ID= 1 DT=15.0 mi n

Area (ha)= 165.62
Total Imp(%)= 53.00 Di r. Conn.(%)= 27.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	87.78	77.84
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1050.78	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 96.49 over (mi n)= 15.00
Storage Coeff. (mi n)= 13.08 (ii) 25.20 (ii)

Unit Hyd. Tpeak (mi n)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04
PEAK FLOW (cms)	9.23	7.21
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	71.10	30.84
TOTAL RAINFALL (mm)	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.42

TOTALS
13.809 (iii)
6.00
41.71
73.10
0.57

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.9 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5338)
IN= 2---> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	9.5300	3.8529
0.6370	1.6350	12.1350	4.2725
2.7300	2.1009	14.3850	4.6808
5.1010	2.7445	14.7850	4.6908
7.0460	3.2936	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (3381)	165.620	13.809	6.00	41.71
OUTFLOW: ID= 1 (5338)	165.620	5.954	6.75	41.71

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 43.12
TIME SHIFT OF PEAK FLOW (mi n) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 3.0265

ADD HYD (8310)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (3382):	433.29	22.247	6.00	36.29
+ ID2= 2 (5338):	165.62	5.954	6.75	41.71
ID = 3 (8310):	598.91	26.377	6.50	37.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (3342)
ID= 1 DT=15.0 mi n

Area (ha)= 586.87 Curve Number (CN)= 56.0
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U.H. Tp(hrs)= 8.20

Unit Hyd Qpeak (cms)= 1.221
PEAK FLOW (cms)= 0.823 (i)
TIME TO PEAK (hrs)= 15.250
RUNOFF VOLUME (mm)= 17.318
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.237

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3341)
ID= 1 DT=15.0 mi n

Area (ha)= 33.23
Total Imp(%)= 51.00 Di r. Conn.(%)= 21.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	16.95	16.28
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	470.67	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 96.49 over (mi n)= 15.00
Storage Coeff. (mi n)= 8.08 (ii) 21.67 (ii)

Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.10 0.05

TOTALS
 PEAK FLOW (cms)= 1.67 1.21 2.431 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 71.10 22.68 32.85
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.31 0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
 CN* = 51.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5334)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.6690	0.4664
0.0400	0.1927	0.8520	0.5318
0.1900	0.2643	1.0360	0.5973
0.3830	0.3576	1.4360	0.6073
0.4460	0.3863	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (3341)	33.230	2.431	6.00	32.85
OUTFLOW: ID= 1 (5334)	33.230	0.802	6.75	32.81

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 32.99
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 0.5174

ADD HYD (8314)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (3342):	586.87	0.823	15.25	17.32
+ ID2= 2 (5334):	33.23	0.802	6.75	32.81
ID = 3 (8314):	620.10	1.095	6.75	18.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0306)
 ID= 1 DT=15.0 min

Area (ha)= 283.97 Curve Number (CN)= 52.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 6.44

Unit Hyd Opeak (cms)= 0.753

PEAK FLOW (cms)= 0.445 (i)
 TIME TO PEAK (hrs)= 13.750
 RUNOFF VOLUME (mm)= 15.218
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.210

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0282)
 ID= 1 DT=15.0 min

Area (ha)= 449.38 Curve Number (CN)= 77.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 1.47

Unit Hyd Opeak (cms)= 5.226

PEAK FLOW (cms)= 5.078 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 32.027
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.438

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0284)
 ID= 1 DT=15.0 min

Area (ha)= 78.93 Curve Number (CN)= 84.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.57

Unit Hyd Opeak (cms)= 2.344

PEAK FLOW (cms)= 2.351 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 38.894
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.532

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8388)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0282):	449.38	5.078	8.00	32.03
+ ID2= 2 (0284):	78.93	2.351	6.50	38.89
ID = 3 (8388):	528.31	6.886	7.25	33.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6286)
 IN= 2--> OUT= 1

Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2861.0) ----->

Distance	Elevation	Manning
0.00	233.00	0.0450
20.58	228.51	0.0450
41.17	227.74	0.0450
51.46	227.41	0.0450
97.77	225.96	0.0450
149.23	223.94	0.0450
200.69	220.84	0.0450
226.42	220.66	0.0450
238.85	220.22	0.0450
241.35	220.01	0.0450 / 0.0350
241.85	219.70	0.0350
245.85	219.72	0.0350
246.35	220.06	0.0350
248.85	220.23	0.0350
303.60	221.64	0.0350
380.79	224.98	0.0450
432.25	229.54	0.0450
457.98	233.33	0.0450
483.71	234.27	0.0450
509.44	233.81	0.0450

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.31	220.01	382E+04	0.8	0.57	82.89
1.06	220.76	525E+05	12.1	0.66	72.51
1.81	221.51	240E+06	88.6	1.05	45.21
2.56	222.26	510E+06	261.7	1.46	32.47
3.31	223.01	843E+06	533.8	1.81	26.34
4.06	223.76	124E+07	913.6	2.10	22.64
4.81	224.51	171E+07	1399.0	2.34	20.33
5.56	225.26	225E+07	2051.0	2.60	18.27
6.31	226.01	285E+07	2907.0	2.91	16.35
7.06	226.76	352E+07	3902.5	3.16	15.04
7.82	227.52	426E+07	5069.9	3.39	14.01
8.57	228.27	507E+07	6433.0	3.62	13.13
9.32	229.02	593E+07	8069.2	3.89	12.24
10.07	229.77	681E+07	9926.7	4.16	11.44
10.82	230.52	772E+07	11962.8	4.42	10.75
11.57	231.27	864E+07	14167.1	4.68	10.17
12.32	232.02	959E+07	16537.1	4.92	9.66
13.07	232.77	105E+08	19070.7	5.16	9.22
13.82	233.52	115E+08	21790.4	5.39	8.82

<---- hydrograph ----> <-pi pe / channel ->
 AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL

INFLOW : ID= 2 (8388) (ha) (cms) (hrs) (mm) (m) (m/s)
 528.31 6.89 7.25 33.05 0.72 0.62
 OUTFLOW: ID= 1 (6286) 528.31 5.82 9.00 33.05 0.64 0.61

CALIB STANDHYD (2861) Area (ha)= 54.86
 ID= 1 DT=15.0 min Total Imp(%)= 45.00 Di r. Conn.(%)= 19.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 24.69 30.17
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 604.76 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 96.49 96.20
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 9.39 (ii) 20.26 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.05

TOTALS
 PEAK FLOW (cms)= 2.40 4.10 5.117 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 71.10 42.05 47.57
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.58 0.65

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5286) IN= 2 --> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	3.6700	1.4285
0.1860	0.6703	4.6080	1.6332
1.0630	0.7754	5.3640	1.8336
1.9790	1.0010	5.7640	1.8436
2.7460	1.1950	0.0000	0.0000

INFLOW : ID= 2 (2861) AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 54.860 5.117 6.00 47.57
 OUTFLOW: ID= 1 (5286) 54.860 2.500 6.75 47.55

PEAK FLOW REDUCTION [Oout/Oin] (%) = 48.86
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 1.1556

CALIB STANDHYD (2862) Area (ha)= 205.65
 ID= 1 DT=15.0 min Total Imp(%)= 22.00 Di r. Conn.(%)= 10.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 45.24 160.41
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1170.90 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 96.49 66.79
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 13.96 (ii) 26.53 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.07 0.04

TOTALS
 PEAK FLOW (cms)= 4.14 13.23 15.143 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.25
 RUNOFF VOLUME (mm)= 71.10 37.15 40.55
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10

RUNOFF COEFFICIENT = 0.97 0.51 0.55

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9286) 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2862):	205.65	15.143	6.25	40.55
+ ID2= 2 (5286):	54.86	2.500	6.75	47.55
=====				
ID = 3 (9286):	260.51	16.974	6.25	42.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8386) 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6286):	528.31	5.824	9.00	33.05
+ ID2= 2 (9286):	260.51	16.974	6.25	42.02
=====				
ID = 3 (8386):	788.82	18.452	6.25	36.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0302) Area (ha)= 473.90 Curve Number (CN)= 58.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Li near Res. (N)= 1.50
 U. H. Tp(hrs)= 1.66

Unit Hyd Opeak (cms)= 4.874
 PEAK FLOW (cms)= 2.715 (i)
 TIME TO PEAK (hrs)= 8.500
 RUNOFF VOLUME (mm)= 18.313
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.251

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0298) Area (ha)= 330.51 Curve Number (CN)= 45.0
 ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Li near Res. (N)= 1.50
 U. H. Tp(hrs)= 1.26

Unit Hyd Opeak (cms)= 4.461
 PEAK FLOW (cms)= 1.541 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 12.163
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.166

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3001) Area (ha)= 0.09
 ID= 1 DT=15.0 min Total Imp(%)= 45.00 Di r. Conn.(%)= 15.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 0.04 0.05
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 24.49 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 96.49 44.00
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 1.37 (ii) 16.23 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.11 0.05

TOTALS
 PEAK FLOW (cms)= 0.00 0.00 0.006 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 71.10 19.07 26.80
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.26 0.37

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 46.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5300)
 IN= 2 ---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.0060	0.0023
0.0020	0.0012	0.0070	0.0026
0.0030	0.0016	0.0090	0.0029
0.0040	0.0019	0.4090	0.0129

INFLOW: ID= 2 (3001)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5300)	0.090	0.006	6.00	26.80
	0.090	0.002	6.75	25.34

PEAK FLOW REDUCTION [Qout/Qin] (%) = 29.38
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0010

CALIB NASHYD (3002)
 ID= 1 DT=15.0 min

Area (ha)= 258.84 Curve Number (CN)= 51.8
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.03

Unit Hyd Qpeak (cms)= 4.290
 PEAK FLOW (cms)= 1.750 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 15.084
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.206

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3 (9300)

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3002):	258.84	1.750	7.50	15.08
+ ID2= 2 (5300):	0.09	0.002	6.75	25.34
ID = 3 (9300):	258.93	1.751	7.50	15.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3 (8395)

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0298):	330.51	1.541	8.00	12.16
+ ID2= 2 (9300):	258.93	1.751	7.50	15.09
ID = 3 (8395):	589.44	3.273	7.75	13.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0288)
 ID= 1 DT=15.0 min

Area (ha)= 340.83 Curve Number (CN)= 78.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 2.21

Unit Hyd Qpeak (cms)= 2.629

PEAK FLOW (cms)= 2.858 (i)
 TIME TO PEAK (hrs)= 9.250
 RUNOFF VOLUME (mm)= 33.083
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.453

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0290)
 ID= 1 DT=15.0 min

Area (ha)= 269.18 Curve Number (CN)= 78.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.07

Unit Hyd Qpeak (cms)= 4.279

PEAK FLOW (cms)= 3.986 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 32.883
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.450

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3 (8397)

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0288):	340.83	2.858	9.25	33.08
+ ID2= 2 (0290):	269.18	3.986	7.25	32.88
ID = 3 (8397):	610.01	6.617	8.00	32.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0296)
 ID= 1 DT=15.0 min

Area (ha)= 293.65 Curve Number (CN)= 76.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.13

Unit Hyd Qpeak (cms)= 4.437

PEAK FLOW (cms)= 3.926 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 31.003
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.424

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0292)
 ID= 1 DT=15.0 min

Area (ha)= 738.49 Curve Number (CN)= 68.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 1.52

Unit Hyd Qpeak (cms)= 8.289

PEAK FLOW (cms)= 6.145 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 24.582
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.336

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0294) Area (ha) = 274.15 Curve Number (CN) = 57.0
 ID= 1 DT=15.0 min U. H. Tp(hrs) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 0.87

Unit Hyd Qpeak (cms) = 5.367

PEAK FLOW (cms) = 2.485 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 17.633
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.241

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8398)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0292):	738.49	6.145	8.25	24.58
+ ID2= 2 (0294):	274.15	2.485	7.00	17.63
ID = 3 (8398):	1012.64	8.463	8.00	22.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6296)
 IN= 2---> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (2961.0) ----->

Distance	Elevation	Manning	
0.00	243.98	0.0400	
42.59	243.18	0.0400	
85.17	241.81	0.0400	
120.66	240.50	0.0400	
156.15	239.56	0.0400	
198.74	236.15	0.0400	
237.78	234.01	0.0400	
241.33	233.82	0.0400	
248.77	233.12	0.0400	0.0400 / 0.0400 Main Channel
249.87	232.32	0.0400	Main Channel
250.37	231.80	0.0400	Main Channel
250.87	232.23	0.0400	Main Channel
251.97	233.10	0.0400	0.0400 / 0.0400 Main Channel
255.37	233.22	0.0400	
259.07	233.87	0.0400	
262.62	234.12	0.0400	
266.17	234.23	0.0400	
283.91	234.73	0.0400	
337.15	241.75	0.0400	
351.34	244.00	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.43	232.23	.510E+03	0.1	0.53	81.07
0.87	232.67	.213E+04	0.7	0.83	51.49
1.30	233.10	.501E+04	2.2	1.10	38.75
1.98	233.78	.242E+05	12.6	1.34	31.93
2.66	234.46	.738E+05	42.1	1.46	29.21
3.34	235.14	.177E+06	127.9	1.84	23.13
4.02	235.82	.314E+06	281.4	2.29	18.61
4.70	236.50	.481E+06	509.4	2.71	15.73
5.38	237.18	.672E+06	815.6	3.11	13.73
6.06	237.86	.886E+06	1200.7	3.46	12.30
6.74	238.54	.112E+07	1669.4	3.80	11.23
7.42	239.22	.139E+07	2226.9	4.11	10.38
8.10	239.90	.168E+07	2808.4	4.28	9.95
8.78	240.58	.202E+07	3470.4	4.40	9.68
9.46	241.26	.240E+07	4331.7	4.61	9.24
10.14	241.94	.283E+07	5335.2	4.83	8.84
10.82	242.62	.330E+07	6482.6	5.03	8.48
11.50	243.30	.381E+07	7764.1	5.21	8.18
12.18	243.98	.439E+07	9101.2	5.31	8.04

<----- hydrograph -----> <- pi pe / channel ->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)

INFLOW : ID= 2 (8398) 1012.64 8.46 8.00 22.70 1.71 1.23

OUTFLOW: ID= 1 (6296) 1012.64 8.22 8.50 22.70 1.69 1.22

ADD HYD (8396)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0296):	293.65	3.926	7.50	31.00
+ ID2= 2 (6296):	1012.64	8.218	8.50	22.70
ID = 3 (8396):	1306.29	11.968	8.25	24.57

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8394)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8396):	1306.29	11.968	8.25	24.57
+ ID2= 2 (8397):	610.01	6.617	8.00	32.99
ID = 3 (8394):	1916.30	18.538	8.00	27.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8392)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8394):	1916.30	18.538	8.00	27.25
+ ID2= 2 (8395):	589.44	3.273	7.75	13.45
ID = 3 (8392):	2505.74	21.800	8.00	24.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6302)
 IN= 2---> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3021.0) ----->

Distance	Elevation	Manning	
0.00	228.10	0.0400	
18.47	227.12	0.0400	
36.95	226.12	0.0400	
46.18	225.84	0.0400	
55.42	225.58	0.0400	
272.47	222.88	0.0400	
277.09	222.76	0.0400	
281.71	222.58	0.0400	
288.54	222.18	0.0400	0.0300 / 0.0300 Main Channel
288.64	221.00	0.0300	Main Channel
290.94	221.00	0.0300	Main Channel
291.04	221.00	0.0300	Main Channel
291.54	222.75	0.0300	0.0300 / 0.0400 Main Channel
300.18	222.83	0.0400	
304.80	223.04	0.0400	
309.42	223.25	0.0400	
318.65	223.69	0.0400	
360.22	225.57	0.0400	
397.16	227.60	0.0400	
457.20	228.35	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.29	221.29	.228E+04	0.2	0.32	165.63
0.59	221.59	.465E+04	0.7	0.45	116.15
0.88	221.88	.713E+04	1.2	0.54	96.87
1.18	222.18	.971E+04	1.9	0.61	86.20
1.57	222.57	.175E+05	3.3	0.59	88.86
1.97	222.97	.427E+05	6.4	0.47	111.39
2.36	223.36	.115E+06	16.1	0.44	119.14
2.76	223.76	.237E+06	37.1	0.49	106.49
3.15	224.15	.409E+06	72.9	0.56	93.40
3.55	224.55	.631E+06	127.0	0.63	82.76
3.94	224.94	.903E+06	202.4	0.70	74.37

4.34	225.34	.123E+07	301.8	0.77	67.68
4.73	225.73	.160E+07	433.5	0.85	61.37
5.13	226.13	.200E+07	602.2	0.95	55.29
5.52	226.52	.242E+07	804.7	1.05	50.13
5.92	226.92	.286E+07	1034.2	1.14	46.11
6.31	227.31	.332E+07	1290.5	1.22	42.88
6.71	227.71	.380E+07	1560.9	1.29	40.55
7.10	228.10	.432E+07	1835.6	1.34	39.19

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8392)	2505.74	21.80	8.00	24.00	2.47	0.45
OUTFLOW: ID= 1 (6302)	2505.74	17.15	10.25	24.00	2.38	0.44

ADD HYD	AREA	OPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0302):	473.90	2.715	8.50	18.31
+ ID2= 2 (6302):	2505.74	17.146	10.25	24.00
=====				
ID = 3 (8390):	2979.64	19.656	10.25	23.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA	OPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8386):	788.82	18.452	6.25	36.01
+ ID2= 2 (8390):	2979.64	19.656	10.25	23.10
=====				
ID = 3 (8348):	3768.46	26.760	10.00	25.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area (ha)= 292.37	Curve Number (CN)= 63.0
NASHYD (0304)	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50
ID= 1 DT=15.0 min	U.H. Tp(hrs)= 2.78	

Unit Hyd Opeak (cms)= 1.793

PEAK FLOW (cms)= 1.311 (i)
 TIME TO PEAK (hrs)= 10.250
 RUNOFF VOLUME (mm)= 21.297
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.291

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)= 299.86	Curve Number (CN)= 82.0
NASHYD (0280)	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50
ID= 1 DT=15.0 min	U.H. Tp(hrs)= 0.85	

Unit Hyd Opeak (cms)= 6.009

PEAK FLOW (cms)= 6.103 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 36.959
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.506

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)= 485.49	Curve Number (CN)= 82.0
NASHYD (0278)	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50
ID= 1 DT=15.0 min	U.H. Tp(hrs)= 1.52	

Unit Hyd Opeak (cms)= 5.453

PEAK FLOW (cms)= 6.254 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 37.239
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.509

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)= 90.89	Curve Number (CN)= 79.0
NASHYD (0276)	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50
ID= 1 DT=15.0 min	U.H. Tp(hrs)= 0.67	

Unit Hyd Opeak (cms)= 2.302

PEAK FLOW (cms)= 2.014 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 33.570
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.459

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)= 392.49	Curve Number (CN)= 75.0
NASHYD (0274)	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50
ID= 1 DT=15.0 min	U.H. Tp(hrs)= 1.08	

Unit Hyd Opeak (cms)= 6.182

PEAK FLOW (cms)= 5.249 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 30.083
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.412

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	AREA	OPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0274):	392.49	5.249	7.50	30.08
+ ID2= 2 (0276):	90.89	2.014	6.75	33.57
=====				
ID = 3 (8360):	483.38	7.141	7.00	30.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA	OPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0278):	485.49	6.254	8.00	37.24
+ ID2= 2 (8360):	483.38	7.141	7.00	30.74
=====				
ID = 3 (8358):	968.87	13.139	7.50	34.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6280)	Routing time step (min)'= 15.00
IN= 2----> OUT= 1	

Distance	Elevation	Manning
0.00	241.14	0.0500
13.32	240.80	0.0500
39.95	240.07	0.0500
96.54	236.21	0.0500
113.19	234.15	0.0500
123.18	232.35	0.0500
143.15	225.80	0.0500
149.81	225.62	0.0500

153.14	225.40	0.0500		
157.30	224.76	0.0500 / 0.0300	Main	Channel
159.80	224.26	0.0300	Main	Channel
162.30	224.85	0.0300	Main	Channel
162.55	224.97	0.0300 / 0.0500	Main	Channel
167.80	225.05	0.0500		
186.43	229.14	0.0500		
213.06	234.75	0.0500		
236.37	237.09	0.0500		
266.33	237.31	0.0500		
292.96	237.83	0.0500		
329.58	241.50	0.0500		

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.50	224.76	.263E+04	0.8	0.65	58.31
1.36	225.62	.278E+05	13.4	1.10	34.54
2.22	226.48	.855E+05	52.6	1.40	27.09
3.09	227.35	.157E+06	120.4	1.74	21.78
3.95	228.21	.242E+06	216.3	2.04	18.64
4.81	229.07	.340E+06	341.8	2.29	16.56
5.67	229.93	.450E+06	498.3	2.52	15.06
6.53	230.79	.574E+06	688.3	2.73	13.90
7.40	231.66	.711E+06	913.8	2.93	12.97
8.26	232.52	.861E+06	1173.4	3.11	12.23
9.12	233.38	.103E+07	1461.3	3.24	11.72
9.98	234.24	.121E+07	1793.3	3.37	11.26
10.85	235.11	.142E+07	2141.0	3.45	11.02
11.71	235.97	.165E+07	2530.4	3.50	10.86
12.57	236.83	.192E+07	2954.0	3.51	10.81
13.43	237.69	.226E+07	3160.3	3.18	11.93
14.29	238.55	.271E+07	3869.4	3.26	11.67
15.16	239.42	.320E+07	4740.3	3.38	11.24
16.02	240.28	.373E+07	5688.4	3.48	10.92

<---- hydrograph ---->				<- pi pe / channel ->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8358)	968.87	13.14	7.50	34.00	1.34
OUTFLOW : ID= 1 (6280)	968.87	12.85	8.00	34.00	1.32

ADD HYD (8354)					
ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
1 + 2 = 3					
ID1= 1 (0280):	299.86	6.103	7.00	36.96	
+ ID2= 2 (6280):	968.87	12.849	8.00	34.00	
=====					
ID = 3 (8354):	1268.73	18.341	7.75	34.70	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD	Area (ha)	Curve Number (CN)
(0270)	243.61	81.0
ID= 1 DT=15.0 mi n	5.00	# of Li near Res. (N)= 1.30
	0.87	

Unit Hyd Opeak (cms) = 3.429

PEAK FLOW (cms) = 3.490 (i)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 35.833
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.490

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD	Area (ha)	Curve Number (CN)
(0268)	215.76	75.0
ID= 1 DT=15.0 mi n	5.00	# of Li near Res. (N)= 1.30
	0.69	

Unit Hyd Opeak (cms) = 3.807

PEAK FLOW (cms) = 3.054 (i)
TIME TO PEAK (hrs) = 7.000

RUNOFF VOLUME (mm) = 29.811
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.408

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD	Area (ha)	Curve Number (CN)
(0266)	508.09	64.0
ID= 1 DT=15.0 mi n	5.00	# of Li near Res. (N)= 1.30
	1.63	

Unit Hyd Opeak (cms) = 3.786

PEAK FLOW (cms) = 2.622 (i)
TIME TO PEAK (hrs) = 9.250
RUNOFF VOLUME (mm) = 21.852
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.299

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD	Area (ha)	Dir. Conn. (%)
(2641)	17.58	70.00
ID= 1 DT=15.0 mi n	75.00	

Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
13.18	4.39	
Dep. Storage (mm) = 2.00	5.00	
Average Slope (%) = 0.50	0.50	
Length (m) = 342.34	40.00	
Mannings n = 0.013	0.250	

Max. Eff. Inten. (mm/hr) = 96.49
Storage Coeff. (mi n) = 6.68 (ii)
Unit Hyd. Tpeak (mi n) = 15.00
Unit Hyd. peak (cms) = 0.10

PEAK FLOW (cms) = 3.07
TIME TO PEAK (hrs) = 6.00
RUNOFF VOLUME (mm) = 71.10
TOTAL RAINFALL (mm) = 73.10
RUNOFF COEFFICIENT = 0.97

TOTALS
3.259 (iii)
6.00
57.98
73.10
0.79

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 66.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5264)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2--> OUT= 1	0.0000	0.0000
DT= 15.0 mi n	0.0600	0.3133
	0.3410	0.3763
	0.6340	0.4336
	0.8800	0.4941

INFLOW : ID= 2 (2641)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW : ID= 1 (5264)	17.580	3.259	6.00	57.98
	17.580	1.047	6.25	57.90

PEAK FLOW REDUCTION [Qout/Qin] (%) = 32.13
TIME SHIFT OF PEAK FLOW (mi n) = 15.00
MAXIMUM STORAGE USED (ha. m.) = 0.5314

CALIB NASHYD	Area (ha)	Curve Number (CN)
(2642)	336.37	69.0
ID= 1 DT=15.0 mi n	5.00	# of Li near Res. (N)= 1.30
	1.28	

Unit Hyd Qpeak (cms)= 3.192

PEAK FLOW (cms)= 2.444 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 25.245
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.345

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 5 columns: ADD HYD, AREA, OPEAK, TPEAK, R.V. for section 9264.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with 5 columns: ADD HYD, AREA, OPEAK, TPEAK, R.V. for section 8380.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6268)
IN= 2----> OUT= 1 Routing time step (min)'= 15.00

DATA FOR SECTION (2681.0)

Table with 4 columns: Distance, Elevation, Manning, Channel.

TRAVEL TIME TABLE

Table with 6 columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV. TIME.

14.94 270.99 .539E+07 10550.5 6.32 8.52
15.78 271.83 .601E+07 12024.7 6.46 8.33

Table with 7 columns: AREA, OPEAK, TPEAK, R.V., MAX DEPTH, MAX VEL for section 8382.

Table with 5 columns: ADD HYD, AREA, OPEAK, TPEAK, R.V. for section 8382.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6270)
IN= 2----> OUT= 1 Routing time step (min)'= 15.00

DATA FOR SECTION (2701.0)

Table with 4 columns: Distance, Elevation, Manning, Channel.

TRAVEL TIME TABLE

Table with 6 columns: DEPTH, ELEV, VOLUME, FLOW RATE, VELOCITY, TRAV. TIME.

Table with 7 columns: AREA, OPEAK, TPEAK, R.V., MAX DEPTH, MAX VEL for section 6270.

ADD HYD	(8384)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0270):	243.61	3.490	7.25	35.83	
+ ID2= 2 (6270):	1077.80	7.431	9.00	25.09	
=====					
ID = 3 (8384):	1321.41	10.559	8.50	27.07	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	(0262)	Area	(ha)	Curve Number	(CN)=
NASHYD	(0262)	Ia	(mm)	# of Linear Res. (N)	=
ID= 1 DT=15.0 min		341.31	5.00	82.0	1.30
		U.H. Tp(hrs)=	1.01		

Unit Hyd Qpeak (cms)= 4.094

PEAK FLOW (cms)= 4.455 (i)
 TIME TO PEAK (hrs)= 7.750
 RUNOFF VOLUME (mm)= 37.033
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.507

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	(0258)	Area	(ha)	Curve Number	(CN)=
NASHYD	(0258)	Ia	(mm)	# of Linear Res. (N)	=
ID= 1 DT=15.0 min		181.99	5.00	79.0	1.30
		U.H. Tp(hrs)=	1.18		

Unit Hyd Qpeak (cms)= 1.881

PEAK FLOW (cms)= 1.930 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 33.887
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.464

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	(0256)	Area	(ha)	Curve Number	(CN)=
NASHYD	(0256)	Ia	(mm)	# of Linear Res. (N)	=
ID= 1 DT=15.0 min		145.79	5.00	67.0	1.30
		U.H. Tp(hrs)=	1.04		

Unit Hyd Qpeak (cms)= 1.707

PEAK FLOW (cms)= 1.174 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 23.749
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.325

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	(0254)	Area	(ha)	Curve Number	(CN)=
NASHYD	(0254)	Ia	(mm)	# of Linear Res. (N)	=
ID= 1 DT=15.0 min		403.00	5.00	55.0	1.30
		U.H. Tp(hrs)=	1.22		

Unit Hyd Qpeak (cms)= 4.028

PEAK FLOW (cms)= 1.990 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 16.662
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.228

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN	(6256)	Routing time step (min)=
IN= 2--> OUT= 1		15.00

<----- DATA FOR SECTION (2561.0) ----->

Distance	Elevation	Manning
0.00	276.07	0.0400
11.68	273.71	0.0400
23.36	271.35	0.0400
29.19	270.30	0.0400
35.03	269.44	0.0400
55.47	267.90	0.0400
78.82	266.24	0.0400
92.50	265.63	0.0400
102.18	265.40	0.0400
105.10	264.95	0.0400 / 0.0350
108.02	264.39	0.0350
110.94	264.72	0.0350
113.86	265.19	0.0350 / 0.0400
116.78	265.49	0.0400
143.05	268.24	0.0400
172.25	270.53	0.0400
207.28	271.95	0.0400
233.55	273.72	0.0400
256.91	274.98	0.0400
289.02	275.97	0.0400

----- TRAVEL TIME TABLE -----					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)
0.56	264.95	.723E+04	1.9	0.90	62.51
1.14	265.53	.307E+05	13.5	1.49	37.92
1.72	266.11	.951E+05	48.6	1.73	32.62
2.30	266.69	.191E+06	117.8	2.09	27.03
2.88	267.27	.314E+06	225.2	2.43	23.24
3.46	267.85	.464E+06	375.5	2.74	20.59
4.04	268.43	.641E+06	573.2	3.03	18.62
4.62	269.01	.846E+06	821.9	3.29	17.15
5.20	269.59	.108E+07	1135.0	3.56	15.87
5.78	270.17	.134E+07	1525.1	3.86	14.64
6.36	270.75	.162E+07	1971.1	4.12	13.71
6.94	271.33	.194E+07	2474.0	4.33	13.04
7.52	271.91	.228E+07	3065.0	4.55	12.42
8.10	272.49	.266E+07	3773.4	4.80	11.75
8.68	273.07	.306E+07	4566.3	5.05	11.17
9.26	273.65	.348E+07	5443.7	5.30	10.66
9.84	274.23	.393E+07	6389.6	5.51	10.25
10.42	274.81	.440E+07	7426.2	5.72	9.88
11.00	275.39	.490E+07	8490.1	5.86	9.63

<---- hydrograph ---->						<- pipe / channel ->	
	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL	
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW : ID= 2 (0254)	403.00	1.99	8.00	16.66	0.56	0.90	
OUTFLOW: ID= 1 (6256)	403.00	1.87	9.75	16.66	0.54	0.90	

ADD HYD	(8370)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (0256):	145.79	1.174	8.00	23.75	
+ ID2= 2 (6256):	403.00	1.869	9.75	16.66	
=====					
ID = 3 (8370):	548.79	2.934	9.25	18.54	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	(0250)	Area	(ha)	Curve Number	(CN)=
NASHYD	(0250)	Ia	(mm)	# of Linear Res. (N)	=
ID= 1 DT=15.0 min		192.88	5.00	70.0	1.30
		U.H. Tp(hrs)=	1.22		

Unit Hyd Qpeak (cms)= 1.930

PEAK FLOW (cms)= 1.508 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 25.980
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.355

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0246) Area (ha)= 759.61 Curve Number (CN)= 55.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.30
U.H. Tp(hrs)= 1.81

Unit Hyd Qpeak (cms)= 5.121

PEAK FLOW (cms)= 2.778 (i)
TIME TO PEAK (hrs)= 10.000
RUNOFF VOLUME (mm)= 16.720
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.229

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0248) Area (ha)= 146.04 Curve Number (CN)= 64.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.30
U.H. Tp(hrs)= 0.78

Unit Hyd Qpeak (cms)= 2.271

PEAK FLOW (cms)= 1.326 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 21.646
TOTAL RAINFALL (mm)= 73.100
RUNOFF COEFFICIENT = 0.296

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8364)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (0246):	759.61	2.778	10.00	16.72	
+ ID2= 2 (0248):	146.04	1.326	7.25	21.65	
=====					
ID = 3 (8364):	905.65	3.933	8.25	17.51	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6250)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

DATA FOR SECTION (2501.0) ----->					
Distance	Elevation	Manning			
0.00	269.96	0.0500			
8.56	268.55	0.0500			
17.13	266.91	0.0500			
21.41	266.13	0.0500			
40.68	263.15	0.0500			
62.09	260.75	0.0500			
85.64	258.02	0.0500			
88.20	257.69	0.0500			
93.20	257.05	0.0500 / 0.0350	Main	Channel	
93.45	256.88	0.0350	Main	Channel	
94.20	256.56	0.0350	Main	Channel	
94.95	256.83	0.0350	Main	Channel	
95.20	257.08	0.0350 / 0.0500	Main	Channel	
100.62	257.45	0.0500			
115.61	258.57	0.0500			
139.16	260.43	0.0500			
152.01	261.95	0.0500			
171.27	264.63	0.0500			
188.40	267.90	0.0500			
211.95	274.18	0.0500			

TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.49	257.05	.205E+04	0.4	0.68	89.65
1.17	257.73	.248E+05	5.7	0.85	72.16
1.85	258.41	.841E+05	25.2	1.10	55.60
2.53	259.09	.180E+06	66.5	1.36	45.14
3.21	259.77	.312E+06	135.7	1.60	38.36
3.89	260.45	.481E+06	238.3	1.82	33.61
4.57	261.13	.681E+06	383.5	2.07	29.60
5.25	261.81	.911E+06	568.9	2.29	26.70

5.93	262.49	.117E+07	800.5	2.51	24.36
6.61	263.17	.146E+07	1079.0	2.72	22.49
7.28	263.84	.177E+07	1413.4	2.94	20.85
7.96	264.52	.210E+07	1797.0	3.14	19.50
8.64	265.20	.246E+07	2241.6	3.35	18.28
9.32	265.88	.283E+07	2739.9	3.55	17.25
10.00	266.56	.323E+07	3295.7	3.75	16.34
10.68	267.24	.364E+07	3908.9	3.94	15.54
11.36	267.92	.408E+07	4578.5	4.12	14.84
12.04	268.60	.453E+07	5320.1	4.32	14.18
12.72	269.28	.499E+07	6109.1	4.50	13.61

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8364)	905.65	3.93	8.25	17.51	0.94	0.78
OUTFLOW: ID= 1 (6250)	905.65	3.66	10.25	17.51	0.90	0.77

ADD HYD (8366)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (0250):	192.88	1.508	8.00	25.98	
+ ID2= 2 (6250):	905.65	3.662	10.25	17.51	
=====					
ID = 3 (8366):	1098.53	5.036	10.00	19.00	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8368)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (8366):	1098.53	5.036	10.00	19.00	
+ ID2= 2 (8370):	548.79	2.934	9.25	18.54	
=====					
ID = 3 (8368):	1647.32	7.940	9.75	18.85	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (5251) Area (ha)= 32.51
ID= 1 DT=15.0 min Total Imp(%)= 75.00 Di r. Conn. (%)= 70.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	24.38	8.13
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	465.55	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	55.83
over (mi n)	15.00	30.00
Storage Coeff. (mi n)=	8.03 (ii)	21.53 (ii)
Unit Hyd. Tpeak (mi n)=	15.00	30.00
Unit Hyd. peak (cms)=	0.10	0.05

PEAK FLOW (cms)=	5.46	0.62	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	5.852 (iii)
RUNOFF VOLUME (mm)=	71.10	30.25	6.00
TOTAL RAINFALL (mm)=	73.10	73.10	58.84
RUNOFF COEFFICIENT =	0.97	0.41	73.10
			0.80

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.6 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5252)				
IN= 2--> OUT= 1				
DT= 15.0 mi n	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)

0.0000	0.0000	2.1750	1.0334
0.1100	0.5793	2.7310	1.1195
0.6300	0.6958	3.1790	1.2039
1.1730	0.8017	3.5790	1.2139
1.6270	0.9137	0.0000	0.0000

AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW: ID= 2 (2521) 32.510 5.852 6.00 58.84
 OUTFLOW: ID= 1 (5252) 32.510 1.915 6.25 58.80

PEAK FLOW REDUCTION [Qout/Qin] (%) = 32.72
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha.m.) = 0.9878

CALIB NASHYD (2522) Area (ha) = 287.47 Curve Number (CN) = 71.5
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.30
 U.H. Tp(hrs) = 1.01

Unit Hyd Opeak (cms) = 3.448
 PEAK FLOW (cms) = 2.701 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 27.086
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.371

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9252) 1 + 2 = 3
 ID1= 1 (2522): 287.47 2.701 8.00 27.09
 + ID2= 2 (5252): 32.51 1.915 6.25 58.80
 ID = 3 (9252): 319.98 4.226 6.50 30.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8372) 1 + 2 = 3
 ID1= 1 (8368): 1647.32 7.940 9.75 18.85
 + ID2= 2 (9252): 319.98 4.226 6.50 30.31
 ID = 3 (8372): 1967.30 10.671 8.75 20.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6258) IN= 2--> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (2581.0) ----->

Distance	Elevation	Manning	
0.00	252.88	0.0500	
15.47	251.89	0.0500	
46.41	248.45	0.0500	
73.48	245.81	0.0500	
88.95	243.75	0.0500	
112.15	242.00	0.0500	
135.35	240.23	0.0500	
162.42	239.76	0.0500	
170.97	239.52	0.0500 / 0.0350	Main Channel
171.58	239.03	0.0350	Main Channel
174.02	239.03	0.0350	Main Channel
176.46	239.03	0.0350	Main Channel
177.07	239.52	0.0350 / 0.0500	Main Channel
185.63	239.67	0.0500	
208.83	239.87	0.0500	
239.77	240.14	0.0500	
274.57	244.93	0.0500	
336.45	249.51	0.0500	
363.52	249.77	0.0500	

382.85 251.78 0.0500

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.49	239.52	.135E+05	2.5	0.92	91.32
1.14	240.17	.175E+06	25.6	0.73	114.22
1.78	240.81	.532E+06	123.5	1.17	71.85
2.43	241.46	.932E+06	285.3	1.54	54.48
3.07	242.10	.138E+07	506.5	1.85	45.25
3.72	242.75	.186E+07	786.8	2.12	39.42
4.36	243.39	.239E+07	1127.3	2.37	35.33
5.01	244.04	.296E+07	1535.8	2.61	32.12
5.65	244.68	.356E+07	2014.3	2.84	29.48
6.30	245.33	.420E+07	2535.9	3.03	27.60
6.94	245.97	.488E+07	3108.4	3.20	26.16
7.59	246.62	.561E+07	3740.6	3.35	24.98
8.23	247.26	.638E+07	4446.7	3.50	23.93
8.88	247.91	.721E+07	5229.7	3.64	22.98
9.52	248.55	.809E+07	6094.3	3.78	22.12
10.17	249.20	.901E+07	7050.4	3.93	21.31
10.81	249.84	.100E+08	7773.0	3.90	21.46
11.46	250.49	.111E+08	8998.8	4.07	20.56
12.10	251.13	.122E+08	10318.2	4.24	19.75

<---- hydrograph ----> <- pi pe / channel ->
 AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW: ID= 2 (8372) 1967.30 10.67 8.75 20.71 0.72 0.84
 OUTFLOW: ID= 1 (6258) 1967.30 9.71 11.00 20.71 0.69 0.85

ADD HYD (8374) 1 + 2 = 3
 ID1= 1 (0258): 181.99 1.930 8.00 33.89
 + ID2= 2 (6258): 1967.30 9.711 11.00 20.71
 ID = 3 (8374): 2149.29 11.272 10.75 21.83

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2601) Area (ha) = 56.22
 ID= 1 DT=15.0 min Total Imp(%) = 65.00 Di. r. Conn. (%) = 48.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 36.54 19.68
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 612.21 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr) = 96.49 103.47
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 9.46 (ii) 20.01 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.09 0.05

PEAK FLOW (cms) = 6.21 2.90 *TOTALS*
 TIME TO PEAK (hrs) = 6.00 6.25 8.146 (iii)
 RUNOFF VOLUME (mm) = 71.10 44.80 57.42
 TOTAL RAINFALL (mm) = 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.61 0.79

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 81.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5260) IN= 2--> OUT= 1
 DT= 15.0 min OUTFLOW (cms) STORAGE (ha.m.) OUTFLOW (cms) STORAGE (ha.m.)

0.0000	0.0000	3.7610	1.6799
0.1900	0.8974	4.7220	1.8499
1.0890	1.0678	5.4970	2.0146
2.0290	1.2667	5.8970	2.0246
2.8140	1.4621	0.0000	0.0000

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (2601)	56.220	8.146	6.00	57.42
OUTFLOW: ID= 1 (5260)	56.220	3.166	6.50	57.40

PEAK FLOW REDUCTION [Qout/Qin] (%) = 38.86
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 1.5490

CALIB NASHYD (2602)	Area (ha) = 420.02	Curve Number (CN) = 83.3
ID= 1 DT=15.0 min	U. H. Tp (hrs) = 1.30	# of Linear Res. (N) = 1.30

Unit Hyd Opeak (cms) = 3.946
 PEAK FLOW (cms) = 4.735 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 38.653
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.529

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9260)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (2602):	420.02	4.735	8.00	38.65
+ ID2= 2 (5260):	56.22	3.166	6.50	57.40
ID = 3 (9260):	476.24	7.137	6.75	40.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8376)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8374):	2149.29	11.272	10.75	21.83
+ ID2= 2 (9260):	476.24	7.137	6.75	40.87
ID = 3 (8376):	2625.53	15.848	10.00	25.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8378)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0262):	341.31	4.455	7.75	37.03
+ ID2= 2 (8376):	2625.53	15.848	10.00	25.28
ID = 3 (8378):	2966.84	19.602	9.50	26.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8362)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8378):	2966.84	19.602	9.50	26.63
+ ID2= 2 (8384):	1321.41	10.559	8.50	27.07
ID = 3 (8362):	4288.25	29.982	9.00	26.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6272)
 IN= 2----> OUT= 1 Routing time step (min) = 15.00

<----- DATA FOR SECTION (2721.0) ----->			
Distance	Elevation	Manning	
0.00	231.01	0.0450	
23.01	223.65	0.0450	
34.51	222.46	0.0450	
51.77	222.11	0.0450	
69.02	221.87	0.0450	
161.06	221.92	0.0450	
166.81	221.91	0.0450	
172.56	221.89	0.0450	
180.57	221.40	0.0450 / 0.0300	Main Channel
181.57	220.60	0.0300	Main Channel
184.07	220.16	0.0300	Main Channel
195.57	221.85	0.0300 / 0.0450	Main Channel
201.32	221.82	0.0450	
207.07	221.77	0.0450	
212.83	221.72	0.0450	
253.09	222.52	0.0450	
408.40	222.65	0.0450	
460.17	223.20	0.0450	
517.69	224.84	0.0450	
569.46	232.57	0.0450	

<----- TRAVEL TIME TABLE ----->					
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)
0.41	220.57	.189E+04	0.5	0.49	60.18
0.83	220.99	.697E+04	3.2	0.82	35.79
1.24	221.40	.145E+05	8.8	1.07	27.45
1.84	222.00	.643E+05	29.3	0.81	36.61
2.44	222.60	.282E+06	132.9	0.83	35.41
3.04	223.20	.707E+06	413.7	1.04	28.50
3.64	223.80	.118E+07	876.8	1.31	22.46
4.24	224.40	.168E+07	1494.8	1.57	18.75
4.84	225.00	.221E+07	2264.1	1.82	16.23
5.44	225.60	.274E+07	3196.3	2.06	14.29
6.04	226.20	.328E+07	4258.6	2.30	12.84
6.65	226.81	.383E+07	5444.7	2.52	11.72
7.25	227.41	.438E+07	6749.7	2.73	10.82
7.85	228.01	.494E+07	8169.4	2.93	10.08
8.45	228.61	.551E+07	9700.7	3.12	9.46
9.05	229.21	.608E+07	11340.8	3.30	8.94
9.65	229.81	.666E+07	13087.4	3.48	8.48
10.25	230.41	.724E+07	14938.5	3.65	8.08
10.85	231.01	.783E+07	16892.4	3.82	7.73

<---- hydrograph ---->						
<-pi pe / channel ->						
	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW: ID= 2 (8362)	4288.25	29.98	9.00	26.77	1.84	0.81
OUTFLOW: ID= 1 (6272)	4288.25	29.40	10.00	26.77	1.84	0.81

CALIB STANDHYD (2721)	Area (ha) = 2.21	Dir. Conn. (%) = 19.00
ID= 1 DT=15.0 min	Total Imp (%) = 49.00	

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha) = 1.08 1.13
 Dep. Storage (mm) = 2.00 5.00
 Average Slope (%) = 0.50 0.50
 Length (m) = 121.38 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 96.49 93.23
 over (min) = 15.00 15.00
 Storage Coeff. (min) = 3.58 (ii) 14.59 (ii)
 Unit Hyd. Tpeak (min) = 15.00 15.00
 Unit Hyd. peak (cms) = 0.11 0.07

TOTALS
 PEAK FLOW (cms) = 0.11 0.21 0.318 (iii)
 TIME TO PEAK (hrs) = 6.00 6.00 6.00
 RUNOFF VOLUME (mm) = 71.10 37.99 44.28
 TOTAL RAINFALL (mm) = 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.52 0.61

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5272)
IN= 2----> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.1480	0.0589
0.0070	0.0284	0.1850	0.0669
0.0430	0.0331	0.2160	0.0746
0.0800	0.0419	0.6160	0.0846
0.1100	0.0497	0.0000	0.0000

INFLOW : ID= 2 (2721)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5272)	2.210	0.318	6.00	44.28
	2.210	0.097	6.50	43.82

PEAK FLOW REDUCTION [Qout/Qin] (%) = 30.34
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha.m.) = 0.0468

CALIB
NASHYD (2722)
ID= 1 DT=15.0 min

Area (ha) = 155.17 Curve Number (CN) = 75.3
Ia (mm) = 5.00 # of Linear Res. (N) = 15.0
U.H. Tp(hrs) = 1.09

Unit Hyd Opeak (cms) = 2.433
PEAK FLOW (cms) = 2.087 (i)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 30.353
TOTAL RAINFALL (mm) = 73.100
RUNOFF COEFFICIENT = 0.415

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9272)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2722):	155.17	2.087	7.50	30.35
+ ID2= 2 (5272):	2.21	0.097	6.50	43.82
ID = 3 (9272):	157.38	2.142	7.25	30.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8356)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6272):	4288.25	29.404	10.00	26.77
+ ID2= 2 (9272):	157.38	2.142	7.25	30.54
ID = 3 (8356):	4445.63	30.985	9.75	26.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8352)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8354):	1268.73	18.341	7.75	34.70
+ ID2= 2 (8356):	4445.63	30.985	9.75	26.90
ID = 3 (8352):	5714.36	47.011	8.50	28.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6304)
IN= 2----> OUT= 1

Routing time step (min) = 15.00

----- DATA FOR SECTION (3041.0) -----

Distance	Elevation	Manning	
0.00	232.08	0.0500	
19.00	231.87	0.0500	
38.00	231.33	0.0500	
66.51	230.44	0.0500	
104.51	228.25	0.0500	
118.76	225.17	0.0500	
128.26	219.86	0.0500	
175.77	219.17	0.0500	
185.27	218.90	0.0500 / 0.0300	Main Channel
185.52	218.65	0.0300	Main Channel
190.02	218.37	0.0300	Main Channel
194.52	218.60	0.0300	Main Channel
194.77	218.85	0.0300 / 0.0500	Main Channel
204.27	219.60	0.0500	
299.28	220.91	0.0500	
327.78	222.36	0.0500	
375.28	225.71	0.0500	
403.79	229.37	0.0500	
432.29	230.43	0.0500	
470.29	232.00	0.0500	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.48	218.85	.121E+05	1.2	0.36	173.94
1.17	219.54	.839E+05	9.8	0.44	142.60
1.86	220.23	.323E+06	41.0	0.47	131.43
2.56	220.93	.708E+06	109.5	0.58	107.67
3.25	221.62	.118E+07	224.3	0.71	87.36
3.94	222.31	.168E+07	376.5	0.84	74.48
4.63	223.00	.222E+07	567.9	0.96	65.26
5.32	223.69	.279E+07	794.7	1.07	58.58
6.02	224.39	.339E+07	1056.2	1.17	53.52
6.71	225.08	.402E+07	1352.4	1.26	49.52
7.40	225.77	.468E+07	1677.3	1.34	46.46
8.09	226.46	.536E+07	2050.0	1.43	43.58
8.79	227.16	.607E+07	2457.9	1.52	41.14
9.48	227.85	.680E+07	2901.0	1.60	39.05
10.17	228.54	.755E+07	3352.7	1.66	37.53
10.86	229.23	.834E+07	3812.5	1.71	36.48
11.55	229.92	.919E+07	4241.8	1.73	36.13
12.25	230.62	.101E+08	4697.0	1.74	35.93
12.94	231.31	.111E+08	5186.3	1.74	35.83

----- hydrograph -----

INFLOW : ID= 2 (8352)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW: ID= 1 (6304)	5714.36	47.01	8.50	28.63	1.93	0.48
	5714.36	38.62	11.25	28.63	1.81	0.47

ADD HYD (8350)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0304):	292.37	1.311	10.25	21.30
+ ID2= 2 (6304):	5714.36	38.620	11.25	28.63
ID = 3 (8350):	6006.73	39.915	11.25	28.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8346)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8348):	3768.46	26.760	10.00	25.80
+ ID2= 2 (8350):	6006.73	39.915	11.25	28.28
ID = 3 (8346):	9775.19	65.369	10.50	27.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0306):	283.97	0.445	13.75	15.32
+ ID2= 2 (8346):	9775.19	65.369	10.50	27.32

ID = 3 (8344):	10059.16	65.769	10.75	26.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5509) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	*****	370.0451
	41.0590	56.7403	*****	863.4386
	48.1390	86.3439	*****	*****
	56.6340	*****	*****	*****
	67.9600	*****	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (8344)	*****	65.769	10.75	26.98
OUTFLOW: ID= 1 (5509)	*****	45.657	15.00	26.98

PEAK FLOW REDUCTION [Out/In] (%) = 69.42
 TIME SHIFT OF PEAK FLOW (min) = 255.00
 MAXIMUM STORAGE USED (ha. m.) = 75.9949

CALIB NASHYD (0332) ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	Area (mm)	# of Linear Res. (N)
	393.44	75.0	5.00	1.50
	U. H. Tp(hrs)=		2.32	

Unit Hyd Qpeak (cms) = 2.894
 PEAK FLOW (cms) = 2.902 (i)
 TIME TO PEAK (hrs) = 9.750
 RUNOFF VOLUME (mm) = 30.269
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.414

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0330) ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	Area (mm)	# of Linear Res. (N)
	468.30	80.0	5.00	1.50
	U. H. Tp(hrs)=		1.09	

Unit Hyd Qpeak (cms) = 7.335
 PEAK FLOW (cms) = 7.311 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 34.924
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.478

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6332) IN= 2---> OUT= 1	Routing time step (min) =
	15.00

<----- DATA FOR SECTION (3321.0) ----->		
Distance	Elevation	Manning
0.00	234.00	0.0380
25.85	227.20	0.0380
96.94	226.44	0.0380
168.03	227.38	0.0380
219.73	225.62	0.0380
342.52	221.57	0.0380
368.37	221.42	0.0380

374.83	221.23	0.0380
379.79	220.98	0.0380 / 0.0300
380.29	220.47	0.0300
381.29	220.47	0.0300
382.79	220.47	0.0300
383.29	220.98	0.0300 / 0.0380
394.22	221.22	0.0380
400.68	221.33	0.0380
407.14	221.44	0.0380
491.16	225.70	0.0380
568.71	227.55	0.0380
607.49	230.14	0.0380
639.80	234.08	0.0380

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.51	220.98	.554E+04	0.9	0.56	107.27
1.20	221.67	.885E+05	12.1	0.49	122.32
1.88	222.35	.309E+06	64.7	0.76	79.70
2.57	223.04	.615E+06	165.4	0.97	61.99
3.25	223.72	.101E+07	322.1	1.16	52.06
3.94	224.41	.148E+07	542.9	1.33	45.51
4.62	225.09	.204E+07	835.4	1.48	40.77
5.31	225.78	.269E+07	1202.8	1.62	37.27
5.99	226.46	.344E+07	1618.0	1.70	35.46
6.68	227.15	.447E+07	1932.0	1.56	38.56
7.36	227.83	.580E+07	2763.9	1.72	34.96
8.05	228.52	.718E+07	3861.9	1.95	30.97
8.73	229.20	.859E+07	5115.4	2.16	27.98
9.42	229.89	.100E+08	6519.2	2.35	25.64
10.10	230.57	.115E+08	8092.3	2.55	23.69
10.79	231.26	.130E+08	9821.5	2.73	22.05
11.47	231.94	.145E+08	11690.0	2.92	20.68
12.16	232.63	.160E+08	13694.9	3.09	19.52
12.84	233.31	.176E+08	15833.9	3.26	18.52

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0330)	468.30	7.31	7.50	34.92	0.90	0.52
OUTFLOW: ID= 1 (6332)	468.30	5.57	9.75	34.92	0.80	0.53

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0332):	393.44	2.902	9.75	30.27
+ ID2= 2 (6332):	468.30	5.567	9.75	34.92

ID = 3 (8322):	861.74	8.469	9.75	32.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0328) ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	Area (mm)	# of Linear Res. (N)
	492.92	77.0	5.00	1.50
	U. H. Tp(hrs)=		1.91	

Unit Hyd Qpeak (cms) = 4.411
 PEAK FLOW (cms) = 4.508 (i)
 TIME TO PEAK (hrs) = 8.750
 RUNOFF VOLUME (mm) = 32.087
 TOTAL RAINFALL (mm) = 73.100
 RUNOFF COEFFICIENT = 0.439

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0326) ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	Area (mm)	# of Linear Res. (N)
	678.91	80.0	5.00	1.50
	U. H. Tp(hrs)=		1.95	

Unit Hyd Qpeak (cms) = 5.941
 PEAK FLOW (cms) = 6.697 (i)

TIME TO PEAK (hrs)= 8.750
 RUNOFF VOLUME (mm)= 35.107
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.480

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (3181)
 ID= 1 DT=15.0 min

Area (ha)= 357.30	Di r. Conn. (%)= 32.00
Total Imp (%)= 56.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	200.09	157.21
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1543.37	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	89.00
over (min)=	15.00	30.00
Storage Coeff. (min)=	16.48 (ii)	27.69 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.07	0.04

TOTALS
 PEAK FLOW (cms)= 21.42 16.96 32.339 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 71.10 37.27 48.10
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.51 0.66

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5318)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	22.8580	8.7280
0.8190	2.5190	27.5380	9.6853
4.2860	5.8947	32.5800	10.8345
11.0600	6.7336	32.9800	10.8445
16.3830	7.4416	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3181)	357.300	32.339	6.00	48.10
OUTFLOW: ID= 1 (5318)	357.300	16.211	6.75	48.09

PEAK FLOW REDUCTION [Qout/Qin] (%) = 50.13
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 7.5534

CALIB
 STANDHYD (3182)
 ID= 1 DT=15.0 min

Area (ha)= 126.13	Di r. Conn. (%)= 12.00
Total Imp (%)= 21.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	26.49	99.64
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	916.99	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	96.49	52.74
over (min)=	15.00	30.00
Storage Coeff. (min)=	12.06 (ii)	25.88 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04

TOTALS
 PEAK FLOW (cms)= 3.22 6.53 7.852 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.25
 RUNOFF VOLUME (mm)= 71.10 30.69 35.54
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.42 0.49

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8334)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3182):	126.13	7.852	6.25	35.54
+ ID2= 2 (5318):	357.30	16.211	6.75	48.09
ID = 3 (8334):	483.43	21.595	6.50	44.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6320)
 IN= 2--> OUT= 1

Routing time step (min) = 15.00

----- DATA FOR SECTION (3201.0) ----->

Distance	Elevation	Manning
0.00	249.00	0.0380
22.92	245.86	0.0380
45.83	244.87	0.0380
91.66	243.11	0.0380
126.03	239.53	0.0380
160.41	237.17	0.0380
166.14	237.06	0.0380
177.59	237.13	0.0380
183.32	237.20	0.0380
189.05	236.70	0.0380 / 0.0350
193.05	235.89	0.0350
197.05	236.64	0.0350 / 0.0380
200.51	236.74	0.0380
206.24	237.03	0.0380
246.34	238.82	0.0380
263.53	243.87	0.0380
389.56	247.64	0.0380
452.58	247.74	0.0380
498.41	248.60	0.0380
567.16	249.84	0.0380

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.37	236.26	.270E+04	0.4	0.58	107.96
0.75	236.64	.108E+05	2.6	0.92	68.01
1.48	237.37	.877E+05	27.4	1.17	53.35
2.20	238.09	.277E+06	122.1	1.65	37.83
2.93	238.82	.540E+06	299.7	2.08	30.01
3.66	239.55	.857E+06	587.8	2.57	24.29
4.39	240.28	.120E+07	977.8	3.04	20.52
5.11	241.00	.158E+07	1458.1	3.46	18.02
5.84	241.73	.198E+07	2028.6	3.84	16.23
6.57	242.46	.240E+07	2690.5	4.19	14.86
7.29	243.18	.285E+07	3431.8	4.50	13.84
8.02	243.91	.335E+07	4149.1	4.64	13.44
8.75	244.64	.393E+07	4746.6	4.51	13.80
9.47	245.36	.463E+07	5618.6	4.53	13.75
10.20	246.09	.545E+07	6781.8	4.66	13.39
10.93	246.82	.635E+07	8274.5	4.87	12.79
11.66	247.55	.733E+07	9988.0	5.10	12.23
12.38	248.27	.853E+07	11422.0	5.01	12.45
13.11	249.00	.989E+07	13749.7	5.20	11.98

----- hydrograph ----->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8334)	483.43	21.60	6.50	44.82	1.31	1.10
OUTFLOW: ID= 1 (6320)	483.43	12.79	7.25	44.82	1.05	1.01

CALIB NASHYD (3202)
 ID= 1 DT=15.0 min | Area (ha)= 84.56 Curve Number (CN)= 80.9
 | Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 | U. H. Tp(hrs)= 0.70

Unit Hyd Qpeak (cms)= 2.054
 PEAK FLOW (cms)= 1.930 (i)
 TIME TO PEAK (hrs)= 6.750
 RUNOFF VOLUME (mm)= 35.589
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.487

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3201)
 ID= 1 DT=15.0 min | Area Total (ha)= 194.18
 | Imp(%)= 45.00 Dir. Conn.(%)= 16.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	87.38	106.80
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1137.78	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (min)	96.49 / 15.00	99.45 / 30.00
Storage Coeff. (min)	13.72 (ii)	24.45 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04
PEAK FLOW (cms)	6.29	13.78
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	71.10	46.63
TOTAL RAINFALL (mm)	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.57

TOTALS
 16.642 (iii)
 0.64

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 78.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5320)
 IN= 2---> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	7.4940	4.4477
	0.2490	1.8139	9.8910	4.7762
	1.2830	3.1517	12.4550	5.0763
	3.7490	3.7610	12.8550	5.0863
	5.0040	4.0198	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3201)	194.180	16.642	6.25	46.63
OUTFLOW: ID= 1 (5320)	194.180	7.396	6.75	46.62

PEAK FLOW REDUCTION [Qout/Oin] (%) = 44.44
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 4.4468

ADD HYD (9320)
 1 + 2 = 3 | AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 ID1= 1 (3202): 84.56 1.930 6.75 35.59
 + ID2= 2 (5320): 194.18 7.396 6.75 46.62
 ID = 3 (9320): 278.74 9.326 6.75 43.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8332)
 1 + 2 = 3 | AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 ID1= 1 (6320): 483.43 12.787 7.25 44.82
 + ID2= 2 (9320): 278.74 9.326 6.75 43.27
 ID = 3 (8332): 762.17 21.451 7.00 44.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (3142)
 ID= 1 DT=15.0 min | Area Total (ha)= 122.75
 | Imp(%)= 5.00 Curve Number (CN)= 78.0
 | U. H. Tp(hrs)= 0.53 # of Linear Res. (N)= 1.50

Unit Hyd Qpeak (cms)= 3.924
 PEAK FLOW (cms)= 3.170 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 32.332
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.442

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3141)
 ID= 1 DT=15.0 min | Area Total (ha)= 42.46
 | Imp(%)= 70.00 Dir. Conn.(%)= 46.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	29.72	12.74
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	532.04	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (min)	96.49 / 15.00	111.92 / 30.00
Storage Coeff. (min)	8.70 (ii)	18.92 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.05
PEAK FLOW (cms)	4.60	2.07
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	71.10	40.28
TOTAL RAINFALL (mm)	73.10	73.10
RUNOFF COEFFICIENT	0.97	0.55

TOTALS
 5.966 (iii)
 6.00
 54.46
 73.10
 0.74

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5314)
 IN= 2---> OUT= 1
 DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	2.8400	1.3064
	0.1440	0.7144	3.5660	1.4269
	0.8230	0.8540	4.1520	1.5451
	1.5320	0.9987	4.5520	1.5551
	2.1250	1.1456	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3141)	42.460	5.966	6.00	54.46
OUTFLOW: ID= 1 (5314)	42.460	2.158	6.50	54.43

PEAK FLOW REDUCTION [Qout/Oin] (%) = 36.16
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 1.1548

ADD HYD (9314)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3142):	122.75	3.170	6.50	32.33
+ ID2= 2 (5314):	42.46	2.158	6.50	54.43
=====				
ID = 3 (9314):	165.21	5.328	6.50	38.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6316)
IN= 2---> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3161.0) ----->				
Distance	Elevation	Manning		
0.00	248.54	0.0350		
27.93	248.34	0.0350		
41.89	246.61	0.0350		
62.83	243.09	0.0350		
132.65	239.00	0.0350		
188.50	236.74	0.0350		
195.48	236.54	0.0350		
202.46	236.32	0.0350		
205.48	236.14	0.0350 / 0.0310	Main Channel	
205.98	235.61	0.0310	Main Channel	
207.98	235.25	0.0310	Main Channel	
209.98	235.53	0.0310	Main Channel	
210.48	236.00	0.0310 / 0.0350	Main Channel	
216.42	236.73	0.0350		
258.31	239.09	0.0350		
328.12	239.84	0.0350		
439.83	241.47	0.0350		
530.58	242.08	0.0350		
586.43	242.93	0.0350		
691.16	248.00	0.0350		

<----- TRAVEL TIME TABLE ----->						
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)	
0.37	235.62	215E+04	0.7	0.77	53.88	
0.75	236.00	634E+04	3.5	1.36	30.45	
1.46	236.71	297E+05	19.3	1.62	25.66	
2.16	237.41	103E+06	75.8	1.84	22.60	
2.87	238.12	229E+06	202.8	2.21	18.79	
3.57	238.82	407E+06	423.3	2.59	16.03	
4.28	239.53	654E+06	727.7	2.77	14.97	
4.99	240.24	103E+07	1242.0	3.01	13.76	
5.69	240.94	151E+07	2002.3	3.31	12.54	
6.40	241.65	210E+07	2987.9	3.55	11.70	
7.10	242.35	286E+07	4302.8	3.75	11.07	
7.81	243.06	374E+07	6165.3	4.10	10.11	
8.51	243.76	468E+07	8642.4	4.60	9.03	
9.22	244.47	565E+07	11496.3	5.06	8.20	
9.93	245.18	666E+07	14714.1	5.50	7.54	
10.63	245.88	770E+07	18290.3	5.91	7.02	
11.34	246.59	877E+07	22221.9	6.30	6.58	
12.04	247.29	988E+07	26464.4	6.67	6.22	
12.75	248.00	110E+08	31061.3	7.01	5.91	

<----- hydrograph ----->					<- pi pe / channel ->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (9314)	165.21	5.33	6.50	38.01	0.83	1.39
OUTFLOW: ID= 1 (6316)	165.21	4.60	7.00	38.01	0.80	1.38

CALIB NASHYD (3162)				
ID= 1 DT=15.0 min				
Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
196.72	5.00	7.00	38.01	
U. H. Tp (hrs) =	0.88			
Curve Number (CN) =	83.0			
# of Linear Res. (N) =	1.50			
Unit Hyd Opeak (cms) =	3.838			
PEAK FLOW (cms) =	4.054 (i)			
TIME TO PEAK (hrs) =	7.000			
RUNOFF VOLUME (mm) =	38.127			
TOTAL RAINFALL (mm) =	73.100			

RUNOFF COEFFICIENT = 0.522

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (3161)				
ID= 1 DT=15.0 min				
Area Total (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
35.62	63.00	7.00	38.13	
Dir. Conn. (%) =	44.00			
IMPERVIOUS	22.44			
PERVIOUS (i)	13.18			
Surface Area (ha) =	22.44			
Dep. Storage (mm) =	2.00			
Average Slope (%) =	0.50			
Length (m) =	487.31			
Mannings n =	0.013			
Max. Eff. Inten. (mm/hr) =	96.49	106.17		
over (min) =	15.00	30.00		
Storage Coeff. (min) =	8.25 (ii)	18.70 (ii)		
Unit Hyd. Tpeak (min) =	15.00	30.00		
Unit Hyd. peak (cms) =	0.09	0.05		
PEAK FLOW (cms) =	3.74	2.05	5.114 (iii)	
TIME TO PEAK (hrs) =	6.00	6.25	6.00	
RUNOFF VOLUME (mm) =	71.10	45.15	56.57	
TOTAL RAINFALL (mm) =	73.10	73.10	73.10	
RUNOFF COEFFICIENT =	0.97	0.62	0.77	

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 81.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5316)				
IN= 2---> OUT= 1				
DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	2.3160	0.9307
	0.0930	0.5020	2.9060	1.0021
	0.5400	0.6637	3.5360	1.0731
	1.1940	0.7495	3.9360	1.0831
	1.6920	0.8359	0.0000	0.0000
INFLOW: ID= 2 (3161)	35.620	5.114	6.00	56.57
OUTFLOW: ID= 1 (5316)	35.620	2.285	6.50	56.53
PEAK FLOW REDUCTION [Qout/Qin] (%) =	44.67			
TIME SHIFT OF PEAK FLOW (min) =	30.00			
MAXIMUM STORAGE USED (ha. m.) =	0.9431			

ADD HYD (9316)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3162):	196.72	4.054	7.00	38.13
+ ID2= 2 (5316):	35.62	2.285	6.50	56.53
=====				
ID = 3 (9316):	232.34	6.108	6.50	40.95

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8338)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6316):	165.21	4.598	7.00	38.01
+ ID2= 2 (9316):	232.34	6.108	6.50	40.95
=====				
ID = 3 (8338):	397.55	10.268	6.75	39.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 CALIB (0312) Area (ha)= 359.44 Curve Number (CN)= 80.0
 NASHYD (0312) I a (mm)= 5.00 # of Linear Res. (N)= 1.50
 ID= 1 DT=15.0 min U. H. Tp(hrs)= 1.36

Unit Hyd Qpeak (cms)= 4.529
 PEAK FLOW (cms)= 4.750 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 35.011
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.479

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB (0308) Area (ha)= 529.30 Curve Number (CN)= 62.0
 NASHYD (0308) I a (mm)= 5.00 # of Linear Res. (N)= 1.50
 ID= 1 DT=15.0 min U. H. Tp(hrs)= 1.62

Unit Hyd Qpeak (cms)= 5.575
 PEAK FLOW (cms)= 3.491 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 20.621
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.282

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB (0310) Area (ha)= 138.28 Curve Number (CN)= 65.0
 NASHYD (0310) I a (mm)= 5.00 # of Linear Res. (N)= 1.50
 ID= 1 DT=15.0 min U. H. Tp(hrs)= 0.76

Unit Hyd Qpeak (cms)= 3.102
 PEAK FLOW (cms)= 1.788 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 22.291
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.305

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (8342) 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 ID1= 1 (0308): 529.30 3.491 8.25 20.62
 + ID2= 2 (0310): 138.28 1.788 7.00 22.29
 ID = 3 (8342): 667.58 5.034 7.75 20.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6312) IN= 2---> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3121.0) ----->
 Di stance El evati on Manni ng
 0.00 265.94 0.0360
 38.07 265.43 0.0360
 59.82 263.98 0.0360
 103.32 254.59 0.0360
 157.70 252.16 0.0360
 217.52 250.45 0.0360
 233.84 247.69 0.0360
 234.71 247.27 0.0360
 239.71 246.38 0.0360
 244.71 246.12 0.0360 /0.0330 Mai n Channel
 247.71 245.17 0.0330 Mai n Channel
 249.71 245.19 0.0330 Mai n Channel
 251.71 245.64 0.0330 /0.0360 Mai n Channel

259.71 246.67 0.0360
 282.78 247.12 0.0360
 315.41 251.60 0.0360
 424.17 256.13 0.0360
 478.55 257.04 0.0360
 516.62 259.37 0.0360
 538.37 266.00 0.0360

----- TRAVEL TIME TABLE ----->
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV. TIME
 (m) (m) (cu. m.) (cms) (m/s) (mi n)
 0.47 245.64 .111E+04 3.1 1.78 6.07
 1.54 246.71 .103E+05 53.9 3.39 3.19
 2.61 247.78 .414E+05 281.3 4.40 2.45
 3.68 248.85 .839E+05 749.1 5.79 1.87
 4.74 249.91 .136E+06 1463.2 6.97 1.55
 5.81 250.98 .201E+06 2354.8 7.61 1.42
 6.88 252.05 .295E+06 3533.0 7.76 1.39
 7.95 253.12 .427E+06 5303.9 8.05 1.34
 9.02 254.19 .594E+06 7856.5 8.58 1.26
 10.09 255.26 .792E+06 11449.2 9.37 1.15
 11.15 256.32 .101E+07 15862.4 10.15 1.06
 12.22 257.39 .127E+07 21074.5 10.75 1.01
 13.29 258.46 .155E+07 28184.1 11.78 0.92
 14.36 259.53 .185E+07 36386.2 12.77 0.85
 15.43 260.60 .215E+07 46110.6 13.89 0.78
 16.50 261.67 .246E+07 56822.5 14.95 0.72
 17.56 262.73 .278E+07 68498.1 15.97 0.68
 18.63 263.80 .310E+07 81118.8 16.94 0.64
 19.70 264.87 .344E+07 93603.2 17.66 0.61

<---- hydrograph ----> <- pi pe / channel ->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8342) 667.58 5.03 7.75 20.97 0.51 1.81
 OUTFLOW: ID= 1 (6312) 667.58 5.03 8.00 20.97 0.51 1.81

 ADD HYD (8340) 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 ID1= 1 (0312): 359.44 4.750 8.00 35.01
 + ID2= 2 (6312): 667.58 5.033 8.00 20.97
 ID = 3 (8340): 1027.02 9.783 8.00 25.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8336) 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 ID1= 1 (8338): 397.55 10.268 6.75 39.73
 + ID2= 2 (8340): 1027.02 9.783 8.00 25.88
 ID = 3 (8336): 1424.57 19.372 7.00 29.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8330) 1 + 2 = 3
 AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 ID1= 1 (8332): 762.17 21.451 7.00 44.25
 + ID2= 2 (8336): 1424.57 19.372 7.00 29.75
 ID = 3 (8330): 2186.74 40.824 7.00 34.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6324) IN= 2---> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3241.0) ----->
 Di stance El evati on Manni ng

0.00	234.38	0.0360	
33.57	232.48	0.0360	
67.14	230.14	0.0360	
83.93	228.80	0.0360	
134.29	227.62	0.0360	
209.82	225.10	0.0360	
218.21	224.86	0.0360	
226.61	224.47	0.0360	
234.00	223.86	0.0360 / 0.0300	Main Channel
234.10	223.66	0.0300	Main Channel
235.00	223.66	0.0300	Main Channel
235.90	223.66	0.0300	Main Channel
236.00	223.86	0.0300 / 0.0360	Main Channel
243.39	224.92	0.0360	
251.78	224.89	0.0360	
335.71	225.64	0.0360	
562.32	226.53	0.0360	
637.85	228.36	0.0360	
705.00	229.80	0.0360	
830.89	234.00	0.0360	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.20	223.86	.145E+04	0.2	0.43	147.27
0.73	224.39	.159E+05	2.8	0.68	93.11
1.27	224.93	.561E+05	11.0	0.75	85.19
1.80	225.46	.217E+06	45.6	0.80	79.20
2.33	225.99	.566E+06	134.4	0.91	70.20
2.87	226.53	.122E+07	338.1	1.06	59.97
3.40	227.06	.206E+07	744.3	1.38	46.16
3.94	227.60	.298E+07	1295.9	1.66	38.38
4.47	228.13	.399E+07	1979.9	1.90	33.59
5.00	228.66	.509E+07	2807.5	2.11	30.21
5.54	229.20	.627E+07	3815.1	2.32	27.41
6.07	229.73	.753E+07	4984.1	2.53	25.17
6.60	230.26	.884E+07	6346.3	2.74	23.20
7.14	230.80	.102E+08	7861.2	2.95	21.61
7.67	231.33	.116E+08	9526.4	3.14	20.29
8.21	231.87	.131E+08	11342.2	3.32	19.18
8.74	232.40	.146E+08	13309.6	3.49	18.22
9.27	232.93	.161E+08	15414.0	3.66	17.41
9.81	233.47	.177E+08	17671.0	3.81	16.70

----- hydrograph ----- <- pipe / channel ->

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8330)	2186.74	40.82	7.00	34.80	1.73
OUTFLOW: ID= 1 (6324)	2186.74	28.35	8.50	34.80	1.53

CALIB NASHYD (3242) ID= 1 DT=15.0 min Area (ha)= 602.18 Curve Number (CN)= 78.8 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50 U.H. Tp(hrs)= 1.80

Unit Hyd Opeak (cms)= 5.708
 PEAK FLOW (cms)= 6.103 (i)
 TIME TO PEAK (hrs)= 8.500
 RUNOFF VOLUME (mm)= 33.847
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.463

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3241) ID= 1 DT=15.0 min Area (ha)= 13.46 Total Imp(%)= 45.00 Dir. Conn.(%)= 15.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	6.06	7.40
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	299.56	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 96.49 over (min)= 15.00
 Storage Coeff. (min)= 6.16 (ii) 16.81 (ii)

Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.10 0.05
 TOTALS
 PEAK FLOW (cms)= 0.51 1.14 1.273 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 71.10 42.19 46.53
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.58 0.64

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 78.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5324)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.9000	0.3505
0.0460	0.1645	1.1310	0.4008
0.2610	0.1903	1.3160	0.4499
0.4860	0.2456	1.7160	0.4599
0.6740	0.2932	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (3241)	13.460	1.273	6.00
OUTFLOW: ID= 1 (5324)	13.460	0.615	6.50

PEAK FLOW REDUCTION [Qout/Qin] (%) = 48.29
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 0.2836

ADD HYD (9324)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (3242):	602.18	6.103	8.50
+ ID2= 2 (5324):	13.46	0.615	6.50
=====			
ID = 3 (9324):	615.64	6.289	8.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8328)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6324):	2186.74	28.349	8.50
+ ID2= 2 (9324):	615.64	6.289	8.00
=====			
ID = 3 (8328):	2802.38	34.596	8.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0322) ID= 1 DT=15.0 min Area (ha)= 513.13 Curve Number (CN)= 80.0 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50 U.H. Tp(hrs)= 1.75

Unit Hyd Opeak (cms)= 5.020
 PEAK FLOW (cms)= 5.538 (i)
 TIME TO PEAK (hrs)= 8.500
 RUNOFF VOLUME (mm)= 35.083
 TOTAL RAINFALL (mm)= 73.100
 RUNOFF COEFFICIENT = 0.480

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8326)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0322):	513.13	5.538	8.50	35.08
+ ID2= 2 (8328):	2802.38	34.596	8.50	34.65
=====				
ID = 3 (8326):	3315.51	40.135	8.50	34.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8324)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0326):	678.91	6.697	8.75	35.11
+ ID2= 2 (8326):	3315.51	40.135	8.50	34.72
=====				
ID = 3 (8324):	3994.42	46.820	8.50	34.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6328)
IN= 2--> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3281.0) ----->

Distance	Elevation	Manning	
0.00	228.00	0.0380	
18.58	224.97	0.0380	
78.98	223.52	0.0380	
125.44	223.28	0.0380	
171.90	221.71	0.0380	
213.72	219.65	0.0380	
218.36	219.40	0.0380	
223.01	219.19	0.0380	
225.95	219.14	0.0380 / 0.0320	Main Channel
226.45	218.14	0.0320	Main Channel
236.95	218.14	0.0320	Main Channel
245.85	218.14	0.0320	Main Channel
245.95	219.14	0.0320 / 0.0380	Main Channel
246.24	219.16	0.0380	
250.88	219.24	0.0380	
255.53	219.39	0.0380	
325.22	221.47	0.0380	
367.03	223.14	0.0380	
404.20	225.17	0.0380	
459.95	228.04	0.0380	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	218.64	.373E+05	3.2	0.33	192.37
1.00	219.14	.752E+05	10.1	0.51	124.71
1.52	219.66	.149E+06	22.2	0.57	112.06
2.04	220.18	.279E+06	43.4	0.59	107.06
2.56	220.70	.464E+06	77.0	0.63	100.52
3.08	221.22	.705E+06	125.7	0.68	93.56
3.61	221.75	.100E+07	192.7	0.73	86.62
4.13	222.27	.135E+07	278.1	0.79	80.92
4.65	222.79	.176E+07	385.9	0.84	75.86
5.17	223.31	.222E+07	514.9	0.89	71.82
5.69	223.83	.280E+07	662.7	0.90	70.40
6.21	224.35	.345E+07	867.7	0.96	66.35
6.73	224.87	.417E+07	1107.0	1.01	62.81
7.25	225.39	.494E+07	1400.0	1.08	58.80
7.78	225.92	.573E+07	1732.2	1.15	55.17
8.30	226.44	.655E+07	2099.9	1.22	52.02
8.82	226.96	.740E+07	2502.8	1.29	49.29
9.34	227.48	.828E+07	2940.9	1.36	46.90
9.86	228.00	.918E+07	3414.3	1.42	44.79

<---- hydrograph ---->

<-pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8324)	3994.42	46.82	8.50	34.79	2.10	0.60
OUTFLOW: ID= 1 (6328)	3994.42	37.55	10.25	34.79	1.89	0.59

ADD HYD (8320)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0328):	492.92	4.508	8.75	32.09
+ ID2= 2 (6328):	3994.42	37.547	10.25	34.79
=====				
ID = 3 (8320):	4487.34	41.837	10.25	34.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8318)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8320):	4487.34	41.837	10.25	34.49
+ ID2= 2 (8322):	861.74	8.469	9.75	32.80
=====				
ID = 3 (8318):	5349.08	50.241	10.00	34.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8316)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (5509):	10059.16	45.657	15.00	26.98
+ ID2= 2 (8318):	5349.08	50.241	10.00	34.22
=====				
ID = 3 (8316):	15408.24	89.618	11.25	29.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8312)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8314):	620.10	1.095	6.75	18.15
+ ID2= 2 (8316):	15408.24	89.618	11.25	29.49
=====				
ID = 3 (8312):	16028.34	90.502	11.25	29.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8308)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8310):	598.91	26.377	6.50	37.79
+ ID2= 2 (8312):	16028.34	90.502	11.25	29.06
=====				
ID = 3 (8308):	16627.25	92.591	11.00	29.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5510)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	*****	74.0090
	66.5450	18.5023	*****	*****
	98.5430	37.0045	*****	*****

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (8308)	*****	92.591	11.00	29.37
OUTFLOW: ID= 1 (5510)	*****	87.775	13.00	29.37

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 94.80
 TIME SHIFT OF PEAK FLOW (min) = 120.00
 MAXIMUM STORAGE USED (ha. m.) = 30.8017

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0336):	2785.00	3.370	22.25	27.75
+ ID2= 2 (5510):	16627.25	87.775	13.00	29.37
=====				
ID = 3 (8240):	19412.25	90.638	13.00	29.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (5508):	9524.19	43.777	12.00	18.52
+ ID2= 2 (8240):	19412.25	90.638	13.00	29.14
=====				
ID = 3 (8238):	28936.44	133.864	12.50	25.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (3427) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	Dir. Conn. (%)=
	249.40	45.00	33.00

	IMPERVIOUS (ha)=	PERVIOUS (i) (%)=
Surface Area	112.23	137.17
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1289.44	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)=	96.49	51.66
over (min)	15.00	30.00
Storage Coeff. (min)=	14.79 (ii)	28.73 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.07	0.04
PEAK FLOW (cms)=	16.16	8.32
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	71.10	27.73
TOTAL RAINFALL (mm)=	73.10	73.10
RUNOFF COEFFICIENT =	0.97	0.38

TOTALS
21.358 (iii)
6.00
42.04
73.10
0.58

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 67.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9425) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	12.4700	5.4211
	0.8500	3.0375	16.6800	6.4838
	4.8400	3.5126	20.9500	7.4161
	9.0000	4.5390	24.3900	8.3284

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (3427)	249.400	21.358	6.00	42.04
OUTFLOW: ID= 1 (9425)	249.400	9.160	6.75	42.04

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 42.89
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 4.6596

RESERVOIR (5342) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)

0.0000	0.0000	3.6600	12.0067
1.2080	11.1244	4.4200	12.4001
2.0570	11.3002	5.2300	12.7452
2.7240	11.3602	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (9425)	249.400	9.160	6.75	42.04
OUTFLOW: ID= 1 (5342)	249.400	0.682	14.50	42.03

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 7.45
TIME SHIFT OF PEAK FLOW (min) = 465.00
MAXIMUM STORAGE USED (ha. m.) = 6.2806

CALIB NASHYD (3422) ID= 1 DT=15.0 min	Area Ia	(ha)= (mm)=	Curve Number (CN)=	# of Linear Res. (N)=
	755.17	5.00	54.5	1.50
	U. H.	TP(hrs)=		
		2.57		

Unit Hyd Opeak (cms) = 5.026

PEAK FLOW (cms) =	2.794 (i)
TIME TO PEAK (hrs) =	10.000
RUNOFF VOLUME (mm) =	16.512
TOTAL RAINFALL (mm) =	73.100
RUNOFF COEFFICIENT =	0.226

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3422):	755.17	2.794	10.00	16.51
+ ID2= 2 (5342):	249.40	0.682	14.50	42.03
=====				
ID = 3 (9342):	1004.57	3.446	10.25	22.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8238):	28936.44	133.864	12.50	25.65
+ ID2= 2 (9342):	1004.57	3.446	10.25	22.85
=====				
ID = 3 (8236):	29941.01	137.100	12.50	25.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8232):	285.79	9.571	6.00	34.97
+ ID2= 2 (8236):	29941.01	137.100	12.50	25.55
=====				
ID = 3 (8234):	30226.80	137.626	12.50	25.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (3442) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	Dir. Conn. (%)=
	188.84	32.00	24.00

	IMPERVIOUS (ha)=	PERVIOUS (i) (%)=
Surface Area	60.43	128.41
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1122.02	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)=	96.49	17.66

over (min) 15.00 45.00
 Storage Coeff. (min)= 13.61 (ii) 35.01 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03

TOTALS
 PEAK FLOW (cms)= 9.21 3.62 10.687 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 71.10 17.43 30.31
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.24 0.41

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 53.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (3441)
 ID= 1 DT=15.0 min
 Area (ha)= 155.15
 Total Imp(%)= 39.00 Dir. Conn.(%)= 29.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 60.51 94.64
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1017.02 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 96.49 19.08
 over (min) 15.00 45.00
 Storage Coeff. (min)= 12.83 (ii) 33.58 (ii)
 Unit Hyd. Tpeak (min)= 15.00 45.00
 Unit Hyd. peak (cms)= 0.08 0.03

TOTALS
 PEAK FLOW (cms)= 9.35 2.95 10.562 (iii)
 TIME TO PEAK (hrs)= 6.00 6.50 6.00
 RUNOFF VOLUME (mm)= 71.10 18.05 33.43
 TOTAL RAINFALL (mm)= 73.10 73.10 73.10
 RUNOFF COEFFICIENT = 0.97 0.25 0.46

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 53.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5344)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.6700	6.1252
0.3000	1.6125	3.3150	7.0314
0.8100	3.0036	3.8990	7.9700
1.4170	4.2559	4.2990	7.9800
1.8540	5.0526	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (3441)	155.150	10.562	6.00
OUTFLOW: ID= 1 (5344)	155.150	1.052	8.25

PEAK FLOW REDUCTION [Qout/Qin](%)= 9.96
 TIME SHIFT OF PEAK FLOW (min)=135.00
 MAXIMUM STORAGE USED (ha.m.)= 3.5039

ADD HYD (9344)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3442):	188.84	10.687	6.00
+ ID2= 2 (5344):	155.15	1.052	8.25
ID = 3 (9344):	343.99	10.957	6.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8230)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8234):	30226.80	137.626	12.50
+ ID2= 2 (9344):	343.99	10.957	6.00
ID = 3 (8230):	30570.79	138.627	12.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8228)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8226):	1952.06	11.661	8.50
+ ID2= 2 (8230):	30570.79	138.627	12.50
ID = 3 (8228):	32522.85	146.202	12.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8190)
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0356):	1015.22	4.610	8.00
+ ID2= 2 (8228):	32522.85	146.202	12.00
ID = 3 (8190):	33538.07	149.263	12.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

** SIMULATION NUMBER: 5 **

READ STORM
 Ptotal = 80.80 mm

Filename: C:\Users\aman\cu\AppData\Local\Temp\ab70bd54-a452-450c-b03c-83cdc4826a98\2515b17a
 Comments: 50-Year 12-Hour SCS II Design Storm

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.25	2.02	3.25	3.23	6.25	14.54	9.25	2.83
0.50	2.02	3.50	3.23	6.50	14.54	9.50	2.83
0.75	2.02	3.75	3.23	6.75	6.46	9.75	2.83
1.00	2.02	4.00	3.23	7.00	6.46	10.00	2.83
1.25	2.02	4.25	4.85	7.25	4.85	10.25	1.62
1.50	2.02	4.50	4.85	7.50	4.85	10.50	1.62
1.75	2.02	4.75	6.46	7.75	4.85	10.75	1.62
2.00	2.02	5.00	6.46	8.00	4.85	11.00	1.62
2.25	2.42	5.25	9.70	8.25	2.83	11.25	1.62
2.50	2.42	5.50	9.70	8.50	2.83	11.50	1.62
2.75	2.42	5.75	38.78	8.75	2.83	11.75	1.62
3.00	2.42	6.00	106.66	9.00	2.83	12.00	1.62

CALIB
 NASHYD (0356)
 ID= 1 DT=15.0 min
 Area (ha)=1015.22
 Ia (mm)= 5.00
 U. H. Tp(hrs)= 1.37
 Curve Number (CN)= 46.0
 # of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 12.651
 PEAK FLOW (cms)= 5.609 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 15.266
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.189

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0352)
 NASHYD
 ID= 1 DT=15.0 min

Area (ha)=	381.43	Curve Number (CN)=	54.0
Ia (mm)=	5.00	# of Linear Res. (N)=	1.50
U. H. Tp(hrs)=	0.82		

Unit Hyd Qpeak (cms)= 7.980
 PEAK FLOW (cms)= 4.018 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 19.394
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.240

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0350)
 NASHYD
 ID= 1 DT=15.0 min

Area (ha)=	366.84	Curve Number (CN)=	48.0
Ia (mm)=	5.00	# of Linear Res. (N)=	1.50
U. H. Tp(hrs)=	1.07		

Unit Hyd Qpeak (cms)= 5.831
 PEAK FLOW (cms)= 2.583 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 16.221
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.201

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0348)
 NASHYD
 ID= 1 DT=15.0 min

Area (ha)=	590.18	Curve Number (CN)=	48.0
Ia (mm)=	5.00	# of Linear Res. (N)=	1.50
U. H. Tp(hrs)=	1.91		

Unit Hyd Qpeak (cms)= 5.267
 PEAK FLOW (cms)= 2.683 (i)
 TIME TO PEAK (hrs)= 9.000
 RUNOFF VOLUME (mm)= 16.307
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.202

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6350)
 IN= 2---> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3501.0) ----->

Distance	Elevation	Manning	
0.00	287.00	0.0500	
23.66	283.72	0.0500	
70.98	280.94	0.0500	
112.38	280.23	0.0500	
171.52	276.80	0.0500	
260.24	274.46	0.0500	
266.16	274.26	0.0500	
272.07	274.12	0.0500	
276.49	274.12	0.0500 / 0.0300	Main Channel
276.99	273.82	0.0300	Main Channel
277.99	273.82	0.0300	Main Channel
279.49	273.82	0.0300	Main Channel
280.49	274.27	0.0300 / 0.0500	Main Channel
283.90	274.27	0.0500	
289.81	274.57	0.0500	
325.30	275.75	0.0500	
396.28	278.98	0.0500	
449.51	280.97	0.0500	
496.83	283.90	0.0500	
585.54	287.92	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
-----------	----------	-----------------	-----------------	----------------	------------------

0.30	274.12	.273E+04	0.9	0.99	49.81
0.98	274.80	.583E+05	20.8	1.05	46.71
1.66	275.48	.203E+06	95.2	1.38	35.53
2.33	276.15	.438E+06	256.0	1.73	28.50
3.01	276.83	.755E+06	524.8	2.05	23.97
3.69	277.51	.114E+07	935.8	2.43	20.26
4.37	278.19	.157E+07	1472.0	2.76	17.82
5.05	278.87	.206E+07	2140.7	3.06	16.06
5.72	279.54	.261E+07	2938.6	3.33	14.79
6.40	280.22	.321E+07	3887.6	3.57	13.77
7.08	280.90	.390E+07	4795.4	3.63	13.56
7.76	281.58	.468E+07	6196.2	3.91	12.58
8.43	282.25	.550E+07	7803.9	4.19	11.74
9.11	282.93	.637E+07	9601.9	4.45	11.05
9.79	283.61	.728E+07	11594.8	4.70	10.46
10.47	284.29	.823E+07	13845.3	4.96	9.91
11.15	284.97	.922E+07	16293.4	5.21	9.43
11.82	285.64	.103E+08	18952.0	5.45	9.02
12.50	286.32	.113E+08	21824.3	5.69	8.65

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	<--- hydrograph --->	<--- pi pe / channel --->
					MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0348)	590.18	2.68	9.00	16.31	0.36	0.99
OUTFLOW: ID= 1 (6350)	590.18	2.62	10.25	16.31	0.36	0.99

ADD HYD (8220)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0350):	366.84	2.583	7.50	16.22
+ ID2= 2 (6350):	590.18	2.617	10.25	16.31
=====				
ID = 3 (8220):	957.02	4.788	8.75	16.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDBY (3461)
 ID= 1 DT=15.0 min

Area Total	(ha)= 33.62	Di r. Conn. (%)=	38.00
Imp (%)=	61.00		

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	20.51	13.11
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	473.43	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	106.66	100.59
over (min)	15.00	30.00
Storage Coeff. (min)=	7.79 (ii)	18.46 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.10	0.05

PEAK FLOW (cms)=	3.41	1.93	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	4.678 (iii)
RUNOFF VOLUME (mm)=	78.80	41.08	55.41
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.51	0.69

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5346)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2---> OUT= 1				
DT= 15.0 min				
	0.0000	0.0000	2.0090	0.6158
	0.2450	0.2524	2.6260	0.6610
	0.6230	0.3644	3.2370	0.7019
	0.9920	0.4690	3.6370	0.7119
	1.2510	0.5428	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3461)	33.620	4.678	6.00	55.41
OUTFLOW: ID= 1 (5346)	33.620	2.485	6.25	55.41

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 53.12
 TIME SHIFT OF PEAK FLOW (mi n) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 0.6679

CALIB	Area (ha)	Curve Number (CN)
NASHYD (3462)	317.31	71.0
ID= 1 DT=15.0 mi n	U.H. Tp(hrs)= 0.96	# of Li near Res. (N)= 1.50

Unit Hyd Qpeak (cms) = 5.625

PEAK FLOW (cms) = 4.897 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 31.656
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.392

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9346)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (3462):	317.31	4.897	7.25	31.66
+ ID2= 2 (5346):	33.62	2.485	6.25	55.41
ID = 3 (9346):	350.93	6.816	6.50	33.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8222)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (8220):	957.02	4.788	8.75	16.27
+ ID2= 2 (9346):	350.93	6.816	6.50	33.93
ID = 3 (8222):	1307.95	10.135	7.50	21.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6352)	Routing time step (mi n)'
IN= 2--> OUT= 1	15.00

Distance	Elevation	Manning	Main Channel
0.00	257.95	0.0500	
7.83	257.34	0.0500	
15.66	256.19	0.0500	
21.53	254.01	0.0500	
41.11	244.06	0.0500	
76.35	241.38	0.0500	
111.58	239.74	0.0500	
113.54	239.61	0.0500	
113.75	239.41	0.0500 / 0.0300	Main Channel
113.85	238.81	0.0300	Main Channel
115.50	238.81	0.0300	Main Channel
116.15	238.81	0.0300	Main Channel
116.25	239.41	0.0300	Main Channel
117.46	239.52	0.0500	
119.41	239.72	0.0500	
121.37	240.04	0.0500	
131.16	241.84	0.0500	
156.61	247.03	0.0500	
176.19	251.46	0.0500	
193.80	258.79	0.0500	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.60	230.61	.451E+04	1.5	1.06	49.40
1.62	231.63	.394E+05	14.6	1.16	45.13
2.64	232.65	.128E+06	58.8	1.44	36.30
3.66	233.67	.269E+06	149.7	1.74	30.00
4.68	234.69	.463E+06	301.5	2.04	25.58
5.70	235.71	.705E+06	530.5	2.36	22.14
6.72	236.73	.980E+06	848.4	2.71	19.25
7.74	237.75	.128E+07	1237.9	3.02	17.29
8.76	238.77	.162E+07	1706.7	3.30	15.83
9.78	239.79	.199E+07	2291.9	3.61	14.45
10.81	240.82	.237E+07	2973.7	3.92	13.30
11.83	241.84	.278E+07	3742.2	4.21	12.38
12.85	242.86	.321E+07	4599.1	4.49	11.62

0.60	239.41	.224E+04	2.2	1.52	17.12
1.58	240.39	.216E+05	22.3	1.61	16.16
2.55	241.36	.804E+05	101.0	1.96	13.27
3.53	242.34	.173E+06	275.6	2.48	10.46
4.50	243.31	.293E+06	553.9	2.95	8.80
5.48	244.29	.438E+06	961.8	3.42	7.60
6.45	245.26	.599E+06	1534.4	3.99	6.51
7.43	246.24	.770E+06	2228.4	4.51	5.76
8.41	247.22	.951E+06	3043.6	4.98	5.21
9.38	248.19	.114E+07	3982.1	5.43	4.78
10.36	249.17	.134E+07	5040.1	5.85	4.44
11.33	250.14	.155E+07	6218.5	6.24	4.16
12.31	251.12	.177E+07	7518.6	6.61	3.93
13.29	252.10	.200E+07	8969.0	6.98	3.72
14.26	253.07	.224E+07	10554.1	7.35	3.53
15.24	254.05	.248E+07	12257.1	7.71	3.37
16.21	255.02	.272E+07	14045.1	8.03	3.23
17.19	256.00	.298E+07	15954.8	8.34	3.11
18.16	256.97	.325E+07	17780.4	8.53	3.04

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	<- pi pe / channel -> MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8222)	1307.95	10.13	7.50	21.01	0.99	1.55
OUTFLOW: ID= 1 (6352)	1307.95	10.08	7.75	21.01	0.98	1.55

ADD HYD (8224)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0352):	381.43	4.018	7.00	19.39
+ ID2= 2 (6352):	1307.95	10.082	7.75	21.01
ID = 3 (8224):	1689.38	13.917	7.50	20.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6354)	Routing time step (mi n)'
IN= 2--> OUT= 1	15.00

Distance	Elevation	Manning	Main Channel
0.00	253.92	0.0500	
7.95	251.83	0.0500	
15.89	249.97	0.0500	
19.87	249.05	0.0500	
47.68	242.00	0.0500	
67.55	237.10	0.0500	
83.44	231.11	0.0500	
85.43	230.75	0.0500	
85.66	230.61	0.0500 / 0.0300	Main Channel
85.76	230.01	0.0300	Main Channel
87.41	230.01	0.0300	Main Channel
88.06	230.01	0.0300	Main Channel
88.16	230.61	0.0300 / 0.0500	Main Channel
89.40	230.72	0.0500	
91.39	230.88	0.0500	
93.37	231.03	0.0500	
133.11	233.96	0.0500	
150.99	235.39	0.0500	
172.84	239.03	0.0500	
196.68	250.00	0.0500	

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.60	230.61	.451E+04	1.5	1.06	49.40
1.62	231.63	.394E+05	14.6	1.16	45.13
2.64	232.65	.128E+06	58.8	1.44	36.30
3.66	233.67	.269E+06	149.7	1.74	30.00
4.68	234.69	.463E+06	301.5	2.04	25.58
5.70	235.71	.705E+06	530.5	2.36	22.14
6.72	236.73	.980E+06	848.4	2.71	19.25
7.74	237.75	.128E+07	1237.9	3.02	17.29
8.76	238.77	.162E+07	1706.7	3.30	15.83
9.78	239.79	.199E+07	2291.9	3.61	14.45
10.81	240.82	.237E+07	2973.7	3.92	13.30
11.83	241.84	.278E+07	3742.2	4.21	12.38
12.85	242.86	.321E+07	4599.1	4.49	11.62

13.87	243.88	.365E+07	5544.6	4.75	10.98
14.89	244.90	.412E+07	6579.7	5.00	10.44
15.91	245.92	.461E+07	7705.9	5.23	9.97
16.93	246.94	.512E+07	8924.6	5.46	9.55
17.95	247.96	.564E+07	10237.5	5.68	9.19
18.97	248.98	.619E+07	11646.2	5.89	8.86

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8224)	1689.38	13.92	7.50	20.65	1.57	1.15
OUTFLOW: ID= 1 (6354)	1689.38	12.91	8.50	20.65	1.48	1.14

CALIB STANDHYD (3541) ID= 1 DT=15.0 min	Area (ha)= 8.01	Imp(%)= 75.00	Di r. Conn.(%)= 70.00
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	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	6.01	2.00
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	231.08	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	106.66	13.49
over (min)	15.00	30.00
Storage Coeff. (min)=	5.07 (ii)	28.91 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.11	0.04

TOTALS	1.637 (iii)	
PEAK FLOW (cms)=	1.61	0.05
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	78.80	12.69
TOTAL RAINFALL (mm)=	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.16

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 35.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5354)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.5360	0.2546
0.0270	0.1427	0.6730	0.2758
0.1550	0.1714	0.7830	0.2966
0.2890	0.1975	1.1830	0.3066
0.4010	0.2251	0.0000	0.0000

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID= 2 (3541)	8.010	1.637	6.00	58.96
OUTFLOW: ID= 1 (5354)	8.010	0.534	6.25	58.80

PEAK FLOW REDUCTION [Qout/Qin] (%)=	32.63
TIME SHIFT OF PEAK FLOW (min)=	15.00
MAXIMUM STORAGE USED (ha.m.)=	0.2625

CALIB
NASHYD (3542)
ID= 1 DT=15.0 min

Area (ha)= 254.67	Curve Number (CN)= 36.7
Ia (mm)= 5.00	# of Linear Res. (N)= 1.50
U.H. Tp(hrs)= 1.37	

Unit Hyd Qpeak (cms)=	3.174
PEAK FLOW (cms)=	1.018 (i)
TIME TO PEAK (hrs)=	8.000
RUNOFF VOLUME (mm)=	11.109
TOTAL RAINFALL (mm)=	80.800
RUNOFF COEFFICIENT =	0.137

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9354) 1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (3542):	254.67	1.018	8.00	11.11
+ ID2= 2 (5354):	8.01	0.534	6.25	58.80
ID= 3 (9354):	262.68	1.235	6.50	12.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8226) 1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (6354):	1689.38	12.911	8.50	20.65
+ ID2= 2 (9354):	262.68	1.235	6.50	12.56
ID= 3 (8226):	1952.06	14.006	8.50	19.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (3402) ID= 1 DT=15.0 min	Area (ha)= 138.83	Imp(%)= 46.00	Di r. Conn.(%)= 28.00
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	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	63.86	74.97
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	962.05	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	106.66	25.06
over (min)	15.00	45.00
Storage Coeff. (min)=	11.92 (ii)	30.53 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03

TOTALS	9.18	3.21	10.510 (iii)
PEAK FLOW (cms)=	6.00	6.50	6.00
TIME TO PEAK (hrs)=	78.80	20.64	36.93
RUNOFF VOLUME (mm)=	80.80	80.80	80.80
TOTAL RAINFALL (mm)=	0.98	0.26	0.46
RUNOFF COEFFICIENT =			

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 47.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3401) ID= 1 DT=15.0 min	Area (ha)= 146.96	Imp(%)= 58.00	Di r. Conn.(%)= 35.00
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	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	85.24	61.72
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	989.81	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	106.66	53.85
over (min)	15.00	30.00
Storage Coeff. (min)=	12.13 (ii)	25.83 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04

TOTALS	12.07	4.12	14.630 (iii)
PEAK FLOW (cms)=	6.00	6.25	6.00
TIME TO PEAK (hrs)=	78.80	23.23	42.68
RUNOFF VOLUME (mm)=	80.80	80.80	80.80
TOTAL RAINFALL (mm)=	0.98	0.29	0.53
RUNOFF COEFFICIENT =			

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 47.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5340)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	11.4780	4.3560
0.3860	2.0347	14.9940	4.9991
3.2310	2.5818	18.1680	5.6243
5.1770	3.1417	18.5680	5.6343
7.7590	3.6318	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3401)	146.960	14.630	6.00	42.68
OUTFLOW: ID= 1 (5340)	146.960	4.994	6.50	42.67

PEAK FLOW REDUCTION [Qout/Qin] (%) = 34.14
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha.m.) = 3.1315

ADD HYD (8232)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3402):	138.83	10.510	6.00	36.93
+ ID2= 2 (5340):	146.96	4.994	6.50	42.67
ID = 3 (8232):	285.79	10.880	6.00	39.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0410)
ID= 1 DT=15.0 min

Area (ha)= 572.01 Curve Number (CN)= 48.0
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 1.46

Unit Hyd Opeak (cms)= 6.698

PEAK FLOW (cms)= 3.210 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 16.276
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.201

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0408)
ID= 1 DT=15.0 min

Area (ha)= 231.62 Curve Number (CN)= 58.0
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 0.64

Unit Hyd Opeak (cms)= 6.198

PEAK FLOW (cms)= 3.331 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 21.683
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.268

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0376)
ID= 1 DT=15.0 min

Area (ha)= 463.85 Curve Number (CN)= 74.0
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 1.07

Unit Hyd Opeak (cms)= 7.380

PEAK FLOW (cms)= 7.184 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 34.493
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.427

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0374)
ID= 1 DT=15.0 min

Area (ha)= 545.70 Curve Number (CN)= 61.0
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 1.51

Unit Hyd Opeak (cms)= 6.158

PEAK FLOW (cms)= 4.432 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 23.989
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.297

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0372)
ID= 1 DT=15.0 min

Area (ha)= 110.42 Curve Number (CN)= 37.0
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 0.96

Unit Hyd Opeak (cms)= 1.954

PEAK FLOW (cms)= 0.576 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 11.182
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.138

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0370)
ID= 1 DT=15.0 min

Area (ha)= 191.85 Curve Number (CN)= 63.0
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 0.67

Unit Hyd Opeak (cms)= 4.860

PEAK FLOW (cms)= 3.080 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 25.072
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.310

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0366)
ID= 1 DT=15.0 min

Area (ha)= 462.62 Curve Number (CN)= 62.0
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 1.06

Unit Hyd Opeak (cms)= 7.451

PEAK FLOW (cms)= 5.064 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 24.589
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.304

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3641)
ID= 1 DT=15.0 min

Area (ha)= 7.85
Total Imp(%)= 45.00 Dir. Conn. (%)= 16.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	3.53	4.32	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	228.76	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	106.66	45.70	
over (min)	15.00	30.00	
Storage Coeff. (min)=	5.04 (ii)	19.67 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.11	0.05	
			TOTALS
PEAK FLOW (cms)=	0.36	0.28	0.533 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	78.80	20.11	29.50
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.25	0.37

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 42.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5364)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.5250	0.2044
0.0270	0.0959	0.6590	0.2337
0.1520	0.1110	0.7670	0.2624
0.2830	0.1432	1.1670	0.2724
0.3930	0.1710	0.0000	0.0000

INFLOW : ID= 2 (3641)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5364)	7.850	0.533	6.00	29.50
	7.850	0.175	6.75	29.39

PEAK FLOW REDUCTION [Qout/Oin] (%) = 32.87
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 0.1178

CALIB
 STANDHYD (3642)
 ID= 1 DT=15.0 min

Area (ha) = 147.42
 Total Imp(%) = 21.00
 Dir. Conn. (%) = 7.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	30.96	116.46
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	991.36	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	106.66	17.00
over (min)	15.00	45.00
Storage Coeff. (min)=	12.14 (ii)	33.87 (ii)
Unit Hyd. Tpeak (min)=	15.00	45.00
Unit Hyd. peak (cms)=	0.08	0.03

			TOTALS
PEAK FLOW (cms)=	2.42	3.22	3.811 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.50
RUNOFF VOLUME (mm)=	78.80	16.11	20.50
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.20	0.25

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 42.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9364)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (3642):	147.42	3.811	6.50	20.50
+ ID2= 2 (5364):	7.85	0.175	6.75	29.39
ID= 3 (9364):	155.27	3.979	6.50	20.95

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8302)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0366):	462.62	5.064	7.50	24.59
+ ID2= 2 (9364):	155.27	3.979	6.50	20.95
ID= 3 (8302):	617.89	8.409	6.50	23.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0358)
 ID= 1 DT=15.0 min

Area (ha) = 429.87
 Curve Number (CN) = 35.0
 Ia (mm) = 5.00
 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 1.03

Unit Hyd Opeak (cms) = 7.091

PEAK FLOW (cms) = 1.973 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 10.392
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.129

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0360)
 ID= 1 DT=15.0 min

Area (ha) = 138.37
 Curve Number (CN) = 46.0
 Ia (mm) = 5.00
 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 0.60

Unit Hyd Opeak (cms) = 3.957

PEAK FLOW (cms) = 1.427 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 15.028
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.186

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8306)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0358):	429.87	1.973	7.50	10.39
+ ID2= 2 (0360):	138.37	1.427	6.75	15.03
ID= 3 (8306):	568.24	3.321	7.00	11.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6362)
 IN= 2--> OUT= 1
 Routing time step (min) = 15.00

----- DATA FOR SECTION (3621.0) ----->

Distance	Elevation	Manning
0.00	261.46	0.0550
27.86	254.23	0.0550
51.07	251.96	0.0550

74.29	250.77	0.0550	
97.50	249.91	0.0550	
125.36	249.40	0.0550	
150.93	247.40	0.0550	
155.93	247.33	0.0550 / 0.0350	Main Channel
157.93	246.85	0.0350	Main Channel
159.18	246.65	0.0350	Main Channel
160.18	246.63	0.0350	Main Channel
160.93	246.85	0.0350	Main Channel
161.93	247.18	0.0350 / 0.0550	Main Channel
163.18	248.03	0.0550	
168.18	248.58	0.0550	
183.18	250.18	0.0550	
201.97	252.59	0.0550	
213.57	256.02	0.0550	
225.18	260.31	0.0550	
229.82	261.00	0.0550	

-----> TRAVEL TIME TABLE <----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.55	247.18	.556E+04	1.9	1.03	49.95
1.28	247.91	.336E+05	16.7	1.53	33.61
2.00	248.63	.913E+05	53.6	1.81	28.40
2.73	249.36	.185E+06	122.3	2.05	25.18
3.46	250.09	.344E+06	224.6	2.02	25.52
4.19	250.82	.574E+06	417.9	2.25	22.91
4.91	251.54	.855E+06	699.1	2.53	20.38
5.64	252.27	.118E+07	1075.3	2.82	18.26
6.37	253.00	.153E+07	1559.9	3.15	16.38
7.10	253.73	.191E+07	2137.6	3.46	14.89
7.82	254.45	.231E+07	2815.2	3.77	13.67
8.55	255.18	.272E+07	3608.5	4.10	12.58
9.28	255.91	.315E+07	4488.2	4.41	11.69
10.01	256.64	.358E+07	5458.0	4.71	10.95
10.73	257.36	.403E+07	6512.4	4.99	10.32
11.46	258.09	.449E+07	7649.8	5.27	9.78
12.19	258.82	.496E+07	8869.5	5.53	9.32
12.92	259.55	.544E+07	10171.2	5.78	8.91
13.64	260.27	.593E+07	11554.8	6.02	8.55

-----> hydrograph <-----> <-- pi pe / channel -->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8306)	568.24	3.32	7.00	11.52	0.62	1.07
OUTFLOW: ID= 1 (6362)	568.24	2.94	8.25	11.52	0.60	1.06

-----> CALIB STANDHYD (3621) <----->

Area (ha) = 11.87	Dir. Conn. (%) = 17.00
Total Imp (%) = 45.00	

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	5.34	6.53
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	281.31	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (mi n)	106.66 / 15.00	48.76 / 30.00
Storage Coeff. (mi n)	5.70 (ii)	19.96 (ii)
Unit Hyd. Tpeak (mi n)	15.00	30.00
Unit Hyd. peak (cms)	0.11	0.05
PEAK FLOW (cms)	0.57	0.45
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	78.80	21.64
TOTAL RAINFALL (mm)	80.80	80.80
RUNOFF COEFFICIENT	0.98	0.27

TOTALS
0.848 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 45.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5362)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.7940	0.3091
0.0400	0.1451	0.9970	0.3534
0.2300	0.1678	1.1610	0.3968
0.4280	0.2166	1.5610	0.4068
0.5940	0.2586	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3621)	11.870	0.848	6.00	31.35
OUTFLOW: ID= 1 (5362)	11.870	0.289	6.75	31.28

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 34.01
TIME SHIFT OF PEAK FLOW (mi n) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.1842

-----> CALIB STANDHYD (3622) <----->

Area (ha) = 106.91	Dir. Conn. (%) = 9.00
Total Imp (%) = 24.00	

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	25.66	81.25
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	844.24	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (mi n)	106.66 / 15.00	19.29 / 45.00
Storage Coeff. (mi n)	11.02 (ii)	31.68 (ii)
Unit Hyd. Tpeak (mi n)	15.00	45.00
Unit Hyd. peak (cms)	0.08	0.03

TOTALS
3.415 (iii)
6.00
23.35
80.80
0.29

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 45.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

-----> ADD HYD (9362) <----->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3622):	106.91	3.415	6.00	23.35
+ ID2= 2 (5362):	11.87	0.289	6.75	31.28
ID = 3 (9362):	118.78	3.443	6.00	24.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

-----> ADD HYD (8304) <----->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6362):	568.24	2.938	8.25	11.52
+ ID2= 2 (9362):	118.78	3.443	6.00	24.14
ID = 3 (8304):	687.02	4.941	6.50	13.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8300)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (8302):	617.89	8.409	6.50	23.67	
+ ID2= 2 (8304):	687.02	4.941	6.50	13.70	
=====					
ID = 3 (8300):	1304.91	13.350	6.50	18.42	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6368)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3681.0) ----->					
Distance	Elevation	Manning			
0.00	230.00	0.0370			
18.48	223.26	0.0370			
36.96	223.05	0.0370			
64.67	222.94	0.0370			
110.87	222.86	0.0370			
133.96	222.74	0.0370			
147.82	222.65	0.0370			
170.92	222.31	0.0370			
174.79	222.26	0.0300	/0.0300	Main Channel	
174.89	221.86	0.0300		Main Channel	
175.54	221.86	0.0300		Main Channel	
176.19	221.86	0.0300		Main Channel	
176.29	222.26	0.0300	/0.0370	Main Channel	
180.16	222.25	0.0370			
184.78	222.28	0.0370			
189.40	222.31	0.0370			
332.60	222.37	0.0370			
450.00	230.00	0.0370			

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)
0.40	222.26	.771E+03	0.3	0.52	41.74
0.81	222.67	.784E+05	30.3	0.51	43.17
1.21	223.07	.210E+06	123.2	0.77	28.42
1.62	223.48	.384E+06	301.0	1.03	21.25
2.03	223.89	.563E+06	550.2	1.28	17.06
2.44	224.30	.747E+06	860.7	1.51	14.46
2.84	224.70	.934E+06	1228.1	1.73	12.68
3.25	225.11	.113E+07	1649.3	1.92	11.37
3.66	225.52	.132E+07	2122.3	2.11	10.37
4.07	225.93	.152E+07	2645.5	2.29	9.58
4.47	226.33	.172E+07	3217.8	2.45	8.92
4.88	226.74	.193E+07	3838.3	2.61	8.38
5.29	227.15	.214E+07	4506.3	2.76	7.92
5.70	227.56	.236E+07	5221.3	2.91	7.52
6.10	227.96	.258E+07	5983.0	3.05	7.17
6.51	228.37	.280E+07	6790.9	3.19	6.87
6.92	228.78	.303E+07	7645.0	3.32	6.60
7.33	229.19	.326E+07	8545.1	3.45	6.35
7.73	229.59	.349E+07	9491.0	3.57	6.13

<----- hydrograph ----->						<-pi pe / channel-->	
AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL		
(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW: ID= 2 (8300)	1304.91	13.35	6.50	18.42	0.58	0.52	
OUTFLOW: ID= 1 (6368)	1304.91	10.52	7.50	18.42	0.54	0.52	

CALIB
STANDHYD (3681)
ID= 1 DT=15.0 min Area (ha)= 2.95
Total Imp(%)= 45.00 Dir. Conn.(%)= 15.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	1.33	1.62
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	140.24	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	106.66	43.38
over (min)=	15.00	30.00
Storage Coeff. (min)=	3.75 (ii)	18.69 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.11	0.05

			TOTALS
PEAK FLOW (cms)=	0.13	0.10	0.193 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	78.80	18.92	27.90
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.23	0.35

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 40.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5368)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha. m.)	(cms)	(ha. m.)
	0.0000	0.0000	0.1970	0.0767
	0.0100	0.0360	0.2470	0.0877
	0.0570	0.0416	0.2880	0.0984
	0.1060	0.0537	0.6880	0.1084
	0.1470	0.0642	0.0000	0.0000

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 (3681)	2.950	0.193	6.00	27.90
OUTFLOW: ID= 1 (5368)	2.950	0.061	6.75	27.60

PEAK FLOW REDUCTION [Qout/Qi n] (%) =	31.69
TIME SHIFT OF PEAK FLOW (min) =	45.00
MAXIMUM STORAGE USED (ha. m.) =	0.0430

CALIB
NASHYD (3682)
ID= 1 DT=15.0 min Area (ha)= 156.53
Ia (mm)= 5.00 Curve Number (CN)= 45.9
U. H. Tp(hrs)= 0.96 # of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)=	2.778
PEAK FLOW (cms)=	1.119 (i)
TIME TO PEAK (hrs)=	7.250
RUNOFF VOLUME (mm)=	15.149
TOTAL RAINFALL (mm)=	80.800
RUNOFF COEFFICIENT =	0.187

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	(9368)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (3682):	156.53	1.119	7.25	15.15	
+ ID2= 2 (5368):	2.95	0.061	6.75	27.60	
=====					
ID = 3 (9368):	159.48	1.164	7.00	15.38	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8298)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (6368):	1304.91	10.523	7.50	18.42	
+ ID2= 2 (9368):	159.48	1.164	7.00	15.38	
=====					
ID = 3 (8298):	1464.39	11.670	7.50	18.09	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0370):	191.85	3.080	6.75	25.07
+ ID2= 2 (8298):	1464.39	11.670	7.50	18.09
=====				
ID = 3 (8296):	1656.24	14.492	7.25	18.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6372)
IN= 2---> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3721.0) ----->				
Distance	Elevation	Manning		
0.00	225.00	0.0390		
30.80	219.38	0.0390		
61.61	219.30	0.0390		
77.01	219.27	0.0390		
469.76	219.14	0.0390		
477.46	219.13	0.0390		
485.16	219.10	0.0390		
492.86	219.09	0.0390		
495.56	219.09	0.0310	/0.0310	Main Channel
500.56	218.51	0.0310		Main Channel
505.46	218.51	0.0310		Main Channel
505.56	219.09	0.0310	/0.0390	Main Channel
508.26	219.09	0.0390		
515.96	219.10	0.0390		
523.67	219.21	0.0390		
562.17	219.32	0.0390		
654.58	219.43	0.0390		
731.59	219.46	0.0390		
762.39	225.00	0.0390		

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.29	218.80	.431E+04	1.7	0.58	43.48
0.58	219.09	.867E+04	5.1	0.89	28.32
0.93	219.44	.200E+06	58.2	0.44	57.35
1.28	219.79	.568E+06	278.6	0.74	33.99
1.62	220.13	.939E+06	630.2	1.01	24.84
1.97	220.48	.131E+07	1090.4	1.25	20.05
2.32	220.83	.169E+07	1647.7	1.47	17.06
2.67	221.18	.206E+07	2294.6	1.68	14.99
3.01	221.52	.244E+07	3025.6	1.87	13.46
3.36	221.87	.282E+07	3836.5	2.05	12.27
3.71	222.22	.321E+07	4723.7	2.22	11.31
4.06	222.57	.359E+07	5684.6	2.39	10.53
4.40	222.91	.398E+07	6716.6	2.55	9.87
4.75	223.26	.437E+07	7817.7	2.70	9.31
5.10	223.61	.476E+07	8986.2	2.85	8.83
5.45	223.96	.515E+07	10220.4	2.99	8.40
5.79	224.30	.555E+07	11519.1	3.13	8.03
6.14	224.65	.594E+07	12880.8	3.27	7.69
6.49	225.00	.634E+07	14304.5	3.40	7.39

<----- hydrograph ----->					<- pipe / channel ->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8296)	1656.24	14.49	7.25	18.90	0.64	0.75
OUTFLOW: ID= 1 (6372)	1656.24	13.64	8.25	18.90	0.64	0.76

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0372):	110.42	0.576	7.25	11.18
+ ID2= 2 (6372):	1656.24	13.636	8.25	18.90
=====				
ID = 3 (8294):	1766.66	14.179	8.25	18.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0374):	545.70	4.432	8.25	23.99
+ ID2= 2 (8294):	1766.66	14.179	8.25	18.42
=====				
ID = 3 (8292):	2312.36	18.611	8.25	19.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5505)				
IN= 2---> OUT= 1				
DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	65.1290	345.3754
	25.4850	24.6697	84.9510	456.3890
	31.1490	98.6787	*****	838.7689
	39.6440	*****	*****	838.7789
	48.1390	*****	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (8292)	2312.360	18.611	8.25	19.73
OUTFLOW: ID= 1 (5505)	2312.360	13.335	11.00	19.73

PEAK FLOW REDUCTION [Qout/Qin] (%) = 71.65			
TIME SHIFT OF PEAK FLOW (min)=165.00			
MAXIMUM STORAGE USED (ha. m.) = 12.9148			

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0376):	463.85	7.184	7.25	34.49
+ ID2= 2 (5505):	2312.36	13.335	11.00	19.73
=====				
ID = 3 (8272):	2776.21	18.024	10.00	22.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0396)			
ID= 1 DT=15.0 min	Area (ha)= 305.21	Curve Number (CN)= 69.0	
	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50	
	U. H. Tp(hrs)= 1.08		
Unit Hyd Opeak	(cms)= 4.811		
PEAK FLOW	(cms)= 4.047 (i)		
TIME TO PEAK	(hrs)= 7.500		
RUNOFF VOLUME	(mm)= 29.979		
TOTAL RAINFALL	(mm)= 80.800		
RUNOFF COEFFICIENT	= 0.371		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0394)			
ID= 1 DT=15.0 min	Area (ha)= 325.45	Curve Number (CN)= 53.0	
	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50	
	U. H. Tp(hrs)= 0.92		
Unit Hyd Opeak	(cms)= 6.013		
PEAK FLOW	(cms)= 3.012 (i)		
TIME TO PEAK	(hrs)= 7.250		
RUNOFF VOLUME	(mm)= 18.867		
TOTAL RAINFALL	(mm)= 80.800		
RUNOFF COEFFICIENT	= 0.233		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0390)			
ID= 1 DT=15.0 min	Area (ha)= 420.00	Curve Number (CN)= 55.0	
	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50	
	U. H. Tp(hrs)= 1.07		
Unit Hyd Opeak	(cms)= 6.013		
PEAK FLOW	(cms)= 3.012 (i)		
TIME TO PEAK	(hrs)= 7.250		
RUNOFF VOLUME	(mm)= 18.867		
TOTAL RAINFALL	(mm)= 80.800		
RUNOFF COEFFICIENT	= 0.233		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Unit Hyd Qpeak (cms) = 6.683

PEAK FLOW (cms) = 3.688 (i)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 20.072
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.248

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0388) Area (ha) = 220.77 Curve Number (CN) = 58.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 0.99

Unit Hyd Qpeak (cms) = 3.819

PEAK FLOW (cms) = 2.265 (i)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 21.891
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.271

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0386) Area (ha) = 241.27 Curve Number (CN) = 61.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 0.90

Unit Hyd Qpeak (cms) = 4.562

PEAK FLOW (cms) = 2.901 (i)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 23.835
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.295

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8286) AREA OPEAK TPEAK R. V.
1 + 2 = 3 (ha) (cms) (hrs) (mm)
ID1= 1 (0386): 241.27 2.901 7.00 23.84
+ ID2= 2 (0388): 220.77 2.265 7.25 21.89

ID = 3 (8286): 462.04 5.164 7.25 22.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0384) Area (ha) = 199.07 Curve Number (CN) = 44.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 0.96

Unit Hyd Qpeak (cms) = 3.537

PEAK FLOW (cms) = 1.336 (i)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 14.242
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.176

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0380) Area (ha) = 182.01 Curve Number (CN) = 40.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 0.55

Unit Hyd Qpeak (cms) = 5.609

PEAK FLOW (cms) = 1.618 (i)
TIME TO PEAK (hrs) = 6.500
RUNOFF VOLUME (mm) = 12.271
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.152

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0382) Area (ha) = 216.59 Curve Number (CN) = 53.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 0.64

Unit Hyd Qpeak (cms) = 5.733

PEAK FLOW (cms) = 2.645 (i)
TIME TO PEAK (hrs) = 6.750
RUNOFF VOLUME (mm) = 18.713
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.232

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8290) AREA OPEAK TPEAK R. V.
1 + 2 = 3 (ha) (cms) (hrs) (mm)
ID1= 1 (0380): 182.01 1.618 6.50 12.27
+ ID2= 2 (0382): 216.59 2.645 6.75 18.71

ID = 3 (8290): 398.60 4.252 6.75 15.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6384) Routing time step (min) = 15.00
IN= 2--> OUT= 1

DATA FOR SECTION (3841.0)

Distance	Elevation	Manning
0.00	294.40	0.0380
10.59	291.93	0.0380
21.17	289.19	0.0380
26.46	287.99	0.0380
31.76	286.79	0.0380
71.45	279.97	0.0380
74.10	279.79	0.0380
76.74	279.71	0.0380
78.99	279.64	0.0380 / 0.0300
79.14	279.30	0.0300
79.39	279.30	0.0300
79.64	279.30	0.0300
79.79	279.64	0.0300 / 0.0380
89.98	279.78	0.0380
119.09	281.86	0.0380
145.55	282.87	0.0380
198.48	284.85	0.0380
211.71	286.31	0.0380
230.23	287.59	0.0380
261.99	294.00	0.0380

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.34	279.64	.715E+03	0.2	0.85	63.42
1.10	280.40	.507E+05	21.9	1.40	38.54
1.85	281.15	.141E+06	91.8	2.10	25.69
2.61	281.91	.269E+06	218.9	2.63	20.46
3.36	282.66	.445E+06	407.1	2.96	18.23
4.12	283.42	.681E+06	698.3	3.31	16.26
4.87	284.17	.977E+06	1111.8	3.68	14.65
5.63	284.93	.133E+07	1674.3	4.06	13.27
6.39	285.69	.173E+07	2457.8	4.59	11.73
7.14	286.44	.215E+07	3378.8	5.07	10.63
7.90	287.20	.261E+07	4418.4	5.47	9.85
8.65	287.95	.310E+07	5676.3	5.92	9.11
9.41	288.71	.361E+07	7150.5	6.40	8.42
10.17	289.47	.414E+07	8783.1	6.86	7.86
10.92	290.22	.469E+07	10575.1	7.30	7.38

11.68	290.98	.525E+07	12520.4	7.72	6.98
12.43	291.73	.582E+07	14619.0	8.12	6.64
13.19	292.49	.642E+07	16863.2	8.50	6.34
13.94	293.24	.703E+07	19259.8	8.86	6.08

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	<-pi pe / channel--> MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8290)	398.60	4.25	6.75	15.77	0.48	0.92
OUTFLOW: ID= 1 (6384)	398.60	3.34	8.00	15.77	0.45	0.90

ADD HYD (8288) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0384):	199.07	1.336	7.25	14.24
+ ID2= 2 (6384):	398.60	3.339	8.00	15.77
ID = 3 (8288):	597.67	4.648	7.75	15.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8284) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8286):	462.04	5.164	7.25	22.91
+ ID2= 2 (8288):	597.67	4.648	7.75	15.26
ID = 3 (8284):	1059.71	9.705	7.50	18.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8280) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0390):	420.00	3.688	7.50	20.07
+ ID2= 2 (8284):	1059.71	9.705	7.50	18.59
ID = 3 (8280):	1479.71	13.393	7.50	19.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0392) ID= 1 DT=15.0 min	Area (ha) = 167.22 Ia (mm) = 5.00 U. H. Tp (hrs) = 0.74	Curve Number (CN) = 62.0 # of Linear Res. (N) = 1.50
--	---	---

Unit Hyd Opeak (cms) = 3.837
 PEAK FLOW (cms) = 2.409 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 24.429
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.302

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0378) ID= 1 DT=15.0 min	Area (ha) = 606.72 Ia (mm) = 5.00 U. H. Tp (hrs) = 1.18	Curve Number (CN) = 55.0 # of Linear Res. (N) = 1.50
--	---	---

Unit Hyd Opeak (cms) = 8.771
 PEAK FLOW (cms) = 4.960 (i)
 TIME TO PEAK (hrs) = 7.750
 RUNOFF VOLUME (mm) = 20.097
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.249

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8282) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0378):	606.72	4.960	7.75	20.10
+ ID2= 2 (0392):	167.22	2.409	7.00	24.43
ID = 3 (8282):	773.94	7.226	7.25	21.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8278) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8280):	1479.71	13.393	7.50	19.01
+ ID2= 2 (8282):	773.94	7.226	7.25	21.03
ID = 3 (8278):	2253.65	20.600	7.50	19.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6394)
IN= 2--> OUT= 1

Routing time step (min)' = 15.00

Distance	Elevation	Manning
0.00	283.00	0.0380
13.48	282.87	0.0380
53.92	280.08	0.0380
74.13	276.62	0.0380
97.72	265.45	0.0380
114.57	256.93	0.0380
131.42	253.04	0.0380
134.79	252.58	0.0380
138.53	251.74	0.0380 / 0.0300
139.03	251.20	0.0300
141.53	251.20	0.0300
144.03	251.20	0.0300
144.53	251.74	0.0300 / 0.0380
148.27	252.69	0.0380
151.64	252.97	0.0380
185.34	255.08	0.0380
219.03	257.54	0.0380
262.84	259.43	0.0380
310.02	262.80	0.0380
333.60	283.00	0.0380

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.54	251.74	.827E+04	4.5	1.51	30.71
2.19	253.39	.758E+05	73.2	2.69	17.27
3.83	255.03	.282E+06	331.8	3.27	14.17
5.48	256.68	.633E+06	917.3	4.03	11.51
7.12	258.32	.112E+07	1889.5	4.69	9.90
8.77	259.97	.179E+07	3410.2	5.31	8.73
10.41	261.61	.259E+07	5627.6	6.05	7.66
12.06	263.26	.351E+07	8605.1	6.83	6.79
13.70	264.90	.447E+07	12544.9	7.81	5.94
15.35	266.55	.546E+07	17101.4	8.71	5.32
16.99	268.19	.648E+07	22251.7	9.56	4.85
18.64	269.84	.752E+07	27979.0	10.36	4.48
20.28	271.48	.858E+07	34269.0	11.11	4.17
21.93	273.13	.967E+07	41110.8	11.83	3.92
23.57	274.77	.108E+08	48495.9	12.51	3.71
25.22	276.42	.119E+08	56418.1	13.17	3.52
26.86	278.06	.131E+08	64410.6	13.69	3.39
28.51	279.71	.143E+08	73002.1	14.18	3.27
30.15	281.35	.156E+08	81379.3	14.50	3.20

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	<-pi pe / channel--> MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8278)	2253.65	20.60	7.50	19.71	0.93	1.68
OUTFLOW: ID= 1 (6394)	2253.65	20.08	8.00	19.71	0.91	1.68

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0394):	325.45	3.012	7.25	18.87
+ ID2= 2 (6394):	2253.65	20.077	8.00	19.71
=====				
ID = 3 (8276):	2579.10	22.946	8.00	19.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6396)
IN= 2---> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3961.0) ----->

Distance	Elevation	Manning	
0.00	263.00	0.0410	
11.75	257.14	0.0410	
23.50	253.97	0.0410	
41.13	247.83	0.0410	
76.38	232.09	0.0410	
135.13	229.07	0.0410	
149.82	228.97	0.0410	
152.75	228.96	0.0410 / 0.0300	Main Channel
154.19	228.73	0.0300	Main Channel
154.69	228.20	0.0300	Main Channel
155.69	228.20	0.0300	Main Channel
156.69	228.20	0.0300	Main Channel
157.19	228.73	0.0300	Main Channel
158.63	228.95	0.0300 / 0.0410	Main Channel
161.57	228.96	0.0410	
164.51	229.71	0.0410	
196.82	241.70	0.0410	
223.26	249.21	0.0410	
246.76	255.13	0.0410	
290.82	263.51	0.0410	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.75	228.95	.780E+04	2.2	0.98	58.06
2.54	230.74	.277E+06	145.1	1.78	31.76
4.33	232.53	.786E+06	609.1	2.63	21.50
6.13	234.33	.140E+07	1481.5	3.59	15.77
7.92	236.12	.207E+07	2677.3	4.39	12.90
9.71	237.91	.280E+07	4184.8	5.09	11.14
11.50	239.70	.357E+07	6001.4	5.71	9.93
13.29	241.49	.441E+07	8129.1	6.27	9.03
15.09	243.29	.530E+07	10549.4	6.77	8.37
16.88	245.08	.625E+07	13300.6	7.24	7.83
18.67	246.87	.726E+07	16396.2	7.68	7.38
20.46	248.66	.834E+07	19809.8	8.07	7.02
22.26	250.46	.949E+07	23532.5	8.43	6.72
24.05	252.25	.107E+08	27641.2	8.77	6.46
25.84	254.04	.120E+08	32155.2	9.10	6.22
27.63	255.83	.134E+08	36901.0	9.37	6.05
29.42	257.62	.149E+08	42147.7	9.64	5.88
31.22	259.42	.164E+08	48149.1	9.97	5.68
33.01	261.21	.181E+08	54644.8	10.29	5.51

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8276)	2579.10	22.95	8.00	19.60	1.01	1.04
OUTFLOW: ID= 1 (6396)	2579.10	20.73	9.00	19.60	0.98	1.04

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0396):	305.21	4.047	7.50	29.98
+ ID2= 2 (6396):	2579.10	20.733	9.00	19.60
=====				
ID = 3 (8274):	2884.31	24.211	9.00	20.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8272):	2776.21	18.024	10.00	22.20
+ ID2= 2 (8274):	2884.31	24.211	9.00	20.70
=====				
ID = 3 (8270):	5660.52	41.573	9.25	21.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5506)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	60.8810	135.6832
31.1490	24.6697	96.2770	900.4431
36.8120	37.0045	96.6770	900.4531
45.3070	86.3439	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (8270)	5660.520	41.573	9.25	21.44
OUTFLOW: ID= 1 (5506)	5660.520	32.561	12.00	21.44

PEAK FLOW REDUCTION [Out/Oi n] (%) = 78.32
TIME SHIFT OF PEAK FLOW (min) = 165.00
MAXIMUM STORAGE USED (ha. m.) = 27.7584

CALIB NASHYD (0406)
ID= 1 DT=15.0 min
Area (ha) = 142.65
Ia (mm) = 5.00
U. H. Tp(hrs) = 0.59
Curve Number (CN) = 66.0
of Li near Res. (N) = 1.50

Unit Hyd Qpeak (cms) = 4.135

PEAK FLOW (cms) = 2.779 (i)
TIME TO PEAK (hrs) = 6.500
RUNOFF VOLUME (mm) = 27.183
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.336

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0404)
ID= 1 DT=15.0 min
Area (ha) = 246.46
Ia (mm) = 5.00
U. H. Tp(hrs) = 0.98
Curve Number (CN) = 47.0
of Li near Res. (N) = 1.50

Unit Hyd Qpeak (cms) = 4.280

PEAK FLOW (cms) = 1.795 (i)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 15.696
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.194

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0402)
ID= 1 DT=15.0 min
Area (ha) = 244.00
Ia (mm) = 5.00
U. H. Tp(hrs) = 1.07
Curve Number (CN) = 61.0
of Li near Res. (N) = 1.50

Unit Hyd Qpeak (cms) = 3.879

PEAK FLOW (cms) = 2.567 (i)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 23.900
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.296

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0400) Area (ha) = 93.97 Curve Number (CN) = 52.0
 ID= 1 DT=15.0 min I a (mm) = 5.00 # of Li near Res. (N) = 1.50
 U. H. Tp(hrs) = 0.44

Unit Hyd Qpeak (cms) = 3.630

PEAK FLOW (cms) = 1.483 (i)
 TIME TO PEAK (hrs) = 6.500
 RUNOFF VOLUME (mm) = 17.889
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.221

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0398) Area (ha) = 328.19 Curve Number (CN) = 55.0
 ID= 1 DT=15.0 min I a (mm) = 5.00 # of Li near Res. (N) = 1.50
 U. H. Tp(hrs) = 0.83

Unit Hyd Qpeak (cms) = 6.759

PEAK FLOW (cms) = 3.522 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 19.985
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.247

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3 (8268)				
ID1= 1 (0398):	328.19	3.522	7.00	19.99
+ ID2= 2 (0400):	93.97	1.483	6.50	17.89
ID = 3 (8268):	422.16	4.864	6.75	19.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6402) Routing time step (min) = 15.00
 IN= 2----> OUT= 1

Distance	Elevation	Manning	Channel
0.00	238.50	0.0360	
11.50	238.00	0.0360	
23.00	237.93	0.0360	
34.49	236.39	0.0360	
63.24	233.98	0.0360	
97.73	228.15	0.0360	
123.60	227.08	0.0360	
126.48	226.61	0.0360	
127.60	226.47	0.0360 / 0.0330	Main Channel
127.85	225.25	0.0330	Main Channel
129.35	225.25	0.0330	Main Channel
130.85	225.25	0.0330	Main Channel
131.10	226.47	0.0330 / 0.0360	Main Channel
132.22	226.59	0.0360	
143.72	227.42	0.0360	
169.59	227.88	0.0360	
192.59	231.19	0.0360	
218.46	233.02	0.0360	
241.45	235.50	0.0360	
284.57	236.43	0.0360	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.41	225.66	.391E+04	1.7	1.36	38.31
0.81	226.06	.804E+04	5.0	1.93	27.02
1.22	226.47	.124E+05	9.1	2.30	22.61
1.84	227.09	.308E+05	23.3	2.36	22.03
2.46	227.71	.904E+05	62.0	2.14	24.31

3.09	228.34	.223E+06	188.3	2.63	19.75
3.71	228.96	.379E+06	410.2	3.38	15.39
4.33	229.58	.550E+06	711.5	4.04	12.89
4.95	230.20	.737E+06	1091.3	4.62	11.25
5.58	230.83	.939E+06	1550.6	5.15	10.10
6.20	231.45	.116E+07	2072.4	5.59	9.31
6.82	232.07	.140E+07	2659.2	5.93	8.77
7.44	232.69	.167E+07	3345.8	6.27	8.30
8.07	233.32	.195E+07	4159.0	6.64	7.83
8.69	233.94	.226E+07	5100.8	7.03	7.39
9.31	234.56	.259E+07	6081.3	7.32	7.11
9.93	235.18	.295E+07	7183.1	7.60	6.84
10.56	235.81	.334E+07	8145.2	7.62	6.83
11.18	236.43	.379E+07	9116.0	7.51	6.92

INFLOW	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
ID= 2 (8268)	422.16	4.86	6.75	19.52	0.80	1.90
OUTFLOW: ID= 1 (6402)	422.16	4.63	7.25	19.52	0.77	1.84

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3 (8266)				
ID1= 1 (0402):	244.00	2.567	7.50	23.90
+ ID2= 2 (6402):	422.16	4.628	7.25	19.52
ID = 3 (8266):	666.16	7.186	7.25	21.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3 (8264)				
ID1= 1 (0404):	246.46	1.795	7.25	15.70
+ ID2= 2 (8266):	666.16	7.186	7.25	21.12
ID = 3 (8264):	912.62	8.981	7.25	19.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3 (8262)				
ID1= 1 (0406):	142.65	2.779	6.50	27.18
+ ID2= 2 (8264):	912.62	8.981	7.25	19.66
ID = 3 (8262):	1055.27	11.503	7.25	20.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3 (8260)				
ID1= 1 (5506):	5660.52	32.561	12.00	21.44
+ ID2= 2 (8262):	1055.27	11.503	7.25	20.68
ID = 3 (8260):	6715.79	37.879	10.25	21.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3 (8258)				
ID1= 1 (0408):	231.62	3.331	6.75	21.68
+ ID2= 2 (8260):	6715.79	37.879	10.25	21.32
ID = 3 (8258):	6947.41	39.172	10.25	21.33

INFLOW : ID= 2 (9424) 158.820 14.105 6.00 47.49
 OUTFLOW: ID= 1 (5414) 158.820 4.095 7.00 47.48

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 29.03
 TIME SHIF T OF PEAK FLOW (mi n) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 3.8974

CALIB STANDHYD (4142)
 ID= 1 DT=15.0 mi n
 Area (ha)= 29.14
 Total Imp(%)= 52.00 Di r. Conn.(%)= 21.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 15.15 13.99
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 440.76 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 106.66 98.96
 over (mi n) 15.00 30.00
 Storage Coeff. (mi n)= 7.46 (ii) 18.21 (ii)
 Uni t Hyd. Tpeak (mi n)= 15.00 30.00
 Uni t Hyd. peak (cms)= 0.10 0.05

PEAK FLOW (cms)= 1.65 2.03 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.25 2.979 (iii)
 RUNOFF VOLUME (mm)= 78.80 39.15 6.00
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.48 0.59

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8254)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (4142): 29.14 2.979 6.00 47.47
 + ID2= 2 (5414): 158.82 4.095 7.00 47.48
 ID = 3 (8254): 187.96 5.690 6.25 47.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (4121)
 ID= 1 DT=15.0 mi n
 Area (ha)= 202.00
 Total Imp(%)= 45.00 Di r. Conn.(%)= 16.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 90.90 111.10
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 1160.46 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 106.66 59.01
 over (mi n) 15.00 30.00
 Storage Coeff. (mi n)= 13.34 (ii) 26.55 (ii)
 Uni t Hyd. Tpeak (mi n)= 15.00 30.00
 Uni t Hyd. peak (cms)= 0.08 0.04

PEAK FLOW (cms)= 7.32 8.02 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.25 12.319 (iii)
 RUNOFF VOLUME (mm)= 78.80 25.67 6.00
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.32 0.42

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 51.5 Ia = Dep. Storage (Above)

- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5412)
 IN= 2 --> OUT= 1
 DT= 15.0 mi n
 OUTFLOW STORAGE OUTFLOW STORAGE
 (cms) (ha. m.) (cms) (ha. m.)
 0.0000 0.0000 6.5000 1.0500
 0.3860 0.5085 7.9000 1.1500
 1.8460 0.6103 9.3000 1.2500
 3.5050 0.7566 9.7000 1.2600
 4.7890 0.8929 0.0000 0.0000

AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (4121) 202.000 12.319 6.00 34.17
 OUTFLOW: ID= 1 (5412) 202.000 11.040 6.25 34.17

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 89.62
 TIME SHIF T OF PEAK FLOW (mi n) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 1.3043

CALIB STANDHYD (4122)
 ID= 1 DT=15.0 mi n
 Area (ha)= 36.70
 Total Imp(%)= 63.00 Di r. Conn.(%)= 22.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 23.12 13.58
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 494.64 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 106.66 102.38
 over (mi n) 15.00 30.00
 Storage Coeff. (mi n)= 8.00 (ii) 18.60 (ii)
 Uni t Hyd. Tpeak (mi n)= 15.00 30.00
 Uni t Hyd. peak (cms)= 0.10 0.05

PEAK FLOW (cms)= 2.14 2.02 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.25 3.443 (iii)
 RUNOFF VOLUME (mm)= 78.80 32.05 6.00
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.40 0.52

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 51.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9412)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (4122): 36.70 3.443 6.00 42.34
 + ID2= 2 (5412): 202.00 11.040 6.25 34.17
 ID = 3 (9412): 238.70 13.664 6.25 35.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8252)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8254): 187.96 5.690 6.25 47.48
 + ID2= 2 (9412): 238.70 13.664 6.25 35.42
 ID = 3 (8252): 426.66 19.354 6.25 40.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6416)
IN= 2----> OUT= 1 Routing time step (min)' = 15.00

----- DATA FOR SECTION (4161.0) ----->

Distance	Elevation	Manning	
0.00	270.07	0.0340	
20.67	267.91	0.0340	
62.01	264.33	0.0340	
113.69	259.75	0.0340	
165.37	253.30	0.0340	
227.38	246.29	0.0340	
232.55	246.03	0.0340	
237.72	246.16	0.0340	
241.39	246.02	0.0340 / 0.0300	Main Channel
241.64	245.75	0.0300	Main Channel
242.89	245.75	0.0300	Main Channel
244.14	245.75	0.0300	Main Channel
244.39	246.02	0.0300 / 0.0340	Main Channel
248.06	246.20	0.0340	
253.22	246.28	0.0340	
258.39	246.63	0.0340	
346.25	252.57	0.0340	
413.43	257.77	0.0340	
465.11	261.78	0.0340	
511.62	270.00	0.0340	

----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.27	246.02	.295E+04	0.6	0.75	88.26
1.53	247.28	.168E+06	69.7	1.65	40.13
2.79	248.54	.490E+06	299.5	2.42	27.28
4.06	249.81	.962E+06	740.1	3.05	21.67
5.32	251.07	.158E+07	1440.0	3.61	18.33
6.58	252.33	.235E+07	2443.8	4.12	16.06
7.84	253.59	.327E+07	3811.2	4.63	14.30
9.10	254.85	.432E+07	5565.1	5.11	12.94
10.37	256.12	.550E+07	7721.3	5.57	11.88
11.63	257.38	.682E+07	10309.9	6.00	11.02
12.89	258.64	.827E+07	13360.6	6.42	10.31
14.15	259.90	.985E+07	16884.5	6.81	9.72
15.42	261.17	.116E+08	20818.4	7.14	9.26
16.68	262.43	.134E+08	25510.4	7.53	8.78
17.94	263.69	.154E+08	30958.0	7.97	8.30
19.20	264.95	.175E+08	36988.3	8.38	7.89
20.46	266.21	.197E+08	43617.1	8.78	7.53
21.73	267.48	.220E+08	50868.1	9.17	7.22
22.99	268.74	.244E+08	58854.0	9.56	6.92

----- hydrograph -----> <- pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8252)	426.66	19.35	6.25	40.73	0.61	0.88
OUTFLOW: ID= 1 (6416)	426.66	8.91	7.25	40.73	0.42	0.80

CALIB NASHYD (4164)
ID= 1 DT=15.0 min
Area (ha)= 89.30
Curve Number (CN)= 76.0
Ia (mm)= 5.00
of Linear Res. (N)= 3.00
U. H. Tp(hrs)= 1.46

Unit Hyd Opeak (cms)= 2.344
PEAK FLOW (cms)= 2.327 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 36.826
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.456

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9427)
IN= 2----> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.4700	2.5000
0.9000	1.0000	5.9800	3.0000
1.7300	1.5000	7.5000	3.5000

3.2200 2.0000 | 8.7400 4.0000

AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
INFLOW : ID= 2 (4164) 89.300 2.327 7.50 36.83
OUTFLOW: ID= 1 (9427) 89.300 1.375 9.25 36.82

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 59.10
TIME SHIFT OF PEAK FLOW (min)=105.00
MAXIMUM STORAGE USED (ha. m.) = 1.2874

RESERVOIR (5416)
IN= 2----> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	1.5980	0.7565
0.0330	0.3532	1.8570	0.8137
0.7450	0.4481	2.0990	0.8707
1.0270	0.5154	2.4990	0.8807
1.3050	0.6350	0.0000	0.0000

AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
INFLOW : ID= 2 (9427) 89.300 1.375 9.25 36.82
OUTFLOW: ID= 1 (5416) 89.300 1.232 10.25 36.79

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 89.59
TIME SHIFT OF PEAK FLOW (min) = 60.00
MAXIMUM STORAGE USED (ha. m.) = 0.6039

CALIB NASHYD (4162)
ID= 1 DT=15.0 min

Area (ha)= 349.99
Ia (mm)= 5.00
U. H. Tp(hrs)= 1.29
Curve Number (CN)= 61.3
of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 4.646

PEAK FLOW (cms)= 3.252 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 24.159
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.299

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9416)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4162):	349.99	3.252	8.00	24.16
+ ID2= 2 (5416):	89.30	1.232	10.25	36.79
ID = 3 (9416):	439.29	4.090	9.00	26.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8250)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6416):	426.66	8.906	7.25	40.73
+ ID2= 2 (9416):	439.29	4.090	9.00	26.73
ID = 3 (8250):	865.95	12.069	7.25	33.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0418)
ID= 1 DT=15.0 min

Area (ha)= 174.09
Ia (mm)= 5.00
U. H. Tp(hrs)= 1.05
Curve Number (CN)= 64.0
of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 2.825

PEAK FLOW (cms) = 2.034 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 26.026
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.322

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (4184) | Area (ha) = 8.70
 ID= 1 DT=15.0 min | Total Imp(%) = 30.00 Di r. Conn.(%) = 10.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	2.61	6.09
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	240.83	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) over (min)	106.66	78.36
Storage Coeff. (min)	5.19 (ii)	16.99 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.11	0.05

TOTALS
 0.767 (iii)
 6.25
 43.38
 80.80
 0.54

PEAK FLOW (cms) = 0.25
 TIME TO PEAK (hrs) = 6.00
 RUNOFF VOLUME (mm) = 78.80
 TOTAL RAINFALL (mm) = 80.80
 RUNOFF COEFFICIENT = 0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 73.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9428) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	0.4300	0.1618
	0.0300	0.0818	0.5800	0.2012
	0.1700	0.0912	0.7300	0.2384
	0.3100	0.1306	0.8500	0.2747

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (4184)	8.700	0.767	6.25	43.38
OUTFLOW: ID= 1 (9428)	8.700	0.394	6.50	43.30

PEAK FLOW REDUCTION [Qout/Qin] (%) = 51.41
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha.m.) = 0.1554

ADD HYD (9423) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0418):	174.09	2.034	7.50	26.03
+ ID2= 2 (9428):	8.70	0.394	6.50	43.30
ID = 3 (9423):	182.79	2.348	7.00	26.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8248) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8250):	865.95	12.069	7.25	33.63
+ ID2= 2 (9423):	182.79	2.348	7.00	26.85

=====

ID = 3 (8248):	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
	1048.74	14.393	7.25	32.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8246) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0420):	175.82	1.800	7.00	18.82
+ ID2= 2 (8248):	1048.74	14.393	7.25	32.45
ID = 3 (8246):	1224.56	16.174	7.25	30.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8244) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (5507):	7519.42	40.270	11.75	20.94
+ ID2= 2 (8246):	1224.56	16.174	7.25	30.49
ID = 3 (8244):	8743.98	50.284	10.50	22.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 STANDHYD (4227) | Area (ha) = 66.80
 ID= 1 DT=15.0 min | Total Imp(%) = 29.00 Di r. Conn.(%) = 15.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	19.37	47.43
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	667.33	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) over (min)	106.66	25.06
Storage Coeff. (min)	9.57 (ii)	28.18 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.09	0.04

TOTALS
 2.54
 6.00
 78.80
 80.80
 0.98

PEAK FLOW (cms) = 2.54
 TIME TO PEAK (hrs) = 6.00
 RUNOFF VOLUME (mm) = 78.80
 TOTAL RAINFALL (mm) = 80.80
 RUNOFF COEFFICIENT = 0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 54.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9426) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	3.3400	1.2303
	0.2300	0.6171	4.4700	1.5350
	1.3000	0.6860	5.6100	1.8228
	2.4100	0.9909	6.5300	2.1041

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (4227)	66.800	3.954	6.00	31.21
OUTFLOW: ID= 1 (9426)	66.800	1.798	6.75	31.20

PEAK FLOW REDUCTION [Qout/Qin] (%) = 45.47
 TIME SHIFT OF PEAK FLOW (min) = 45.00

MAXIMUM STORAGE USED (ha. m.) = 0.8326

RESERVOIR (5422)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.3110	2.6782
0.2180	1.7874	5.4130	2.9186
1.2490	1.9110	6.3010	3.1540
2.3250	2.1760	6.7010	3.1640
3.2250	2.4038	0.0000	0.0000

INFLOW : ID= 2 (9426)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5422)	66.800	1.798	6.75	31.20
	66.800	0.153	15.25	31.17

PEAK FLOW REDUCTION [Qout/Qin] (%) = 8.50
TIME SHIFT OF PEAK FLOW (min) = 510.00
MAXIMUM STORAGE USED (ha. m.) = 1.2538

CALIB NASHYD (4222)
ID= 1 DT=15.0 min

Area (ha) = 713.41 Curve Number (CN) = 54.0
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 1.95

Unit Hyd Opeak (cms) = 6.252
PEAK FLOW (cms) = 3.855 (i)
TIME TO PEAK (hrs) = 9.000
RUNOFF VOLUME (mm) = 19.591
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.242

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9422)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4222):	713.41	3.855	9.00	19.59
+ ID2= 2 (5422):	66.80	0.153	15.25	31.17
=====				
ID = 3 (9422):	780.21	3.986	9.00	20.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8242)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8244):	8743.98	50.284	10.50	22.28
+ ID2= 2 (9422):	780.21	3.986	9.00	20.58
=====				
ID = 3 (8242):	9524.19	54.172	10.25	22.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5508)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	*****	197.3574
76.4550	30.8371	*****	394.7148
*****	61.6742	*****	394.7248
*****	*****	0.0000	0.0000

INFLOW : ID= 2 (8242)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5508)	9524.190	54.172	10.25	22.14
	9524.190	51.747	11.75	22.14

PEAK FLOW REDUCTION [Qout/Qin] (%) = 95.52
TIME SHIFT OF PEAK FLOW (min) = 90.00

MAXIMUM STORAGE USED (ha. m.) = 20.8822

CALIB NASHYD (0336)
ID= 1 DT=15.0 min

Area (ha) = 2785.00 Curve Number (CN) = 72.0
Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 15.39

Unit Hyd Opeak (cms) = 3.090
PEAK FLOW (cms) = 3.992 (i)
TIME TO PEAK (hrs) = 22.250
RUNOFF VOLUME (mm) = 32.865
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.407

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3382)
ID= 1 DT=15.0 min

Area Total Imp(%) = 39.00 Dir. Conn. (%) = 20.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 169.98 264.31
Dep. Storage (mm) = 2.00 5.00
Average Slope (%) = 0.50 0.50
Length (m) = 1699.59 40.00
Mannings n = 0.013 0.250
Max. Eff. Inten. (mm/hr) = 106.66 65.39
over (min) = 15.00 30.00
Storage Coeff. (min) = 16.77 (ii) 29.45 (ii)
Unit Hyd. Tpeak (min) = 15.00 30.00
Unit Hyd. peak (cms) = 0.07 0.04

PEAK FLOW (cms) = 17.80 20.09 *TOTALS*
TIME TO PEAK (hrs) = 6.00 6.25 30.431 (iii)
RUNOFF VOLUME (mm) = 78.80 32.61 6.00
TOTAL RAINFALL (mm) = 80.80 80.80 41.85
RUNOFF COEFFICIENT = 0.98 0.40 80.80
0.52

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 64.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3381)
ID= 1 DT=15.0 min

Area Total Imp(%) = 53.00 Dir. Conn. (%) = 27.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 87.78 77.84
Dep. Storage (mm) = 2.00 5.00
Average Slope (%) = 0.50 0.50
Length (m) = 1050.78 40.00
Mannings n = 0.013 0.250
Max. Eff. Inten. (mm/hr) = 106.66 86.27
over (min) = 15.00 30.00
Storage Coeff. (min) = 12.57 (ii) 23.92 (ii)
Unit Hyd. Tpeak (min) = 15.00 30.00
Unit Hyd. peak (cms) = 0.08 0.04

PEAK FLOW (cms) = 10.36 8.72 *TOTALS*
TIME TO PEAK (hrs) = 6.00 6.25 15.946 (iii)
RUNOFF VOLUME (mm) = 78.80 36.25 47.74
TOTAL RAINFALL (mm) = 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.45 0.59

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 64.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5338)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	9.5300	3.8529
	0.6370	1.6350	12.1350	4.2725
	2.7300	2.1009	14.3850	4.6808
	5.1010	2.7445	14.7850	4.6908
	7.0460	3.2936	0.0000	0.0000
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3381)	165.620	15.946	6.00	47.74
OUTFLOW: ID= 1 (5338)	165.620	7.069	6.75	47.74
PEAK FLOW REDUCTION [Qout/Qin] (%) = 44.33				
TIME SHIFT OF PEAK FLOW (min) = 45.00				
MAXIMUM STORAGE USED (ha. m.) = 3.3590				

ADD HYD (8310)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3382):	433.29	30.431	6.00	41.85
+ ID2= 2 (5338):	165.62	7.069	6.75	47.74
ID = 3 (8310):	598.91	34.971	6.25	43.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (3342)			
ID= 1 DT=15.0 min			
Area (ha)	= 586.87	Curve Number (CN)	= 56.0
Ia (mm)	= 5.00	# of Linear Res. (N)	= 1.50
U. H. Tp (hrs)	= 8.20		
Unit Hyd Qpeak (cms)	= 1.221		
PEAK FLOW (cms)	= 0.991 (i)		
TIME TO PEAK (hrs)	= 15.250		
RUNOFF VOLUME (mm)	= 20.856		
TOTAL RAINFALL (mm)	= 80.800		
RUNOFF COEFFICIENT	= 0.258		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3341)			
ID= 1 DT=15.0 min			
Area (ha)	= 33.23	Dir. Conn. (%)	= 21.00
Total Imp (%)	= 51.00		
IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)	= 16.95		16.28
Dep. Storage (mm)	= 2.00		5.00
Average Slope (%)	= 0.50		0.50
Length (m)	= 470.67		40.00
Mannings n	= 0.013		0.250
Max. Eff. Inten. (mm/hr)	= 106.66		65.60
over (min)	= 15.00		30.00
Storage Coeff. (min)	= 7.76 (ii)		20.43 (ii)
Unit Hyd. Tpeak (min)	= 15.00		30.00
Unit Hyd. peak (cms)	= 0.10		0.05
TOTALS			
PEAK FLOW (cms)	= 1.87		1.49
TIME TO PEAK (hrs)	= 6.00		6.25
RUNOFF VOLUME (mm)	= 78.80		26.99
TOTAL RAINFALL (mm)	= 80.80		80.80
RUNOFF COEFFICIENT	= 0.98		0.33

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 51.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5334)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	0.6690	0.4644
	0.0400	0.1927	0.8520	0.5318
	0.1900	0.2643	1.0360	0.5973
	0.3830	0.3576	1.4360	0.6073
	0.4460	0.3863	0.0000	0.0000
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3341)	33.230	2.804	6.00	37.87
OUTFLOW: ID= 1 (5334)	33.230	0.980	6.75	37.83
PEAK FLOW REDUCTION [Qout/Qin] (%) = 34.94				
TIME SHIFT OF PEAK FLOW (min) = 45.00				
MAXIMUM STORAGE USED (ha. m.) = 0.5837				

ADD HYD (8314)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3342):	586.87	0.991	15.25	20.86
+ ID2= 2 (5334):	33.23	0.980	6.75	37.83
ID = 3 (8314):	620.10	1.336	6.75	21.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0306)			
ID= 1 DT=15.0 min			
Area (ha)	= 283.97	Curve Number (CN)	= 52.0
Ia (mm)	= 5.00	# of Linear Res. (N)	= 1.50
U. H. Tp (hrs)	= 6.44		
Unit Hyd Qpeak (cms)	= 0.753		
PEAK FLOW (cms)	= 0.538 (i)		
TIME TO PEAK (hrs)	= 13.750		
RUNOFF VOLUME (mm)	= 18.507		
TOTAL RAINFALL (mm)	= 80.800		
RUNOFF COEFFICIENT	= 0.229		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0282)			
ID= 1 DT=15.0 min			
Area (ha)	= 449.38	Curve Number (CN)	= 77.0
Ia (mm)	= 5.00	# of Linear Res. (N)	= 1.50
U. H. Tp (hrs)	= 1.47		
Unit Hyd Qpeak (cms)	= 5.226		
PEAK FLOW (cms)	= 5.992 (i)		
TIME TO PEAK (hrs)	= 8.000		
RUNOFF VOLUME (mm)	= 37.664		
TOTAL RAINFALL (mm)	= 80.800		
RUNOFF COEFFICIENT	= 0.466		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0284)			
ID= 1 DT=15.0 min			
Area (ha)	= 78.93	Curve Number (CN)	= 84.0
Ia (mm)	= 5.00	# of Linear Res. (N)	= 1.50
U. H. Tp (hrs)	= 0.57		
Unit Hyd Qpeak (cms)	= 2.344		
PEAK FLOW (cms)	= 2.746 (i)		
TIME TO PEAK (hrs)	= 6.500		
RUNOFF VOLUME (mm)	= 45.199		
TOTAL RAINFALL (mm)	= 80.800		
RUNOFF COEFFICIENT	= 0.559		

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	AREA	OPEAK	TPEAK	R.V.
(8388)	(ha)	(cms)	(hrs)	(mm)
1 + 2 = 3				
ID1= 1 (0282):	449.38	5.992	8.00	37.66
+ ID2= 2 (0284):	78.93	2.746	6.50	45.20
ID = 3 (8388):	528.31	8.107	7.25	38.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6286)
IN= 2----> OUT= 1 Routing time step (mi n)' = 15.00

<----- DATA FOR SECTION (2861.0) ----->

Distance	Elevation	Manning	
0.00	233.00	0.0450	
20.58	228.51	0.0450	
41.17	227.74	0.0450	
51.46	227.41	0.0450	
97.77	225.96	0.0450	
149.23	223.94	0.0450	
200.69	220.84	0.0450	
226.42	220.66	0.0450	
238.85	220.22	0.0450	
241.35	220.01	0.0450 / 0.0350	Main Channel
241.85	219.70	0.0350	Main Channel
245.85	219.72	0.0350	Main Channel
246.35	220.06	0.0350	Main Channel
248.85	220.23	0.0350	Main Channel
303.60	221.64	0.0350	Main Channel
380.79	224.98	0.0450	
432.25	229.54	0.0450	
457.98	233.33	0.0450	
483.71	234.27	0.0450	
509.44	233.81	0.0450	

DEPTH	ELEV	VOLUME	TRAVEL TIME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(hrs)	(cms)	(m/s)	(mi n)
0.31	220.01	382E+04	0.8	0.57	0.57	82.89
1.06	220.76	525E+05	12.1	0.66	0.66	72.51
1.81	221.51	240E+06	88.6	1.05	1.05	45.21
2.56	222.26	510E+06	261.7	1.46	1.46	32.47
3.31	223.01	843E+06	533.8	1.81	1.81	26.34
4.06	223.76	124E+07	913.6	2.10	2.10	22.64
4.81	224.51	171E+07	1399.0	2.34	2.34	20.33
5.56	225.26	225E+07	2051.0	2.60	2.60	18.27
6.31	226.01	285E+07	2907.0	2.91	2.91	16.35
7.06	226.76	352E+07	3902.5	3.16	3.16	15.04
7.82	227.52	426E+07	5069.9	3.39	3.39	14.01
8.57	228.27	507E+07	6433.0	3.62	3.62	13.13
9.32	229.02	593E+07	8069.2	3.89	3.89	12.24
10.07	229.77	681E+07	9926.7	4.16	4.16	11.44
10.82	230.52	772E+07	11962.8	4.42	4.42	10.75
11.57	231.27	864E+07	14167.1	4.68	4.68	10.17
12.32	232.02	959E+07	16537.1	4.92	4.92	9.66
13.07	232.77	105E+08	19070.7	5.16	5.16	9.22
13.82	233.52	115E+08	21790.4	5.39	5.39	8.82

	AREA	OPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW : ID= 2 (8388)	528.31	8.11	7.25	38.79	0.80	0.62
OUTFLOW: ID= 1 (6286)	528.31	6.86	9.00	38.79	0.71	0.62

CALIB STANDHYD (2861)
ID= 1 DT=15.0 mi n Area (ha)= 54.86
Total Imp(%)= 45.00 Di r. Conn.(%)= 19.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	24.69	30.17
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50

Length (m)=	604.76	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	106.66	110.98
over (mi n)	15.00	30.00
Storage Coeff. (mi n)=	9.02 (ii)	19.28 (ii)
Uni t Hyd. Tpeak (mi n)=	15.00	30.00
Uni t Hyd. peak (cms)=	0.09	0.05
PEAK FLOW (cms)=	2.68	4.84
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	78.80	48.61
TOTAL RAI NFALL (mm)=	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.60

TOTALS
5.922 (iii)
6.00
54.35
80.80
0.67

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5286)
IN= 2----> OUT= 1
DT= 15.0 mi n

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	3.6700	1.4285
	0.1860	0.6703	4.6080	1.6332
	1.0630	0.7754	5.3640	1.8336
	1.9790	1.0010	5.7640	1.8436
	2.7460	1.1950	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (2861)	54.860	5.922	6.00	54.35
OUTFLOW: ID= 1 (5286)	54.860	3.028	6.50	54.33

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 51.14
TIME SHIF T OF PEAK FLOW (mi n) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 1.2876

CALIB STANDHYD (2862)
ID= 1 DT=15.0 mi n

Area (ha)= 205.65
Total Imp(%)= 22.00 Di r. Conn.(%)= 10.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	45.24	160.41
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1170.90	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	106.66	77.81
over (mi n)	15.00	30.00
Storage Coeff. (mi n)=	13.41 (ii)	25.24 (ii)
Uni t Hyd. Tpeak (mi n)=	15.00	30.00
Uni t Hyd. peak (cms)=	0.08	0.04

TOTALS
17.942 (iii)
6.25
46.87
80.80
0.58

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (2862):	205.65	17.942	6.25	46.87
+ ID2= 2 (5286):	54.86	3.028	6.50	54.33
=====				
ID = 3 (9286):	260.51	20.291	6.25	48.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6286):	528.31	6.862	9.00	38.79
+ ID2= 2 (9286):	260.51	20.291	6.25	48.44
=====				
ID = 3 (8386):	788.82	22.089	6.25	41.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD	Area	Curve Number
ID= 1 DT=15.0 min	(ha)	(CN)
	473.90	58.0
	Imp(mm)= 5.00	# of Linear Res. (N)= 1.50
	U. H. Tp(hrs)= 1.66	

Unit Hyd Qpeak (cms)= 4.874

PEAK FLOW (cms)= 3.272 (i)
 TIME TO PEAK (hrs)= 8.500
 RUNOFF VOLUME (mm)= 22.015
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.272

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD	Area	Curve Number
ID= 1 DT=15.0 min	(ha)	(CN)
	330.51	45.0
	Imp(mm)= 5.00	# of Linear Res. (N)= 1.50
	U. H. Tp(hrs)= 1.26	

Unit Hyd Qpeak (cms)= 4.461

PEAK FLOW (cms)= 1.875 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 14.769
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.183

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD	Area	Di r. Conn. (%)
ID= 1 DT=15.0 min	Total	(%)
	0.09	15.00
	Imp(%)= 45.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	0.04	0.05
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	24.49	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 106.66 over (min)= 15.00
 Storage Coeff. (min)= 1.32 (ii)
 Unit Hyd. Tpeak (min)= 15.00
 Unit Hyd. peak (cms)= 0.11

TOTALS
 0.007 (iii)
 6.00
 31.14
 80.80
 0.39

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%

YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 46.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5300)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1	(cms)	(ha. m.)	(cms)	(ha. m.)
DT= 15.0 min	0.0000	0.0000	0.0060	0.0023
	0.0020	0.0012	0.0070	0.0026
	0.0030	0.0016	0.0090	0.0029
	0.0040	0.0019	0.4090	0.0129

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (3001)	0.090	0.007	6.00	31.14
OUTFLOW: ID= 1 (5300)	0.090	0.002	6.75	29.69

PEAK FLOW REDUCTION [Qout/Qin] (%) = 29.78
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha. m.) = 0.0012

CALIB NASHYD	Area	Curve Number
ID= 1 DT=15.0 min	(ha)	(CN)
	258.84	51.8
	Imp(mm)= 5.00	# of Linear Res. (N)= 1.50
	U. H. Tp(hrs)= 1.03	

Unit Hyd Qpeak (cms)= 4.290

PEAK FLOW (cms)= 2.122 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 18.227
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.226

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (3002):	258.84	2.122	7.50	18.23
+ ID2= 2 (5300):	0.09	0.002	6.75	29.69
=====				
ID = 3 (9300):	258.93	2.123	7.50	18.23

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0298):	330.51	1.875	8.00	14.77
+ ID2= 2 (9300):	258.93	2.123	7.50	18.23
=====				
ID = 3 (8395):	589.44	3.975	7.75	16.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD	Area	Curve Number
ID= 1 DT=15.0 min	(ha)	(CN)
	340.83	78.0
	Imp(mm)= 5.00	# of Linear Res. (N)= 1.50
	U. H. Tp(hrs)= 2.21	

Unit Hyd Qpeak (cms)= 2.629

PEAK FLOW (cms)= 3.362 (i)
 TIME TO PEAK (hrs)= 9.250
 RUNOFF VOLUME (mm)= 38.847
 TOTAL RAINFALL (mm)= 80.800

RUNOFF COEFFICIENT = 0.481

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0290)				
ID= 1 DT=15.0 min	Area (ha)	= 269.18	Curve Number (CN)	= 78.0
	Ia (mm)	= 5.00	# of Linear Res. (N)	= 1.50
	U. H. Tp (hrs)	= 1.07		

Unit Hyd Opeak (cms) = 4.279

PEAK FLOW (cms) = 4.704 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 38.611
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.478

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8397)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0288):	340.83	3.362	9.25	38.85
+ ID2= 2 (0290):	269.18	4.704	7.25	38.61
ID = 3 (8397):	610.01	7.793	8.00	38.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0296)				
ID= 1 DT=15.0 min	Area (ha)	= 293.65	Curve Number (CN)	= 76.0
	Ia (mm)	= 5.00	# of Linear Res. (N)	= 1.50
	U. H. Tp (hrs)	= 1.13		

Unit Hyd Opeak (cms) = 4.437

PEAK FLOW (cms) = 4.643 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 36.515
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.452

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0292)				
ID= 1 DT=15.0 min	Area (ha)	= 738.49	Curve Number (CN)	= 68.0
	Ia (mm)	= 5.00	# of Linear Res. (N)	= 1.50
	U. H. Tp (hrs)	= 1.52		

Unit Hyd Opeak (cms) = 8.289

PEAK FLOW (cms) = 7.336 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 29.254
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.362

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0294)				
ID= 1 DT=15.0 min	Area (ha)	= 274.15	Curve Number (CN)	= 57.0
	Ia (mm)	= 5.00	# of Linear Res. (N)	= 1.50
	U. H. Tp (hrs)	= 0.87		

Unit Hyd Opeak (cms) = 5.367

PEAK FLOW (cms) = 3.004 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 21.217
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.263

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8398)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0292):	738.49	7.336	8.00	29.25
+ ID2= 2 (0294):	274.15	3.004	7.00	21.22
ID = 3 (8398):	1012.64	10.132	8.00	27.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6296)		Routing time step (min)' = 15.00
IN= 2 ->> OUT= 1		

<----- DATA FOR SECTION (2961.0) ----->				
Distance	Elevation	Manning		
0.00	243.98	0.0400		
42.59	243.18	0.0400		
85.17	241.81	0.0400		
120.66	240.50	0.0400		
156.15	239.56	0.0400		
198.74	236.15	0.0400		
237.78	234.01	0.0400		
241.33	233.82	0.0400		
248.77	233.12	0.0400 / 0.0400	Main Channel	
249.87	232.32	0.0400	Main Channel	
250.37	231.80	0.0400	Main Channel	
250.87	232.23	0.0400	Main Channel	
251.97	233.10	0.0400 / 0.0400	Main Channel	
255.37	233.22	0.0400		
259.07	233.87	0.0400		
262.62	234.12	0.0400		
266.17	234.23	0.0400		
283.91	234.73	0.0400		
337.15	241.75	0.0400		
351.34	244.00	0.0400		

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.43	232.23	510E+03	0.1	0.53	81.07
0.87	232.67	213E+04	0.7	0.83	51.49
1.30	233.10	501E+04	2.2	1.10	38.75
1.98	233.78	242E+05	12.6	1.34	31.93
2.66	234.46	738E+05	42.1	1.46	29.21
3.34	235.14	177E+06	127.9	1.84	23.13
4.02	235.82	314E+06	281.4	2.29	18.61
4.70	236.50	481E+06	509.4	2.71	15.73
5.38	237.18	672E+06	815.6	3.11	13.73
6.06	237.86	886E+06	1200.7	3.46	12.30
6.74	238.54	112E+07	1669.4	3.80	11.23
7.42	239.22	139E+07	2226.9	4.11	10.38
8.10	239.90	168E+07	2808.4	4.28	9.95
8.78	240.58	202E+07	3470.4	4.40	9.68
9.46	241.26	240E+07	4331.7	4.61	9.24
10.14	241.94	283E+07	5335.2	4.83	8.84
10.82	242.62	330E+07	6482.6	5.03	8.48
11.50	243.30	381E+07	7764.1	5.21	8.18
12.18	243.98	439E+07	9101.2	5.31	8.04

<---- hydrograph ---->					<- pipe / channel ->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8398)	1012.64	10.13	8.00	27.08	1.82	1.27
OUTFLOW: ID= 1 (6296)	1012.64	9.86	8.50	27.08	1.80	1.26

ADD HYD (8396)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0296):	293.65	4.643	7.50	36.52
+ ID2= 2 (6296):	1012.64	9.865	8.50	27.08
ID = 3 (8396):	1306.29	14.310	8.25	29.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8394)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (8396):	1306.29	14.310	8.25	29.20	
+ ID2= 2 (8397):	610.01	7.793	8.00	38.74	
ID = 3 (8394):	1916.30	22.076	8.00	32.24	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8392)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (8394):	1916.30	22.076	8.00	32.24	
+ ID2= 2 (8395):	589.44	3.975	7.75	16.29	
ID = 3 (8392):	2505.74	26.034	8.00	28.49	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6302)
IN= 2--> OUT= 1

Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3021.0) ----->

Distance	Elevation	Manning	
0.00	228.10	0.0400	
18.47	227.12	0.0400	
36.95	226.12	0.0400	
46.18	225.84	0.0400	
55.42	225.58	0.0400	
272.47	222.88	0.0400	
277.09	222.76	0.0400	
281.71	222.58	0.0400	
288.54	222.18	0.0400 / 0.0300	Main Channel
288.64	221.00	0.0300	Main Channel
290.94	221.00	0.0300	Main Channel
291.04	221.00	0.0300	Main Channel
291.54	222.75	0.0300 / 0.0400	Main Channel
300.18	222.83	0.0400	
304.80	223.04	0.0400	
309.42	223.25	0.0400	
318.65	223.69	0.0400	
360.22	225.57	0.0400	
397.16	227.60	0.0400	
457.20	228.35	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(mi n)
0.29	221.29	.228E+04	0.2	0.32	165.63
0.59	221.59	.465E+04	0.7	0.45	116.15
0.88	221.88	.713E+04	1.2	0.54	96.87
1.18	222.18	.971E+04	1.9	0.61	86.20
1.57	222.57	.175E+05	3.3	0.59	88.86
1.97	222.97	.427E+05	6.4	0.47	111.39
2.36	223.36	.115E+06	16.1	0.44	119.14
2.76	223.76	.237E+06	37.1	0.49	106.49
3.15	224.15	.409E+06	72.9	0.56	93.40
3.55	224.55	.631E+06	127.0	0.63	82.76
3.94	224.94	.903E+06	202.4	0.70	74.37
4.34	225.34	.123E+07	301.8	0.77	67.68
4.73	225.73	.160E+07	433.5	0.85	61.37
5.13	226.13	.200E+07	602.2	0.95	55.29
5.52	226.52	.242E+07	804.7	1.05	50.13
5.92	226.92	.286E+07	1034.2	1.14	46.11
6.31	227.31	.332E+07	1290.5	1.22	42.88
6.71	227.71	.380E+07	1560.9	1.29	40.55
7.10	228.10	.432E+07	1835.6	1.34	39.19

	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW: ID= 2 (8392)	2505.74	26.03	8.00	28.49	2.55	0.46
OUTFLOW: ID= 1 (6302)	2505.74	20.56	10.25	28.49	2.44	0.45

ADD HYD	(8390)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (8302):	473.90	3.272	8.50	22.02	
+ ID2= 2 (8302):	2505.74	20.563	10.25	28.49	
ID = 3 (8390):	2979.64	23.591	10.00	27.46	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8348)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1 (8386):	788.82	22.089	6.25	41.98	
+ ID2= 2 (8390):	2979.64	23.591	10.00	27.46	
ID = 3 (8348):	3768.46	31.911	9.75	30.50	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0304)
ID= 1 DT=15.0 min

Area (ha)= 292.37
Curve Number (CN)= 63.0
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 2.78

Unit Hyd Opeak (cms)= 1.793

PEAK FLOW (cms)= 1.571 (i)
TIME TO PEAK (hrs)= 10.250
RUNOFF VOLUME (mm)= 25.482
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.315

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0280)
ID= 1 DT=15.0 min

Area (ha)= 299.86
Curve Number (CN)= 82.0
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 0.85

Unit Hyd Opeak (cms)= 6.009

PEAK FLOW (cms)= 7.151 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 43.109
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.534

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0278)
ID= 1 DT=15.0 min

Area (ha)= 485.49
Curve Number (CN)= 82.0
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 1.52

Unit Hyd Opeak (cms)= 5.453

PEAK FLOW (cms)= 7.318 (i)
TIME TO PEAK (hrs)= 8.000
RUNOFF VOLUME (mm)= 43.435
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.538

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0276)
ID= 1 DT=15.0 min

Area (ha)= 90.89
Curve Number (CN)= 79.0
Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 0.67

Unit Hyd Qpeak (cms) = 2.302

PEAK FLOW (cms) = 2.373 (i)
TIME TO PEAK (hrs) = 6.750
RUNOFF VOLUME (mm) = 39.356
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.487

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0274) Area (ha) = 392.49 Curve Number (CN) = 75.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
U.H. Tp(hrs) = 1.08

Unit Hyd Qpeak (cms) = 6.182

PEAK FLOW (cms) = 6.216 (i)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 35.482
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.439

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8360)
1 + 2 = 3
ID1= 1 (0274): 392.49 6.216 7.50 35.48
+ ID2= 2 (0276): 90.89 2.373 6.75 39.36
=====

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8358)
1 + 2 = 3
ID1= 1 (0278): 485.49 7.318 8.00 43.44
+ ID2= 2 (8360): 483.38 8.455 7.00 36.21
=====

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6280)
IN= 2--> OUT= 1 Routing time step (min) = 15.00

DATA FOR SECTION (2801.0) ----->

Distance	Elevation	Manning
0.00	241.14	0.0500
13.32	240.80	0.0500
39.95	240.07	0.0500
96.54	236.21	0.0500
113.19	234.15	0.0500
123.18	232.35	0.0500
143.15	225.80	0.0500
149.81	225.62	0.0500
153.14	225.40	0.0500
157.30	224.76	0.0500 / 0.0300
159.80	224.26	0.0300
162.30	224.85	0.0300
162.55	224.97	0.0300 / 0.0500
167.80	225.05	0.0500
186.43	229.14	0.0500
213.06	234.75	0.0500
236.37	237.09	0.0500
266.33	237.31	0.0500
292.96	237.83	0.0500
329.58	241.50	0.0500

TRAVEL TIME TABLE
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME

(m)	(m)	(cu. m.)	(cms)	(m/s)	(mi n)
0.50	224.76	.263E+04	0.8	0.65	58.31
1.36	225.62	.278E+05	13.4	1.10	34.54
2.22	226.48	.855E+05	52.6	1.40	27.09
3.09	227.35	.157E+06	120.4	1.74	21.78
3.95	228.21	.242E+06	216.3	2.04	18.64
4.81	229.07	.340E+06	341.8	2.29	16.56
5.67	229.93	.450E+06	498.3	2.52	15.06
6.53	230.79	.574E+06	688.3	2.73	13.90
7.40	231.66	.711E+06	913.8	2.93	12.97
8.26	232.52	.861E+06	1173.4	3.11	12.23
9.12	233.38	.103E+07	1461.3	3.24	11.72
9.98	234.24	.121E+07	1793.3	3.37	11.26
10.85	235.11	.142E+07	2141.0	3.45	11.02
11.71	235.97	.165E+07	2530.4	3.50	10.86
12.57	236.83	.192E+07	2954.0	3.51	10.81
13.43	237.69	.226E+07	3160.3	3.18	11.93
14.29	238.55	.271E+07	3869.4	3.26	11.67
15.16	239.42	.320E+07	4740.3	3.38	11.24
16.02	240.28	.373E+07	5688.4	3.48	10.92

hydrograph
AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
(ha) (cms) (hrs) (mm) (m) (m/s)
INFLOW: ID= 2 (8358) 968.87 15.47 7.50 39.83 1.41 1.11
OUTFLOW: ID= 1 (6280) 968.87 15.03 8.25 39.83 1.40 1.11

ADD HYD (8354)
1 + 2 = 3
ID1= 1 (0280): 299.86 7.151 7.00 43.11
+ ID2= 2 (6280): 968.87 15.029 8.25 39.83
=====

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
NASHYD (0270) Area (ha) = 243.61 Curve Number (CN) = 81.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.30
U.H. Tp(hrs) = 0.87

Unit Hyd Qpeak (cms) = 3.429

PEAK FLOW (cms) = 4.097 (i)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 41.870
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.518

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0268) Area (ha) = 215.76 Curve Number (CN) = 75.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.30
U.H. Tp(hrs) = 0.69

Unit Hyd Qpeak (cms) = 3.807

PEAK FLOW (cms) = 3.621 (i)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 35.162
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.435

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
NASHYD (0266) Area (ha) = 508.09 Curve Number (CN) = 64.0
ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.30
U.H. Tp(hrs) = 1.63

Unit Hyd Qpeak (cms) = 3.786

PEAK FLOW (cms)= 3.140 (i)
 TIME TO PEAK (hrs)= 9.000
 RUNOFF VOLUME (mm)= 26.119
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.323

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (2641) | Area (ha)= 17.58
 ID= 1 DT=15.0 min | Total Imp(%)= 75.00 | Di r. Conn.(%)= 70.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	13.18	4.39
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	342.34	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	106.66	59.50
over (min)=	15.00	30.00
Storage Coeff. (min)=	6.41 (ii)	19.58 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.10	0.05

TOTALS
 3.649 (iii)
 6.00
 64.87
 80.80
 0.80

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	1.1760	0.5589
0.0600	0.3133	1.4770	0.6054
0.3410	0.3763	1.7190	0.6511
0.6340	0.4336	2.1190	0.6611
0.8800	0.4941	0.0000	0.0000

INFLOW : ID= 2 (2641)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5264)	17.580	3.649	6.00	64.87
	17.580	1.290	6.25	64.80

PEAK FLOW REDUCTION [Qout/Qin] (%) = 35.35
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha.m.) = 0.5828

CALIB
 NASHYD (2642) | Area (ha)= 336.37 | Curve Number (CN)= 69.0
 ID= 1 DT=15.0 min | Ia (mm)= 5.00 | # of Linear Res. (N)= 1.30
 U.H. Tp(hrs)= 1.28

Unit Hyd Qpeak (cms)= 3.192

PEAK FLOW (cms)= 2.915 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 30.009
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.371

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9264) | 1 + 2 = 3 | AREA OPEAK TPEAK R. V.

	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (2642):	336.37	2.915	8.00	30.01
+ ID2= 2 (5264):	17.58	1.290	6.25	64.80
ID = 3 (9264):	353.95	3.522	6.50	31.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8380) | 1 + 2 = 3 | AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0266): 508.09 3.140 9.00 26.12
 + ID2= 2 (9264): 353.95 3.522 6.50 31.74
 ID = 3 (8380): 862.04 6.310 8.00 28.43

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6268) | Routing time step (min) = 15.00
 IN= 2 -> OUT= 1

DATA FOR SECTION (2681.0) ----->
 Di stance El evati on Manni ng
 0.00 274.03 0.0500
 10.55 272.73 0.0500
 21.10 271.28 0.0500
 50.12 266.83 0.0500
 87.04 260.36 0.0500
 92.32 260.06 0.0500
 94.95 259.93 0.0500
 101.00 256.87 0.0500
 103.00 256.58 0.0500 / 0.0350 Mai n Channel
 104.00 256.32 0.0350 Mai n Channel
 106.00 256.05 0.0350 Mai n Channel
 108.00 256.25 0.0350 Mai n Channel
 110.78 256.65 0.0350 / 0.0500 Mai n Channel
 113.42 256.81 0.0500
 116.06 257.00 0.0500
 155.62 260.24 0.0500
 187.27 263.35 0.0500
 211.01 267.22 0.0500
 224.20 269.60 0.0500
 261.13 272.68 0.0500

TRAVEL TIME TABLE ----->
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV. TIME
 (m) (m) (cu. m.) (cms) (m/s) (mi n)
 0.53 256.58 .697E+04 2.1 0.97 55.64
 1.38 257.43 .477E+05 25.9 1.76 30.66
 2.22 258.27 .123E+06 84.3 2.22 24.26
 3.07 259.12 .231E+06 185.4 2.60 20.72
 3.92 259.97 .371E+06 335.8 2.92 18.42
 4.77 260.82 .560E+06 547.6 3.15 17.06
 5.61 261.66 .790E+06 845.6 3.46 15.57
 6.46 262.51 .106E+07 1225.0 3.75 14.37
 7.31 263.36 .136E+07 1693.3 4.02 13.37
 8.16 264.21 .169E+07 2283.2 4.35 12.36
 9.00 265.05 .206E+07 2969.1 4.66 11.54
 9.85 265.90 .245E+07 3754.4 4.96 10.86
 10.70 266.75 .286E+07 4643.0 5.24 10.28
 11.55 267.60 .331E+07 5636.1 5.50 9.78
 12.39 268.44 .378E+07 6746.2 5.76 9.34
 13.24 269.29 .428E+07 7972.2 6.01 8.96
 14.09 270.14 .482E+07 9218.9 6.18 8.71
 14.94 270.99 .539E+07 10550.5 6.32 8.52
 15.78 271.83 .601E+07 12024.7 6.46 8.33

hydrograph -----> <- pi pe / channel ->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8380) 862.04 6.31 8.00 28.43 0.68 1.05
 OUTFLOW: ID= 1 (6268) 862.04 6.08 9.25 28.43 0.67 1.04

ADD HYD (8382) | 1 + 2 = 3 | AREA OPEAK TPEAK R. V.

	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0268):	215.76	3.621	7.00	35.16
+ ID2= 2 (6268):	862.04	6.081	9.25	28.43
ID = 3 (8382):	1077.80	9.038	8.25	29.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6270)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2701.0) ----->

Distance	Elevation	Manning	
0.00	245.98	0.0500	
8.14	245.66	0.0500	
16.28	245.16	0.0500	
20.35	244.84	0.0500	
38.66	242.98	0.0500	
48.83	240.65	0.0500	
63.07	235.91	0.0500	
65.11	235.18	0.0500	
68.43	234.34	0.0500 /0.0300	Main Channel
68.68	233.95	0.0300	Main Channel
69.18	233.89	0.0300	Main Channel
69.68	233.95	0.0300	Main Channel
71.21	234.48	0.0300 /0.0500	Main Channel
81.38	236.44	0.0500	
95.63	236.66	0.0500	
120.04	237.00	0.0500	
148.53	241.77	0.0500	
158.70	242.34	0.0500	
187.18	244.03	0.0500	
201.42	244.36	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.45	234.34	.217E+04	0.7	0.99	52.83
0.98	234.87	.966E+04	4.6	1.49	35.20
1.50	235.39	.250E+05	13.7	1.73	30.41
2.03	235.92	.475E+05	29.5	1.95	26.87
2.56	236.45	.772E+05	52.5	2.14	24.50
3.09	236.98	.1142E+06	82.4	1.83	28.73
3.61	237.50	.246E+06	158.1	2.03	25.91
4.14	238.03	.357E+06	262.0	2.31	22.73
4.67	238.56	.477E+06	392.1	2.59	20.27
5.20	239.09	.604E+06	547.8	2.86	18.38
5.72	239.61	.739E+06	729.2	3.11	16.90
6.25	240.14	.882E+06	936.5	3.34	15.70
6.78	240.67	.103E+07	1169.7	3.57	14.72
7.31	241.20	.119E+07	1425.9	3.77	13.94
7.83	241.72	.136E+07	1710.4	3.96	13.26
8.36	242.25	.154E+07	1967.9	4.02	13.07
8.89	242.78	.174E+07	2264.9	4.09	12.83
9.42	243.31	.196E+07	2589.3	4.15	12.65
9.94	243.83	.221E+07	2953.5	4.22	12.46

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8382)	1077.80	9.04	8.25	29.77	1.24	1.60
OUTFLOW: ID= 1 (6270)	1077.80	8.87	9.00	29.77	1.23	1.59

ADD HYD (8384)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0270):	243.61	4.097	7.25	41.87
+ ID2= 2 (6270):	1077.80	8.870	9.00	29.77
ID = 3 (8384):	1321.41	12.553	8.50	32.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0262)	341.31	82.0
ID= 1 DT=15.0 min	5.00	# of Linear Res. (N)= 1.30

U. H. Tp(hrs)= 1.01

Unit Hyd Opeak (cms)	Value
Unit Hyd Opeak (cms)	4.094
PEAK FLOW (cms)	5.213 (i)
TIME TO PEAK (hrs)	7.750
RUNOFF VOLUME (mm)	43.196
TOTAL RAINFALL (mm)	80.800
RUNOFF COEFFICIENT	0.535

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0258)	181.99	79.0
ID= 1 DT=15.0 min	5.00	# of Linear Res. (N)= 1.30
U. H. Tp(hrs)=	1.18	

Unit Hyd Opeak (cms)	Value
Unit Hyd Opeak (cms)	1.881
PEAK FLOW (cms)	2.269 (i)
TIME TO PEAK (hrs)	8.000
RUNOFF VOLUME (mm)	39.727
TOTAL RAINFALL (mm)	80.800
RUNOFF COEFFICIENT	0.492

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0256)	145.79	67.0
ID= 1 DT=15.0 min	5.00	# of Linear Res. (N)= 1.30
U. H. Tp(hrs)=	1.04	

Unit Hyd Opeak (cms)	Value
Unit Hyd Opeak (cms)	1.707
PEAK FLOW (cms)	1.403 (i)
TIME TO PEAK (hrs)	8.000
RUNOFF VOLUME (mm)	28.295
TOTAL RAINFALL (mm)	80.800
RUNOFF COEFFICIENT	0.350

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha)	Curve Number (CN)
NASHYD (0254)	403.00	55.0
ID= 1 DT=15.0 min	5.00	# of Linear Res. (N)= 1.30
U. H. Tp(hrs)=	1.22	

Unit Hyd Opeak (cms)	Value
Unit Hyd Opeak (cms)	4.028
PEAK FLOW (cms)	2.406 (i)
TIME TO PEAK (hrs)	8.000
RUNOFF VOLUME (mm)	20.083
TOTAL RAINFALL (mm)	80.800
RUNOFF COEFFICIENT	0.249

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6256)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2561.0) ----->

Distance	Elevation	Manning	
0.00	276.07	0.0400	
11.68	273.71	0.0400	
23.36	271.35	0.0400	
29.19	270.30	0.0400	
35.03	269.44	0.0400	
55.47	267.90	0.0400	
78.82	266.24	0.0400	
90.50	265.63	0.0400	
102.18	265.40	0.0400	
105.10	264.95	0.0400 /0.0350	Main Channel
108.02	264.39	0.0350	Main Channel
110.94	264.72	0.0350	Main Channel
113.86	265.19	0.0350 /0.0400	Main Channel
116.78	265.49	0.0400	

143.05	268.24	0.0400
172.25	270.53	0.0400
207.28	271.95	0.0400
233.55	273.72	0.0400
256.91	274.98	0.0400
289.02	275.97	0.0400

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	TRAVEL TIME (hrs)	VELOCITY (m/s)	TRAV. TIME (min)
0.56	264.95	.723E+04	1.9	0.90	62.51
1.14	265.53	.307E+05	13.5	1.49	37.92
1.72	266.11	.951E+05	48.6	1.73	32.62
2.30	266.69	.191E+06	117.8	2.09	27.03
2.88	267.27	.314E+06	225.2	2.43	23.24
3.46	267.85	.464E+06	375.5	2.74	20.59
4.04	268.43	.641E+06	573.2	3.03	18.62
4.62	269.01	.846E+06	821.9	3.29	17.15
5.20	269.59	.108E+07	1135.0	3.56	15.87
5.78	270.17	.134E+07	1525.1	3.86	14.64
6.36	270.75	.162E+07	1971.1	4.12	13.71
6.94	271.33	.194E+07	2474.0	4.33	13.04
7.52	271.91	.228E+07	3065.0	4.55	12.42
8.10	272.49	.266E+07	3773.4	4.80	11.75
8.68	273.07	.306E+07	4566.3	5.05	11.17
9.26	273.65	.348E+07	5443.7	5.30	10.66
9.84	274.23	.393E+07	6389.6	5.51	10.25
10.42	274.81	.440E+07	7426.2	5.72	9.88
11.00	275.39	.490E+07	8490.1	5.86	9.63

<---- hydrograph ---->				<-pi pe / channel ->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (0254)	403.00	2.41	8.00	20.08	0.58
OUTFLOW: ID= 1 (6256)	403.00	2.26	9.75	20.08	0.91

ADD HYD (8370)				
ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0256):	145.79	1.403	8.00	28.30
+ ID2= 2 (6256):	403.00	2.263	9.75	20.08
ID = 3 (8370):	548.79	3.538	9.00	22.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0250)				
ID	Area (ha)	(ha)	Curve Number (CN)	# of Linear Res. (N)
ID= 1 DT=15.0 min	192.88	5.00	70.0	1.30
	U. H. Tp(hrs)=	1.22		

Unit Hyd Opeak (cms) = 1.930

PEAK FLOW (cms) = 1.797 (i)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 30.845
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.382

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0246)				
ID	Area (ha)	(ha)	Curve Number (CN)	# of Linear Res. (N)
ID= 1 DT=15.0 min	759.61	5.00	55.0	1.30
	U. H. Tp(hrs)=	1.81		

Unit Hyd Opeak (cms) = 5.121

PEAK FLOW (cms) = 3.349 (i)
TIME TO PEAK (hrs) = 10.000
RUNOFF VOLUME (mm) = 20.153
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.249

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0248)			
ID	Area (ha)	(ha)	Curve Number (CN)
ID= 1 DT=15.0 min	146.04	5.00	64.0
	U. H. Tp(hrs)=	0.78	# of Linear Res. (N) = 1.30

Unit Hyd Opeak (cms) = 2.271

PEAK FLOW (cms) = 1.592 (i)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 25.873
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.320

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8364)				
ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0246):	759.61	3.349	10.00	20.15
+ ID2= 2 (0248):	146.04	1.592	7.25	25.87
ID = 3 (8364):	905.65	4.741	8.25	21.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6250)		Routing time step (min) = 15.00
IN=	OUT=	
2	1	

<----- DATA FOR SECTION (2501.0) ----->			
Distance	Elevation	Manning	
0.00	269.96	0.0500	
8.56	268.55	0.0500	
17.13	266.91	0.0500	
21.41	266.13	0.0500	
40.68	263.15	0.0500	
62.09	260.75	0.0500	
85.64	258.02	0.0500	
88.20	257.69	0.0500	
93.20	257.05	0.0500 / 0.0350	Main Channel
93.45	256.88	0.0350	Main Channel
94.20	256.56	0.0350	Main Channel
94.95	256.83	0.0350	Main Channel
95.20	257.08	0.0350 / 0.0500	Main Channel
100.62	257.45	0.0500	
115.61	258.57	0.0500	
139.16	260.43	0.0500	
152.01	261.95	0.0500	
171.27	264.63	0.0500	
188.40	267.90	0.0500	
211.95	274.18	0.0500	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	TRAVEL TIME (hrs)	VELOCITY (m/s)	TRAV. TIME (min)
0.49	257.05	.205E+04	0.4	0.68	89.65
1.17	257.73	.248E+05	5.7	0.85	72.16
1.85	258.41	.841E+05	25.2	1.10	55.60
2.53	259.09	.180E+06	66.5	1.36	45.14
3.21	259.77	.312E+06	135.7	1.60	38.36
3.89	260.45	.481E+06	238.3	1.82	33.61
4.57	261.13	.681E+06	383.5	2.07	29.60
5.25	261.81	.911E+06	568.9	2.29	26.70
5.93	262.49	.117E+07	800.5	2.51	24.36
6.61	263.17	.146E+07	1079.0	2.72	22.49
7.28	263.84	.177E+07	1413.4	2.94	20.85
7.96	264.52	.210E+07	1797.0	3.14	19.50
8.64	265.20	.246E+07	2241.6	3.35	18.28
9.32	265.88	.283E+07	2739.9	3.55	17.25
10.00	266.56	.323E+07	3295.7	3.75	16.34
10.68	267.24	.364E+07	3908.9	3.94	15.54
11.36	267.92	.408E+07	4578.5	4.12	14.84
12.04	268.60	.453E+07	5320.1	4.32	14.18
12.72	269.28	.499E+07	6109.1	4.50	13.61

<---- hydrograph ---->				<-pi pe / channel ->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)

INFLOW : ID= 2 (8364) 905.65 4.74 8.25 21.08 1.04 0.81
 OUTFLOW: ID= 1 (6250) 905.65 4.43 10.25 21.08 1.00 0.80

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8366):	192.88	1.797	8.00	30.85
+ ID2= 2 (6250):	905.65	4.429	10.25	21.08
ID = 3 (8366):	1098.53	6.064	9.75	22.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8366):	1098.53	6.064	9.75	22.79
+ ID2= 2 (8370):	548.79	3.538	9.00	22.26
ID = 3 (8368):	1647.32	9.574	9.50	22.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2521) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	32.51 75.00	Di r. Conn. (%)=	70.00
	IMPERVIOUS	PERVIOUS (i)			
Surface Area	(ha)=	24.38	8.13		
Dep. Storage	(mm)=	2.00	5.00		
Average Slope	(%)=	0.50	0.50		
Length	(m)=	465.55	40.00		
Mannings n	=	0.013	0.250		
Max. Eff. Inten.	(mm/hr)=	106.66	65.85		
over	(min)	15.00	30.00		
Storage Coeff.	(min)=	7.71 (ii)	20.36 (ii)		
Unit Hyd. Tpeak	(min)=	15.00	30.00		
Unit Hyd. peak	(cms)=	0.10	0.05		
				TOTALS	
PEAK FLOW	(cms)=	6.09	0.75	6.570 (iii)	
TIME TO PEAK	(hrs)=	6.00	6.25	6.00	
RUNOFF VOLUME	(mm)=	78.80	35.64	65.85	
TOTAL RAINFALL	(mm)=	80.80	80.80	80.80	
RUNOFF COEFFICIENT	=	0.98	0.44	0.81	

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5252) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	2.1750	1.0334
	0.1100	0.5793	2.7310	1.1195
	0.6300	0.6958	3.1790	1.2039
	1.1730	0.8017	3.5790	1.2139
	1.6270	0.9137	0.0000	0.0000

INFLOW : ID= 2 (2521)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5252)	32.510	6.570	6.00	65.85
	32.510	2.357	6.25	65.81

PEAK FLOW	REDUCTION [Qout/Qi n] (%)	35.87
TIME SHIFT OF PEAK FLOW	(min)=	15.00
MAXIMUM STORAGE USED	(ha.m.)=	1.0638

CALIB NASHYD (2522) ID= 1 DT=15.0 min	Area Ia	(ha)= (mm)=	287.47 5.00	Curve Number # of LI near Res. (N)=	71.5 1.30
	U. H.	Tp(hrs)=	1.01		

Unit Hyd Opeak (cms) = 3.448
 PEAK FLOW (cms) = 3.209 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 32.098
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.397

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2522):	287.47	3.209	8.00	32.10
+ ID2= 2 (5252):	32.51	2.357	6.25	65.81
ID = 3 (9252):	319.98	5.006	6.50	35.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8368):	1647.32	9.574	9.50	22.62
+ ID2= 2 (9252):	319.98	5.006	6.50	35.52
ID = 3 (8372):	1967.30	12.819	8.75	24.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6258)
 IN= 2--> OUT= 1 Routing time step (min) = 15.00

<----- DATA FOR SECTION (2581.0) ----->		
Di stance	Elevation	Manning
0.00	252.88	0.0500
15.47	251.89	0.0500
46.41	248.45	0.0500
73.48	245.81	0.0500
88.95	243.75	0.0500
112.15	242.00	0.0500
135.35	240.23	0.0500
162.42	239.76	0.0500
170.97	239.52	0.0500 / 0.0350
171.58	239.03	0.0350
174.02	239.03	0.0350
176.46	239.03	0.0350
177.07	239.52	0.0350 / 0.0500
185.63	239.67	0.0500
208.83	239.87	0.0500
239.77	240.14	0.0500
274.57	244.93	0.0500
336.45	249.51	0.0500
363.52	249.77	0.0500
382.85	251.78	0.0500

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.49	239.52	.135E+05	2.5	0.92	91.32
1.14	240.17	.175E+06	25.6	0.73	114.22
1.78	240.81	.532E+06	123.5	1.17	71.85
2.43	241.46	.932E+06	285.3	1.54	54.48
3.07	242.10	.138E+07	506.5	1.85	45.25
3.72	242.75	.186E+07	786.8	2.12	39.42
4.36	243.39	.239E+07	1127.3	2.37	35.33
5.01	244.04	.296E+07	1535.8	2.61	32.12
5.65	244.68	.356E+07	2014.3	2.84	29.48
6.30	245.33	.420E+07	2535.9	3.03	27.60

6.94	245.97	.488E+07	3108.4	3.20	26.16
7.59	246.62	.561E+07	3740.6	3.35	24.98
8.23	247.26	.638E+07	4446.7	3.50	23.93
8.88	247.91	.721E+07	5229.7	3.64	22.98
9.52	248.55	.809E+07	6094.3	3.78	22.12
10.17	249.20	.901E+07	7050.4	3.93	21.31
10.81	249.84	.100E+08	7773.0	3.90	21.46
11.46	250.49	.111E+08	8998.8	4.07	20.56
12.10	251.13	.122E+08	10318.2	4.24	19.75

<---- hydrograph ----> <-pi pe / channel -->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8372)	1967.30	12.82	8.75	24.71	0.78	0.82
OUTFLOW: ID= 1 (6258)	1967.30	11.59	11.00	24.71	0.74	0.83

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0258):	181.99	2.269	8.00	39.73
+ ID2= 2 (6258):	1967.30	11.595	11.00	24.71
ID = 3 (8374):	2149.29	13.417	10.75	25.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2601) ID= 1 DT=15.0 min	Area Total	(ha) Imp(%)	= 56.22 = 65.00	Di r. Conn. (%)	= 48.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	36.54	19.68
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	612.21	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	106.66	118.76
Storage Coeff. (min)	15.00	30.00
Unit Hyd. Tpeak (min)	9.09 (ii)	19.08 (ii)
Unit Hyd. peak (cms)	15.00	30.00
	0.09	0.05
PEAK FLOW (cms)	6.94	3.40
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	78.80	51.58
TOTAL RAINFALL (mm)	80.80	80.80
RUNOFF COEFFICIENT	0.98	0.64

TOTALS
9.236 (iii)
6.00
64.65
80.80
0.80

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 81.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5260) IN= 2----> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	3.7610	1.6799
	0.1900	0.8974	4.7220	1.8499
	1.0890	1.0678	5.4970	2.0146
	2.0290	1.2667	5.8970	2.0246
	2.8140	1.4621	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2601)	56.220	9.236	6.00	64.65
OUTFLOW: ID= 1 (5260)	56.220	3.755	6.50	64.63

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 40.65
TIME SHIF OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha.m.) = 1.6947

CALIB NASHYD (2602) ID= 1 DT=15.0 min	Area Ia	(ha) (mm)	= 420.02 = 5.00	Curve Number # of LI near Res. (N)	(CN) = 83.3 (N) = 1.30
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Unit Hyd Opeak (cms) = 3.946

PEAK FLOW (cms) = 5.527 (i)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 44.978
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.557

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2602):	420.02	5.527	8.00	44.98
+ ID2= 2 (5260):	56.22	3.755	6.50	64.63
ID = 3 (9260):	476.24	8.388	6.50	47.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8374):	2149.29	13.417	10.75	25.99
+ ID2= 2 (9260):	476.24	8.388	6.50	47.30
ID = 3 (8376):	2625.53	18.706	10.00	29.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0262):	341.31	5.213	7.75	43.20
+ ID2= 2 (8376):	2625.53	18.706	10.00	29.85
ID = 3 (8378):	2966.84	23.072	9.50	31.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8378):	2966.84	23.072	9.50	31.39
+ ID2= 2 (8384):	1321.41	12.553	8.50	32.00
ID = 3 (8362):	4288.25	35.379	9.00	31.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6272) IN= 2----> OUT= 1	Routing time step (min)	= 15.00
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<----- DATA FOR SECTION (2721.0) ----->

Di stance	El evati on	Manni ng
0.00	231.01	0.0450
23.01	223.65	0.0450
34.51	222.46	0.0450
51.77	222.11	0.0450
69.02	221.87	0.0450
161.06	221.92	0.0450
166.81	221.91	0.0450
172.56	221.89	0.0450

180.57	221.40	0.0450	/0.0300	Main Channel
181.57	220.60	0.0300		Main Channel
184.07	220.16	0.0300		Main Channel
195.57	221.85	0.0300	/0.0450	Main Channel
201.32	221.82	0.0450		
207.07	221.77	0.0450		
212.83	221.72	0.0450		
253.09	222.52	0.0450		
408.40	222.65	0.0450		
460.17	223.20	0.0450		
517.69	224.84	0.0450		
569.46	232.57	0.0450		

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.41	220.57	.189E+04	0.5	0.49	60.18
0.83	220.99	.697E+04	3.2	0.82	35.79
1.24	221.40	.145E+05	8.8	1.07	27.45
1.84	222.00	.643E+05	29.3	0.81	36.61
2.44	222.60	.282E+06	132.9	0.83	35.41
3.04	223.20	.707E+06	413.7	1.04	28.50
3.64	223.80	.118E+07	876.8	1.31	22.46
4.24	224.40	.168E+07	1494.8	1.57	18.75
4.84	225.00	.221E+07	2264.1	1.82	16.23
5.44	225.60	.274E+07	3196.3	2.06	14.29
6.04	226.20	.328E+07	4258.6	2.30	12.84
6.65	226.81	.383E+07	5444.7	2.52	11.72
7.25	227.41	.438E+07	6749.7	2.73	10.82
7.85	228.01	.494E+07	8169.4	2.93	10.08
8.45	228.61	.551E+07	9700.7	3.12	9.46
9.05	229.21	.608E+07	11340.8	3.30	8.94
9.65	229.81	.666E+07	13087.4	3.48	8.48
10.25	230.41	.724E+07	14938.5	3.65	8.08
10.85	231.01	.783E+07	16892.4	3.82	7.73

		<---- hydrograph ---->				<-pi pe / channel ->	
		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW :	ID= 2 (8362)	4288.25	35.38	9.00	31.58	1.88	0.81
OUTFLOW:	ID= 1 (6272)	4288.25	34.89	9.75	31.58	1.87	0.81

CALIB STANDBYD (2721) ID= 1 DT=15.0 mi n	Area (ha)= 2.21 Total Imp(%)= 49.00	Di r. Conn. (%)= 19.00
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	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	1.08	1.13
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	121.38	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	106.66	108.34
over (mi n)	15.00	15.00
Storage Coeff. (mi n)=	3.44 (ii)	13.80 (ii)
Uni t Hyd. Tpeak (mi n)=	15.00	15.00
Uni t Hyd. peak (cms)=	0.11	0.08

TOTALS	(cms)= 0.12	0.25	0.370 (iii)
PEAK FLOW	(hrs)= 6.00	6.00	6.00
TIME TO PEAK	(mm)= 78.80	44.17	50.75
RUNOFF VOLUME	(mm)= 80.80	80.80	80.80
TOTAL RAINFALL	= 0.98	0.55	0.63

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 73.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5272) IN= 2---> OUT= 1 DT= 15.0 mi n	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
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0.0000	0.0000	0.1480	0.0589
0.0070	0.0284	0.1850	0.0669
0.0430	0.0331	0.2160	0.0746
0.0800	0.0419	0.6160	0.0846
0.1100	0.0497	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2721)	2.210	0.370	6.00	50.75
OUTFLOW: ID= 1 (5272)	2.210	0.116	6.25	50.29

PEAK FLOW REDUCTION [Out/Oi n] (%) = 31.33
TIME SHIF T OF PEAK FLOW (mi n) = 15.00
MAXIMUM STORAGE USED (ha. m.) = 0.0521

CALIB NASHYD (2722) ID= 1 DT=15.0 mi n	Area (ha)= 155.17 Ia (mm)= 5.00 U. H. Tp (hrs)= 1.09	Curve Number (CN)= 75.3 # of Li near Res. (N)= 1.50
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Uni t Hyd Opeak (cms)=	2.433
PEAK FLOW (cms)=	2.471 (i)
TIME TO PEAK (hrs)=	7.500
RUNOFF VOLUME (mm)=	35.785
TOTAL RAINFALL (mm)=	80.800
RUNOFF COEFFICIENT =	0.443

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9272) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2722):	155.17	2.471	7.50	35.78
+ ID2= 2 (5272):	2.21	0.116	6.25	50.29
ID = 3 (9272):	157.38	2.538	7.25	35.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8356) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6272):	4288.25	34.890	9.75	31.58
+ ID2= 2 (9272):	157.38	2.538	7.25	35.99
ID = 3 (8356):	4445.63	36.774	9.50	31.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8352) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8354):	1268.73	21.461	7.75	40.61
+ ID2= 2 (8356):	4445.63	36.774	9.50	31.73
ID = 3 (8352):	5714.36	55.922	8.75	33.70

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6304) IN= 2---> OUT= 1	Routing time step (mi n)'= 15.00
--------------------------------------	----------------------------------

<----- DATA FOR SECTION (3041.0) ----->		
Di stance	El evati on	Manni ng
0.00	232.08	0.0500
19.00	231.87	0.0500
38.00	231.33	0.0500
66.51	230.44	0.0500
104.51	228.25	0.0500
118.76	225.17	0.0500

128.26	219.86	0.0500	
175.77	219.17	0.0500	
185.27	218.90	0.0500 / 0.0300	Main Channel
185.52	218.65	0.0300	Main Channel
190.02	218.37	0.0300	Main Channel
194.52	218.60	0.0300	Main Channel
194.77	218.85	0.0300 / 0.0500	Main Channel
204.27	219.60	0.0500	
299.28	220.91	0.0500	
327.78	222.36	0.0500	
375.28	225.71	0.0500	
403.79	229.37	0.0500	
432.29	230.43	0.0500	
470.29	232.00	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.48	218.85	.121E+05	1.2	0.36	173.94
1.17	219.54	.839E+05	9.8	0.44	142.60
1.86	220.23	.323E+06	41.0	0.47	131.43
2.56	220.93	.708E+06	109.5	0.58	107.67
3.25	221.62	.118E+07	224.3	0.71	87.36
3.94	222.31	.168E+07	376.5	0.84	74.48
4.63	223.00	.222E+07	567.9	0.96	65.26
5.32	223.69	.279E+07	794.7	1.07	58.58
6.02	224.39	.339E+07	1056.2	1.17	53.52
6.71	225.08	.402E+07	1352.4	1.26	49.52
7.40	225.77	.468E+07	1677.3	1.34	46.46
8.09	226.46	.536E+07	2050.0	1.43	43.58
8.79	227.16	.607E+07	2457.9	1.52	41.14
9.48	227.85	.680E+07	2901.0	1.60	39.05
10.17	228.54	.755E+07	3352.7	1.66	37.53
10.86	229.23	.834E+07	3812.5	1.71	36.48
11.55	229.92	.919E+07	4241.8	1.73	36.13
12.25	230.62	.101E+08	4697.0	1.74	35.93
12.94	231.31	.111E+08	5186.3	1.74	35.83

<---- hydrograph ----> <-pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8352)	5714.36	55.92	8.75	33.70	2.02	0.49
OUTFLOW: ID= 1 (6304)	5714.36	45.65	11.25	33.70	1.91	0.48

ADD HYD (8350)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0304):	292.37	1.571	10.25	25.48
+ ID2= 2 (6304):	5714.36	45.646	11.25	33.70
=====				
ID = 3 (8350):	6006.73	47.194	11.25	33.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8346)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8348):	3768.46	31.911	9.75	30.50
+ ID2= 2 (8350):	6006.73	47.194	11.25	33.30
=====				
ID = 3 (8346):	9775.19	77.662	10.50	32.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8344)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0306):	283.97	0.538	13.75	18.51
+ ID2= 2 (8346):	9775.19	77.662	10.50	32.22
=====				
ID = 3 (8344):	10059.16	78.140	10.50	31.83

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5509)
IN= 2---> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	*****	370.0451
41.0590	56.7403	*****	863.4386
48.1390	86.3439	*****	*****
56.6340	*****	*****	*****
67.9600	*****	0.0000	0.0000

INFLOW : ID= 2 (8344)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
*****	*****	78.140	10.50	31.83
OUTFLOW: ID= 1 (5509)	*****	50.474	15.50	31.83

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 64.59
TIME SHIFT OF PEAK FLOW (mi n) = 300.00
MAXIMUM STORAGE USED (ha. m.) = 96.5216

CALIB NASHYD (0332)
ID= 1 DT=15.0 mi n

Area (ha)	OPEAK (cms)	TPEAK (hrs)	Curve Number (CN)	# of Linear Res. (N)
393.44	5.00	2.32	75.0	1.50

Unit Hyd Opeak (cms) = 2.894

PEAK FLOW (cms) = 3.428 (i)
TIME TO PEAK (hrs) = 9.500
RUNOFF VOLUME (mm) = 35.702
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.442

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0330)
ID= 1 DT=15.0 mi n

Area (ha)	OPEAK (cms)	TPEAK (hrs)	Curve Number (CN)	# of Linear Res. (N)
468.30	5.00	1.09	80.0	1.50

Unit Hyd Opeak (cms) = 7.335

PEAK FLOW (cms) = 8.597 (i)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 40.876
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.506

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6332)
IN= 2---> OUT= 1

Routing time step (mi n) = 15.00

<----- DATA FOR SECTION (3321.0) ----->

Distance	Elevation	Manning	
0.00	234.00	0.0380	
25.85	227.20	0.0380	
96.94	226.44	0.0380	
168.03	227.38	0.0380	
219.73	225.62	0.0380	
342.52	221.57	0.0380	
368.37	221.42	0.0380	
374.83	221.23	0.0380	
379.79	220.98	0.0380 / 0.0300	Main Channel
380.29	220.47	0.0300	Main Channel
381.29	220.47	0.0300	Main Channel
382.79	220.47	0.0300	Main Channel
383.29	220.98	0.0300 / 0.0380	Main Channel
394.22	221.22	0.0380	
400.68	221.33	0.0380	
407.14	221.44	0.0380	
491.16	225.70	0.0380	
568.71	227.55	0.0380	
607.49	230.14	0.0380	
639.80	234.08	0.0380	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.51	220.98	.554E+04	0.9	0.56	107.27
1.20	221.67	.885E+05	12.1	0.49	122.32
1.88	222.35	.309E+06	64.7	0.76	79.70
2.57	223.04	.615E+06	165.4	0.97	61.99
3.25	223.72	.101E+07	322.1	1.16	52.06
3.94	224.41	.148E+07	542.9	1.33	45.51
4.62	225.09	.204E+07	835.4	1.48	40.77
5.31	225.78	.269E+07	1202.8	1.62	37.27
5.99	226.46	.344E+07	1618.0	1.70	35.46
6.68	227.15	.447E+07	1932.0	1.56	38.56
7.36	227.83	.580E+07	2763.9	1.72	34.96
8.05	228.52	.718E+07	3861.9	1.95	30.97
8.73	229.20	.859E+07	5115.4	2.16	27.98
9.42	229.89	.100E+08	6519.2	2.35	25.64
10.10	230.57	.115E+08	8092.3	2.55	23.69
10.79	231.26	.130E+08	9821.5	2.73	22.05
11.47	231.94	.145E+08	11690.0	2.92	20.68
12.16	232.63	.160E+08	13694.9	3.09	19.52
12.84	233.31	.176E+08	15833.9	3.26	18.52

		<--- hydrograph --->			<- pi pe / channel -->		
		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW :	ID= 2 (0330)	468.30	8.60	7.25	40.88	0.98	0.51
OUTFLOW:	ID= 1 (6332)	468.30	6.50	9.50	40.88	0.85	0.53

ADD HYD (8322)		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3					
ID1=	1 (0332):	393.44	3.428	9.50	35.70
+ ID2=	2 (6332):	468.30	6.501	9.50	40.88
ID =	3 (8322):	861.74	9.928	9.50	38.51

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0328)		Area (ha)	Curve Number (CN)	# of Linear Res. (N)
ID=	1 DT=15.0 mi n	492.92	77.0	1.50
U. H.	TP(hrs)=	5.00		
		1.91		

Unit Hyd	Opeak (cms)=	4.411
PEAK FLOW	(cms)=	5.314 (i)
TIME TO PEAK	(hrs)=	8.750
RUNOFF VOLUME	(mm)=	37.735
TOTAL RAINFALL	(mm)=	80.800
RUNOFF COEFFICIENT	=	0.467

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0326)		Area (ha)	Curve Number (CN)	# of Linear Res. (N)
ID=	1 DT=15.0 mi n	678.91	80.0	1.50
U. H.	TP(hrs)=	5.00		
		1.95		

Unit Hyd	Opeak (cms)=	5.941
PEAK FLOW	(cms)=	7.858 (i)
TIME TO PEAK	(hrs)=	8.750
RUNOFF VOLUME	(mm)=	41.091
TOTAL RAINFALL	(mm)=	80.800
RUNOFF COEFFICIENT	=	0.509

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3181)		Area (ha)	Dir. Conn. (%)
ID=	1 DT=15.0 mi n	357.30	32.00
Total Imp(%)	=	56.00	

Surface Area	(ha)=	200.09	PERVIOUS (i)	157.21
--------------	-------	--------	--------------	--------

Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	1543.37	40.00
Mannings n	=	0.013	0.250
Max. Eff. Inten.	(mm/hr)=	106.66	103.56
over	(mi n)	15.00	30.00
Storage Coeff.	(mi n)=	15.83 (ii)	26.38 (ii)
Unit Hyd. Tpeak	(mi n)=	15.00	30.00
Unit Hyd. peak	(cms)=	0.07	0.04
PEAK FLOW	(cms)=	24.10	20.29
TIME TO PEAK	(hrs)=	6.00	6.25
RUNOFF VOLUME	(mm)=	78.80	43.39
TOTAL RAINFALL	(mm)=	80.80	80.80
RUNOFF COEFFICIENT	=	0.98	0.54

TOTALS
37.286 (iii)
6.00
54.72
80.80
0.68

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5318)			
IN= 2--> OUT= 1		DT= 15.0 mi n	
OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	22.8580	8.7280
0.8190	2.5190	27.5380	9.6853
4.2860	5.8947	32.5800	10.8345
11.0600	6.7336	32.9800	10.8445
16.3830	7.4416	0.0000	0.0000

		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW :	ID= 2 (3181)	357.300	37.286	6.00	54.72
OUTFLOW:	ID= 1 (5318)	357.300	19.449	6.50	54.72

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 52.16
TIME SHIFT OF PEAK FLOW (mi n) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 8.1721

CALIB STANDHYD (3182)		Area (ha)	Dir. Conn. (%)
ID=	1 DT=15.0 mi n	126.13	12.00
Total Imp(%)	=	21.00	

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)=	26.49	99.64
Dep. Storage	(mm)=	2.00	5.00
Average Slope	(%)=	0.50	0.50
Length	(m)=	916.99	40.00
Mannings n	=	0.013	0.250

Max. Eff. Inten.	(mm/hr)=	106.66	62.18
over	(mi n)	15.00	30.00
Storage Coeff.	(mi n)=	11.58 (ii)	24.52 (ii)
Unit Hyd. Tpeak	(mi n)=	15.00	30.00
Unit Hyd. peak	(cms)=	0.08	0.04

TOTALS
9.365 (iii)
6.25
41.27
80.80
0.51

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8334)

1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3182):	126.13	9.365	6.25	41.27
+ ID2= 2 (5318):	357.30	19.449	6.50	54.72

ID = 3 (8334):	483.43	25.889	6.50	51.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6320)
IN= 2---> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3201.0) ----->

Distance	Elevation	Manning		
0.00	249.00	0.0380		
22.92	245.86	0.0380		
45.83	244.87	0.0380		
91.66	243.11	0.0380		
126.03	239.53	0.0380		
160.41	237.17	0.0380		
166.14	237.06	0.0380		
177.59	237.13	0.0380		
183.32	237.20	0.0380		
189.05	236.70	0.0350	0.0350	Main Channel
193.05	235.89	0.0350	0.0350	Main Channel
197.05	236.64	0.0350	0.0380	Main Channel
200.51	236.74	0.0380		
206.24	237.03	0.0380		
246.34	238.82	0.0380		
263.53	243.87	0.0380		
389.56	247.64	0.0380		
452.58	247.74	0.0380		
498.41	248.60	0.0380		
567.16	249.84	0.0380		

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.37	236.26	.270E+04	0.4	0.58	107.96
0.75	236.64	.108E+05	2.6	0.92	68.01
1.48	237.37	.877E+05	27.4	1.17	53.35
2.20	238.09	.277E+06	122.1	1.65	37.83
2.93	238.82	.540E+06	299.7	2.08	30.01
3.66	239.55	.857E+06	587.8	2.57	24.29
4.39	240.28	.120E+07	977.8	3.04	20.52
5.11	241.00	.158E+07	1458.1	3.46	18.02
5.84	241.73	.198E+07	2028.6	3.84	16.23
6.57	242.46	.240E+07	2690.5	4.19	14.86
7.29	243.18	.285E+07	3431.8	4.50	13.84
8.02	243.91	.335E+07	4149.1	4.64	13.44
8.75	244.64	.393E+07	4746.6	4.51	13.80
9.47	245.36	.463E+07	5618.6	4.53	13.75
10.20	246.09	.545E+07	6781.8	4.66	13.39
10.93	246.82	.635E+07	8274.5	4.87	12.79
11.66	247.55	.733E+07	9988.0	5.10	12.23
12.38	248.27	.853E+07	11422.0	5.01	12.45
13.11	249.00	.989E+07	13749.7	5.20	11.98

<----- hydrograph -----> <----- pipe / channel ----->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8334)	483.43	25.89	6.50	51.21	1.43	1.15
OUTFLOW: ID= 1 (6320)	483.43	15.77	7.25	51.21	1.14	1.03

CALIB NASHYD (3202)
ID= 1 DT=15.0 min
Area (ha)= 84.56
Ia (mm)= 5.00
U.H. Tp (hrs)= 0.70
Curve Number (CN)= 80.9
of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms)= 2.054
PEAK FLOW (cms)= 2.267 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 41.592
TOTAL RAINFALL (mm)= 80.800
RUNOFF COEFFICIENT = 0.515

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANHYD (3201)
ID= 1 DT=15.0 min
Area Total (ha)= 194.18
Imp(%)= 45.00
Dir. Conn. (%)= 16.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 87.38 106.80
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 1137.78 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 106.66 114.75
over (min)= 15.00 30.00
Storage Coeff. (min)= 13.19 (ii) 23.31 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.08 0.04

PEAK FLOW (cms)= 7.07 16.31 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.25 19.423 (iii)
RUNOFF VOLUME (mm)= 78.80 48.51 53.36
TOTAL RAINFALL (mm)= 80.80 80.80 80.80
RUNOFF COEFFICIENT = 0.98 0.60 0.66

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 78.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5320)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	7.4940	4.4477
0.2490	1.8139	9.8910	4.7762
1.2830	3.1517	12.4550	5.0763
3.7490	3.7610	12.8550	5.0863
5.0040	4.0198	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (3201)	194.180	19.423	6.25	53.36
OUTFLOW: ID= 1 (5320)	194.180	9.793	6.75	53.35

PEAK FLOW REDUCTION [Out/Oin] (%) = 50.42
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 4.8101

ADD HYD (9320)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3202):	84.56	2.267	6.75	41.59
+ ID2= 2 (5320):	194.18	9.793	6.75	53.35

ID = 3 (9320):	278.74	12.061	6.75	49.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8332)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6320):	483.43	15.768	7.25	51.21
+ ID2= 2 (9320):	278.74	12.061	6.75	49.78

ID = 3 (8332):	762.17	26.447	7.00	50.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (3142) | Area (ha)= 122.75 Curve Number (CN)= 78.0
 ID= 1 DT=15.0 min | I a (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.53

Unit Hyd Qpeak (cms)= 3.924
 PEAK FLOW (cms)= 3.743 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 37.965
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.470

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3141) | Area (ha)= 42.46
 ID= 1 DT=15.0 min | Total Imp(%)= 70.00 Dir. Conn.(%)= 46.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 29.72 12.74
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 532.04 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 106.66 129.49
 over (min)= 15.00 30.00
 Storage Coeff. (min)= 8.36 (ii) 18.00 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.09 0.05

TOTALS
 PEAK FLOW (cms)= 5.13 2.45 6.767 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 78.80 46.65 61.44
 TOTAL RAINFALL (mm)= 80.80 80.80 80.80
 RUNOFF COEFFICIENT = 0.98 0.58 0.76

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.0 I a = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5314) | IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.8400	1.3064
0.1440	0.7144	3.5660	1.4269
0.8230	0.8540	4.1520	1.5451
1.5320	0.9987	4.5520	1.5551
2.1250	1.1456	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3141): 42.460	6.767	6.00	61.44
OUTFLOW: ID= 1 (5314): 42.460	2.599	6.50	61.41

PEAK FLOW REDUCTION [Qout/Qin] (%) = 38.40
 TIME SHIF OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 1.2622

ADD HYD (9314) | 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3142): 122.75	3.743	6.50	37.96
+ ID2= 2 (5314): 42.46	2.599	6.50	61.41
ID = 3 (9314): 165.21	6.342	6.50	43.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6316) | Routing time step (min)'= 15.00
 IN= 2---> OUT= 1

<----- DATA FOR SECTION (3161.0) ----->
 Distance Elevation Manning
 0.00 248.54 0.0350
 27.93 248.34 0.0350
 41.89 246.61 0.0350
 62.83 243.09 0.0350
 132.65 239.00 0.0350
 188.50 236.74 0.0350
 195.48 236.54 0.0350
 202.46 236.32 0.0350
 205.48 236.14 0.0350 / 0.0310 Main Channel
 205.98 235.61 0.0310 Main Channel
 207.98 235.25 0.0310 Main Channel
 209.98 235.53 0.0310 Main Channel
 210.48 236.00 0.0310 / 0.0350 Main Channel
 216.42 236.73 0.0350
 258.31 239.09 0.0350
 328.12 239.84 0.0350
 439.83 241.47 0.0350
 530.58 242.08 0.0350
 586.43 242.93 0.0350
 691.16 248.00 0.0350

<----- TRAVEL TIME TABLE ----->
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV. TIME
 (m) (m) (cu. m.) (cms) (m/s) (mi n)
 0.37 235.62 215E+04 0.7 0.77 53.88
 0.75 236.00 634E+04 3.5 1.36 30.45
 1.46 236.71 297E+05 19.3 1.62 25.66
 2.16 237.41 103E+06 75.8 1.84 22.60
 2.87 238.12 229E+06 202.8 2.21 18.79
 3.57 238.82 407E+06 423.3 2.59 16.03
 4.28 239.53 654E+06 727.7 2.77 14.97
 4.99 240.24 103E+07 1242.0 3.01 13.76
 5.69 240.94 151E+07 2002.3 3.31 12.54
 6.40 241.65 210E+07 2987.9 3.55 11.70
 7.10 242.35 286E+07 4302.8 3.75 11.07
 7.81 243.06 374E+07 6165.3 4.10 10.11
 8.51 243.76 468E+07 8642.4 4.60 9.03
 9.22 244.47 565E+07 11496.3 5.06 8.20
 9.93 245.18 666E+07 14714.1 5.50 7.54
 10.63 245.88 770E+07 18290.3 5.91 7.02
 11.34 246.59 877E+07 22221.9 6.30 6.58
 12.04 247.29 988E+07 26464.4 6.67 6.22
 12.75 248.00 110E+08 31061.3 7.01 5.91

<---- hydrograph ----> <- pipe / channel ->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (9314) 165.21 6.34 6.50 43.99 0.88 1.40
 OUTFLOW: ID= 1 (6316) 165.21 5.40 7.00 43.99 0.84 1.39

CALIB NASHYD (3162) | Area (ha)= 196.72 Curve Number (CN)= 83.0
 ID= 1 DT=15.0 min | I a (mm)= 5.00 # of Linear Res. (N)= 1.50
 U. H. Tp(hrs)= 0.88

Unit Hyd Qpeak (cms)= 3.838
 PEAK FLOW (cms)= 4.741 (i)
 TIME TO PEAK (hrs)= 7.000
 RUNOFF VOLUME (mm)= 44.391
 TOTAL RAINFALL (mm)= 80.800
 RUNOFF COEFFICIENT = 0.549

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3161) | Area (ha)= 35.62
 ID= 1 DT=15.0 min | Total Imp(%)= 63.00 Dir. Conn.(%)= 44.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 22.44 13.18
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 487.31 40.00

Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr) = 106.66 121.78
 over (min) = 15.00 30.00
 Storage Coeff. (min) = 7.93 (ii) 17.81 (ii)
 Unit Hyd. Tpeak (min) = 15.00 30.00
 Unit Hyd. peak (cms) = 0.10 0.05

TOTALS
 5.799 (iii)
 6.00
 63.77
 80.80
 0.79

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 81.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5316)
 IN= 2---> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.3160	0.9307
0.0930	0.5020	2.9060	1.0021
0.5400	0.6637	3.5360	1.0731
1.1940	0.7495	3.9360	1.0831
1.6920	0.8359	0.0000	0.0000

INFLOW : ID= 2 (3161)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
35.620	35.620	5.799	6.00	63.77
OUTFLOW: ID= 1 (5316)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
35.620	35.620	2.765	6.50	63.73

PEAK FLOW REDUCTION [Qout/Qin] (%) = 47.68
 TIME SHIF T OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 1.0135

ADD HYD (9316)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3162):	196.72	4.741	7.00	44.39
+ ID2= 2 (5316):	35.62	2.765	6.50	63.73
-----	-----	-----	-----	-----
ID = 3 (9316):	232.34	7.252	6.50	47.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8338)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6316):	165.21	5.401	7.00	43.99
+ ID2= 2 (9316):	232.34	7.252	6.50	47.36
-----	-----	-----	-----	-----
ID = 3 (8338):	397.55	12.089	6.75	45.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0312)
 ID= 1 DT=15.0 min

Area (ha) = 359.44 Curve Number (CN) = 80.0
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp (hrs) = 1.36

Unit Hyd Opeak (cms) = 4.529
 PEAK FLOW (cms) = 5.575 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 40.979
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.507

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0308)
 ID= 1 DT=15.0 min

Area (ha) = 529.30 Curve Number (CN) = 62.0
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp (hrs) = 1.62

Unit Hyd Opeak (cms) = 5.575
 PEAK FLOW (cms) = 4.194 (i)
 TIME TO PEAK (hrs) = 8.250
 RUNOFF VOLUME (mm) = 24.698
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.306

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0310)
 ID= 1 DT=15.0 min

Area (ha) = 138.28 Curve Number (CN) = 65.0
 Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U. H. Tp (hrs) = 0.76

Unit Hyd Opeak (cms) = 3.102
 PEAK FLOW (cms) = 2.144 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 26.616
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.329

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8342)
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0308):	529.30	4.194	8.25	24.70
+ ID2= 2 (0310):	138.28	2.144	7.00	26.62
-----	-----	-----	-----	-----
ID = 3 (8342):	667.58	6.044	7.75	25.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6312)
 IN= 2---> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3121.0) ----->

Distance	Elevation	Manning
0.00	265.94	0.0360
38.07	265.43	0.0360
59.82	263.98	0.0360
103.32	254.59	0.0360
157.70	252.16	0.0360
217.52	250.45	0.0360
233.84	247.69	0.0360
234.71	247.27	0.0360
239.71	246.38	0.0360
244.71	246.12	0.0360 / 0.0330
247.71	245.17	0.0330
249.71	245.19	0.0330
251.71	245.64	0.0330 / 0.0360
259.71	246.67	0.0360
282.78	247.12	0.0360
315.41	251.60	0.0360
424.17	256.13	0.0360
478.55	257.04	0.0360
516.62	259.37	0.0360
538.37	266.00	0.0360

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.47	245.64	111E+04	3.1	1.78	6.07
1.54	246.71	103E+05	53.9	3.39	3.19
2.61	247.78	414E+05	281.3	4.40	2.45
3.68	248.85	839E+05	749.1	5.79	1.87

4.74	249.91	.136E+06	1463.2	6.97	1.55
5.81	250.98	.201E+06	2354.8	7.61	1.42
6.88	252.05	.295E+06	3533.0	7.76	1.39
7.95	253.12	.427E+06	5303.9	8.05	1.34
9.02	254.19	.594E+06	7856.5	8.58	1.26
10.09	255.26	.792E+06	11449.2	9.37	1.15
11.15	256.32	.101E+07	15862.4	10.15	1.06
12.22	257.39	.127E+07	21074.5	10.75	1.01
13.29	258.46	.155E+07	28184.1	11.78	0.92
14.36	259.53	.185E+07	36386.2	12.77	0.85
15.43	260.60	.215E+07	46110.6	13.89	0.78
16.50	261.67	.246E+07	56822.5	14.95	0.72
17.56	262.73	.278E+07	68498.1	15.97	0.68
18.63	263.80	.310E+07	81118.8	16.94	0.64
19.70	264.87	.344E+07	93603.2	17.66	0.61

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8342)	667.58	6.04	7.75	25.10	0.53	1.83
OUTFLOW: ID= 1 (6312)	667.58	6.04	8.00	25.10	0.53	1.83

ADD HYD (8340)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0312):	359.44	5.575	8.00	40.98
+ ID2= 2 (6312):	667.58	6.040	8.00	25.10
ID = 3 (8340):	1027.02	11.615	8.00	30.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8336)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (8338):	397.55	12.089	6.75	45.96
+ ID2= 2 (8340):	1027.02	11.615	8.00	30.65
ID = 3 (8336):	1424.57	22.809	7.00	34.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8330)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (8332):	762.17	26.447	7.00	50.69
+ ID2= 2 (8336):	1424.57	22.809	7.00	34.92
ID = 3 (8330):	2186.74	49.256	7.00	40.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6324)	Routing time step (min) = 15.00
IN= 2--> OUT= 1	

Distance	Elevation	Manning	
0.00	234.38	0.0360	
33.57	232.48	0.0360	
67.14	230.14	0.0360	
83.93	228.80	0.0360	
134.29	227.62	0.0360	
209.82	225.10	0.0360	
218.21	224.86	0.0360	
226.61	224.47	0.0360	
234.00	223.86	0.0360 / 0.0300	Main Channel
234.10	223.66	0.0300	Main Channel
235.00	223.66	0.0300	Main Channel
235.90	223.66	0.0300	Main Channel
236.00	223.86	0.0300 / 0.0360	Main Channel
243.39	224.92	0.0360	
251.78	224.89	0.0360	

335.71	225.64	0.0360
562.32	226.53	0.0360
637.85	228.36	0.0360
705.00	229.80	0.0360
830.89	234.00	0.0360

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	TRAVEL TIME FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.20	223.86	145E+04	0.2	0.43	147.27
0.73	224.39	159E+05	2.8	0.68	93.11
1.27	224.93	561E+05	11.0	0.75	85.19
1.80	225.46	217E+06	45.6	0.80	79.20
2.33	225.99	566E+06	134.4	0.91	70.20
2.87	226.53	122E+07	338.1	1.06	59.97
3.40	227.06	206E+07	744.3	1.38	46.16
3.94	227.60	298E+07	1295.9	1.66	38.38
4.47	228.13	399E+07	1979.9	1.90	33.59
5.00	228.66	509E+07	2807.5	2.11	30.21
5.54	229.20	627E+07	3815.1	2.32	27.41
6.07	229.73	753E+07	4984.1	2.53	25.17
6.60	230.26	884E+07	6346.3	2.74	23.20
7.14	230.80	102E+08	7861.2	2.95	21.61
7.67	231.33	116E+08	9526.4	3.14	20.29
8.21	231.87	131E+08	11342.2	3.32	19.18
8.74	232.40	146E+08	13309.6	3.49	18.22
9.27	232.93	161E+08	15414.0	3.66	17.41
9.81	233.47	177E+08	17671.0	3.81	16.70

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8330)	2186.74	49.26	7.00	40.42	1.82	0.81
OUTFLOW: ID= 1 (6324)	2186.74	34.01	8.25	40.42	1.62	0.78

CALIB NASHYD (3242)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 min	602.18	78.8
	U. H. Tp(hrs)= 1.80	# of Linear Res. (N)= 1.50

Unit Hyd Opeak (cms) = 5.708
PEAK FLOW (cms) = 7.177 (i)
TIME TO PEAK (hrs) = 8.500
RUNOFF VOLUME (mm) = 39.693
TOTAL RAINFALL (mm) = 80.800
RUNOFF COEFFICIENT = 0.491

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3241)	Area Total	Imp(%)	Dir. Conn. (%)
ID= 1 DT=15.0 min	13.46	45.00	15.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) = 6.06	6.06	7.40
Dep. Storage (mm) = 2.00	2.00	5.00
Average Slope (%) = 0.50	0.50	0.50
Length (m) = 299.56	299.56	40.00
Mannings n = 0.013	0.013	0.250
Max. Eff. Inten. (mm/hr) = 106.66	106.66	116.68
over (min) = 15.00	15.00	30.00
Storage Coeff. (min) = 5.92 (ii)	5.92	15.98 (ii)
Unit Hyd. Tpeak (min) = 15.00	15.00	30.00
Unit Hyd. peak (cms) = 0.10	0.10	0.05

		TOTALS
PEAK FLOW (cms) = 0.57	1.34	1.474 (iii)
TIME TO PEAK (hrs) = 6.00	6.25	6.00
RUNOFF VOLUME (mm) = 78.80	48.76	53.27
TOTAL RAINFALL (mm) = 80.80	80.80	80.80
RUNOFF COEFFICIENT = 0.98	0.60	0.66

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 78.2 Ia = Dep. Storage (Above)

- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5324)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	0.9000	0.3505
	0.0460	0.1645	1.1310	0.4008
	0.2610	0.1903	1.3160	0.4499
	0.4860	0.2456	1.7160	0.4599
	0.6740	0.2932	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (3241)	13.460	1.474	6.00	53.27
OUTFLOW: ID= 1 (5324)	13.460	0.755	6.50	53.20

PEAK FLOW REDUCTION [Oout/Oi n] (%) = 51.20
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 0.3172

ADD HYD (9324)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3242):	602.18	7.177	8.50	39.69
+ ID2= 2 (5324):	13.46	0.755	6.50	53.20
ID = 3 (9324):	615.64	7.405	8.25	39.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8328)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6324):	2186.74	34.008	8.25	40.42
+ ID2= 2 (9324):	615.64	7.405	8.25	39.99
ID = 3 (8328):	2802.38	41.412	8.25	40.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0322)				
ID= 1 DT=15.0 min				
Area (ha)	Curve Number (CN)	U. H. (mm)	TP (hrs)	# of Linear Res. (N)
513.13	80.0	5.00	1.75	1.50

Unit Hyd Opeak (cms) = 5.020
 PEAK FLOW (cms) = 6.502 (i)
 TIME TO PEAK (hrs) = 8.250
 RUNOFF VOLUME (mm) = 41.043
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.508

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8326)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0322):	513.13	6.502	8.25	41.06
+ ID2= 2 (8328):	2802.38	41.412	8.25	40.32
ID = 3 (8326):	3315.51	47.915	8.25	40.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8324)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0326):	678.91	7.858	8.75	41.09
+ ID2= 2 (8326):	3315.51	47.915	8.25	40.44
ID = 3 (8324):	3994.42	55.719	8.25	40.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6328)		Routing time step (min)'= 15.00		
IN= 2--> OUT= 1				

<----- DATA FOR SECTION (3281.0) ----->				
Distance (m)	Elevation	Manning		
0.00	228.00	0.0380		
18.58	224.97	0.0380		
78.98	223.52	0.0380		
125.44	223.28	0.0380		
171.90	221.71	0.0380		
213.72	219.65	0.0380		
218.36	219.40	0.0380		
223.01	219.19	0.0380		
225.95	219.14	0.0380 / 0.0320	Main Channel	
226.45	218.14	0.0320	Main Channel	
236.95	218.14	0.0320	Main Channel	
245.85	218.14	0.0320	Main Channel	
245.95	219.14	0.0320 / 0.0380	Main Channel	
246.24	219.16	0.0380		
250.88	219.24	0.0380		
255.53	219.39	0.0380		
325.22	221.47	0.0380		
367.03	223.14	0.0380		
404.20	225.17	0.0380		
459.95	228.04	0.0380		

<----- TRAVEL TIME TABLE ----->						
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)	
0.50	218.64	373E+05	3.2	0.33	192.37	
1.00	219.14	752E+05	10.1	0.51	124.71	
1.52	219.66	149E+06	22.2	0.57	112.06	
2.04	220.18	279E+06	43.4	0.59	107.06	
2.56	220.70	464E+06	77.0	0.63	100.52	
3.08	221.22	705E+06	125.7	0.68	93.56	
3.61	221.75	100E+07	192.7	0.73	86.62	
4.13	222.27	135E+07	278.1	0.79	80.92	
4.65	222.79	176E+07	385.9	0.84	75.86	
5.17	223.31	222E+07	514.9	0.89	71.82	
5.69	223.83	280E+07	662.7	0.90	70.40	
6.21	224.35	345E+07	867.7	0.96	66.35	
6.73	224.87	417E+07	1107.0	1.01	62.81	
7.25	225.39	494E+07	1400.0	1.08	58.80	
7.78	225.92	573E+07	1732.2	1.15	55.17	
8.30	226.44	655E+07	2099.9	1.22	52.02	
8.82	226.96	740E+07	2502.8	1.29	49.29	
9.34	227.48	828E+07	2940.9	1.36	46.90	
9.86	228.00	918E+07	3414.3	1.42	44.79	

<---- hydrograph ---->					<- pi pe / channel ->	
INFLOW: ID= 2 (8324)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
3994.42	3994.42	55.72	8.25	40.55	2.23	0.61
OUTFLOW: ID= 1 (6328)	3994.42	44.20	10.25	40.55	2.05	0.59

ADD HYD (8320)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0328):	492.92	5.314	8.75	37.73
+ ID2= 2 (6328):	3994.42	44.199	10.25	40.55
ID = 3 (8320):	4487.34	49.239	10.25	40.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8320):	4487.34	49.239	10.25	40.24
+ ID2= 2 (8322):	861.74	9.928	9.50	38.51
=====				
ID = 3 (8318):	5349.08	59.076	10.00	39.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (5509):	10059.16	50.474	15.50	31.83
+ ID2= 2 (8318):	5349.08	59.076	10.00	39.96
=====				
ID = 3 (8316):	15408.24	101.108	10.75	34.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8314):	620.10	1.336	6.75	21.77
+ ID2= 2 (8316):	15408.24	101.108	10.75	34.66
=====				
ID = 3 (8312):	16028.34	102.161	10.75	34.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8310):	598.91	34.971	6.25	43.48
+ ID2= 2 (8312):	16028.34	102.161	10.75	34.16
=====				
ID = 3 (8308):	16627.25	104.824	10.50	34.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5510) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	*****	74.0090
	66.5450	18.5023	*****	*****
	98.5430	37.0045	*****	*****

INFLOW : ID= 2 (8308)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5510)	*****	104.824	10.50	34.49
	*****	99.025	12.75	34.49

PEAK FLOW REDUCTION [Qout/Qin] (%) = 94.47
 TIME SHIFT OF PEAK FLOW (min) = 135.00
 MAXIMUM STORAGE USED (ha. m.) = 37.5767

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0336):	2785.00	3.992	22.25	32.86
+ ID2= 2 (5510):	16627.25	99.025	12.75	34.49
=====				
ID = 3 (8240):	19412.25	102.393	13.00	34.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (5508):	9524.19	51.747	11.75	22.14
+ ID2= 2 (8240):	19412.25	102.393	13.00	34.26
=====				
ID = 3 (8238):	28936.44	153.484	12.25	30.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (3427) ID= 1 DT=15.0 min	Area (ha)	Imp (%)	Di r. Conn. (%)
	249.40	45.00	33.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	112.23	137.17
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1289.44	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	106.66	61.19
Storage Coeff. (min)	15.00	30.00
Unit Hyd. Tpeak (min)	14.21	27.23 (ii)
Unit Hyd. peak (cms)	15.00	30.00 (ii)
	0.07	0.04

PEAK FLOW (cms)	18.17	10.17	*TOTALS* 24.571 (iii)
TIME TO PEAK (hrs)	6.00	6.25	6.00
RUNOFF VOLUME (mm)	78.80	32.79	47.97
TOTAL RAINFALL (mm)	80.80	80.80	80.80
RUNOFF COEFFICIENT	0.98	0.41	0.59

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 67.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9425) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	12.4700	5.4211
	0.8500	3.0375	16.6800	6.4838
	4.8400	3.5126	20.9500	7.4161
	9.0000	4.5390	24.3900	8.3284

INFLOW : ID= 2 (3427)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (9425)	249.400	24.571	6.00	47.97
	249.400	11.138	6.50	47.97

PEAK FLOW REDUCTION [Qout/Qin] (%) = 45.33
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 5.1501

RESERVOIR (5342) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	3.6600	12.0067
	1.2080	11.1244	4.4200	12.4001
	2.0570	11.3002	5.2300	12.7452
	2.7240	11.3602	0.0000	0.0000

INFLOW : ID= 2 (9425)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5342)	249.400	11.138	6.50	47.97
	249.400	0.805	13.00	47.96

PEAK FLOW REDUCTION [Qout/Qin] (%) = 7.23
 TIME SHIFT OF PEAK FLOW (min) = 390.00
 MAXIMUM STORAGE USED (ha. m.) = 7.4126

CALIB
STANDHYD (3422)
ID= 1 DT=15.0 min

Area (ha)=	755.17	Curve Number (CN)=	54.5
Ia (mm)=	5.00	# of Linear Res. (N)=	1.50
U. H. Tp (hrs)=	2.57		

Unit Hyd Qpeak (cms) = 5.026

PEAK FLOW (cms) = 3.373 (i)
 TIME TO PEAK (hrs) = 10.000
 RUNOFF VOLUME (mm) = 19.910
 TOTAL RAINFALL (mm) = 80.800
 RUNOFF COEFFICIENT = 0.246

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9342) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3422):	755.17	3.373	10.00	19.91
+ ID2= 2 (5342):	249.40	0.805	13.00	47.96
=====				
ID = 3 (9342):	1004.57	4.155	10.25	26.87

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8236) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8238):	28936.44	153.484	12.25	30.27
+ ID2= 2 (9342):	1004.57	4.155	10.25	26.87
=====				
ID = 3 (8236):	29941.01	157.430	12.25	30.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8234) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8232):	285.79	10.880	6.00	39.88
+ ID2= 2 (8236):	29941.01	157.430	12.25	30.16
=====				
ID = 3 (8234):	30226.80	158.183	12.00	30.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (3442)
ID= 1 DT=15.0 min

Area (ha)=	155.15	Dir. Conn. (%) =	29.00
Total Imp (%) =	39.00		

- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (3441)
ID= 1 DT=15.0 min

Area (ha)=	155.15	Dir. Conn. (%) =	29.00
Total Imp (%) =	39.00		

Surface Area (ha)=	60.51	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)=	2.00		94.64
Average Slope (%)=	0.50		5.00
Length (m)=	1017.02		0.50
Mannings n =	0.013		40.00
			0.250

Max. Eff. Inten. (mm/hr)=	106.66	over (min)	23.06
			45.00
Storage Coeff. (min)=	12.33 (ii)		31.56 (ii)
Unit Hyd. Tpeak (min)=	15.00		45.00
Unit Hyd. peak (cms)=	0.08		0.03

PEAK FLOW (cms)=	10.50	3.66	*TOTALS* 12.014 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	78.80	21.68	38.24
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.27	0.47

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 53.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5344)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.6700	6.1252
0.3000	1.6125	3.3150	7.0314
0.8100	3.0036	3.8990	7.9700
1.4170	4.2559	4.2990	7.9800
1.8540	5.0526	0.0000	0.0000

INFLOW : ID= 2 (3441)	155.150	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5344)	155.150		12.014	6.00	38.24
			1.273	8.25	38.23

PEAK FLOW REDUCTION [Qout/Qin] (%) = 10.60
 TIME SHIFT OF PEAK FLOW (min) = 135.00
 MAXIMUM STORAGE USED (ha. m.) = 3.9619

ADD HYD (9344) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3442):	188.84	12.203	6.00	34.85
+ ID2= 2 (5344):	155.15	1.273	8.25	38.23
=====				
ID = 3 (9344):	343.99	12.515	6.00	36.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8230) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8234):	30226.80	158.183	12.00	30.25
+ ID2= 2 (9344):	343.99	12.515	6.00	36.37
=====				
ID = 3 (8230):	30570.79	159.679	12.00	30.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (3442)
ID= 1 DT=15.0 min

Area (ha)=	188.84	Dir. Conn. (%) =	24.00
Total Imp (%) =	32.00		

Surface Area (ha)=	60.43	IMPERVIOUS	PERVIOUS (i)
Dep. Storage (mm)=	2.00		128.41
Average Slope (%)=	0.50		5.00
Length (m)=	1122.02		0.50
Mannings n =	0.013		40.00
			0.250

Max. Eff. Inten. (mm/hr)=	106.66	over (min)	21.37
			45.00
Storage Coeff. (min)=	13.08 (ii)		32.91 (ii)
Unit Hyd. Tpeak (min)=	15.00		45.00
Unit Hyd. peak (cms)=	0.08		0.03

PEAK FLOW (cms)=	10.34	4.52	*TOTALS* 12.203 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	78.80	20.97	34.85
TOTAL RAINFALL (mm)=	80.80	80.80	80.80
RUNOFF COEFFICIENT =	0.98	0.26	0.43

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 53.0 Ia = Dep. Storage (Above)

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8228):	1952.06	14.006	8.50	19.56
+ ID2= 2 (8230):	30570.79	159.679	12.00	30.32
=====				
ID = 3 (8228):	32522.85	168.993	12.00	29.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0356):	1015.22	5.609	8.00	15.27
+ ID2= 2 (8228):	32522.85	168.993	12.00	29.68
=====				
ID = 3 (8190):	33538.07	172.690	12.00	29.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

** SIMULATION NUMBER: 6 **

READ STORM
Ptotal = 88.50 mm

Filename: C:\Users\amanluc\AppData\Local\Temp\ab70bd54-a452-450c-b03c-83cdc4826a98\fa8df533
Comments: 100-Year 12-Hour SCS II Design Storm

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.25	2.21	3.25	3.54	6.25	15.93	9.25	3.10
0.50	2.21	3.50	3.54	6.50	15.93	9.50	3.10
0.75	2.21	3.75	3.54	6.75	7.08	9.75	3.10
1.00	2.21	4.00	3.54	7.00	7.08	10.00	3.10
1.25	2.21	4.25	5.31	7.25	5.31	10.25	1.77
1.50	2.21	4.50	5.31	7.50	5.31	10.50	1.77
1.75	2.21	4.75	7.08	7.75	5.31	10.75	1.77
2.00	2.21	5.00	7.08	8.00	5.31	11.00	1.77
2.25	2.65	5.25	10.62	8.25	3.10	11.25	1.77
2.50	2.65	5.50	10.62	8.50	3.10	11.50	1.77
2.75	2.65	5.75	42.48	8.75	3.10	11.75	1.77
3.00	2.65	6.00	116.82	9.00	3.10	12.00	1.77

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
(0356)	1015.22	5.609	8.00	15.27
=====				
	381.43	5.00	0.82	54.0

Unit Hyd Qpeak (cms) = 12.651

PEAK FLOW (cms) = 6.685 (i)
TIME TO PEAK (hrs) = 8.000
RUNOFF VOLUME (mm) = 18.151
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.205

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
(0352)	381.43	5.00	0.82	54.0
=====				
	381.43	5.00	0.82	54.0

Unit Hyd Qpeak (cms) = 7.980

PEAK FLOW (cms) = 4.770 (i)
TIME TO PEAK (hrs) = 7.000
RUNOFF VOLUME (mm) = 22.930
TOTAL RAINFALL (mm) = 88.500

RUNOFF COEFFICIENT = 0.259

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
(0350)	366.84	5.00	1.07	48.0
=====				
	366.84	5.00	1.07	48.0

Unit Hyd Qpeak (cms) = 5.831

PEAK FLOW (cms) = 3.076 (i)
TIME TO PEAK (hrs) = 7.500
RUNOFF VOLUME (mm) = 19.261
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.218

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
(0348)	590.18	5.00	1.91	48.0
=====				
	590.18	5.00	1.91	48.0

Unit Hyd Qpeak (cms) = 5.267

PEAK FLOW (cms) = 3.191 (i)
TIME TO PEAK (hrs) = 9.000
RUNOFF VOLUME (mm) = 19.364
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.219

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6350)
IN= 2--> OUT= 1 Routing time step (min) = 15.00

Distance	Elevation	Manning
0.00	287.00	0.0500
23.66	283.72	0.0500
70.98	280.94	0.0500
112.38	280.23	0.0500
171.52	276.80	0.0500
260.24	274.46	0.0500
266.16	274.26	0.0500
272.07	274.12	0.0500
276.49	274.12	0.0500 / 0.0300
276.99	273.82	0.0300
277.99	273.82	0.0300
279.49	273.82	0.0300
280.49	274.27	0.0300 / 0.0500
283.90	274.27	0.0500
289.81	274.57	0.0500
325.30	275.75	0.0500
396.28	278.98	0.0500
449.51	280.97	0.0500
496.83	283.90	0.0500
585.54	287.92	0.0500

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.30	274.12	.273E+04	0.9	0.99	49.81
0.98	274.80	.583E+05	20.8	1.05	46.71
1.66	275.48	.203E+06	95.2	1.38	35.53
2.33	276.15	.438E+06	256.0	1.73	28.50
3.01	276.83	.755E+06	524.8	2.05	23.97
3.69	277.51	.114E+07	935.8	2.43	20.26
4.37	278.19	.157E+07	1472.0	2.76	17.82
5.05	278.87	.206E+07	2140.7	3.06	16.06
5.72	279.54	.261E+07	2938.6	3.33	14.79
6.40	280.22	.321E+07	3887.6	3.57	13.77
7.08	280.90	.390E+07	4795.4	3.63	13.56
7.76	281.58	.468E+07	6196.2	3.91	12.58
8.43	282.25	.550E+07	7803.9	4.19	11.74
9.11	282.93	.637E+07	9601.9	4.45	11.05
9.79	283.61	.728E+07	11594.8	4.70	10.46

10. 47 284. 29 . 823E+07 13845. 3 4. 96 9. 91
 11. 15 284. 97 . 922E+07 16293. 4 5. 21 9. 43
 11. 82 285. 64 . 103E+08 18952. 0 5. 45 9. 02
 12. 50 286. 32 . 113E+08 21824. 3 5. 69 8. 65

<---- hydrograph ----> <-pi pe / channel ->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (0348) 590. 18 3. 19 9. 00 19. 36 0. 38 0. 99
 OUTFLOW: ID= 1 (6350) 590. 18 3. 11 10. 25 19. 36 0. 37 0. 99

ADD HYD (8220)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0350): 366. 84 3. 076 7. 50 19. 26
 + ID2= 2 (6350): 590. 18 3. 111 10. 25 19. 36
 ID= 3 (8220): 957. 02 5. 697 8. 75 19. 32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (3461)
 ID= 1 DT=15. 0 mi n
 Area (ha)= 33. 62
 Total Imp(%)= 61. 00 Di r. Conn. (%)= 38. 00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 20. 51 13. 11
 Dep. Storage (mm)= 2. 00 5. 00
 Average Slope (%)= 0. 50 0. 50
 Length (m)= 473. 43 40. 00
 Mannings n = 0. 013 0. 250

Max. Eff. Inten. (mm/hr)= 116. 82 115. 45
 over (mi n) 15. 00 30. 00
 Storage Coeff. (mi n)= 7. 51 (ii) 17. 61 (ii)
 Uni t Hyd. Tpeak (mi n)= 15. 00 30. 00
 Uni t Hyd. Tpeak (cms)= 0. 10 0. 05

TOTALS
 PEAK FLOW (cms)= 3. 77 2. 26 5. 262 (iii)
 TIME TO PEAK (hrs)= 6. 00 6. 25 6. 00
 RUNOFF VOLUME (mm)= 86. 50 47. 16 62. 11
 TOTAL RAINFALL (mm)= 88. 50 88. 50 88. 50
 RUNOFF COEFFICIENT = 0. 98 0. 53 0. 70

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 69. 8 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5346)
 IN= 2--> OUT= 1
 DT= 15. 0 mi n
 OUTFLOW STORAGE OUTFLOW STORAGE
 (cms) (ha. m.) (cms) (ha. m.)
 0. 0000 0. 0000 2. 0090 0. 6158
 0. 2450 0. 2524 2. 6260 0. 6610
 0. 6230 0. 3644 3. 2370 0. 7019
 0. 9920 0. 4690 3. 6370 0. 7119
 1. 2510 0. 5428 0. 0000 0. 0000

AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (3461) 33. 620 5. 262 6. 00 62. 11
 OUTFLOW: ID= 1 (5346) 33. 620 3. 278 6. 25 62. 10

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 62. 28
 TIME SHIF T OF PEAK FLOW (mi n) = 15. 00
 MAXIMUM STORAGE USED (ha. m.) = 0. 7037

CALIB NASHYD (3462)
 ID= 1 DT=15. 0 mi n
 Area (ha)= 317. 31 Curve Number (CN)= 71. 0
 Ia (mm)= 5. 00 # of Li near Res. (N)= 1. 50

U. H. Tp(hrs)= 0. 96

Uni t Hyd Opeak (cms)= 5. 625

PEAK FLOW (cms)= 5. 721 (i)
 TIME TO PEAK (hrs)= 7. 250
 RUNOFF VOLUME (mm)= 36. 834
 TOTAL RAINFALL (mm)= 88. 500
 RUNOFF COEFFICIENT = 0. 416

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9346)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (3462): 317. 31 5. 721 7. 25 36. 83
 + ID2= 2 (5346): 33. 62 3. 278 6. 25 62. 10
 ID= 3 (9346): 350. 93 7. 961 6. 50 39. 25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8222)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8220): 957. 02 5. 697 8. 75 19. 32
 + ID2= 2 (9346): 350. 93 7. 961 6. 50 39. 25
 ID= 3 (8222): 1307. 95 11. 838 7. 50 24. 67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6352)
 IN= 2--> OUT= 1 Routing time step (mi n)' = 15. 00

DATA FOR SECTION (3521. 0) ----->
 Di stance El evation Manning
 0. 00 257. 95 0. 0500
 7. 83 257. 34 0. 0500
 15. 66 256. 19 0. 0500
 21. 53 254. 01 0. 0500
 41. 11 244. 06 0. 0500
 76. 35 241. 38 0. 0500
 111. 58 239. 74 0. 0500
 113. 54 239. 61 0. 0500
 113. 75 239. 41 0. 0500 / 0. 0300 Mai n Channel
 113. 85 238. 81 0. 0300 Mai n Channel
 115. 50 238. 81 0. 0300 Mai n Channel
 116. 15 238. 81 0. 0300 Mai n Channel
 116. 25 239. 41 0. 0300 Mai n Channel
 117. 46 239. 52 0. 0500
 119. 41 239. 72 0. 0500
 121. 37 240. 04 0. 0500
 131. 16 241. 84 0. 0500
 156. 61 247. 03 0. 0500
 176. 19 251. 46 0. 0500
 193. 80 258. 79 0. 0500

TRAVEL TIME TABLE
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV. TIME
 (m) (m) (cu. m.) (cms) (m/s) (mi n)
 0. 60 239. 41 . 224E+04 2. 2 1. 52 17. 12
 1. 58 240. 39 . 216E+05 22. 3 1. 61 16. 16
 2. 55 241. 36 . 804E+05 101. 0 1. 96 13. 27
 3. 53 242. 34 . 173E+06 275. 6 2. 48 10. 46
 4. 50 243. 31 . 293E+06 553. 9 2. 95 8. 80
 5. 48 244. 29 . 438E+06 961. 8 3. 42 7. 60
 6. 45 245. 26 . 599E+06 1534. 4 3. 99 6. 51
 7. 43 246. 24 . 770E+06 2228. 4 4. 51 5. 76
 8. 41 247. 22 . 951E+06 3043. 6 4. 98 5. 21
 9. 38 248. 19 . 114E+07 3982. 1 5. 43 4. 78
 10. 36 249. 17 . 134E+07 5040. 1 5. 85 4. 44
 11. 33 250. 14 . 155E+07 6218. 5 6. 24 4. 16
 12. 31 251. 12 . 177E+07 7518. 6 6. 61 3. 93
 13. 29 252. 10 . 200E+07 8969. 0 6. 98 3. 72
 14. 26 253. 07 . 224E+07 10554. 1 7. 35 3. 53

15.24 254.05 .248E+07 12257.1 7.71 3.37
 16.21 255.02 .272E+07 14045.1 8.03 3.23
 17.19 256.00 .298E+07 15954.8 8.34 3.11
 18.16 256.97 .325E+07 17780.4 8.53 3.04

<----- hydrograph -----> <-pi pe / channel ->
 AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8222) 1307.95 11.84 7.50 24.67 1.07 1.56
 OUTFLOW: ID= 1 (6352) 1307.95 11.78 7.75 24.67 1.06 1.56

ADD HYD (8224)
 1 + 2 = 3

AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0352): 381.43 4.770 7.00 22.93
 + ID2= 2 (6352): 1307.95 11.781 7.75 24.67
 ID = 3 (8224): 1689.38 16.320 7.50 24.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6354)
 IN= 2--> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3541.0) ----->

Distance	Elevation	Manning	
0.00	253.92	0.0500	
7.95	251.83	0.0500	
15.89	249.97	0.0500	
19.87	249.05	0.0500	
47.68	242.00	0.0500	
67.55	237.10	0.0500	
83.44	231.11	0.0500	
85.43	230.75	0.0500	
85.66	230.61	0.0500 / 0.0300	Main Channel
85.76	230.01	0.0300	Main Channel
87.41	230.01	0.0300	Main Channel
88.06	230.01	0.0300	Main Channel
88.16	230.61	0.0300 / 0.0500	Main Channel
89.40	230.72	0.0500	
91.39	230.88	0.0500	
93.37	231.03	0.0500	
133.11	233.96	0.0500	
150.99	235.39	0.0500	
172.84	239.03	0.0500	
196.68	250.00	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.60	230.61	.451E+04	1.5	1.06	49.40
1.62	231.63	.394E+05	14.6	1.16	45.13
2.64	232.65	.128E+06	58.8	1.44	36.30
3.66	233.67	.269E+06	149.7	1.74	30.00
4.68	234.69	.463E+06	301.5	2.04	25.58
5.70	235.71	.705E+06	530.5	2.36	22.14
6.72	236.73	.980E+06	848.4	2.71	19.25
7.74	237.75	.128E+07	1237.9	3.02	17.29
8.76	238.77	.162E+07	1706.7	3.30	15.83
9.78	239.79	.199E+07	2291.9	3.61	14.45
10.81	240.82	.237E+07	2973.7	3.92	13.30
11.83	241.84	.278E+07	3742.2	4.21	12.38
12.85	242.86	.321E+07	4599.1	4.49	11.62
13.87	243.88	.365E+07	5544.6	4.75	10.98
14.89	244.90	.412E+07	6579.7	5.00	10.44
15.91	245.92	.461E+07	7705.9	5.23	9.97
16.93	246.94	.512E+07	8924.6	5.46	9.55
17.95	247.96	.564E+07	10237.5	5.68	9.19
18.97	248.98	.619E+07	11646.2	5.89	8.86

<----- hydrograph -----> <-pi pe / channel ->
 AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8224) 1689.38 16.32 7.50 24.28 1.66 1.16
 OUTFLOW: ID= 1 (6354) 1689.38 15.22 8.50 24.28 1.63 1.16

CALIB STANHYD (3541)
 ID= 1 DT=15.0 min
 Area (ha)= 8.01
 Total Imp(%)= 75.00
 Dir. Conn. (%)= 70.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 6.01 2.00
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 231.08 40.00
 Mannings n = 0.013 0.250
 Max. Eff. Inten. (mm/hr)= 116.82 16.13
 over (min) 15.00 30.00
 Storage Coeff. (min)= 4.89 (ii) 27.08 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.11 0.04

TOTALS
 PEAK FLOW (cms)= 1.76 0.06 1.804 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 86.50 15.11 65.08
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.17 0.74

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 35.4 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5354)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.5360	0.2546
0.0270	0.1427	0.6730	0.2758
0.1550	0.1714	0.7830	0.2966
0.2890	0.1975	1.1830	0.3066
0.4010	0.2251	0.0000	0.0000

AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (3541) 8.010 1.804 6.00 65.08
 OUTFLOW: ID= 1 (5354) 8.010 0.656 6.25 64.92

PEAK FLOW REDUCTION [Oout/Oi n] (%) = 36.39
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 0.2856

CALIB NASHYD (3542)
 ID= 1 DT=15.0 min

Area (ha)= 254.67
 Curve Number (CN)= 36.7
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 1.37

Unit Hyd Opeak (cms)= 3.174
 PEAK FLOW (cms)= 1.219 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 13.282
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.150

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9354)
 1 + 2 = 3

AREA OPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (3542): 254.67 1.219 8.00 13.28
 + ID2= 2 (5354): 8.01 0.656 6.25 64.92
 ID = 3 (9354): 262.68 1.455 6.50 14.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6354):	1689.38	15.220	8.50	24.28
+ ID2= 2 (9354):	262.68	1.455	6.50	14.86
=====				
ID = 3 (8226):	1952.06	16.522	8.50	23.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (3402) ID= 1 DT=15.0 min	Area (ha)	Imp(%)	Di r. Conn. (%)
	138.83	46.00	28.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	63.86	74.97
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	962.05	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	116.82	48.50
over (min)	15.00	30.00
Storage Coeff. (min)=	11.50 (ii)	25.78 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04

TOTALS		
PEAK FLOW (cms)=	10.18	4.51
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	86.50	24.32
TOTAL RAINFALL (mm)=	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.27

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 47.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3401) ID= 1 DT=15.0 min	Area (ha)	Imp(%)	Di r. Conn. (%)
	146.96	58.00	35.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	85.24	61.72
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	989.81	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	116.82	63.32
over (min)	15.00	30.00
Storage Coeff. (min)=	11.69 (ii)	24.54 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.08	0.04

TOTALS		
PEAK FLOW (cms)=	13.39	4.98
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	86.50	27.27
TOTAL RAINFALL (mm)=	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.31

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 47.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5340) IN= 2--> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)

0.0000	0.0000	11.4780	4.3560
0.3860	2.0347	14.9940	4.9991
3.2310	2.5818	18.1680	5.6243
5.1770	3.1417	18.5680	5.6343
7.7590	3.6318	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (3401)	146.960	16.507	6.00	48.00
OUTFLOW: ID= 1 (5340)	146.960	6.304	6.50	47.99

PEAK FLOW REDUCTION [Out/Oi n] (%) = 38.19
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha.m.) = 3.3732

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3402):	138.83	12.974	6.00	41.73
+ ID2= 2 (5340):	146.96	6.304	6.50	47.99
=====				
ID = 3 (8232):	285.79	14.039	6.00	44.95

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0410) ID= 1 DT=15.0 min	Area (ha)	Imp(%)	Di r. Conn. (%)	Curve Number (CN)	# of Linear Res. (N)
	572.01	5.00	1.46	48.0	1.50

Unit Hyd Opeak (cms) = 6.698

PEAK FLOW (cms)=	3.821 (i)
TIME TO PEAK (hrs)=	8.000
RUNOFF VOLUME (mm)=	19.326
TOTAL RAINFALL (mm)=	88.500
RUNOFF COEFFICIENT =	0.218

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0408) ID= 1 DT=15.0 min	Area (ha)	Imp(%)	Di r. Conn. (%)	Curve Number (CN)	# of Linear Res. (N)
	231.62	5.00	0.64	58.0	1.50

Unit Hyd Opeak (cms) = 6.198

PEAK FLOW (cms)=	3.942 (i)
TIME TO PEAK (hrs)=	6.750
RUNOFF VOLUME (mm)=	25.554
TOTAL RAINFALL (mm)=	88.500
RUNOFF COEFFICIENT =	0.289

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0376) ID= 1 DT=15.0 min	Area (ha)	Imp(%)	Di r. Conn. (%)	Curve Number (CN)	# of Linear Res. (N)
	463.85	5.00	1.07	74.0	1.50

Unit Hyd Opeak (cms) = 7.380

PEAK FLOW (cms)=	8.365 (i)
TIME TO PEAK (hrs)=	7.250
RUNOFF VOLUME (mm)=	39.991
TOTAL RAINFALL (mm)=	88.500
RUNOFF COEFFICIENT =	0.452

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0374) ID= 1 DT=15.0 min	Area (ha)	Imp(%)	Di r. Conn. (%)	Curve Number (CN)	# of Linear Res. (N)
	545.70	5.00		61.0	1.50

U. H. Tp(hrs)= 1.51

Unit Hyd Qpeak (cms)= 6.158

PEAK FLOW (cms)= 5.223 (i)
TIME TO PEAK (hrs)= 8.250
RUNOFF VOLUME (mm)= 28.198
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.319

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0372) Area (ha)= 110.42 Curve Number (CN)= 37.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 0.96

Unit Hyd Qpeak (cms)= 1.954

PEAK FLOW (cms)= 0.691 (i)
TIME TO PEAK (hrs)= 7.250
RUNOFF VOLUME (mm)= 13.367
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.151

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0370) Area (ha)= 191.85 Curve Number (CN)= 63.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 0.67

Unit Hyd Qpeak (cms)= 4.860

PEAK FLOW (cms)= 3.631 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 29.417
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.332

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0366) Area (ha)= 462.62 Curve Number (CN)= 62.0
ID= 1 DT=15.0 min Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
U. H. Tp(hrs)= 1.06

Unit Hyd Qpeak (cms)= 7.451

PEAK FLOW (cms)= 5.968 (i)
TIME TO PEAK (hrs)= 7.500
RUNOFF VOLUME (mm)= 28.878
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.326

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3641) Area (ha)= 7.85 Curve Number (CN)= 62.0
ID= 1 DT=15.0 min Total Imp(%)= 45.00 Dir. Conn.(%)= 16.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 3.53 4.32
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 228.76 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.82 53.95
over (min) 15.00 30.00
Storage Coeff. (min)= 4.86 (ii) 18.55 (ii)
Unit Hyd. Tpeak (min)= 15.00 30.00
Unit Hyd. peak (cms)= 0.11 0.05

PEAK FLOW (cms)= 0.40 0.34 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.25 0.607 (iii)
6.00

RUNOFF VOLUME (mm)= 86.50 23.69 33.74
TOTAL RAINFALL (mm)= 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.27 0.38

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 42.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5364)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.5250	0.2044
0.0270	0.0959	0.6590	0.2337
0.1520	0.1110	0.7670	0.2624
0.2830	0.1432	1.1670	0.2724
0.3930	0.1710	0.0000	0.0000

AREA OPEAK TPEAK R. V.
(ha) (cms) (hrs) (mm)
INFLOW : ID= 2 (3641) 7.850 0.607 6.00 33.74
OUTFLOW: ID= 1 (5364) 7.850 0.211 6.75 33.63

PEAK FLOW REDUCTION [Out/Oin] (%) = 34.70
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha. m.) = 0.1272

CALIB STANDHYD (3642) Area (ha)= 147.42
ID= 1 DT=15.0 min Total Imp(%)= 21.00 Dir. Conn.(%)= 7.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 30.96 116.46
Dep. Storage (mm)= 2.00 5.00
Average Slope (%)= 0.50 0.50
Length (m)= 991.36 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.82 20.26
over (min) 15.00 45.00
Storage Coeff. (min)= 11.71 (ii) 31.96 (ii)
Unit Hyd. Tpeak (min)= 15.00 45.00
Unit Hyd. peak (cms)= 0.08 0.03

PEAK FLOW (cms)= 2.69 3.95 *TOTALS*
TIME TO PEAK (hrs)= 6.00 6.50 4.579 (iii)
RUNOFF VOLUME (mm)= 86.50 19.11 23.83
TOTAL RAINFALL (mm)= 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.22 0.27

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 42.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9364)
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3642):	147.42	4.579	6.50	23.83
+ ID2= 2 (5364):	7.85	0.211	6.75	33.63
ID = 3 (9364):	155.27	4.785	6.50	24.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8302)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0366):	462.62	5.968	7.50	28.88
+ ID2= 2 (9364):	155.27	4.785	6.50	24.32
ID = 3 (8302):	617.89	10.029	6.50	27.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0358)	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	Curve Number (CN)
ID= 1 DT=15.0 min	429.87	5.00	1.03	35.0	1.50

Unit Hyd Opeak (cms) = 7.091

PEAK FLOW (cms)	2.367 (i)
TIME TO PEAK (hrs)	7.500
RUNOFF VOLUME (mm)	12.436
TOTAL RAINFALL (mm)	88.500
RUNOFF COEFFICIENT	0.141

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0360)	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	Curve Number (CN)
ID= 1 DT=15.0 min	138.37	5.00	0.60	46.0	1.50

Unit Hyd Opeak (cms) = 3.957

PEAK FLOW (cms)	1.703 (i)
TIME TO PEAK (hrs)	6.750
RUNOFF VOLUME (mm)	17.868
TOTAL RAINFALL (mm)	88.500
RUNOFF COEFFICIENT	0.202

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8306)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0358):	429.87	2.367	7.50	12.44
+ ID2= 2 (0360):	138.37	1.703	6.75	17.87
ID = 3 (8306):	568.24	3.976	7.00	13.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6362)	Routing time step (min)
IN= 2--> OUT= 1	15.00

<----- DATA FOR SECTION (3621.0) ----->

Distance	Elevation	Manning	
0.00	261.46	0.0550	
27.86	254.23	0.0550	
51.07	251.96	0.0550	
74.29	250.77	0.0550	
97.50	249.91	0.0550	
125.36	249.40	0.0550	
150.93	247.40	0.0550	
155.93	247.33	0.0550 / 0.0350	Main Channel
157.93	246.85	0.0350	Main Channel
159.18	246.65	0.0350	Main Channel
160.18	246.63	0.0350	Main Channel
160.93	246.85	0.0350	Main Channel
161.93	247.18	0.0350 / 0.0550	Main Channel
163.18	248.03	0.0550	
168.18	248.58	0.0550	
183.18	250.18	0.0550	
201.97	252.59	0.0550	
213.57	256.02	0.0550	

225.18	260.31	0.0550
229.82	261.00	0.0550

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.55	247.18	556E+04	1.9	1.03	49.95
1.28	247.91	336E+05	16.7	1.53	33.61
2.00	248.63	913E+05	53.6	1.81	28.40
2.73	249.36	185E+06	122.3	2.05	25.18
3.46	250.09	344E+06	224.6	2.02	25.52
4.19	250.82	574E+06	417.9	2.25	22.91
4.91	251.54	855E+06	699.1	2.53	20.38
5.64	252.27	118E+07	1075.3	2.82	18.26
6.37	253.00	153E+07	1559.9	3.15	16.38
7.10	253.73	191E+07	2137.6	3.46	14.89
7.82	254.45	231E+07	2815.2	3.77	13.67
8.55	255.18	272E+07	3608.5	4.10	12.58
9.28	255.91	315E+07	4488.2	4.41	11.69
10.01	256.64	358E+07	5458.0	4.71	10.95
10.73	257.36	403E+07	6512.4	4.99	10.32
11.46	258.09	449E+07	7649.8	5.27	9.78
12.19	258.82	496E+07	8869.5	5.53	9.32
12.92	259.55	544E+07	10171.2	5.78	8.91
13.64	260.27	593E+07	11554.8	6.02	8.55

INFLOW	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
ID= 2 (8306)	568.24	3.98	7.00	13.76	0.65	1.08
OUTFLOW: ID= 1 (6362)	568.24	3.52	8.25	13.76	0.63	1.07

CALIB STANDHYD (3621)	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	Curve Number (CN)
ID= 1 DT=15.0 min	11.87	45.00	17.00	17.00	1.50

Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
5.34	5.34	6.53
2.00	2.00	5.00
0.50	0.50	0.50
281.31	281.31	40.00
0.013	0.013	0.250

Max. Eff. Inten. (mm/hr)	116.82	57.46
over (min)	15.00	30.00
Storage Coeff. (min)	5.50 (ii)	18.85 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.11	0.05

PEAK FLOW (cms)	0.63	0.54	*TOTALS*
TIME TO PEAK (hrs)	6.00	6.25	0.966 (iii)
RUNOFF VOLUME (mm)	86.50	25.45	35.82
TOTAL RAINFALL (mm)	88.50	88.50	88.50
RUNOFF COEFFICIENT	0.98	0.29	0.40

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 45.7 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5362)	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
IN= 2--> OUT= 1				
DT= 15.0 min				
0.0000	0.0000	0.7940	0.3091	
0.0400	0.1451	0.9970	0.3534	
0.2300	0.1678	1.1610	0.3968	
0.4280	0.2166	1.5610	0.4068	
0.5940	0.2586	0.0000	0.0000	

INFLOW	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID= 2 (3621)	11.870	0.966	6.00	35.82
OUTFLOW: ID= 1 (5362)	11.870	0.349	6.75	35.75

PEAK FLOW REDUCTION [Qout/Qin] (%) = 36.08
 TIME SHIFT OF PEAK FLOW (min) = 45.00
 MAXIMUM STORAGE USED (ha.m.) = 0.2002

CALIB
 STANDHYD (3622) Area (ha) = 106.91
 ID= 1 DT=15.0 min Total Imp(%) = 24.00 Dir. Conn. (%) = 9.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) =	25.66	81.25
Dep. Storage (mm) =	2.00	5.00
Average Slope (%) =	0.50	0.50
Length (m) =	844.24	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr) =	116.82	22.94
over (min) =	15.00	30.00
Storage Coeff. (min) =	10.63 (ii)	29.91 (ii)
Unit Hyd. Tpeak (min) =	15.00	30.00
Unit Hyd. peak (cms) =	0.09	0.04

TOTALS
 4.725 (iii)

PEAK FLOW (cms) =	2.59	3.49	4.725 (iii)
TIME TO PEAK (hrs) =	6.00	6.25	6.00
RUNOFF VOLUME (mm) =	86.50	21.14	27.02
TOTAL RAINFALL (mm) =	88.50	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.24	0.31

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 45.7 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9362) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3622):	106.91	4.725	6.00	27.02
+ ID2= 2 (5362):	11.87	0.349	6.75	35.75
=====				
ID = 3 (9362):	118.78	4.757	6.00	27.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8304) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6362):	568.24	3.522	8.25	13.76
+ ID2= 2 (9362):	118.78	4.757	6.00	27.89
=====				
ID = 3 (8304):	687.02	5.912	6.25	16.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8300) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8302):	617.89	10.029	6.50	27.73
+ ID2= 2 (8304):	687.02	5.912	6.25	16.20
=====				
ID = 3 (8300):	1304.91	15.471	6.50	21.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6368) IN= 2--> OUT= 1	Routing time step (min) = 15.00
-------------------------------------	---------------------------------

<----- DATA FOR SECTION (3681.0) ----->

Distance	Elevation	Manning
0.00	230.00	0.0370
18.48	223.26	0.0370
36.96	223.05	0.0370
64.67	222.94	0.0370
110.87	222.86	0.0370
133.96	222.74	0.0370
147.82	222.65	0.0370
170.92	222.31	0.0370
174.79	222.26	0.0370 / 0.0300
174.89	221.86	0.0300
175.54	221.86	0.0300
176.19	221.86	0.0300
176.29	222.26	0.0300 / 0.0370
180.16	222.25	0.0370
184.78	222.28	0.0370
189.40	222.31	0.0370
332.60	222.37	0.0370
450.00	230.00	0.0370

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.40	222.26	.771E+03	0.3	0.52	41.74
0.81	222.67	.784E+05	30.3	0.51	43.17
1.21	223.07	.210E+06	123.2	0.77	28.42
1.62	223.48	.384E+06	301.0	1.03	21.25
2.03	223.89	.563E+06	550.2	1.28	17.06
2.44	224.30	.747E+06	860.7	1.51	14.46
2.84	224.70	.934E+06	1228.1	1.73	12.68
3.25	225.11	.113E+07	1649.3	1.92	11.37
3.66	225.52	.132E+07	2122.3	2.11	10.37
4.07	225.93	.152E+07	2645.5	2.29	9.58
4.47	226.33	.172E+07	3217.8	2.45	8.92
4.88	226.74	.193E+07	3838.3	2.61	8.38
5.29	227.15	.214E+07	4506.3	2.76	7.92
5.70	227.56	.236E+07	5221.3	2.91	7.52
6.10	227.96	.258E+07	5983.0	3.05	7.17
6.51	228.37	.280E+07	6790.9	3.19	6.87
6.92	228.78	.303E+07	7645.0	3.32	6.60
7.33	229.19	.326E+07	8545.1	3.45	6.35
7.73	229.59	.349E+07	9491.0	3.57	6.13

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8300)	1304.91	15.47	6.50	21.66	0.61	0.52
OUTFLOW: ID= 1 (6368)	1304.91	12.33	7.50	21.66	0.56	0.52

CALIB
 STANDHYD (3681) Area (ha) = 2.95
 ID= 1 DT=15.0 min Total Imp(%) = 45.00 Dir. Conn. (%) = 15.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) =	1.33	1.62
Dep. Storage (mm) =	2.00	5.00
Average Slope (%) =	0.50	0.50
Length (m) =	140.24	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr) =	116.82	51.29
over (min) =	15.00	30.00
Storage Coeff. (min) =	3.62 (ii)	17.59 (ii)
Unit Hyd. Tpeak (min) =	15.00	30.00
Unit Hyd. peak (cms) =	0.11	0.05

PEAK FLOW (cms) =	0.14	0.12	*TOTALS* 0.219 (iii)
TIME TO PEAK (hrs) =	6.00	6.25	6.00
RUNOFF VOLUME (mm) =	86.50	22.32	31.94
TOTAL RAINFALL (mm) =	88.50	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.25	0.36

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 40.6 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5368)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.1970	0.0767
0.0100	0.0360	0.2470	0.0877
0.0570	0.0416	0.2880	0.0984
0.1060	0.0537	0.6880	0.1084
0.1470	0.0642	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (3681)	2.950	0.219	6.00	31.94
OUTFLOW: ID= 1 (5368)	2.950	0.074	6.75	31.64

PEAK FLOW REDUCTION [Oout/Oin] (%) = 33.71
TIME SHIFT OF PEAK FLOW (min) = 45.00
MAXIMUM STORAGE USED (ha.m.) = 0.0464

CALIB NASHYD (3682)
ID= 1 DT=15.0 min

Area (ha) = 156.53 Curve Number (CN) = 45.9
U. H. Tp (hrs) = 0.96 # of Linear Res. (N) = 1.50

Unit Hyd Opeak (cms) = 2.778

PEAK FLOW (cms) = 1.334 (i)
TIME TO PEAK (hrs) = 7.250
RUNOFF VOLUME (mm) = 18.013
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.204

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9368)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3682):	156.53	1.334	7.25	18.01
+ ID2= 2 (5368):	2.95	0.074	6.75	31.64
ID = 3 (9368):	159.48	1.392	7.25	18.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8298)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6368):	1304.91	12.329	7.50	21.66
+ ID2= 2 (9368):	159.48	1.392	7.25	18.27
ID = 3 (8298):	1464.39	13.699	7.50	21.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8296)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0370):	191.85	3.631	6.75	29.42
+ ID2= 2 (8298):	1464.39	13.699	7.50	21.29
ID = 3 (8296):	1656.24	17.063	7.25	22.23

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6372)
IN= 2--> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (3721.0) ----->

Distance (m)	Elevation (m)	Manning	
0.00	225.00	0.0390	
30.80	219.38	0.0390	
61.61	219.30	0.0390	
77.01	219.27	0.0390	
469.76	219.14	0.0390	
477.46	219.13	0.0390	
485.16	219.10	0.0390	
492.86	219.09	0.0390	
495.56	219.09	0.0390 / 0.0310	Main Channel
495.66	218.51	0.0310	Main Channel
500.56	218.51	0.0310	Main Channel
505.46	218.51	0.0310	Main Channel
505.56	219.09	0.0310 / 0.0390	Main Channel
508.26	219.09	0.0390	
515.96	219.10	0.0390	
523.67	219.21	0.0390	
562.17	219.32	0.0390	
654.58	219.43	0.0390	
731.59	219.46	0.0390	
762.39	225.00	0.0390	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.29	218.80	431E+04	1.7	0.58	43.48
0.58	219.09	867E+04	5.1	0.89	28.32
0.93	219.44	200E+06	58.2	0.44	57.35
1.28	219.79	568E+06	278.6	0.74	33.99
1.62	220.13	939E+06	630.2	1.01	24.84
1.97	220.48	131E+07	1090.4	1.25	20.05
2.32	220.83	169E+07	1647.7	1.47	17.06
2.67	221.18	206E+07	2294.6	1.68	14.99
3.01	221.52	244E+07	3025.6	1.87	13.46
3.36	221.87	282E+07	3836.5	2.05	12.27
3.71	222.22	321E+07	4723.7	2.22	11.31
4.06	222.57	359E+07	5684.6	2.39	10.53
4.40	222.91	398E+07	6716.6	2.55	9.87
4.75	223.26	437E+07	7817.7	2.70	9.31
5.10	223.61	476E+07	8986.2	2.85	8.83
5.45	223.96	515E+07	10220.4	2.99	8.40
5.79	224.30	555E+07	11519.1	3.13	8.03
6.14	224.65	594E+07	12880.8	3.27	7.69
6.49	225.00	634E+07	14304.5	3.40	7.39

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8296)	1656.24	17.06	7.25	22.23	0.66	0.72
OUTFLOW: ID= 1 (6372)	1656.24	15.96	8.25	22.23	0.65	0.74

ADD HYD (8294)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0372):	110.42	0.691	7.25	13.37
+ ID2= 2 (6372):	1656.24	15.964	8.25	22.23
ID = 3 (8294):	1766.66	16.613	8.25	21.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8292)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0374):	545.70	5.223	8.25	28.20
+ ID2= 2 (8294):	1766.66	16.613	8.25	21.68
ID = 3 (8292):	2312.36	21.836	8.25	23.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5505)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW STORAGE | OUTFLOW STORAGE

(cms)	(ha. m.)	(cms)	(ha. m.)
0.0000	0.0000	65.1290	345.3754
25.4850	24.6697	84.9510	456.3890
31.1490	98.6787	*****	838.7689
39.6440	*****	*****	838.7789
48.1390	*****	0.0000	0.0000

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8292)	2312.360	21.836	8.25	23.22
OUTFLOW: ID= 1 (5505)	2312.360	15.704	11.00	23.22

PEAK FLOW REDUCTION [Out/In] (%) = 71.92
 TIME SHIFT OF PEAK FLOW (min) = 165.00
 MAXIMUM STORAGE USED (ha. m.) = 15.2089

ADD HYD (8272)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0376):	463.85	8.365	7.25	39.99
+ ID2= 2 (5505):	2312.36	15.704	11.00	23.22
=====	=====	=====	=====	=====
ID = 3 (8272):	2776.21	21.107	10.00	26.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0396)	Area (ha) = 305.21	Curve Number (CN) = 69.0
ID= 1 DT=15.0 min	Ia (mm) = 5.00	# of Linear Res. (N) = 1.50
	U. H. Tp(hrs) = 1.08	

Unit Hyd Opeak (cms) = 4.811

PEAK FLOW (cms) = 4.736 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 34.962
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.395

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0394)	Area (ha) = 325.45	Curve Number (CN) = 53.0
ID= 1 DT=15.0 min	Ia (mm) = 5.00	# of Linear Res. (N) = 1.50
	U. H. Tp(hrs) = 0.92	

Unit Hyd Opeak (cms) = 6.013

PEAK FLOW (cms) = 3.577 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 22.323
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.252

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0390)	Area (ha) = 420.00	Curve Number (CN) = 55.0
ID= 1 DT=15.0 min	Ia (mm) = 5.00	# of Linear Res. (N) = 1.50
	U. H. Tp(hrs) = 1.07	

Unit Hyd Opeak (cms) = 6.683

PEAK FLOW (cms) = 4.371 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 23.713
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.268

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0388)	Area (ha) = 220.77	Curve Number (CN) = 58.0
---------------------	--------------------	--------------------------

ID= 1 DT=15.0 min	Ia (mm) = 5.00	# of Linear Res. (N) = 1.50
	U. H. Tp(hrs) = 0.99	

Unit Hyd Opeak (cms) = 3.819

PEAK FLOW (cms) = 2.679 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 25.800
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.292

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0386)	Area (ha) = 241.27	Curve Number (CN) = 61.0
ID= 1 DT=15.0 min	Ia (mm) = 5.00	# of Linear Res. (N) = 1.50
	U. H. Tp(hrs) = 0.90	

Unit Hyd Opeak (cms) = 4.562

PEAK FLOW (cms) = 3.426 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 28.018
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.317

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8286)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0386):	241.27	3.426	7.00	28.02
+ ID2= 2 (0388):	220.77	2.679	7.25	25.80
=====	=====	=====	=====	=====
ID = 3 (8286):	462.04	6.100	7.25	26.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0384)	Area (ha) = 199.07	Curve Number (CN) = 44.0
ID= 1 DT=15.0 min	Ia (mm) = 5.00	# of Linear Res. (N) = 1.50
	U. H. Tp(hrs) = 0.96	

Unit Hyd Opeak (cms) = 3.537

PEAK FLOW (cms) = 1.596 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 16.955
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.192

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0380)	Area (ha) = 182.01	Curve Number (CN) = 40.0
ID= 1 DT=15.0 min	Ia (mm) = 5.00	# of Linear Res. (N) = 1.50
	U. H. Tp(hrs) = 0.55	

Unit Hyd Opeak (cms) = 5.609

PEAK FLOW (cms) = 1.939 (i)
 TIME TO PEAK (hrs) = 6.500
 RUNOFF VOLUME (mm) = 14.644
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.165

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0382)	Area (ha) = 216.59	Curve Number (CN) = 53.0
ID= 1 DT=15.0 min	Ia (mm) = 5.00	# of Linear Res. (N) = 1.50
	U. H. Tp(hrs) = 0.64	

Unit Hyd Qpeak (cms)= 5.733

PEAK FLOW (cms)= 3.143 (i)
TIME TO PEAK (hrs)= 6.750
RUNOFF VOLUME (mm)= 22.142
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.250

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	(8290)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1	(0380):	182.01	1.939	6.50	14.64
+ ID2= 2	(0382):	216.59	3.143	6.75	22.14
ID = 3	(8290):	398.60	5.066	6.75	18.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6384)
IN= 2----> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3841.0) ----->

Distance	Elevation	Manning	
0.00	294.40	0.0380	
10.59	291.93	0.0380	
21.17	289.19	0.0380	
26.46	287.99	0.0380	
31.76	286.79	0.0380	
71.45	279.97	0.0380	
74.10	279.79	0.0380	
76.74	279.71	0.0380	
78.99	279.64	0.0380 / 0.0300	Main Channel
79.14	279.30	0.0300	Main Channel
79.39	279.30	0.0300	Main Channel
79.64	279.30	0.0300	Main Channel
79.79	279.64	0.0300 / 0.0380	Main Channel
89.98	279.78	0.0380	
119.09	281.86	0.0380	
145.55	282.87	0.0380	
198.48	284.85	0.0380	
211.71	286.31	0.0380	
230.23	287.59	0.0380	
261.99	294.00	0.0380	

<----- TRAVEL TIME TABLE ----->

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)
0.34	279.64	.715E+03	0.2	0.85	63.42
1.10	280.40	.507E+05	21.9	1.40	38.54
1.85	281.15	.141E+06	91.8	2.10	25.69
2.61	281.91	.269E+06	218.9	2.63	20.46
3.36	282.66	.445E+06	407.1	2.96	18.23
4.12	283.42	.681E+06	698.3	3.31	16.26
4.87	284.17	.977E+06	1111.8	3.68	14.65
5.63	284.93	.133E+07	1674.3	4.06	13.27
6.39	285.69	.173E+07	2457.8	4.59	11.73
7.14	286.44	.215E+07	3378.8	5.07	10.63
7.90	287.20	.261E+07	4418.4	5.47	9.85
8.65	287.95	.310E+07	5676.3	5.92	9.11
9.41	288.71	.361E+07	7150.5	6.40	8.42
10.17	289.47	.414E+07	8783.1	6.86	7.86
10.92	290.22	.469E+07	10575.1	7.30	7.38
11.68	290.98	.525E+07	12520.4	7.72	6.98
12.43	291.73	.582E+07	14619.0	8.12	6.64
13.19	292.49	.642E+07	16863.2	8.50	6.34
13.94	293.24	.703E+07	19259.8	8.86	6.08

	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
INFLOW: ID= 2 (8290)	398.60	5.07	6.75	18.72	0.51	0.93
OUTFLOW: ID= 1 (6384)	398.60	3.99	7.75	18.72	0.47	0.91

ADD HYD	(8288)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1	(0384):	199.07	1.596	7.25	16.95
+ ID2= 2	(6384):	398.60	3.985	7.75	18.72
ID = 3	(8288):	597.67	5.554	7.75	18.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8284)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1	(8286):	462.04	6.100	7.25	26.96
+ ID2= 2	(8288):	597.67	5.554	7.75	18.13
ID = 3	(8284):	1059.71	11.532	7.50	21.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(8280)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1	(0390):	420.00	4.371	7.50	23.71
+ ID2= 2	(8284):	1059.71	11.532	7.50	21.98
ID = 3	(8280):	1479.71	15.903	7.50	22.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0392)
ID= 1 DT=15.0 min
Area (ha)= 167.22
Ia (mm)= 5.00
U. H. Tp(hrs)= 0.74
Curve Number (CN)= 62.0
of Li near Res. (N)= 1.50

Unit Hyd Qpeak (cms)= 3.837
PEAK FLOW (cms)= 2.840 (i)
TIME TO PEAK (hrs)= 7.000
RUNOFF VOLUME (mm)= 28.690
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.324

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0378)
ID= 1 DT=15.0 min
Area (ha)= 606.72
Ia (mm)= 5.00
U. H. Tp(hrs)= 1.18
Curve Number (CN)= 55.0
of Li near Res. (N)= 1.50

Unit Hyd Qpeak (cms)= 8.771
PEAK FLOW (cms)= 5.877 (i)
TIME TO PEAK (hrs)= 7.750
RUNOFF VOLUME (mm)= 23.743
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.268

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	(8282)	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	
ID1= 1	(0378):	606.72	5.877	7.75	23.74
+ ID2= 2	(0392):	167.22	2.840	7.00	28.69
ID = 3	(8282):	773.94	8.554	7.25	24.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8280):	1479.71	15.903	7.50	22.47
+ ID2= 2 (8282):	773.94	8.554	7.25	24.81
=====				
ID = 3 (8278):	2253.65	24.428	7.50	23.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6394)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3941.0) ----->

Distance	Elevation	Manning		
0.00	283.00	0.0380		
13.48	282.87	0.0380		
53.92	280.08	0.0380		
74.13	276.62	0.0380		
97.72	265.45	0.0380		
114.57	256.93	0.0380		
131.42	253.04	0.0380		
134.79	252.58	0.0380		
138.53	251.74	0.0380 / 0.0300	Main Channel	
139.03	251.20	0.0300	Main Channel	
141.53	251.20	0.0300	Main Channel	
144.03	251.20	0.0300	Main Channel	
144.53	251.74	0.0300 / 0.0380	Main Channel	
148.27	252.69	0.0380		
151.64	252.97	0.0380		
185.34	255.08	0.0380		
219.03	257.54	0.0380		
262.84	259.43	0.0380		
310.02	262.80	0.0380		
333.60	283.00	0.0380		

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.54	251.74	827E+04	4.5	1.51	30.71
2.19	253.39	758E+05	73.2	2.69	17.27
3.83	255.03	282E+06	331.8	3.27	14.17
5.48	256.68	633E+06	917.3	4.03	11.51
7.12	258.32	112E+07	1889.5	4.69	9.90
8.77	259.97	179E+07	3410.2	5.31	8.73
10.41	261.61	259E+07	5627.6	6.05	7.66
12.06	263.26	351E+07	8605.1	6.83	6.79
13.70	264.90	447E+07	12544.9	7.81	5.94
15.35	266.55	546E+07	17101.4	8.71	5.32
16.99	268.19	648E+07	22251.7	9.56	4.85
18.64	269.84	752E+07	27979.0	10.36	4.48
20.28	271.48	858E+07	34269.0	11.11	4.17
21.93	273.13	967E+07	41110.8	11.83	3.92
23.57	274.77	108E+08	48495.9	12.51	3.71
25.22	276.42	119E+08	56418.1	13.17	3.52
26.86	278.06	131E+08	64410.6	13.69	3.39
28.51	279.71	143E+08	73002.1	14.18	3.27
30.15	281.35	156E+08	81379.3	14.50	3.20

<----- hydrograph ----->				<-pi pe / channel ->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8278)	2253.65	24.43	7.50	23.28	1.02
OUTFLOW: ID= 1 (6394)	2253.65	23.85	8.00	23.28	1.00

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0394):	325.45	3.577	7.25	22.32
+ ID2= 2 (6394):	2253.65	23.853	8.00	23.28
=====				
ID = 3 (8276):	2579.10	27.252	8.00	23.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6396)
IN= 2--> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (3961.0) ----->

Distance	Elevation	Manning		
0.00	263.00	0.0410		
11.75	257.14	0.0410		
23.50	253.97	0.0410		
41.13	247.83	0.0410		
76.38	232.09	0.0410		
135.13	229.07	0.0410		
149.82	228.97	0.0410		
152.75	228.96	0.0410 / 0.0300	Main Channel	
154.19	228.73	0.0300	Main Channel	
154.69	228.20	0.0300	Main Channel	
155.69	228.20	0.0300	Main Channel	
156.69	228.20	0.0300	Main Channel	
157.19	228.73	0.0300	Main Channel	
158.63	228.95	0.0300 / 0.0410	Main Channel	
161.57	228.96	0.0410		
164.51	229.71	0.0410		
196.82	241.70	0.0410		
223.26	249.21	0.0410		
246.76	255.13	0.0410		
290.82	263.51	0.0410		

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.75	228.95	780E+04	2.2	0.98	58.06
2.54	230.74	277E+06	145.1	1.78	31.76
4.33	232.53	786E+06	609.1	2.63	21.50
6.13	234.33	140E+07	1481.5	3.59	15.77
7.92	236.12	207E+07	2677.3	4.39	12.90
9.71	237.91	280E+07	4184.8	5.09	11.14
11.50	239.70	357E+07	6001.4	5.71	9.93
13.29	241.49	441E+07	8129.1	6.27	9.03
15.09	243.29	530E+07	10549.4	6.77	8.37
16.88	245.08	625E+07	13300.6	7.24	7.83
18.67	246.87	726E+07	16396.2	7.68	7.38
20.46	248.66	834E+07	19809.8	8.07	7.02
22.26	250.46	949E+07	23532.5	8.43	6.72
24.05	252.25	107E+08	27641.2	8.77	6.46
25.84	254.04	120E+08	32155.2	9.10	6.22
27.63	255.83	134E+08	36901.0	9.37	6.05
29.42	257.62	149E+08	42147.7	9.64	5.88
31.22	259.42	164E+08	48149.1	9.97	5.68
33.01	261.21	181E+08	54644.8	10.29	5.51

<----- hydrograph ----->				<-pi pe / channel ->	
AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8276)	2579.10	27.25	8.00	23.16	1.06
OUTFLOW: ID= 1 (6396)	2579.10	24.64	9.00	23.16	1.03

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0396):	305.21	4.736	7.50	34.96
+ ID2= 2 (6396):	2579.10	24.639	9.00	23.16
=====				
ID = 3 (8274):	2884.31	28.747	8.75	24.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8272):	2776.21	21.107	10.00	26.02
+ ID2= 2 (8274):	2884.31	28.747	8.75	24.40
=====				
ID = 3 (8270):	5660.52	48.960	9.25	25.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5506)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	60.8810	135.6832
31.1490	24.6697	96.2770	900.4431
36.8120	37.0045	96.6770	900.4531
45.3070	86.3439	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (8270)	5660.520	48.960	9.25	25.20
OUTFLOW: ID= 1 (5506)	5660.520	35.985	12.25	25.20

PEAK FLOW REDUCTION [Qout/Oin](%) = 73.50
TIME SHIFT OF PEAK FLOW (min) = 180.00
MAXIMUM STORAGE USED (ha. m.) = 35.2177

CALIB NASHYD (0406)
ID= 1 DT=15.0 min

Area (ha)	Curve Number (CN)
142.65	66.0
Ia (mm) = 5.00	# of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 0.59	

Unit Hyd Qpeak (cms) =	4.135
PEAK FLOW (cms) =	3.269 (i)
TIME TO PEAK (hrs) =	6.500
RUNOFF VOLUME (mm) =	31.802
TOTAL RAINFALL (mm) =	88.500
RUNOFF COEFFICIENT =	0.359

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0404)
ID= 1 DT=15.0 min

Area (ha)	Curve Number (CN)
246.46	47.0
Ia (mm) = 5.00	# of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 0.98	

Unit Hyd Qpeak (cms) =	4.280
PEAK FLOW (cms) =	2.141 (i)
TIME TO PEAK (hrs) =	7.250
RUNOFF VOLUME (mm) =	18.650
TOTAL RAINFALL (mm) =	88.500
RUNOFF COEFFICIENT =	0.211

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0402)
ID= 1 DT=15.0 min

Area (ha)	Curve Number (CN)
244.00	61.0
Ia (mm) = 5.00	# of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 1.07	

Unit Hyd Qpeak (cms) =	3.879
PEAK FLOW (cms) =	3.028 (i)
TIME TO PEAK (hrs) =	7.500
RUNOFF VOLUME (mm) =	28.095
TOTAL RAINFALL (mm) =	88.500
RUNOFF COEFFICIENT =	0.317

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0400)
ID= 1 DT=15.0 min

Area (ha)	Curve Number (CN)
93.97	52.0
Ia (mm) = 5.00	# of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 0.44	

Unit Hyd Qpeak (cms) =	3.630
PEAK FLOW (cms) =	1.763 (i)
TIME TO PEAK (hrs) =	6.500
RUNOFF VOLUME (mm) =	21.182
TOTAL RAINFALL (mm) =	88.500
RUNOFF COEFFICIENT =	0.239

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0398)
ID= 1 DT=15.0 min

Area (ha)	Curve Number (CN)
328.19	55.0
Ia (mm) = 5.00	# of Linear Res. (N) = 1.50
U. H. Tp(hrs) = 0.83	

Unit Hyd Qpeak (cms) =	6.759
PEAK FLOW (cms) =	4.178 (i)
TIME TO PEAK (hrs) =	7.000
RUNOFF VOLUME (mm) =	23.611
TOTAL RAINFALL (mm) =	88.500
RUNOFF COEFFICIENT =	0.267

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8268)
1 + 2 = 3

ID	Area (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0398):	328.19	4.178	7.00	23.61
+ ID2= 2 (0400):	93.97	1.763	6.50	21.18
ID = 3 (8268):	422.16	5.776	6.75	23.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6402)
IN= 2--> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (4021.0) ----->

Distance	Elevation	Manning	
0.00	238.50	0.0360	
11.50	238.00	0.0360	
23.00	237.93	0.0360	
34.49	236.39	0.0360	
63.24	233.98	0.0360	
97.73	228.15	0.0360	
123.60	227.08	0.0360	
126.48	226.61	0.0360	
127.60	226.47	0.0360 / 0.0330	Main Channel
127.85	225.25	0.0330	Main Channel
129.35	225.25	0.0330	Main Channel
130.85	225.25	0.0330	Main Channel
131.10	226.47	0.0330 / 0.0360	Main Channel
132.22	226.59	0.0360	
143.72	227.42	0.0360	
169.59	227.88	0.0360	
192.59	231.19	0.0360	
218.46	233.02	0.0360	
241.45	235.50	0.0360	
284.57	236.43	0.0360	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (Cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.41	225.66	391E+04	1.7	1.36	38.31
0.81	226.06	804E+04	5.0	1.93	27.02
1.22	226.47	124E+05	9.1	2.30	22.61
1.84	227.09	308E+05	23.3	2.36	22.03
2.46	227.71	904E+05	62.0	2.14	24.31
3.09	228.34	223E+06	188.3	2.63	19.75
3.71	228.96	379E+06	410.2	3.38	15.39
4.33	229.58	550E+06	711.5	4.04	12.89
4.95	230.20	737E+06	1091.3	4.62	11.25
5.58	230.83	939E+06	1550.6	5.15	10.10
6.20	231.45	116E+07	2072.4	5.59	9.31
6.82	232.07	140E+07	2659.2	5.93	8.77
7.44	232.69	167E+07	3345.8	6.27	8.30
8.07	233.32	195E+07	4159.0	6.64	7.83
8.69	233.94	226E+07	5100.8	7.03	7.39
9.31	234.56	259E+07	6081.3	7.32	7.11
9.93	235.18	295E+07	7183.1	7.60	6.84
10.56	235.81	334E+07	8145.2	7.62	6.83
11.18	236.43	379E+07	9116.0	7.51	6.92

	<--- hydrograph --->				<--pi pe / channel -->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8268)	422.16	5.78	6.75	23.07	0.89	1.99
OUTFLOW: ID= 1 (6402)	422.16	5.53	7.25	23.07	0.87	1.97

ADD HYD (8266)						
1 + 2 = 3						
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)		
ID1= 1 (0402):	244.00	3.028	7.50	28.09		
+ ID2= 2 (6402):	422.16	5.530	7.25	23.07		
ID = 3 (8266):	666.16	8.550	7.25	24.91		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8264)						
1 + 2 = 3						
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)		
ID1= 1 (0404):	246.46	2.141	7.25	18.65		
+ ID2= 2 (8266):	666.16	8.550	7.25	24.91		
ID = 3 (8264):	912.62	10.690	7.25	23.22		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8262)						
1 + 2 = 3						
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)		
ID1= 1 (0406):	142.65	3.269	6.50	31.80		
+ ID2= 2 (8264):	912.62	10.690	7.25	23.22		
ID = 3 (8262):	1055.27	13.662	7.00	24.38		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8260)						
1 + 2 = 3						
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)		
ID1= 1 (5506):	5660.52	35.985	12.25	25.20		
+ ID2= 2 (8262):	1055.27	13.662	7.00	24.38		
ID = 3 (8260):	6715.79	41.454	10.50	25.07		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8258)						
1 + 2 = 3						
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)		
ID1= 1 (0408):	231.62	3.942	6.75	25.55		
+ ID2= 2 (8260):	6715.79	41.454	10.50	25.07		
ID = 3 (8258):	6947.41	42.951	10.00	25.08		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8256)						
1 + 2 = 3						
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)		
ID1= 1 (0410):	572.01	3.821	8.00	19.33		
+ ID2= 2 (8258):	6947.41	42.951	10.00	25.08		
ID = 3 (8256):	7519.42	46.379	10.00	24.65		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5507)			
IN= 2--> OUT= 1			
DT= 15.0 mi n			
OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	90.6140	67.8416
39.6440	12.3348	*****	160.3529
48.1390	18.5023	*****	160.3629
67.9600	37.0045	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (8256)	7519.421	46.379	10.00	24.65
OUTFLOW: ID= 1 (5507)	7519.421	44.136	12.25	24.65

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 95.16
 TIME SHIFT OF PEAK FLOW (mi n) = 135.00
 MAXIMUM STORAGE USED (ha. m.) = 15.6050

CALIB NASHVD (0420)			
ID= 1 DT=15.0 mi n	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
	175.82	53.0	1.50
	U. H. Tp (hrs) = 0.81		

Unit Hyd Opeak (cms) = 3.692
 PEAK FLOW (cms) = 2.139 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 22.269
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.252

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (4141)			
ID= 1 DT=15.0 mi n	Area Total	(ha) = 156.72 Imp (%) = 52.00	Di r. Conn. (%) = 21.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	81.49	75.23
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1022.15	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	116.82	113.88
over (mi n)	15.00	30.00
Storage Coeff. (mi n)	11.92 (ii)	22.08 (ii)
Unit Hyd. Tpeak (mi n)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04
PEAK FLOW (cms)	8.51	11.61
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	86.50	45.04
TOTAL RAINFALL (mm)	88.50	88.50
RUNOFF COEFFICIENT	0.98	0.51

TOTALS
 16.080 (iii)
 6.00
 53.75
 88.50
 0.61

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.8 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (4144)			
ID= 1 DT=15.0 mi n	Area Total	(ha) = 2.10 Imp (%) = 30.00	Di r. Conn. (%) = 10.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	0.63	1.47
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	118.32	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 116.82 104.16
 over (min)= 15.00 15.00
 Storage Coeff. (mi n)= 3.27 (ii) 13.79 (ii)
 Uni t Hyd. Tpeak (mi n)= 15.00 15.00
 Uni t Hyd. peak (cms)= 0.11 0.08

PEAK FLOW (cms)= 0.07 0.31 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.00 0.378 (iii)
 RUNOFF VOLUME (mm)= 86.50 52.21 6.00
 TOTAL RAINFALL (mm)= 88.50 88.50 55.63
 RUNOFF COEFFICIENT = 0.98 0.59 88.50
 0.63

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9429)
 IN= 2---> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.1000	0.0386
0.0100	0.0195	0.1400	0.0480
0.0400	0.0218	0.1700	0.0569
0.0700	0.0312	0.2000	0.0655

INFLOW : ID= 2 (4144)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
	2.100	0.378	6.00	55.63
OUTFLOW: ID= 1 (9429)	2.100	0.137	6.25	55.40

PEAK FLOW REDUCTION [Oout/Oi n] (%) = 36.24
 TIME SHIF T OF PEAK FLOW (mi n) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 0.0482

ADD HYD (9424)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4141):	156.72	16.080	6.00	53.75
+ ID2= 2 (9429):	2.10	0.137	6.25	55.40
=====				
ID = 3 (9424):	158.82	16.156	6.00	53.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5414)
 IN= 2---> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.7060	4.4896
0.1870	1.6651	5.4940	5.4773
2.0210	2.0447	6.0800	6.4738
2.9590	2.4658	6.4800	6.4838
3.7490	3.5575	0.0000	0.0000

INFLOW : ID= 2 (9424)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
	158.820	16.156	6.00	53.77
OUTFLOW: ID= 1 (5414)	158.820	4.611	7.00	53.75

PEAK FLOW REDUCTION [Oout/Oi n] (%) = 28.54
 TIME SHIF T OF PEAK FLOW (mi n) = 60.00
 MAXIMUM STORAGE USED (ha. m.) = 4.4037

CALIB STANDHYD (4142)
 ID= 1 DT=15.0 mi n

Area Total	(ha)	Imp(%)	Di r. Conn. (%)
	29.14	52.00	21.00

Surface Area (ha)	IMPERVIOUS	PERVIOUS (i)
	15.15	13.99

Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 440.76 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.82 113.88
 over (mi n)= 15.00 30.00
 Storage Coeff. (mi n)= 7.20 (ii) 17.35 (ii)
 Uni t Hyd. Tpeak (mi n)= 15.00 30.00
 Uni t Hyd. peak (cms)= 0.10 0.05

PEAK FLOW (cms)= 1.82 2.39 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.25 3.395 (iii)
 RUNOFF VOLUME (mm)= 86.50 45.04 6.00
 TOTAL RAINFALL (mm)= 88.50 88.50 53.75
 RUNOFF COEFFICIENT = 0.98 0.51 88.50
 0.61

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 66.8 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8254)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4142):	29.14	3.395	6.00	53.75
+ ID2= 2 (5414):	158.82	4.611	7.00	53.75
=====				
ID = 3 (8254):	187.96	6.322	6.25	53.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (4121)
 ID= 1 DT=15.0 mi n

Area Total	(ha)	Imp(%)	Di r. Conn. (%)
	202.00	45.00	16.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	90.90	111.10
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1160.46	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 116.82 69.18
 over (mi n)= 15.00 30.00
 Storage Coeff. (mi n)= 12.87 (ii) 25.26 (ii)
 Uni t Hyd. Tpeak (mi n)= 15.00 30.00
 Uni t Hyd. peak (cms)= 0.08 0.04

PEAK FLOW (cms)= 8.13 9.66 *TOTALS*
 TIME TO PEAK (hrs)= 6.00 6.25 14.194 (iii)
 RUNOFF VOLUME (mm)= 86.50 30.03 6.00
 TOTAL RAINFALL (mm)= 88.50 88.50 39.07
 RUNOFF COEFFICIENT = 0.98 0.34 88.50
 0.44

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 51.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5412)
 IN= 2---> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	6.5000	1.0500
0.3860	0.5085	7.9000	1.1500
1.8460	0.6103	9.3000	1.2500
3.5050	0.7566	9.7000	1.2600
4.7890	0.8929	0.0000	0.0000

INFLOW : ID= 2 (4121) AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 202.000 14.194 6.00 39.07
 OUTFLOW: ID= 1 (5412) 202.000 14.040 6.25 39.07

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 98.92
 TIME SHIFT OF PEAK FLOW (mi n) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 1.4071

CALIB STANDHYD (4122) | Area (ha)= 36.70
 ID= 1 DT=15.0 mi n | Total Imp(%)= 63.00 Di r. Conn.(%)= 22.00

IMPERVIOUS PERVIOUS (i)
 Surface Area (ha)= 23.12 13.58
 Dep. Storage (mm)= 2.00 5.00
 Average Slope (%)= 0.50 0.50
 Length (m)= 494.64 40.00
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.82 118.89
 over (mi n) = 15.00 30.00
 Storage Coeff. (mi n)= 7.71 (ii) 17.70 (ii)
 Unit Hyd. Tpeak (mi n)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.10 0.05

PEAK FLOW (cms)= 2.37 2.40 3.920 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 86.50 37.17 48.02
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.42 0.54

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 51.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9412) | AREA OPEAK TPEAK R. V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (4122): 36.70 3.920 6.00 48.02
 + ID2= 2 (5412): 202.00 14.040 6.25 39.07
 ID = 3 (9412): 238.70 17.080 6.25 40.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8252) | AREA OPEAK TPEAK R. V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (8254): 187.96 6.322 6.25 53.75
 + ID2= 2 (9412): 238.70 17.080 6.25 40.44
 ID = 3 (8252): 426.66 23.401 6.25 46.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6416) | Routing time step (mi n)' = 15.00
 IN= 2----> OUT= 1

<----- DATA FOR SECTION (4161.0) ----->
 Di stance El evation Mann ing
 0.00 270.07 0.0340
 20.67 267.91 0.0340
 62.01 264.33 0.0340
 113.69 259.75 0.0340
 165.37 253.30 0.0340
 227.38 246.29 0.0340
 232.55 246.03 0.0340
 237.72 246.16 0.0340

241.39 246.02 0.0340 /0.0300 Mai n Channel
 241.64 245.75 0.0300 Mai n Channel
 242.89 245.75 0.0300 Mai n Channel
 244.14 245.75 0.0300 Mai n Channel
 244.39 246.02 0.0300 /0.0340 Mai n Channel
 248.06 246.20 0.0340
 253.22 246.28 0.0340
 258.39 246.63 0.0340
 346.25 252.57 0.0340
 413.43 257.77 0.0340
 465.11 261.78 0.0340
 511.62 270.00 0.0340

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.27	246.02	.295E+04	0.6	0.75	88.26
1.53	247.28	.168E+06	69.7	1.65	40.13
2.79	248.54	.490E+06	299.5	2.42	27.28
4.06	249.81	.962E+06	740.1	3.05	21.67
5.32	251.07	.158E+07	1440.0	3.61	18.33
6.58	252.33	.235E+07	2443.8	4.12	16.06
7.84	253.59	.327E+07	3811.2	4.63	14.30
9.10	254.85	.432E+07	5565.1	5.11	12.94
10.37	256.12	.550E+07	7721.3	5.57	11.88
11.63	257.38	.682E+07	10309.9	6.00	11.02
12.89	258.64	.827E+07	13360.6	6.42	10.31
14.15	259.90	.985E+07	16884.5	6.81	9.72
15.42	261.17	.116E+08	20818.4	7.14	9.26
16.68	262.43	.134E+08	25510.4	7.53	8.78
17.94	263.69	.154E+08	30958.0	7.97	8.30
19.20	264.95	.175E+08	36988.3	8.38	7.89
20.46	266.21	.197E+08	43617.1	8.78	7.53
21.73	267.48	.220E+08	50868.1	9.17	7.22
22.99	268.74	.244E+08	58854.0	9.56	6.92

<---- hydrograph ----> <-pi pe / channel ->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW : ID= 2 (8252) 426.66 23.40 6.25 46.31 0.69 0.91
 OUTFLOW: ID= 1 (6416) 426.66 10.26 7.25 46.31 0.45 0.81

CALIB NASHYD (4164) | Area (ha)= 89.30 Curve Number (CN)= 76.0
 ID= 1 DT=15.0 mi n | Ia (mm)= 5.00 # of Li near Res. (N)= 3.00
 U. H. Tp(hrs)= 1.46

Unit Hyd Opeak (cms)= 2.344
 PEAK FLOW (cms)= 2.699 (i)
 TIME TO PEAK (hrs)= 7.500
 RUNOFF VOLUME (mm)= 42.586
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.481

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9427) |
 IN= 2----> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	4.4700	2.5000
0.9000	1.0000	5.9800	3.0000
1.7300	1.5000	7.5000	3.5000
3.2200	2.0000	8.7400	4.0000

INFLOW : ID= 2 (4164) AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 89.300 2.699 7.50 42.59
 OUTFLOW: ID= 1 (9427) 89.300 1.642 9.00 42.58

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 60.86
 TIME SHIFT OF PEAK FLOW (mi n) = 90.00
 MAXIMUM STORAGE USED (ha. m.) = 1.4512

RESERVOIR (5416) |
 IN= 2----> OUT= 1

DT= 15.0 min		OUTFLOW	STORAGE	OUTFLOW	STORAGE
		(cms)	(ha. m.)	(cms)	(ha. m.)
		0.0000	0.0000	1.5980	0.7565
		0.0330	0.3532	1.8570	0.8137
		0.7450	0.4481	2.0990	0.8707
		1.0270	0.5154	2.4990	0.8807
		1.3050	0.6350	0.0000	0.0000

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (9427)	89.300	1.642	9.00	42.58
OUTFLOW: ID= 1 (5416)	89.300	1.452	10.25	42.55

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 88.39
 TIME SHIFT OF PEAK FLOW (min) = 75.00
 MAXIMUM STORAGE USED (ha. m.) = 0.6961

CALIB	Area	(ha)	Curve Number	(CN)
NASHYD (4162)	349.99		61.3	
ID= 1 DT=15.0 min	1a	(mm) = 5.00	# of Linear Res. (N) = 1.50	
	U. H. Tp	(hrs) = 1.29		

Unit Hyd Qpeak (cms) = 4.646

PEAK FLOW (cms) = 3.831 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 28.391
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.321

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (4162):	349.99	3.831	8.00	28.39
+ ID2= 2 (5416):	89.30	1.452	10.25	42.55
ID = 3 (9416):	439.29	4.785	9.00	31.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6416):	426.66	10.255	7.25	46.31
+ ID2= 2 (9416):	439.29	4.785	9.00	31.27
ID = 3 (8250):	865.95	13.991	7.25	38.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB	Area	(ha)	Curve Number	(CN)
NASHYD (0418)	174.09		64.0	
ID= 1 DT=15.0 min	1a	(mm) = 5.00	# of Linear Res. (N) = 1.50	
	U. H. Tp	(hrs) = 1.05		

Unit Hyd Qpeak (cms) = 2.825

PEAK FLOW (cms) = 2.392 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 30.508
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.345

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area	(ha)	Di r. Conn. (%)
STANDHYD (4184)	8.70		10.00
ID= 1 DT=15.0 min	Total Imp	(%) = 30.00	

IMPERVIOUS PERVIOUS (i)

Surface Area	(ha) =	2.61	6.09
Dep. Storage	(mm) =	2.00	5.00
Average Slope	(%) =	0.50	0.50
Length	(m) =	240.83	40.00
Mannings n	=	0.013	0.250

Max. Eff. Inten.	(mm/hr) =	116.82	90.20
Storage over	(mi n) =	15.00	30.00
Storage Coeff.	(mi n) =	5.01 (ii)	16.16 (ii)
Unit Hyd. Tpeak	(mi n) =	15.00	30.00
Unit Hyd. Tpeak	(cms) =	0.11	0.05

PEAK FLOW	(cms) =	0.27	0.85	*TOTALS*
TIME TO PEAK	(hrs) =	6.00	6.25	0.896 (iii)
RUNOFF VOLUME	(mm) =	86.50	45.40	6.25
TOTAL RAINFALL	(mm) =	88.50	88.50	49.51
RUNOFF COEFFICIENT	=	0.98	0.51	88.50
				0.56

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 73.0 1a = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9428)		OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2--> OUT= 1		(cms)	(ha. m.)	(cms)	(ha. m.)
DT= 15.0 min		0.0000	0.0000	0.4300	0.1618
		0.0300	0.0818	0.5800	0.2012
		0.1700	0.0912	0.7300	0.2384
		0.3100	0.1306	0.8500	0.2747

	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (4184)	8.700	0.896	6.25	49.51
OUTFLOW: ID= 1 (9428)	8.700	0.462	6.50	49.43

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 51.61
 TIME SHIFT OF PEAK FLOW (min) = 15.00
 MAXIMUM STORAGE USED (ha. m.) = 0.1729

ADD HYD	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0418):	174.09	2.392	7.50	30.51
+ ID2= 2 (9428):	8.70	0.462	6.50	49.43
ID = 3 (9423):	182.79	2.757	7.00	31.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8250):	865.95	13.991	7.25	38.68
+ ID2= 2 (9423):	182.79	2.757	7.00	31.41
ID = 3 (8248):	1048.74	16.715	7.25	37.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0420):	175.82	2.139	7.00	22.27
+ ID2= 2 (8248):	1048.74	16.715	7.25	37.41
ID = 3 (8246):	1224.56	18.829	7.25	35.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (5507):	7519.42	44.136	12.25	24.65
+ ID2= 2 (8246):	1224.56	18.829	7.25	35.24

ID = 3 (8244):	8743.98	55.553	10.00	26.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (4227) ID= 1 DT=15.0 min	Area Total	(ha)= Imp(%)=	66.80 29.00	Di r. Conn. (%)=	15.00
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	IMPERVIOUS (ha)	PERVIOUS (i) (ha)
Surface Area	19.37	47.43
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	667.33	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)=	116.82	48.34
over (min)	15.00	30.00
Storage Coeff. (min)=	9.23 (ii)	23.54 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.04

TOTALS
PEAK FLOW (cms)= 2.81 2.98 4.664 (iii)
TIME TO PEAK (hrs)= 6.00 6.25 6.00
RUNOFF VOLUME (mm)= 86.50 26.83 35.78
TOTAL RAINFALL (mm)= 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.30 0.40

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 54.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9426) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)	
	0.0000	0.0000	3.3400	1.2303	
	0.2300	0.6171	4.4700	1.5350	
	1.3000	0.6860	5.6100	1.8228	
	2.4100	0.9909	6.5300	2.1041	
		AREA	OPEAK	TPEAK	R. V.
		(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (4227)	66.800	4.664	6.00	35.78	
OUTFLOW: ID= 1 (9426)	66.800	2.228	6.50	35.77	

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 47.76
TIME SHIF T OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 0.9590

RESERVOIR (5422) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)	
	0.0000	0.0000	4.3110	2.6782	
	0.2180	1.7874	5.4130	2.9186	
	1.2490	1.9110	6.3010	3.1540	
	2.3250	2.1760	6.7010	3.1640	
	3.2250	2.4038	0.0000	0.0000	
		AREA	OPEAK	TPEAK	R. V.

INFLOW : ID= 2 (9426)	(ha)	(cms)	(hrs)	(mm)
66.800	66.800	2.228	6.50	35.77
OUTFLOW: ID= 1 (5422)	66.800	0.179	14.25	35.74

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 8.04
TIME SHIF T OF PEAK FLOW (min) = 465.00
MAXIMUM STORAGE USED (ha. m.) = 1.4693

CALIB NASHYD (4222) ID= 1 DT=15.0 min	Area Ia	(ha)= (mm)=	713.41 5.00	Curve Number # of Li near Res. (N)=	54.0 1.50
---	------------	----------------	----------------	--	--------------

Unit Hyd Opeak (cms)= 6.252

PEAK FLOW (cms)= 4.565 (i)
TIME TO PEAK (hrs)= 9.000
RUNOFF VOLUME (mm)= 23.163
TOTAL RAINFALL (mm)= 88.500
RUNOFF COEFFICIENT = 0.262

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9422) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (4222):	713.41	4.565	9.00	23.16
+ ID2= 2 (5422):	66.80	0.179	14.25	35.74

ID = 3 (9422):	780.21	4.726	9.00	24.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8242) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8244):	8743.98	55.553	10.00	26.13
+ ID2= 2 (9422):	780.21	4.726	9.00	24.24

ID = 3 (8242):	9524.19	60.208	10.00	25.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5508) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)	
	0.0000	0.0000	*****	197.3574	
	76.4550	30.8371	*****	394.7148	
	*****	61.6742	*****	394.7248	
			0.0000	0.0000	
		AREA	OPEAK	TPEAK	R. V.
		(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (8242)	9524.190	60.208	10.00	25.98	
OUTFLOW: ID= 1 (5508)	9524.190	57.965	11.50	25.97	

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 96.27
TIME SHIF T OF PEAK FLOW (min) = 90.00
MAXIMUM STORAGE USED (ha. m.) = 23.3852

CALIB NASHYD (0336) ID= 1 DT=15.0 min	Area Ia	(ha)= (mm)=	2785.00 5.00	Curve Number # of Li near Res. (N)=	72.0 1.50
---	------------	----------------	-----------------	--	--------------

Unit Hyd Opeak (cms)= 3.090

PEAK FLOW (cms)= 4.639 (i)
TIME TO PEAK (hrs)= 22.250
RUNOFF VOLUME (mm)= 38.196
TOTAL RAINFALL (mm)= 88.500

RUNOFF COEFFICIENT = 0.432

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3382) ID= 1 DT=15.0 min		Area Total (ha)= 433.29 Imp(%)= 39.00	Dir. Conn. (%)= 20.00
		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)= 168.98		264.31
Dep. Storage	(mm)= 2.00		5.00
Average Slope	(%)= 0.50		0.50
Length	(m)= 1699.59		40.00
Mannings n	= 0.013		0.250
Max. Eff. Inten. (mm/hr)= over (min)	116.82 15.00		76.01 30.00
Storage Coeff. (min)=	16.18 (ii)		28.11 (ii)
Unit Hyd. Tpeak (min)=	15.00		30.00
Unit Hyd. peak (cms)=	0.07		0.04
PEAK FLOW (cms)=	19.82		24.00
TIME TO PEAK (hrs)=	6.00		6.25
RUNOFF VOLUME (mm)=	86.50		37.86
TOTAL RAINFALL (mm)=	88.50		88.50
RUNOFF COEFFICIENT =	0.98		0.43
		TOTALS	35.026 (iii)

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3381) ID= 1 DT=15.0 min		Area Total (ha)= 165.62 Imp(%)= 53.00	Dir. Conn. (%)= 27.00
		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)= 87.78		77.84
Dep. Storage	(mm)= 2.00		5.00
Average Slope	(%)= 0.50		0.50
Length	(m)= 1050.78		40.00
Mannings n	= 0.013		0.250
Max. Eff. Inten. (mm/hr)= over (min)	116.82 15.00		99.70 30.00
Storage Coeff. (min)=	12.12 (ii)		22.83 (ii)
Unit Hyd. Tpeak (min)=	15.00		30.00
Unit Hyd. peak (cms)=	0.08		0.04
PEAK FLOW (cms)=	11.50		10.33
TIME TO PEAK (hrs)=	6.00		6.25
RUNOFF VOLUME (mm)=	86.50		41.86
TOTAL RAINFALL (mm)=	88.50		88.50
RUNOFF COEFFICIENT =	0.98		0.47
		TOTALS	18.166 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 64.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5338) IN= 2---> OUT= 1 DT= 15.0 min		OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
		0.0000	0.0000	9.5300	3.8529
		0.6370	1.6350	12.1350	4.2725
		2.7300	2.1009	14.3850	4.6808
		5.1010	2.7445	14.7850	4.6908
		7.0460	3.2936	0.0000	0.0000
		AREA	OPEAK	TPEAK	R.V.

INFLOW : ID= 2 (3381) (ha) (cms) (hrs) (mm)
165.620 18.166 6.00 53.92
OUTFLOW: ID= 1 (5338) 165.620 8.509 6.50 53.91

PEAK FLOW REDUCTION [Qout/Qin] (%) = 46.84
TIME SHIFT OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha.m.) = 3.6821

ADD HYD. (8310) 1 + 2 = 3		AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (3382):		433.29	35.026	6.00	47.59
+ ID2= 2 (5338):		165.62	8.509	6.50	53.91
ID = 3 (8310):		598.91	40.821	6.25	49.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (3342) ID= 1 DT=15.0 min		Area (ha)= 586.87 Ia (mm)= 5.00	Curve Number (CN)= 56.0 # of Linear Res. (N)= 1.50
		U. H. Tp (hrs)= 8.20	

Unit Hyd Opeak (cms) = 1.221

PEAK FLOW (cms) = 1.170 (i)
TIME TO PEAK (hrs) = 15.250
RUNOFF VOLUME (mm) = 24.620
TOTAL RAINFALL (mm) = 88.500
RUNOFF COEFFICIENT = 0.278

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3341) ID= 1 DT=15.0 min		Area Total (ha)= 33.23 Imp(%)= 51.00	Dir. Conn. (%)= 21.00
		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)= 16.95		16.28
Dep. Storage	(mm)= 2.00		5.00
Average Slope	(%)= 0.50		0.50
Length	(m)= 470.67		40.00
Mannings n	= 0.013		0.250
Max. Eff. Inten. (mm/hr)= over (min)	116.82 15.00		76.76 30.00
Storage Coeff. (min)=	7.49 (ii)		19.38 (ii)
Unit Hyd. Tpeak (min)=	15.00		30.00
Unit Hyd. peak (cms)=	0.10		0.05
PEAK FLOW (cms)=	2.06		1.78
TIME TO PEAK (hrs)=	6.00		6.25
RUNOFF VOLUME (mm)=	86.50		31.52
TOTAL RAINFALL (mm)=	88.50		88.50
RUNOFF COEFFICIENT =	0.98		0.36
		TOTALS	3.192 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 51.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5334) IN= 2---> OUT= 1 DT= 15.0 min		OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
		0.0000	0.0000	0.6690	0.4664
		0.0400	0.1927	0.8520	0.5318
		0.1900	0.2643	1.0360	0.5973
		0.3830	0.3576	1.4360	0.6073
		0.4460	0.3863	0.0000	0.0000
		AREA	OPEAK	TPEAK	R.V.

INFLOW : ID= 2 (3341) (ha) (cms) (hrs) (mm)
 33.230 3.192 6.00 43.06
 OUTFLOW: ID= 1 (5334) 33.230 1.677 6.50 43.03

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 52.52
 TIME SHIFT OF PEAK FLOW (mi n) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 0.6222

ADD HYD (8314)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (3342):	586.87	1.170	15.25	24.62
+ ID2= 2 (5334):	33.23	1.677	6.50	43.03
ID = 3 (8314):	620.10	2.040	6.50	25.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0306)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 mi n	283.97	52.0
	U. H. Tp(hrs)= 6.44	# of Linear Res. (N) = 1.50

Unit Hyd Qpeak (cms) = 0.753

PEAK FLOW (cms) = 0.637 (i)
 TIME TO PEAK (hrs) = 13.750
 RUNOFF VOLUME (mm) = 21.914
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.248

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0282)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 mi n	449.38	77.0
	U. H. Tp(hrs)= 1.47	# of Linear Res. (N) = 1.50

Unit Hyd Qpeak (cms) = 5.226

PEAK FLOW (cms) = 6.939 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 43.497
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.491

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0284)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 mi n	78.93	84.0
	U. H. Tp(hrs)= 0.57	# of Linear Res. (N) = 1.50

Unit Hyd Qpeak (cms) = 2.344

PEAK FLOW (cms) = 3.150 (i)
 TIME TO PEAK (hrs) = 6.500
 RUNOFF VOLUME (mm) = 51.645
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.584

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8388)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0282):	449.38	6.939	8.00	43.50
+ ID2= 2 (0284):	78.93	3.150	6.50	51.65
ID = 3 (8388):	528.31	9.369	7.25	44.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6286)
 IN= 2----> OUT= 1 Routing time step (mi n)' = 15.00

<----- DATA FOR SECTION (2861.0) ----->

Distance	Elevation	Manning
0.00	233.00	0.0450
20.58	228.51	0.0450
41.17	227.74	0.0450
51.46	227.41	0.0450
97.77	225.96	0.0450
149.23	223.94	0.0450
200.69	220.84	0.0450
226.42	220.66	0.0450
238.85	220.22	0.0450
241.35	220.01	0.0450 / 0.0350
241.85	219.70	0.0350
245.85	219.72	0.0350
246.35	220.06	0.0350
248.85	220.23	0.0350
303.60	221.64	0.0350
380.79	224.98	0.0450
432.25	229.54	0.0450
457.98	233.33	0.0450
483.71	234.27	0.0450
509.44	233.81	0.0450

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.31	220.01	382E+04	0.8	0.57	82.89
1.06	220.76	525E+05	12.1	0.66	72.51
1.81	221.51	240E+06	88.6	1.05	45.21
2.56	222.26	510E+06	261.7	1.46	32.47
3.31	223.01	843E+06	533.8	1.81	26.34
4.06	223.76	124E+07	913.6	2.10	22.64
4.81	224.51	171E+07	1399.0	2.34	20.33
5.56	225.26	225E+07	2051.0	2.60	18.27
6.31	226.01	285E+07	2907.0	2.91	16.35
7.06	226.76	352E+07	3902.5	3.16	15.04
7.82	227.52	426E+07	5069.9	3.39	14.01
8.57	228.27	507E+07	6433.0	3.62	13.13
9.32	229.02	593E+07	8069.2	3.89	12.24
10.07	229.77	681E+07	9926.7	4.16	11.44
10.82	230.52	772E+07	11962.8	4.42	10.75
11.57	231.27	864E+07	14167.1	4.68	10.17
12.32	232.02	959E+07	16537.1	4.92	9.66
13.07	232.77	105E+08	19070.7	5.16	9.22
13.82	233.52	115E+08	21790.4	5.39	8.82

<---- hydrograph ----> <- pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8388)	528.31	9.37	7.25	44.71	0.88	0.63
OUTFLOW: ID= 1 (6286)	528.31	7.94	9.00	44.71	0.78	0.62

CALIB STANDHYD (2861)	Area (ha)	Curve Number (CN)
ID= 1 DT=15.0 mi n	54.86	45.00
	Total Imp(%) = 45.00	Di r. Conn. (%) = 19.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	24.69	30.17
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	604.76	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr) over (mi n)	116.82	125.98
Storage Coeff. (mi n)	15.00	30.00
Unit Hyd. Tpeak (mi n)	8.70 (ii)	18.45 (ii)
Unit Hyd. peak (cms)	15.00	30.00
	0.09	0.05
PEAK FLOW (cms)	2.97	5.60
TIME TO PEAK (hrs)	6.00	6.25
RUNOFF VOLUME (mm)	86.50	55.31
TOTAL RAINFALL (mm)	88.50	88.50
RUNOFF COEFFICIENT	0.98	0.62

TOTALS

6.750 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5286)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.6700	1.4285
0.1860	0.6703	4.6080	1.6332
1.0630	0.7754	5.3640	1.8336
1.9790	1.0010	5.7640	1.8436
2.7460	1.1950	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2861)	54.860	6.750	6.00	61.24
OUTFLOW: ID= 1 (5286)	54.860	3.503	6.50	61.22

PEAK FLOW REDUCTION [Qout/Qin] (%) = 51.89
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 1.4052

CALIB
 STANDHYD (2862)
 ID= 1 DT=15.0 min

Area (ha) = 205.65
 Total Imp(%) = 22.00
 Dir. Conn. (%) = 10.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	45.24	160.41
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	1170.90	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	116.82	89.07
over (min)	15.00	30.00
Storage Coeff. (min)	12.93 (ii)	24.14 (ii)
Unit Hyd. Tpeak (min)	15.00	30.00
Unit Hyd. peak (cms)	0.08	0.04

	TOTALS		
PEAK FLOW (cms)	5.16	18.61	20.856 (iii)
TIME TO PEAK (hrs)	6.00	6.25	
RUNOFF VOLUME (mm)	86.50	49.65	53.33
TOTAL RAINFALL (mm)	88.50	88.50	88.50
RUNOFF COEFFICIENT	0.98	0.56	0.60

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 79.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9286)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2862):	205.65	20.856	6.25	53.33
+ ID2= 2 (5286):	54.86	3.503	6.50	61.22
-----	-----	-----	-----	-----
ID = 3 (9286):	260.51	23.616	6.25	54.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8386)
 1 + 2 = 3

AREA	OPEAK	TPEAK	R. V.
------	-------	-------	-------

	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (6286):	528.31	7.939	9.00	44.71
+ ID2= 2 (9286):	260.51	23.616	6.25	54.99
-----	-----	-----	-----	-----
ID = 3 (8386):	788.82	25.756	6.25	48.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
 NASHYD (0302)
 ID= 1 DT=15.0 min

Area (ha) = 473.90
 Curve Number (CN) = 58.0
 Ia (mm) = 5.00
 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 1.66

Unit Hyd Opeak (cms)	= 4.874
PEAK FLOW (cms)	= 3.864 (i)
TIME TO PEAK (hrs)	= 8.500
RUNOFF VOLUME (mm)	= 25.946
TOTAL RAINFALL (mm)	= 88.500
RUNOFF COEFFICIENT	= 0.293

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 NASHYD (0298)
 ID= 1 DT=15.0 min

Area (ha) = 330.51
 Curve Number (CN) = 45.0
 Ia (mm) = 5.00
 # of Linear Res. (N) = 1.50
 U. H. Tp(hrs) = 1.26

Unit Hyd Opeak (cms)	= 4.461
PEAK FLOW (cms)	= 2.236 (i)
TIME TO PEAK (hrs)	= 8.000
RUNOFF VOLUME (mm)	= 17.571
TOTAL RAINFALL (mm)	= 88.500
RUNOFF COEFFICIENT	= 0.199

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
 STANDHYD (3001)
 ID= 1 DT=15.0 min

Area (ha) = 0.09
 Total Imp(%) = 45.00
 Dir. Conn. (%) = 15.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	0.04	0.05
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	24.49	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)	116.82	62.09
over (min)	15.00	15.00
Storage Coeff. (min)	1.27 (ii)	14.22 (ii)
Unit Hyd. Tpeak (min)	15.00	15.00
Unit Hyd. peak (cms)	0.11	0.07

	TOTALS		
PEAK FLOW (cms)	0.00	0.01	0.010 (iii)
TIME TO PEAK (hrs)	6.00	6.00	6.00
RUNOFF VOLUME (mm)	86.50	26.79	35.64
TOTAL RAINFALL (mm)	88.50	88.50	88.50
RUNOFF COEFFICIENT	0.98	0.30	0.40

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 46.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5300)
 IN= 2--> OUT= 1
 DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.0060	0.0023

0.0020	0.0012	0.0070	0.0026
0.0030	0.0016	0.0090	0.0029
0.0040	0.0019	0.4090	0.0129

	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3001)	0.090	0.010	6.00	35.64
OUTFLOW: ID= 1 (5300)	0.090	0.002	6.50	34.12

PEAK FLOW REDUCTION [Qout/Qin] (%) = 23.46
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 0.0014

CALIB NASHYD (3002) ID= 1 DT=15.0 min	Area (ha) = 258.84 Ia (mm) = 5.00 U. H. Tp(hrs) = 1.03	Curve Number (CN) = 51.8 # of Linear Res. (N) = 1.50
---	--	---

Unit Hyd Qpeak (cms) = 4.290
 PEAK FLOW (cms) = 2.520 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 21.586
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.244

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9300) 1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3002):	258.84	2.520	7.50	21.59
+ ID2= 2 (5300):	0.09	0.002	6.50	34.12
=====				
ID = 3 (9300):	258.93	2.522	7.50	21.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8395) 1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0298):	330.51	2.236	8.00	17.57
+ ID2= 2 (9300):	258.93	2.522	7.50	21.59
=====				
ID = 3 (8395):	589.44	4.729	7.75	19.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0288) ID= 1 DT=15.0 min	Area (ha) = 340.83 Ia (mm) = 5.00 U. H. Tp(hrs) = 2.21	Curve Number (CN) = 78.0 # of Linear Res. (N) = 1.50
---	--	---

Unit Hyd Qpeak (cms) = 2.629
 PEAK FLOW (cms) = 3.883 (i)
 TIME TO PEAK (hrs) = 9.250
 RUNOFF VOLUME (mm) = 44.801
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.506

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0290) ID= 1 DT=15.0 min	Area (ha) = 269.18 Ia (mm) = 5.00 U. H. Tp(hrs) = 1.07	Curve Number (CN) = 78.0 # of Linear Res. (N) = 1.50
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Unit Hyd Qpeak (cms) = 4.279
 PEAK FLOW (cms) = 5.447 (i)
 TIME TO PEAK (hrs) = 7.250

RUNOFF VOLUME (mm) = 44.529
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.503

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8397) 1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0288):	340.83	3.883	9.25	44.80
+ ID2= 2 (0290):	269.18	5.447	7.25	44.53
=====				
ID = 3 (8397):	610.01	9.010	8.00	44.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0296) ID= 1 DT=15.0 min	Area (ha) = 293.65 Ia (mm) = 5.00 U. H. Tp(hrs) = 1.13	Curve Number (CN) = 76.0 # of Linear Res. (N) = 1.50
---	--	---

Unit Hyd Qpeak (cms) = 4.437
 PEAK FLOW (cms) = 5.388 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 42.226
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.477

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0292) ID= 1 DT=15.0 min	Area (ha) = 738.49 Ia (mm) = 5.00 U. H. Tp(hrs) = 1.52	Curve Number (CN) = 68.0 # of Linear Res. (N) = 1.50
---	--	---

Unit Hyd Qpeak (cms) = 8.289
 PEAK FLOW (cms) = 8.591 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 34.153
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.386

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0294) ID= 1 DT=15.0 min	Area (ha) = 274.15 Ia (mm) = 5.00 U. H. Tp(hrs) = 0.87	Curve Number (CN) = 57.0 # of Linear Res. (N) = 1.50
---	--	---

Unit Hyd Qpeak (cms) = 5.367
 PEAK FLOW (cms) = 3.559 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 25.026
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.283

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8398) 1 + 2 = 3	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0292):	738.49	8.591	8.00	34.15
+ ID2= 2 (0294):	274.15	3.559	7.00	25.03
=====				
ID = 3 (8398):	1012.64	11.892	8.00	31.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6296)
IN= 2----> OUT= 1

Routing time step (mi n)'= 15.00

<----- DATA FOR SECTION (2961.0) ----->

Distance	Elevation	Manning	
0.00	243.98	0.0400	
42.59	243.18	0.0400	
85.17	241.81	0.0400	
120.66	240.50	0.0400	
156.15	239.56	0.0400	
198.74	236.15	0.0400	
237.78	234.01	0.0400	
241.33	233.82	0.0400	
248.77	233.12	0.0400 /0.0400	Main Channel
249.87	232.32	0.0400	Main Channel
250.37	231.80	0.0400	Main Channel
250.87	232.23	0.0400	Main Channel
251.97	233.10	0.0400 /0.0400	Main Channel
255.37	233.22	0.0400	
259.07	233.87	0.0400	
262.62	234.12	0.0400	
266.17	234.23	0.0400	
283.91	234.73	0.0400	
337.15	241.75	0.0400	
351.34	244.00	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.43	232.23	.510E+03	0.1	0.53	81.07
0.87	232.67	.213E+04	0.7	0.83	51.49
1.30	233.10	.501E+04	2.2	1.10	38.75
1.98	233.78	.242E+05	12.6	1.34	31.93
2.66	234.46	.738E+05	42.1	1.46	29.21
3.34	235.14	.177E+06	127.9	1.84	23.13
4.02	235.82	.314E+06	281.4	2.29	18.61
4.70	236.50	.481E+06	509.4	2.71	15.73
5.38	237.18	.672E+06	815.6	3.11	13.73
6.06	237.86	.886E+06	1200.7	3.46	12.30
6.74	238.54	.112E+07	1669.4	3.80	11.23
7.42	239.22	.139E+07	2226.9	4.11	10.38
8.10	239.90	.168E+07	2808.4	4.28	9.95
8.78	240.58	.202E+07	3470.4	4.40	9.68
9.46	241.26	.240E+07	4331.7	4.61	9.24
10.14	241.94	.283E+07	5335.2	4.83	8.84
10.82	242.62	.330E+07	6482.6	5.03	8.48
11.50	243.30	.381E+07	7764.1	5.21	8.18
12.18	243.98	.439E+07	9101.2	5.31	8.04

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8398)	1012.64	11.89	8.00	31.68	1.93	1.32
OUTFLOW: ID= 1 (6296)	1012.64	11.62	8.25	31.68	1.91	1.31

ADD HYD (8396)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0296):	293.65	5.388	7.50	42.23
+ ID2= 2 (6296):	1012.64	11.616	8.25	31.68
ID = 3 (8396):	1306.29	16.780	8.00	34.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8394)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8396):	1306.29	16.780	8.00	34.05
+ ID2= 2 (8397):	610.01	9.010	8.00	44.68
ID = 3 (8394):	1916.30	25.790	8.00	37.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8392)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8394):	1916.30	25.790	8.00	37.44
+ ID2= 2 (8395):	589.44	4.729	7.75	19.34
ID = 3 (8392):	2505.74	30.497	8.00	33.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6302)
IN= 2----> OUT= 1

Routing time step (mi n)'= 15.00

<----- DATA FOR SECTION (3021.0) ----->

Distance	Elevation	Manning	
0.00	228.10	0.0400	
18.47	227.12	0.0400	
36.95	226.12	0.0400	
46.18	225.84	0.0400	
55.42	225.58	0.0400	
272.47	222.88	0.0400	
277.09	222.76	0.0400	
281.71	222.58	0.0400	
288.54	222.18	0.0400 /0.0300	Main Channel
288.64	221.00	0.0300	Main Channel
290.94	221.00	0.0300	Main Channel
291.04	221.00	0.0300	Main Channel
291.54	222.75	0.0300 /0.0400	Main Channel
300.18	222.83	0.0400	
304.80	223.04	0.0400	
309.42	223.25	0.0400	
318.65	223.69	0.0400	
360.22	225.57	0.0400	
397.16	227.60	0.0400	
457.20	228.35	0.0400	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.29	221.29	.228E+04	0.2	0.32	165.63
0.59	221.59	.465E+04	0.7	0.45	116.15
0.88	221.88	.713E+04	1.2	0.54	96.87
1.18	222.18	.971E+04	1.9	0.61	86.20
1.57	222.57	.175E+05	3.3	0.59	88.86
1.97	222.97	.427E+05	6.4	0.47	111.39
2.36	223.36	.115E+06	16.1	0.44	119.14
2.76	223.76	.237E+06	37.1	0.49	106.49
3.15	224.15	.409E+06	72.9	0.56	93.40
3.55	224.55	.631E+06	127.0	0.63	82.76
3.94	224.94	.903E+06	202.4	0.70	74.37
4.34	225.34	.123E+07	301.8	0.77	67.68
4.73	225.73	.160E+07	433.5	0.85	61.37
5.13	226.13	.200E+07	602.2	0.95	55.29
5.52	226.52	.242E+07	804.7	1.05	50.13
5.92	226.92	.286E+07	1034.2	1.14	46.11
6.31	227.31	.332E+07	1290.5	1.22	42.88
6.71	227.71	.380E+07	1560.9	1.29	40.55
7.10	228.10	.432E+07	1835.6	1.34	39.19

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8392)	2505.74	30.50	8.00	33.18	2.64	0.47
OUTFLOW: ID= 1 (6302)	2505.74	24.10	10.25	33.18	2.51	0.46

ADD HYD (8390)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0302):	473.90	3.864	8.50	25.95
+ ID2= 2 (6302):	2505.74	24.104	10.25	33.18
ID = 3 (8390):	2979.64	27.727	10.00	32.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8386):	788.82	25.756	6.25	48.11
+ ID2= 2 (8390):	2979.64	27.727	10.00	32.03
=====				
ID = 3 (8348):	3768.46	37.313	9.75	35.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
(0304)	292.37	63.0	1.50
U.H. Tp(hrs)=	5.00		
	2.78		

Unit Hyd Qpeak (cms) = 1.793

PEAK FLOW (cms) = 1.844 (i)
 TIME TO PEAK (hrs) = 10.250
 RUNOFF VOLUME (mm) = 29.899
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.338

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
(0280)	299.86	82.0	1.50
U.H. Tp(hrs)=	5.00		
	0.85		

Unit Hyd Qpeak (cms) = 6.009

PEAK FLOW (cms) = 8.228 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 49.420
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.558

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
(0278)	485.49	82.0	1.50
U.H. Tp(hrs)=	5.00		
	1.52		

Unit Hyd Qpeak (cms) = 5.453

PEAK FLOW (cms) = 8.411 (i)
 TIME TO PEAK (hrs) = 8.000
 RUNOFF VOLUME (mm) = 49.794
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.563

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)	# of Linear Res. (N)
(0276)	90.89	79.0	1.50
U.H. Tp(hrs)=	5.00		
	0.67		

Unit Hyd Qpeak (cms) = 2.302

PEAK FLOW (cms) = 2.744 (i)
 TIME TO PEAK (hrs) = 6.750
 RUNOFF VOLUME (mm) = 45.323
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.512

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD	Area (ha)	Curve Number (CN)
(0274)	392.49	75.0

ID= 1 DT=15.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 1.08

Unit Hyd Qpeak (cms) = 6.182

PEAK FLOW (cms) = 7.225 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 41.085
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.464

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0274):	392.49	7.225	7.25	41.09
+ ID2= 2 (0276):	90.89	2.744	6.75	45.32
=====				
ID = 3 (8360):	483.38	9.821	7.00	41.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0278):	485.49	8.411	8.00	49.79
+ ID2= 2 (8360):	483.38	9.821	7.00	41.88
=====				
ID = 3 (8358):	968.87	17.878	7.50	45.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6280) | Routing time step (min) = 15.00
 IN= 2 -> OUT= 1

<----- DATA FOR SECTION (2801,0) ----->		
Distance	Elevation	Manning
0.00	241.14	0.0500
13.32	240.80	0.0500
39.95	240.07	0.0500
96.54	236.21	0.0500
113.19	234.15	0.0500
123.18	232.35	0.0500
143.15	225.80	0.0500
149.81	225.62	0.0500
153.14	225.40	0.0500
157.30	224.76	0.0500 / 0.0300
159.80	224.26	0.0300
162.30	224.85	0.0300
162.55	224.97	0.0300 / 0.0500
167.80	225.05	0.0500
186.43	229.14	0.0500
213.06	234.75	0.0500
236.37	237.09	0.0500
266.33	237.31	0.0500
292.96	237.83	0.0500
329.58	241.50	0.0500

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	224.76	.263E+04	0.8	0.65	58.31
1.36	225.62	.278E+05	13.4	1.10	34.54
2.22	226.48	.855E+05	52.6	1.40	27.09
3.09	227.35	.157E+06	120.4	1.74	21.78
3.95	228.21	.242E+06	216.3	2.04	18.64
4.81	229.07	.340E+06	341.8	2.29	16.56
5.67	229.93	.450E+06	498.3	2.52	15.06
6.53	230.79	.574E+06	688.3	2.73	13.90
7.40	231.66	.711E+06	913.8	2.93	12.97
8.26	232.52	.861E+06	1173.4	3.11	12.23
9.12	233.38	.103E+07	1461.3	3.24	11.72
9.98	234.24	.121E+07	1793.3	3.37	11.26
10.85	235.11	.142E+07	2141.0	3.45	11.02
11.71	235.97	.165E+07	2530.4	3.50	10.86

12.57	236.83	.192E+07	2954.0	3.51	10.81
13.43	237.69	.226E+07	3160.3	3.18	11.93
14.29	238.55	.271E+07	3869.4	3.26	11.67
15.16	239.42	.320E+07	4740.3	3.38	11.24
16.02	240.28	.373E+07	5688.4	3.48	10.92

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8358)	968.87	17.88	7.50	45.85	1.46	1.13
OUTFLOW: ID= 1 (6280)	968.87	17.31	8.25	45.85	1.45	1.12

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0280):	299.86	8.228	7.00	49.42
+ ID2= 2 (6280):	968.87	17.313	8.25	45.85
ID = 3 (8354):	1268.73	24.661	7.75	46.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0270) ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)
	243.61	81.0
	Ia (mm)= 5.00	# of Linear Res. (N)= 1.30
	U.H. Tp(hrs)= 0.87	

Unit Hyd Qpeak (cms)=	3.429
PEAK FLOW (cms)=	4.722 (i)
TIME TO PEAK (hrs)=	7.250
RUNOFF VOLUME (mm)=	48.074
TOTAL RAINFALL (mm)=	88.500
RUNOFF COEFFICIENT =	0.543

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0268) ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)
	215.76	75.0
	Ia (mm)= 5.00	# of Linear Res. (N)= 1.30
	U.H. Tp(hrs)= 0.69	

Unit Hyd Qpeak (cms)=	3.807
PEAK FLOW (cms)=	4.211 (i)
TIME TO PEAK (hrs)=	7.000
RUNOFF VOLUME (mm)=	40.714
TOTAL RAINFALL (mm)=	88.500
RUNOFF COEFFICIENT =	0.460

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0266) ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)
	508.09	64.0
	Ia (mm)= 5.00	# of Linear Res. (N)= 1.30
	U.H. Tp(hrs)= 1.63	

Unit Hyd Qpeak (cms)=	3.786
PEAK FLOW (cms)=	3.687 (i)
TIME TO PEAK (hrs)=	9.000
RUNOFF VOLUME (mm)=	30.617
TOTAL RAINFALL (mm)=	88.500
RUNOFF COEFFICIENT =	0.346

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (2641) ID= 1 DT=15.0 min	Area (ha)	Total Imp(%)	Dir. Conn. (%)
	17.58	75.00	70.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)=	13.18	4.39
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	342.34	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr)=	116.82	69.21
Storage over (min)=	15.00	30.00
Storage Coeff. (min)=	6.18 (ii)	18.58 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.10	0.05

PEAK FLOW (cms)=	3.76	0.44	*TOTALS*
TIME TO PEAK (hrs)=	6.00	6.25	4.044 (iii)
RUNOFF VOLUME (mm)=	86.50	37.62	6.00
TOTAL RAINFALL (mm)=	88.50	88.50	71.84
RUNOFF COEFFICIENT =	0.98	0.43	88.50
			0.81

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 66.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5264) IN= 2---> OUT= 1 DT= 15.0 min	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
	0.0000	0.0000	1.1760	0.5589
	0.0600	0.3133	1.4770	0.6054
	0.3410	0.3763	1.7190	0.6511
	0.6340	0.4336	2.1190	0.6611
	0.8800	0.4941	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW : ID= 2 (2641)	17.580	4.044	6.00	71.84
OUTFLOW: ID= 1 (5264)	17.580	1.572	6.25	71.76

PEAK FLOW REDUCTION [Qout/Qin] (%) = 38.87
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha.m.) = 0.6369

CALIB NASHYD (2642) ID= 1 DT=15.0 min	Area (ha)	Curve Number (CN)
	336.37	69.0
	Ia (mm)= 5.00	# of Linear Res. (N)= 1.30
	U.H. Tp(hrs)= 1.28	

Unit Hyd Qpeak (cms)=	3.192
PEAK FLOW (cms)=	3.410 (i)
TIME TO PEAK (hrs)=	8.000
RUNOFF VOLUME (mm)=	34.996
TOTAL RAINFALL (mm)=	88.500
RUNOFF COEFFICIENT =	0.395

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (2642):	336.37	3.410	8.00	35.00
+ ID2= 2 (5264):	17.58	1.572	6.25	71.76
ID = 3 (9264):	353.95	4.149	6.50	36.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0266):	508.09	3.687	9.00	30.62

+ ID2= 2 (9264): 353.95 4.149 6.50 36.82
 ID = 3 (8380): 862.04 7.383 8.00 33.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6268) | Routing time step (min)' = 15.00
 IN= 2--> OUT= 1

<----- DATA FOR SECTION (2681.0) ----->

Distance	Elevation	Manning		
0.00	274.03	0.0500		
10.55	272.73	0.0500		
21.10	271.28	0.0500		
50.12	266.83	0.0500		
87.04	260.36	0.0500		
92.32	260.06	0.0500		
94.95	259.93	0.0500		
101.00	256.87	0.0500		
103.00	256.58	0.0500 / 0.0350	Main	Channel
104.00	256.32	0.0350	Main	Channel
106.00	256.05	0.0350	Main	Channel
108.00	256.25	0.0350	Main	Channel
110.78	256.65	0.0350 / 0.0500	Main	Channel
113.42	256.81	0.0500		
116.06	257.00	0.0500		
155.62	260.24	0.0500		
187.27	263.35	0.0500		
211.01	267.22	0.0500		
224.20	269.60	0.0500		
261.13	272.68	0.0500		

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.53	256.58	.697E+04	2.1	0.97	55.64
1.38	257.43	.477E+05	25.9	1.76	30.66
2.22	258.27	.123E+06	84.3	2.22	24.26
3.07	259.12	.231E+06	185.4	2.60	20.72
3.92	259.97	.371E+06	335.8	2.92	18.42
4.77	260.82	.560E+06	547.6	3.15	17.06
5.61	261.66	.790E+06	845.6	3.46	15.57
6.46	262.51	1.06E+07	1225.0	3.75	14.37
7.31	263.36	.136E+07	1693.3	4.02	13.37
8.16	264.21	.169E+07	2283.2	4.35	12.36
9.00	265.05	.206E+07	2969.1	4.66	11.54
9.85	265.90	.245E+07	3754.4	4.96	10.86
10.70	266.75	.286E+07	4643.0	5.24	10.28
11.55	267.60	.331E+07	5636.1	5.50	9.78
12.39	268.44	.378E+07	6746.2	5.76	9.34
13.24	269.29	.428E+07	7972.2	6.01	8.96
14.09	270.14	.482E+07	9218.9	6.18	8.71
14.94	270.99	.539E+07	10550.5	6.32	8.52
15.78	271.83	.601E+07	12024.7	6.46	8.33

<----- hydrograph -----> <-pi pe / channel-->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8380)	862.04	7.38	8.00	33.16	0.72	1.07
OUTFLOW: ID= 1 (6268)	862.04	7.13	9.25	33.16	0.71	1.07

ADD HYD (8382) | 1 + 2 = 3 | AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 ID1= 1 (0268): 215.76 4.211 7.00 40.71
 + ID2= 2 (6268): 862.04 7.127 9.25 33.16
 ID = 3 (8382): 1077.80 10.583 8.25 34.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6270) | Routing time step (min)' = 15.00
 IN= 2--> OUT= 1

<----- DATA FOR SECTION (2701.0) ----->

Distance	Elevation	Manning
----------	-----------	---------

0.00	245.98	0.0500		
8.14	245.66	0.0500		
16.28	245.16	0.0500		
20.35	244.84	0.0500		
38.66	242.98	0.0500		
48.83	240.65	0.0500		
63.07	235.91	0.0500		
65.11	235.18	0.0500		
68.43	234.34	0.0500 / 0.0300	Main	Channel
68.68	233.95	0.0300	Main	Channel
69.18	233.89	0.0300	Main	Channel
69.68	233.95	0.0300	Main	Channel
71.21	234.48	0.0300 / 0.0500	Main	Channel
81.38	236.44	0.0500		
95.63	236.66	0.0500		
120.04	237.00	0.0500		
148.53	241.77	0.0500		
158.70	242.34	0.0500		
187.18	244.03	0.0500		
201.42	244.36	0.0500		

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.45	234.34	.217E+04	0.7	0.99	52.83
0.98	234.87	.966E+04	4.6	1.49	35.20
1.50	235.39	.250E+05	13.7	1.73	30.41
2.03	235.92	.475E+05	29.5	1.95	26.87
2.56	236.45	.772E+05	52.5	2.14	24.50
3.09	236.98	.142E+06	82.4	1.83	28.73
3.61	237.50	.246E+06	158.1	2.03	25.91
4.14	238.03	.357E+06	262.0	2.31	22.73
4.67	238.56	.477E+06	392.1	2.59	20.27
5.20	239.09	.604E+06	547.8	2.86	18.38
5.72	239.61	.739E+06	729.2	3.11	16.90
6.25	240.14	.882E+06	936.5	3.34	15.70
6.78	240.67	.103E+07	1169.7	3.57	14.72
7.31	241.20	.119E+07	1425.9	3.77	13.94
7.83	241.72	.136E+07	1710.4	3.96	13.26
8.36	242.25	.154E+07	1967.9	4.02	13.07
8.89	242.78	.174E+07	2264.9	4.09	12.83
9.42	243.31	.196E+07	2589.3	4.15	12.65
9.94	243.83	.221E+07	2953.5	4.22	12.46

<----- hydrograph -----> <-pi pe / channel-->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8382)	1077.80	10.58	8.25	34.68	1.32	1.64
OUTFLOW: ID= 1 (6270)	1077.80	10.39	8.75	34.68	1.31	1.63

ADD HYD (8384) | 1 + 2 = 3 | AREA (ha) OPEAK (cms) TPEAK (hrs) R. V. (mm)
 ID1= 1 (0270): 243.61 4.722 7.25 48.07
 + ID2= 2 (6270): 1077.80 10.387 8.75 34.68
 ID = 3 (8384): 1321.41 14.652 8.50 37.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0262) | Area (ha) = 341.31 Curve Number (CN) = 82.0
 ID= 1 DT=15.0 min | Ia (mm) = 5.00 # of Linear Res. (N) = 1.30
 U. H. Tp(hrs) = 1.01

Unit Hyd Opeak (cms) = 4.094
 PEAK FLOW (cms) = 5.995 (i)
 TIME TO PEAK (hrs) = 7.500
 RUNOFF VOLUME (mm) = 49.519
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.560

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB

NASHYD (0258) Area (ha)= 181.99 Curve Number (CN)= 79.0
 ID= 1 DT=15.0 min I a (mm)= 5.00 # of Linear Res. (N)= 1.30
 U. H. Tp(hrs)= 1.18

Uni t Hyd Qpeak (cms)= 1.881

PEAK FLOW (cms)= 2.620 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 45.751
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.517

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0256) Area (ha)= 145.79 Curve Number (CN)= 67.0
 ID= 1 DT=15.0 min I a (mm)= 5.00 # of Linear Res. (N)= 1.30
 U. H. Tp(hrs)= 1.04

Uni t Hyd Qpeak (cms)= 1.707

PEAK FLOW (cms)= 1.643 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 33.068
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.374

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0254) Area (ha)= 403.00 Curve Number (CN)= 55.0
 ID= 1 DT=15.0 min I a (mm)= 5.00 # of Linear Res. (N)= 1.30
 U. H. Tp(hrs)= 1.22

Uni t Hyd Qpeak (cms)= 4.028

PEAK FLOW (cms)= 2.850 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 23.726
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.268

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6256) Routing time step (min)'= 15.00
 IN= 2--> OUT= 1

<----- DATA FOR SECTION (2561.0) ----->			
Distance	Elevation	Manning	
0.00	276.07	0.0400	
11.68	273.71	0.0400	
23.36	271.35	0.0400	
29.19	270.30	0.0400	
35.03	269.44	0.0400	
55.47	267.90	0.0400	
78.82	266.24	0.0400	
90.50	265.63	0.0400	
102.18	265.40	0.0400	
105.10	264.95	0.0400 / 0.0350	Main Channel
108.02	264.39	0.0350	Main Channel
110.94	264.72	0.0350	Main Channel
113.86	265.19	0.0350 / 0.0400	Main Channel
116.78	265.49	0.0400	
143.05	268.24	0.0400	
172.25	270.53	0.0400	
207.28	271.95	0.0400	
233.55	273.72	0.0400	
256.91	274.98	0.0400	
289.02	275.97	0.0400	

<----- TRAVEL TIME TABLE ----->					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.56	264.95	.723E+04	1.9	0.90	62.51
1.14	265.53	.307E+05	13.5	1.49	37.92
1.72	266.11	.951E+05	48.6	1.73	32.62
2.30	266.69	.191E+06	117.8	2.09	27.03
2.88	267.27	.314E+06	225.2	2.43	23.24

3.46	267.85	.464E+06	375.5	2.74	20.59
4.04	268.43	.641E+06	573.2	3.03	18.62
4.62	269.01	.846E+06	821.9	3.29	17.15
5.20	269.59	.108E+07	1135.0	3.56	15.87
5.78	270.17	.134E+07	1525.1	3.86	14.64
6.36	270.75	.162E+07	1971.1	4.12	13.71
6.94	271.33	.194E+07	2474.0	4.33	13.04
7.52	271.91	.228E+07	3065.0	4.55	12.42
8.10	272.49	.266E+07	3773.4	4.80	11.75
8.68	273.07	.306E+07	4566.3	5.05	11.17
9.26	273.65	.348E+07	5443.7	5.30	10.66
9.84	274.23	.393E+07	6389.6	5.51	10.25
10.42	274.81	.440E+07	7426.2	5.72	9.88
11.00	275.39	.490E+07	8490.1	5.86	9.63

<---- hydrograph ---->							<-pi pe / channel ->	
INFLOW : ID= 2 (0254)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)		
OUTFLOW: ID= 1 (6256)	403.00	2.85	8.00	23.73	0.61	0.93		
	403.00	2.68	9.75	23.73	0.60	0.93		

ADD HYD (8370)					
ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (0256):	145.79	1.643	8.00	33.07	
+ ID2= 2 (6256):	403.00	2.683	9.75	23.73	

ID = 3 (8370):	548.79	4.183	9.00	26.21	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0250) Area (ha)= 192.88 Curve Number (CN)= 70.0
 ID= 1 DT=15.0 min I a (mm)= 5.00 # of Linear Res. (N)= 1.30
 U. H. Tp(hrs)= 1.22

Uni t Hyd Qpeak (cms)= 1.930

PEAK FLOW (cms)= 2.099 (i)
 TIME TO PEAK (hrs)= 8.000
 RUNOFF VOLUME (mm)= 35.932
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.406

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0246) Area (ha)= 759.61 Curve Number (CN)= 55.0
 ID= 1 DT=15.0 min I a (mm)= 5.00 # of Linear Res. (N)= 1.30
 U. H. Tp(hrs)= 1.81

Uni t Hyd Qpeak (cms)= 5.121

PEAK FLOW (cms)= 3.958 (i)
 TIME TO PEAK (hrs)= 10.000
 RUNOFF VOLUME (mm)= 23.809
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.269

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0248) Area (ha)= 146.04 Curve Number (CN)= 64.0
 ID= 1 DT=15.0 min I a (mm)= 5.00 # of Linear Res. (N)= 1.30
 U. H. Tp(hrs)= 0.78

Uni t Hyd Qpeak (cms)= 2.271

PEAK FLOW (cms)= 1.874 (i)
 TIME TO PEAK (hrs)= 7.250
 RUNOFF VOLUME (mm)= 30.328
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.343

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0246):	759.61	3.958	10.00	23.81
+ ID2= 2 (0248):	146.04	1.874	7.25	30.33

ID = 3 (8364):	905.65	5.601	8.25	24.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6250)
IN= 2---> OUT= 1 Routing time step (min)' = 15.00

<----- DATA FOR SECTION (2501.0) ----->

Distance	Elevation	Manning	
0.00	269.96	0.0500	
8.56	268.55	0.0500	
17.13	266.91	0.0500	
21.41	266.13	0.0500	
40.68	263.15	0.0500	
62.09	260.75	0.0500	
85.64	258.02	0.0500	
88.20	257.69	0.0500	
93.20	257.05	0.0500 / 0.0350	Main Channel
93.45	256.88	0.0350	Main Channel
94.20	256.56	0.0350	Main Channel
94.95	256.83	0.0350	Main Channel
95.20	257.08	0.0350 / 0.0500	Main Channel
100.62	257.45	0.0500	
115.61	258.57	0.0500	
139.16	260.43	0.0500	
152.01	261.95	0.0500	
171.27	264.63	0.0500	
188.40	267.90	0.0500	
211.95	274.18	0.0500	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.49	257.05	.205E+04	0.4	0.68	89.65
1.17	257.73	.248E+05	5.7	0.85	72.16
1.85	258.41	.841E+05	25.2	1.10	55.60
2.53	259.09	.180E+06	66.5	1.36	45.14
3.21	259.77	.312E+06	135.7	1.60	38.36
3.89	260.45	.481E+06	238.3	1.82	33.61
4.57	261.13	.681E+06	383.5	2.07	29.60
5.25	261.81	.911E+06	568.9	2.29	26.70
5.93	262.49	.117E+07	800.5	2.51	24.36
6.61	263.17	.146E+07	1079.0	2.72	22.49
7.28	263.84	.177E+07	1413.4	2.94	20.85
7.96	264.52	.210E+07	1797.0	3.14	19.50
8.64	265.20	.246E+07	2241.6	3.35	18.28
9.32	265.88	.283E+07	2739.9	3.55	17.25
10.00	266.56	.323E+07	3295.7	3.75	16.34
10.68	267.24	.364E+07	3908.9	3.94	15.54
11.36	267.92	.408E+07	4578.5	4.12	14.84
12.04	268.60	.453E+07	5320.1	4.32	14.18
12.72	269.28	.499E+07	6109.1	4.50	13.61

<----- hydrograph ----->

<- pi pe / channel ->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8364)	905.65	5.60	8.25	24.86	1.15	0.84
OUTFLOW: ID= 1 (6250)	905.65	5.25	10.25	24.86	1.11	0.83

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0250):	192.88	2.099	8.00	35.93
+ ID2= 2 (6250):	905.65	5.247	10.25	24.86

ID = 3 (8366):	1098.53	7.167	9.75	26.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8366):	1098.53	7.167	9.75	26.80
+ ID2= 2 (8370):	548.79	4.183	9.00	26.21

ID = 3 (8368):	1647.32	11.318	9.50	26.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB
STANDHYD (2521)
ID= 1 DT=15.0 min Area Total (ha)= 32.51 Imp(%)= 75.00 Di r. Conn.(%)= 70.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	24.38	8.13
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	465.55	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	116.82	76.24
over (min)=	15.00	30.00
Storage Coeff. (min)=	7.44 (ii)	19.36 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.10	0.05
PEAK FLOW (cms)=	6.72	0.89
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	86.50	41.23
TOTAL RAINFALL (mm)=	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.47

TOTALS
7.297 (iii)
6.00
72.92
88.50
0.82

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 70.6 Ia = Dep Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5252)
IN= 2---> OUT= 1
DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	2.1750	1.0334
	0.1100	0.5793	2.7310	1.1195
	0.6300	0.6958	3.1790	1.2039
	1.1730	0.8017	3.5790	1.2139
	1.6270	0.9137	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (2521)	32.510	7.297	6.00	72.92
OUTFLOW: ID= 1 (5252)	32.510	2.887	6.25	72.88

PEAK FLOW REDUCTION [Qout/Qin] (%) = 39.56
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha. m.) = 1.1594

CALIB
NASHYD (2522)
ID= 1 DT=15.0 min Area (ha)= 287.47 Curve Number (CN)= 71.5
Ia (mm)= 5.00 # of Linear Res. (N)= 1.30
U. H. Tp(hrs)= 1.01

Unit Hyd Opeak (cms)=	3.448
PEAK FLOW (cms)=	3.743 (i)
TIME TO PEAK (hrs)=	7.750
RUNOFF VOLUME (mm)=	37.327
TOTAL RAINFALL (mm)=	88.500
RUNOFF COEFFICIENT =	0.422

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9252)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (2522):	287.47	3.743	7.75	37.33
+ ID2= 2 (5252):	32.51	2.887	6.25	72.88

ID = 3 (9252):	319.98	5.902	6.50	40.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8372)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8368):	1647.32	11.318	9.50	26.61
+ ID2= 2 (9252):	319.98	5.902	6.50	40.94

ID = 3 (8372):	1967.30	15.109	8.75	28.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6258)
IN= 2---> OUT= 1 Routing time step (min)'= 15.00

<----- DATA FOR SECTION (2581.0) ----->

Distance	Elevation	Manning	
0.00	252.88	0.0500	
15.47	251.89	0.0500	
46.41	248.45	0.0500	
73.48	245.81	0.0500	
88.95	243.75	0.0500	
112.15	242.00	0.0500	
135.35	240.23	0.0500	
162.42	239.76	0.0500	
170.97	239.52	0.0500 / 0.0350	Main Channel
171.58	239.03	0.0350	Main Channel
174.02	239.03	0.0350	Main Channel
176.46	239.03	0.0350	Main Channel
177.07	239.52	0.0350 / 0.0500	Main Channel
185.63	239.67	0.0500	
208.83	239.87	0.0500	
239.77	240.14	0.0500	
274.57	244.93	0.0500	
336.45	249.51	0.0500	
363.52	249.77	0.0500	
382.85	251.78	0.0500	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.49	239.52	.135E+05	2.5	0.92	91.32
1.14	240.17	.175E+06	25.6	0.73	114.22
1.78	240.81	.532E+06	123.5	1.17	71.85
2.43	241.46	.932E+06	285.3	1.54	54.48
3.07	242.10	.138E+07	506.5	1.85	45.25
3.72	242.75	.186E+07	786.8	2.12	39.42
4.36	243.39	.239E+07	1127.3	2.37	35.33
5.01	244.04	.296E+07	1535.8	2.61	32.12
5.65	244.68	.356E+07	2014.3	2.84	29.48
6.30	245.33	.420E+07	2535.9	3.03	27.60
6.94	245.97	.488E+07	3108.4	3.20	26.16
7.59	246.62	.561E+07	3740.6	3.35	24.98
8.23	247.26	.638E+07	4446.7	3.50	23.93
8.88	247.91	.721E+07	5229.7	3.64	22.98
9.52	248.55	.809E+07	6094.3	3.78	22.12
10.17	249.20	.901E+07	7050.4	3.93	21.31
10.81	249.84	.100E+08	7773.0	3.90	21.46
11.46	250.49	.111E+08	8998.8	4.07	20.56
12.10	251.13	.122E+08	10318.2	4.24	19.75

<--- hydrograph ---> <--- pipe / channel --->

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX. DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8372)	1967.30	15.11	8.75	28.94	0.84	0.81
OUTFLOW: ID= 1 (6258)	1967.30	13.58	11.00	28.94	0.80	0.82

ADD HYD (8374)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0258):	181.99	2.620	8.00	45.75
+ ID2= 2 (6258):	1967.30	13.577	11.00	28.94

ID = 3 (8374):	2149.29	15.667	10.75	30.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (2601)			
ID= 1 DT=15.0 min			
Area Total	(ha)	Imp(%)	Di r. Conn. (%)
	56.22	65.00	48.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	36.54	19.68
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	612.21	40.00
Mannings n =	0.013	0.250
Max. Eff. Inten. (mm/hr)=	116.82	134.20
over (min)=	15.00	30.00
Storage Coeff. (min)=	8.77 (ii)	18.28 (ii)
Unit Hyd. Tpeak (min)=	15.00	30.00
Unit Hyd. peak (cms)=	0.09	0.05
PEAK FLOW (cms)=	7.67	3.92
TIME TO PEAK (hrs)=	6.00	6.25
RUNOFF VOLUME (mm)=	86.50	58.48
TOTAL RAINFALL (mm)=	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.66
TOTALS		10.343 (iii)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 81.5 Ia = Dep Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5260)
IN= 2---> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	3.7610	1.6799
0.1900	0.8974	4.7220	1.8499
1.0890	1.0678	5.4970	2.0146
2.0290	1.2667	5.8970	2.0246
2.8140	1.4621	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2601)	56.220	10.343	6.00	71.93
OUTFLOW: ID= 1 (5260)	56.220	4.470	6.50	71.91

PEAK FLOW REDUCTION [Out/Oi n] (%) = 43.22
TIME SHIF T OF PEAK FLOW (min) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 1.8379

CALIB NASHYD (2602)			
ID= 1 DT=15.0 min			
Area	(ha)	Curve Number (CN)	# of Linear Res. (N)
Ia	420.02	83.3	1.30
U. H. Tp	(mm)= 5.00		
	(hrs)= 1.30		

Unit Hyd Qpeak (cms)=	3.946
PEAK FLOW (cms)=	6.339 (i)
TIME TO PEAK (hrs)=	8.000
RUNOFF VOLUME (mm)=	51.454
TOTAL RAINFALL (mm)=	88.500
RUNOFF COEFFICIENT =	0.581

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9260)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (2602):	420.02	6.339	8.00	51.45	
+ ID2= 2 (5260):	56.22	4.470	6.50	71.91	
=====					
ID = 3 (9260):	476.24	9.817	6.50	53.87	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8376)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (8374):	2149.29	15.667	10.75	30.36	
+ ID2= 2 (9260):	476.24	9.817	6.50	53.87	
=====					
ID = 3 (8376):	2625.53	21.681	10.00	34.62	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8378)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (0262):	341.31	5.995	7.50	49.52	
+ ID2= 2 (8376):	2625.53	21.681	10.00	34.62	
=====					
ID = 3 (8378):	2966.84	26.671	9.50	36.34	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8362)					
1 + 2 = 3					
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 (8378):	2966.84	26.671	9.50	36.34	
+ ID2= 2 (8384):	1321.41	14.652	8.50	37.15	
=====					
ID = 3 (8362):	4288.25	41.006	8.75	36.59	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6272)
IN= 2----> OUT= 1

Routing time step (min) = 15.00

<----- DATA FOR SECTION (2721.0) ----->

Distance	Elevation	Manning	
0.00	231.01	0.0450	
23.01	223.65	0.0450	
34.51	222.46	0.0450	
51.77	222.11	0.0450	
69.02	221.87	0.0450	
161.06	221.92	0.0450	
166.81	221.91	0.0450	
172.56	221.89	0.0450	
180.57	221.40	0.0450 / 0.0300	Main Channel
181.57	220.60	0.0300	Main Channel
184.07	220.16	0.0300	Main Channel
195.57	221.85	0.0300 / 0.0450	Main Channel
201.32	221.82	0.0450	
207.07	221.77	0.0450	
212.83	221.72	0.0450	
253.09	222.52	0.0450	
408.40	222.65	0.0450	
460.17	223.20	0.0450	
517.69	224.84	0.0450	
569.46	232.57	0.0450	

<----- TRAVEL TIME TABLE ----->
DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV.TIME

(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)
0.41	220.57	.189E+04	0.5	0.49	60.18
0.83	220.99	.697E+04	3.2	0.82	35.79
1.24	221.40	.145E+05	8.8	1.07	27.45
1.84	222.00	.643E+05	29.3	0.81	36.61
2.44	222.60	.282E+06	132.9	0.83	35.41
3.04	223.20	.707E+06	413.7	1.04	28.50
3.64	223.80	.118E+07	876.8	1.31	22.46
4.24	224.40	.169E+07	1494.8	1.57	18.75
4.84	225.00	.221E+07	2264.1	1.82	16.23
5.44	225.60	.274E+07	3196.3	2.06	14.29
6.04	226.20	.328E+07	4258.6	2.30	12.84
6.65	226.81	.383E+07	5444.7	2.52	11.72
7.25	227.41	.438E+07	6749.7	2.73	10.82
7.85	228.01	.494E+07	8169.4	2.93	10.08
8.45	228.61	.551E+07	9700.7	3.12	9.46
9.05	229.21	.608E+07	11340.8	3.30	8.94
9.65	229.81	.666E+07	13087.4	3.48	8.48
10.25	230.41	.724E+07	14938.5	3.65	8.08
10.85	231.01	.783E+07	16892.4	3.82	7.73

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8362)	4288.25	41.01	8.75	36.59	1.91	0.81
OUTFLOW: ID= 1 (6272)	4288.25	40.44	9.75	36.59	1.91	0.81

CALIB
STANDHYD (2721)
ID= 1 DT=15.0 min

	Area (ha)	Imp (%)	Di r. Conn. (%)
Total	2.21	49.00	19.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	1.08	1.13
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	121.38	40.00
Mannings n	0.013	0.250

	Max. Eff. Inten. (mm/hr)	over (min)	Storage Coeff. (min)	Uni t Hyd. Tpeak (min)	Uni t Hyd. peak (cms)
	116.82	123.79	15.00	3.32 (ii)	13.14 (ii)
			15.00	15.00	15.00
			0.11	0.08	0.08

	PEAK FLOW (cms)	TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAI NFALL (mm)	RUNOFF COEFFICIENT
	0.14	0.29	86.50	88.50	0.98
	0.14	6.00	6.00	88.50	0.57
	0.14	6.00	50.52	88.50	0.65

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 73.1 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5272)
IN= 2----> OUT= 1
DT= 15.0 min

	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	0.1480	0.0589
	0.0070	0.0284	0.1850	0.0669
	0.0430	0.0331	0.2160	0.0746
	0.0800	0.0419	0.6160	0.0846
	0.1100	0.0497	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (2721)	2.210	0.424	6.00	57.35
OUTFLOW: ID= 1 (5272)	2.210	0.141	6.25	56.89

PEAK FLOW REDUCTION [Qout/Qin] (%) = 33.19
TIME SHIFT OF PEAK FLOW (min) = 15.00
MAXIMUM STORAGE USED (ha. m.) = 0.0577

 CALIB (2722) Area (ha) = 155.17 Curve Number (CN) = 75.3
 NASHYD (2722) Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 ID= 1 DT=15.0 min U. H. Tp(hrs) = 1.09

Unit Hyd Opeak (cms) = 2.433
 PEAK FLOW (cms) = 2.870 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 41.420
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.468

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 ADD HYD (9272)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (2722): 155.17 2.870 7.25 41.42
 + ID2= 2 (5272): 2.21 0.141 6.25 56.89

 ID = 3 (9272): 157.38 2.949 7.25 41.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8356)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (6272): 4288.25 40.441 9.75 36.59
 + ID2= 2 (9272): 157.38 2.949 7.25 41.64

 ID = 3 (8356): 4445.63 42.651 9.50 36.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8352)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8354): 1268.73 24.661 7.75 46.69
 + ID2= 2 (8356): 4445.63 42.651 9.50 36.77

 ID = 3 (8352): 5714.36 64.805 8.75 38.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ROUTE CHN (6304)
 IN= 2----> OUT= 1
 Routing time step (min) = 15.00

 DATA FOR SECTION (3041.0) -----
 Distance Elevation Manning
 0.00 232.08 0.0500
 19.00 231.87 0.0500
 38.00 231.33 0.0500
 66.51 230.44 0.0500
 104.51 228.25 0.0500
 118.76 225.17 0.0500
 128.26 219.86 0.0500
 175.77 219.17 0.0500
 185.27 218.90 0.0500 / 0.0300 Main Channel
 185.52 218.65 0.0300 Main Channel
 190.02 218.37 0.0300 Main Channel
 194.52 218.60 0.0300 Main Channel
 194.77 218.85 0.0300 / 0.0500 Main Channel
 204.27 219.60 0.0500
 299.28 220.91 0.0500
 327.78 222.36 0.0500
 375.28 225.71 0.0500
 403.79 229.37 0.0500
 432.29 230.43 0.0500
 470.29 232.00 0.0500

 TRAVEL TIME TABLE
 DEPTH ELEV VOLUME FLOW RATE VELOCITY TRAV. TIME
 (m) (m) (cu. m.) (cms) (m/s) (min)
 0.48 218.85 .121E+05 1.2 0.36 173.94
 1.17 219.54 .839E+05 9.8 0.44 142.60
 1.86 220.23 .323E+06 41.0 0.57 131.43
 2.56 220.93 .708E+06 109.5 0.58 107.67
 3.25 221.62 .118E+07 224.3 0.71 87.36
 3.94 222.31 .168E+07 376.5 0.84 74.48
 4.63 223.00 .222E+07 567.9 0.96 65.26
 5.32 223.69 .279E+07 794.7 1.07 58.58
 6.02 224.39 .339E+07 1056.2 1.17 53.52
 6.71 225.08 .402E+07 1352.4 1.26 49.52
 7.40 225.77 .468E+07 1677.3 1.34 46.46
 8.09 226.46 .536E+07 2050.0 1.43 43.58
 8.79 227.16 .607E+07 2457.9 1.52 41.14
 9.48 227.85 .680E+07 2901.0 1.60 39.05
 10.17 228.54 .755E+07 3352.7 1.66 37.53
 10.86 229.23 .834E+07 3812.5 1.71 36.48
 11.55 229.92 .919E+07 4241.8 1.73 36.13
 12.25 230.62 .101E+08 4697.0 1.74 35.93
 12.94 231.31 .111E+08 5186.3 1.74 35.83

 <--- hydrograph ---> <- pi pe / channel ->
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL
 (ha) (cms) (hrs) (mm) (m) (m/s)
 INFLOW: ID= 2 (8352) 5714.36 64.81 8.75 38.97 2.10 0.51
 OUTFLOW: ID= 1 (6304) 5714.36 53.19 11.00 38.97 1.98 0.49

 ADD HYD (8350)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0304): 292.37 1.844 10.25 29.90
 + ID2= 2 (6304): 5714.36 53.186 11.00 38.97

 ID = 3 (8350): 6006.73 55.013 11.00 38.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8346)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (8348): 3768.46 37.313 9.75 35.39
 + ID2= 2 (8350): 6006.73 55.013 11.00 38.53

 ID = 3 (8346): 9775.19 90.815 10.25 37.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 ADD HYD (8344)
 1 + 2 = 3
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 ID1= 1 (0306): 283.97 0.637 13.75 21.91
 + ID2= 2 (8346): 9775.19 90.815 10.25 37.32

 ID = 3 (8344): 10059.16 91.371 10.25 36.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

 RESERVOIR (5509)
 IN= 2----> OUT= 1
 DT= 15.0 min
 OUTFLOW STORAGE OUTFLOW STORAGE
 (cms) (ha. m.) (cms) (ha. m.)
 0.0000 0.0000 ***** 370.0451
 41.0590 56.7403 ***** 863.4386
 48.1390 86.3439 ***** *****
 56.6340 ***** *****
 67.9600 ***** 0.0000 0.0000
 AREA OPEAK TPEAK R. V.
 (ha) (cms) (hrs) (mm)
 INFLOW: ID= 2 (8344) ***** 91.371 10.25 36.88

OUTFLOW: ID= 1 (5509) ***** 55.685 15.75 36.88

PEAK FLOW REDUCTION [Qout/Qin] (%) = 60.94
 TIME SHIF OF PEAK FLOW (min) = 330.00
 MAXIMUM STORAGE USED (ha. m.) = 119.2309

CALIB NASHYD (0332) Area (ha) = 393.44 Curve Number (CN) = 75.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 2.32

Unit Hyd Opeak (cms) = 2.894

PEAK FLOW (cms) = 3.974 (i)
 TIME TO PEAK (hrs) = 9.500
 RUNOFF VOLUME (mm) = 41.340
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.467

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0330) Area (ha) = 468.30 Curve Number (CN) = 80.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.09

Unit Hyd Opeak (cms) = 7.335

PEAK FLOW (cms) = 9.926 (i)
 TIME TO PEAK (hrs) = 7.250
 RUNOFF VOLUME (mm) = 47.004
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.531

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ROUTE CHN (6332) Routing time step (min) = 15.00
 IN= 2--> OUT= 1

DATA FOR SECTION (3321.0)

Distance	Elevation	Manning	
0.00	234.00	0.0380	
25.85	227.20	0.0380	
96.94	226.44	0.0380	
168.03	227.38	0.0380	
219.73	225.62	0.0380	
342.52	221.57	0.0380	
368.37	221.42	0.0380	
374.83	221.23	0.0380	
379.79	220.98	0.0380 / 0.0300	Main Channel
380.29	220.47	0.0300	Main Channel
381.29	220.47	0.0300	Main Channel
382.79	220.47	0.0300	Main Channel
383.29	220.98	0.0300 / 0.0380	Main Channel
394.22	221.22	0.0380	
400.68	221.33	0.0380	
407.14	221.44	0.0380	
491.16	225.70	0.0380	
568.71	227.55	0.0380	
607.49	230.14	0.0380	
639.80	234.08	0.0380	

TRAVEL TIME TABLE

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.51	220.98	.554E+04	0.9	0.56	107.27
1.20	221.67	.885E+05	12.1	0.49	122.32
1.88	222.35	.309E+06	64.7	0.76	79.70
2.57	223.04	.615E+06	165.4	0.97	61.99
3.25	223.72	.101E+07	322.1	1.16	52.06
3.94	224.41	.148E+07	542.9	1.33	45.51
4.62	225.09	.204E+07	835.4	1.48	40.77
5.31	225.78	.269E+07	1202.8	1.62	37.27
5.99	226.46	.344E+07	1618.0	1.70	35.46
6.68	227.15	.447E+07	1932.0	1.56	38.56
7.36	227.83	.580E+07	2763.9	1.72	34.96
8.05	228.52	.718E+07	3861.9	1.95	30.97
8.73	229.20	.859E+07	5115.4	2.16	27.98

9.42	229.89	.100E+08	6519.2	2.35	25.64
10.10	230.57	.115E+08	8092.3	2.55	23.69
10.79	231.26	.130E+08	9821.5	2.73	22.05
11.47	231.94	.145E+08	11690.0	2.92	20.68
12.16	232.63	.160E+08	13694.9	3.09	19.52
12.84	233.31	.176E+08	15833.9	3.26	18.52

hydrograph

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (0330)	468.30	9.93	7.25	47.00	1.06	0.50
OUTFLOW: ID= 1 (6332)	468.30	7.46	9.50	47.00	0.91	0.52

ADD HYD (8322)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 (0332):	393.44	3.974	9.50	41.34
+ ID2= 2 (6332):	468.30	7.455	9.50	47.00
ID = 3 (8322):	861.74	11.429	9.50	44.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0328) Area (ha) = 492.92 Curve Number (CN) = 77.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.91

Unit Hyd Opeak (cms) = 4.411

PEAK FLOW (cms) = 6.150 (i)
 TIME TO PEAK (hrs) = 8.750
 RUNOFF VOLUME (mm) = 43.578
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.492

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0326) Area (ha) = 678.91 Curve Number (CN) = 80.0
 ID= 1 DT=15.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 1.50
 U.H. Tp(hrs) = 1.95

Unit Hyd Opeak (cms) = 5.941

PEAK FLOW (cms) = 9.054 (i)
 TIME TO PEAK (hrs) = 8.750
 RUNOFF VOLUME (mm) = 47.251
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.534

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3181) Area (ha) = 357.30
 ID= 1 DT=15.0 min Total Imp(%) = 56.00 Dir. Conn. (%) = 32.00

	IMPERVIOUS (ha)	PERVIOUS (i)
Surface Area	200.09	157.21
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	1543.37	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) =	116.82	118.46
over (mi n)	15.00	30.00
Storage Coeff. (mi n) =	15.27 (ii)	25.26 (ii)
Unit Hyd. Tpeak (mi n) =	15.00	30.00
Unit Hyd. peak (cms) =	0.07	0.04

PEAK FLOW (cms) =	26.82	23.77	*TOTALS*
TIME TO PEAK (hrs) =	6.00	6.25	42.397 (iii)
RUNOFF VOLUME (mm) =	86.50	49.67	6.00
TOTAL RAINFALL (mm) =	88.50	88.50	61.46
			88.50

RUNOFF COEFFICIENT = 0.98 0.56 0.69

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5318)
IN= 2--> OUT= 1
DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	22.8580	8.7280
0.8190	2.5190	27.5380	9.6853
4.2860	5.8947	32.5800	10.8345
11.0600	6.7336	32.9800	10.8445
16.3830	7.4416	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3181)	357.300	42.397	6.00	61.46
OUTFLOW: ID= 1 (5318)	357.300	23.161	6.50	61.45

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 54.63
TIME SHIFT OF PEAK FLOW (mi n) = 30.00
MAXIMUM STORAGE USED (ha. m.) = 8.8668

CALIB
STANDHYD (3182)
ID= 1 DT=15.0 mi n

Area (ha) = 126.13
Total Imp (%) = 21.00
Di r. Conn. (%) = 12.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) =	26.49	99.64
Dep. Storage (mm) =	2.00	5.00
Average Slope (%) =	0.50	0.50
Length (m) =	916.99	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr) =	116.82	71.96
Storage Coeff. (mi n) =	15.00	30.00
Unit Hyd. Tpeak (mi n) =	11.17 (ii)	23.37 (ii)
Unit Hyd. peak (cms) =	15.00	30.00
	0.08	0.04

TOTALS
PEAK FLOW (cms) = 4.00 9.42 10.959 (iii)
TIME TO PEAK (hrs) = 6.00 6.25 6.25
RUNOFF VOLUME (mm) = 86.50 41.82 47.18
TOTAL RAINFALL (mm) = 88.50 88.50 88.50
RUNOFF COEFFICIENT = 0.98 0.47 0.53

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8334)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3182):	126.13	10.959	6.25	47.18
+ ID2= 2 (5318):	357.30	23.161	6.50	61.45
ID = 3 (8334):	483.43	30.555	6.50	57.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6320)
IN= 2--> OUT= 1

Routing time step (mi n) = 15.00

<----- DATA FOR SECTION (3201.0) ----->

Distance	Elevation	Manning
0.00	249.00	0.0380
22.92	245.86	0.0380
45.83	244.87	0.0380
91.66	243.11	0.0380
126.03	239.53	0.0380
160.41	237.17	0.0380
166.14	237.06	0.0380
177.59	237.13	0.0380
183.32	237.20	0.0380
189.05	236.70	0.0380 / 0.0350
193.05	235.89	0.0350
197.05	236.64	0.0350 / 0.0380
200.51	236.74	0.0380
206.24	237.03	0.0380
246.34	238.82	0.0380
263.53	243.87	0.0380
389.56	247.64	0.0380
452.58	247.74	0.0380
498.41	248.60	0.0380
567.16	249.84	0.0380

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.37	236.26	270E+04	0.4	0.58	107.96
0.75	236.64	108E+05	2.6	0.92	68.01
1.48	237.37	877E+05	27.4	1.17	53.35
2.20	238.09	277E+06	122.1	1.65	37.83
2.93	238.82	540E+06	299.7	2.08	30.01
3.66	239.55	857E+06	587.8	2.57	24.29
4.39	240.28	120E+07	977.8	3.04	20.52
5.11	241.00	158E+07	1458.1	3.46	18.02
5.84	241.73	198E+07	2028.6	3.84	16.23
6.57	242.46	240E+07	2690.5	4.19	14.86
7.29	243.18	285E+07	3431.8	4.50	13.84
8.02	243.91	335E+07	4149.1	4.64	13.44
8.75	244.64	393E+07	4746.6	4.51	13.80
9.47	245.36	463E+07	5618.6	4.53	13.75
10.20	246.09	545E+07	6781.8	4.66	13.39
10.93	246.82	635E+07	8274.5	4.87	12.79
11.66	247.55	733E+07	9988.0	5.10	12.23
12.38	248.27	853E+07	11422.0	5.01	12.45
13.11	249.00	989E+07	13749.7	5.20	11.98

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8334)	483.43	30.56	6.50	57.73	1.50	1.18
OUTFLOW: ID= 1 (6320)	483.43	18.89	7.00	57.73	1.23	1.07

CALIB
NASHYD (3202)
ID= 1 DT=15.0 mi n

Area (ha) = 84.56
Curve Number (CN) = 80.9
U. H. Tp (hrs) = 5.00
of Linear Res. (N) = 1.50

Unit Hyd Qpeak (cms) = 2.054

PEAK FLOW (cms) =	2.614 (i)
TIME TO PEAK (hrs) =	6.750
RUNOFF VOLUME (mm) =	47.762
TOTAL RAINFALL (mm) =	88.500
RUNOFF COEFFICIENT =	0.540

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB
STANDHYD (3201)
ID= 1 DT=15.0 mi n

Area (ha) = 194.18
Total Imp (%) = 45.00
Di r. Conn. (%) = 16.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) =	87.38	106.80
Dep. Storage (mm) =	2.00	5.00
Average Slope (%) =	0.50	0.50
Length (m) =	1137.78	40.00
Mannings n =	0.013	0.250

Max. Eff. Inten. (mm/hr) =	116.82	130.27
over (mi n)	15.00	30.00

Storage Coeff. (min)= 12.71 (ii) 22.34 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.08 0.04

PEAK FLOW (cms)= 7.85 18.92 *TOTALS* 22.286 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.25
 RUNOFF VOLUME (mm)= 86.50 55.20 60.21
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.62 0.68

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 78.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5320)
 IN= 2----> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	7.4940	4.4477
0.2490	1.8139	9.8910	4.7762
1.2830	3.1517	12.4550	5.0763
3.7490	3.7610	12.8550	5.0863
5.0040	4.0198	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3201)	194.180	22.286	6.25	60.21
OUTFLOW: ID= 1 (5320)	194.180	12.123	6.75	60.20

PEAK FLOW REDUCTION [Qout/Qin] (%) = 54.40
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 5.1456

ADD HYD (9320)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3202):	84.56	2.614	6.75	47.76
+ ID2= 2 (5320):	194.18	12.123	6.75	60.20
=====				
ID = 3 (9320):	278.74	14.737	6.75	56.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8332)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6320):	483.43	18.890	7.00	57.73
+ ID2= 2 (9320):	278.74	14.737	6.75	56.42
=====				
ID = 3 (8332):	762.17	31.854	6.75	57.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (3142)
 ID= 1 DT=15.0 mi n

Area (ha)= 122.75 Curve Number (CN)= 78.0
 Ia (mm)= 5.00 # of Linear Res. (N)= 1.50
 U.H. Tp(hrs)= 0.53

Unit Hyd Opeak (cms)= 3.924

PEAK FLOW (cms)= 4.337 (i)
 TIME TO PEAK (hrs)= 6.500
 RUNOFF VOLUME (mm)= 43.783
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.495

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3141)
 ID= 1 DT=15.0 mi n

Area Total (ha)= 42.46
 Imp(%)= 70.00 Dir. Conn.(%)= 46.00

	IMPERVIOUS (ha)	PERVIOUS (i) (mm)
Surface Area	29.72	12.74
Dep. Storage	2.00	5.00
Average Slope	0.50	0.50
Length	532.04	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 116.82 147.39
 over (min) = 15.00 30.00
 Storage Coeff. (min)= 8.06 (ii) 17.22 (ii)
 Unit Hyd. Tpeak (min)= 15.00 30.00
 Unit Hyd. peak (cms)= 0.10 0.05

PEAK FLOW (cms)= 5.67 2.84 *TOTALS* 7.583 (iii)
 TIME TO PEAK (hrs)= 6.00 6.25 6.00
 RUNOFF VOLUME (mm)= 86.50 53.18 68.51
 TOTAL RAINFALL (mm)= 88.50 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.60 0.77

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 72.9 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5314)
 IN= 2----> OUT= 1
 DT= 15.0 mi n

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.8400	1.3064
0.1440	0.7144	3.5660	1.4269
0.8230	0.8540	4.1520	1.5451
1.5320	0.9987	4.5520	1.5551
2.1250	1.1456	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3141)	42.460	7.583	6.00	68.51
OUTFLOW: ID= 1 (5314)	42.460	3.096	6.50	68.48

PEAK FLOW REDUCTION [Qout/Qin] (%) = 40.83
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = 1.3696

ADD HYD (9314)
 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3142):	122.75	4.337	6.50	43.78
+ ID2= 2 (5314):	42.46	3.096	6.50	68.48
=====				
ID = 3 (9314):	165.21	7.433	6.50	50.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6316)
 IN= 2----> OUT= 1

Routing time step (min)' = 15.00

<----- DATA FOR SECTION (3161.0) ----->

Distance	Elevation	Manning
0.00	248.54	0.0350
27.93	248.34	0.0350
41.89	246.61	0.0350
62.83	243.09	0.0350
132.65	239.00	0.0350
188.50	236.74	0.0350
195.48	236.54	0.0350
202.46	236.32	0.0350
205.48	236.14	0.0350 / 0.0310
205.98	235.61	0.0310

Main Channel
 Main Channel

207.98	235.25	0.0310	Main Channel
209.98	235.53	0.0310	Main Channel
210.48	236.00	0.0310 / 0.0350	Main Channel
216.42	236.73	0.0350	
258.31	239.09	0.0350	
328.12	239.84	0.0350	
439.83	241.47	0.0350	
530.58	242.08	0.0350	
586.43	242.93	0.0350	
691.16	248.00	0.0350	

TRAVEL TIME TABLE					
DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.37	235.62	.215E+04	0.7	0.77	53.88
0.75	236.00	.634E+04	3.5	1.36	30.45
1.46	236.71	.297E+05	19.3	1.62	25.66
2.16	237.41	.103E+06	75.8	1.84	22.60
2.87	238.12	.229E+06	202.8	2.21	18.79
3.57	238.82	.407E+06	423.3	2.59	16.03
4.28	239.53	.654E+06	727.7	2.77	14.97
4.99	240.24	.103E+07	1242.0	3.01	13.76
5.69	240.94	.151E+07	2002.3	3.31	12.54
6.40	241.65	.210E+07	2987.9	3.55	11.70
7.10	242.35	.286E+07	4302.8	3.75	11.07
7.81	243.06	.374E+07	6165.3	4.10	10.11
8.51	243.76	.468E+07	8642.4	4.60	9.03
9.22	244.47	.568E+07	11496.3	5.06	8.20
9.93	245.18	.666E+07	14714.1	5.50	7.54
10.63	245.88	.770E+07	18290.3	5.91	7.02
11.34	246.59	.877E+07	22221.9	6.30	6.58
12.04	247.29	.988E+07	26464.4	6.67	6.22
12.75	248.00	.110E+08	31061.3	7.01	5.91

<---- hydrograph ---->					<-pi pe / channel -->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (9314)	165.21	7.43	6.50	50.13	0.93	1.42
OUTFLOW: ID= 1 (6316)	165.21	6.25	7.00	50.13	0.87	1.40

CALIB NASHYD (3162)	Area (ha)= 196.72	Curve Number (CN)= 83.0
ID= 1 DT=15.0 mi n	Ia (mm)= 5.00	# of Li near Res. (N)= 1.50
	U. H. Tp(hrs)= 0.88	

Unit Hyd Qpeak (cms)= 3.838

PEAK FLOW (cms)= 5.446 (i)

TIME TO PEAK (hrs)= 7.000

RUNOFF VOLUME (mm)= 50.807

TOTAL RAINFALL (mm)= 88.500

RUNOFF COEFFICIENT = 0.574

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3161)	Area (ha)= 35.62	Dir. Conn. (%)= 44.00
ID= 1 DT=15.0 mi n	Total Imp(%)= 63.00	

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	22.44	13.18
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	0.50	0.50
Length (m)	487.31	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr)= 116.82

over (mi n)= 15.00

Storage Coeff. (mi n)= 7.64 (ii)

Unit Hyd. Tpeak (mi n)= 15.00

Unit Hyd. peak (cms)= 0.10

TOTALS

PEAK FLOW (cms)= 4.60

TIME TO PEAK (hrs)= 6.00

RUNOFF VOLUME (mm)= 86.50

TOTAL RAINFALL (mm)= 88.50

RUNOFF COEFFICIENT = 0.98

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 81.5 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5316)				
IN= 2--> OUT= 1				
DT= 15.0 mi n				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	2.3160	0.9307
	0.0930	0.5020	2.9060	1.0021
	0.5400	0.6637	3.5360	1.0731
	1.1940	0.7495	3.9360	1.0831
	1.6920	0.8359	0.0000	0.0000
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW : ID= 2 (3161)	35.620	6.495	6.00	71.03
OUTFLOW: ID= 1 (5316)	35.620	3.294	6.25	70.99

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 50.72

TIME SHIF T OF PEAK FLOW (mi n) = 15.00

MAXIMUM STORAGE USED (ha. m.) = 1.0778

ADD HYD (9316)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3162):	196.72	5.446	7.00	50.81
+ ID2= 2 (5316):	35.62	3.294	6.25	70.99
=====				
ID = 3 (9316):	232.34	8.400	6.50	53.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8338)				
1 + 2 = 3				
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (6316):	165.21	6.251	7.00	50.13
+ ID2= 2 (9316):	232.34	8.400	6.50	53.90
=====				
ID = 3 (8338):	397.55	13.922	6.75	52.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0312)	Area (ha)= 359.44	Curve Number (CN)= 80.0
ID= 1 DT=15.0 mi n	Ia (mm)= 5.00	# of Li near Res. (N)= 1.50
	U. H. Tp(hrs)= 1.36	

Unit Hyd Qpeak (cms)= 4.529

PEAK FLOW (cms)= 6.425 (i)

TIME TO PEAK (hrs)= 8.000

RUNOFF VOLUME (mm)= 47.122

TOTAL RAINFALL (mm)= 88.500

RUNOFF COEFFICIENT = 0.532

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD (0308)	Area (ha)= 529.30	Curve Number (CN)= 62.0
ID= 1 DT=15.0 mi n	Ia (mm)= 5.00	# of Li near Res. (N)= 1.50
	U. H. Tp(hrs)= 1.62	

Unit Hyd Qpeak (cms)= 5.575

PEAK FLOW (cms)= 4.939 (i)

TIME TO PEAK (hrs)= 8.250

RUNOFF VOLUME (mm)= 29.006

TOTAL RAINFALL (mm)= 88.500

RUNOFF COEFFICIENT = 0.328

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ID= 1 DT=15.0 min	Area (ha) = 138.28 Ia (mm) = 5.00 U. H. Tp(hrs) = 0.76	Curve Number (CN) = 65.0 # of Linear Res. (N) = 1.50
--------------------------------------	--	---

Unit Hyd Opeak (cms) = 3.102

PEAK FLOW (cms) = 2.521 (i)
 TIME TO PEAK (hrs) = 7.000
 RUNOFF VOLUME (mm) = 31.169
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = 0.352

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8342) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0308):	529.30	4.939	8.25	29.01
+ ID2= 2 (0310):	138.28	2.521	7.00	31.17
ID = 3 (8342):	667.58	7.113	7.75	29.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6312) IN= 2----> OUT= 1	Routing time step (min)' = 15.00
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<----- DATA FOR SECTION (3121.0) ----->

Distance	Elevation	Manning	
0.00	265.94	0.0360	
38.07	265.43	0.0360	
59.82	263.98	0.0360	
103.32	254.59	0.0360	
157.70	252.16	0.0360	
217.52	250.45	0.0360	
233.84	247.69	0.0360	
234.71	247.27	0.0360	
239.71	246.38	0.0360	
244.71	246.12	0.0360 / 0.0330	Main Channel
247.71	245.17	0.0330	Main Channel
249.71	245.19	0.0330	Main Channel
251.71	245.64	0.0330 / 0.0360	Main Channel
259.71	246.67	0.0360	
282.78	247.12	0.0360	
315.41	251.60	0.0360	
424.17	256.13	0.0360	
478.55	257.04	0.0360	
516.62	259.37	0.0360	
538.37	266.00	0.0360	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.47	245.64	.111E+04	3.1	1.78	6.07
1.54	246.71	.103E+05	53.9	3.39	3.19
2.61	247.78	.414E+05	281.3	4.40	2.45
3.68	248.85	.839E+05	749.1	5.79	1.87
4.74	249.91	.136E+06	1463.2	6.97	1.55
5.81	250.98	.201E+06	2354.8	7.61	1.42
6.88	252.05	.295E+06	3533.0	7.76	1.39
7.95	253.12	.427E+06	5303.9	8.05	1.34
9.02	254.19	.594E+06	7856.5	8.58	1.26
10.09	255.26	.792E+06	11449.2	9.37	1.15
11.15	256.32	.101E+07	15862.4	10.15	1.06
12.22	257.39	.127E+07	21074.5	10.75	1.01
13.29	258.46	.155E+07	28184.1	11.78	0.92
14.36	259.53	.185E+07	36386.2	12.77	0.85
15.43	260.60	.215E+07	46110.6	13.89	0.78
16.50	261.67	.246E+07	56822.5	14.95	0.72
17.56	262.73	.278E+07	68498.1	15.97	0.68
18.63	263.80	.310E+07	81118.8	16.94	0.64
19.70	264.87	.344E+07	93603.2	17.66	0.61

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW : ID= 2 (8342)	667.58	7.11	7.75	29.45	0.56	1.85
OUTFLOW: ID= 1 (6312)	667.58	7.11	7.75	29.45	0.55	1.85

ADD HYD (8340) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0312):	359.44	6.425	8.00	47.12
+ ID2= 2 (6312):	667.58	7.109	7.75	29.45
ID = 3 (8340):	1027.02	13.531	8.00	35.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8336) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8338):	397.55	13.922	6.75	52.33
+ ID2= 2 (8340):	1027.02	13.531	8.00	35.64
ID = 3 (8336):	1424.57	26.391	7.00	40.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8330) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8332):	762.17	31.854	6.75	57.25
+ ID2= 2 (8336):	1424.57	26.391	7.00	40.30
ID = 3 (8330):	2186.74	57.932	6.75	46.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6324) IN= 2----> OUT= 1	Routing time step (min)' = 15.00
---------------------------------------	----------------------------------

<----- DATA FOR SECTION (3241.0) ----->

Distance	Elevation	Manning	
0.00	234.38	0.0360	
33.57	232.48	0.0360	
67.14	230.14	0.0360	
83.93	228.80	0.0360	
134.29	227.62	0.0360	
209.82	225.10	0.0360	
218.21	224.86	0.0360	
226.61	224.47	0.0360	
234.00	223.86	0.0360 / 0.0300	Main Channel
234.10	223.66	0.0300	Main Channel
235.00	223.66	0.0300	Main Channel
235.90	223.66	0.0300	Main Channel
236.00	223.86	0.0300 / 0.0360	Main Channel
243.39	224.92	0.0360	
251.78	224.89	0.0360	
335.71	225.64	0.0360	
562.32	226.53	0.0360	
637.85	228.36	0.0360	
705.00	229.80	0.0360	
830.89	234.00	0.0360	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.20	223.86	.145E+04	0.2	0.43	147.27
0.73	224.39	.159E+05	2.8	0.68	93.11
1.27	224.93	.561E+05	11.0	0.75	85.19
1.80	225.46	.217E+06	45.6	0.80	79.20
2.33	225.99	.566E+06	134.4	0.91	70.20
2.87	226.53	.122E+07	338.1	1.06	59.97

3.40	227.06	.206E+07	744.3	1.38	46.16
3.94	227.60	.298E+07	1295.9	1.66	38.38
4.47	228.13	.399E+07	1979.9	1.90	33.59
5.00	228.66	.509E+07	2807.5	2.11	30.21
5.54	229.20	.627E+07	3815.1	2.32	27.41
6.07	229.73	.753E+07	4984.1	2.53	25.17
6.60	230.26	.884E+07	6346.3	2.74	23.20
7.14	230.80	.102E+08	7861.2	2.95	21.61
7.67	231.33	.116E+08	9526.4	3.14	20.29
8.21	231.87	.131E+08	11342.2	3.32	19.18
8.74	232.40	.146E+08	13309.6	3.49	18.22
9.27	232.93	.161E+08	15414.0	3.66	17.41
9.81	233.47	.177E+08	17671.0	3.81	16.70

<---- hydrograph ---->							<- pi pe / channel ->	
	AREA	OPEAK	TPEAK	R. V.	MAX DEPTH	MAX VEL		
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)		
INFLOW : ID= 2 (8330)	2186.74	57.93	6.75	46.21	1.88	0.82		
OUTFLOW: ID= 1 (6324)	2186.74	39.98	8.25	46.21	1.71	0.79		

CALIB NASHYD (3242)	Area (ha)= 602.18	Curve Number (CN)= 78.8
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50
	U. H. Tp(hrs)= 1.80	

Unit Hyd Qpeak (cms)= 5.708
 PEAK FLOW (cms)= 8.287 (i)
 TIME TO PEAK (hrs)= 8.500
 RUNOFF VOLUME (mm)= 45.725
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.517

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3241)	Area (ha)= 13.46	Dir. Conn. (%)= 15.00
ID= 1 DT=15.0 min	Total Imp(%)= 45.00	

Surface Area (ha)= 6.06	PERVIOUS (i) 7.40
Dep. Storage (mm)= 2.00	5.00
Average Slope (%)= 0.50	0.50
Length (m)= 299.56	40.00
Mannings n = 0.013	0.250
Max. Eff. Inten. (mm/hr)= 116.82	132.41
over (min)= 15.00	30.00
Storage Coeff. (min)= 5.71 (ii)	15.27 (ii)
Unit Hyd. Tpeak (min)= 15.00	30.00
Unit Hyd. peak (cms)= 0.11	0.05

TOTALS
 PEAK FLOW (cms)= 0.62 1.55
 TIME TO PEAK (hrs)= 6.00 6.25
 RUNOFF VOLUME (mm)= 86.50 55.46
 TOTAL RAINFALL (mm)= 88.50 88.50
 RUNOFF COEFFICIENT = 0.98 0.63

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 78.2 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5324)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
IN= 2----> OUT= 1	(cms)	(ha. m.)	(cms)	(ha. m.)
DT= 15.0 min	0.0000	0.0000	0.9000	0.3505
	0.0460	0.1645	1.1310	0.4008
	0.2610	0.1903	1.3160	0.4499
	0.4860	0.2456	1.7160	0.4599
	0.6740	0.2932	0.0000	0.0000

INFLOW : ID= 2 (3241)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 (5324)	13.460	1.681	6.00	60.12
		0.874	6.50	60.05

PEAK FLOW REDUCTION [Qout/Qin] (%) = 51.98
 TIME SHIF OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha. m.) = 0.3464

ADD HYD (9324)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (3242):	602.18	8.287	8.50	45.72
+ ID2= 2 (5324):	13.46	0.874	6.50	60.05
=====				
ID = 3 (9324):	615.64	8.556	8.25	46.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8328)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (6324):	2186.74	39.981	8.25	46.21
+ ID2= 2 (9324):	615.64	8.556	8.25	46.04
=====				
ID = 3 (8328):	2802.38	48.537	8.25	46.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB NASHYD (0322)	Area (ha)= 513.13	Curve Number (CN)= 80.0
ID= 1 DT=15.0 min	Ia (mm)= 5.00	# of Linear Res. (N)= 1.50
	U. H. Tp(hrs)= 1.75	

Unit Hyd Qpeak (cms)= 5.020
 PEAK FLOW (cms)= 7.498 (i)
 TIME TO PEAK (hrs)= 8.250
 RUNOFF VOLUME (mm)= 47.219
 TOTAL RAINFALL (mm)= 88.500
 RUNOFF COEFFICIENT = 0.534

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (8326)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0322):	513.13	7.498	8.25	47.22
+ ID2= 2 (8328):	2802.38	48.537	8.25	46.17
=====				
ID = 3 (8326):	3315.51	56.035	8.25	46.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8324)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 (0326):	678.91	9.054	8.75	47.25
+ ID2= 2 (8326):	3315.51	56.035	8.25	46.33
=====				
ID = 3 (8324):	3994.42	65.037	8.25	46.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN (6328)	Routing time step (min) = 15.00
IN= 2----> OUT= 1	

<----- DATA FOR SECTION (3281.0) ----->

Distance	Elevation	Manning	
0.00	228.00	0.0380	
18.58	224.97	0.0380	
78.98	223.52	0.0380	
125.44	223.28	0.0380	
171.90	221.71	0.0380	
213.72	219.65	0.0380	
218.36	219.40	0.0380	
223.01	219.19	0.0380	
225.95	219.14	0.0380 / 0.0320	Main Channel
226.45	218.14	0.0320	Main Channel
236.95	218.14	0.0320	Main Channel
245.85	218.14	0.0320	Main Channel
245.95	219.14	0.0320 / 0.0380	Main Channel
246.24	219.16	0.0380	
250.88	219.24	0.0380	
255.53	219.39	0.0380	
325.22	221.47	0.0380	
367.03	223.14	0.0380	
404.20	225.17	0.0380	
459.95	228.04	0.0380	

<----- TRAVEL TIME TABLE ----->

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.50	218.64	.373E+05	3.2	0.33	192.37
1.00	219.14	.752E+05	10.1	0.51	124.71
1.52	219.66	.149E+06	22.2	0.57	112.06
2.04	220.18	.279E+06	43.4	0.59	107.06
2.56	220.70	.464E+06	77.0	0.63	100.52
3.08	221.22	.705E+06	125.7	0.68	93.56
3.61	221.75	.100E+07	192.7	0.73	86.62
4.13	222.27	.135E+07	278.1	0.79	80.92
4.65	222.79	.176E+07	385.9	0.84	75.86
5.17	223.31	.222E+07	514.9	0.89	71.82
5.69	223.83	.280E+07	662.7	0.90	70.40
6.21	224.35	.345E+07	867.7	0.96	66.35
6.73	224.87	.417E+07	1107.0	1.01	62.81
7.25	225.39	.494E+07	1400.0	1.08	58.80
7.78	225.92	.573E+07	1732.2	1.15	55.17
8.30	226.44	.655E+07	2099.9	1.22	52.02
8.82	226.96	.740E+07	2502.8	1.29	49.29
9.34	227.48	.828E+07	2940.9	1.36	46.90
9.86	228.00	.918E+07	3414.3	1.42	44.79

<---- hydrograph ---->					<-pi pe / channel -->	
	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
INFLOW: ID= 2 (8324)	3994.42	65.04	8.25	46.49	2.38	0.62
OUTFLOW: ID= 1 (6328)	3994.42	51.24	10.00	46.49	2.17	0.60

ADD HYD (8320)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0328):	492.92	6.150	8.75	43.58
+ ID2= 2 (6328):	3994.42	51.239	10.00	46.49
=====				
ID = 3 (8320):	4487.34	57.152	10.00	46.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8318)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8320):	4487.34	57.152	10.00	46.17
+ ID2= 2 (8322):	861.74	11.429	9.50	44.42
=====				
ID = 3 (8318):	5349.08	68.524	10.00	45.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8316)				
1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.

	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (5509):	10059.16	55.685	15.75	36.88
+ ID2= 2 (8318):	5349.08	68.524	10.00	45.89
=====				
ID = 3 (8316):	15408.24	112.978	11.00	40.01

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8312)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8314):	620.10	2.040	6.50	25.61
+ ID2= 2 (8316):	15408.24	112.978	11.00	40.01
=====				
ID = 3 (8312):	16028.34	114.211	11.00	39.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8308)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8310):	598.91	40.821	6.25	49.34
+ ID2= 2 (8312):	16028.34	114.211	11.00	39.45
=====				
ID = 3 (8308):	16627.25	117.147	10.50	39.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (5510)				
IN= 2----> OUT= 1				
DT= 15.0 min				
	OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
	0.0000	0.0000	*****	74.0090
	66.5450	18.5023	*****	*****
	98.5430	37.0045	*****	*****

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (8308)	*****	117.147	10.50	39.81
OUTFLOW: ID= 1 (5510)	*****	107.869	13.25	39.81

PEAK FLOW REDUCTION [Out/Oi n] (%) = 92.08
 TIME SHIFT OF PEAK FLOW (min) = 165.00
 MAXIMUM STORAGE USED (ha. m.) = 47.8888

ADD HYD (8240)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0336):	2785.00	4.639	22.25	38.20
+ ID2= 2 (5510):	16627.25	107.869	13.25	39.81
=====				
ID = 3 (8240):	19412.25	111.871	13.25	39.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8238)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (5508):	9524.19	57.965	11.50	25.97
+ ID2= 2 (8240):	19412.25	111.871	13.25	39.58
=====				
ID = 3 (8238):	28936.44	168.298	12.50	35.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (3427)			
ID= 1 DT=15.0 min	Area (ha)	Total Imp (%)	Di r. Conn. (%)
	249.40	45.00	33.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	112.23	137.17	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	1289.44	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	116.82	71.13	
over (min)	15.00	30.00	
Storage Coeff. (min)=	13.71 (ii)	25.96 (ii)	
Unit Hyd. Tpeak (min)=	15.00	30.00	
Unit Hyd. peak (cms)=	0.08	0.04	
			TOTALS
PEAK FLOW (cms)=	20.19	12.14	27.903 (iii)
TIME TO PEAK (hrs)=	6.00	6.25	6.00
RUNOFF VOLUME (mm)=	86.50	38.07	54.05
TOTAL RAINFALL (mm)=	88.50	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.43	0.61

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 67.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (9425)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha. m.)	(cms)	(ha. m.)
	0.0000	0.0000	12.4700	5.4211
	0.8500	3.0375	16.6800	6.4838
	4.8400	3.5126	20.9500	7.4161
	9.0000	4.5390	24.3900	8.3284
	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (3427)	249.400	27.903	6.00	54.05
OUTFLOW: ID= 1 (9425)	249.400	12.869	6.50	54.05
	PEAK FLOW REDUCTION [Qout/Qin] (%)=	46.12		
	TIME SHIFT OF PEAK FLOW (min)=	30.00		
	MAXIMUM STORAGE USED (ha. m.)=	5.5855		

RESERVOIR (5342)				
IN= 2--> OUT= 1				
DT= 15.0 min				
	OUTFLOW	STORAGE	OUTFLOW	STORAGE
	(cms)	(ha. m.)	(cms)	(ha. m.)
	0.0000	0.0000	3.6600	12.0067
	1.2080	11.1244	4.4200	12.4001
	2.0570	11.3002	5.2300	12.7452
	2.7240	11.3602	0.0000	0.0000
	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW : ID= 2 (9425)	249.400	12.869	6.50	54.05
OUTFLOW: ID= 1 (5342)	249.400	0.938	12.25	54.04
	PEAK FLOW REDUCTION [Qout/Qin] (%)=	7.29		
	TIME SHIFT OF PEAK FLOW (min)=	345.00		
	MAXIMUM STORAGE USED (ha. m.)=	8.6448		

CALIB NASHYD (3422)				
ID= 1 DT=15.0 min				
	Area	(ha)=	755.17	Curve Number (CN)=
	Ia	(mm)=	5.00	# of Linear Res. (N)=
	U. H. Tp	(hrs)=	2.57	1.50
	Unit Hyd Opeak	(cms)=	5.026	
	PEAK FLOW	(cms)=	3.989 (i)	
	TIME TO PEAK	(hrs)=	10.000	
	RUNOFF VOLUME	(mm)=	23.531	
	TOTAL RAINFALL	(mm)=	88.500	
	RUNOFF COEFFICIENT	=	0.266	

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (9342)				
1 + 2 = 3				
	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (3422):	755.17	3.989	10.00	23.53
+ ID2= 2 (5342):	249.40	0.938	12.25	54.04
ID = 3 (9342):	1004.57	4.906	10.25	31.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8236)				
1 + 2 = 3				
	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8238):	28936.44	168.298	12.50	35.10
+ ID2= 2 (9342):	1004.57	4.906	10.25	31.11
ID = 3 (8236):	29941.01	172.870	12.50	34.97

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8234)				
1 + 2 = 3				
	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (8232):	285.79	14.039	6.00	44.95
+ ID2= 2 (8236):	29941.01	172.870	12.50	34.97
ID = 3 (8234):	30226.80	173.540	12.25	35.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CALIB STANDHYD (3442)				
ID= 1 DT=15.0 min				
	Area Total	(ha)=	188.84	Di r. Conn. (%)=
	Imp	(%)=	32.00	24.00

	IMPERVIOUS	PERVIOUS (i)	
Surface Area (ha)=	60.43	128.41	
Dep. Storage (mm)=	2.00	5.00	
Average Slope (%)=	0.50	0.50	
Length (m)=	1122.02	40.00	
Mannings n =	0.013	0.250	
Max. Eff. Inten. (mm/hr)=	116.82	25.33	
over (min)	15.00	45.00	
Storage Coeff. (min)=	12.61 (ii)	31.14 (ii)	
Unit Hyd. Tpeak (min)=	15.00	45.00	
Unit Hyd. peak (cms)=	0.08	0.03	
		TOTALS	
PEAK FLOW (cms)=	11.48	5.49	13.770 (iii)
TIME TO PEAK (hrs)=	6.00	6.50	6.00
RUNOFF VOLUME (mm)=	86.50	24.72	39.55
TOTAL RAINFALL (mm)=	88.50	88.50	88.50
RUNOFF COEFFICIENT =	0.98	0.28	0.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 53.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD (3441)				
ID= 1 DT=15.0 min				
	Area Total	(ha)=	155.15	Di r. Conn. (%)=
	Imp	(%)=	39.00	29.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)=	60.51	94.64
Dep. Storage (mm)=	2.00	5.00
Average Slope (%)=	0.50	0.50
Length (m)=	1017.02	40.00

Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.82 44.64
over (min) = 15.00 30.00

Storage Coeff. (min)= 11.89 (ii) 26.66 (ii)

Unit Hyd. Tpeak (min)= 15.00 30.00

Unit Hyd. peak (cms)= 0.08 0.04

PEAK FLOW (cms)= 11.65 5.14 14.830 (iii)

TIME TO PEAK (hrs)= 6.00 6.25 6.00

RUNOFF VOLUME (mm)= 86.50 25.53 43.21

TOTAL RAINFALL (mm)= 88.50 88.50 88.50

RUNOFF COEFFICIENT = 0.98 0.29 0.49

TOTALS

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN* = 53.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR (5344)
IN= 2--> OUT= 1
DT= 15.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.6700	6.1252
0.3000	1.6125	3.3150	7.0314
0.8100	3.0036	3.8990	7.9700
1.4170	4.2559	4.2990	7.9800
1.8540	5.0526	0.0000	0.0000

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
INFLOW: ID= 2 (3441)	155.150	14.830	6.00	43.21
OUTFLOW: ID= 1 (5344)	155.150	1.524	8.00	43.20

PEAK FLOW REDUCTION [Qout/Qin] (%) = 10.28

TIME SHIFT OF PEAK FLOW (min) = 120.00

MAXIMUM STORAGE USED (ha. m.) = 4.4541

ADD HYD (9344)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (3442):	188.84	13.770	6.00	39.55
+ ID2= 2 (5344):	155.15	1.524	8.00	43.20
ID = 3 (9344):	343.99	14.192	6.00	41.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8230)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8234):	30226.80	173.540	12.25	35.06
+ ID2= 2 (9344):	343.99	14.192	6.00	41.20
ID = 3 (8230):	30570.79	175.159	12.00	35.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8228)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (8226):	1952.06	16.522	8.50	23.01
+ ID2= 2 (8230):	30570.79	175.159	12.00	35.13
ID = 3 (8228):	32522.85	186.046	12.00	34.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (8190)
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 (0356):	1015.22	6.685	8.00	18.15
+ ID2= 2 (8228):	32522.85	186.046	12.00	34.41
ID = 3 (8190):	33538.07	190.507	11.75	33.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

Unit-Flow Rate Calculations

Note: Refer to Table 4-13 to Table 4-41 in the Town of East Gwillimbury SWMMMP for Peak Flow Summary
 Note: Hydrologic Model Output Provided in Appendix F

Storm Event	Comparison Point		Proposed Development Area (ha)		Comparison Point		Proposed Development Area (ha)		Comparison Point		Proposed Development Area (ha)		Comparison Point		Proposed Development Area (ha)		Comparison Point		Proposed Development Area (ha)		Comparison Point		Proposed Development Area (ha)	
	Peak Flow (m ³ /s)	Unit Flow (m ³ /s/ha)	Peak Flow (m ³ /s)	Unit Flow (m ³ /s/ha)	Peak Flow (m ³ /s)	Unit Flow (m ³ /s/ha)	Peak Flow (m ³ /s)	Unit Flow (m ³ /s/ha)	Peak Flow (m ³ /s)	Unit Flow (m ³ /s/ha)	Peak Flow (m ³ /s)	Unit Flow (m ³ /s/ha)	Peak Flow (m ³ /s)	Unit Flow (m ³ /s/ha)	Peak Flow (m ³ /s)	Unit Flow (m ³ /s/ha)	Peak Flow (m ³ /s)	Unit Flow (m ³ /s/ha)	Peak Flow (m ³ /s)	Unit Flow (m ³ /s/ha)	Peak Flow (m ³ /s)	Unit Flow (m ³ /s/ha)	Peak Flow (m ³ /s)	Unit Flow (m ³ /s/ha)
	9244	90.5	9236	14.6	9232	86.7	9216	34	9252	10.1	9242	15.5	9228	43.6	9226	116.5	9220	27.1	9218	42.1				
2	3.154	0.035	0.886	0.061	0.662	0.008	1.046	0.031	0.298	0.030	0.393	0.025	0.727	0.017	1.097	0.009	0.547	0.020	0.514	0.012				
5	5.319	0.059	1.748	0.120	1.449	0.017	1.777	0.052	0.503	0.050	0.671	0.043	1.476	0.034	1.719	0.015	0.943	0.035	1.223	0.029				
10	6.991	0.077	2.137	0.146	1.965	0.023	2.550	0.075	0.665	0.066	0.889	0.057	1.906	0.044	2.200	0.019	1.230	0.045	1.429	0.034				
25	9.301	0.103	3.117	0.213	2.602	0.030	3.449	0.101	0.892	0.088	1.197	0.077	2.492	0.057	2.854	0.024	1.613	0.060	1.735	0.041				
50	11.148	0.123	3.778	0.259	3.123	0.036	4.084	0.120	1.076	0.107	1.446	0.093	3.011	0.069	3.434	0.029	1.917	0.071	1.990	0.047				
100	13.380	0.148	4.297	0.294	3.784	0.044	4.562	0.134	1.273	0.126	1.713	0.111	3.655	0.084	4.024	0.035	2.218	0.082	2.262	0.054				

Storm Event	Comparison Point		Proposed Development Area (ha)		Comparison Point		Proposed Development Area (ha)		Comparison Point		Proposed Development Area (ha)		Comparison Point		Proposed Development Area (ha)		Comparison Point		Proposed Development Area (ha)		Comparison Point		Proposed Development Area (ha)	
	Peak Flow (m ³ /s)	Unit Flow (m ³ /s/ha)	Peak Flow (m ³ /s)	Unit Flow (m ³ /s/ha)	Peak Flow (m ³ /s)	Unit Flow (m ³ /s/ha)	Peak Flow (m ³ /s)	Unit Flow (m ³ /s/ha)	Peak Flow (m ³ /s)	Unit Flow (m ³ /s/ha)	Peak Flow (m ³ /s)	Unit Flow (m ³ /s/ha)	Peak Flow (m ³ /s)	Unit Flow (m ³ /s/ha)	Peak Flow (m ³ /s)	Unit Flow (m ³ /s/ha)	Peak Flow (m ³ /s)	Unit Flow (m ³ /s/ha)	Peak Flow (m ³ /s)	Unit Flow (m ³ /s/ha)	Peak Flow (m ³ /s)	Unit Flow (m ³ /s/ha)	Peak Flow (m ³ /s)	Unit Flow (m ³ /s/ha)
	9210	176.4	8148	54.4	9342	50.4	9422	52.3	9416	20.4	9423	8.7	515	98.4	516	22.6	518	32.3	524	270.4				
2	1.066	0.006	15.899	0.292	1.136	0.023	1.070	0.020	1.005	0.049	0.574	0.066	3.636	0.037	2.717	0.120	2.650	0.082	2.377	0.009				
5	1.778	0.010	23.258	0.428	1.986	0.039	1.829	0.035	1.882	0.111	0.970	0.111	6.916	0.070	5.348	0.237	4.902	0.152	4.256	0.016				
10	2.313	0.013	29.922	0.550	2.649	0.053	2.438	0.047	2.523	0.124	1.277	0.147	9.457	0.096	7.456	0.330	6.595	0.204	5.660	0.021				
25	3.322	0.019	41.170	0.757	3.570	0.071	3.292	0.063	3.388	0.166	1.702	0.196	12.774	0.130	10.448	0.462	8.888	0.275	7.461	0.028				
50	3.875	0.022	47.333	0.870	4.304	0.085	3.986	0.076	4.090	0.200	2.042	0.235	15.406	0.157	12.890	0.570	10.680	0.331	8.783	0.032				
100	4.443	0.025	53.690	0.987	5.080	0.101	4.726	0.090	4.758	0.233	2.401	0.276	18.126	0.184	15.442	0.683	12.502	0.387	11.309	0.042				

APPENDIX G
Phosphorus Loading Rate Calculations

Existing (Base Case Scenario)

Note: Based on Table 2-3 and Table 2-4 (Estimation of Phosphorus Loadings to Lake Simcoe, Sept 2010)

Subwatershed	Existing Land Use Category (ha)										Total (ha)
	Hay-Pasture	Cropland	Forest	Wetland	Quarry	Turf-Sod	Unpaved Road	Transition	Low Intensity Development	High Intensity Development	
Black River	5822	7734	7919	8756	333	1103	47	2016	2117	1415	37282
East Holland	2796	4282	3903	2498	48	1169	17	1453	1632	6710	24508
Maskinonge	871	2696	558	522	809	0	10	257	182	330	6335
West Holland	4815	12037	5157	2614	6	282	58	1802	1693	2151	30615

Subwatershed	Condensed Existing Land Use Category (ha)										Total (ha)
	Agriculture	Forest	Open Water / Wetland	Rural Development, Estate	Open Space	Low Density Residential, Medium Density Residential, Commercial / Institutional	Transition	Low Intensity Development	High Intensity Development		
Black River	14962	7919	8756	2164	2016	1415	2016	2117	1415	37262	
East Holland	8295	3903	2498	1649	1453	6710	1453	1632	6710	24508	
Maskinonge	4476	558	522	192	257	330	257	182	330	6335	
West Holland	17140	5157	2614	1751	1802	2151	1802	1693	2151	30615	

Subwatershed	Base Case Average Annual Total Phosphorus (kg/year)										Total (ha)
	Agriculture	Forest	Open Water / Wetland	Rural Development, Estate	Open Space	Low Density Residential, Medium Density Residential, Commercial / Institutional	Transition	Low Intensity Development	High Intensity Development		
Black River	1938	9	1219	332	687	556	687	4425	4739	4739	
East Holland	2084	16	855	68	560	4425	560	112	8008	8008	
Maskinonge	562	0	351	16	3	112	3	1064	1064	1064	
West Holland	2834	13	1716	37	2189	528	2189	528	7317	7317	

Subwatershed	Base Case Average Annual Total Phosphorus Loading Rate (kg/ha)									
	Agriculture	Forest	Open Water / Wetland	Rural Development, Estate	Open Space	Low Density Residential, Medium Density Residential, Commercial / Institutional	Transition	Low Intensity Development	High Intensity Development	
Black River	0.1291	0.0011	0.1392	0.1534	0.3408	0.3929	0.3408	0.3929	0.3929	
East Holland	0.2512	0.0041	0.3423	0.0412	0.3854	0.6595	0.3854	0.6595	0.6595	
Maskinonge	0.1300	0.0000	0.6724	0.0833	0.0117	0.3394	0.0117	0.3394	0.3394	
West Holland	0.1653	0.0025	0.6565	0.0211	1.2148	0.2455	1.2148	0.2455	0.2455	

Future (Growth Scenario)

Note: Based on Table 3-1 and Table 3-2 (Estimation of Phosphorus Loadings to Lake Simcoe, Sept 2010)

Subwatershed	Growth Land Use Category (ha)										Total (ha)
	Hay-Pasture	Cropland	Forest	Wetland	Quarry	Turf-Sod	Unpaved Road	Transition	Low Intensity Development	High Intensity Development	
Black River	5667	7476	7665	8758	333	852	47	1832	2117	2618	37263
East Holland	2173	3071	3060	2494	37	1028	17	1125	1632	9850	24507
Maskinonge	834	2409	531	522	803	0	10	233	182	811	6335
West Holland	4421	11187	4817	2614	3	260	58	1612	1693	3949	30614

Subwatershed	Condensed Growth Case Land Use Category (ha)										Total (ha)
	Agriculture	Open Water / Wetland	Rural Development, Estate	Open Space	Low Density Residential, Commercial / Institutional	Medium Density Residential, Commercial / Institutional	High Intensity Development				
Black River	14228	7665	2164	1832	2818	9850	37263				
East Holland	6309	3080	1649	1125	2818	9850	24507				
Maskinonge	4046	531	192	233	611	6335	6335				
West Holland	15871	4817	1751	1612	3949	30614	30614				

Subwatershed	Growth Case Average Annual Total Phosphorus (kg/year)										Total (ha)
	Agriculture	Open Water / Wetland	Rural Development, Estate	Open Space	Low Density Residential, Commercial / Institutional	Medium Density Residential, Commercial / Institutional	High Intensity Development				
Black River	1885	8	332	685	1902	5993	5993				
East Holland	1557	12	88	538	6335	12584	12584				
Maskinonge	520	0	13	3	611	1472	1472				
West Holland	2641	12	37	2198	1760	8365	8365				

Subwatershed	Growth Case Average Annual Total Phosphorus Loading Rate (kg/ha)									
	Agriculture	Open Water / Wetland	Rural Development, Estate	Open Space	Low Density Residential, Commercial / Institutional	Medium Density Residential, Commercial / Institutional	High Intensity Development			
Black River	0.1311	0.0010	0.1372	0.1534	0.3739	0.7265	0.7265			
East Holland	0.2468	0.0039	0.0412	0.4782	0.9661	0.9661	0.9661			
Maskinonge	0.1285	0.0000	0.0628	0.0877	0.0129	0.7534	0.7534			
West Holland	0.1664	0.0025	0.0211	1.3573	0.4597	0.4597	0.4597			

APPENDIX H

Existing Conditions Phosphorus Loading Calculations

Existing Phosphorus Loading Calculations

Catchment No.	Subwatershed	Hydrologic Soil Group	Land Use Category										Total (kg/year)
			Agriculture (ha)	Forest (ha)	Wetland/Open Water (ha)	Rural Development (ha)	Residential (ha)	Estate (ha)	Commercial (ha)	Institutional (ha)	Open Space (ha)		
Mount Albert													
3	Black River	Type D Bottomland Type B Type C	0.0 0.0 89.7 59.5	5.7 0.0 1.1 0.0	0.0 0.0 6.5 0.0	0.0 0.0 0.0 0.0	0.0 0.0 57.2 29.8	0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	0.0 19.1 0.0 0.0	
Phosphorus Loading Rate (kg/ha)			0.129	0.001	0.139	0.153	0.393	0.153	0.393	0.393	0.393	0.341	
TP (kg/year)			19.3	0.0	0.9	0.0	34.2	0.0	0.0	0.0	0.0	60.9	
4	Black River	Type B Type C Type A	4.1 0.6 19.1	8.0 1.6 5.4	0.0 2.6 5.7	0.0 0.0 0.0	4.7 14.9 37.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	
Phosphorus Loading Rate (kg/ha)			0.129	0.001	0.139	0.153	0.393	0.153	0.393	0.393	0.393	0.341	
TP (kg/year)			3.1	0.0	1.2	0.0	22.2	0.0	0.0	0.0	0.0	26.5	
Sharon													
218	East Holland	Type B Bottomland Type C	10.2 0.0 27.3	0.0 0.0 27.7	0.0 6.1 0.0	0.0 0.0 0.0	0.0 0.0 7.6	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	
Phosphorus Loading Rate (kg/ha)			0.251	0.004	0.342	0.041	0.659	0.041	0.659	0.659	0.659	0.385	
TP (kg/year)			9.4	0.1	2.1	0.0	5.0	0.0	0.0	0.0	0.0	16.6	
216	East Holland	Type B Bottomland Type C Type A	0.8 0.0 34.2 0.0	0.0 0.0 2.6 0.0	0.0 4.2 0.0 0.0	0.0 0.0 0.0 0.0	0.0 0.0 27.1 6.2	0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	
Phosphorus Loading Rate (kg/ha)			0.251	0.004	0.342	0.041	0.659	0.041	0.659	0.659	0.659	0.385	
TP (kg/year)			8.8	0.0	1.4	0.0	22.0	0.0	0.0	0.0	0.0	32.2	
214	East Holland	Bottomland Type C Type B Type A	0.0 21.1 0.0 0.0	0.0 11.4 0.0 0.0	7.6 0.0 1.6 0.0	0.0 0.0 0.0 0.0	0.0 24.5 89.2 0.0	0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	
Phosphorus Loading Rate (kg/ha)			0.251	0.004	0.342	0.041	0.659	0.041	0.659	0.659	0.659	0.385	
TP (kg/year)			8.8	0.0	1.4	0.0	22.0	0.0	0.0	0.0	0.0	32.2	
TOTAL													
87.4													

APPENDIX I

Future Conditions Phosphorus Loading Calculations

Future Phosphorus Loading Calculations

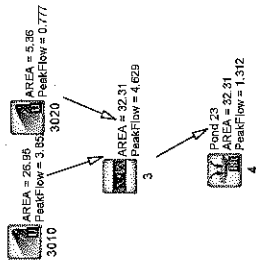
Catchment No.	Subwatershed	Hydrologic Soil Group	Land Use Category										Total (kg/year)
			Agriculture (ha)	Forest (ha)	Wetland / Open Water (ha)	Med. Density Residential (ha)	Low Density Residential (ha)	Estate (ha)	Institutional (ha)	Commercial (ha)	Open Space (ha)		
Mount Albert													
3B	Black River	Type C Type B Type D Bottomland Type A	0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	0.0 4.9 5.7 3.4 0.8	0.0 1.9 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0	62.2 84.8 0.0 4.4 21.1	0.0 0.0 0.0 0.0 0.0	0.0 7.7 0.0 0.0 0.0	0.0 0.0 0.0 0.3 3.0	22.6 41.0 0.0 15.1 1.2	
Phosphorus Loading Rate (kg/ha) TP (kg/year)			0.131	0.001	0.137	0.727	0.153	0.727	0.727	0.727	0.727	0.374	
4B	Black River	Type C Type B Type A	0.0 2.7 0.0 6.2	0.0 0.0 0.0 0.0	2.0 0.0 0.0 0.0	1.4 0.0 0.0 0.0	0.0 0.0 0.0 0.0	125.3 14.5 8.2 19.1	0.0 0.0 0.0 0.0	5.6 0.0 2.6 2.2	2.5 7.1 0.3 25.1	29.8 0.1 3.5 0.5	
Phosphorus Loading Rate (kg/ha) TP (kg/year)			0.131	0.001	0.137	0.727	0.153	0.727	0.727	0.727	0.727	0.374	
TOTAL			1.2	0.0	0.0	0.0	0.0	30.4	0.0	3.5	23.6	60.3	
Sharon													
218	East Holland	Type D Bottomland Type B	0.0 0.0 0.0	11.3 4.9 1.9	0.3 0.4 0.0	0.0 0.0 0.0	0.0 0.0 0.0	42.2 0.0 7.5	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.9 0.0	
Phosphorus Loading Rate (kg/ha) TP (kg/year)			0.247	0.004	0.350	0.968	0.041	0.968	0.041	0.968	0.968	0.478	
210	East Holland	Type D	0.0	0.0	0.0	0.0	0.0	11.1	0.0	0.0	0.0	0.0	
Phosphorus Loading Rate (kg/ha) TP (kg/year)			0.247	0.004	0.350	0.968	0.041	0.968	0.041	0.968	0.968	0.478	
214	East Holland	Type D Type B Type A Bottomland	0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	8.3 1.6 0.0 4.1	0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	10.7 49.7 85.6 3.0	0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0	0.0 0.5 1.6 0.0 3.9	
Phosphorus Loading Rate (kg/ha) TP (kg/year)			0.247	0.004	0.350	0.968	0.041	0.968	0.041	0.968	0.968	0.478	
7B	Black River	Type B Type A Type D	0.0 0.0 0.0	0.0 0.0 0.0	4.9 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	133.9 27.8 105.6	0.0 0.0 0.0	0.0 0.0 1.8	0.0 0.0 0.0	2.9 8.0 12.5	
Phosphorus Loading Rate (kg/ha) TP (kg/year)			0.0	0.0	0.0	0.0	0.0	23.2	0.0	0.0	0.0	141.7	

APPENDIX J
Water Balance Calculations

Item	Code	Unit	QTY	UNIT PRICE	AMOUNT	TAX	TOTAL	DISCOUNT	NET	AMOUNT	TAX	TOTAL
410	Type C	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
410	Type B	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
410	Type A	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
410	Type D	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
224	Type C	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
224	Type B	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
224	Type A	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
224	Type D	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
208	Type C	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
208	Type B	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
208	Type A	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
208	Type D	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
210	Type C	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
210	Type B	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
210	Type A	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
210	Type D	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
214	Type C	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
214	Type B	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
214	Type A	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
214	Type D	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
7	Type C	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
7	Type B	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
7	Type A	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
7	Type D	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Total										0.0	0.0	0.0
Total Invoice Price										0.0	0.0	0.0

APPENDIX K
Sensitivity Analysis Hydrologic Model Output

POND 23 - BASE FILE



V V I SSSSS U U A A L
 V V I SS U U A A L
 V V I SS U U A A A A L
 V V I SS U U A A L
 V V I SSSSS UUUU A A LLLL

OOO TTTT TTTT H H Y Y M M OOO
 O O T T H H Y Y M M O O
 O O T T H H Y Y M M O O
 OOO T T H H Y Y M M OOO

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***** D E T A I L E D O U T P U T *****

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COMMENTS:

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TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	0.0	3.50	20.52	6.75	8.45	10.00	1.21
.50	1.21	3.75	20.52	7.00	8.45	10.25	1.21
.75	1.21	4.00	20.52	7.25	8.45	10.50	1.21
1.00	1.21	4.25	20.52	7.50	4.83	10.75	1.21
1.25	1.21	4.50	55.52	7.75	4.83	11.00	1.21
1.50	1.21	4.75	55.52	8.00	4.83	11.25	1.21
1.75	1.21	5.00	55.52	8.25	4.83	11.50	1.21
2.00	1.21	5.25	55.52	8.50	2.41	11.75	1.21
2.25	1.21	5.50	15.69	8.75	2.41	12.00	1.21
2.50	7.24	5.75	15.69	9.00	2.41	12.25	1.21
2.75	7.24	6.00	15.69	9.25	2.41		
3.00	7.24	6.25	15.69	9.50	1.21		
3.25	7.24	6.50	8.45	9.75	1.21		

CALLIB

STANDRD (3020) | Area (ha) = 5.36 | Dlt. Conn. (\$) = 87.00
 ID= 1 pr= 5.0 min | Total Imp (%) = 87.00

Surface Area (ha) = 1.66
 Dep. Storage (mm) = 1.00
 Average Slope (%) = 1.00
 Length (m) = 189.00
 Manning's n = .013

NOTE: RAINFALL WAS TRANSDERRED TO 5.0 MIN. TIME STEP.

TIME	TRANSFORMED HYETOGRAPH				TIME
	RAIN	TIME	RAIN	TIME	
0.083	0.00	3.167	7.24	6.250	0.083
1.67	0.00	3.230	7.24	6.333	1.67
3.30	0.00	3.333	20.52	6.417	3.30
4.97	1.21	3.417	20.52	6.500	4.97
6.60	1.21	3.583	20.52	6.583	6.60
8.23	1.21	3.667	20.52	6.750	8.23
9.87	1.21	3.750	20.52	6.833	9.87
11.50	1.21	3.833	20.52	6.917	11.50
13.13	1.21	3.917	20.52	7.000	13.13
14.77	1.21	4.000	20.52	7.083	14.77
16.40	1.21	4.083	20.52	7.167	16.40
18.03	1.21	4.167	20.52	7.250	18.03
19.67	1.21	4.250	20.52	7.333	19.67
21.30	1.21	4.333	55.52	7.417	21.30
22.93	1.21	4.417	55.52	7.500	22.93
24.57	1.21	4.500	55.52	7.583	24.57
26.20	1.21	4.583	55.52	7.667	26.20
27.83	1.21	4.667	55.52	7.750	27.83
29.47	1.21	4.750	55.52	7.833	29.47
31.10	1.21	4.833	55.52	7.917	31.10
32.73	1.21	4.917	55.52	8.000	32.73
34.37	1.21	5.000	55.52	8.083	34.37
36.00	1.21	5.083	55.52	8.167	36.00
37.63	1.21	5.167	55.52	8.250	37.63
39.27	1.21	5.250	55.52	8.333	39.27
40.90	1.21	5.333	55.52	8.417	40.90
42.53	1.21	5.417	55.52	8.500	42.53
44.17	1.21	5.500	55.52	8.583	44.17
45.80	1.21	5.583	55.52	8.667	45.80
47.43	1.21	5.667	55.52	8.750	47.43
49.07	1.21	5.750	55.52	8.833	49.07
50.70	1.21	5.833	55.52	8.917	50.70
52.33	1.21	5.917	55.52	9.000	52.33
53.97	1.21	6.000	55.52	9.083	53.97
55.60	1.21	6.083	55.52	9.167	55.60
57.23	1.21	6.167	55.52	9.250	57.23

Max. Eff. Inten. (mm/hr) = 55.52
 Storage Coeff. (mm) = 5.90
 Utlc Hyd. Peak (mm) = 4.74 (4.1)
 Utlc Hyd. Peak (mm) = 5.00
 Utlc Hyd. Peak (mm) = 4.22
 PRK FLOW (cms) = .72
 TIME TO PRK (hrs) = 5.25
 RNOFF VOLUME (mm) = 119.71
 TOTAL RAINFALL (mm) = 120.71
 RNOFF COEFFICIENT = .99

WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
 CN* = 62.0 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

FROM THE STORAGE COEFFICIENT
 (11) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Q/ALT | Area (ha) = 26.45
 STANDRD (3010) | Total Imp (%) = 79.00
 ID= 1 pr= 5.0 min | Dlt. Conn. (\$) = 79.00

Surface Area (ha) = 21.29
 Dep. Storage (mm) = 1.00
 Average Slope (%) = 1.50
 Length (m) = 423.90
 Manning's n = .013

TIME	TRANSFORMED HYETOGRAPH				TIME
	RAIN	TIME	RAIN	TIME	
0.083	0.00	3.167	7.24	6.250	0.083
1.67	0.00	3.230	7.24	6.333	1.67
3.30	0.00	3.333	20.52	6.417	3.30
4.97	1.21	3.417	20.52	6.500	4.97
6.60	1.21	3.583	20.52	6.583	6.60
8.23	1.21	3.667	20.52	6.750	8.23
9.87	1.21	3.750	20.52	6.833	9.87
11.50	1.21	3.833	20.52	6.917	11.50
13.13	1.21	3.917	20.52	7.000	13.13
14.77	1.21	4.000	20.52	7.083	14.77
16.40	1.21	4.083	20.52	7.167	16.40
18.03	1.21	4.167	20.52	7.250	18.03
19.67	1.21	4.250	20.52	7.333	19.67
21.30	1.21	4.333	55.52	7.417	21.30
22.93	1.21	4.417	55.52	7.500	22.93
24.57	1.21	4.500	55.52	7.583	24.57
26.20	1.21	4.583	55.52	7.667	26.20
27.83	1.21	4.667	55.52	7.750	27.83
29.47	1.21	4.750	55.52	7.833	29.47
31.10	1.21	4.833	55.52	7.917	31.10
32.73	1.21	4.917	55.52	8.000	32.73
34.37	1.21	5.000	55.52	8.083	34.37
36.00	1.21	5.083	55.52	8.167	36.00
37.63	1.21	5.167	55.52	8.250	37.63
39.27	1.21	5.250	55.52	8.333	39.27
40.90	1.21	5.333	55.52	8.417	40.90
42.53	1.21	5.417	55.52	8.500	42.53
44.17	1.21	5.500	55.52	8.583	44.17
45.80	1.21	5.583	55.52	8.667	45.80
47.43	1.21	5.667	55.52	8.750	47.43
49.07	1.21	5.750	55.52	8.833	49.07
50.70	1.21	5.833	55.52	8.917	50.70
52.33	1.21	5.917	55.52	9.000	52.33
53.97	1.21	6.000	55.52	9.083	53.97
55.60	1.21	6.083	55.52	9.167	55.60
57.23	1.21	6.167	55.52	9.250	57.23

Max. Eff. Inten. (mm/hr) = 55.52
 Storage Coeff. (mm) = 5.90
 Utlc Hyd. Peak (mm) = 4.81 (4.1)
 Utlc Hyd. Peak (mm) = 5.00
 Utlc Hyd. Peak (mm) = 4.18
 PRK FLOW (cms) = 3.28
 TIME TO PRK (hrs) = 5.25
 RNOFF VOLUME (mm) = 119.71
 TOTAL RAINFALL (mm) = 120.71
 RNOFF COEFFICIENT = .99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA	QPEAK	TPEAK	R.V.	OUTFLOW	
					(cms)	(ha, m.)
1	76.0	1777	110.87	5.25	1.5400	1.5400
2	76.0	1777	110.87	5.25	1.5400	1.5400
3	76.0	1777	110.87	5.25	1.5400	1.5400
4	76.0	1777	110.87	5.25	1.5400	1.5400
5	76.0	1777	110.87	5.25	1.5400	1.5400
6	76.0	1777	110.87	5.25	1.5400	1.5400
7	76.0	1777	110.87	5.25	1.5400	1.5400
8	76.0	1777	110.87	5.25	1.5400	1.5400
9	76.0	1777	110.87	5.25	1.5400	1.5400
10	76.0	1777	110.87	5.25	1.5400	1.5400
11	76.0	1777	110.87	5.25	1.5400	1.5400
12	76.0	1777	110.87	5.25	1.5400	1.5400
13	76.0	1777	110.87	5.25	1.5400	1.5400
14	76.0	1777	110.87	5.25	1.5400	1.5400
15	76.0	1777	110.87	5.25	1.5400	1.5400
16	76.0	1777	110.87	5.25	1.5400	1.5400
17	76.0	1777	110.87	5.25	1.5400	1.5400
18	76.0	1777	110.87	5.25	1.5400	1.5400
19	76.0	1777	110.87	5.25	1.5400	1.5400
20	76.0	1777	110.87	5.25	1.5400	1.5400
21	76.0	1777	110.87	5.25	1.5400	1.5400
22	76.0	1777	110.87	5.25	1.5400	1.5400
23	76.0	1777	110.87	5.25	1.5400	1.5400
24	76.0	1777	110.87	5.25	1.5400	1.5400
25	76.0	1777	110.87	5.25	1.5400	1.5400
26	76.0	1777	110.87	5.25	1.5400	1.5400
27	76.0	1777	110.87	5.25	1.5400	1.5400
28	76.0	1777	110.87	5.25	1.5400	1.5400
29	76.0	1777	110.87	5.25	1.5400	1.5400
30	76.0	1777	110.87	5.25	1.5400	1.5400
31	76.0	1777	110.87	5.25	1.5400	1.5400
32	76.0	1777	110.87	5.25	1.5400	1.5400
33	76.0	1777	110.87	5.25	1.5400	1.5400
34	76.0	1777	110.87	5.25	1.5400	1.5400
35	76.0	1777	110.87	5.25	1.5400	1.5400
36	76.0	1777	110.87	5.25	1.5400	1.5400
37	76.0	1777	110.87	5.25	1.5400	1.5400
38	76.0	1777	110.87	5.25	1.5400	1.5400
39	76.0	1777	110.87	5.25	1.5400	1.5400
40	76.0	1777	110.87	5.25	1.5400	1.5400
41	76.0	1777	110.87	5.25	1.5400	1.5400
42	76.0	1777	110.87	5.25	1.5400	1.5400
43	76.0	1777	110.87	5.25	1.5400	1.5400
44	76.0	1777	110.87	5.25	1.5400	1.5400
45	76.0	1777	110.87	5.25	1.5400	1.5400
46	76.0	1777	110.87	5.25	1.5400	1.5400
47	76.0	1777	110.87	5.25	1.5400	1.5400
48	76.0	1777	110.87	5.25	1.5400	1.5400
49	76.0	1777	110.87	5.25	1.5400	1.5400
50	76.0	1777	110.87	5.25	1.5400	1.5400
51	76.0	1777	110.87	5.25	1.5400	1.5400
52	76.0	1777	110.87	5.25	1.5400	1.5400
53	76.0	1777	110.87	5.25	1.5400	1.5400
54	76.0	1777	110.87	5.25	1.5400	1.5400
55	76.0	1777	110.87	5.25	1.5400	1.5400
56	76.0	1777	110.87	5.25	1.5400	1.5400
57	76.0	1777	110.87	5.25	1.5400	1.5400
58	76.0	1777	110.87	5.25	1.5400	1.5400
59	76.0	1777	110.87	5.25	1.5400	1.5400
60	76.0	1777	110.87	5.25	1.5400	1.5400

MAXIMUM STORAGE USED (hr.m.) = 2.3738

POND 23 - 6HR STORM

FINISH

```

V V I SSSSS V U A L
V V I SS U U A A L
V V I SS U U A A A L
V V I SS U U A A L
V V I SSSSS UUUUU A A LLLL
OOO TTTTTT H H Y Y M M OOO
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
OOO T T H H Y Y M M OOO

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 ** SIMULATION NUMBER: 1 **

File names: S:\2010 Projects\Dr-EE (E10)\
 Calcs\Hydrologic Models\Sensitivity Analysis\
 Comments: 100yr/6hr

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
1.25	1.21	1.75	20.52	3.25	9.45
.50	1.21	2.00	20.52	3.50	9.45
1.75	1.21	2.25	55.52	3.75	4.83
1.00	1.21	2.50	55.52	4.00	4.83
1.25	7.24	2.75	15.69	4.25	2.41
1.50	7.24	3.00	15.69	4.50	2.41

CALIB (3020) Area (ha) = 5.36
 STANDBY ID = 1 DT = 5.0 min Total Imp (t) = 87.00 DLX. Conn. (t) = 87.00
 IMPERVIOUS PERVIOUS (I)

NOTE: RAINFALL WAS TRANSFERRED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
1.21	1.583	20.52	3.083	8.45	4.58	1.21	1.21
1.67	1.21	1.667	20.52	3.167	8.45	1.21	1.21
2.50	1.21	1.750	20.52	3.250	8.45	1.21	1.21
3.33	1.21	1.833	20.52	3.333	8.45	1.21	1.21
4.17	1.21	1.917	20.52	3.417	8.45	1.21	1.21
5.00	1.21	2.000	20.52	3.500	8.45	1.21	1.21
5.83	1.21	2.083	20.52	3.583	8.45	1.21	1.21
6.67	1.21	2.167	20.52	3.667	8.45	1.21	1.21
7.50	1.21	2.250	20.52	3.750	8.45	1.21	1.21
8.33	1.21	2.333	20.52	3.833	8.45	1.21	1.21
9.17	1.21	2.417	20.52	3.917	8.45	1.21	1.21
10.00	1.21	2.500	20.52	4.000	8.45	1.21	1.21
1.083	1.21	2.583	15.69	4.083	2.41	5.58	1.21
1.167	1.21	2.667	15.69	4.167	2.41	5.67	1.21
1.250	1.21	2.750	15.69	4.250	2.41	5.75	1.21
1.333	1.21	2.833	15.69	4.333	2.41	5.83	1.21
1.417	1.21	2.917	15.69	4.417	2.41	5.92	1.21
1.500	1.21	3.000	15.69	4.500	2.41	6.00	1.21

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 (1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
 CN* = 62.0 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

STANDARD (3010)	Area (ha)	Dlr. Coeff. (%)	Total Imp (%)
STANDARD (3010)	26.95		79.00
Imp 1 DT= 5.0 min	79.00		

IMPERVIOUS	PERVIOUS (%)
Surface Area (ha)	21.729
Dep. Storage (mm)	1.07
Average Slope (%)	1.50
Length (m)	423.90
Manning's n	.013

Max. Eff. Inten. (mm/hr)	over (min)	Storage Coeff. (min)	Unit Hyd. Peak (mm)
55.52	28.23	20.00	18.51 (11)
5.00	5.00	18.51 (11)	5.00
5.00	5.00	20.00	1.18
5.00	5.00	2.41	0.06
5.00	5.00	6.00	0.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
2.50	59.35	60.36	.98
2.67	24.31	60.36	.41
2.50	52.12	60.36	.86

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
32.31	4.272	2.50	52.39
32.31	4.272	2.50	52.12
32.31	4.272	2.50	52.12

INFLOW (ID= 2 (0003))	OUTFLOW (ID= 1 (0004))	PEAK FLOW REDUCTION [Out/In] (%)	TIME SHIFT OF PEAK FLOW (min)	MAXIMUM STORAGE USED (ha.m.)
32.31	32.31	7.55	95.00	1.4025

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.000	0.000	3.340	1.5400
0.140	1.520	3.340	1.7350
0.370	3.070	3.630	2.1930
0.580	4.650	3.760	2.1330
0.730	6.270	1.0880	2.3970
0.860	7.950	2.3790	2.5440
0.970	9.750	4.0460	2.5740
1.070	11.650	6.0700	2.5670
1.160	13.650	8.3800	2.5200
1.240	15.750	10.9000	2.4400

FINISH

POND 23 - 24HR STORM

V V I SSSSS U U A L
 V V I SS U U A A L
 V V I SS U U A A A A A L
 V V I SS U U A A L
 W I SSSSS UUUU A A LLLLL

OOO TTTTT H H Y Y M M
 O O T T H H Y Y M M
 O O T T H H Y Y M M
 O O T T H H Y Y M M

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***** D E T A I L E D O U T P U T *****

Input filename: C:\Program Files\Visual FORTRAN 2.3.2\voan.dat
 Output filename: S:\2010 Projects\UP-EE (EIO)\WRW10-487 East Gwillimbury SWM Master
 Plan\Calce\Hydrologic Models\Sensitivity Analysis\Bas
 Summary filename: S:\2010 Projects\UP-EE (EIO)\WRW10-487 East Gwillimbury SWM Master
 Plan\Calce\Hydrologic Models\Sensitivity Analysis\Bas

DATE: 3/30/2011 TIME: 11:40:43 AM

USER:

COMMENTS:

 *** SIMULATION NUMBER: 1 ***

--- READ STORM ---
 --- Pictorial=130.88 mm ---
 --- Filename: S:\2010 Projects\UP-EE (EIO)\
 Calce\Hydrologic Models\Sensitivity Analysis\

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	0.00	6.50	11.13	12.75	4.58	19.00	6.5
0.50	6.5	7.00	11.13	13.00	4.58	19.25	6.5
1.00	6.5	7.25	11.13	13.25	4.58	19.50	6.5
1.25	6.5	7.50	11.13	13.50	4.58	19.75	6.5
1.50	6.5	7.75	11.13	13.75	4.58	20.00	6.5
1.75	6.5	8.00	11.13	14.00	4.58	20.25	6.5
2.00	6.5	8.25	11.13	14.25	4.58	20.50	6.5
2.25	6.5	8.50	11.13	14.50	4.58	20.75	6.5
2.50	6.5	8.75	11.13	14.75	4.58	21.00	6.5
2.75	6.5	9.00	11.13	15.00	4.58	21.25	6.5
3.00	6.5	9.25	11.13	15.25	4.58	21.50	6.5
3.25	6.5	9.50	11.13	15.50	4.58	21.75	6.5
3.50	6.5	9.75	11.13	15.75	4.58	22.00	6.5
3.75	6.5	10.00	11.13	16.00	4.58	22.25	6.5
4.00	6.5	10.25	11.13	16.25	4.58	22.50	6.5

4.00	6.5	10.25	30.11	16.50	1.31	22.75	6.5
4.25	3.93	10.50	8.51	19.75	1.31	23.00	6.5
4.50	3.93	10.75	8.51	17.25	1.31	23.25	6.5
4.75	3.93	11.00	8.51	17.50	1.31	23.50	6.5
5.00	3.93	11.25	8.51	17.50	1.31	24.00	6.5
5.25	3.93	11.50	8.51	17.75	1.31	24.00	6.5
5.50	3.93	11.75	8.51	18.00	1.31	24.25	6.5
5.75	3.93	12.00	8.51	18.25	1.31	24.25	6.5
6.00	3.93	12.25	8.51	18.50	1.31	24.25	6.5
6.25	3.93	12.50	4.58	18.75	1.31	24.25	6.5

--- CALIB ---
 --- SPANDRID (3020) ---
 --- ID= 1 DF= 5.0 mth ---
 Area (ha)= 5.36
 Total Imp(%)= 87.00 Dlr. Conn.(%)= 87.00

--- IMPERVIOUS ---
 Surface Area (ha)= 4.66
 Dep. Storage (mm)= 1.70
 Average Slope (%)= 1.00
 Length (m)= 189.00
 Manning's n = .250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	0.00	6.167	3.93	12.250	8.51	18.33	6.5
0.167	0.00	6.330	3.93	12.333	4.58	18.42	6.5
0.250	0.00	6.333	11.13	12.417	4.58	18.50	6.5
0.333	6.5	6.417	11.13	12.500	4.58	18.57	6.5
0.417	6.5	6.500	11.13	12.583	4.58	18.65	6.5
0.500	6.5	6.583	11.13	12.667	4.58	18.75	6.5
0.583	6.5	6.667	11.13	12.750	4.58	18.82	6.5
0.667	6.5	6.750	11.13	12.833	4.58	18.92	6.5
0.750	6.5	6.833	11.13	12.917	4.58	19.00	6.5
0.833	6.5	6.917	11.13	13.000	4.58	19.08	6.5
0.917	6.5	7.000	11.13	13.083	4.58	19.17	6.5
1.000	6.5	7.083	11.13	13.167	4.58	19.25	6.5
1.083	6.5	7.167	11.13	13.250	4.58	19.33	6.5
1.167	6.5	7.250	11.13	13.333	4.58	19.42	6.5
1.250	6.5	7.333	11.13	13.417	4.58	19.50	6.5
1.333	6.5	7.417	11.13	13.500	4.58	19.58	6.5
1.417	6.5	7.500	11.13	13.583	4.58	19.67	6.5
1.500	6.5	7.583	11.13	13.667	4.58	19.75	6.5
1.583	6.5	7.667	11.13	13.750	4.58	19.83	6.5
1.667	6.5	7.750	11.13	13.833	4.58	19.92	6.5
1.750	6.5	7.833	11.13	13.917	4.58	20.00	6.5
1.833	6.5	7.917	11.13	14.000	4.58	20.08	6.5
1.917	6.5	8.000	11.13	14.083	4.58	20.17	6.5
2.000	6.5	8.083	11.13	14.167	4.58	20.25	6.5
2.083	6.5	8.167	11.13	14.250	4.58	20.33	6.5
2.167	6.5	8.250	11.13	14.333	4.58	20.42	6.5
2.250	6.5	8.333	11.13	14.417	4.58	20.50	6.5
2.333	6.5	8.417	11.13	14.500	4.58	20.58	6.5
2.417	6.5	8.500	11.13	14.583	4.58	20.67	6.5
2.500	6.5	8.583	11.13	14.667	4.58	20.75	6.5
2.583	6.5	8.667	11.13	14.750	4.58	20.83	6.5
2.667	6.5	8.750	11.13	14.833	4.58	20.92	6.5
2.750	6.5	8.833	11.13	14.917	4.58	21.00	6.5
2.833	6.5	8.917	11.13	15.000	4.58	21.08	6.5
2.917	6.5	9.000	11.13	15.083	4.58	21.17	6.5
3.000	6.5	9.083	11.13	15.167	4.58	21.25	6.5
3.083	6.5	9.167	11.13	15.250	4.58	21.33	6.5
3.167	6.5	9.250	11.13	15.333	4.58	21.42	6.5
3.250	6.5	9.333	11.13	15.417	4.58	21.50	6.5
3.333	6.5	9.417	11.13	15.500	4.58	21.58	6.5

PEAK FLOW (cms)	TIME TO PEAK (hrs)	RUNOFF VOLUME (mm)	TOTAL RAINFALL (mm)	RUNOFF COEFFICIENT
3.417	.65	9.500	30.11	115.583
3.500	.65	9.467	30.11	115.467
3.583	.65	9.433	30.11	115.350
3.667	.65	9.400	30.11	115.233
3.750	.65	9.367	30.11	115.117
3.833	.65	9.333	30.11	115.000
3.917	.65	9.300	30.11	114.883
4.000	.65	9.267	30.11	114.767
4.083	.65	9.233	30.11	114.650
4.167	.65	9.200	30.11	114.533
4.250	.65	9.167	30.11	114.417
4.333	.65	9.133	30.11	114.300
4.417	.65	9.100	30.11	114.183
4.500	.65	9.067	30.11	114.067
4.583	.65	9.033	30.11	113.950
4.667	.65	9.000	30.11	113.833
4.750	.65	8.967	30.11	113.717
4.833	.65	8.933	30.11	113.600
4.917	.65	8.900	30.11	113.483
5.000	.65	8.867	30.11	113.367
5.083	.65	8.833	30.11	113.250
5.167	.65	8.800	30.11	113.133
5.250	.65	8.767	30.11	113.017
5.333	.65	8.733	30.11	112.900
5.417	.65	8.700	30.11	112.783
5.500	.65	8.667	30.11	112.667
5.583	.65	8.633	30.11	112.550
5.667	.65	8.600	30.11	112.433
5.750	.65	8.567	30.11	112.317
5.833	.65	8.533	30.11	112.200
5.917	.65	8.500	30.11	112.083
6.000	.65	8.467	30.11	111.967
6.083	.65	8.433	30.11	111.850

Max Eff. Inten. (mm/hr) = 30.11
 Storage Coeff. (m) = 1.00
 Unit Hyd. Peak (mm) = 10.00
 Unit Hyd. Peak (mm) = 10.00
 RINOFF COEFFICIENT = .99

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 62.0 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

AREA (ha)	QPEAK (cms)	TPPEAK (hrs)	R.V. (mm)
26.95	32.31	10.25	119.58
79.00	32.31	10.25	119.37

INFLOW: ID= 1 (0003) 32.31
 OUTFLOW: ID= 1 (0004) 32.31

PEAK FLOW REDUCTION [Out/In] (%) = 39.58
 TIME SHIFT OF PEAK FLOW (min) = 25.00
 MAXIMUM STORAGE USED (ha.m.) = 2.3143

AREA (ha)	QPEAK (cms)	TPPEAK (hrs)	R.V. (mm)
32.31	32.31	10.25	119.58
1.01	1.01	10.67	119.37

INFLOW: ID= 2 (0003) 32.31
 OUTFLOW: ID= 2 (0004) 1.01

PEAK FLOW REDUCTION [Out/In] (%) = 39.58
 TIME SHIFT OF PEAK FLOW (min) = 25.00
 MAXIMUM STORAGE USED (ha.m.) = 2.3143

FINISH

2.583	5.43	5.67	11.77	8.750	1.81	11.83	.91
2.667	5.43	5.750	11.77	8.833	1.81	11.92	.91
2.750	5.43	5.833	11.77	8.917	1.81	12.00	.91
2.833	5.43	5.917	11.77	9.000	1.81	12.08	.91
2.917	5.43	6.000	11.77	9.083	1.81	12.17	.91
3.000	5.43	6.083	11.77	9.167	1.81	12.25	.91
3.083	5.43	6.167	11.77	9.250	1.81		
Max. Eff. Inten. (mm/hr) =	41.64	103.48					
Storage Coeff. (mm) =	5.00	10.00					
Unit Hyd. Peak (mm) =	5.00	10.00	(11)				
Unit Hyd. Peak (cms) =	.21	.11					
PEAK FLOW (cms) =	5.4	.04					
PEAK FLOW (ft ³ /s) =	5.25	5.25					
RUNOFF VOLUME (mm) =	89.53	32.39					
TOTAL RAINFALL (mm) =	90.53	90.53					
RUNOFF COEFFICIENT =	.99	.36					

(1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSSES:
 CN* = 62.0 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALLS	Area (ha) =	26.95	Dly. Conn. (%) =	79.00
STANDYD (3010)	Total Imp(%) =	79.00		
ID= 1 DE= 5.0 m/h				
Surface Area (ha) =	21.29			
Dep. Storage (mm) =	1.00			
Average Slope (%) =	1.50			
Length (m) =	423.90			
Manning's n =	.013			
Max. Eff. Inten. (mm/hr) =	41.64			
over (mm) =	10.00			
Storage Coeff. (mm) =	7.64 (11)			
Unit Hyd. Peak (mm) =	10.00			
Unit Hyd. Peak (cms) =	.13			
PEAK FLOW (cms) =	2.46			
TIME TO PEAK (hrs) =	5.33			
RUNOFF VOLUME (mm) =	89.53			
TOTAL RAINFALL (mm) =	90.53			
RUNOFF COEFFICIENT =	.99			

(1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSSES:
 CN* = 76.0 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0003)	AREA	QPEAK	TPPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(ft ³ /s)	(mm)
ID= 1 (3020)	5.36	5.75	5.25	82.10
+ ID= 2 (3010)	26.95	2.824	5.25	80.57
ID = 3 (0003)	32.31	3.400	5.25	80.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0004)			
IN= 2--> OUT= 1			
DE= 5.0 m/h			
OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
.0000	.0000	.3340	1.5400
.0010	.1520	.3490	1.7350
.0140	.3070	.3630	1.9320
.0370	.4630	.3760	2.1330
.0580	.6270	1.0880	2.3370
.0730	.7930	2.3790	2.5440
.0860	.9790	4.0460	2.7670
.0970	1.1840	6.0170	2.9670
.1020	1.3930	.0000	.0000
.1180	1.6490		
AREA (ha)	QPEAK (cms)	TPPEAK (ft ³ /s)	R.V. (mm)
32.31	3.40	5.25	80.62
	.37	7.50	80.66

INFLOW : ID= 2 (0003) 32.31 3.40 7.50
 OUTFLOW : ID= 1 (0004) 32.31 3.40 7.50
 PEAK FLOW REDUCTION [Out/In] (%) = 10.80
 TIME STEP OF PEAK FLOW (min) = 35.00
 MAXIMUM STORAGE USED (ha.m.) = 1.9950

FINISH

POUND 23 - +25% RAINFALL INTENSITY

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V V I SSSSS U U A L
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
V V I SSSSS UUUU A A LLLLL
OOO TTTT H H Y Y M M OOO
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
OOO T T H H Y Y M M OOO
  
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***** D E T A I L E D O U T P U T *****

Input Filename: C:\Program Files\Visual QTRIMO 2.3.2\voain.dat
 Output Filename: S:\2010 Projects\DE (E10)\WRW10-487 East Gwallowbury SWM Master
 Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas
 Summary Filename: S:\2010 Projects\DE (E10)\WRW10-487 East Gwallowbury SWM Master
 Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas

DATE: 3/30/2011 TIME: 11:55:05 AM
 USER:

COMMENTS:

 *** SIMULATION NUMBER: 1 ***

--- READ STORM ---
 --- Plot=120.71 mm ---
 --- Comments: 100yr/12hr ---
 --- Filenames: S:\2010 Projects\DE (E10)\
 WRW10-487 East Gwallowbury SWM Master Plan\
 Calcs\Hydrologic Models\Existing LGCA Files\

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
1.25	1.00	3.50	20.52	6.75	8.45	10.00	1.21
1.50	1.21	3.75	20.52	7.00	8.45	10.25	1.21
1.75	1.21	4.00	20.52	7.25	8.45	10.50	1.21
2.00	1.21	4.25	20.52	7.50	8.45	10.75	1.21
2.25	1.21	4.50	20.52	7.75	8.45	11.00	1.21
2.50	1.21	4.75	20.52	8.00	8.45	11.25	1.21
2.75	1.21	5.00	20.52	8.25	8.45	11.50	1.21
3.00	1.21	5.25	20.52	8.50	8.45	11.75	1.21
3.25	1.21	5.50	20.52	8.75	8.45	12.00	1.21
3.50	1.21	5.75	20.52	9.00	8.45	12.25	1.21
3.75	1.21	6.00	20.52	9.25	8.45	12.50	1.21
4.00	1.21	6.25	20.52	9.50	8.45	12.75	1.21
4.25	1.21	6.50	20.52	9.75	8.45	13.00	1.21

MODIFYING PARAMETERS
 CNSE= 1
 Multiplication Factor= 1.25
 Time shift (min) = .00

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
1.25	1.00	3.50	25.65	6.75	10.56	10.00	1.51
1.50	1.51	3.75	25.65	7.00	10.56	10.25	1.51
1.75	1.51	4.00	25.65	7.25	10.56	10.50	1.51
2.00	1.51	4.25	25.65	7.50	10.56	10.75	1.51
2.25	1.51	4.50	25.65	7.75	10.56	11.00	1.51
2.50	1.51	4.75	25.65	8.00	10.56	11.25	1.51
2.75	1.51	5.00	25.65	8.25	10.56	11.50	1.51
3.00	1.51	5.25	25.65	8.50	10.56	11.75	1.51
3.25	1.51	5.50	25.65	8.75	10.56	12.00	1.51
3.50	1.51	5.75	25.65	9.00	10.56	12.25	1.51
3.75	1.51	6.00	25.65	9.25	10.56	12.50	1.51
4.00	1.51	6.25	25.65	9.50	10.56	12.75	1.51
4.25	1.51	6.50	25.65	9.75	10.56	13.00	1.51

CM1IB
 STANCHYD (3020) | Area (ha)= 5.36
 ID= 1 DT= 5.0 min | Total Imp(%)= 87.00 D.L. Conn. (%)= 87.00

Surface Area (ha)= 4.66
 Dep. Storage (mm)= 1.00
 Average Slope (%)= 1.00
 Length (m)= 189.00
 Manning's n = .013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYDROGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
1.25	1.00	3.167	9.05	6.250	19.61	9.42	1.51
1.50	1.51	3.333	9.05	6.333	19.61	9.42	1.51
1.75	1.51	3.500	9.05	6.500	19.61	9.42	1.51
2.00	1.51	3.667	9.05	6.667	19.61	9.42	1.51
2.25	1.51	3.833	9.05	6.833	19.61	9.42	1.51
2.50	1.51	4.000	9.05	7.000	19.61	9.42	1.51
2.75	1.51	4.167	9.05	7.167	19.61	9.42	1.51
3.00	1.51	4.333	9.05	7.333	19.61	9.42	1.51
3.25	1.51	4.500	9.05	7.500	19.61	9.42	1.51
3.50	1.51	4.667	9.05	7.667	19.61	9.42	1.51
3.75	1.51	4.833	9.05	7.833	19.61	9.42	1.51
4.00	1.51	5.000	9.05	8.000	19.61	9.42	1.51
4.25	1.51	5.167	9.05	8.167	19.61	9.42	1.51
4.50	1.51	5.333	9.05	8.333	19.61	9.42	1.51
4.75	1.51	5.500	9.05	8.500	19.61	9.42	1.51
5.00	1.51	5.667	9.05	8.667	19.61	9.42	1.51
5.25	1.51	5.833	9.05	8.833	19.61	9.42	1.51
5.50	1.51	6.000	9.05	9.000	19.61	9.42	1.51
5.75	1.51	6.167	9.05	9.167	19.61	9.42	1.51
6.00	1.51	6.333	9.05	9.333	19.61	9.42	1.51
6.25	1.51	6.500	9.05	9.500	19.61	9.42	1.51
6.50	1.51	6.667	9.05	9.667	19.61	9.42	1.51
6.75	1.51	6.833	9.05	9.833	19.61	9.42	1.51
7.00	1.51	7.000	9.05	10.000	19.61	9.42	1.51
7.25	1.51	7.167	9.05	10.167	19.61	9.42	1.51
7.50	1.51	7.333	9.05	10.333	19.61	9.42	1.51
7.75	1.51	7.500	9.05	10.500	19.61	9.42	1.51
8.00	1.51	7.667	9.05	10.667	19.61	9.42	1.51
8.25	1.51	7.833	9.05	10.833	19.61	9.42	1.51
8.50	1.51	8.000	9.05	11.000	19.61	9.42	1.51
8.75	1.51	8.167	9.05	11.167	19.61	9.42	1.51
9.00	1.51	8.333	9.05	11.333	19.61	9.42	1.51
9.25	1.51	8.500	9.05	11.500	19.61	9.42	1.51
9.50	1.51	8.667	9.05	11.667	19.61	9.42	1.51
9.75	1.51	8.833	9.05	11.833	19.61	9.42	1.51
10.00	1.51	9.000	9.05	12.000	19.61	9.42	1.51

2.632	9.05	5.667	19.61	8.750	3.01	11.83	1.51
2.667	9.05	5.754	19.61	8.833	3.01	11.82	1.51
2.750	9.05	5.833	19.61	8.917	3.01	12.08	1.51
2.833	9.05	5.917	19.61	9.000	3.01	12.08	1.51
2.917	9.05	6.000	19.61	9.083	3.01	12.17	1.51
3.000	9.05	6.083	19.61	9.167	3.01	12.25	1.51
3.083	9.05	6.167	19.61	9.250	3.01		

Max Eff. Inten. (mm/Hr) = 69.40
 Storage Coeff. (mln) = 4.33 (11)
 Unit Hyd. Peak (mln) = 5.00
 Unit Hyd. Peak (cms) = .23
 PEAK FLOW (cms) = .90
 TIME TO PEAK (hrs) = 5.17
 RAINFALL (mm) = 149.89
 TOTAL RAINFALL (mm) = 150.89
 RAINFALL COEFFICIENT = .99

***** WARNING: STORAGE CORP. IS SMALLER THAN TIME STEP!
 (1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
 CN* = 62.0 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 FROM THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

***** WARNING: STORAGE CORP. IS SMALLER THAN TIME STEP!
 (1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
 CN* = 62.0 Ia = Dep. Storage (Above)
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 (1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
 CN* = 62.0 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 FROM THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0004)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
ID= 2 -> OUT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 5.0 min				
	.0000	.0000	.3340	1.5400
	.0140	.1520	.3490	1.7350
	.0370	.3070	.3630	1.9320
	.0590	.4650	.3760	2.1310
	.0730	.6270	.3890	2.3370
	.0860	.7930	.4020	2.5440
	.0970	.9750	.4150	2.7540
	.1020	1.1610	.4280	2.9670
	.1180	1.3490	.4410	3.1830

***** WARNING: STORAGE CORP. IS SMALLER THAN TIME STEP!
 (1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
 CN* = 62.0 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 FROM THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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 (1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
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 (1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
 CN* = 62.0 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 FROM THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

POUND 23 - CN AMCI

V V I SSSS V U A L
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
V V I SSSS UUUU A A LLLL
OOO TTTT H H Y Y X X OOO
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O

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***** D E T A I L E D O U T P U T *****

Input filename: C:\Program Files\Visual OTTHMO 2.3.2\voain.dat
Output filename: S:\2010 Projects\DE-EE (E10)\WRW10-487 East Gwillimbury SWM Master
Plan\CalecsHydrologic Models\Sensitivity Analysis\Bas
Summary filename: S:\2010 Projects\DE-EE (E10)\WRW10-487 East Gwillimbury SWM Master
Plan\CalecsHydrologic Models\Sensitivity Analysis\Bas

DATE: 3/21/2011

USER:

TIME: 8:00:23 PM

COMMENTS:

** SIMULATION NUMBER: 1 **

READ STORM
Filename: S:\2010 Projects\DE-EE (E10)\
WRW10-487 East Gwillimbury SWM Master Plan\
CalecsHydrologic Models\Existing LSRCN Files\
Protal=120.71 mm
Comments: 100yr/12hr

Table with 12 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show time intervals and corresponding rainfall amounts in mm/hr.

CALIB
STANDARD (3020)
Area (ha)= 5.36
Total Imp(%)= 87.00 Dir. Conn.(%)= 87.00
ID= 1 DF= 5.0 min.
Surface Area (ha)= 4.66 IMPERVIOUS PERVIOUS (L)
Dep. Storage (mm)= 1.00 1.50
Average Slope (%)= 1.00 2.00
Length (m)= 189.00 40.00
Mannings n = .013 .250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETROGRAPH
Table with 12 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show time intervals and corresponding rainfall amounts in mm/hr.

Max.Eff.Inten.(mm/hr)= 55.52 60.42
over (min) 5.00 10.00
Storage Coeff. (min)= 4.74 (li) 8.91 (li)
Unit Hyd. Peak (cms)= 5.00 10.00
Unit Hyd. peak (cms)= .22
PEAK FLOW (cms)= .72
TIME TO PEAK (hrs)= 5.25
RUNOFF VOLUME (mm)= 119.71 30.24
TOTAL RAINFALL (mm)= 120.71 120.71
RUNOFF COEFFICIENT = .99 .75
**TOTALS*
.753 (lii)
5.25
108.08
120.71
.90

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
 CN = 42.0 IA = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 TO THE BASEFLOW COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

PEAK FLOW REDUCTION [Out/In] (%) = 25.24
 TIME STEP OF PEAK FLOW (min) = 65.00
 MAXIMUM STORAGE (mm) = 2,3421

FINISH

CALLS
 STANDARD (3010) Area (ha) = 26.95 Dir. Conn. (%) = 79.00
 ID= 1 DT= 5.0 min Total Imp(%) = 79.00

Surface Area (ha) = 21.29
 Dep. Storage (mm) = 1.00
 Average Slope (%) = 1.50
 Length (m) = 423.90
 Manning's n = .013

Max. Eff. Inten. (mm/hr) = 55.52
 Storage Coeff. (mm) = 2.00
 DLE Hgt. Peak (mm) = 5.91 (11)
 DLE Hgt. Peak (mm) = 20.00
 DLE Hgt. Peak (mm) = 1.18

PEAK FLOW (cms) = 3.28
 TIME TO PEAK (hrs) = 5.25
 RUNOFF VOLUME (mm) = 119.71
 TOTAL RAINFALL (mm) = 120.71
 RUNOFF COEFFICIENT = .99

(1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
 CN = 58.0 IA = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 TO THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0003)
 1 + 2 = 3
 AREA ABEK TPEAK R.V.
 (ha) (cms) (hrs) (mm)

ID1= 1 (3020): 5.36 .753 5.25 108.08
 + ID2= 2 (3010): 26.95 3.644 5.25 104.41
 ID = 3 (0003): 32.31 4.397 5.25 105.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0004)
 IN= 2 --> OUT= 1
 DT= 5.0 min

OUTFLOW (mm)	STORAGE (mm)	OUTFLOW (mm)	STORAGE (mm)
.0000	.0000	.0000	.0000
.0146	.1520	.3140	1.1850
.0378	.3070	.3630	1.3320
.0588	.4650	.3760	2.1330
.0730	.6270	1.0880	2.3370
.0860	.7930	2.3790	2.5440
.0970	.9750	4.0460	2.7540
.3020	1.1610	6.0170	2.9670
.3180	1.3490	.0000	.0000

AREA ABEK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW: ID= 1 (0003) 32.31 4.40 5.25 105.02
 OUTFLOW: ID= 1 (0004) 32.31 1.11 6.33 104.85

POND 23 - CN AMC III

V V I SSSSS U U A I I
 V V I SS U U A A I I
 V V I SS U U A A I I
 V V I SSSSS UUUU A A ILLLL
 OOO ITTTT H H Y Y M M OOO
 O O T T H H Y Y M M O O
 O O T T H H Y Y M M O O
 OOO T T H H Y Y M M OOO

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***** D E T A I L E D O U T P U T *****

Input filename: C:\Program Files\Visual OTTHYMO 2.3.2\voind.dat
 Output filename: s:\2010 Projects\Ad-EE (E10)\WRW10-487 East Gwillimbury SWM Master
 Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas
 Summary filename: s:\2010 Projects\Ad-EE (E10)\WRW10-487 East Gwillimbury SWM Master
 Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas

DATE: 3/21/2011 TIME: 7:43:13 PM

USER:

COMMENTS:

 ** SIMULATION NUMBER: 1 **

Filename: S:\2010 Projects\Ad-EE (E10)\
 WRW10-487 East Gwillimbury SWM Master Plan\
 Calcs\Hydrologic Models\Existing LSRON Files\
 Pectal=120.71 mm Comments: 100yr/12hr

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
1.25	0.00	3.50	20.52	6.75	8.45	10.00	1.21
1.50	1.21	3.75	20.52	7.00	8.45	10.25	1.21
1.75	1.21	4.00	20.52	7.25	8.45	10.50	1.21
2.00	1.21	4.25	20.52	7.50	4.83	10.75	1.21
2.25	1.21	4.50	35.52	7.75	4.83	11.00	1.21
2.50	1.21	4.75	55.52	8.00	4.83	11.25	1.21
2.75	1.21	5.00	55.52	8.25	4.83	11.50	1.21
3.00	1.21	5.25	55.52	8.50	2.41	11.75	1.21
3.25	1.21	5.50	15.69	8.75	2.41	12.00	1.21
3.50	1.21	5.75	15.69	9.00	2.41	12.25	1.21
3.75	1.21	6.00	15.69	9.25	2.41	12.50	1.21
4.00	1.21	6.25	15.69	9.50	2.41	12.75	1.21
4.25	1.21	6.50	8.45	9.75	2.41	13.00	1.21

CSLR (3020) Area (ha)= 5.36
 SWABAYD Total Imp(%)= 87.00 Dlr. Conn. (\$) = 87.00
 Imp. 1 Dps. 5.0 min

Surface Area (ha)= 4.66 IMPERVIOUS PERVIOUS (I)
 Dep. Storage (mm)= 1.00
 Average Slope (%)= 1.00
 Length (m)= 189.00
 Manlings n = .013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYDROGRAPH											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
1.25	0.00	3.167	7.24	6.250	15.69	9.33	1.21	1.21	1.21	1.21	1.21
1.50	1.21	3.250	7.24	6.333	8.45	9.42	1.21	1.21	1.21	1.21	1.21
1.75	1.21	3.333	20.52	6.417	8.45	9.50	1.21	1.21	1.21	1.21	1.21
2.00	1.21	3.417	20.52	6.500	8.45	9.58	1.21	1.21	1.21	1.21	1.21
2.25	1.21	3.500	20.52	6.583	8.45	9.67	1.21	1.21	1.21	1.21	1.21
2.50	1.21	3.583	20.52	6.667	8.45	9.75	1.21	1.21	1.21	1.21	1.21
2.75	1.21	3.667	20.52	6.750	8.45	9.83	1.21	1.21	1.21	1.21	1.21
3.00	1.21	3.750	20.52	6.833	8.45	9.92	1.21	1.21	1.21	1.21	1.21
3.25	1.21	3.833	20.52	6.917	8.45	10.00	1.21	1.21	1.21	1.21	1.21
3.50	1.21	3.917	20.52	7.000	8.45	10.08	1.21	1.21	1.21	1.21	1.21
3.75	1.21	4.000	20.52	7.083	8.45	10.17	1.21	1.21	1.21	1.21	1.21
4.00	1.21	4.083	20.52	7.167	8.45	10.25	1.21	1.21	1.21	1.21	1.21
4.25	1.21	4.167	20.52	7.250	8.45	10.33	1.21	1.21	1.21	1.21	1.21
4.50	1.21	4.250	20.52	7.333	4.83	10.42	1.21	1.21	1.21	1.21	1.21
4.75	1.21	4.333	20.52	7.417	4.83	10.50	1.21	1.21	1.21	1.21	1.21
5.00	1.21	4.417	20.52	7.500	4.83	10.59	1.21	1.21	1.21	1.21	1.21
5.25	1.21	4.500	20.52	7.583	4.83	10.67	1.21	1.21	1.21	1.21	1.21
5.50	1.21	4.583	20.52	7.667	4.83	10.75	1.21	1.21	1.21	1.21	1.21
5.75	1.21	4.667	20.52	7.750	4.83	10.83	1.21	1.21	1.21	1.21	1.21
6.00	1.21	4.750	20.52	7.833	4.83	10.92	1.21	1.21	1.21	1.21	1.21
6.25	1.21	4.833	20.52	7.917	4.83	11.00	1.21	1.21	1.21	1.21	1.21
6.50	1.21	4.917	20.52	8.000	4.83	11.08	1.21	1.21	1.21	1.21	1.21
6.75	1.21	5.000	20.52	8.083	4.83	11.17	1.21	1.21	1.21	1.21	1.21
7.00	1.21	5.083	20.52	8.167	4.83	11.25	1.21	1.21	1.21	1.21	1.21
7.25	1.21	5.167	20.52	8.250	4.83	11.33	1.21	1.21	1.21	1.21	1.21
7.50	1.21	5.250	20.52	8.333	2.41	11.42	1.21	1.21	1.21	1.21	1.21
7.75	1.21	5.333	15.69	8.417	2.41	11.50	1.21	1.21	1.21	1.21	1.21
8.00	1.21	5.417	15.69	8.500	2.41	11.58	1.21	1.21	1.21	1.21	1.21
8.25	1.21	5.500	15.69	8.583	2.41	11.67	1.21	1.21	1.21	1.21	1.21
8.50	1.21	5.583	15.69	8.667	2.41	11.75	1.21	1.21	1.21	1.21	1.21
8.75	1.21	5.667	15.69	8.750	2.41	11.83	1.21	1.21	1.21	1.21	1.21
9.00	1.21	5.750	15.69	8.833	2.41	11.92	1.21	1.21	1.21	1.21	1.21
9.25	1.21	5.833	15.69	8.917	2.41	12.00	1.21	1.21	1.21	1.21	1.21
9.50	1.21	5.917	15.69	9.000	2.41	12.08	1.21	1.21	1.21	1.21	1.21
9.75	1.21	6.000	15.69	9.083	2.41	12.17	1.21	1.21	1.21	1.21	1.21
10.00	1.21	6.083	15.69	9.167	2.41	12.25	1.21	1.21	1.21	1.21	1.21
10.25	1.21	6.167	15.69	9.250	2.41	12.33	1.21	1.21	1.21	1.21	1.21

Max. Eff. Inten. (mm/hr)= 55.52 43.81
 over 5.00 10.00
 Storage Coeff. (min)= 4.74 (il) 6.91 (il)
 Unit Hyd. Tpeak (min)= 5.00 10.00
 Unit Hyd. Peak (cms)= .22 .12
 *TOTALS+
 PEAK FLOW (cms)= .72 5.06
 TIME TO PEAK (hrs)= 5.25 5.25
 RUNOFF VOLUME (mm)= 119.71 76.10
 TOTAL RAINFALL (mm)= 120.71 120.71
 RUNOFF COEFFICIENT = .99 .63
 ***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
 (11) TIME STEP (DT) = Dep. Storage (Above)
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

PEAK FLOW REDUCTION [Cent./Q/h] (%) = 30.40
 PEAK FLOW REDUCTION [Cent./Q/h] (%) = 40.5975
 MAXIMUM STORAGE (M³) = 2.5975

FINISH

 | CALLS | | Area (ha) = 26.95 | Dir. Com. (%) = 79.00
 | STANHYD (3010) | | Total Imp (%) = 79.00 |
 | ID= 1 DT= 5.0 min | |

 Surface Area (ha) = 21.29
 Dep. Storage (mm) = 1.00
 Average Slope (S) = 1.50
 Length (m) = 423.90
 Manning's n = .013
 Max Eff. Inten. (mm/hr) = 55.52
 Storage Coeff. (M1) = 2.00
 Ditch Spd. Peak (mm) = 6.81 (11)
 Ditch Spd. Peak (mm) = 5.00
 Ditch Spd. Peak (mm) = 20.00
 Ditch Spd. Peak (mm) = 18
 TOTALS
 PEAK FLOW (cms) = 3.28
 TIME TO PEAK (hrs) = 5.25
 RUNOFF VOLUME (mm) = 119.71
 TOTAL RAINFALL (mm) = 120.71
 RUNOFF COEFFICIENT = .99

(1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
 CN* = 89.0 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | R2D HYD. (0003) | | AREA OPEN TPRK R.V.
 | 1 + 2 = 3 | | (ha) (cms) (hrs) (mm)
 ID1 = 1 (3020) : 5.36 .801 5.25 114.04
 + ID2 = 2 (3010) : 26.95 4.017 5.25 113.97
 ID = 3 (0003) : 32.31 4.818 5.25 113.98

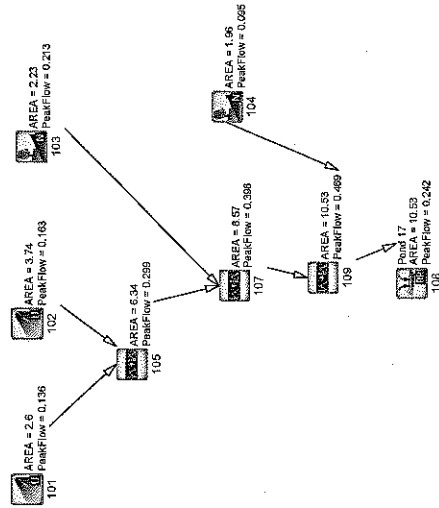
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0004)	
IN= 2--> OUF= 1	
DT= 5.0 min	

 OUFLOW (cms) | STORAGE (M³) | OUFLOW (cms) | STORAGE (M³)
 (mm) | (M³) | (mm) | (M³)
 .0140 | 1320 | .0140 | 1320
 .0370 | 4550 | .0370 | 4550
 .0580 | 4650 | .0580 | 4650
 .0730 | 6270 | .0730 | 6270
 .0860 | 7330 | .0860 | 7330
 .0970 | 9750 | .0970 | 9750
 .3020 | 1.1610 | .3020 | 1.1610
 .3180 | 1.3490 | .3180 | 1.3490

 AREA OPEN TPRK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (0003) 32.31 4.82 5.25 113.98
 OUFLOW : ID= 1 (0004) 32.31 1.46 5.92 113.81

POND 17 - BASE FILE



```

V V I SSSSS U U A A L
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
W I SSSSS UUUUU A A LLLL
OOO TTTT H H Y Y M M OOO
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
  
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***** D E T A I L E D O U T P U T *****

Input filename: C:\Program Files\Visual CTRWMO 2.3.2\win.dat
 Output filename: S:\2010 Projects\AD-EE (E10)\WRW10-487 East Gwillimbury SWM Master
 Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas
 Summary filename: S:\2010 Projects\AD-EE (E10)\WRW10-487 East Gwillimbury SWM Master
 Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas

DATE: 3/30/2011

TIME: 11:16:26 AM

USER:

COMMENTS:

 ** SIMULATION NUMBER: 1 **

File name: S:\2010 Projects\AD-EE (E10)\
 WRW10-487 East Gwillimbury SWM Master Plan\
 Calcs\Hydrologic Models\Existing\BSCN Files\
 Comments: 100-Year 12-Hour SCS II Design Storm

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	2.21	3.25	3.54	6.25	15.93	9.25	3.10
0.50	2.21	3.50	3.54	6.50	15.93	9.50	3.10
0.75	2.21	3.75	3.54	6.75	7.08	9.75	3.10
1.00	2.21	4.00	3.54	7.00	7.08	10.00	3.10
1.25	2.21	4.25	5.31	7.25	5.31	10.25	1.77
1.50	2.21	4.50	5.31	7.50	5.31	10.50	1.77
1.75	2.21	4.75	7.08	7.75	5.31	10.75	1.77
2.00	2.21	5.00	7.08	8.00	5.31	11.00	1.77
2.25	2.65	5.25	10.62	8.25	3.10	11.25	1.77
2.50	2.65	5.50	10.62	8.50	3.10	11.50	1.77
2.75	2.65	5.75	42.48	8.75	3.10	11.75	1.77
3.00	2.65	6.00	116.82	9.00	3.10	12.00	1.77

READ STORM | CAV1B | Area (ha)= 1.96 Curve Number (CN)= 60.0
 | NASHED (0104) |

ID= 1 DT=12.0 min | Ia (mm) = 2.50 # of Linear Res. (N) = 4.00
 U.H. Tp(hrs) = .153

NOTE: RAINFALL WAS TRANSFORMED TO 12.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAM

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
2.20	2.21	3.20	3.54	6.20	15.93	9.20	3.10
.400	2.21	3.400	3.54	6.400	15.93	9.40	3.10
.600	2.21	3.600	3.54	6.600	15.93	9.60	3.10
.800	2.21	3.800	3.54	6.800	15.93	9.80	3.10
1.000	2.21	4.000	3.54	7.000	15.93	10.00	3.10
1.200	2.21	4.200	3.54	7.200	15.93	10.20	3.10
1.400	2.21	4.400	3.54	7.400	15.93	10.40	3.10
1.600	2.21	4.600	3.54	7.600	15.93	10.60	3.10
1.800	2.21	4.800	3.54	7.800	15.93	10.80	3.10
2.000	2.21	5.000	3.54	8.000	15.93	11.00	3.10
2.200	2.45	5.400	10.62	8.400	3.10	11.40	1.77
2.400	2.45	5.800	26.55	8.600	3.10	11.60	1.77
2.600	2.45	6.200	61.06	8.800	3.10	11.80	1.77
2.800	2.45	6.600	116.82	9.000	3.10	12.00	1.77

Unit Hyd. Peak (cms) = .169
 PEAK FLOW (cms) = .095 (1)
 TIME TO PEAK (hrs) = 6.400
 RINOFF VOLUME (mm) = 29.008
 TOTAL RAINFALL (mm) = 88.500
 RINOFF COEFFICIENT = .328
 (1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALLS (0102) | Area (ha) = 2.23 Curve Number (CN) = 60.0
 ID= 1 DT=10.0 min | Ia (mm) = 2.50 # of Linear Res. (N) = 4.00
 U.H. Tp(hrs) = .20
 NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAM

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
1.67	2.21	3.167	3.54	6.167	15.93	9.17	3.10
.333	2.21	3.333	3.54	6.333	15.93	9.33	3.10
.500	2.21	3.500	3.54	6.500	15.93	9.50	3.10
.667	2.21	3.667	3.54	6.667	15.93	9.67	3.10
.833	2.21	3.833	3.54	6.833	15.93	9.83	3.10
1.000	2.21	4.000	3.54	7.000	15.93	10.00	3.10
1.167	2.21	4.167	3.54	7.167	15.93	10.17	3.10
1.333	2.21	4.333	3.54	7.333	15.93	10.33	3.10
1.500	2.21	4.500	3.54	7.500	15.93	10.50	3.10
1.667	2.21	4.667	3.54	7.667	15.93	10.67	3.10
1.833	2.21	4.833	3.54	7.833	15.93	10.83	3.10
2.000	2.21	5.000	3.54	8.000	15.93	11.00	3.10
2.167	2.45	5.167	10.62	8.167	3.10	11.17	1.77
2.333	2.45	5.333	10.62	8.333	3.10	11.33	1.77
2.500	2.45	5.500	10.62	8.500	3.10	11.50	1.77
2.667	2.45	5.667	10.62	8.667	3.10	11.67	1.77
2.833	2.45	5.833	10.62	8.833	3.10	11.83	1.77
3.000	2.45	6.000	10.62	9.000	3.10	12.00	1.77

Unit Hyd. Peak (cms) = .529
 PEAK FLOW (cms) = .213 (1)
 TIME TO PEAK (hrs) = 6.000

RINOFF VOLUME (mm) = 29.111
 TOTAL RAINFALL (mm) = 88.500
 RINOFF COEFFICIENT = .328
 (1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

IMPERVIOUS

Area (ha)	Dlr. Conn. (%)	PERVIOUS (1)
3.74	1.00	1.00
1.04	3.70	3.70
1.00	2.50	2.50
157.00	1.00	157.00
.015	.200	.200
116.82	26.82	26.82
10.00	40.00	40.00
3.43 (11)	32.65 (11)	32.65 (11)
10.00	40.00	40.00
.16	.03	.03

PEAK FLOW (cms) = .01
 TIME TO PEAK (hrs) = 6.00
 RINOFF VOLUME (mm) = 87.50
 TOTAL RAINFALL (mm) = 88.50
 RINOFF COEFFICIENT = .99
 ***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) ON PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN = 60.0
 (11) TIME STEP (TOP) SHOULD BE SMALLER OR EQUAL
 (11) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALLS (0101) | Area (ha) = 2.60 Dlr. Conn. (%) = 12.00
 ID= 1 DT=10.0 min | Total Imp (%) = 25.00

IMPERVIOUS	PERVIOUS (1)
1.65	1.95
1.00	2.50
.50	1.50
131.00	131.00
.150	.200
98.23	46.65
20.00	40.00
16.16 (11)	34.76 (11)
20.00	40.00
.06	.03

Max Eff. Inflow (mm/hr) = 98.23
 Storage Coeff. (mbd) = 20.00
 Unit Hyd. Peak (cms) = 20.00
 Unit Hyd. Peak (cms) = .06

PEAK FLOW (cms) = .06
 TIME TO PEAK (hrs) = 6.17
 RINOFF VOLUME (mm) = 87.50
 TOTAL RAINFALL (mm) = 88.50
 RINOFF COEFFICIENT = .99

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) ON PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN = 60.0 Ia = Dep. Storage (Above)
 TOTALS
 .136 (11)
 6.33
 38.95
 88.50
 .44

POND 17 - 6HR STORM

V V I SSSS U U A L
V V I SS U U A A L
V V I SS U U A A L
V V I SS U U A A L
V V I SS U U A A L
V V I SSSS UUUU A A LLLL
OOO TTTT H H Y Y M M
OOO T T H H Y Y M M
OOO T T H H Y Y M M
OOO T T H H Y Y M M

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***** D E T A I L E D O U T P U T *****

Input filename: C:\Program Files\Visual OTRMNO 2.3.2\vojn.dat
Output filename: S:\2010 Projects\DE-EE (E10)\WR\W10-487 East Gwillimbury SWM Master Plan\Calca\Hydrologic Models\Sensitivity Analysis\Bas
Summary filename: S:\2010 Projects\DE-EE (E10)\WR\W10-487 East Gwillimbury SWM Master Plan\Calca\Hydrologic Models\Sensitivity Analysis\Bas

DATE: 3/30/2011 TIME: 11:28:14 AM

USER:

COMMENTS:

***** SIMULATION NUMBER: 1 **

FILENAME: S:\2010 Projects\DE-EE (E10)\WR\W10-487 East Gwillimbury SWM Master Plan\Calca\Hydrologic Models\Sensitivity Analysis\
Comments: 100-Year 6-Hour SCS II Design Storm

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows showing time intervals and rainfall amounts in mm/hr and hrs.

CALIB (01.04) Area (ha) = 1.96 Curve Number (CN) = 60.0
NASHVD (01.04) La (mm) = 2.50 # of Linear Res. (N) = 4.00
ID= 1 DT=12.0 min U.R. Tp(hrs) = .55

(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with columns: ADD HYD, ID, AREA, QPEAK, TPEAK, R.V. (ha, cms, hrs, mm). Rows for ID1, ID2, ID3.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: ADD HYD, ID, AREA, QPEAK, TPEAK, R.V. (ha, cms, hrs, mm). Rows for ID1, ID2, ID3.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: ADD HYD, ID, AREA, QPEAK, TPEAK, R.V. (ha, cms, hrs, mm). Rows for ID1, ID2, ID3.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with columns: RESERVOIR, IN, DT, OUTFLOW, STORAGE, OUTFLOW, AREA, QPEAK, TPEAK, R.V., INFLOW, OUTFLOW, PEAK FLOW REDUCTION, TIME SHIFT OF PEAK FLOW, MAXIMUM STORAGE USED.

FINISH

NOTE: RAINFALL WAS TRANSFORMED TO 12.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
200	2.21	1.800	3.54	3.400	11.15	5.00	3.10
400	2.21	2.000	3.54	3.600	11.50	5.20	3.10
600	2.21	2.200	5.31	3.800	6.64	5.40	2.10
800	2.21	2.400	6.64	4.000	5.31	5.60	1.77
1,000	2.21	2.600	8.85	4.200	5.31	5.80	1.77
1,200	2.65	2.800	18.59	4.400	3.65	6.00	1.77
1,400	2.65	3.000	42.48	4.600	3.10		
1,600	3.10	3.200	116.82	4.800	3.10		

Unit Hyd Opeak (cms) = .169

PEAK FLOW (cms) = .072 (1)

TIME TO PEAK (hrs) = 3.600

RUNOFF VOLUME (mm) = 16.279

TOTAL RAINFALL (mm) = 63.720

RUNOFF COEFFICIENT = .235

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

GALEB	AREA	HA	CURVE NUMBER	CN
1	2	3	4	5
GALEB	2.23	2.23	60.0	60.0
WASHED	2.50	2.50	60.0	60.0
DR=10.0 min	2.20	2.20	60.0	60.0
U.R. Tp(hrs)	2.20	2.20	60.0	60.0

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
1.67	2.21	1.867	3.54	3.437	10.92	4.87	3.10
3.67	2.21	1.867	3.54	3.437	10.92	4.87	3.10
5.67	2.21	2.000	3.54	3.600	15.93	5.00	3.10
7.67	2.21	2.167	5.31	3.667	7.08	5.17	3.10
9.67	2.21	2.333	6.19	3.833	6.19	5.33	2.43
11.67	2.21	2.500	7.08	4.000	5.31	5.50	1.77
13.67	2.65	2.667	10.62	4.167	5.31	5.67	1.77
15.67	1.333	2.833	26.55	4.333	4.20	5.83	1.77
17.67	1.500	3.000	42.48	4.500	3.10	6.00	1.77

Unit Hyd Opeak (cms) = .529

PEAK FLOW (cms) = .154 (1)

TIME TO PEAK (hrs) = 3.333

RUNOFF VOLUME (mm) = 16.337

TOTAL RAINFALL (mm) = 63.720

RUNOFF COEFFICIENT = .236

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

GALEB	AREA	HA	D.P. CORR. (%)
1	2	3	4
GALEB	3.74	3.74	1.00
STANDARD	1.00	1.00	1.00
DR=10.0 min	1.00	1.00	1.00
Total Imp (%)	1.00	1.00	1.00

Surface Area (ha) = .04

Dep. Storage (mm) = 1.00

Average Slope (%) = 1.00

Length (m) = 157.00

Manning's n = .015

IMPERVIOUS RATIOS (1)

PERVIOUS RATIOS (1)

PERVIOUS RATIOS (1)

PERVIOUS RATIOS (1)

PERVIOUS RATIOS (1)

PERVIOUS RATIOS (1)

PERVIOUS RATIOS (1)

PERVIOUS RATIOS (1)

PERVIOUS RATIOS (1)

PERVIOUS RATIOS (1)

PERVIOUS RATIOS (1)

PERVIOUS RATIOS (1)

Max Eff. Inten. (mm/hr) = 116.82

Storage Coeff. (mm) = 10.00

Unit Hyd. Opeak (mm/hr) = 3.43 (11)

Time Hyd. Peak (hrs) = 10.00

Unit Hyd. Peak (mm) = .16

PEAK FLOW (cms) = .01

TIME TO PEAK (hrs) = 3.17

RUNOFF VOLUME (mm) = 62.72

TOTAL RAINFALL (mm) = 63.72

RUNOFF COEFFICIENT = .98

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP; YOU SHOULD CONSIDER SPLITTING THE AREA.

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP; YOU SHOULD CONSIDER SPLITTING THE AREA.

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**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP; YOU SHOULD CONSIDER SPLITTING THE AREA.

GALEB	AREA	HA	D.P. CORR. (%)
1	2	3	4
GALEB	2.60	2.60	12.00
STANDARD	25.00	25.00	12.00
DR=10.0 min	25.00	25.00	12.00
Total Imp (%)	25.00	25.00	12.00

Surface Area (ha) = .63

Dep. Storage (mm) = 1.00

Average Slope (%) = .50

Length (m) = 131.00

Manning's n = .150

Max Eff. Inten. (mm/hr) = 91.60

Storage Coeff. (mm) = 20.00

Unit Hyd. Opeak (mm/hr) = 16.62 (11)

Time Hyd. Peak (hrs) = 20.00

Unit Hyd. Peak (mm) = .98

PEAK FLOW (cms) = .06

TIME TO PEAK (hrs) = 3.33

RUNOFF VOLUME (mm) = 62.72

TOTAL RAINFALL (mm) = 63.72

RUNOFF COEFFICIENT = .98

**** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

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**** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) ON PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 60.0 Ia = Dep. Storage (Above)

(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	AREA	OPREK	TPREK	R.V.
1 + 2 = 3	4	5	6	7
ID1 = 1 (0102):	3.74	.109	3.83	16.71
+ ID2 = 2 (0101):	2.60	.108	3.50	23.73
ID = 3 (0105):	6.34	.216	3.67	19.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

POUND 17 - 24HR STORM

```

+-----+
| ADD HYD (0107) |
| 1 + 2 = 3 |
+-----+
| ID1= 1 (0103): |
| + ID2= 2 (0105): |
| ID = 3 (0107): |
+-----+

```

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
2.23	.154	3.33	16.34
6.34	.216	3.67	19.59
8.57	.313	3.33	18.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

+-----+
| ADD HYD (0109) |
| 1 + 2 = 3 |
+-----+
| ID1= 1 (0104): |
| + ID2= 2 (0107): |
| ID = 3 (0109): |
+-----+

```

AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
10.53	.359	3.33	18.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

+-----+
| RESERVOIR (0108) |
| IN= 2--> OUT= 1 |
| DTP= 5.0 min |
+-----+

```

OUTFLOW (cms)	STORAGE (ha.m.)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
.0000	.0000	.0170	.0925	
.0070	.0015	.4100	.1756	
10.53	10.53	4.73	18.27	
10.53	10.53	4.50	18.27	

INFLOW : ID= 2 (0109)
 OUTFLOW : ID= 1 (0108)

PEAK FLOW REDUCTION [(Qout/Qin) (8)] = 35.85
 TIME SHIFT OF PEAK FLOW (min) = 70.00
 MAXIMUM STORAGE USED (ha.m.) = .1161

FINISH

```

V V I SSSSS U V A L
V V I SS U V A A I
V V I SS O U A A A A I
V V I SS U V A A I
VV I SSSSS OUUUU A A ILLL
OOO TTTTT TTTTT H H Y Y M M OOO
O O T T H H Y Y MM MM O O
O O T T H H Y Y M M O O
OOO T T H H Y Y M M OOO

```

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***** D E T A I L E D O U T P U T *****

Input filename: C:\Program Files\Visual OTTHYRO 2.3.2\voain.dat
 Output filename: S:\2010 Projects\UP-ES (E10)\WRW10-487 East Gwillimbury SRM Master Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas
 Summary filename: S:\2010 Projects\UP-ES (E10)\WRW10-487 East Gwillimbury SRM Master Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas

DATE: 3/30/2011 TIME: 11:42:25 AM

USER:

COMMENTS:

 SIMULATION NUMBER: 1

File name: S:\2010 Projects\UP-ES (E10)\
 Calcs\Hydrologic Models\Sensitivity Analysis\
 Comments: 100-Year 24-Hour SCS II Design Storm

TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)
1.25	2.21	6.25	3.54	12.25	15.93
.50	2.21	6.50	3.54	12.50	15.93
1.75	2.21	6.75	3.54	12.75	15.93
1.25	2.21	7.00	3.54	13.00	15.93
1.50	2.21	7.50	3.54	13.50	7.08
1.75	2.21	8.00	3.54	14.00	7.08
2.00	2.21	8.25	3.54	14.25	5.31
2.25	2.21	8.50	3.54	14.50	5.31
2.50	2.21	8.75	3.54	14.75	5.31
2.75	2.21	9.00	3.54	15.00	5.31
3.00	2.21	9.25	3.54	15.25	5.31
3.25	2.21	9.50	3.54	15.50	5.31
3.50	2.21	9.75	3.54	15.75	5.31

TIME	RAINF	TIME	RAINF	TIME	RAINF	TIME	RAINF
4.00	2.21	10.00	7.08	16.00	5.31	22.00	1.77
4.25	2.65	10.25	10.62	16.25	3.10	22.25	1.77
4.50	2.65	10.50	10.62	16.50	3.10	22.50	1.77
4.75	2.65	10.75	10.62	16.75	3.10	22.75	1.77
5.00	2.65	11.00	10.62	17.00	3.10	23.00	1.77
5.25	2.65	11.25	42.48	17.25	3.10	23.25	1.77
5.50	2.65	11.50	42.48	17.50	3.10	23.50	1.77
5.75	2.65	11.75	116.82	17.75	3.10	23.75	1.77
6.00	2.65	12.00	116.82	18.00	3.10	24.00	1.77

NOTE: RAINFALL WAS TRANSFORMED TO 12.0 MIN. TIME STEP.

TIME	RAINF	TIME	RAINF	TIME	RAINF	TIME	RAINF
1.000	2.21	7.000	3.54	13.000	15.93	19.000	3.10
1.200	2.21	7.200	3.54	13.200	15.93	19.200	3.10
1.400	2.21	7.400	3.54	13.400	15.93	19.400	3.10
1.600	2.21	7.600	3.54	13.600	15.93	19.600	3.10
1.800	2.21	7.800	3.54	13.800	15.93	19.800	3.10
2.000	2.21	8.000	3.54	14.000	15.93	20.000	3.10
2.200	2.21	8.200	3.54	14.200	15.93	20.200	3.10
2.400	2.21	8.400	3.54	14.400	15.93	20.400	3.10
2.600	2.21	8.600	3.54	14.600	15.93	20.600	3.10
2.800	2.21	8.800	3.54	14.800	15.93	20.800	3.10
3.000	2.21	9.000	3.54	15.000	15.93	21.000	3.10
3.200	2.21	9.200	7.08	15.200	5.31	21.200	1.77
3.400	2.21	9.400	7.08	15.400	5.31	21.400	1.77
3.600	2.21	9.600	7.08	15.600	5.31	21.600	1.77
3.800	2.21	9.800	7.08	15.800	5.31	21.800	1.77
4.000	2.21	10.000	7.08	16.000	5.31	22.000	1.77
4.200	2.65	10.200	10.62	16.200	3.10	22.200	1.77
4.400	2.65	10.400	10.62	16.400	3.10	22.400	1.77
4.600	2.65	10.600	10.62	16.600	3.10	22.600	1.77
4.800	2.65	10.800	10.62	16.800	3.10	22.800	1.77
5.000	2.65	11.000	10.62	17.000	3.10	23.000	1.77
5.200	2.65	11.200	42.48	17.200	3.10	23.200	1.77
5.400	2.65	11.400	42.48	17.400	3.10	23.400	1.77
5.600	2.65	11.600	116.82	17.600	3.10	23.600	1.77
5.800	2.65	11.800	116.82	17.800	3.10	23.800	1.77
6.000	2.65	12.000	116.82	18.000	3.10	24.000	1.77

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Unit Hyd. Gpeak (cms) = 1.69
 PEAK FLOW (cms) = .237 (1)
 TIME TO PEAK (hrs) = 12.000
 RONOPE VOLUME (mm) = 88.690
 TOTAL RAINFALL (mm) = 177.000
 RONOPE COEFFICIENT = .501

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME	RAINF	TIME	RAINF	TIME	RAINF	TIME	RAINF
1.000	2.21	7.000	3.54	13.000	15.93	19.000	3.10
1.200	2.21	7.200	3.54	13.200	15.93	19.200	3.10
1.400	2.21	7.400	3.54	13.400	15.93	19.400	3.10
1.600	2.21	7.600	3.54	13.600	15.93	19.600	3.10
1.800	2.21	7.800	3.54	13.800	15.93	19.800	3.10
2.000	2.21	8.000	3.54	14.000	15.93	20.000	3.10
2.200	2.21	8.200	3.54	14.200	15.93	20.200	3.10
2.400	2.21	8.400	3.54	14.400	15.93	20.400	3.10
2.600	2.21	8.600	3.54	14.600	15.93	20.600	3.10
2.800	2.21	8.800	3.54	14.800	15.93	20.800	3.10
3.000	2.21	9.000	3.54	15.000	15.93	21.000	3.10
3.200	2.21	9.200	7.08	15.200	5.31	21.200	1.77
3.400	2.21	9.400	7.08	15.400	5.31	21.400	1.77
3.600	2.21	9.600	7.08	15.600	5.31	21.600	1.77
3.800	2.21	9.800	7.08	15.800	5.31	21.800	1.77
4.000	2.21	10.000	7.08	16.000	5.31	22.000	1.77
4.200	2.65	10.200	10.62	16.200	3.10	22.200	1.77
4.400	2.65	10.400	10.62	16.400	3.10	22.400	1.77
4.600	2.65	10.600	10.62	16.600	3.10	22.600	1.77
4.800	2.65	10.800	10.62	16.800	3.10	22.800	1.77
5.000	2.65	11.000	10.62	17.000	3.10	23.000	1.77
5.200	2.65	11.200	42.48	17.200	3.10	23.200	1.77
5.400	2.65	11.400	42.48	17.400	3.10	23.400	1.77
5.600	2.65	11.600	116.82	17.600	3.10	23.600	1.77
5.800	2.65	11.800	116.82	17.800	3.10	23.800	1.77
6.000	2.65	12.000	116.82	18.000	3.10	24.000	1.77

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Unit Hyd. Gpeak (cms) = .529
 PEAK FLOW (cms) = .432 (1)
 TIME TO PEAK (hrs) = 12.000
 RONOPE VOLUME (mm) = 89.004
 TOTAL RAINFALL (mm) = 177.000
 RONOPE COEFFICIENT = .503

Max. Eff. Inten. (mm/hr) = 116.82
 Storage Coeff. (mm) = 10.00
 Unit Hyd. Gpeak (mm) = 10.00

Surface Area (ha) = 3.74
 Dep. Storage (mm) = 1.00
 Average Slope (ft) = 157.00
 Manning's n = .200

IMPERVIOUS BEHAVIOR (1)
 Dfr. Conn. (ft) = 1.00

Unit Hyd. Peak (cms) = .16
 PEAK FLOW (cms) = .01
 TIME TO PEAK (hrs) = 12.17
 RUNOFF VOLUME (mm) = 176.00
 TOTAL RAINFALL (mm) = 177.00
 RUNOFF COEFFICIENT = .99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) ON PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 50.0 Ia = Dep. Storage (above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALLS
 | STANHYD (0101) | Area (ha) = 2.60
 |-----|-----| Total Imp (s) = 25.00 Dir. Conn. (s) = 12.00
 | ID= 1 DT=10.0 min |
 |-----|-----|
 Surface Area (ha) = 1.65 IMPERVIOUS PERVIOUS (i)
 Dep. Storage (mm) = 1.00 1.95
 Average Slope (s) = 1.50 2.50
 Length (m) = 131.00 131.00
 Mannings n = .150 .200
 Max. Eff. Inchn. (mm/hr) = 116.82 89.83
 Storage Coeff. (min) = 20.00 30.00
 Unit Hyd. Peak (cms) = 20.00 30.00 (ii)
 Unit Hyd. Peak (cms) = .07 .04
 PEAK FLOW (cms) = .09 .29
 TIME TO PEAK (hrs) = 12.00 12.17
 RUNOFF VOLUME (mm) = 176.00 95.80
 TOTAL RAINFALL (mm) = 177.00 177.00
 RUNOFF COEFFICIENT = .99 .54

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

- (i) ON PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 50.0 Ia = Dep. Storage (above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0105) | AREA QPEAK TPEAK R.V.
 | 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID= 1 (0102): 3.74 .473 12.17 89.43
 + ID2= 2 (0101): 2.60 .363 12.17 105.42
 ID = 3 (0105): 6.34 .836 12.17 95.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0107) | AREA QPEAK TPEAK R.V.
 | 1 + 2 = 3 | (ha) (cms) (hrs) (mm)

ID1= 1 (0103): 2.23 .432 12.00 89.00
 + ID2= 2 (0105): 6.34 .836 12.17 95.99
 ID = 3 (0107): 8.57 1.196 12.00 94.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0109) | AREA QPEAK TPEAK R.V.
 | 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0104): 1.96 .237 12.20 88.69
 + ID2= 2 (0107): 8.57 1.196 12.00 94.17
 ID = 3 (0109): 10.53 1.387 12.00 93.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0108) |
 | IN= 2---> OUT= 1 |
 | DT= 5.0 min |
 |-----|-----|
 OUTFLOW STORAGE | OUTFLOW STORAGE
 (cms) (ha.m.) | (cms) (ha.m.)
 .0000 .0000 | .0170 .0525
 .0070 .0015 | .4100 .1756
 AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 INFLOW : ID= 2 (0109) 10.53 1.39 12.00 93.16
 OUTFLOW: ID= 1 (0108) 10.53 .89 12.50 93.15
 PEAK FLOW REDUCTION (Out/In) (s) = 63.96
 TIME SHIFT OF PEAK FLOW (min) = 30.00
 MAXIMUM STORAGE USED (ha.m.) = .2771

FINISH

POND 17 - 25% RAINFALL INTENSITY

V V I SSSSS U U A A L
 V V I SSS U U A A L
 V V I SSS U U A A A L
 V V I SSS U U A A A L
 V V I SSSSS UUUUU A A LLLLLL
 OOO TTTTTT H H Y Y M M K K OOO
 O O T T H H Y Y M M K K O O
 O O T T H H Y Y M M K K O O
 OOO T T H H Y Y M M K K OOO

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**** D E T A I L E D O U T P U T ****

Input filename: C:\Program Files\Visual CTRX\2.3.2\voim.dat
 Output filename: S:\2010 Projects\DR-EE (E10)\WRAV10-487 East Gullinbury SWM Master
 Plan\Calcs\Hydrologic Model\WRAV10-487 East Gullinbury SWM Master
 Summary filename: S:\2010 Projects\DR-EE (E10)\WRAV10-487 East Gullinbury SWM Master
 Plan\Calcs\Hydrologic Model\WRAV10-487 East Gullinbury SWM Master

DATE: 3/21/2011

TIME: 6:34:33 PM

USBR:

COMMENTS:

 ** SIMULATION NUMBER: 1 **

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
1.00	2.21	4.00	3.54	7.00	7.08	10.00	3.10	3.10	1.77
1.25	2.21	4.25	3.54	7.25	7.08	10.25	3.10	3.10	1.77
1.50	2.21	4.50	3.54	7.50	7.08	10.50	3.10	3.10	1.77
1.75	2.21	4.75	3.54	7.75	7.08	11.00	3.10	3.10	1.77
2.00	2.21	5.00	3.54	8.00	7.08	11.25	3.10	3.10	1.77
2.25	2.21	5.25	3.54	8.25	7.08	11.50	3.10	3.10	1.77
2.50	2.21	5.50	3.54	8.50	7.08	11.75	3.10	3.10	1.77
2.75	2.21	5.75	3.54	8.75	7.08	12.00	3.10	3.10	1.77
3.00	2.21	6.00	3.54	9.00	7.08	12.00	3.10	3.10	1.77

filename: S:\2010 Projects\DR-EE (E10)\
 WRAV10-487 East Gullinbury SWM Master Plan\
 Calcs\Hydrologic Model\WRAV10-487 East Gullinbury SWM Master Plan

Pictal= 89.50 mm
 Comments: 100-Year 12-hour SCS II Design Storm

MODIFYING PARAMETERS
 Multiplication Factor = .75
 Time Shift (min) = .00

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
1.250	1.66	4.500	3.98	7.500	3.98	10.500	1.33	1.33	1.33
1.500	1.66	4.750	3.98	7.750	3.98	10.750	1.33	1.33	1.33
1.750	1.66	5.000	3.98	8.000	3.98	11.000	1.33	1.33	1.33
2.000	1.66	5.250	3.98	8.250	3.98	11.250	1.33	1.33	1.33
2.250	1.99	5.500	7.97	8.500	7.97	11.500	1.33	1.33	1.33
2.500	1.99	5.750	7.97	8.750	7.97	11.750	1.33	1.33	1.33
2.750	1.99	6.000	7.97	9.000	7.97	12.000	1.33	1.33	1.33

NOTE: RAINFALL WAS TRANSFORMED TO 12.0 MIN. TIME STEP.
 U.S. Tp(hrs) = .55

UNIT HYD OPEAK (cms) = .169

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
1.600	1.66	4.800	4.65	7.800	3.98	10.800	1.33	1.33	1.33
1.800	1.66	5.000	4.65	8.000	3.98	11.000	1.33	1.33	1.33
2.000	1.66	5.200	4.65	8.200	3.98	11.200	1.33	1.33	1.33
2.200	1.99	5.400	7.97	8.400	7.97	11.400	1.33	1.33	1.33
2.400	1.99	5.600	7.97	8.600	7.97	11.600	1.33	1.33	1.33
2.600	1.99	5.800	7.97	8.800	7.97	11.800	1.33	1.33	1.33
2.800	1.99	6.000	7.97	9.000	7.97	12.000	1.33	1.33	1.33

PEAK FLOW (cms) = .057 (3)
 TIME TO PEAK (hrs) = 1.7430
 MODIFIED PEAK (cms) = 66.372
 MODIFIED RAINFALL (mm) = 66.372
 KOROBE COEFFICIENT = .264

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

Surface Area (ha) = 3.74
 Dep. Storage (mm) = 1.00
 Average Slope (ft) = 1.00
 Length (mi) = 1.00
 Mannings n = .200
 Max. Eff. Inten. (mm/hr) = 13.09
 Storage Coeff. (min) = 50.00
 Unit Hyd. Tpeak (min) = 42.78 (11)
 Unit Hyd. peak (cms) = 50.00
 PEAK FLOW (cms) = 13.09
 TIME TO PEAK (hrs) = 6.67
 RUNOFF VOLUME (mm) = 17.58
 TOTAL RAINFALL (mm) = 66.38
 RUNOFF COEFFICIENT = .26

Unit Hyd Qpeak (cms) = .529
 PEAK FLOW (cms) = .128 (1)
 TIME TO PEAK (hrs) = 6.00
 RUNOFF VOLUME (mm) = 17.583
 TOTAL RAINFALL (mm) = 66.375
 RUNOFF COEFFICIENT = .265

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALLB	AREA	HA	DIR. CONN. (%)	PERVIOUS (%)
STANDBY (0102)	97.61	3.74	1.00	1.00
ID= 1 OF=10.0 min				
Surface Area	97.61	3.74		
Dep. Storage	1.00	1.00		
Average Slope	1.00	1.00		
Length	157.00	157.00		
Mannings n	.200			
Max. Eff. Inten. (mm/hr)	13.09			
Storage Coeff. (min)	50.00			
Unit Hyd. Tpeak (min)	42.78 (11)			
Unit Hyd. peak (cms)	50.00			
PEAK FLOW (cms)	.01			
TIME TO PEAK (hrs)	6.67			
RUNOFF VOLUME (mm)	17.58			
TOTAL RAINFALL (mm)	66.38			
RUNOFF COEFFICIENT	.26			

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 60.0 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALLB	AREA	HA	DIR. CONN. (%)
STANDBY (0101)	2.60	2.60	12.00
ID= 1 OF=10.0 min			

Surface Area (ha) = 1.00
 Dep. Storage (mm) = 1.00
 Average Slope (ft) = 1.00
 Length (mi) = 131.00
 Mannings n = .200
 Max. Eff. Inten. (mm/hr) = 73.68
 Storage Coeff. (min) = 20.00
 Unit Hyd. Tpeak (min) = 18.13 (11)
 Unit Hyd. peak (cms) = 20.00
 PEAK FLOW (cms) = .06
 TIME TO PEAK (hrs) = 6.17
 RUNOFF VOLUME (mm) = 65.38
 TOTAL RAINFALL (mm) = 66.38
 RUNOFF COEFFICIENT = .98

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

CN* = 60.0 Ia = Dep. Storage (Above)

(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

THAN THE STORAGE COEFFICIENT.

(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0105)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3				
ID1= 1 (0102):	5.74	.081	6.67	17.97
+ ID2= 2 (0102):	2.60	.073	6.17	25.23
ID = 3 (0105):	6.34	.153	6.67	20.95

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 60.0 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0109)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3				
ID1= 1 (0104):	1.96	.057	6.40	17.52
+ ID2= 2 (0107):	8.57	.223	6.00	20.08
ID = 3 (0109):	10.53	.252	6.00	19.60

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:

CN* = 60.0 Ia = Dep. Storage (Above)

(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

THAN THE STORAGE COEFFICIENT.

(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

POND 17 - +25% RAINFALL INTENSITY

```

I DTE= 5.0 min |
OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.)
.0010 | .0010 | .0110 | .0325
.0070 | .0015 | .4100 | .1756

```

INFLOW : ID= 2 (0109) (ha) (cms) (ft) (ft) (ft)

OUTFLOW: ID= 1 (0108) 10.53 .25 6.00 19.60

7.67 7.67 19.59

PEAK FLOW REDUCTION (Out/In) (%) = 35.78

TIME SKEW OF PEAK FLOW (min) = 100.00

MAXIMUM STORAGE USED (ha.m.) = .1080

FINISH

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**** D E F A I L T O U T P U T ****

Input filename: C:\Program Files\Visual\GTP\PRO 2.3 2\vsoln.dat
 Output filename: s:\2010 Projects\D-EE (E10)\WRM10-487 East Gwillimbury SWM Master Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas Summary filename: s:\2010 Projects\D-EE (E10)\WRM10-487 East Gwillimbury SWM Master Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas

DATE: 3/21/2011

TIME: 6:41:12 PM

USER:

COMMENTS:

***** SIMULATION NUMBER: 1 ****

 READ STORM | | Filename: s:\2010 Projects\D-EE (E10)\WRM10-487 East Gwillimbury SWM Master Plan\Calcs\Hydrologic Models\Existing IAPRC Files\Protal= 88.50 mm | Comments: 100-Year 12-Hour SCS II Design Storm

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
1.25	2.21	3.25	3.54	6.25	15.93	9.25	3.10
1.50	2.21	3.50	3.54	6.50	15.93	9.50	3.10
1.75	2.21	3.75	3.54	6.75	15.93	9.75	3.10
2.00	2.21	4.00	3.54	7.00	15.93	10.00	3.10
2.25	2.21	4.25	3.54	7.25	15.93	10.25	3.10
2.50	2.65	4.50	3.54	7.50	15.93	10.50	3.10
2.75	2.65	4.75	3.54	7.75	15.93	10.75	3.10
3.00	2.65	5.00	3.54	8.00	15.93	11.00	3.10
		5.25	10.62	8.25	3.10	11.25	1.77
		5.50	10.62	8.50	3.10	11.50	1.77
		5.75	42.46	8.75	3.10	11.75	1.77
		6.00	116.82	9.00	3.10	12.00	1.77

MODIFY STORM |
 CASE= 1 |
 Multiplication Factor= 1.25
 Time shift (min)= .00

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
2.50	2.77	3.250	4.43	6.250	19.91	9.25	3.87
3.00	2.77	3.500	4.43	6.500	19.91	9.50	3.87
3.50	2.77	3.750	4.43	6.750	8.85	9.75	3.87
4.00	2.77	4.000	4.43	7.000	8.85	10.00	3.87
4.50	2.77	4.250	6.64	7.250	6.64	10.25	2.21
5.00	2.77	4.500	6.64	7.500	6.64	10.50	2.21
5.50	2.77	4.750	8.85	7.750	6.64	10.75	2.21
6.00	2.77	5.000	8.85	8.000	6.64	11.00	2.21
6.50	3.32	5.250	13.28	8.250	3.87	11.25	2.21
7.00	3.32	5.500	13.28	8.500	3.87	11.50	2.21
7.50	3.32	5.750	13.28	8.750	3.87	11.75	2.21
8.00	3.32	6.000	146.02	9.000	3.87	12.00	2.21

Unit Hyd Qpeak (cms)= .529

PEAK FLOW (cms)= .312 (i)
 TIME TO PEAK (hrs)= 6.000
 RUNOFF VOLUME (mm)= 42.347
 TOTAL RAINFALL (mm)= 110.625
 RUNOFF COEFFICIENT = .383

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALLIB (0104) | Area (ha)= 1.96 Curve Number (CN)= 60.0
 WASHYD (0104) | Area (ha)= 2.50 # of Linear Res. (N)= 4.00
 ID= 1 DT=12.0 min | V.H. Tp (hrs)= .55

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
2.50	2.77	3.250	4.43	6.250	19.91	9.25	3.87
3.00	2.77	3.500	4.43	6.500	19.91	9.50	3.87
3.50	2.77	3.750	4.43	6.750	8.85	9.75	3.87
4.00	2.77	4.000	4.43	7.000	8.85	10.00	3.87
4.50	2.77	4.250	6.64	7.250	6.64	10.25	2.21
5.00	2.77	4.500	6.64	7.500	6.64	10.50	2.21
5.50	2.77	4.750	8.85	7.750	6.64	10.75	2.21
6.00	2.77	5.000	8.85	8.000	6.64	11.00	2.21
6.50	3.32	5.250	13.28	8.250	3.87	11.25	2.21
7.00	3.32	5.500	13.28	8.500	3.87	11.50	2.21
7.50	3.32	5.750	13.28	8.750	3.87	11.75	2.21
8.00	3.32	6.000	146.02	9.000	3.87	12.00	2.21

NOTE: RAINFALL WAS TRANSFORMED TO 12.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
2.50	2.77	3.250	4.43	6.250	19.91	9.25	3.87
3.00	2.77	3.500	4.43	6.500	19.91	9.50	3.87
3.50	2.77	3.750	4.43	6.750	8.85	9.75	3.87
4.00	2.77	4.000	4.43	7.000	8.85	10.00	3.87
4.50	2.77	4.250	6.64	7.250	6.64	10.25	2.21
5.00	2.77	4.500	6.64	7.500	6.64	10.50	2.21
5.50	2.77	4.750	8.85	7.750	6.64	10.75	2.21
6.00	2.77	5.000	8.85	8.000	6.64	11.00	2.21
6.50	3.32	5.250	13.28	8.250	3.87	11.25	2.21
7.00	3.32	5.500	13.28	8.500	3.87	11.50	2.21
7.50	3.32	5.750	13.28	8.750	3.87	11.75	2.21
8.00	3.32	6.000	146.02	9.000	3.87	12.00	2.21

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
2.50	2.77	3.250	4.43	6.250	19.91	9.25	3.87
3.00	2.77	3.500	4.43	6.500	19.91	9.50	3.87
3.50	2.77	3.750	4.43	6.750	8.85	9.75	3.87
4.00	2.77	4.000	4.43	7.000	8.85	10.00	3.87
4.50	2.77	4.250	6.64	7.250	6.64	10.25	2.21
5.00	2.77	4.500	6.64	7.500	6.64	10.50	2.21
5.50	2.77	4.750	8.85	7.750	6.64	10.75	2.21
6.00	2.77	5.000	8.85	8.000	6.64	11.00	2.21
6.50	3.32	5.250	13.28	8.250	3.87	11.25	2.21
7.00	3.32	5.500	13.28	8.500	3.87	11.50	2.21
7.50	3.32	5.750	13.28	8.750	3.87	11.75	2.21
8.00	3.32	6.000	146.02	9.000	3.87	12.00	2.21

MODIFY STORM |
 CASE= 1 |
 Multiplication Factor= 1.25
 Time shift (min)= .00

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
2.77	3.167	3.500	4.43	6.167	19.91	9.17	3.87
3.25	2.77	3.333	4.43	6.333	19.91	9.33	3.87
3.75	2.77	3.500	4.43	6.500	19.91	9.50	3.87
4.25	2.77	3.667	4.43	6.667	8.85	9.67	3.87
4.75	2.77	3.833	4.43	6.833	8.85	9.83	3.87
5.25	2.77	4.000	4.43	7.000	8.85	10.00	3.87
5.75	2.77	4.167	6.64	7.167	6.64	10.17	2.21
6.25	2.77	4.333	6.64	7.333	6.64	10.33	2.21
6.75	2.77	4.500	6.64	7.500	6.64	10.50	2.21
7.25	2.77	4.667	8.85	7.667	6.64	10.67	2.21
7.75	2.77	4.833	8.85	7.833	6.64	10.83	2.21
8.25	2.77	5.000	8.85	8.000	6.64	11.00	2.21
8.75	3.32	5.167	13.28	8.167	3.87	11.17	2.21
9.25	3.32	5.333	13.28	8.333	3.87	11.33	2.21
9.75	3.32	5.500	13.28	8.500	3.87	11.50	2.21
10.25	3.32	5.667	13.28	8.667	3.87	11.67	2.21
10.75	3.32	5.833	13.28	8.833	3.87	11.83	2.21
11.25	3.32	6.000	146.02	9.000	3.87	12.00	2.21

Unit Hyd Qpeak (cms)= .529

PEAK FLOW (cms)= .312 (i)
 TIME TO PEAK (hrs)= 6.000
 RUNOFF VOLUME (mm)= 42.347
 TOTAL RAINFALL (mm)= 110.625
 RUNOFF COEFFICIENT = .383

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALLIB (0103) | Area (ha)= 3.74
 WASHYD (0103) | Area (ha)= 1.00 Dlr. Conn. (%)= 1.00
 ID= 1 DT=10.0 min | Total Imp (%)= 1.00

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
2.77	3.167	3.500	4.43	6.167	19.91	9.17	3.87
3.25	2.77	3.333	4.43	6.333	19.91	9.33	3.87
3.75	2.77	3.500	4.43	6.500	19.91	9.50	3.87
4.25	2.77	3.667	4.43	6.667	8.85	9.67	3.87
4.75	2.77	3.833	4.43	6.833	8.85	9.83	3.87
5.25	2.77	4.000	4.43	7.000	8.85	10.00	3.87
5.75	2.77	4.167	6.64	7.167	6.64	10.17	2.21
6.25	2.77	4.333	6.64	7.333	6.64	10.33	2.21
6.75	2.77	4.500	6.64	7.500	6.64	10.50	2.21
7.25	2.77	4.667	8.85	7.667	6.64	10.67	2.21
7.75	2.77	4.833	8.85	7.833	6.64	10.83	2.21
8.25	2.77	5.000	8.85	8.000	6.64	11.00	2.21
8.75	3.32	5.167	13.28	8.167	3.87	11.17	2.21
9.25	3.32	5.333	13.28	8.333	3.87	11.33	2.21
9.75	3.32	5.500	13.28	8.500	3.87	11.50	2.21
10.25	3.32	5.667	13.28	8.667	3.87	11.67	2.21
10.75	3.32	5.833	13.28	8.833	3.87	11.83	2.21
11.25	3.32	6.000	146.02	9.000	3.87	12.00	2.21

Unit Hyd Qpeak (cms)= .169

PEAK FLOW (cms)= .139 (i)
 TIME TO PEAK (hrs)= 6.400
 RUNOFF VOLUME (mm)= 42.198
 TOTAL RAINFALL (mm)= 110.625
 RUNOFF COEFFICIENT = .381

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALLIB (0103) | Area (ha)= 2.23 Curve Number (CN)= 60.0
 WASHYD (0103) | Area (ha)= 2.50 # of Linear Res. (N)= 4.00
 ID= 1 DT=10.0 min | U.H. Tp (hrs)= .20

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
2.77	3.167	3.500	4.43	6.167	19.91	9.17	3.87
3.25	2.77	3.333	4.43	6.333	19.91	9.33	3.87
3.75	2.77	3.500	4.43	6.500	19.91	9.50	3.87
4.25	2.77	3.667	4.43	6.667	8.85	9.67	3.87
4.75	2.77	3.833	4.43	6.833	8.85	9.83	3.87
5.25	2.77	4.000	4.43	7.000	8.85	10.00	3.87
5.75	2.77	4.167	6.64	7.167	6.64	10.17	2.21
6.25	2.77	4.333	6.64	7.333	6.64	10.33	2.21
6.75	2.77	4.500	6.64	7.500	6.64	10.50	2.21
7.25	2.77	4.667	8.85	7.667	6.64	10.67	2.21
7.75	2.77	4.833	8.85	7.833	6.64	10.83	2.21
8.25	2.77	5.000	8.85	8.000	6.64	11.00	2.21
8.75	3.32	5.167	13.28	8.167	3.87	11.17	2.21
9.25	3.32	5.333	13.28	8.333	3.87	11.33	2.21
9.75	3.32	5.500	13.28	8.500	3.87	11.50	2.21
10.25	3.32	5.667	13.28	8.667	3.87	11.67	2.21
10.75	3.32	5.833	13.28	8.833	3.87	11.83	2.21
11.25	3.32	6.000	146.02	9.000	3.87	12.00	2.21

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
2.77	3.167	3.500	4.43	6.167	19.91	9.17	3.87
3.25	2.77	3.333	4.43	6.333	19.91	9.33	3.87
3.75	2.77	3.500	4.43	6.500	19.91	9.50	3.87
4.25	2.77	3.667	4.43	6.667	8.85	9.67	3.87
4.75	2.77	3.833	4.43	6.833	8.85	9.83	3.87
5.25	2.77	4.000	4.43	7.000	8.85	10.00	3.87
5.75	2.77	4.167	6.64	7.167	6.64	10.17	2.21
6.25	2.77	4.333	6.64	7.333	6.64	10.33	2.21
6.75	2.77	4.500	6.64	7.500	6.64	10.50	2.21
7.25	2.77	4.667	8.85	7.667	6.64	10.67	2.21
7.75	2.77	4.833	8.85	7.833	6.64	10.83	2.21
8.25	2.77	5.000	8.85	8.000	6.64	11.00	2.21
8.75	3.32	5.167	13.28	8.167	3.87	11.17	2.21
9.25	3.32	5.333	13.28	8.333	3.87	11.33	2.21
9.75	3.32	5.500	13.28	8.500	3.87	11.50	2.21
10.25	3.32	5.667	13.28	8.667	3.87	11.67	2.21
10.75	3.32	5.833	13.28	8.833	3.87	11.83	2.21
11.25	3.32	6.000	146.02	9.000	3.87	12.00	2.21

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

CALLIB (0103) | Area (ha)= 2.60

IMPERVIOUS PERVIOUS (1)

Surface Area	(ha)	=	146.02	67.62	30.00	(11)
Depth Storage	(mm)	=	10.00	13.79	29.83	(11)
Average Slope	(%)	=	.50	10.00	30.00	
Length	(m)	=	131.00	131.00		
Manning's n		=	.150	.200		
Max Eff. Inletn. (mm/hr)		=	146.02	67.62	30.00	
Storage Coeff. (m/n)		=	13.79	29.83	30.00	
Unit Hyd. Peak (cms)		=	10.00	30.00	.04	
Peak Flow (cms)		=	.09	.18	.215	(11)
Time to Peak (hrs)		=	6.00	6.33	6.33	
Runoff Volume (mm)		=	109.62	110.63	54.12	
Total Rainfall (mm)		=	110.63	110.63	110.63	
Runoff Coefficient		=	.99	.42	.49	

TOTALS

***** WARNING: FOR BASINS WITH IMPERVIOUS RATIOS BELOW 20%
 100 SHOULD CONSIDER SPLITTING THE AREA.

{1} CN PROCEDURE SELECTED FOR PERVIOUS LOSSSES:
 CN* = 60.0 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (11) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0105) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0102): 3.74 .271 6.33 42.81
 + ID2= 2 (0105): 2.60 .215 6.33 54.12
 ID = 3 (0105): 6.34 .485 6.33 47.45

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0107) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0103): 2.23 .312 6.00 42.35
 + ID2= 2 (0105): 6.34 .485 6.33 47.45
 ID = 3 (0107): 8.57 .664 6.00 46.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0109) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1= 1 (0104): 1.96 .139 6.40 42.20
 + ID2= 2 (0107): 8.57 .684 6.00 46.12
 ID = 3 (0109): 10.53 .771 6.17 45.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0108) |
 INP 2--> OUP 1 |

DT= 5.0 min

OUTFLOW (cms)	0.0070	STORAGE (ha)	0.0015	OUTFLOW (cms)	0.4100	STORAGE (mm)	1.756
INFLOW : ID= 2 (0109)	10.53	AREA (ha)	.77	TPEAK (hrs)	6.17	R.V. (mm)	45.38
OUTFLOW: ID= 1 (0108)	10.53	QPEAK (cms)	.43	TPEAK (hrs)	6.75	R.V. (mm)	45.38

PEAK FLOW RESTRICTION [Oult/OIn] (%) = 55.28
 TIME SHIFT OF PEAK FLOW (min) = 35.00
 MAXIMUM STORAGE USED (ha.m.) = .1793

FINISH

POND 17 - CN AMCI

V V I SSSS U U A I
 V V I SS U U A A I
 V V I SS U U A A A A I
 V V I SS U U A A I
 W V I SSSS UUUU A A LLLL

OOO TTTT TTTT H H Y Y M M OOO
 O O T T T H H Y Y M M O O
 O O T T T H H Y Y M M O O
 OOO T T T H H Y Y M M OOO

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***** D E T A I L E D O U T P U T *****

Input filename: C:\Program Files\Visual OFHYMO 2.3.2\vein.dat
 Output filename: S:\2010 Projects\P-EE (E10)\WRAW10-487 East Gwillimbury SWM Master
 Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bns
 Summary filename: S:\2010 Projects\P-EE (E10)\WRAW10-487 East Gwillimbury SWM Master
 Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bns

DATE: 3/21/2011 TIME: 8:01:35 PM
 USER:

COMMENTS:

 ** SIMULATION NUMBER: 1 **

Filename: S:\2010 Projects\P-EE (E10)\
 WRAW10-487 East Gwillimbury SWM Master Plan\
 Calcs\Hydrologic Models\Existing LSRA Files\
 Comments: 100-Year 12-Hour SCS II Design Storm

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
2.21	3.25	3.54	6.25	15.93	9.25
3.10	3.50	3.54	6.50	15.93	9.50
4.00	3.75	3.54	6.75	7.08	3.10
5.00	4.00	3.54	7.00	7.08	3.10
6.00	4.25	3.54	7.25	5.31	10.50
7.08	4.50	5.31	7.53	5.31	10.50
8.00	4.75	7.08	7.75	5.31	10.75
9.00	5.00	7.08	8.00	5.31	11.00
10.00	5.25	10.62	8.25	3.10	11.25
11.00	5.50	10.62	8.50	3.10	11.50
12.00	5.75	42.48	8.75	3.10	11.75
13.00	6.00	116.82	9.00	3.10	12.00

CALIB (0104) | Area (ha)= 1.96 Curve Number (CN)= 40.0
 NASHYD | Ia (cms)= 2.50 # of Linear Res. (N)= 4.00
 ID= 1 DT=12.0 min | U.H. Tp (hrs)= .55

NOTE: RAINFALL WAS TRANSFORMED TO 12.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
2.21	3.200	3.54	6.200	15.93	9.20
3.10	3.400	3.54	6.400	15.93	9.40
4.00	3.600	3.54	6.600	11.51	9.60
5.00	3.800	3.54	6.800	7.08	9.80
6.00	4.000	3.54	7.000	7.08	10.00
7.08	4.200	5.31	7.200	5.31	10.20
8.00	4.400	5.31	7.400	5.31	10.40
9.00	4.600	6.19	7.600	5.31	10.60
10.00	4.800	7.08	7.800	5.31	10.80
11.00	5.000	7.08	8.000	5.31	11.00
12.00	5.200	10.62	8.200	3.10	11.20
13.00	5.400	10.62	8.400	3.10	11.40
14.00	5.600	26.55	8.600	3.10	11.60
15.00	5.800	61.06	8.800	3.10	11.80
16.00	6.000	116.82	9.000	3.10	12.00

Unit Hyd Qpeak (cms)= .169

PEAK FLOW (cms)= .050 (1)
 TIME TO PEAK (hrs)= 6.400
 RUNOFF VOLUME (mm)= 23.860
 TOTAL RAINFALL (mm)= 88.000
 RUNOFF COEFFICIENT = .179

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0103) | Area (ha)= 2.23 Curve Number (CN)= 40.0
 NASHYD | Ia (cms)= 2.50 # of Linear Res. (N)= 4.00
 ID= 1 DT=10.0 min | U.H. Tp (hrs)= .20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
2.21	3.333	3.54	6.333	15.93	9.33
3.10	3.500	3.54	6.500	15.93	9.50
4.00	3.667	3.54	6.667	7.08	9.67
5.00	3.833	3.54	6.833	7.08	9.83
6.00	4.000	3.54	7.000	7.08	10.00
7.08	4.167	5.31	7.167	5.31	10.17
8.00	4.333	5.31	7.333	5.31	10.33
9.00	4.500	5.31	7.500	5.31	10.50
10.00	4.667	7.08	7.667	5.31	10.67
11.00	4.833	7.08	7.833	5.31	10.83
12.00	5.000	7.08	8.000	5.31	11.00
13.00	5.167	10.62	8.167	3.10	11.17
14.00	5.333	10.62	8.333	3.10	11.33
15.00	5.500	10.62	8.500	3.10	11.50
16.00	5.667	26.55	8.667	3.10	11.67
17.00	5.833	61.06	8.833	3.10	11.83
18.00	6.000	116.82	9.000	3.10	12.00

Unit Hyd Qpeak (cms)= .529

PEAK FLOW (cms) = .114 (1)
 TIME TO PEAK (hrs) = 6.00
 TIME TO RISE (hrs) = 15.912
 TOTAL RAINFALL (mm) = 88.500
 RINFOW COEFFICIENT = .180

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALLIB	Area (ha)	Dif. Conn. (%)
STANDHYD (0102)	3.74	1.00
ID= 1 DF=10.0 min		
Surface Area (ha)	1.04	
Dep. Storage (mm)	2.50	
Average Slope (%)	1.00	
Length (m)	157.00	
Manning's n	.015	
Max. Eff. Inten. (mm/hr)	116.82	10.14
Storage Coeff. (mb)	10.00	50.00
Unit Hyd. Peak (mb)	3.43 (11)	46.56 (11)
Unit Hyd. Peak (cms)	10.00	50.00
Unit Hyd. Peak (cms)	.16	.02
PEAK FLOW (cms)	.01	.07
TIME TO PEAK (hrs)	6.00	6.67
RINFOW VOLUME (mm)	87.50	15.84
TOTAL RAINFALL (mm)	88.50	88.50
RINFOW COEFFICIENT	.99	.18

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEPI *****
 ***** WARNING: FOR AREAS WITH IMPERVIOUS PORTS BELOW 208 YOU SHOULD CONSIDER SPLITTING THE AREA.
 (1) ON PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 (11) TIME STEPI (TP) SHOULD BE SMALLER OR EQUAL TO
 (11) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALLIB	Area (ha)	Dif. Conn. (%)
STANDHYD (0101)	2.60	12.00
ID= 1 DF=10.0 min		
Surface Area (ha)	.65	1.95
Dep. Storage (mm)	1.00	2.50
Average Slope (%)	1.50	1.50
Length (m)	131.00	131.00
Manning's n	.150	.200
Max. Eff. Inten. (mm/hr)	98.23	19.12
Storage Coeff. (mb)	20.00	50.00
Unit Hyd. Peak (mb)	16.16 (11)	42.74 (11)
Unit Hyd. Peak (cms)	20.00	50.00
Unit Hyd. Peak (cms)	.06	.03
PEAK FLOW (cms)	.06	.05
TIME TO PEAK (hrs)	6.17	6.67
RINFOW VOLUME (mm)	87.50	18.15
TOTAL RAINFALL (mm)	88.50	88.50
RINFOW COEFFICIENT	.99	.21

***** WARNING: FOR AREAS WITH IMPERVIOUS PORTS BELOW 208 YOU SHOULD CONSIDER SPLITTING THE AREA.
 ***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEPI *****
 ***** WARNING: FOR AREAS WITH IMPERVIOUS PORTS BELOW 208 YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) ON PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 (11) TIME STEPI (TP) SHOULD BE SMALLER OR EQUAL TO
 (11) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0105)	AREA (ha)	QPEAK (cms)	TPPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID= 1 (0102):	3.74	.069	6.67	16.55
+ ID= 2 (0101):	2.60	.087	6.17	26.46
ID = 3 (0105):	6.34	.140	6.67	20.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.
 ADD HYD (0107)
 1 + 2 = 3
 AREA (ha) QPEAK (cms) TPPEAK (hrs) R.V. (mm)
 ID= 1 (0103): 2.23 .114 6.00 15.86
 + ID= 2 (0105): 6.34 .140 6.67 20.61
 ID = 3 (0107): 8.57 .221 6.00 19.39

RESERVOIR (0108)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUT= 1				
DF= 5.0 min				
AREA (ha)	QPEAK (cms)	TPPEAK (hrs)	R.V. (mm)	
INFLOW: ID= 2 (0108)	10.53	.25	5.93	18.73
OUTFLOW: ID= 1 (0108)	10.53	.09	7.93	18.72
PEAK FLOW REDUCTION [Qout/ Qin] (%) = 31.77				
TIME SHIFT OF PEAK FLOW (mb) = -10.00				
MAXIMUM STORAGE USED (ha.m.) = 1.055				

FINISH

PEAK FLOW (cms) = 345 (1)
 TIME TO PEAK (hrs) = 6.000
 RUNOFF VOLUME (mm) = 45.320
 TOTAL RAINFALL (mm) = 88.500
 RUNOFF COEFFICIENT = .519

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

---CALIB (0102)---
 | ID= 1 DT=10.0 min | Area (ha) = 3.74
 | Total Imp(%) = 1.00 | Dir. Conn.(%) = 1.00
 ---RESERVOIR (0109)---
 | ID= 1 DT=10.0 min | Area (ha) = 2.60
 | Total Imp(%) = 25.00 | Dir. Conn.(%) = 12.00

---SURFACE AREA---
 | Surface Area (ha) = 1.04
 | Dep. Storage (mm) = 1.00
 | Average Slope (%) = 157.00
 | Manning's n = .025
 ---RESERVOIR (0109)---
 | ID= 1 DT=10.0 min | Area (ha) = 2.60
 | Total Imp(%) = 25.00 | Dir. Conn.(%) = 12.00

---PEAK FLOW---
 | TIME TO PEAK (hrs) = 6.00
 | RUNOFF VOLUME (mm) = 87.50
 | TOTAL RAINFALL (mm) = 88.50
 | RUNOFF COEFFICIENT = .99
 ---RESERVOIR (0109)---
 | ID= 1 DT=10.0 min | Area (ha) = 2.60
 | Total Imp(%) = 25.00 | Dir. Conn.(%) = 12.00

---PEAK FLOW---
 | TIME TO PEAK (hrs) = 6.17
 | RUNOFF VOLUME (mm) = 87.50
 | TOTAL RAINFALL (mm) = 88.50
 | RUNOFF COEFFICIENT = .99
 ---RESERVOIR (0109)---
 | ID= 1 DT=10.0 min | Area (ha) = 2.60
 | Total Imp(%) = 25.00 | Dir. Conn.(%) = 12.00

---PEAK FLOW---
 | TIME TO PEAK (hrs) = 6.17
 | RUNOFF VOLUME (mm) = 87.50
 | TOTAL RAINFALL (mm) = 88.50
 | RUNOFF COEFFICIENT = .99
 ---RESERVOIR (0109)---
 | ID= 1 DT=10.0 min | Area (ha) = 2.60
 | Total Imp(%) = 25.00 | Dir. Conn.(%) = 12.00

---PEAK FLOW---
 | TIME TO PEAK (hrs) = 6.17
 | RUNOFF VOLUME (mm) = 87.50
 | TOTAL RAINFALL (mm) = 88.50
 | RUNOFF COEFFICIENT = .99
 ---RESERVOIR (0109)---
 | ID= 1 DT=10.0 min | Area (ha) = 2.60
 | Total Imp(%) = 25.00 | Dir. Conn.(%) = 12.00

---PEAK FLOW---
 | TIME TO PEAK (hrs) = 6.17
 | RUNOFF VOLUME (mm) = 87.50
 | TOTAL RAINFALL (mm) = 88.50
 | RUNOFF COEFFICIENT = .99
 ---RESERVOIR (0109)---
 | ID= 1 DT=10.0 min | Area (ha) = 2.60
 | Total Imp(%) = 25.00 | Dir. Conn.(%) = 12.00

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSSES:
 CN* = 77.0 Ia = Dep. Storage (above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

---ADD HYD (0107)---
 | 1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 | ID1= 1 (0102): 3.74 .303 6.33 46.10
 + ID2= 2 (0101): 2.60 .205 6.50 53.96
 | ID = 3 (0109): 6.34 .506 6.33 49.52
 ---RESERVOIR (0109)---
 | ID= 1 DT=10.0 min | Area (ha) = 2.60
 | Total Imp(%) = 25.00 | Dir. Conn.(%) = 12.00

---ADD HYD (0107)---
 | 1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 | ID1= 1 (0103): 2.23 .345 6.00 45.92
 + ID2= 2 (0105): 6.34 .506 6.33 49.32
 | ID = 3 (0107): 8.57 .681 6.17 48.44
 ---RESERVOIR (0109)---
 | ID= 1 DT=10.0 min | Area (ha) = 2.60
 | Total Imp(%) = 25.00 | Dir. Conn.(%) = 12.00

---ADD HYD (0107)---
 | 1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 | ID1= 1 (0104): 1.96 .154 6.40 45.76
 + ID2= 2 (0107): 8.57 .681 6.17 48.44
 | ID = 3 (0109): 10.53 .814 6.17 47.93
 ---RESERVOIR (0109)---
 | ID= 1 DT=10.0 min | Area (ha) = 2.60
 | Total Imp(%) = 25.00 | Dir. Conn.(%) = 12.00

---ADD HYD (0107)---
 | 1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 | ID1= 1 (0104): 1.96 .154 6.40 45.76
 + ID2= 2 (0107): 8.57 .681 6.17 48.44
 | ID = 3 (0109): 10.53 .814 6.17 47.93
 ---RESERVOIR (0109)---
 | ID= 1 DT=10.0 min | Area (ha) = 2.60
 | Total Imp(%) = 25.00 | Dir. Conn.(%) = 12.00

---ADD HYD (0107)---
 | 1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 | ID1= 1 (0104): 1.96 .154 6.40 45.76
 + ID2= 2 (0107): 8.57 .681 6.17 48.44
 | ID = 3 (0109): 10.53 .814 6.17 47.93
 ---RESERVOIR (0109)---
 | ID= 1 DT=10.0 min | Area (ha) = 2.60
 | Total Imp(%) = 25.00 | Dir. Conn.(%) = 12.00

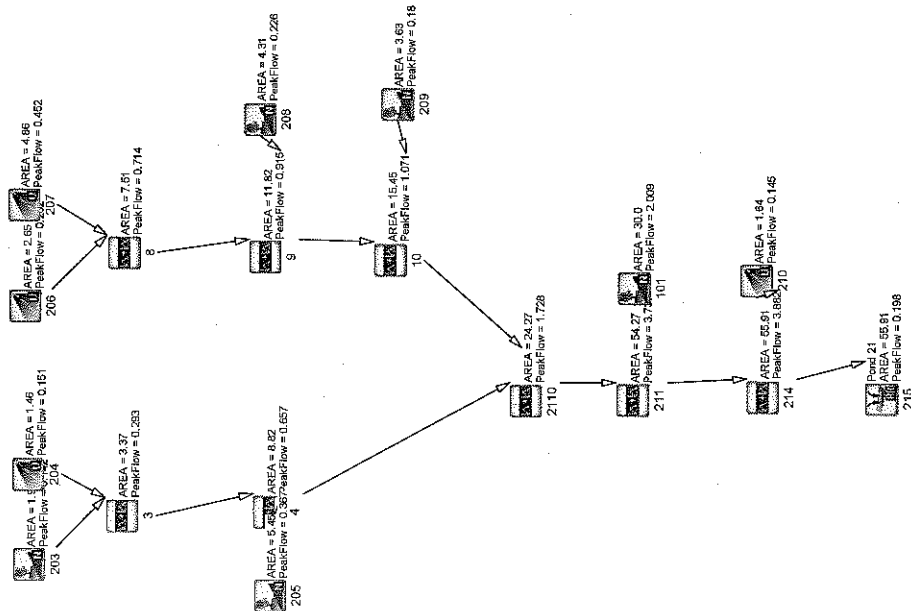
---ADD HYD (0107)---
 | 1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 | ID1= 1 (0104): 1.96 .154 6.40 45.76
 + ID2= 2 (0107): 8.57 .681 6.17 48.44
 | ID = 3 (0109): 10.53 .814 6.17 47.93
 ---RESERVOIR (0109)---
 | ID= 1 DT=10.0 min | Area (ha) = 2.60
 | Total Imp(%) = 25.00 | Dir. Conn.(%) = 12.00

---ADD HYD (0107)---
 | 1 + 2 = 3 | AREA (ha) QPEAK (cms) TPEAK (hrs) R.V. (mm)
 | ID1= 1 (0104): 1.96 .154 6.40 45.76
 + ID2= 2 (0107): 8.57 .681 6.17 48.44
 | ID = 3 (0109): 10.53 .814 6.17 47.93
 ---RESERVOIR (0109)---
 | ID= 1 DT=10.0 min | Area (ha) = 2.60
 | Total Imp(%) = 25.00 | Dir. Conn.(%) = 12.00

***** WARNING: FOR AREAS WITH INTERVIOUS PATIORS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

FINISH

POND 21 - BASE FILE



V V I SSSSS U U A A L
 V V I SS U U A A L
 V V I SS U U A A A A L
 V V I SS U U A A L
 V V I SSSSS UUUU A A LLLL
 OOO TTTT H H Y Y M M OOO
 O O T T H H Y Y M M O O
 O O T T H H Y Y M M O O
 O O T T H H Y Y M M O O

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***** DETAILED OUTPUT *****

Input filename: C:\Program Files\Visual OTTHMO 2.3.2\voim.dat
 Output filename: S:\2010 Projects\D-EE (E10)\WR\W10-487 East Gwillimbury SWM Master Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas
 Summary filename: S:\2010 Projects\D-EE (E10)\WR\W10-487 East Gwillimbury SWM Master Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas

DATE: 3/30/2011 TIME: 11:10:20 AM
 USER:

COMMENTS:

 ** SIMULATION NUMBER: 1 **

Filename: S:\2010 Projects\D-EE (E10)\WR\W10-487 East Gwillimbury SWM Master Plan\Calcs\Hydrologic Models\Existing USRCA Files\Protal=L20.71.ran
 Comments: 109Hr/Lhr

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
-25	0.00	3.50	20.52	6.75	8.45	10.00	1.21
0.50	1.21	3.75	20.52	7.00	8.45	10.25	1.21
1.00	1.21	4.00	20.52	7.25	8.45	10.50	1.21
1.25	1.21	4.25	20.52	7.50	8.45	11.00	1.21
1.50	1.21	4.50	20.52	7.75	8.45	11.25	1.21
1.75	1.21	4.75	20.52	8.00	8.45	11.50	1.21
2.00	1.21	5.00	20.52	8.25	8.45	11.75	1.21
2.25	1.21	5.25	20.52	8.50	8.45	12.00	1.21
2.50	1.21	5.50	20.52	8.75	8.45	12.25	1.21
2.75	1.21	5.75	20.52	9.00	8.45	12.50	1.21
3.00	1.21	6.00	20.52	9.25	8.45	12.75	1.21
3.25	1.21	6.25	20.52	9.50	8.45	13.00	1.21
3.50	1.21	6.50	20.52	9.75	8.45	13.25	1.21

STANDARD (0207) | Area (ha) = 30.00 Curve Number (CN) = 54.2
ID= 1 DT= 5.0 min | Ia (mm) = 1.50 # of Linear Res. (N) = 3.00
U.H. Tp(hrs) = .18

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.05	0.00	0.05	0.00	0.05	0.00	0.05	0.00
0.10	0.00	0.10	0.00	0.10	0.00	0.10	0.00
0.15	0.00	0.15	0.00	0.15	0.00	0.15	0.00
0.20	0.00	0.20	0.00	0.20	0.00	0.20	0.00
0.25	0.00	0.25	0.00	0.25	0.00	0.25	0.00
0.30	0.00	0.30	0.00	0.30	0.00	0.30	0.00
0.35	0.00	0.35	0.00	0.35	0.00	0.35	0.00
0.40	0.00	0.40	0.00	0.40	0.00	0.40	0.00
0.45	0.00	0.45	0.00	0.45	0.00	0.45	0.00
0.50	0.00	0.50	0.00	0.50	0.00	0.50	0.00
0.55	0.00	0.55	0.00	0.55	0.00	0.55	0.00
0.60	0.00	0.60	0.00	0.60	0.00	0.60	0.00
0.65	0.00	0.65	0.00	0.65	0.00	0.65	0.00
0.70	0.00	0.70	0.00	0.70	0.00	0.70	0.00
0.75	0.00	0.75	0.00	0.75	0.00	0.75	0.00
0.80	0.00	0.80	0.00	0.80	0.00	0.80	0.00
0.85	0.00	0.85	0.00	0.85	0.00	0.85	0.00
0.90	0.00	0.90	0.00	0.90	0.00	0.90	0.00
0.95	0.00	0.95	0.00	0.95	0.00	0.95	0.00
1.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00
1.05	0.00	1.05	0.00	1.05	0.00	1.05	0.00
1.10	0.00	1.10	0.00	1.10	0.00	1.10	0.00
1.15	0.00	1.15	0.00	1.15	0.00	1.15	0.00
1.20	0.00	1.20	0.00	1.20	0.00	1.20	0.00
1.25	0.00	1.25	0.00	1.25	0.00	1.25	0.00
1.30	0.00	1.30	0.00	1.30	0.00	1.30	0.00
1.35	0.00	1.35	0.00	1.35	0.00	1.35	0.00
1.40	0.00	1.40	0.00	1.40	0.00	1.40	0.00
1.45	0.00	1.45	0.00	1.45	0.00	1.45	0.00
1.50	0.00	1.50	0.00	1.50	0.00	1.50	0.00
1.55	0.00	1.55	0.00	1.55	0.00	1.55	0.00
1.60	0.00	1.60	0.00	1.60	0.00	1.60	0.00
1.65	0.00	1.65	0.00	1.65	0.00	1.65	0.00
1.70	0.00	1.70	0.00	1.70	0.00	1.70	0.00
1.75	0.00	1.75	0.00	1.75	0.00	1.75	0.00
1.80	0.00	1.80	0.00	1.80	0.00	1.80	0.00
1.85	0.00	1.85	0.00	1.85	0.00	1.85	0.00
1.90	0.00	1.90	0.00	1.90	0.00	1.90	0.00
1.95	0.00	1.95	0.00	1.95	0.00	1.95	0.00
2.00	0.00	2.00	0.00	2.00	0.00	2.00	0.00
2.05	0.00	2.05	0.00	2.05	0.00	2.05	0.00
2.10	0.00	2.10	0.00	2.10	0.00	2.10	0.00
2.15	0.00	2.15	0.00	2.15	0.00	2.15	0.00
2.20	0.00	2.20	0.00	2.20	0.00	2.20	0.00
2.25	0.00	2.25	0.00	2.25	0.00	2.25	0.00
2.30	0.00	2.30	0.00	2.30	0.00	2.30	0.00
2.35	0.00	2.35	0.00	2.35	0.00	2.35	0.00
2.40	0.00	2.40	0.00	2.40	0.00	2.40	0.00
2.45	0.00	2.45	0.00	2.45	0.00	2.45	0.00
2.50	0.00	2.50	0.00	2.50	0.00	2.50	0.00
2.55	0.00	2.55	0.00	2.55	0.00	2.55	0.00
2.60	0.00	2.60	0.00	2.60	0.00	2.60	0.00
2.65	0.00	2.65	0.00	2.65	0.00	2.65	0.00
2.70	0.00	2.70	0.00	2.70	0.00	2.70	0.00
2.75	0.00	2.75	0.00	2.75	0.00	2.75	0.00
2.80	0.00	2.80	0.00	2.80	0.00	2.80	0.00
2.85	0.00	2.85	0.00	2.85	0.00	2.85	0.00
2.90	0.00	2.90	0.00	2.90	0.00	2.90	0.00
2.95	0.00	2.95	0.00	2.95	0.00	2.95	0.00
3.00	0.00	3.00	0.00	3.00	0.00	3.00	0.00

DATE HYD QPEAK (cms) = 6.366
PEAK FLOW (cms) = 2.009 (1)
TIME TO PEAK (hrs) = 2.230
TOTAL RAINFALL (mm) = 42.46
RINF COEFFICIENT = 120.718
RINF COEFFICIENT = .352

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
CALIB (0209) | Area (ha) = 3.63 Curve Number (CN) = 56.3
ID= 1 DT= 5.0 min | Ia (mm) = 1.50 # of Linear Res. (N) = 3.00
U.H. Tp(hrs) = .58
DATE HYD QPEAK (cms) = .239
PEAK FLOW (cms) = .180 (1)
TIME TO PEAK (hrs) = 5.300
TOTAL RAINFALL (mm) = 44.216
TOTAL RAINFALL (mm) = 120.710

RUNOFF COEFFICIENT = .372
(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.05	0.00	0.05	0.00	0.05	0.00	0.05	0.00
0.10	0.00	0.10	0.00	0.10	0.00	0.10	0.00
0.15	0.00	0.15	0.00	0.15	0.00	0.15	0.00
0.20	0.00	0.20	0.00	0.20	0.00	0.20	0.00
0.25	0.00	0.25	0.00	0.25	0.00	0.25	0.00
0.30	0.00	0.30	0.00	0.30	0.00	0.30	0.00
0.35	0.00	0.35	0.00	0.35	0.00	0.35	0.00
0.40	0.00	0.40	0.00	0.40	0.00	0.40	0.00
0.45	0.00	0.45	0.00	0.45	0.00	0.45	0.00
0.50	0.00	0.50	0.00	0.50	0.00	0.50	0.00
0.55	0.00	0.55	0.00	0.55	0.00	0.55	0.00
0.60	0.00	0.60	0.00	0.60	0.00	0.60	0.00
0.65	0.00	0.65	0.00	0.65	0.00	0.65	0.00
0.70	0.00	0.70	0.00	0.70	0.00	0.70	0.00
0.75	0.00	0.75	0.00	0.75	0.00	0.75	0.00
0.80	0.00	0.80	0.00	0.80	0.00	0.80	0.00
0.85	0.00	0.85	0.00	0.85	0.00	0.85	0.00
0.90	0.00	0.90	0.00	0.90	0.00	0.90	0.00
0.95	0.00	0.95	0.00	0.95	0.00	0.95	0.00
1.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00
1.05	0.00	1.05	0.00	1.05	0.00	1.05	0.00
1.10	0.00	1.10	0.00	1.10	0.00	1.10	0.00
1.15	0.00	1.15	0.00	1.15	0.00	1.15	0.00
1.20	0.00	1.20	0.00	1.20	0.00	1.20	0.00
1.25	0.00	1.25	0.00	1.25	0.00	1.25	0.00
1.30	0.00	1.30	0.00	1.30	0.00	1.30	0.00
1.35	0.00	1.35	0.00	1.35	0.00	1.35	0.00
1.40	0.00	1.40	0.00	1.40	0.00	1.40	0.00
1.45	0.00	1.45	0.00	1.45	0.00	1.45	0.00
1.50	0.00	1.50	0.00	1.50	0.00	1.50	0.00
1.55	0.00	1.55	0.00	1.55	0.00	1.55	0.00
1.60	0.00	1.60	0.00	1.60	0.00	1.60	0.00
1.65	0.00	1.65	0.00	1.65	0.00	1.65	0.00
1.70	0.00	1.70	0.00	1.70	0.00	1.70	0.00
1.75	0.00	1.75	0.00	1.75	0.00	1.75	0.00
1.80	0.00	1.80	0.00	1.80	0.00	1.80	0.00
1.85	0.00	1.85	0.00	1.85	0.00	1.85	0.00
1.90	0.00	1.90	0.00	1.90	0.00	1.90	0.00
1.95	0.00	1.95	0.00	1.95	0.00	1.95	0.00
2.00	0.00	2.00	0.00	2.00	0.00	2.00	0.00
2.05	0.00	2.05	0.00	2.05	0.00	2.05	0.00
2.10	0.00	2.10	0.00	2.10	0.00	2.10	0.00
2.15	0.00	2.15	0.00	2.15	0.00	2.15	0.00
2.20	0.00	2.20	0.00	2.20	0.00	2.20	0.00
2.25	0.00	2.25	0.00	2.25	0.00	2.25	0.00
2.30	0.00	2.30	0.00	2.30	0.00	2.30	0.00
2.35	0.00	2.35	0.00	2.35	0.00	2.35	0.00
2.40	0.00	2.40	0.00	2.40	0.00	2.40	0.00
2.45	0.00	2.45	0.00	2.45	0.00	2.45	0.00
2.50	0.00	2.50	0.00	2.50	0.00	2.50	0.00
2.55	0.00	2.55	0.00	2.55	0.00	2.55	0.00
2.60	0.00	2.60	0.00	2.60	0.00	2.60	0.00
2.65	0.00	2.65	0.00	2.65	0.00	2.65	0.00
2.70	0.00	2.70	0.00	2.70	0.00	2.70	0.00
2.75	0.00	2.75	0.00	2.75	0.00	2.75	0.00
2.80	0.00	2.80	0.00	2.80	0.00	2.80	0.00
2.85	0.00	2.85	0.00	2.85	0.00	2.85	0.00
2.90	0.00	2.90	0.00	2.90	0.00	2.90	0.00
2.95	0.00	2.95	0.00	2.95	0.00	2.95	0.00
3.00	0.00	3.00	0.00	3.00	0.00	3.00	0.00

DATE HYD QPEAK (cms) = 17.50
PEAK FLOW (cms) = 4.86
TIME TO PEAK (hrs) = 1.50
TOTAL RAINFALL (mm) = 10.00
RINF COEFFICIENT = 120.718
RINF COEFFICIENT = .379

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
CALIB (0207) | Area (ha) = 4.86
ID= 1 DT= 5.0 min | Total Imp(%) = 17.50
Dif. Com.(%) = 8.80
IMPERVIOUS PERVIOUS (1)
Surface Area (ha) = .85
Dep. Storage (mm) = 1.00
Average Slope (S) = 450.00
Length (m) = .013
Manning's n = .030
Max Eff. Inten. (mm/hr) = 55.52
Storage Coeff. (mm) = 9.81 (11)
Trotl Hyd. Peak (mm) = 10.00
Trotl Hyd. Peak (cms) = .11
PEAK FLOW (cms) = .07
TIME TO PEAK (hrs) = 5.25
TOTAL RAINFALL (mm) = 120.71
RINF COEFFICIENT = .99
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA. *****

(1) ON PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN = 65.5 Ia = Dep. Storage (Above)
(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
(11) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
CALIB (0206) | Area (ha) = 2.65
ID= 1 DT= 5.0 min | Total Imp(%) = 24.50
Dif. Com.(%) = 12.30
IMPERVIOUS PERVIOUS (1)
Surface Area (ha) = .65
Dep. Storage (mm) = 1.00
Average Slope (S) = 340.00
Length (m) = .013
Manning's n = .030
Max Eff. Inten. (mm/hr) = 55.52
Storage Coeff. (mm) = 5.65 (11)
Trotl Hyd. Peak (mm) = 10.00
Trotl Hyd. Peak (cms) = .11

Unit Hyd. Tpeak (min)= 5.00
 Unit Hyd. Peak (cms)= .70
 PEAK FLOW (cms)= .05
 TIME TO PEAK (hrs)= 5.25
 RUNOFF VOLUME (mm)= 319.71
 TOTAL RAINFALL (mm)= 120.71
 RUNOFF COEFFICIENT = .99

Area (ha)= 1.91
 Ia (cms)= 1.50
 U.H. Tp (hrs)= .09
 Area (ha)= .811
 (cms)= .142 (I)
 TIME TO PEAK (hrs)= 5.250
 RUNOFF VOLUME (mm)= 44.327
 TOTAL RAINFALL (mm)= 120.710
 RUNOFF COEFFICIENT = .367

(I) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Area (ha)= 1.54
 Total Imp (S)= 27.00
 Dir. Conn. (S)= 8.80
 IMPERVIOUS PERVIOUS (I)
 Surface Area (ha)= 1.20
 Dep. Storage (mm)= 5.00
 Average Slope (%)= 2.00
 Length (m)= 30.00
 Mannings n = .030

Max. Eff. Inten. (mm/hr)= 55.52
 Storage Coeff. over (min)= 5.00
 Unit Hyd. Tpeak (min)= 1.93 (II)
 Unit Hyd. Peak (cms)= 5.00
 Unit Hyd. Peak (cms)= .31
 PEAK FLOW (cms)= .02
 TIME TO PEAK (hrs)= 4.75
 RUNOFF VOLUME (mm)= 119.71
 TOTAL RAINFALL (mm)= 120.71
 RUNOFF COEFFICIENT = .99

TOTALS
 PEAK FLOW (cms)= 12
 TIME TO PEAK (hrs)= 5.25
 RUNOFF VOLUME (mm)= 54.42
 TOTAL RAINFALL (mm)= 120.71
 RUNOFF COEFFICIENT = .45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(I) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
 CN* = 55.0 Ia = Dep. Storage (Above)
 (II) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (III) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Area (ha)= 1.54
 Total Imp (S)= 27.00
 Dir. Conn. (S)= 8.80
 IMPERVIOUS PERVIOUS (I)
 Surface Area (ha)= 1.20
 Dep. Storage (mm)= 5.00
 Average Slope (%)= 2.00
 Length (m)= 30.00
 Mannings n = .030

Max. Eff. Inten. (mm/hr)= 55.52
 Storage Coeff. over (min)= 5.00
 Unit Hyd. Tpeak (min)= 1.93 (II)
 Unit Hyd. Peak (cms)= 5.00
 Unit Hyd. Peak (cms)= .31
 PEAK FLOW (cms)= .02
 TIME TO PEAK (hrs)= 4.75
 RUNOFF VOLUME (mm)= 119.71
 TOTAL RAINFALL (mm)= 120.71
 RUNOFF COEFFICIENT = .99

TOTALS
 PEAK FLOW (cms)= 12
 TIME TO PEAK (hrs)= 5.25
 RUNOFF VOLUME (mm)= 54.42
 TOTAL RAINFALL (mm)= 120.71
 RUNOFF COEFFICIENT = .45

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(I) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
 CN* = 55.0 Ia = Dep. Storage (Above)
 (II) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (III) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Unit Hyd. Tpeak (min)= 5.00
 Unit Hyd. Peak (cms)= .70
 PEAK FLOW (cms)= .05
 TIME TO PEAK (hrs)= 5.25
 RUNOFF VOLUME (mm)= 319.71
 TOTAL RAINFALL (mm)= 120.71
 RUNOFF COEFFICIENT = .99

Area (ha)= 1.91
 Ia (cms)= 1.50
 U.H. Tp (hrs)= .09
 Area (ha)= .811
 (cms)= .142 (I)
 TIME TO PEAK (hrs)= 5.250
 RUNOFF VOLUME (mm)= 44.327
 TOTAL RAINFALL (mm)= 120.710
 RUNOFF COEFFICIENT = .367

(I) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Area (ha)= 1.54
 Total Imp (S)= 27.00
 Dir. Conn. (S)= 8.80
 IMPERVIOUS PERVIOUS (I)
 Surface Area (ha)= 1.20
 Dep. Storage (mm)= 5.00
 Average Slope (%)= 2.00
 Length (m)= 30.00
 Mannings n = .030

Max. Eff. Inten. (mm/hr)= 55.52
 Storage Coeff. over (min)= 5.00
 Unit Hyd. Tpeak (min)= 1.93 (II)
 Unit Hyd. Peak (cms)= 5.00
 Unit Hyd. Peak (cms)= .31
 PEAK FLOW (cms)= .02
 TIME TO PEAK (hrs)= 4.75
 RUNOFF VOLUME (mm)= 119.71
 TOTAL RAINFALL (mm)= 120.71
 RUNOFF COEFFICIENT = .99

TOTALS
 PEAK FLOW (cms)= 12
 TIME TO PEAK (hrs)= 5.25
 RUNOFF VOLUME (mm)= 54.42
 TOTAL RAINFALL (mm)= 120.71
 RUNOFF COEFFICIENT = .45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(I) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
 CN* = 55.0 Ia = Dep. Storage (Above)
 (II) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (III) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Area (ha)= 1.54
 Total Imp (S)= 27.00
 Dir. Conn. (S)= 8.80
 IMPERVIOUS PERVIOUS (I)
 Surface Area (ha)= 1.20
 Dep. Storage (mm)= 5.00
 Average Slope (%)= 2.00
 Length (m)= 30.00
 Mannings n = .030

Max. Eff. Inten. (mm/hr)= 55.52
 Storage Coeff. over (min)= 5.00
 Unit Hyd. Tpeak (min)= 1.93 (II)
 Unit Hyd. Peak (cms)= 5.00
 Unit Hyd. Peak (cms)= .31
 PEAK FLOW (cms)= .02
 TIME TO PEAK (hrs)= 4.75
 RUNOFF VOLUME (mm)= 119.71
 TOTAL RAINFALL (mm)= 120.71
 RUNOFF COEFFICIENT = .99

TOTALS
 PEAK FLOW (cms)= 12
 TIME TO PEAK (hrs)= 5.25
 RUNOFF VOLUME (mm)= 54.42
 TOTAL RAINFALL (mm)= 120.71
 RUNOFF COEFFICIENT = .45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(I) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:
 CN* = 55.0 Ia = Dep. Storage (Above)
 (II) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (III) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

+ ID# 2 (0203): 1.91 .142 5.25 44.33
 ID = 3 (0003): 3.37 .293 5.25 54.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0009) |
 1 + 2 = 3 |
 AREA OPEAK TPPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID# 1 (0208): 4.31 .226 5.90 45.72
 + ID# 2 (0008): 7.51 .714 5.25 63.71
 ID = 3 (0009): 11.82 .915 5.25 57.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0004) |
 1 + 2 = 3 |
 AREA OPEAK TPPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID# 1 (0205): 5.45 .367 5.33 47.68
 + ID# 2 (0003): 3.37 .293 5.25 54.29
 ID = 3 (0004): 8.82 .657 5.25 50.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0010) |
 1 + 2 = 3 |
 AREA OPEAK TPPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID# 1 (0209): 3.63 .180 5.30 44.92
 + ID# 2 (0009): 11.82 .915 5.25 57.15
 ID = 3 (0010): 15.45 1.071 5.25 54.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (2110) |
 1 + 2 = 3 |
 AREA OPEAK TPPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID# 1 (0010): 15.45 1.071 5.25 54.27
 + ID# 2 (0004): 8.82 .657 5.25 50.21
 ID = 3 (2110): 24.27 1.728 5.25 52.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0211) |
 1 + 2 = 3 |
 AREA OPEAK TPPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID# 1 (0101): 30.00 2.009 5.25 42.45
 + ID# 2 (2110): 24.27 1.728 5.25 52.80
 ID = 3 (0211): 54.27 3.737 5.25 47.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0214) |
 1 + 2 = 3 |
 AREA OPEAK TPPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID# 1 (0211): 54.27 3.737 5.25 47.07
 + ID# 2 (0210): 1.64 .145 5.25 54.42
 ID = 3 (0214): 55.91 3.882 5.25 47.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0215) |
 IN=2-->OUT=1 |
 DT= 5.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
.0000	.0000	.1730	.9050
.0030	.1460	.1840	1.0100
.0200	.2830	.1850	1.0280
.0700	.4200	.1820	1.0450
.1120	.5550	.1820	1.3810
.1510	.7510	.0000	.0000

PEAK FLOW REDUCTION [Oout/Oin] (%) = 5.10
 TIME SHARE OF PEAK FLOW (min) = 250.00
 MAXIMUM STORAGE USED (ha.m.) = 2.1765

FINISH

POND 21 - 6HR STORM

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME		RAIN		TRANSFORMED HYETOGRAPH		TIME		RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
0.05	0.00	0.05	0.00	0.05	0.00	0.05	0.00	0.05	0.00
0.10	0.00	0.10	0.00	0.10	0.00	0.10	0.00	0.10	0.00
0.15	0.00	0.15	0.00	0.15	0.00	0.15	0.00	0.15	0.00
0.20	0.00	0.20	0.00	0.20	0.00	0.20	0.00	0.20	0.00
0.25	0.00	0.25	0.00	0.25	0.00	0.25	0.00	0.25	0.00
0.30	0.00	0.30	0.00	0.30	0.00	0.30	0.00	0.30	0.00
0.35	0.00	0.35	0.00	0.35	0.00	0.35	0.00	0.35	0.00
0.40	0.00	0.40	0.00	0.40	0.00	0.40	0.00	0.40	0.00
0.45	0.00	0.45	0.00	0.45	0.00	0.45	0.00	0.45	0.00
0.50	0.00	0.50	0.00	0.50	0.00	0.50	0.00	0.50	0.00
0.55	0.00	0.55	0.00	0.55	0.00	0.55	0.00	0.55	0.00
0.60	0.00	0.60	0.00	0.60	0.00	0.60	0.00	0.60	0.00
0.65	0.00	0.65	0.00	0.65	0.00	0.65	0.00	0.65	0.00
0.70	0.00	0.70	0.00	0.70	0.00	0.70	0.00	0.70	0.00
0.75	0.00	0.75	0.00	0.75	0.00	0.75	0.00	0.75	0.00
0.80	0.00	0.80	0.00	0.80	0.00	0.80	0.00	0.80	0.00
0.85	0.00	0.85	0.00	0.85	0.00	0.85	0.00	0.85	0.00
0.90	0.00	0.90	0.00	0.90	0.00	0.90	0.00	0.90	0.00
0.95	0.00	0.95	0.00	0.95	0.00	0.95	0.00	0.95	0.00
1.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00	1.00	0.00
1.05	0.00	1.05	0.00	1.05	0.00	1.05	0.00	1.05	0.00
1.10	0.00	1.10	0.00	1.10	0.00	1.10	0.00	1.10	0.00
1.15	0.00	1.15	0.00	1.15	0.00	1.15	0.00	1.15	0.00
1.20	0.00	1.20	0.00	1.20	0.00	1.20	0.00	1.20	0.00
1.25	0.00	1.25	0.00	1.25	0.00	1.25	0.00	1.25	0.00
1.30	0.00	1.30	0.00	1.30	0.00	1.30	0.00	1.30	0.00
1.35	0.00	1.35	0.00	1.35	0.00	1.35	0.00	1.35	0.00
1.40	0.00	1.40	0.00	1.40	0.00	1.40	0.00	1.40	0.00
1.45	0.00	1.45	0.00	1.45	0.00	1.45	0.00	1.45	0.00
1.50	0.00	1.50	0.00	1.50	0.00	1.50	0.00	1.50	0.00
1.55	0.00	1.55	0.00	1.55	0.00	1.55	0.00	1.55	0.00
1.60	0.00	1.60	0.00	1.60	0.00	1.60	0.00	1.60	0.00
1.65	0.00	1.65	0.00	1.65	0.00	1.65	0.00	1.65	0.00
1.70	0.00	1.70	0.00	1.70	0.00	1.70	0.00	1.70	0.00
1.75	0.00	1.75	0.00	1.75	0.00	1.75	0.00	1.75	0.00
1.80	0.00	1.80	0.00	1.80	0.00	1.80	0.00	1.80	0.00
1.85	0.00	1.85	0.00	1.85	0.00	1.85	0.00	1.85	0.00
1.90	0.00	1.90	0.00	1.90	0.00	1.90	0.00	1.90	0.00
1.95	0.00	1.95	0.00	1.95	0.00	1.95	0.00	1.95	0.00
2.00	0.00	2.00	0.00	2.00	0.00	2.00	0.00	2.00	0.00
2.05	0.00	2.05	0.00	2.05	0.00	2.05	0.00	2.05	0.00
2.10	0.00	2.10	0.00	2.10	0.00	2.10	0.00	2.10	0.00
2.15	0.00	2.15	0.00	2.15	0.00	2.15	0.00	2.15	0.00
2.20	0.00	2.20	0.00	2.20	0.00	2.20	0.00	2.20	0.00
2.25	0.00	2.25	0.00	2.25	0.00	2.25	0.00	2.25	0.00
2.30	0.00	2.30	0.00	2.30	0.00	2.30	0.00	2.30	0.00
2.35	0.00	2.35	0.00	2.35	0.00	2.35	0.00	2.35	0.00
2.40	0.00	2.40	0.00	2.40	0.00	2.40	0.00	2.40	0.00
2.45	0.00	2.45	0.00	2.45	0.00	2.45	0.00	2.45	0.00
2.50	0.00	2.50	0.00	2.50	0.00	2.50	0.00	2.50	0.00
2.55	0.00	2.55	0.00	2.55	0.00	2.55	0.00	2.55	0.00
2.60	0.00	2.60	0.00	2.60	0.00	2.60	0.00	2.60	0.00
2.65	0.00	2.65	0.00	2.65	0.00	2.65	0.00	2.65	0.00
2.70	0.00	2.70	0.00	2.70	0.00	2.70	0.00	2.70	0.00
2.75	0.00	2.75	0.00	2.75	0.00	2.75	0.00	2.75	0.00
2.80	0.00	2.80	0.00	2.80	0.00	2.80	0.00	2.80	0.00
2.85	0.00	2.85	0.00	2.85	0.00	2.85	0.00	2.85	0.00
2.90	0.00	2.90	0.00	2.90	0.00	2.90	0.00	2.90	0.00
2.95	0.00	2.95	0.00	2.95	0.00	2.95	0.00	2.95	0.00
3.00	0.00	3.00	0.00	3.00	0.00	3.00	0.00	3.00	0.00

Unit Hyd Qpeak (cms) = 6.366

PEAK FLOW (cms) = 1.001 (1)
 TIME TO PEAK (hrs) = 2.500
 RUNOFF VOLUME (mm) = 12.629
 TOTAL RAINFALL (mm) = 60.355
 RUNOFF COEFFICIENT = .209

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Unit Hyd Qpeak (cms) = .239
 PEAK FLOW (cms) = .072 (1)
 TIME TO PEAK (hrs) = 3.000
 RUNOFF VOLUME (mm) = 13.550
 TOTAL RAINFALL (mm) = 60.355
 RUNOFF COEFFICIENT = .224

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Unit Hyd Qpeak (cms) = .305
 PEAK FLOW (cms) = .091 (1)
 TIME TO PEAK (hrs) = 3.000
 RUNOFF VOLUME (mm) = 13.829
 TOTAL RAINFALL (mm) = 60.355
 RUNOFF COEFFICIENT = .229

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Unit Hyd Qpeak (cms) = .54
 PEAK FLOW (cms) = 1.50 (1)
 TIME TO PEAK (hrs) = 1.50
 RUNOFF VOLUME (mm) = 60.355
 TOTAL RAINFALL (mm) = 60.355
 RUNOFF COEFFICIENT = .254

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Unit Hyd Qpeak (cms) = .18
 PEAK FLOW (cms) = 30.00 (1)
 TIME TO PEAK (hrs) = 1.50
 RUNOFF VOLUME (mm) = 1.50
 TOTAL RAINFALL (mm) = 60.355
 RUNOFF COEFFICIENT = .18

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Input filename: C:\Program Files\Visual OTTHMO 2.3.2\voain.dat
 Output filename: S:\2010 Projects\p-EE (E10)\WR\W10-487 East Gwillimbury SWM Master
 Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas
 Summary filename: S:\2010 Projects\p-EE (E10)\WR\W10-487 East Gwillimbury SWM Master
 Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas

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***** D E T A I L E D O U T P U T *****

DATE: 3/30/2011 TIME: 11:30:01 AM

USER:

COMMENTS:

 ** SIMULATION NUMBER: 1 **

READ STORM | File name: S:\2010 Projects\p-EE (E10)\
 WR\W10-487 East Gwillimbury SWM Master Plan\
 Calcs\Hydrologic Models\Sensitivity Analysis\
 Ptotal= 60.36 mm | Comments: 100yr/6hr

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	1.21	1.75	20.52	3.25	8.45	4.75	1.21
.50	1.21	2.00	20.52	3.50	8.45	5.00	1.21
.75	1.21	2.25	20.52	3.75	8.45	5.25	1.21
1.00	1.21	2.50	20.52	4.00	8.45	5.50	1.21
1.25	1.21	2.75	20.52	4.25	8.45	5.75	1.21
1.50	1.21	3.00	20.52	4.50	8.45	6.00	1.21

Unit Hyd Qpeak (cms) = 54.2
 PEAK FLOW (cms) = 30.00 (1)
 TIME TO PEAK (hrs) = 1.50
 RUNOFF VOLUME (mm) = 1.50
 TOTAL RAINFALL (mm) = 60.355
 RUNOFF COEFFICIENT = .18

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CANLE |
 | STANHYD (0207) | Area (ha) = 4.86
 | ID= 1 DT= 5.0 min | Total Imp(\$)= 17.50 Dfr. Conn. (\$) = 8.80

IMPERVIOUS PERVIOUS (I)
 Surface Area (ha) = .85
 Dep. Storage (mm) = 1.00
 Average Slope (S) = .50
 Length (m) = 450.00
 Manning's n = .013
 Max.Eff.Inten.(mm/hr)= 55.52
 over (min) = 10.00
 Storage Coeff. (min) = 9.81 (11)
 Unit Hyd. Peak (mm) = 10.00
 Unit Hyd. Peak (cms) = .11
 PEAK FLOW (cms) = .06
 TIME TO PEAK (hrs) = 2.50
 RINFOW VOLUME (mm) = 59.35
 TOTAL RAINFALL (mm) = 60.36
 RINFOW COEFFICIENT = .98
 TOTALS
 PEAK FLOW (cms) = .19
 TIME TO PEAK (hrs) = 2.58
 RINFOW VOLUME (mm) = 21.30
 TOTAL RAINFALL (mm) = 60.36
 RINFOW COEFFICIENT = .35

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.
 (I) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.5 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CANLE |
 | STANHYD (0206) | Area (ha) = 2.65
 | ID= 1 DT= 5.0 min | Total Imp(\$)= 24.50 Dfr. Conn. (\$) = 12.30

IMPERVIOUS PERVIOUS (I)
 Surface Area (ha) = .65
 Dep. Storage (mm) = 1.00
 Average Slope (S) = 1.80
 Length (m) = 340.00
 Manning's n = .013
 Max.Eff.Inten.(mm/hr)= 55.52
 over (min) = 5.00
 Storage Coeff. (min) = 5.65 (11)
 Unit Hyd. Peak (mm) = 5.00
 Unit Hyd. Peak (cms) = .20
 PEAK FLOW (cms) = .05
 TIME TO PEAK (hrs) = 2.50
 RINFOW VOLUME (mm) = 59.35
 TOTAL RAINFALL (mm) = 60.36
 RINFOW COEFFICIENT = .98
 TOTALS
 PEAK FLOW (cms) = .11
 TIME TO PEAK (hrs) = 2.58
 RINFOW VOLUME (mm) = 23.39
 TOTAL RAINFALL (mm) = 60.36
 RINFOW COEFFICIENT = .39

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.
 (I) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.5 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CANLE |
 | STANHYD (0205) | Area (ha) = 5.45
 | ID= 1 DT= 5.0 min | Total Imp(\$)= 1.50 Dfr. Conn. (\$) = 56.7

U.H. Tp(hrs) = .31

Unit Hyd Peak (cms) = .671
 PEAK FLOW (cms) = 1.66 (I)
 TIME TO PEAK (hrs) = 2.667
 RINFOW VOLUME (mm) = 14.576
 TOTAL RAINFALL (mm) = 60.355
 RINFOW COEFFICIENT = .242
 (I) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CANLE |
 | STANHYD (0204) | Area (ha) = 1.46
 | ID= 1 DT= 5.0 min | Total Imp(\$)= 27.00 Dfr. Conn. (\$) = 13.50

IMPERVIOUS PERVIOUS (I)
 Surface Area (ha) = .39
 Dep. Storage (mm) = 1.00
 Average Slope (S) = 1.70
 Length (m) = 145.00
 Manning's n = .013
 Max.Eff.Inten.(mm/hr)= 55.52
 over (min) = 10.00
 Storage Coeff. (min) = 3.45 (11)
 Unit Hyd. Peak (mm) = 5.00
 Unit Hyd. Peak (cms) = .26
 PEAK FLOW (cms) = .03
 TIME TO PEAK (hrs) = 2.50
 RINFOW VOLUME (mm) = 59.35
 TOTAL RAINFALL (mm) = 60.36
 RINFOW COEFFICIENT = .98
 TOTALS
 PEAK FLOW (cms) = .07
 TIME TO PEAK (hrs) = 2.58
 RINFOW VOLUME (mm) = 18.60
 TOTAL RAINFALL (mm) = 60.36
 RINFOW COEFFICIENT = .31

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(I) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.5 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CANLE |
 | STANHYD (0203) | Area (ha) = 1.91
 | ID= 1 DT= 5.0 min | Total Imp(\$)= 1.50 Dfr. Conn. (\$) = 57.3

IMPERVIOUS PERVIOUS (I)
 Surface Area (ha) = 1.81
 Dep. Storage (mm) = 1.50
 Average Slope (S) = .09
 Length (m) = .811
 Manning's n = .013
 Max.Eff.Inten.(mm/hr)= 55.52
 over (min) = 5.00
 Storage Coeff. (min) = 5.65 (11)
 Unit Hyd. Peak (mm) = 5.00
 Unit Hyd. Peak (cms) = .20
 PEAK FLOW (cms) = .083 (I)
 TIME TO PEAK (hrs) = 2.500
 RINFOW VOLUME (mm) = 13.433
 TOTAL RAINFALL (mm) = 60.355
 RINFOW COEFFICIENT = .223
 (I) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CANLE |
 | STANHYD (0210) | Area (ha) = 1.64
 | ID= 1 DT= 5.0 min | Total Imp(\$)= 27.00 Dfr. Conn. (\$) = 8.80

IMPERVIOUS PERVIOUS (I)

1 1 + 2 = 3 |
 SURFACE AREA (ha) = 1.70
 DEPTH STORAGE (mm) = 5.00
 AVERAGE SLOPE (%) = 2.00
 LENGTH (m) = 30.00
 MANNINGS N = .030
 Max. Eff. Inten. (mm/hr) = 55.52
 over (min) = 5.00
 Storage Coeff. (min) = 1.93 (11)
 Unit Hyd. Peak (min) = 5.00
 Unit Hyd. Peak (cms) = .23
 PEAK FLOW (cms) = .02
 TIME TO PEAK (hrs) = 2.42
 RUNOFF VOLUME (mm) = 59.35
 TORRAT RAINFALL (mm) = 60.36
 RUNOFF COEFFICIENT = .70

TOTALS
 .094 (11)
 2.50
 18.23
 60.36
 .70

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SELITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 55.0 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0008) |
 1 + 2 = 3 |
 AREA (ha) = 1.70
 QPEAK (cms) = 5.00
 TPEAK (hrs) = 2.42
 R.V. (mm) = 21.30
 ID1= 1 (0207): 4.86 .249 2.58 21.30
 + ID2= 2 (0208): 2.65 .157 2.50 23.39
 ID = 3 (0008): 7.51 .401 2.50 22.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0003) |
 1 + 2 = 3 |
 AREA (ha) = 1.70
 QPEAK (cms) = 5.00
 TPEAK (hrs) = 2.42
 R.V. (mm) = 21.30
 ID1= 1 (0204): 4.46 .089 2.50 24.13
 + ID2= 2 (0203): 1.91 .083 2.50 19.43
 ID = 3 (0003): 3.37 .182 2.50 18.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0009) |
 1 + 2 = 3 |
 AREA (ha) = 1.70
 QPEAK (cms) = 5.00
 TPEAK (hrs) = 2.42
 R.V. (mm) = 21.30
 ID1= 1 (0208): 4.31 .091 3.00 19.83
 + ID2= 2 (0008): 7.51 .401 2.50 22.04
 ID = 3 (0009): 11.82 .457 2.58 19.04

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0004) |
 1 + 2 = 3 |
 AREA (ha) = 1.70
 QPEAK (cms) = 5.00
 TPEAK (hrs) = 2.42
 R.V. (mm) = 21.30
 ID1= 1 (0205): 5.45 .166 2.67 14.58
 + ID2= 2 (0003): 3.37 .182 2.50 18.07
 ID = 3 (0004): 8.82 .326 2.50 15.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0010) |
 1 + 2 = 3 |
 AREA (ha) = 1.70
 QPEAK (cms) = 5.00
 TPEAK (hrs) = 2.42
 R.V. (mm) = 21.30
 ID1= 1 (0209): 3.63 .072 5.00 13.53
 + ID2= 2 (0009): 11.82 .457 2.58 19.04
 ID = 3 (0010): 15.45 .506 2.58 17.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (2110) |
 1 + 2 = 3 |
 AREA (ha) = 1.70
 QPEAK (cms) = 5.00
 TPEAK (hrs) = 2.42
 R.V. (mm) = 21.30
 ID1= 1 (0010): 15.45 .506 2.58 17.75
 + ID2= 2 (0004): 8.82 .326 2.50 15.91
 ID = 3 (2110): 24.27 .820 2.50 17.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0211) |
 1 + 2 = 3 |
 AREA (ha) = 1.70
 QPEAK (cms) = 5.00
 TPEAK (hrs) = 2.42
 R.V. (mm) = 21.30
 ID1= 1 (0101): 30.00 1.001 2.50 12.63
 + ID2= 2 (2110): 24.27 .820 2.50 17.08
 ID = 3 (0211): 54.27 1.821 2.50 14.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0214) |
 1 + 2 = 3 |
 AREA (ha) = 1.70
 QPEAK (cms) = 5.00
 TPEAK (hrs) = 2.42
 R.V. (mm) = 21.30
 ID1= 1 (0211): 54.27 1.821 2.50 14.62
 + ID2= 2 (0210): 1.64 .094 2.50 16.23
 ID = 3 (0214): 55.91 1.915 2.50 14.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0215) |
 IN= 2 -> OUT= 1 |
 DT= 5.0 min |
 STORAGE (ha) = 4.950
 OUTFLOW (cms) = 17.30
 .0000
 .0430
 .0200
 .0710
 .4190
 .1870
 1.0380
 1.2050

INFLOW : ID= 2 (0214)
 OUTFLOW: ID= 1 (0215)

AREA (ha)	ORPEAK (cms)	TRPEAK (hrs)	R.V. (mm)
55.91	1.92	2.50	14.73
55.91	1.14	4.75	13.54

PEAK FLOW REDUCTION [Overt/Out] (%) = 7.17
 TIME SHIF OF PEAK FLOW (min) = 135.00
 MAXIMUM STORAGE USED (ha.m.) = .6826

FINISH

POND 21 - 24HR STORM

```

Y V I I SSSS U U A A L
V V I I SS U U A A L
V V I I SS U U A A L
VV I I SSSS UUUU A A LLLL
OOO TTTT TTTT H H Y Y M M OOO
O O T T T T H H Y Y M M O O
O O T T T T H H Y Y M M O O
OOO T T T T H H Y Y M M OOO
  
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***** D E T A I L E D O U T P U T *****

Input filename: C:\Program Files\Visual C++\VC98\bin\clbcatq.dll
 Output filename: S:\2010 Projects\D-EE (E10)\VRVW0-487 East Gullinbury SWM Master Plan\Calcs\Hydrologic Model\Sensitivity Analysis\Bas
 Summary filename: S:\2010 Projects\D-EE (E10)\VRVW0-487 East Gullinbury SWM Master Plan\Calcs\Hydrologic Model\Sensitivity Analysis\Bas

DATE: 3/30/2011

TIME: 11:43:11 AM

USERS:

COMMENTS:

 ** SIMULATION NUMBER: 1 **

TIME	RAINF	TIME	RAINF	TIME	RAINF	TIME	RAINF
0.00	0.00	6.50	11.13	12.75	4.58	19.00	.65
.25	.30	6.75	11.13	13.00	4.58	19.25	.65
.50	.63	7.00	11.13	13.25	4.58	19.50	.65
.75	.63	7.25	11.13	13.50	4.58	20.00	.65
1.00	.63	7.50	11.13	13.75	4.58	20.25	.65
1.25	.63	7.75	11.13	14.00	4.58	20.50	.65
1.50	.63	8.00	11.13	14.25	4.58	20.75	.65
1.75	.65	8.25	11.13	14.50	2.62	21.00	.65
2.00	.65	8.50	30.11	14.75	2.62	21.25	.65
2.25	.65	8.75	30.11	15.00	2.62	21.50	.65
2.50	.65	9.00	30.11	15.25	2.62	21.75	.65
2.75	.65	9.25	30.11	15.50	2.62	22.00	.65
3.00	.65	9.50	30.11	15.75	2.62	22.25	.65
3.25	.65	9.75	30.11	16.00	2.62	22.50	.65
3.50	.65	10.00	30.11	16.25	2.62	22.50	.65

File name: S:\2010 Projects\D-EE (E10)\
 VRVW0-487 East Gullinbury SWM Master Plan\
 Calcs\Hydrologic Model\Sensitivity Analysis\
 Comments: 100yr/24hr

TIME	RAIN	TRANSFORMED	HYDROGRAPH	TIME	RAIN		
hrs	mm/hr	mm/hr	hrs	hrs	mm/hr		
4.00	.65	10.25	30.11	16.50	1.31	22.75	.65
4.25	.65	10.50	8.51	16.75	1.31	23.00	.65
4.50	3.93	10.75	8.51	17.00	1.31	23.25	.65
4.75	3.93	11.00	8.51	17.25	1.31	23.50	.65
5.00	3.93	11.25	8.51	17.50	1.31	23.75	.65
5.25	3.93	11.50	8.51	17.75	1.31	24.00	.65
5.50	3.93	11.75	8.51	18.00	1.31	24.25	.65
5.75	3.93	12.00	8.51	18.25	1.31		.65
6.00	3.93	12.25	8.51	18.50	.65		.65
6.25	3.93	12.50	4.58	18.75	.65		.65

Area (ha) = 30.00 Curve Number (CN) = 54.2
 Ia (mm) = 1.50 # of Linear Res. (N) = 3.00
 U.H. Tp (hrs) = .18

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TRANSFORMED	HYDROGRAPH	TIME	RAIN		
hrs	mm/hr	mm/hr	hrs	hrs	mm/hr		
.083	.00	6.167	3.93	12.250	8.51	18.33	.65
.167	.00	6.250	3.93	12.333	4.58	18.42	.65
.250	.00	6.333	11.13	12.417	4.58	18.50	.65
.333	.65	6.417	11.13	12.500	4.58	18.58	.65
.417	.65	6.500	11.13	12.583	4.58	18.67	.65
.500	.65	6.583	11.13	12.667	4.58	18.75	.65
.583	.65	6.667	11.13	12.750	4.58	18.83	.65
.667	.65	6.750	11.13	12.833	4.58	18.92	.65
.750	.65	6.833	11.13	12.917	4.58	19.00	.65
.833	.65	6.917	11.13	13.000	4.58	19.08	.65
.917	.65	7.000	11.13	13.083	4.58	19.17	.65
1.000	.65	7.083	11.13	13.167	4.58	19.25	.65
1.083	.65	7.167	11.13	13.250	4.58	19.33	.65
1.167	.65	7.250	11.13	13.333	4.58	19.42	.65
1.250	.65	7.333	11.13	13.417	4.58	19.50	.65
1.333	.65	7.417	11.13	13.500	4.58	19.58	.65
1.417	.65	7.500	11.13	13.583	4.58	19.67	.65
1.500	.65	7.583	11.13	13.667	4.58	19.75	.65
1.583	.65	7.667	11.13	13.750	4.58	19.83	.65
1.667	.65	7.750	11.13	13.833	4.58	19.92	.65
1.750	.65	7.833	11.13	13.917	4.58	20.00	.65
1.833	.65	7.917	11.13	14.000	4.58	20.08	.65
1.917	.65	8.000	11.13	14.083	4.58	20.17	.65
2.000	.65	8.083	11.13	14.167	4.58	20.25	.65
2.083	.65	8.167	11.13	14.250	4.58	20.33	.65
2.167	.65	8.250	11.13	14.333	2.62	20.42	.65
2.250	.65	8.333	11.13	14.417	2.62	20.50	.65
2.333	.65	8.417	30.11	14.500	2.62	20.58	.65
2.417	.65	8.500	30.11	14.583	2.62	20.67	.65
2.500	.65	8.583	30.11	14.667	2.62	20.75	.65
2.583	.65	8.667	30.11	14.750	2.62	20.83	.65
2.667	.65	8.750	30.11	14.833	2.62	20.92	.65
2.750	.65	8.833	30.11	14.917	2.62	21.00	.65
2.833	.65	8.917	30.11	15.000	2.62	21.08	.65
2.917	.65	9.000	30.11	15.083	2.62	21.17	.65
3.000	.65	9.083	30.11	15.167	2.62	21.25	.65
3.083	.65	9.167	30.11	15.250	2.62	21.33	.65
3.167	.65	9.250	30.11	15.333	2.62	21.42	.65
3.250	.65	9.333	30.11	15.417	2.62	21.50	.65
3.333	.65	9.417	30.11	15.500	2.62	21.58	.65
3.417	.65	9.500	30.11	15.583	2.62	21.67	.65
3.500	.65	9.583	30.11	15.667	2.62	21.75	.65
3.583	.65	9.667	30.11	15.750	2.62	21.83	.65
3.667	.65	9.750	30.11	15.833	2.62	21.92	.65
3.750	.65	9.833	30.11	15.917	2.62	22.00	.65
3.833	.65	9.917	30.11	16.000	2.62	22.08	.65

Unit Hyd Qpeak (cms) = 6.366
 PEAK FLOW (cms) = 1.213 (1)
 TIME TO PEAK (hrs) = 10.250
 RUNOFF VOLUME (mm) = 48.519
 TOTAL RAINFALL (mm) = 130.880
 RUNOFF COEFFICIENT = .371
 (1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0209) | Area (ha) = 3.63 Curve Number (CN) = 56.3
 NASHYD | ID= 1 DT= 5.0 mln | Ia (mm) = 1.50 # of Linear Res. (N) = 3.00
 U.H. Tp (hrs) = .58

Unit Hyd Qpeak (cms) = .239
 PEAK FLOW (cms) = .134 (1)
 TIME TO PEAK (hrs) = 10.417
 RUNOFF VOLUME (mm) = 51.261
 TOTAL RAINFALL (mm) = 130.880
 RUNOFF COEFFICIENT = .392
 (1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0208) | Area (ha) = 4.31 Curve Number (CN) = 57.0
 NASHYD | ID= 1 DT= 5.0 mln | Ia (mm) = 1.50 # of Linear Res. (N) = 3.00
 U.H. Tp (hrs) = .54

Unit Hyd Qpeak (cms) = .305
 PEAK FLOW (cms) = .164 (1)
 TIME TO PEAK (hrs) = 10.333
 RUNOFF VOLUME (mm) = 52.146
 TOTAL RAINFALL (mm) = 130.880
 RUNOFF COEFFICIENT = .399
 (1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CATCH (0207) | Area (ha) = 4.86
 STANBYD (0207) | Total Imp(%) = 17.50
 ID= 1 DT= 5.0 min | Dtr. Conn. (%) = 8.80

Surface Area (ha) = .95
 Dep. Storage (mm) = 1.00
 Average Slope (%) = .50
 Length (m) = 450.00
 Manning's n = .013
 Max. Eff. Inten. (mm/hr) = 30.11
 over (mm) = 15.00
 Storage Coeff. (mm) = 12.55 (11)
 Unit Hyd. Peak (mm) = 15.00
 Unit Hyd. Peak (cms) = .07
 PEAK FLOW (cms) = .04
 TIME TO PEAK (hrs) = 10.25
 RUNOFF VOLUME (mm) = 129.88
 TOTAL RAINFALL (mm) = 130.88
 RUNOFF COEFFICIENT = .99
 TOTALS
 266 (111)
 70.29
 130.88
 .54

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.5 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CATCH (0206) | Area (ha) = 2.65
 STANBYD (0206) | Total Imp(%) = 24.50
 ID= 1 DT= 5.0 min | Dtr. Conn. (%) = 12.30

Surface Area (ha) = .65
 Dep. Storage (mm) = 1.00
 Average Slope (%) = 1.80
 Length (m) = 340.00
 Manning's n = .013
 Max. Eff. Inten. (mm/hr) = 30.11
 over (mm) = 5.00
 Storage Coeff. (mm) = 7.21 (11)
 Unit Hyd. Peak (mm) = 5.00
 Unit Hyd. Peak (cms) = .17
 PEAK FLOW (cms) = .03
 TIME TO PEAK (hrs) = 9.42
 RUNOFF VOLUME (mm) = 129.88
 TOTAL RAINFALL (mm) = 130.88
 RUNOFF COEFFICIENT = .99
 TOTALS
 154 (111)
 10.25
 130.88
 .57

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.5 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CATCH (0203) | Area (ha) = 5.45
 STANBYD (0203) | Total Imp(%) = 1.50
 ID= 1 DT= 5.0 min | U.H. Tp(hrs) = .31
 Curve Number (CN) = 58.7
 # of Linear Res. (N) = 3.00

Unit Hyd. Peak (cms) = .671
 PEAK FLOW (cms) = .235 (1)
 TIME TO PEAK (hrs) = 10.250
 RUNOFF VOLUME (mm) = 54.314
 TOTAL RAINFALL (mm) = 130.880
 RUNOFF COEFFICIENT = .415
 (1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CATCH (0204) | Area (ha) = 1.46
 STANBYD (0204) | Total Imp(%) = 27.00
 ID= 1 DT= 5.0 min | Dtr. Conn. (%) = 13.50

Surface Area (ha) = .39
 Dep. Storage (mm) = 1.00
 Average Slope (%) = 1.70
 Length (m) = 145.00
 Manning's n = .013
 Max. Eff. Inten. (mm/hr) = 30.11
 over (mm) = 5.00
 Storage Coeff. (mm) = 4.40 (11)
 Unit Hyd. Peak (mm) = 5.00
 Unit Hyd. Peak (cms) = .23
 PEAK FLOW (cms) = .02
 TIME TO PEAK (hrs) = 9.12
 RUNOFF VOLUME (mm) = 129.88
 TOTAL RAINFALL (mm) = 130.88
 RUNOFF COEFFICIENT = .99
 TOTALS
 10.25
 66.97
 130.88
 .51
 .56

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.5 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CATCH (0210) | Area (ha) = 1.64

Unit Hyd. Peak (cms) = .811
 PEAK FLOW (cms) = .082 (1)
 TIME TO PEAK (hrs) = 10.250
 RUNOFF VOLUME (mm) = 50.547
 TOTAL RAINFALL (mm) = 130.880
 RUNOFF COEFFICIENT = .386
 (1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

DT= 1 DT= 5.0 min | Total Imp.(%)= 27.00 Dlr. Com.(%)= 8.80

IMPERVIOUS		PERVIOUS (i)	
Surface Area (ha)	1.46	1.20	
Dep. Storage (mm)	1.00	5.00	
Average Slope (%)	.50	2.00	
Length (m)	30.00	10.00	
Manning's n	.013	.030	
Max.Eff.Inten.(mm/hr)	30.11	21.32	
over (min)	5.00	10.00	
Storage Coeff. (min)	2.47 (ii)	5.71 (ii)	
Unit Hyd. Peak (min)	5.00	10.00	
Unit Hyd. peak (cms)	.29	.15	
PEAK FLOW (cms)	.01	.07	
TIME TO PEAK (hrs)	8.83	10.25	
RUNOFF VOLUME (mm)	129.88	54.90	
TOTAL RAINFALL (mm)	130.88	130.88	
RUNOFF COEFFICIENT	.99	.42	

**** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 **** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) ON PROCEDURE SELECTED FOR PVIOUS LOSSES:
 CN* = 25.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0207):	4.86	2.66	10.25	70.29
+ ID2= 2 (0208):	2.65	.154	10.25	74.09
ID = 3 (0008):	7.51	.420	10.25	71.63

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0204):	1.46	.087	10.25	75.46
+ ID2= 2 (0203):	1.91	.082	10.25	50.59
ID = 3 (0003):	3.37	.169	10.25	61.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0206):	4.31	.164	10.33	52.15
+ ID2= 2 (0008):	7.51	.420	10.25	71.63
ID = 3 (0009):	11.82	.582	10.25	64.53

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0205):	5.45	.235	10.25	54.31
+ ID2= 2 (0003):	3.37	.169	10.25	61.34
ID = 3 (0004):	8.82	.405	10.25	57.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0209):	3.82	.38	10.25	51.28
+ ID2= 2 (0009):	11.82	.582	10.25	64.53
ID = 3 (0010):	15.45	.713	10.25	61.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0010):	15.45	.713	10.25	61.41
+ ID2= 2 (0004):	8.82	.405	10.25	57.00
ID = 3 (2110):	24.27	1.118	10.25	59.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0101):	30.00	1.213	10.25	48.52
+ ID2= 2 (2110):	24.27	1.118	10.25	59.81
ID = 3 (0211):	54.27	2.331	10.25	53.57

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0211):	54.27	2.331	10.25	53.57
+ ID2= 2 (0210):	1.64	.082	10.25	61.50
ID = 3 (0214):	55.91	2.413	10.25	53.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR	OUTFLOW (cms)	STORAGE (ha-cms)	OUTFLOW (cms)	STORAGE (ha-cms)
1				
2 -> OUT= 1				
DT= 5.0 min				
	.0000	.0000	.1730	.9050

3.083 5.43 | 6.167 11.77 | 9.250 1.81 |

Unit Hyd Qpeak (cms) = 6.366
 PEAK FLOW (cms) = 1.234 (I)
 TIME TO PEAK (hrs) = 5.250
 RUNOFF VOLUME (mm) = 26.029
 TOTAL RAINFALL (mm) = 90.532
 RUNOFF COEFFICIENT = .288

(I) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALLIB (02049) | Area (ha) = 3.63 Curve Number (CN) = 56.3
 NASHVD | Ia (mm) = 1.50 # of Linear Res. (N) = 3.00
 ID= 1 Dp= 5.0 min | U.H. Tp (hrs) = .58

Unit Hyd Qpeak (cms) = .239
 PEAK FLOW (cms) = .110 (I)
 TIME TO PEAK (hrs) = 5.583
 RUNOFF VOLUME (mm) = 27.697
 TOTAL RAINFALL (mm) = 90.532
 RUNOFF COEFFICIENT = .306

(I) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALLIB (02048) | Area (ha) = 4.31 Curve Number (CN) = 57.0
 NASHVD | Ia (mm) = 1.50 # of Linear Res. (N) = 3.00
 ID= 1 Dp= 5.0 min | U.H. Tp (hrs) = .54

Unit Hyd Qpeak (cms) = .305
 PEAK FLOW (cms) = .138 (I)
 TIME TO PEAK (hrs) = 5.500
 RUNOFF VOLUME (mm) = 28.243
 TOTAL RAINFALL (mm) = 90.532
 RUNOFF COEFFICIENT = .312

(I) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALLIB (02047) | Area (ha) = 4.86 Curve Number (CN) = 57.0
 NASHVD | Ia (mm) = 1.50 # of Linear Res. (N) = 3.00
 ID= 1 Dp= 5.0 min | U.H. Tp (hrs) = .58

Unit Hyd Qpeak (cms) = .399
 PEAK FLOW (cms) = .165 (I)
 TIME TO PEAK (hrs) = 5.250
 RUNOFF VOLUME (mm) = 29.746
 TOTAL RAINFALL (mm) = 90.532
 RUNOFF COEFFICIENT = .328

(I) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

MODIFYING PARAMETERS
 Multiplication Factor = .75
 Time Shift (min) = .00

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.00	3.500	15.39	6.750	6.34	10.00	.91	
.500	9.1	3.750	15.39	7.000	6.34	10.25	.91
1.000	9.1	4.000	15.39	7.250	6.34	10.50	.91
1.250	9.1	4.250	15.39	7.500	6.34	10.75	.91
1.500	9.1	4.500	15.39	7.750	6.34	11.00	.91
1.750	9.1	4.750	15.39	8.000	6.34	11.25	.91
2.000	9.1	5.000	15.39	8.250	6.34	11.50	.91
2.250	9.1	5.250	15.39	8.500	6.34	11.75	.91
2.500	9.1	5.500	15.39	8.750	6.34	12.00	.91
2.750	9.1	5.750	15.39	9.000	6.34	12.25	.91
3.000	9.1	6.000	15.39	9.250	6.34	12.50	.91
3.250	9.1	6.250	15.39	9.500	6.34	12.75	.91
3.500	9.1	6.500	15.39	9.750	6.34	13.00	.91

CALLIB (0101) | Area (ha) = 30.00 Curve Number (CN) = 54.2
 NASHVD | Ia (mm) = 1.50 # of Linear Res. (N) = 3.00
 ID= 1 Dp= 5.0 min | U.H. Tp (hrs) = .18

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.00	3.167	5.43	6.250	11.77	9.33	.91	
.167	0.0	3.250	5.43	6.333	6.34	9.42	.91
.333	0.0	3.333	15.39	6.417	6.34	9.50	.91
.500	9.1	3.417	15.39	6.500	6.34	9.58	.91
.667	9.1	3.500	15.39	6.583	6.34	9.67	.91
.833	9.1	3.583	15.39	6.667	6.34	9.75	.91
1.000	9.1	3.667	15.39	6.750	6.34	9.83	.91
1.167	9.1	3.750	15.39	6.833	6.34	9.92	.91
1.333	9.1	3.833	15.39	6.917	6.34	10.00	.91
1.500	9.1	3.917	15.39	7.000	6.34	10.08	.91
1.667	9.1	4.000	15.39	7.083	6.34	10.17	.91
1.833	9.1	4.083	15.39	7.167	6.34	10.25	.91
2.000	9.1	4.167	15.39	7.250	6.34	10.33	.91
2.167	9.1	4.250	15.39	7.333	6.34	10.42	.91
2.333	9.1	4.333	15.39	7.417	6.34	10.50	.91
2.500	9.1	4.417	15.39	7.500	6.34	10.58	.91
2.667	9.1	4.500	15.39	7.583	6.34	10.67	.91
2.833	9.1	4.583	15.39	7.667	6.34	10.75	.91
3.000	9.1	4.667	15.39	7.750	6.34	10.83	.91
3.167	9.1	4.750	15.39	7.833	6.34	10.92	.91
3.333	9.1	4.833	15.39	7.917	6.34	11.00	.91
3.500	9.1	4.917	15.39	8.000	6.34	11.08	.91
3.667	9.1	5.000	15.39	8.083	6.34	11.17	.91
3.833	9.1	5.083	15.39	8.167	6.34	11.25	.91
4.000	9.1	5.167	15.39	8.250	6.34	11.33	.91
4.167	9.1	5.250	15.39	8.333	6.34	11.42	.91
4.333	9.1	5.333	15.39	8.417	6.34	11.50	.91
4.500	9.1	5.417	15.39	8.500	6.34	11.58	.91
4.667	9.1	5.500	15.39	8.583	6.34	11.67	.91
4.833	9.1	5.583	15.39	8.667	6.34	11.75	.91
5.000	9.1	5.667	15.39	8.750	6.34	11.83	.91
5.167	9.1	5.750	15.39	8.833	6.34	11.92	.91
5.333	9.1	5.833	15.39	8.917	6.34	12.00	.91
5.500	9.1	5.917	15.39	9.000	6.34	12.08	.91
5.667	9.1	6.000	15.39	9.083	6.34	12.17	.91
5.833	9.1	6.083	15.39	9.167	6.34	12.25	.91

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SEPARATING THE AREA.

- (1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 65.5 Ia = Dep. Storage (above)
- (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CELLS	Area (ha) =	2.65	Dlr. Conn. (%) =	12.30
STANDHYD (0206)	Total Imp (%) =	24.50		
IMPERVIOUS PERVIOUS (1)				
Surface Area	(ha) =	1.63		
Dep. Storage	(mm) =	1.80		
Average Slope	(%) =	340.00		
Manning n		.013		

Max Eff. Inten. (mm/hr) =	41.64	26.59
Storage Coeff. (mb) =	5.00	10.00
Unit Hyd. Peak (mb) =	6.34 (11)	9.44 (11)
Unit Hyd. Peak (cms) =	5.00	10.00
Unit Hyd. Peak (cms) =	.19	.12
PEAK FLOW (cms) =	.04	.14
TIME TO PEAK (hrs) =	5.25	5.25
RUNOFF VOLUME (mm) =	89.53	36.92
TOTAL RAINFALL (mm) =	90.53	90.53
RUNOFF COEFFICIENT =	.99	.41

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SEPARATING THE AREA.

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 65.5 Ia = Dep. Storage (above)

(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CELLS	Area (ha) =	5.45	Curve Number (CN) =	58.7
STANDHYD (0205)	Total Imp (%) =	1.50	# of Linear Res. (N) =	3.00
IMPERVIOUS PERVIOUS (1)				
Surface Area	(ha) =	1.00		
Dep. Storage	(mm) =	1.00		
Average Slope	(%) =	30.50		
Manning n		.019		

Unit Hyd. Peak (cms) =	.671
PEAK FLOW (cms) =	.227 (1)
TIME TO PEAK (hrs) =	5.33
RUNOFF VOLUME (mm) =	25.53
TOTAL RAINFALL (mm) =	90.53
RUNOFF COEFFICIENT =	.287

CELLS	Area (ha) =	1.46	Dlr. Conn. (%) =	13.50
STANDHYD (0204)	Total Imp (%) =	27.00		
IMPERVIOUS PERVIOUS (1)				
Surface Area	(ha) =	.39		
Dep. Storage	(mm) =	1.00		
Average Slope	(%) =	145.00		

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP! YOU SHOULD CONSIDER SEPARATING THE AREA.

- (1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 65.5 Ia = Dep. Storage (above)
- (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CELLS	Area (ha) =	1.91	Curve Number (CN) =	57.3
STANDHYD (0203)	Total Imp (%) =	1.50	# of Linear Res. (N) =	3.00
IMPERVIOUS PERVIOUS (1)				
Surface Area	(ha) =	1.44		
Dep. Storage	(mm) =	1.00		
Average Slope	(%) =	30.50		
Manning n		.019		

Unit Hyd. Peak (cms) =	.811
PEAK FLOW (cms) =	.089 (1)
TIME TO PEAK (hrs) =	5.280
RUNOFF VOLUME (mm) =	27.406
TOTAL RAINFALL (mm) =	90.53
RUNOFF COEFFICIENT =	.303

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP! YOU SHOULD CONSIDER SEPARATING THE AREA.

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 65.5 Ia = Dep. Storage (above)

(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CELLS	Area (ha) =	1.64	Dlr. Conn. (%) =	8.80
STANDHYD (0210)	Total Imp (%) =	27.00		
IMPERVIOUS PERVIOUS (1)				
Surface Area	(ha) =	.44		
Dep. Storage	(mm) =	1.00		
Average Slope	(%) =	30.50		
Manning n		.019		

Unit Hyd. Peak (cms) =	.31
PEAK FLOW (cms) =	.02
TIME TO PEAK (hrs) =	4.75
RUNOFF VOLUME (mm) =	89.53
TOTAL RAINFALL (mm) =	90.53
RUNOFF COEFFICIENT =	.99

CELLS	Area (ha) =	1.07	Dlr. Conn. (%) =	34.88
STANDHYD (0204)	Total Imp (%) =	27.00		
IMPERVIOUS PERVIOUS (1)				
Surface Area	(ha) =	.39		
Dep. Storage	(mm) =	1.00		
Average Slope	(%) =	145.00		

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP! YOU SHOULD CONSIDER SEPARATING THE AREA.

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN* = 65.5 Ia = Dep. Storage (above)

(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

(111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CV* = 55.0 T₀ = Def. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0008)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0207)	4.86	.288	5.25	40.46
+ ID2= 2 (0206)	2.65	.177	5.25	43.39
ID = 3 (0009)	7.51	.465	5.25	41.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0003)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0204)	1.46	.101	5.25	44.43
+ ID2= 2 (0203)	1.91	.089	5.25	27.41
ID = 3 (0003)	3.37	.190	5.25	34.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0009)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0208)	4.31	.138	5.50	28.24
+ ID2= 2 (0008)	7.51	.465	5.25	41.50
ID = 3 (0009)	11.82	.587	5.25	36.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0004)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0205)	5.46	.183	5.25	29.66
+ ID2= 2 (0003)	3.37	.190	5.25	34.78
ID = 3 (0004)	8.82	.413	5.25	31.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0010)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0209)	3.63	.110	5.58	27.70
+ ID2= 2 (0009)	11.82	.587	5.25	36.66
ID = 3 (0010)	15.45	.661	5.25	34.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (2110)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0010)	15.45	.681	5.25	34.56
+ ID2= 2 (0004)	8.82	.413	5.25	31.58
ID = 3 (2110)	24.27	1.094	5.25	33.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0211)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0101)	30.00	1.234	5.25	26.03
+ ID2= 2 (2110)	24.27	1.094	5.25	33.47
ID = 3 (0211)	54.27	2.328	5.25	29.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0214)	AREA (ha)	QPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0211)	94.27	2.328	5.25	29.36
+ ID2= 2 (0210)	1.64	.091	5.25	34.08
ID = 3 (0214)	95.91	2.420	5.25	29.52

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0215)	INFLOW (cms)	OUTFLOW (cms)	STORAGE (ha.m.)	R.V. (mm)
IN= 2--> OUT= 1				
DT= 5.0 min				
	.0000	.1730	.9050	
	.0030	.1840	1.0100	
	.0200	.1850	1.0380	
	.0710	.1870	1.2050	
	.1120	.1890	1.3810	
	.1310	.1900	.3000	

INFLOW	OUTFLOW	STORAGE	TPEAK (hrs)	R.V. (mm)
ID= 2 (0214)	ID= 1 (0215)			
55.91	2.42	55.91	5.25	29.52
	.19		8.67	28.20

PEAK FLOW REDUCTION (Qout/Qin) (%) = 7.76
 TIME SHIFT OF PEAK FLOW (min) = 205.00
 MAXIMUM STORAGE (ha.m.) = 1.2766

FINISH

ROUND 21 - 425% RAINFALL INTENSITY

Input filename: C:\Program Files\Visual C++\VC98\bin\cl.exe
Output filename: S:\2010 Projects\D-EE (EI0)\WR\W10-487 East Gallinberry SWM Master
Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas
Summary filename: S:\2010 Projects\D-EE (EI0)\WR\W10-487 East Gallinberry SWM Master
Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas

**** D E F A I L T E D O U T P U T ****

DATE: 3/21/2011 TIME: 8:07:37 PM
USERS:
COMMENTS:

***** SIMULATION NUMBERS: 1 ***

Table with columns: TIME, RAIN, Hrs, TIME, RAIN, Hrs, TIME, RAIN, Hrs, TIME, RAIN, Hrs, TIME, RAIN, Hrs. It shows rainfall intensity over time for different simulation numbers.

MODIFYING PARAMETERS
Multiplication Factor = 1.25
Time Shift (min) = .00

Table with columns: TIME, RAIN, Hrs, TIME, RAIN, Hrs, TIME, RAIN, Hrs, TIME, RAIN, Hrs, TIME, RAIN, Hrs. It shows rainfall intensity over time, including a section for 'TRANSFORMED HYETOGRAPH'.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

YOU SHOULD CONSIDER SPLITTING THE AREA.
 (1) ON PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.5 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Unit Hyd Qpeak (cms) = 6.366
 PEAK FLOW (cms) = 2.886 (1)
 TIME TO PEAK (hrs) = 5.230
 RUNOFF VOLUME (mm) = 61.130
 TOTAL RAINFALL (mm) = 150.887
 RUNOFF COEFFICIENT = .405
 (1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB (0209) | Area (ha) = 3.63 Curve Number (CN) = 56.3
 | STANHYD (0206) | ID= 1 DT= 5.0 min | Ia (mm) = 1.50 # of Linear Res. (N) = 3.00
 | | U.H. Tp (hrs) = .59

 Unit Hyd Qpeak (cms) = .239
 PEAK FLOW (cms) = .260 (1)
 TIME TO PEAK (hrs) = 5.500
 RUNOFF VOLUME (mm) = 64.396
 TOTAL RAINFALL (mm) = 150.887
 RUNOFF COEFFICIENT = .427
 (1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB (0208) | Area (ha) = 4.31 Curve Number (CN) = 57.0
 | STANHYD (0206) | ID= 1 DT= 5.0 min | Ia (mm) = 1.50 # of Linear Res. (N) = 3.00
 | | U.H. Tp (hrs) = .54

 Unit Hyd Qpeak (cms) = .305
 PEAK FLOW (cms) = .326 (1)
 TIME TO PEAK (hrs) = 5.500
 RUNOFF VOLUME (mm) = 65.442
 TOTAL RAINFALL (mm) = 150.887
 RUNOFF COEFFICIENT = .434
 (1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

YOU SHOULD CONSIDER SPLITTING THE AREA.
 (1) ON PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.5 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Unit Hyd Qpeak (cms) = 6.366
 PEAK FLOW (cms) = 2.886 (1)
 TIME TO PEAK (hrs) = 5.230
 RUNOFF VOLUME (mm) = 61.130
 TOTAL RAINFALL (mm) = 150.887
 RUNOFF COEFFICIENT = .405
 (1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB (0206) | Area (ha) = 2.65
 | STANHYD (0206) | ID= 1 DT= 5.0 min | Total Imp(%) = 24.50 Dir. Conn.(%) = 12.30

 Surface Area (ha) = .65 IMPERVIOUS PERVIOUS (1)
 Dep. Storage (mm) = 1.00
 Average Slope (ft) = 1.80
 Length (ft) = 340.00
 Mannings n = .013
 Max. Eff. Inten. (mm/hr) = 69.40
 over (min) = 5.00
 Storage Coeff. (min) = 5.17 (11)
 Unit Hyd. Peak (min) = 5.00
 Unit Hyd. peak (cms) = .21
 PEAK FLOW (cms) = .06
 TIME TO PEAK (hrs) = 5.25
 RUNOFF VOLUME (mm) = 149.89
 TOTAL RAINFALL (mm) = 150.89
 RUNOFF COEFFICIENT = .99
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.
 (1) ON PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.5 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

YOU SHOULD CONSIDER SPLITTING THE AREA.
 (1) ON PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.5 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Unit Hyd Qpeak (cms) = 6.366
 PEAK FLOW (cms) = 2.886 (1)
 TIME TO PEAK (hrs) = 5.230
 RUNOFF VOLUME (mm) = 61.130
 TOTAL RAINFALL (mm) = 150.887
 RUNOFF COEFFICIENT = .405
 (1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB (0207) | Area (ha) = 4.86
 | STANHYD (0207) | ID= 1 DT= 5.0 min | Total Imp(%) = 17.50 Dir. Conn.(%) = 8.80

 Surface Area (ha) = 1.85 IMPERVIOUS PERVIOUS (1)
 Dep. Storage (mm) = 1.00
 Average Slope (ft) = .50
 Length (ft) = 450.00
 Mannings n = .013
 Max. Eff. Inten. (mm/hr) = 69.40
 over (min) = 10.00
 Storage Coeff. (min) = 8.97 (11)
 Unit Hyd. Peak (min) = 10.00
 Unit Hyd. peak (cms) = .12
 PEAK FLOW (cms) = .08
 TIME TO PEAK (hrs) = 5.25
 RUNOFF VOLUME (mm) = 86.26
 TOTAL RAINFALL (mm) = 150.89
 RUNOFF COEFFICIENT = .57
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

YOU SHOULD CONSIDER SPLITTING THE AREA.
 (1) ON PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.5 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Unit Hyd Qpeak (cms) = 6.366
 PEAK FLOW (cms) = 2.886 (1)
 TIME TO PEAK (hrs) = 5.230
 RUNOFF VOLUME (mm) = 61.130
 TOTAL RAINFALL (mm) = 150.887
 RUNOFF COEFFICIENT = .405
 (1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 CALIB (0204) | Area (ha) = 1.46
 | STANHYD (0204) | ID= 1 DT= 5.0 min | Total Imp(%) = 27.00 Dir. Conn.(%) = 13.50

 Surface Area (ha) = .29 IMPERVIOUS PERVIOUS (1)
 Dep. Storage (mm) = 1.00
 Average Slope (ft) = 1.90
 Length (ft) = 145.00
 Mannings n = .013
 Max. Eff. Inten. (mm/hr) = 69.40
 over (min) = 5.00
 Storage Coeff. (min) = 5.17 (11)
 Unit Hyd. Peak (min) = 5.00
 Unit Hyd. peak (cms) = .21
 PEAK FLOW (cms) = .06
 TIME TO PEAK (hrs) = 5.25
 RUNOFF VOLUME (mm) = 149.89
 TOTAL RAINFALL (mm) = 150.89
 RUNOFF COEFFICIENT = .99
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

Max Eff. Inten. (mm/hr) = 69.40
 over (min) = 5.00
 Storage Coeff. (mb) = 3.15 (11)
 Unit Hyd. Peak (mb) = 5.00
 Unit Hyd. peak (cms) = .27

PEAK FLOW (cms) = .04
 TIME TO PEAK (hrs) = 5.00
 RINFOW VOLUME (mm) = 149.89
 TOTAL RAINFALL (mm) = 150.89
 RINFOW COEFFICIENT = .99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS PORTION BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) ON PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 65.5 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 TO THE STORAGE COEFFICIENT.
 (11) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB (0203) | Area (ha) = 1.91 | Curve Number (CN) = 57.3
 HASHVD (0203) | Ia (mm) = 1.50 # of Inflow Res. (N) = 3.00
 ID= 1 DT= 5.0 min | U.H. Tp (hrs) = .09

Date Hyd Opreak (cms) = .811
 PEAK FLOW (cms) = .202 (1)
 TIME TO PEAK (hrs) = 5.250
 RINFOW VOLUME (mm) = 63.440
 TOTAL RAINFALL (mm) = 150.887
 RINFOW COEFFICIENT = .420

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB | Area (ha) = 1.64 | Dlr. Conn. (%) = 8.80
 STANDARD (0210) | ID= 1 DT= 5.0 min | Total Imp (%) = 27.00

IMPERVIOUS PERVIOUS (1)
 Surface Area (ha) = .44
 Dep. Storage (mm) = 1.00
 Average Slope (%) = 3.30
 Manning n = .013
 Max. Eff. Inten. (mm/hr) = 69.40
 over (min) = 5.00
 Storage Coeff. (mb) = 1.77 (11)
 Unit Hyd. Peak (mb) = 5.00
 Unit Hyd. peak (cms) = .32

PEAK FLOW (cms) = .03
 TIME TO PEAK (hrs) = 4.67
 RINFOW VOLUME (mm) = 149.89
 TOTAL RAINFALL (mm) = 150.89
 RINFOW COEFFICIENT = .99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS PORTION BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.
 (1) ON PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 53.0 Ia = Dep. Storage (Above)

(11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 TO THE STORAGE COEFFICIENT.
 (11) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0008) | AREA (ha) | QPEAK (cms) | TPPEAK (hrs) | R.V. (mm)
 1 + 2 = 3 | 7.51 | .994 | 5.25 | 87.73
 ID= 1 (0207): 2.65 .365 5.25 90.42
 + ID= 2 (0206): 4.86 .629 5.25 86.26
 ID = 3 (0008): 7.51 .994 5.25 87.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0003) | AREA (ha) | QPEAK (cms) | TPPEAK (hrs) | R.V. (mm)
 1 + 2 = 3 | 1.46 | .206 | 5.25 | 63.41
 ID= 1 (0204): 1.46 .206 5.25 63.41
 + ID= 2 (0203): 1.91 .201 5.25 63.41
 ID = 3 (0003): 3.37 .407 5.25 75.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0009) | AREA (ha) | QPEAK (cms) | TPPEAK (hrs) | R.V. (mm)
 1 + 2 = 3 | 4.31 | .326 | 5.50 | 65.44
 ID= 1 (0208): 4.31 .326 5.50 65.44
 + ID= 2 (0008): 7.51 .994 5.25 87.73
 ID = 3 (0009): 11.82 1.286 5.25 79.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0004) | AREA (ha) | QPEAK (cms) | TPPEAK (hrs) | R.V. (mm)
 1 + 2 = 3 | 5.45 | .524 | 5.33 | 68.00
 ID= 1 (0205): 5.45 .524 5.33 68.00
 + ID= 2 (0003): 3.37 .407 5.25 75.76
 ID = 3 (0004): 8.82 .928 5.25 70.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0010) | AREA (ha) | QPEAK (cms) | TPPEAK (hrs) | R.V. (mm)
 1 + 2 = 3 | 3.63 | .280 | 5.50 | 64.40
 ID= 1 (0209): 3.63 .280 5.50 64.40
 + ID= 2 (0009): 11.82 1.286 5.25 79.60
 ID = 3 (0010): 15.45 1.514 5.25 76.03

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

POND 21 - CH AMC I

V V I SSSS U U A L
 V V I S U U A A L
 V V I S U U A A A L
 V V I SSSS UUUU A A LLLL
 O O TTTT H H Y Y M M O O O
 O O TTTT H H Y Y M M O O O
 O O TTTT H H Y Y M M O O O

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***** D E T A I L E D O U T P U T *****

Input FileNames: C:\Program Files\Visual CTR\PMO 2.3.2\voain.dat
 Output FileNames: S:\2010 Projects\D-EE (E10)\WR\W10-487 East Gwallimbury SWM Master
 Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas
 Summary FileNames: S:\2010 Projects\D-EE (E10)\WR\W10-487 East Gwallimbury SWM Master
 Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas

DATE: 3/21/2011 TIME: 7:59:52 PM
 USER:

COMMENTS:

 ** SIMULATION NUMBER: 1 **

--- READ STORM --- FileNames: S:\2010 Projects\D-EE (E10)\
 WR\W10-487 East Gwallimbury SWM Master Plan\
 Calcs\Hydrologic Models\Existing LBRCA Files\
 --- Pretal=120.71 mm --- Comments: 100yr/12hr

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
.25	1.00	3.50	20.52	6.75	8.45	10.00	1.21
.50	1.21	3.75	20.52	7.00	8.45	10.25	1.21
1.00	1.21	4.00	20.52	7.25	4.83	10.75	1.21
1.25	1.21	4.25	20.52	7.50	4.83	11.00	1.21
1.50	1.21	4.50	20.52	7.75	4.83	11.25	1.21
1.75	1.21	4.75	20.52	8.00	4.83	11.50	1.21
2.00	1.21	5.00	20.52	8.25	4.83	11.75	1.21
2.25	1.21	5.25	20.52	8.50	4.83	12.00	1.21
2.50	7.24	5.75	15.69	9.00	2.41	12.00	1.21
2.75	7.24	6.00	15.69	9.25	2.41	12.25	1.21
3.00	7.24	6.25	15.69	9.50	2.41	12.25	1.21
3.25	7.24	6.50	15.69	9.75	2.41	12.25	1.21

ADD HYD	(2110)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	(mm)
ID1= 1 (0010):	13.45	1.514	5.25	76.03	70.96
+ ID2= 2 (0040):	8.82	.928	5.25	74.19	
ID = 3 (2110):	24.27	2.442	5.25	74.19	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(0211)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	(mm)
ID1= 1 (0101):	30.00	2.886	5.25	61.13	
+ ID2= 2 (2110):	24.27	2.442	5.25	74.19	
ID = 3 (0211):	54.27	5.329	5.25	66.97	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(0214)	AREA	QPEAK	TPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)	(mm)
ID1= 1 (0211):	54.27	5.329	5.25	66.97	
+ ID2= 2 (0210):	1.64	.202	5.25	76.01	
ID = 3 (0214):	55.91	5.530	5.25	67.23	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0215)	OUTFLOW	STORAGE	OUTFLOW	STORAGE	R.V.
ID= 2 > 001-1	(cms)	(ha-ft)	(cms)	(ha-ft)	(mm)
ID= 5.0 min	.0000	.0000	.1730	30.50	
	.0030	.1460	.1840	1.0100	
	.0200	.2630	.1850	1.0380	
	.0710	.4190	.1870	1.2050	
	.1120	.5550	.1890	1.3810	
	.1510	.7510	.0000	.0000	

INFLOW : ID= 2 (0214): 55.91 (cms) 5.53 (hrs) 5.25 (mm) 67.23 (mm)
 OUTFLOW: ID= 1 (0215): 55.91 (cms) .21 (hrs) 9.67 (mm) 65.61 (mm)

PEAK FLOW REDUCTION {Qout/Qin} (%) = 3.79
 TIME SHIFT OF PEAK FLOW (min) = 268.00
 MAXIMUM STORAGE USED (ha-ft) = 3.2137

FINISH


```

Storage Coeff. (min) = 5.00      15.00
Unit Hyd. Peak (min) = 5.00 (ii) 10.42 (iii)
Unit Hyd. Peak (cms) = .20
PEAK FLOW (cms) = .05          1.13 (iii)
TIME TO PEAK (hrs) = 5.25     5.25
RUNOFF VOLUME (mm) = 119.71   36.32
TOTAL RAINFALL (mm) = 120.71 120.71
RUNOFF COEFFICIENT = .99      .39

*TOTALS*
Area (ha) = 1.91      Curve Number (CN) = 37.0
Ia (mm) = 1.50       # of Linear Res. (N) = 3.00
U.H. Tp (hrs) = .09
Unit Hyd Qpeak (cms) = .811
PEAK FLOW (cms) = .080 (i)
TIME TO PEAK (hrs) = 5.250
RUNOFF VOLUME (mm) = 24.786
TOTAL RAINFALL (mm) = 120.710
RUNOFF COEFFICIENT = .205
  
```

(1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
 CN* = 45.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB |
STANDHYD (0210) |
ID= 1 DT= 5.0 min |
Area (ha) = 1.64      Dir. Conn. (%) = 8.80
Total Imp (%) = 27.00
IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = .44
Dep. Storage (mm) = 1.00
Average Slope (%) = .50
Length (m) = 30.00
Manning's n = .013
Max.Eff.Inten. (mm/hr) = 55.52
Storage Coeff. (min) = 5.00
Unit Hyd. Peak (min) = 1.93 (ii)
Unit Hyd. Peak (cms) = 3.00
PEAK FLOW (cms) = .02
TIME TO PEAK (hrs) = 4.75
RUNOFF VOLUME (mm) = 119.71
TOTAL RAINFALL (mm) = 120.71
RUNOFF COEFFICIENT = .99
*TOTALS*
  
```

(1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
 CN* = 35.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB |
STANDHYD (0204) |
ID= 1 DT= 5.0 min |
Area (ha) = 1.46
Total Imp (%) = 27.00 Dir. Conn. (%) = 13.50
IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = .39
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.70
Length (m) = 145.00
Manning's n = .013
Max.Eff.Inten. (mm/hr) = 55.52
Storage Coeff. (min) = 3.00
Unit Hyd. Peak (min) = 5.00 (ii)
Unit Hyd. Peak (cms) = 3.00
PEAK FLOW (cms) = .03
TIME TO PEAK (hrs) = 5.00
RUNOFF VOLUME (mm) = 119.71
TOTAL RAINFALL (mm) = 120.71
RUNOFF COEFFICIENT = .99
*TOTALS*
  
```

(1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
 CN* = 45.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB |
STANDHYD (0203) |
ID= 1 DT= 5.0 min |
Area (ha) = 1.91      Curve Number (CN) = 37.0
Total Imp (%) = 27.00 Dir. Conn. (%) = 8.80
IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = .44
Dep. Storage (mm) = 1.00
Average Slope (%) = .50
Length (m) = 30.00
Manning's n = .013
Max.Eff.Inten. (mm/hr) = 55.52
Storage Coeff. (min) = 5.00
Unit Hyd. Peak (min) = 1.93 (ii)
Unit Hyd. Peak (cms) = 3.00
PEAK FLOW (cms) = .02
TIME TO PEAK (hrs) = 4.75
RUNOFF VOLUME (mm) = 119.71
TOTAL RAINFALL (mm) = 120.71
RUNOFF COEFFICIENT = .99
*TOTALS*
  
```

(1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
 CN* = 45.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

CALIB |
STANDHYD (0203) |
ID= 1 DT= 5.0 min |
Area (ha) = 1.91      Curve Number (CN) = 37.0
Total Imp (%) = 27.00 Dir. Conn. (%) = 8.80
IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = .44
Dep. Storage (mm) = 1.00
Average Slope (%) = .50
Length (m) = 30.00
Manning's n = .013
Max.Eff.Inten. (mm/hr) = 55.52
Storage Coeff. (min) = 5.00
Unit Hyd. Peak (min) = 1.93 (ii)
Unit Hyd. Peak (cms) = 3.00
PEAK FLOW (cms) = .02
TIME TO PEAK (hrs) = 4.75
RUNOFF VOLUME (mm) = 119.71
TOTAL RAINFALL (mm) = 120.71
RUNOFF COEFFICIENT = .99
*TOTALS*
  
```

(1) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:
 CN* = 45.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ID#	AREA (ha)	QPEAK (cms)	TPPEAK (hrs)	R.V. (mm)
ID# 1 (0204):	1.46	.107	5.25	48.04
+ ID# 2 (0208):	1.91	.080	5.25	24.79
ID = 3 (0009):	3.37	.187	5.25	34.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ID#	AREA (ha)	QPEAK (cms)	TPPEAK (hrs)	R.V. (mm)
ID# 1 (0208):	4.31	.124	5.50	25.76
+ ID# 2 (0008):	7.51	.480	5.25	43.89
ID = 3 (0009):	11.82	.588	5.25	37.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ID#	AREA (ha)	QPEAK (cms)	TPPEAK (hrs)	R.V. (mm)
ID# 1 (0205):	5.45	.209	5.33	27.50
+ ID# 2 (0003):	3.37	.187	5.25	34.86
ID = 3 (0004):	8.82	.392	5.25	30.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ID#	AREA (ha)	QPEAK (cms)	TPPEAK (hrs)	R.V. (mm)
ID# 1 (0209):	3.83	.097	5.48	24.90
+ ID# 2 (0009):	11.82	.588	5.25	37.28
ID = 3 (0010):	15.45	.670	5.25	34.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ID#	AREA (ha)	QPEAK (cms)	TPPEAK (hrs)	R.V. (mm)
ID# 1 (0010):	15.45	.670	5.25	34.37
+ ID# 2 (0004):	8.82	.392	5.25	30.32
ID = 3 (0210):	24.27	1.062	5.25	32.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ID#	AREA (ha)	QPEAK (cms)	TPPEAK (hrs)	R.V. (mm)
ID# 1 (0101):	30.00	1.091	5.25	23.14
+ ID# 2 (0210):	24.27	1.062	5.25	32.90
ID = 3 (0211):	54.27	2.153	5.25	27.51

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ID#	AREA (ha)	QPEAK (cms)	TPPEAK (hrs)	R.V. (mm)
ID# 1 (0211):	54.27	2.153	5.25	27.51
+ ID# 2 (0210):	1.64	.094	5.25	35.66
ID = 3 (0214):	55.91	2.247	5.25	27.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

OUTFLOW (cms)	STORAGE (ha-m)	OUTFLOW (cms)	STORAGE (ha-m)
0000	0.000	0000	0.000
0030	1.460	0030	1.010
0200	2.630	0200	1.638
0710	4.190	0710	1.205
1120	5.560	1120	1.381
1510	7.510	1510	1.000

PEAK FLOW REDUCTION [Out/In] (%) = 8.31
 TIME SHIFT OF PEAK FLOW (min) = 205.00
 MAXIMUM STORAGE (ha-m) = 1.1871

FINISH

POND 21 - CN AMC III

V V I SSSSS U U A L
 V V I SS U U A A L
 V V I SS U U A A L
 V V I SS U U A A L
 V V I SSSSS UUUU A A LLLLL

OOO TTTT H H Y Y M M OOO
 O O T T H H Y Y M M O O
 O O T T H H Y Y M M O O
 OOO T T H H Y Y M M OOO

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***** DETAILED OUTPUT *****

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 Output filename: s:\2010 Projects\AD-EE (E10)\WRW10-487 East Gwillimbury SWM Master
 Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas
 Summary filename: s:\2010 Projects\AD-EE (E10)\WRW10-487 East Gwillimbury SWM Master
 Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas

DATE: 3/21/2011 TIME: 7:45:07 PM

USER:

COMMENTS:

 ** SIMULATION NUMBER: 1 ***

File name: s:\2010 Projects\AD-EE (E10)\
 WRW10-487 East Gwillimbury SWM Master Plan\
 Calcs\Hydrologic Models\Existing\ASRCN Files\
 User: 106yr\zshr

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.25	0.00	3.50	20.52	6.75	8.45	10.00	1.21		
0.50	1.21	3.75	20.52	7.00	8.45	10.25	1.21		
0.75	1.00	4.00	20.52	7.25	8.45	10.50	1.21		
1.00	1.21	4.25	20.52	7.50	4.83	10.75	1.21		
1.25	1.21	4.50	55.52	7.75	4.83	11.00	1.21		
1.50	1.21	4.75	55.52	8.00	4.83	11.25	1.21		
1.75	1.21	5.00	55.52	8.25	4.83	11.50	1.21		
2.00	1.21	5.25	55.52	8.50	2.41	11.75	1.21		
2.25	1.21	5.50	15.69	8.75	2.41	12.00	1.21		
2.50	7.24	5.75	15.69	9.00	2.41	12.25	1.21		
2.75	7.24	6.00	15.69	9.25	2.41				
3.00	7.24	6.25	15.69	9.50	1.21				
3.25	7.24	6.50	8.45	9.75	1.21				

Area (ha) = 30.00 Curve Number (CN) = 72.5
 Ia (mm) = 1.50 # of Linear Res. (N) = 3.00
 U.R. Tp (hrs) = 1.18

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYDROGRAPH -----											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.83	0.00	3.167	7.24	6.250	15.69	9.33	1.21				
1.67	0.00	3.250	20.52	6.333	8.45	9.42	1.21				
2.50	1.21	3.333	20.52	6.417	8.45	9.50	1.21				
3.33	1.21	3.417	20.52	6.500	8.45	9.58	1.21				
4.17	1.21	3.500	20.52	6.583	8.45	9.67	1.21				
5.00	1.21	3.583	20.52	6.667	8.45	9.75	1.21				
5.83	1.21	3.667	20.52	6.750	8.45	9.83	1.21				
6.67	1.21	3.750	20.52	6.833	8.45	9.92	1.21				
7.50	1.21	3.833	20.52	6.917	8.45	10.00	1.21				
8.33	1.21	3.917	20.52	7.000	8.45	10.08	1.21				
9.17	1.21	4.000	20.52	7.083	8.45	10.17	1.21				
10.00	1.21	4.083	20.52	7.167	8.45	10.25	1.21				
10.83	1.21	4.167	20.52	7.250	8.45	10.33	1.21				
11.67	1.21	4.250	20.52	7.333	4.83	10.42	1.21				
12.50	1.21	4.333	55.52	7.417	4.83	10.50	1.21				
13.33	1.21	4.417	55.52	7.500	4.83	10.58	1.21				
14.17	1.21	4.500	55.52	7.583	4.83	10.67	1.21				
15.00	1.21	4.583	55.52	7.667	4.83	10.75	1.21				
15.83	1.21	4.667	55.52	7.750	4.83	10.83	1.21				
16.67	1.21	4.750	55.52	7.833	4.83	10.92	1.21				
17.50	1.21	4.833	55.52	7.917	4.83	11.00	1.21				
18.33	1.21	4.917	55.52	8.000	4.83	11.08	1.21				
19.17	1.21	5.000	55.52	8.083	4.83	11.17	1.21				
20.00	1.21	5.083	55.52	8.167	4.83	11.25	1.21				
20.83	1.21	5.167	55.52	8.250	4.83	11.33	1.21				
21.67	1.21	5.250	55.52	8.333	2.41	11.42	1.21				
22.50	1.21	5.333	15.69	8.417	2.41	11.50	1.21				
23.33	7.24	5.417	15.69	8.500	2.41	11.58	1.21				
24.17	7.24	5.500	15.69	8.583	2.41	11.67	1.21				
25.00	7.24	5.583	15.69	8.667	2.41	11.75	1.21				
25.83	7.24	5.667	15.69	8.750	2.41	11.83	1.21				
26.67	7.24	5.750	15.69	8.833	2.41	11.92	1.21				
27.50	7.24	5.833	15.69	8.917	2.41	12.00	1.21				
28.33	7.24	5.917	15.69	9.000	2.41	12.08	1.21				
29.17	7.24	6.000	15.69	9.083	2.41	12.17	1.21				
30.00	7.24	6.083	15.69	9.167	2.41	12.25	1.21				
30.83	7.24	6.167	15.69	9.250	2.41						

Unit Hyd Opeak (cms) = 6.366

PEAK FLOW (cms) = 3.079 (1)
 TIME TO PEAK (hrs) = 5.250
 RUNOFF VOLUME (mm) = 65.739
 TOTAL RAINFALL (mm) = 120.710
 RUNOFF COEFFICIENT = .545

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Area (ha) = 3.63 Curve Number (CN) = 76.5
 Ia (mm) = 1.50 # of Linear Res. (N) = 3.00
 U.R. Tp (hrs) = 1.38

Unit Hyd Opeak (cms) = .239

PEAK FLOW (cms) = .299 (1)

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALLIB (0203) | Area (ha) = 1.91 Curve Number (CN) = 74.0
 RGSHPD (0203) | U.S. Hyd. Res. (hr) = 3.00
 ID = 1 DT = 5.0 min | U.H. Tp (hrs) = .09

Unit Hyd Qpeak (cms) = .811

PEAK FLOW (cms) = .203 (i)
 TIME TO PEAK (hrs) = 5.250
 RUNOFF VOLUME (mm) = 65.600
 TOTAL RAINFALL (mm) = 120.710
 RUNOFF COEFFICIENT = .543

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALLIB (0210) | Area (ha) = 1.64
 RGSHPD (0210) | Total Imp.(%) = 27.00 Dlr. Conn.(%) = 8.80
 ID = 1 DT = 5.0 min |

IMPERVIOUS PERVIOUS (i)

Surface Area (ha) = 1.44
 Dep. Storage (mm) = 1.00
 Average Slope (%) = .50
 Length (m) = 30.00
 Mannings n = .013

Max. Eff. Inten. (mm/hr) = 55.52
 over (min) = 5.00
 Storage Coeff. (min) = 1.93 (ii)
 Unit Hyd. Peak (cms) = 5.00

Unit Hyd. Peak (cms) = .31

PEAK FLOW (cms) = .02
 TIME TO PEAK (hrs) = 4.75
 RUNOFF VOLUME (mm) = 119.71
 TOTAL RAINFALL (mm) = 120.71
 RUNOFF COEFFICIENT = .99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 73.0 Ia = Dep. Storage (Above)
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0008) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1 = 1 (0207): 4.86 .610 5.25 84.42
 + ID2 = 2 (0206): 2.65 .343 5.25 87.00
 ID = 3 (0008): 7.51 .952 5.25 85.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0003) |

1 1 + 2 = 3 | AREA QPEAK TPEAK R.V.
 (ha) (cms) (hrs) (mm)
 ID1 = 1 (0204): 1.46 .194 5.25 87.93
 + ID2 = 2 (0203): 1.91 .203 5.25 85.60
 ID = 3 (0003): 3.37 .397 5.25 75.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0009) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1 = 1 (0205): 4.31 .345 5.50 68.17
 + ID2 = 2 (0008): 7.51 .952 5.25 85.33
 ID = 3 (0009): 11.82 1.269 5.25 79.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0004) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1 = 1 (0205): 5.45 .558 5.25 72.03
 + ID2 = 2 (0003): 3.37 .397 5.25 75.27
 ID = 3 (0004): 8.82 .955 5.25 73.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0010) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1 = 1 (0209): 3.63 .299 5.50 72.05
 + ID2 = 2 (0009): 11.82 1.269 5.25 79.07
 ID = 3 (0010): 15.45 1.538 5.25 77.42

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (2110) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1 = 1 (0010): 13.45 1.538 5.25 77.42
 + ID2 = 2 (0004): 6.62 .955 5.25 73.27
 ID = 3 (2110): 24.27 2.492 5.25 75.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0211) | AREA QPEAK TPEAK R.V.
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)
 ID1 = 1 (0101): 30.00 3.079 5.25 65.74
 + ID2 = 2 (2110): 24.27 2.492 5.25 75.91
 ID = 3 (0211): 54.27 5.571 5.25 70.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	(0214)	AREA	QPEAK	TPPEAK	R.V.
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)
ID1 = 1 (0211)		54.27	5.571	5.25	70.29
+ ID2 = 2 (0210)		1.64	.197	5.25	75.26
ID = 3 (0214)		55.91	5.767	5.25	70.43

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

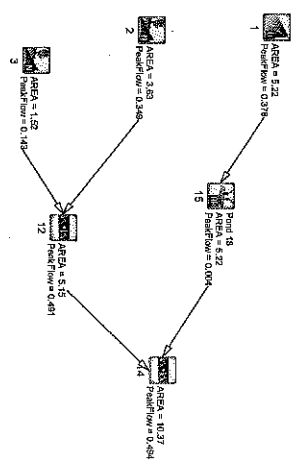
RESERVOIR	(0215)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
ID#		(cms)	(ha.m.)	(cms)	(ha.m.)
ID# = 1		.0000	.0000	.1730	.8050
ID# = 2		.0030	.1460	.1340	1.0100
ID# = 3		.0200	.2630	.1850	1.0380
ID# = 4		.0710	.4190	.1870	1.2050
ID# = 5		.1120	.5550	.1890	1.3810
ID# = 6		.1510	.7510	.0000	.0000

INFLOW	ID#	AREA	QPEAK	TPPEAK	R.V.
INFLOW : ID# 2 (0214)		(ha)	(cms)	(hrs)	(mm)
INFLOW : ID# 1 (0215)		55.91	5.77	5.25	70.43
INFLOW : ID# 1 (0215)		55.91	.21	9.58	60.78

PEAK FLOW REDUCTION [Qout/Qin] (%) = 3.67
 STATE PREFIX OF PEAK FLOW (mm) = 260.00
 MAXIMUM STORAGE (ha.m.) = 3.3906

FINISH

POND 18 - BASE FILE



| RASHVD (0003) | Area (ha) = 1.52 Curve Number (CN) = 80.0
 | IDe 1 DPr 5.0 min | Ia (mm) = 5.00 # of Linear Res. (N) = 3.00
 | | U.H. Tp (hrs) = .05

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME		RAIN		HYETOGRAPH		TIME		RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.00	0.00	3.167	7.24	6.250	15.69	9.33	1.21	1.21	1.21
0.083	1.67	0.00	3.250	7.24	6.333	8.45	9.42	1.21	1.21
0.167	0.00	3.333	20.52	6.417	8.45	9.50	1.21	1.21	1.21
0.250	0.00	3.417	20.52	6.500	8.45	9.58	1.21	1.21	1.21
0.333	1.21	3.500	20.52	6.583	8.45	9.67	1.21	1.21	1.21
0.417	1.21	3.583	20.52	6.667	8.45	9.75	1.21	1.21	1.21
0.500	1.21	3.667	20.52	6.750	8.45	9.83	1.21	1.21	1.21
0.583	1.21	3.750	20.52	6.833	8.45	9.92	1.21	1.21	1.21
0.667	1.21	3.833	20.52	6.917	8.45	10.00	1.21	1.21	1.21
0.750	1.21	3.917	20.52	7.000	8.45	10.08	1.21	1.21	1.21
0.833	1.21	4.000	20.52	7.083	8.45	10.17	1.21	1.21	1.21
0.917	1.21	4.083	20.52	7.167	8.45	10.25	1.21	1.21	1.21
1.000	1.21	4.167	20.52	7.250	8.45	10.33	1.21	1.21	1.21
1.083	1.21	4.250	20.52	7.333	8.45	10.42	1.21	1.21	1.21
1.167	1.21	4.333	20.52	7.417	8.45	10.50	1.21	1.21	1.21
1.250	1.21	4.417	20.52	7.500	8.45	10.58	1.21	1.21	1.21
1.333	1.21	4.500	20.52	7.583	8.45	10.67	1.21	1.21	1.21
1.417	1.21	4.583	20.52	7.667	8.45	10.75	1.21	1.21	1.21
1.500	1.21	4.667	20.52	7.750	8.45	10.83	1.21	1.21	1.21
1.583	1.21	4.750	20.52	7.833	8.45	10.92	1.21	1.21	1.21
1.667	1.21	4.833	20.52	7.917	8.45	11.00	1.21	1.21	1.21
1.750	1.21	4.917	20.52	8.000	8.45	11.08	1.21	1.21	1.21
1.833	1.21	5.000	20.52	8.083	8.45	11.17	1.21	1.21	1.21
1.917	1.21	5.083	20.52	8.167	8.45	11.25	1.21	1.21	1.21
2.000	1.21	5.167	20.52	8.250	8.45	11.33	1.21	1.21	1.21
2.083	1.21	5.250	20.52	8.333	8.45	11.42	1.21	1.21	1.21
2.167	1.21	5.333	20.52	8.417	8.45	11.50	1.21	1.21	1.21
2.250	1.21	5.417	20.52	8.500	8.45	11.58	1.21	1.21	1.21
2.333	1.21	5.500	20.52	8.583	8.45	11.67	1.21	1.21	1.21
2.417	1.21	5.583	20.52	8.667	8.45	11.75	1.21	1.21	1.21
2.500	1.21	5.667	20.52	8.750	8.45	11.83	1.21	1.21	1.21
2.583	1.21	5.750	20.52	8.833	8.45	11.92	1.21	1.21	1.21
2.667	1.21	5.833	20.52	8.917	8.45	12.00	1.21	1.21	1.21
2.750	1.21	5.917	20.52	9.000	8.45	12.08	1.21	1.21	1.21
2.833	1.21	6.000	20.52	9.083	8.45	12.17	1.21	1.21	1.21
2.917	1.21	6.083	20.52	9.167	8.45	12.25	1.21	1.21	1.21
3.000	1.21	6.167	20.52	9.250	8.45	12.33	1.21	1.21	1.21

Unit Hyd Qpeak (cms) = 1.161

PEAK FLOW (cms) = 1.143 (i)
 TIME TO PEAK (hrs) = 5.250
 RUNOFF VOLUME (mm) = 57.002
 RUNOFF COEFFICIENT = .472

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

| CALIB (0002) | Area (ha) = 3.63 Curve Number (CN) = 75.0
 | IDe 1 DPr 5.0 min | Ia (mm) = 5.00 # of Linear Res. (N) = 3.00
 | | U.H. Tp (hrs) = .06

Unit Hyd Qpeak (cms) = 2.311
 PEAK FLOW (cms) = 349 (i)
 TIME TO PEAK (hrs) = 5.250
 RUNOFF VOLUME (mm) = 57.254
 TOTAL RAINFALL (mm) = 120.710

V V I SSSSS U U A A L
 V V I SS U U A A L
 V V I SS U U A A L
 V V I SSSSS UUUU A A LLLL

OOO TTTT H H Y Y M M OOO
 O O T T H H Y Y M M O O
 O O T T H H Y Y M M O O
 OOO T T H H Y Y M M OOO

***** D E T A I L E D O U T P U T *****

Input filename: C:\Program Files\Visual OTTYMO 2.3.2\voind.dat
 Output filename: S:\2010 Projects\A-EE (E10)\WRA10-487 East Gwillingbury SWM Master
 Plan\Cales\Hydrologic Models\Sensitivity Analysis\Bas
 Summary filename: S:\2010 Projects\A-EE (E10)\WRA10-487 East Gwillingbury SWM Master
 Plan\Cales\Hydrologic Models\Sensitivity Analysis\Bas

DATE: 3/30/2011 TIME: 11:20:18 AM

USER:

COMMENTS:

***** SIMULATION NUMBER: 1 **

| READ STORM | Filename: S:\2010 Projects\A-EE (E10)\
 | | WRA10-487 East Gwillingbury SWM Master Plan\
 | | Cales\Hydrologic Models\Existing LSPCA Files\
 | | Ptotal=120.71 mm | Comments: 100yr/12hr

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.00	0.00	3.167	7.24	6.250	15.69	9.33	1.21
0.083	1.67	0.00	3.250	7.24	6.333	8.45	9.42
0.167	0.00	3.333	20.52	6.417	8.45	9.50	1.21
0.250	0.00	3.417	20.52	6.500	8.45	9.58	1.21
0.333	1.21	3.500	20.52	6.583	8.45	9.67	1.21
0.417	1.21	3.583	20.52	6.667	8.45	9.75	1.21
0.500	1.21	3.667	20.52	6.750	8.45	9.83	1.21
0.583	1.21	3.750	20.52	6.833	8.45	9.92	1.21
0.667	1.21	3.833	20.52	6.917	8.45	10.00	1.21
0.750	1.21	3.917	20.52	7.000	8.45	10.08	1.21
0.833	1.21	4.000	20.52	7.083	8.45	10.17	1.21
0.917	1.21	4.083	20.52	7.167	8.45	10.25	1.21
1.000	1.21	4.167	20.52	7.250	8.45	10.33	1.21
1.083	1.21	4.250	20.52	7.333	8.45	10.42	1.21
1.167	1.21	4.333	20.52	7.417	8.45	10.50	1.21
1.250	1.21	4.417	20.52	7.500	8.45	10.58	1.21
1.333	1.21	4.500	20.52	7.583	8.45	10.67	1.21
1.417	1.21	4.583	20.52	7.667	8.45	10.75	1.21
1.500	1.21	4.667	20.52	7.750	8.45	10.83	1.21
1.583	1.21	4.750	20.52	7.833	8.45	10.92	1.21
1.667	1.21	4.833	20.52	7.917	8.45	11.00	1.21
1.750	1.21	4.917	20.52	8.000	8.45	11.08	1.21
1.833	1.21	5.000	20.52	8.083	8.45	11.17	1.21
1.917	1.21	5.083	20.52	8.167	8.45	11.25	1.21
2.000	1.21	5.167	20.52	8.250	8.45	11.33	1.21
2.083	1.21	5.250	20.52	8.333	8.45	11.42	1.21
2.167	1.21	5.333	20.52	8.417	8.45	11.50	1.21
2.250	1.21	5.417	20.52	8.500	8.45	11.58	1.21
2.333	1.21	5.500	20.52	8.583	8.45	11.67	1.21
2.417	1.21	5.583	20.52	8.667	8.45	11.75	1.21
2.500	1.21	5.667	20.52	8.750	8.45	11.83	1.21
2.583	1.21	5.750	20.52	8.833	8.45	11.92	1.21
2.667	1.21	5.833	20.52	8.917	8.45	12.00	1.21
2.750	1.21	5.917	20.52	9.000	8.45	12.08	1.21
2.833	1.21	6.000	20.52	9.083	8.45	12.17	1.21
2.917	1.21	6.083	20.52	9.167	8.45	12.25	1.21
3.000	1.21	6.167	20.52	9.250	8.45	12.33	1.21

| CALIB |

RUNOFF COEFFICIENT = .475

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB	Area (ha) = 5.22	Dia. Cent. (%) = 20.00
STANDHYD (0001)	Total Imp (%) = 20.00	
ID = 1 DF = 5.0 min		

Surface Area (ha) =	1.04	IMPERVIOUS (1)	4.18
Dep. Storage (mm) =	1.00		1.50
Average Slope (%) =	1.00		2.00
Length (m) =	186.50		40.00
Manning's n =	.013		.250
Max. Eff. Inflow (mm/hr) =	55.52		22.86
Storage Coeff. (mm) =	5.20		2.40
Dike Hgt. Peak (mm) =	4.70 (11)		17.45 (11)
Dike Hgt. Peak (cms) =	5.00		20.00
Dike Hgt. Peak (cms) =	.22		.05

PEAK FLOW (cms) =	.16		.22		.378 (11)
TIME TO PEAK (hrs) =	5.25		5.33		5.25
RUNOFF VOLUME (mm) =	115.71		38.08		54.40
TOTAL RAINFALL (mm) =	120.71		120.71		120.71
RUNOFF COEFFICIENT =	.99		.32		.45

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) ON PROCEDURES SELECTED FOR Pervious Losses:
 CN2 = 50.0 Ia = Dep. Storage (Above)
 (11) TIME STEP (DF) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (11) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0012)	AREA	OPEAK	TPPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1 = 1 (0003)	1.52	.143	5.25	57.00
+ ID2 = 2 (0002)	3.63	.349	5.25	57.33
ID = 3 (0012)	5.15	.491	5.25	57.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0015)	OUTFLOW (cms)	STORAGE (ha-ft.)	OUTFLOW (cms)	STORAGE (ha-ft.)
DF = 5.0 min				
	.0000	.0000	.0960	.5250
	.0070	.4250	.1390	.6800
	.0640	.4300	.1920	.6150
	.0730	.4650	.2540	.6510

PEAK FLOW REDUCTION [Qeal/Qun] (%) = 1.18
 TIME SPLIT OF PEAK FLOW (min) = 40.00
 MAXIMUM STORAGE USED (ha-ft.) = .2723

ADD HYD (0014)	AREA	OPEAK	TPPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1 = 1 (0012)	5.15	.491	5.25	57.24
+ ID2 = 2 (0015)	5.22	.004	12.58	33.44
ID = 3 (0014)	10.37	.494	5.25	45.26

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

POND 18 - 24HR STORM

```

POND FLOW (cms) = 1.16
TIME TO PEAK (hrs) = 2.50
ROOF VOLUME (mm) = 59.35
TOTAL RAINFALL (mm) = 60.36
ROOF COEFFICIENT = .98
TOTAL FLOW (cms) = 1.18
TOTAL RAINFALL (mm) = 60.36
ROOF COEFFICIENT = .98

```

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) ON PROCEDURE SELECTED FOR PREVIOUS LOSSES:
 CR = 50.0 Ia = Dep. Storage (Above)
 (11) TIME STEP (TP) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD (0012) | AREA QPEAK TPEAK R.V.
1 + 2 = 3 | (ha) (cms) (hrs) (mm)
ID1= 1 (0003): | 1.42 | 2.37 | 2.50 | 18.78
4 ID2= 2 (0002): | 3.43 | .237 | 2.50 | 18.78
ID = 3 (0012): | 5.15 | .341 | 2.50 | 19.04

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

RESERVOIR (0015) |
ID= 2--> OUT= 1 |
DT= 5.0 min |
OUTFLOW (cms) | STORAGE (ha.m.) | OUTFLOW (cms) | STORAGE (ha.m.)
.0000 | .0000 | .0960 | .9250
.0070 | .4250 | .1390 | .5800
.0640 | .4300 | .1920 | .6150
.0730 | .4650 | .2540 | .6510

```

```

IMPLOV: ID= 1 (0001) | AREA QPEAK TPEAK R.V.
5.22 | .90 | 2.50 | 20.73
OUTFLOW: ID= 1 (0015) | 5.22 | .90 | 2.50 | 12.88

```

PEAK FLOW REDUCTION [out/In] (%) = .77
 TIME SHIFT OF PEAK FLOW (min) = 245.00
 MAXIMUM STORAGE USED (ha.m.) = .1056

```

ADD HYD (0014) | AREA QPEAK TPEAK R.V.
1 + 2 = 3 | (ha) (cms) (hrs) (mm)
ID= 1 (0012): | 5.15 | .341 | 2.50 | 19.04
+ ID= 2 (0015): | 5.22 | .902 | 6.58 | 12.88
ID = 3 (0014): | 10.37 | .342 | 2.50 | 15.94

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

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V V I I SSSSS U U U A A L
V V I I SS U U U A A L
V V I I SS U U U A A L
V V I I SS U U U A A L
V V I I SSSSS UUUUU A A LLLL
OOO TTTT TTTT H H Y Y M M M OOO
O O T T T H H Y Y M M M O O
O O T T T H H Y Y M M M O O
O O T T T H H Y Y M M M O O

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***** D E T A I L E D O U T P U T *****

Input filename: C:\Program Files\Visual Information Systems\SWMM\SWMM2.3\swm2n.dat
 Output filename: S:\2010 Projects\DEE (E10)\WR\W10-487 East Gallinbury SWM Master
 Plan\GIS\Hydrologic Model\100-Year 24-Hour SCS II Design Storm
 Summary filename: S:\2010 Projects\DEE (E10)\WR\W10-487 East Gallinbury SWM Master
 Plan\GIS\Hydrologic Model\Sensitivity Analysis\bas

DATE: 3/30/2011 TIME: 11:44:24 AM

USER: _____
 COMMENTS: _____

 ** SIMULATION NUMBERS: 1 1 **

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
1.00	2.21	7.00	3.54	13.00	15.93	18.00	3.10		
1.25	2.21	7.25	3.54	13.25	15.93	18.25	3.10		
1.50	2.21	7.50	3.54	13.50	15.93	18.50	3.10		
1.75	2.21	7.75	3.54	13.75	15.93	18.75	3.10		
2.00	2.21	8.00	3.54	14.00	15.93	19.00	3.10		
2.25	2.21	8.25	3.54	14.25	15.93	19.25	3.10		
2.50	2.21	8.50	3.54	14.50	15.93	19.50	3.10		
2.75	2.21	8.75	3.54	14.75	15.93	19.75	3.10		
3.00	2.21	9.00	3.54	15.00	15.93	20.00	3.10		
3.25	2.21	9.25	3.54	15.25	15.93	20.25	3.10		
3.50	2.21	9.50	3.54	15.50	15.93	20.50	3.10		
3.75	2.21	9.75	3.54	15.75	15.93	20.75	3.10		

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
4.00	2.21	10.00	7.08	16.00	5.31
4.25	2.65	10.25	30.62	16.25	3.10
4.50	2.65	10.50	10.62	16.50	3.10
4.75	2.65	10.75	10.62	16.75	3.10
5.00	2.65	11.00	10.62	17.00	3.10
5.25	2.65	11.25	42.48	17.25	3.10
5.50	2.65	11.50	42.48	17.50	3.10
5.75	2.65	11.75	116.82	17.75	3.10
6.00	2.65	12.00	116.82	18.00	3.10

Area (ha) = 1.96 Curve Number (CN) = 60.0
 Ia (mm) = 2.50 # of Linear Res. (N) = 4.00
 U.R. Tp (hrs) = .55

NOTE: RAINFALL WAS TRANSFORMED TO 12.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
2.21	6.200	3.54	12.200	15.93	19.20
6.400	2.21	6.400	3.54	12.400	15.93
6.600	2.21	6.600	3.54	12.600	15.93
6.800	2.21	6.800	3.54	12.800	15.93
7.000	2.21	7.000	3.54	13.000	15.93
7.200	2.21	7.200	3.54	13.200	7.08
7.400	2.21	7.400	3.54	13.400	7.08
7.600	2.21	7.600	3.54	13.600	7.08
7.800	2.21	7.800	3.54	13.800	7.08
8.000	2.21	8.000	3.54	14.000	7.08
8.200	2.21	8.200	3.54	14.200	7.08
8.400	2.21	8.400	3.54	14.400	7.08
8.600	2.21	8.600	3.54	14.600	7.08
8.800	2.21	8.800	3.54	14.800	7.08
9.000	2.21	9.000	3.54	15.000	7.08
9.200	2.21	9.200	7.08	15.200	5.31
9.400	2.21	9.400	7.08	15.400	5.31
9.600	2.21	9.600	7.08	15.600	5.31
9.800	2.21	9.800	7.08	15.800	5.31
10.000	2.21	10.000	7.08	16.000	5.31
10.200	2.21	10.200	10.62	16.200	3.10
10.400	2.21	10.400	10.62	16.400	3.10
10.600	2.21	10.600	10.62	16.600	3.10
10.800	2.21	10.800	10.62	16.800	3.10
11.000	2.21	11.000	10.62	17.000	3.10
11.200	2.21	11.200	42.48	17.200	3.10
11.400	2.21	11.400	42.48	17.400	3.10
11.600	2.21	11.600	116.82	17.600	3.10
11.800	2.21	11.800	116.82	17.800	3.10
12.000	2.21	12.000	116.82	18.000	3.10

Unit Hyd Qpeak (cms) = .169
 PEAK FLOW (cms) = .237 (1)
 TIME TO PEAK (hrs) = 12.200
 RUNOFF VOLUME (mm) = 38.690
 TOTAL RAINFALL (mm) = 177.000
 RUNOFF COEFFICIENT = .501
 (1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
2.21	6.167	3.54	12.167	15.93	18.17
6.333	2.21	6.333	3.54	12.333	15.93
6.500	2.21	6.500	3.54	12.500	15.93
6.667	2.21	6.667	3.54	12.667	15.93
6.833	2.21	6.833	3.54	12.833	15.93
7.000	2.21	7.000	3.54	13.000	15.93
7.167	2.21	7.167	3.54	13.167	7.08
7.333	2.21	7.333	3.54	13.333	7.08
7.500	2.21	7.500	3.54	13.500	7.08
7.667	2.21	7.667	3.54	13.667	7.08
7.833	2.21	7.833	3.54	13.833	7.08
8.000	2.21	8.000	3.54	14.000	7.08
8.167	2.21	8.167	3.54	14.167	7.08
8.333	2.21	8.333	3.54	14.333	5.31
8.500	2.21	8.500	3.54	14.500	5.31
8.667	2.21	8.667	3.54	14.667	5.31
8.833	2.21	8.833	3.54	14.833	5.31
9.000	2.21	9.000	3.54	15.000	5.31
9.167	2.21	9.167	7.08	15.167	5.31
9.333	2.21	9.333	7.08	15.333	5.31
9.500	2.21	9.500	7.08	15.500	5.31
9.667	2.21	9.667	7.08	15.667	5.31
9.833	2.21	9.833	7.08	15.833	5.31
10.000	2.21	10.000	7.08	16.000	5.31
10.167	2.21	10.167	10.62	16.167	3.10
10.333	2.21	10.333	10.62	16.333	3.10
10.500	2.21	10.500	10.62	16.500	3.10
10.667	2.21	10.667	10.62	16.667	3.10
10.833	2.21	10.833	10.62	16.833	3.10
11.000	2.21	11.000	10.62	17.000	3.10
11.167	2.21	11.167	42.48	17.167	3.10
11.333	2.21	11.333	42.48	17.333	3.10
11.500	2.21	11.500	42.48	17.500	3.10
11.667	2.21	11.667	116.82	17.667	3.10
11.833	2.21	11.833	116.82	17.833	3.10
12.000	2.21	12.000	116.82	18.000	3.10

Unit Hyd Qpeak (cms) = .529
 PEAK FLOW (cms) = .432 (1)
 TIME TO PEAK (hrs) = 12.000
 RUNOFF VOLUME (mm) = 89.004
 TOTAL RAINFALL (mm) = 177.000
 RUNOFF COEFFICIENT = .503
 (1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Unit Hyd Qpeak (cms) = .169
 PEAK FLOW (cms) = .237 (1)
 TIME TO PEAK (hrs) = 12.200
 RUNOFF VOLUME (mm) = 38.690
 TOTAL RAINFALL (mm) = 177.000
 RUNOFF COEFFICIENT = .501
 (1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Area (ha) = 1.96 Curve Number (CN) = 60.0
 Ia (mm) = 2.50 # of Linear Res. (N) = 4.00
 U.R. Tp (hrs) = .55

Area (ha) = 1.96 Curve Number (CN) = 60.0
 Ia (mm) = 2.50 # of Linear Res. (N) = 4.00
 U.R. Tp (hrs) = .55

Area (ha) = 3.74
 Total Imp(%) = 1.00 Dir. Comb.(%) = 1.00
 INFERVIOUS FERVIOUS (1)
 Surface Area (ha) = .04
 Dep. Storage (mm) = 2.50
 Average Slope (ft) = 1.00
 Length (m) = 157.00
 Manning's n = .200
 Max.Eff.Inten.(mm/hr) = 116.82
 Area (mm) = 13.43
 Storage Coeff. (mm) = 23.22 (11)
 Unit Hyd. Tpeak (min) = 10.00 30.00

Ditch Hyd. peak (cms) = .16
 PEAK FLOW (cms) = .01
 TIME TO PEAK (hrs) = 12.00
 RINOFF VOLUME (mm) = 176.00
 TOTAL RAINFALL (mm) = 177.00
 RINOFF COEFFICIENT = .99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 60.0 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

AREA	Area (ha)	Imperv. (%)	Del. Conn. (%)
SPRNGD (0101)	2.60	25.00	12.00
DT= 1 DT=10.0 min			
Surface Area (ha)	1.95		
Dep. Storage (mm)	2.50		
Average Slope (%)	1.50		
Length (m)	131.00		
Manning's n	.200		
Max. Eff. Inten. (mm/hr)	116.82		
Storage Coeff. (min)	20.00		
Date Hyd. Peak (min)	15.08 (11)		
Date Hyd. Peak (cms)	20.00		
PEAK FLOW (cms)	.07		
TIME TO PEAK (hrs)	.29		
RINOFF VOLUME (mm)	176.00		
TOTAL RAINFALL (mm)	177.00		
RINOFF COEFFICIENT	.99		

***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.

(1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
 CN* = 60.0 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	AREA	QPEAK	TPPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0103):	3.74	.473	12.17	89.43
+ ID2= 2 (0101):	2.60	.363	12.17	105.42
ID = 3 (0105):	6.34	.836	12.17	95.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD	AREA	QPEAK	TPPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0107):				
ID = 2 (0109):				

ID	QPEAK	TPPEAK	R.V.
ID1= 1 (0103):	2.23	.432	12.00
+ ID2= 2 (0109):	6.34	.836	12.17
ID = 3 (0107):	8.57	1.196	12.00
ID = 3 (0109):	10.53	1.387	12.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0108)	AREA	QPEAK	TPPEAK	R.V.
ID= 2--> OUT= 1	(ha)	(cms)	(hrs)	(mm)
DT= 5.0 min				
OUTFLOW (cms)	1.96	.237	12.20	89.69
STORAGE (ha.m.)	8.57	1.196	12.00	94.17
PEAK FLOW (cms)	.0070	.0015	.4100	.1756

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

IMPFLOW : ID= 2 (0109)	AREA	QPEAK	TPPEAK	R.V.
OUTFLOW : ID= 1 (0108)	(ha)	(cms)	(hrs)	(mm)
PEAK FLOW REDUCTION [Qout/Qin] (%)				
TIME SLIP OF PEAK FLOW (min)				
MAXIMUM STORAGE USED (mm)				
				53.96
				30.00
				2771

FINISH

POND 18 - 25% RAINFALL INTENSITY

V V I SSSS U U A L
 V V I SS U U A A L
 V V I SS U U Appaa L
 V V I SS U U A A L
 VV I SSSS UUUUU A A LLLLL
 OOO TTTT H H Y Y M M OOO
 O O T T H H Y Y M M O O
 O O T T H H Y Y M M O O
 OOO T T H H Y Y M M OOO

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***** D E T A I L E D O U T P U T *****

Input filename: C:\Program Files\Visual CTRP90 2.3.2\voln.dat
 Output filename: S:\2010 Projects\AD-EE (E10)\WRW10-487 East Gwillingbury SWM Master
 Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas
 Summary filename: S:\2010 Projects\AD-EE (E10)\WRW10-487 East Gwillingbury SWM Master
 Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas

DATE: 3/30/2011 TIME: 11:53:37 AM

USER:

COMMENTS:

 ** SIMULATION NUMBER: 1 ***

--- HEAD STORMS ---
 File name: S:\2010 Projects\AD-EE (E10)\
 WRW10-487 East Gwillingbury SWM Master Plan\
 Calcs\Hydrologic Models\Existing LSRCA Files\
 Ptotal=120.71 mm Comments: 100yr/12hr

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
1.00	1.21	3.75	20.52	7.00	8.45	10.25	1.21
1.25	1.21	4.00	20.52	7.25	8.45	10.50	1.21
1.50	1.21	4.25	20.52	7.50	8.45	10.75	1.21
1.75	1.21	4.50	20.52	7.75	8.45	11.00	1.21
2.00	1.21	4.75	20.52	8.00	8.45	11.25	1.21
2.25	1.21	5.00	20.52	8.25	8.45	11.50	1.21
2.50	1.21	5.25	20.52	8.50	8.45	11.75	1.21
2.75	1.21	5.50	20.52	8.75	8.45	12.00	1.21
3.00	1.21	5.75	20.52	9.00	8.45	12.25	1.21
3.25	1.21	6.00	20.52	9.25	8.45	12.50	1.21
3.50	1.21	6.25	20.52	9.50	8.45	12.75	1.21
3.75	1.21	6.50	20.52	9.75	8.45	13.00	1.21
4.00	1.21	6.75	20.52	10.00	8.45	13.25	1.21
4.25	1.21	7.00	20.52	10.25	8.45	13.50	1.21
4.50	1.21	7.25	20.52	10.50	8.45	13.75	1.21
4.75	1.21	7.50	20.52	10.75	8.45	14.00	1.21
5.00	1.21	7.75	20.52	11.00	8.45	14.25	1.21
5.25	1.21	8.00	20.52	11.25	8.45	14.50	1.21
5.50	1.21	8.25	20.52	11.50	8.45	14.75	1.21
5.75	1.21	8.50	20.52	11.75	8.45	15.00	1.21
6.00	1.21	8.75	20.52	12.00	8.45	15.25	1.21
6.25	1.21	9.00	20.52	12.25	8.45	15.50	1.21
6.50	1.21	9.25	20.52	12.50	8.45	15.75	1.21
6.75	1.21	9.50	20.52	12.75	8.45	16.00	1.21
7.00	1.21	9.75	20.52	13.00	8.45	16.25	1.21
7.25	1.21	10.00	20.52	13.25	8.45	16.50	1.21
7.50	1.21	10.25	20.52	13.50	8.45	16.75	1.21
7.75	1.21	10.50	20.52	13.75	8.45	17.00	1.21
8.00	1.21	10.75	20.52	14.00	8.45	17.25	1.21
8.25	1.21	11.00	20.52	14.25	8.45	17.50	1.21
8.50	1.21	11.25	20.52	14.50	8.45	17.75	1.21
8.75	1.21	11.50	20.52	14.75	8.45	18.00	1.21
9.00	1.21	11.75	20.52	15.00	8.45	18.25	1.21
9.25	1.21	12.00	20.52	15.25	8.45	18.50	1.21
9.50	1.21	12.25	20.52	15.50	8.45	18.75	1.21
9.75	1.21	12.50	20.52	15.75	8.45	19.00	1.21
10.00	1.21	12.75	20.52	16.00	8.45	19.25	1.21
10.25	1.21	13.00	20.52	16.25	8.45	19.50	1.21
10.50	1.21	13.25	20.52	16.50	8.45	19.75	1.21
10.75	1.21	13.50	20.52	16.75	8.45	20.00	1.21
11.00	1.21	13.75	20.52	17.00	8.45	20.25	1.21
11.25	1.21	14.00	20.52	17.25	8.45	20.50	1.21
11.50	1.21	14.25	20.52	17.50	8.45	20.75	1.21
11.75	1.21	14.50	20.52	17.75	8.45	21.00	1.21
12.00	1.21	14.75	20.52	18.00	8.45	21.25	1.21
12.25	1.21	15.00	20.52	18.25	8.45	21.50	1.21
12.50	1.21	15.25	20.52	18.50	8.45	21.75	1.21
12.75	1.21	15.50	20.52	18.75	8.45	22.00	1.21
13.00	1.21	15.75	20.52	19.00	8.45	22.25	1.21
13.25	1.21	16.00	20.52	19.25	8.45	22.50	1.21
13.50	1.21	16.25	20.52	19.50	8.45	22.75	1.21
13.75	1.21	16.50	20.52	19.75	8.45	23.00	1.21
14.00	1.21	16.75	20.52	20.00	8.45	23.25	1.21
14.25	1.21	17.00	20.52	20.25	8.45	23.50	1.21
14.50	1.21	17.25	20.52	20.50	8.45	23.75	1.21
14.75	1.21	17.50	20.52	20.75	8.45	24.00	1.21
15.00	1.21	17.75	20.52	21.00	8.45	24.25	1.21
15.25	1.21	18.00	20.52	21.25	8.45	24.50	1.21
15.50	1.21	18.25	20.52	21.50	8.45	24.75	1.21
15.75	1.21	18.50	20.52	21.75	8.45	25.00	1.21
16.00	1.21	18.75	20.52	22.00	8.45	25.25	1.21
16.25	1.21	19.00	20.52	22.25	8.45	25.50	1.21
16.50	1.21	19.25	20.52	22.50	8.45	25.75	1.21
16.75	1.21	19.50	20.52	22.75	8.45	26.00	1.21
17.00	1.21	19.75	20.52	23.00	8.45	26.25	1.21
17.25	1.21	20.00	20.52	23.25	8.45	26.50	1.21
17.50	1.21	20.25	20.52	23.50	8.45	26.75	1.21
17.75	1.21	20.50	20.52	23.75	8.45	27.00	1.21
18.00	1.21	20.75	20.52	24.00	8.45	27.25	1.21
18.25	1.21	21.00	20.52	24.25	8.45	27.50	1.21
18.50	1.21	21.25	20.52	24.50	8.45	27.75	1.21
18.75	1.21	21.50	20.52	24.75	8.45	28.00	1.21
19.00	1.21	21.75	20.52	25.00	8.45	28.25	1.21
19.25	1.21	22.00	20.52	25.25	8.45	28.50	1.21
19.50	1.21	22.25	20.52	25.50	8.45	28.75	1.21
19.75	1.21	22.50	20.52	25.75	8.45	29.00	1.21
20.00	1.21	22.75	20.52	26.00	8.45	29.25	1.21
20.25	1.21	23.00	20.52	26.25	8.45	29.50	1.21
20.50	1.21	23.25	20.52	26.50	8.45	29.75	1.21
20.75	1.21	23.50	20.52	26.75	8.45	30.00	1.21
21.00	1.21	23.75	20.52	27.00	8.45	30.25	1.21
21.25	1.21	24.00	20.52	27.25	8.45	30.50	1.21
21.50	1.21	24.25	20.52	27.50	8.45	30.75	1.21
21.75	1.21	24.50	20.52	27.75	8.45	31.00	1.21
22.00	1.21	24.75	20.52	28.00	8.45	31.25	1.21
22.25	1.21	25.00	20.52	28.25	8.45	31.50	1.21
22.50	1.21	25.25	20.52	28.50	8.45	31.75	1.21
22.75	1.21	25.50	20.52	28.75	8.45	32.00	1.21
23.00	1.21	25.75	20.52	29.00	8.45	32.25	1.21
23.25	1.21	26.00	20.52	29.25	8.45	32.50	1.21
23.50	1.21	26.25	20.52	29.50	8.45	32.75	1.21
23.75	1.21	26.50	20.52	29.75	8.45	33.00	1.21
24.00	1.21	26.75	20.52	30.00	8.45	33.25	1.21
24.25	1.21	27.00	20.52	30.25	8.45	33.50	1.21
24.50	1.21	27.25	20.52	30.50	8.45	33.75	1.21
24.75	1.21	27.50	20.52	30.75	8.45	34.00	1.21
25.00	1.21	27.75	20.52	31.00	8.45	34.25	1.21
25.25	1.21	28.00	20.52	31.25	8.45	34.50	1.21
25.50	1.21	28.25	20.52	31.50	8.45	34.75	1.21
25.75	1.21	28.50	20.52	31.75	8.45	35.00	1.21
26.00	1.21	28.75	20.52	32.00	8.45	35.25	1.21
26.25	1.21	29.00	20.52	32.25	8.45	35.50	1.21
26.50	1.21	29.25	20.52	32.50	8.45	35.75	1.21
26.75	1.21	29.50	20.52	32.75	8.45	36.00	1.21
27.00	1.21	29.75	20.52	33.00	8.45	36.25	1.21
27.25	1.21	30.00	20.52	33.25	8.45	36.50	1.21
27.50	1.21	30.25	20.52	33.50	8.45	36.75	1.21
27.75	1.21	30.50	20.52	33.75	8.45	37.00	1.21
28.00	1.21	30.75	20.52	34.00	8.45	37.25	1.21
28.25	1.21	31.00	20.52	34.25	8.45	37.50	1.21
28.50	1.21	31.25	20.52	34.50	8.45	37.75	1.21
28.75	1.21	31.50	20.52	34.75	8.45	38.00	1.21
29.00	1.21	31.75	20.52	35.00	8.45	38.25	1.21
29.25	1.21	32.00	20.52	35.25	8.45	38.50	1.21
29.50	1.21	32.25	20.52	35.50	8.45	38.75	1.21
29.75	1.21	32.50	20.52	35.75	8.45	39.00	1.21
30.00	1.21	32.75	20.52	36.00	8.45	39.25	1.21
30.25	1.21	33.00	20.52	36.25	8.45	39.50	1.21
30.50	1.21	33.25	20.52	36.50	8.45	39.75	1.21
30.75	1.21	33.50	20.52	36.75	8.45	40.00	1.21
31.00	1.21	33.75	20.52	37.00	8.45	40.25	1.21
31.25	1.21	34.00	20.52	37.25	8.45	40.50	1.21
31.50	1.21	34.25	20.52	37.50	8.45	40.75	1.21
31.75	1.21	34.50	20.52	37.75	8.45	41.00	1.21
32.00	1.21	34.75	20.52	38.00	8.45	41.25	1.21
32.25	1.21	35.00	20.52	38.25	8.45	41.50	1.21
32.50	1.21	35.25	20.52	38.50	8.45	41.75	1.21
32.75	1.21	35.50	20.52	38.75	8.45	42.00	1.21
33.00	1.21	35.75	20.52	39.00	8.45	42.25	1.21
33.25	1.21	36.00	20.52	39.25	8.45	42.50	1.21
33.50	1.21	36.25	20.52	39.50	8.45	42.75	1.21
33.75	1.21	36.50	20.52	39.75	8.45	43.00	1.21
34.00	1.21	36.75	20.52	40.00	8.45	43.25	1.21
34.25	1.21	37.00	20.52	40.25	8.45	43.50	1.21
34.50	1.21	37.25	20.52	40.50	8.45	43.75	1.21
34.75	1.21	37.50	20.52	40.75	8.45	44.00	1.21
35.00	1.21	37.75	20.52	41.00	8.45	44.25	1.21
35.25	1.21	38.00	20.52	41.25	8.45	44.50	1.21
35.50	1.21	38.25	20.52	41.50	8.45	44.75	1.21
35.75	1.21	38.					

3.083 5.43 | 6.167 11.77 | 9.250 1.81 |

Utlc Hyd Qpeak (cms) = 1.161
 PEAK FLOW (cms) = .097 (L)
 TIME TO PEAK (hrs) = 5.250
 RINOFF VOLUME (mm) = 37.453
 TOTAL RAINFALL (mm) = 90.532
 RINOFF COEFFICIENT = .414

{1} PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALLIB (0002) | Area (ha) = 3.63 Curve Number (CN) = 75.0
 NASHVD (0001) | ID= 1 DT= 5.0 min | Ia (mm) = 5.00 # of Linear Res. (N) = 3.00
 U.H. Tp(hrs) = .06

Utlc Hyd Qpeak (cms) = 2.311
 PEAK FLOW (cms) = .233 (L)
 TIME TO PEAK (hrs) = 5.250
 RINOFF VOLUME (mm) = 36.883
 TOTAL RAINFALL (mm) = 90.532
 RINOFF COEFFICIENT = .407

{1} PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALLIB (0001) | Area (ha) = 5.22
 SYNVD (0001) | ID= 1 DT= 5.0 min | Total Imp(%) = 20.00
 Dlr. Conn. (8) = 20.00

IMPERVIOUS PERCENTS (L)
 Surface Area (ha) = 1.04
 Dep. Storage (mm) = 1.00
 Average Slope (%) = 1.00
 Permeability n = .013
 Max. Eff. Inten. (mm/hr) = 41.64
 Storage Coeff. (mb) = 5.00
 Utlc Hyd. peak (mb) = 5.27 (L)
 Utlc Hyd. peak (cms) = 5.00
 Utlc Hyd. peak (cms) = 2.21

PEAK FLOW (cms) = .12
 TIME TO PEAK (hrs) = 5.25
 RINOFF VOLUME (mm) = 89.53
 TOTAL RAINFALL (mm) = 90.53
 RINOFF COEFFICIENT = .99
 TOTALS
 .240 (L)
 5.25
 36.39
 90.53
 .40

***WARNING FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20% YOU SHOULD CONSIDER SPLITTING THE AREA.

- (1) CN PROCEDURE SELECTED FOR PERVIOUS LOSSSES: CN = 50.0 In Dep. Storage (mb)
- (L) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (L) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0012)	AREA (ha)	QPEAK (cms)	TPPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0003):	3.63	.097	5.25	37.45
+ ID2= 2 (0002):		.231		36.88

ID = 3 (0012): 5.15 .328 5.25 37.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0015)	OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
IN= 2--> OUF= 1				
DT= 5.0 min				
	.0000	.0000	.0960	.5250
	.0070	.4250	.1390	.8800
	.0640	.4300	.1920	.6150
	.0730	.4650	.2540	.6510

INFLOW : ID= 2 (0001)
 OUTFLOW: ID= 1 (0015)
 PEAK FLOW REDUCTION (Ouf/Cln) (%) = 1.25
 TIME STEP OR PEAK FLOW (min) = 445.00
 MAXIMUM STORAGE (ha.m.) = .1920

ADD HYD (0014)	AREA (ha)	QPEAK (cms)	TPPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 (0012):	5.15	.328	5.25	37.05
+ ID2= 2 (0015):		.003	12.67	22.37
ID = 3 (0014):	10.37	.329	5.25	29.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

POND 18 - +25% RAINFALL INTENSITY

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V V I SSSSS U U A L L
V V I SS U U A A L L
V V I SS U U A A A A L L
V V I SS U U A A L L
V V I SSSSS UUUU A A LLLLL
OOO TTTT H H Y Y M M OOO
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
OOO T T H H Y Y M M OOO

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***** D E T A I L E D O U T P U T *****

Input filename: C:\Program Files\Visual QTRWMO 2.3.2\voind.dat
 Output filename: S:\2010 Projects\DE (E10)\WR\W10-487 East Gwillimbury SWM Master
 Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas
 Summary filename: S:\2010 Projects\DE (E10)\WR\W10-487 East Gwillimbury SWM Master
 Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas

DATE: 3/21/2011 TIME: 9:09:35 PM

USER:

COMMENTS:

 ** SIMULATION NUMBER: 1 **

FILENAME: S:\2010 Projects\DE (E10)\
 WR\W10-487 East Gwillimbury SWM Master Plan\
 Calcs\Hydrologic Models\Existing USCA Files\
 P\ Pond=120.71 mm | Comments: 100yr/12hr

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.25	0.00	3.75	20.52	6.75	8.45
0.50	1.21	4.00	20.52	7.00	8.45
0.75	1.21	4.25	20.52	7.25	8.45
1.00	1.21	4.50	35.52	7.50	4.83
1.25	1.21	4.75	55.52	7.75	4.83
1.50	1.21	5.00	55.52	8.00	4.83
1.75	1.21	5.25	55.52	8.25	4.83
2.00	1.21	5.50	55.52	8.50	4.83
2.25	1.21	5.75	15.69	8.75	2.41
2.50	7.24	6.00	15.69	9.00	2.41
2.75	7.24	6.25	15.69	9.25	2.41
3.00	7.24	6.50	15.69	9.50	2.41
3.25	7.24	6.75	8.45	9.75	1.21

MODIFY STORM
 CASE= 1
 MODIFYING PARAMETERS
 Multiplication Factor= 1.25
 Time shift (min) = .00

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.250	0.00	3.500	25.65	6.750	10.56
0.500	1.51	3.750	25.65	7.000	10.56
0.750	1.51	4.000	25.65	7.250	10.56
1.000	1.51	4.250	25.65	7.500	6.04
1.250	1.51	4.500	69.40	7.750	6.04
1.500	1.51	4.750	69.40	8.000	6.04
1.750	1.51	5.000	69.40	8.250	9.04
2.000	1.51	5.250	69.40	8.500	9.04
2.250	1.51	5.500	19.61	8.750	3.01
2.500	9.05	5.750	19.61	9.000	3.01
2.750	9.05	6.000	19.61	9.250	3.01
3.000	9.05	6.250	19.61	9.500	1.51
3.250	9.05	6.500	10.56	9.750	1.51

CALIB (0003) | Area (ha)= 1.52 | Curve Number (CN)= 60.0
 NASHYO | Ia (mm)= 5.00 | # of Linear Res. (N)= 3.00
 ID= 1 DT= 5.0 min | U.H. Tp (hrs)= .05

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.833	0.00	3.167	9.05	6.250	19.61
1.667	0.00	3.250	9.05	6.333	10.56
2.500	0.00	3.333	25.65	6.417	10.56
3.333	1.51	3.417	25.65	6.500	10.56
4.167	1.51	3.500	25.65	6.583	10.56
5.000	1.51	3.583	25.65	6.667	10.56
5.833	1.51	3.667	25.65	6.750	10.56
6.667	1.51	3.750	25.65	6.833	10.56
7.500	1.51	3.833	25.65	6.917	10.56
8.333	1.51	3.917	25.65	7.000	10.56
9.167	1.51	4.000	25.65	7.083	10.56
1.000	1.51	4.083	25.65	7.167	10.56
1.083	1.51	4.167	25.65	7.250	10.56
1.167	1.51	4.250	25.65	7.333	6.04
1.250	1.51	4.333	69.40	7.417	6.04
1.333	1.51	4.417	69.40	7.500	6.04
1.417	1.51	4.500	69.40	7.583	6.04
1.500	1.51	4.583	69.40	7.667	6.04
1.583	1.51	4.667	69.40	7.750	6.04
1.667	1.51	4.750	69.40	7.833	6.04
1.750	1.51	4.833	69.40	7.917	6.04
1.833	1.51	4.917	69.40	8.000	6.04
1.917	1.51	5.000	69.40	8.083	6.04
2.000	1.51	5.083	69.40	8.167	6.04
2.083	1.51	5.167	69.40	8.250	6.04
2.167	1.51	5.250	69.40	8.333	3.01
2.250	1.51	5.333	19.61	8.417	3.01
2.333	9.05	5.417	19.61	8.500	3.01
2.417	9.05	5.500	19.61	8.583	3.01
2.500	9.05	5.583	19.61	8.667	3.01
2.583	9.05	5.667	19.61	8.750	3.01
2.667	9.05	5.750	19.61	8.833	3.01
2.750	9.05	5.833	19.61	8.917	3.01
2.833	9.05	5.917	19.61	9.000	3.01
2.917	9.05	6.000	19.61	9.083	3.01
3.000	9.05	6.083	19.61	9.167	3.01

3.083 9.05 | 6.167 19.61 | 9.250 3.01 |

Unit Hyd Opak (cms) = 1.161
 PEAK FLOW (cms) = .189 (1)
 TIME TO PEAK (hrs) = 5.250
 RINFLOW VOLUME (mm) = 77.552
 TOTAL RAINFALL (mm) = 150.987
 RINFLOW COEFFICIENT = .514

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
 CULVERT (0002) | AREA (ha) = 3.63 | CURVE NUMBER (CN) = 75.0
 ID= 1 DF= 5.0 min | IA (mm) = 5.00 | # OF LINESAR REAS. (N) = 3.00
 U.H. TP (hrs) = .06

Unit Hyd Opak (cms) = 2.311
 PEAK FLOW (cms) = .471 (1)
 TIME TO PEAK (hrs) = 5.250
 RINFLOW VOLUME (mm) = 79.211
 TOTAL RAINFALL (mm) = 150.987
 RINFLOW COEFFICIENT = .525

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CULVERT (0001) | AREA (ha) = 5.22
 ID= 1 DF= 5.0 min | Total Imp(%) = 20.00 | Dir. Conn. (%) = 20.00

Surface Area (ha) = 3.04
 Dep. Storage (mm) = 1.00
 Average Slope (%) = 1.00
 Length (ft) = 186.50
 Manning's n = .013

Max Eff. Inten. (mm/hr) = 69.40
 over (min) = 5.00
 Storage Coeff. (mln) = 4.30 (11)
 Unit Hyd. Peak (mln) = 5.00
 Unit Hyd. Peak (cms) = .23

PRK FLOW (cms) = .20
 TIME TO PEAK (hrs) = 5.17
 RINFLOW VOLUME (mm) = 149.69
 TOTAL RAINFALL (mm) = 150.89
 RINFLOW COEFFICIENT = .99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.
 (1) ON PROCEDURE SELECTED FOR PREVIOUS LOSSES:
 CN* = 50.0 IA = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 THAN THE STORAGE COEFFICIENT.
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	AREA	QPEAK	TPPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID= 1 (0003):	1.52	1.89	5.25	77.55

+ ID= 2 (0002): 3.63 .471 5.25 79.21
 ID = 3 (0012): 5.15 .660 5.25 78.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0015)	IN= 2 -> OUT= 1	DF= 5.0 min	OUTFLOW (cms)	STORAGE (ha.in.)	OUTFLOW (cms)	STORAGE (ha.in.)
			.0000	.0000	.0960	.5250
			.0070	.4250	.1390	.5800
			.0640	.4300	.1920	.6150
			.0730	.4650	.2340	.6510

INFLOW : ID= 2 (0001) 5.22
 OUTFLOW : ID= 1 (0015) 5.22
 PEAK FLOW REDUCTION [Qout/Qin] (%) = 1.16
 TIME SHIFT OF PEAK FLOW (min) = 440.00
 MAXIMUM STORAGE USED (ha.in.) = .3716

ADD HYD	AREA	QPEAK	TPPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID= 1 (0012):	5.15	.660	5.25	78.72
+ ID= 2 (0015):	5.22	.006	12.58	43.64
ID = 3 (0014):	10.37	.663	5.25	62.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

POND 18 - CN AMC I

V V I SSSSS U U A L
 V V I SS U U A A L
 V V I SS U U A A L
 V V I SSSS UUUU A A LLLL

OOO TTTT H H Y Y M M OOO
 O O T T H H Y Y M M O O
 O O T T H H Y Y M M O O
 OOO T T H H Y Y M M OOO

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***** D E T A I L E D O U T P U T *****

Input filenames: C:\Program Files\Visual\OTRYMO 2.3.2\voisn.dat
 Output filenames: S:\2010 Projects\AD-BE (E10)\WR\W10-487 East Gwillimbury SWM Master
 Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas
 Summary filenames: S:\2010 Projects\AD-BE (E10)\WR\W10-487 East Gwillimbury SWM Master
 Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas

DATE: 3/21/2011

USER:

COMMENTS:

 ** SIMULATION NUMBER: 1 **

--- READ STORY --- File names: S:\2010 Projects\AD-BE (E10)\
 WR\W10-487 East Gwillimbury SWM Master Plan\
 Calcs\Hydrologic Models\Existing USCSA Files\
 --- Ptotal=120.71 mm --- Comments: 100yr/12hr

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.00	3.50	20.52	6.75	8.45	10.08	1.21	
0.25	3.75	20.52	7.00	8.45	10.25	1.21	
0.50	4.00	20.52	7.25	8.45	10.50	1.21	
0.75	4.25	20.52	7.50	8.45	10.75	1.21	
1.00	4.50	20.52	7.75	8.45	11.00	1.21	
1.25	4.75	20.52	8.00	8.45	11.25	1.21	
1.50	5.00	20.52	8.25	8.45	11.50	1.21	
1.75	5.25	20.52	8.50	8.45	11.75	1.21	
2.00	5.50	20.52	8.75	8.45	12.00	1.21	
2.25	5.75	20.52	9.00	8.45	12.25	1.21	
2.50	6.00	20.52	9.25	8.45	12.50	1.21	
2.75	6.25	20.52	9.50	8.45	12.75	1.21	
3.00	6.50	20.52	9.75	8.45	13.00	1.21	
3.25	6.75	20.52	10.00	8.45	13.25	1.21	

--- CALIB ---
 | WASHYD (0002) | Area (ha)= 1.52 Curve Number (CN)= 63.0
 | ID= 1 DF= 5.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 3.00
 | U.H. Tp (hrs)= .05

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH			
TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr
0.00	3.167	7.24	6.250
0.167	3.250	7.24	6.333
0.333	3.333	20.52	6.417
0.500	3.417	20.52	6.500
0.667	3.500	20.52	6.583
0.833	3.583	20.52	6.667
1.000	3.667	20.52	6.750
1.167	3.750	20.52	6.833
1.333	3.833	20.52	6.917
1.500	3.917	20.52	7.000
1.667	4.000	20.52	7.083
1.833	4.083	20.52	7.167
2.000	4.167	20.52	7.250
2.167	4.250	20.52	7.333
2.333	4.333	55.52	7.417
2.500	4.417	55.52	7.500
2.667	4.500	55.52	7.583
2.833	4.583	55.52	7.667
3.000	4.667	55.52	7.750
3.167	4.750	55.52	7.833
3.333	4.833	55.52	7.917
3.500	4.917	55.52	8.000
3.667	5.000	55.52	8.083
3.833	5.083	55.52	8.167
4.000	5.167	55.52	8.250
4.167	5.250	55.52	8.333
4.333	5.333	15.69	8.417
4.500	5.417	15.69	8.500
4.667	5.500	15.69	8.583
4.833	5.583	15.69	8.667
5.000	5.667	15.69	8.750
5.167	5.750	15.69	8.833
5.333	5.833	15.69	8.917
5.500	5.917	15.69	9.000
5.667	6.000	15.69	9.083
5.833	6.083	15.69	9.167
6.000	6.167	15.69	9.250

Unit Hyd Qpeak (cms)= 1.161

PEAK FLOW (cms)= 1.02 (i)
 TIME TO PEAK (hrs)= 5.250
 RUNOFF VOLUME (mm)= 38.565
 TOTAL RAINFALL (mm)= 120.710
 RUNOFF COEFFICIENT = .319

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

--- CALIB ---
 | WASHYD (0002) | Area (ha)= 3.63 Curve Number (CN)= 56.0
 | ID= 1 DF= 5.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 3.00
 | U.H. Tp (hrs)= .06

Unit Hyd Qpeak (cms)= 2.311

PEAK FLOW (cms)= .231 (i)

TIME TO PEAK (hrs) = 5.250
 RUNOFF VOLUME (mm) = 36.439
 TOTAL RAINFALL (mm) = 120.710
 RUNOFF COEFFICIENT = .302

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Max. Eff. Inten. (mm/hr) = 55.52
 Storage Coeff. (ft) = 2.30
 Time to Peak (hrs) = 23.02 (11)
 Date Hgt. Peak (ft) = 5.00
 Date Hgt. Peak (ft) = 23.02 (11)
 Date Hgt. Peak (ft) = .05

PEAK FLOW (cms) = .16
 TIME TO PEAK (hrs) = 5.25
 RINOFF VOLUME (mm) = 119.71
 TOTAL RAINFALL (mm) = 120.71
 RINOFF COEFFICIENT = .99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.
 (1) CN PROCEDURE SELECTED FOR IMPERVIOUS LOSSERS:
 CN = 30.0 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 (11) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	AREA	OPEAK	TPPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1 = 1 (0003):	1.52	.102	5.25	38.56
ID2 = 2 (0002):	3.63	.231	5.25	36.44
ID = 3 (0012):	5.15	.332	5.25	37.07

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

PEAK FLOW REDUCTION [Out/Qt] (%) = 1.26
 TIME SHEET OF PEAK FLOW (min) = 440.00
 MAXIMUM STORAGE (ha.m.) = .1996

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Max. Eff. Inten. (mm/hr) = 55.52
 Storage Coeff. (ft) = 2.30
 Time to Peak (hrs) = 23.02 (11)
 Date Hgt. Peak (ft) = 5.00
 Date Hgt. Peak (ft) = 23.02 (11)
 Date Hgt. Peak (ft) = .05

PEAK FLOW (cms) = .16
 TIME TO PEAK (hrs) = 5.25
 RINOFF VOLUME (mm) = 119.71
 TOTAL RAINFALL (mm) = 120.71
 RINOFF COEFFICIENT = .99

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
 ***** WARNING: FOR AREAS WITH IMPERVIOUS RATIOS BELOW 20%
 YOU SHOULD CONSIDER SPLITTING THE AREA.
 (1) CN PROCEDURE SELECTED FOR IMPERVIOUS LOSSERS:
 CN = 30.0 Ia = Dep. Storage (Above)
 (11) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
 (11) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD	AREA	OPEAK	TPPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1 = 1 (0012):	5.15	.332	5.25	37.07
ID2 = 2 (0015):	5.22	.003	12.58	24.54
ID = 3 (0014):	10.37	.334	5.25	30.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

POND 18 - CN AMC III

V V I SSSS U U A L
 V V I SS U U A A L
 V V I SS U U A A L L
 V V I SSSS UUUU A A LLLL

OOO TTTT H H Y Y M M OOO
 O O T T H H Y Y M M O O
 O O T T H H Y Y M M O O
 OOO T T H H Y Y M M OOO

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***** D E T A I L E D O U T P U T *****

Input filename: C:\Program Files\Visual OTTHMO 2.3.2\voicn.dat
 Output filename: s:\2010 Projects\A-EE (E10)\VRAW10-487 East Gwillimbury SWM Master
 Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas
 Summary filename: s:\2010 Projects\A-EE (E10)\VRAW10-487 East Gwillimbury SWM Master
 Plan\Calcs\Hydrologic Models\Sensitivity Analysis\Bas

DATE: 3/21/2011 TIME: 7:45:37 PM
 USER:

COMMENTS:

 ** SIMULATION NUMBER: 1 **

FILENAME: S:\2010 Projects\A-EE (E10)\
 VRAW10-487 East Gwillimbury SWM Master Plan\
 Calcs\Hydrologic Models\Existing USPCA Files\
 Ptotal=120.71 mm | Comments: 100yr/12hr

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.00	0.00	3.167	7.24	6.250	15.69	9.33	1.21
0.167	0.00	3.250	7.24	6.333	8.45	9.42	1.21
0.333	1.00	3.333	20.52	6.417	8.45	9.58	1.21
0.500	1.21	3.417	20.52	6.500	8.45	9.67	1.21
0.667	1.21	3.500	20.52	6.583	8.45	9.75	1.21
0.833	1.21	3.583	20.52	6.667	8.45	9.83	1.21
1.000	1.21	3.667	20.52	6.750	8.45	9.92	1.21
1.167	1.21	3.750	20.52	6.833	8.45	10.00	1.21
1.333	1.21	3.833	20.52	6.917	8.45	10.08	1.21
1.500	1.21	3.917	20.52	7.000	8.45	10.17	1.21
1.667	1.21	4.000	20.52	7.083	8.45	10.25	1.21
1.833	1.21	4.083	20.52	7.167	8.45	10.33	1.21
2.000	1.21	4.167	20.52	7.250	8.45	10.42	1.21
2.167	1.21	4.250	20.52	7.333	8.45	10.50	1.21
2.333	1.21	4.333	55.52	7.417	4.83	10.58	1.21
2.500	1.21	4.417	55.52	7.500	4.83	10.67	1.21
2.667	1.21	4.500	55.52	7.583	4.83	10.75	1.21
2.833	1.21	4.583	55.52	7.667	4.83	10.83	1.21
3.000	1.21	4.667	55.52	7.750	4.83	10.92	1.21
3.167	1.21	4.750	55.52	7.833	4.83	11.00	1.21
3.333	1.21	4.833	55.52	7.917	4.83	11.08	1.21
3.500	1.21	4.917	55.52	8.000	4.83	11.17	1.21
3.667	1.21	5.000	55.52	8.083	4.83	11.25	1.21
3.833	1.21	5.083	55.52	8.167	4.83	11.33	1.21
4.000	1.21	5.167	55.52	8.250	4.83	11.42	1.21
4.167	1.21	5.250	55.52	8.333	4.83	11.50	1.21
4.333	1.21	5.333	15.69	8.417	2.41	11.58	1.21
4.500	1.21	5.417	15.69	8.500	2.41	11.67	1.21
4.667	1.21	5.500	15.69	8.583	2.41	11.75	1.21
4.833	1.21	5.583	15.69	8.667	2.41	11.83	1.21
5.000	1.21	5.667	15.69	8.750	2.41	11.92	1.21
5.167	1.21	5.750	15.69	8.833	2.41	12.00	1.21
5.333	1.21	5.833	15.69	8.917	2.41	12.08	1.21
5.500	1.21	5.917	15.69	9.000	2.41	12.17	1.21
5.667	1.21	6.000	15.69	9.083	2.41	12.25	1.21
5.833	1.21	6.083	15.69	9.167	2.41	12.33	1.21
6.000	1.21	6.167	15.69	9.250	2.41	12.41	1.21

Unit Hyd Qpeak (cms)= 1.168 (1)
 PEAK FLOW (cms)= 5.250
 TIME TO PEAK (hrs)= 72.536
 RUNOFF VOLUME (mm)= 120.710
 TOTAL RAINFALL (mm)= 601
 RUNOFF COEFFICIENT = .601

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | CALIF | (0003) | Area (ha)= 1.52 Curve Number (CN)= 91.0
 | NASHVD | ID= 1 DT= 5.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 3.00
 |-----| U.H. Tp (hrs)= .05

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

----- TRANSFORMED HYETOGRAPH -----

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.00	0.00	3.167	7.24	6.250	15.69	9.33	1.21
0.167	0.00	3.250	7.24	6.333	8.45	9.42	1.21
0.333	1.00	3.333	20.52	6.417	8.45	9.58	1.21
0.500	1.21	3.417	20.52	6.500	8.45	9.67	1.21
0.667	1.21	3.500	20.52	6.583	8.45	9.75	1.21
0.833	1.21	3.583	20.52	6.667	8.45	9.83	1.21
1.000	1.21	3.667	20.52	6.750	8.45	9.92	1.21
1.167	1.21	3.750	20.52	6.833	8.45	10.00	1.21
1.333	1.21	3.833	20.52	6.917	8.45	10.08	1.21
1.500	1.21	3.917	20.52	7.000	8.45	10.17	1.21
1.667	1.21	4.000	20.52	7.083	8.45	10.25	1.21
1.833	1.21	4.083	20.52	7.167	8.45	10.33	1.21
2.000	1.21	4.167	20.52	7.250	8.45	10.42	1.21
2.167	1.21	4.250	20.52	7.333	8.45	10.50	1.21
2.333	1.21	4.333	55.52	7.417	4.83	10.58	1.21
2.500	1.21	4.417	55.52	7.500	4.83	10.67	1.21
2.667	1.21	4.500	55.52	7.583	4.83	10.75	1.21
2.833	1.21	4.583	55.52	7.667	4.83	10.83	1.21
3.000	1.21	4.667	55.52	7.750	4.83	10.92	1.21
3.167	1.21	4.750	55.52	7.833	4.83	11.00	1.21
3.333	1.21	4.833	55.52	7.917	4.83	11.08	1.21
3.500	1.21	4.917	55.52	8.000	4.83	11.17	1.21
3.667	1.21	5.000	55.52	8.083	4.83	11.25	1.21
3.833	1.21	5.083	55.52	8.167	4.83	11.33	1.21
4.000	1.21	5.167	55.52	8.250	4.83	11.42	1.21
4.167	1.21	5.250	55.52	8.333	4.83	11.50	1.21
4.333	1.21	5.333	15.69	8.417	2.41	11.58	1.21
4.500	1.21	5.417	15.69	8.500	2.41	11.67	1.21
4.667	1.21	5.500	15.69	8.583	2.41	11.75	1.21
4.833	1.21	5.583	15.69	8.667	2.41	11.83	1.21
5.000	1.21	5.667	15.69	8.750	2.41	11.92	1.21
5.167	1.21	5.750	15.69	8.833	2.41	12.00	1.21
5.333	1.21	5.833	15.69	8.917	2.41	12.08	1.21
5.500	1.21	5.917	15.69	9.000	2.41	12.17	1.21
5.667	1.21	6.000	15.69	9.083	2.41	12.25	1.21
5.833	1.21	6.083	15.69	9.167	2.41	12.33	1.21
6.000	1.21	6.167	15.69	9.250	2.41	12.41	1.21

Unit Hyd Qpeak (cms)= 1.161

PEAK FLOW (cms)= .168 (1)
 TIME TO PEAK (hrs)= 5.250
 RUNOFF VOLUME (mm)= 72.536
 TOTAL RAINFALL (mm)= 120.710
 RUNOFF COEFFICIENT = .601

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

 | CALIF | (0002) | Area (ha)= 3.63 Curve Number (CN)= 86.0
 | NASHVD | ID= 1 DT= 5.0 min | Ia (mm)= 5.00 # of Linear Res. (N)= 3.00
 |-----| U.H. Tp (hrs)= .06

Unit Hyd Qpeak (cms)= 2.311

PEAK FLOW (cms)= .434 (1)

TIME TO PEAK (hrs) = 5.250
 RUNOFF VOLUME (mm) = 76.414
 TOTAL RAINFALL (mm) = 120.710
 RUNOFF COEFFICIENT = .633

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CAULDS			
SPANDHYD (0001)	Area (ha) = 5.22		
ID= 1 DT= 5.0 min	Total Imp(1) = 20.00	Dist. Conn. (8) = 20.00	
Surface Area	INSERVIOWS	PERVIOWS (1)	
Dep. Storage (mm) = 1.04		4.18	
Average Slope (1) = 1.00		2.00	
Length (m) = 186.50		40.00	
Manning's n = .013		.250	
Max. Eff. Inflow (mm/hr) = 55.52		36.50	
Storage Coeff. (min) = 5.00		20.00	
Unit Hyd. Peak (mm) = 4.70 (11)		15.26 (11)	
Unit Hyd. Peak (cms) = 5.00		20.00	
Unit Hyd. Peak (cms) = .22		.07	
PEAK FLOW (cms) = .16		.37	
TIME TO PEAK (hrs) = 5.25		5.25	
RUNOFF VOLUME (mm) = 118.71		62.31	
TOTAL RAINFALL (mm) = 120.71		120.71	
RUNOFF COEFFICIENT = .99		.52	
			TOTALS
			.534 (11)

***** WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEEP!
 ***** WARNING: FOR AREAS WITH INSERVIOWS PERIOWS BELOW 208
 YOU SHOULD CONSIDER SPLITTING THE AREA.
 (1) ON PROCEDURE SELECTED FOR PERVIOWS LOSSES.
 (11) TIME STEEP (TP) SHOULD BE SMALLER OR EQUAL
 TO DEP STORAGE (DBSD).
 (111) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0012)	AREA	QPEAK	TPPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0003):	1.52	.168	5.25	72.54
+ ID2= 2 (0002):	3.63	.434	5.25	76.41
ID = 3 (0012):	5.15	.602	5.25	75.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

RESERVOIR (0015)	OUTFLOW	STORAGE	OUTFLOW	STORAGE
ID= 2--> OMT= 1	(cms)	(ha.m.)	(cms)	(ha.m.)
DT= 5.0 min	.0000	.0000	.0960	.5250
	.0070	.4250	.1390	.8800
	.0640	.4300	.1920	.6150
	.0730	.4650	.2510	.6510
INFLOW: ID= 2 (0001)	AREA	QPEAK	TPPEAK	R.V.
OUTFLOW: ID= 1 (0015)	(ha)	(cms)	(hrs)	(mm)
	5.22	.53	5.25	73.79
		.01	12.58	45.36

PEAK FLOW REDUCTION (Coef./Qin) (1) = 1.14
 FIVE STEEP PEAK FLOW (mm) = 430.00
 MAXIMUM STORAGE (ha.m.) = .5894

(1) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0014)	AREA	QPEAK	TPPEAK	R.V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 (0012):	5.15	.602	5.25	75.27
+ ID2= 2 (0015):	5.22	.006	12.58	45.36
ID = 3 (0014):	10.37	.605	5.25	60.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

APPENDIX L
Notice Of Commencement



Town of
East Gwillimbury

**TOWN OF EAST GWILLIMBURY
CLASS ENVIRONMENTAL ASSESSMENT
STORMWATER MANAGEMENT MASTER PLAN
NOTICE OF STUDY COMMENCEMENT**

Further to the objectives identified in the Lake Simcoe Protection Plan (June 2009), the Town of East Gwillimbury has retained a consultant to develop a Stormwater Management Master Plan to define all anticipated works necessary to maintain, expand and improve the existing storm drainage system (including ponds) while protecting the valued natural resources both within and beyond Town limits. This Class Environmental Assessment (EA) Master Plan is intended to improve the management of stormwater for both existing and planned development which is based on changes in land use as outlined in the Consolidated Official Plan 2031 for the Town of East Gwillimbury (June 2010). Cole Engineering Group Ltd. will be working closely with the Town of East Gwillimbury and the Lake Simcoe Region Conservation Authority to establish recommendations to address these improvements.

A key component of the study will be consultation with interested stakeholders (public, landowners and regulatory agencies). Public consultation will involve one (1) Public Information Centre (PIC) and a Public Education Campaign. These sessions will be designed to obtain input from the public to assist informing the decision making process and will provide updates as the study progresses.

The first PIC is anticipated to be held in February 2011. The purpose of this meeting will be to provide interested parties with an opportunity to review and discuss issues related to the study and the alternative design solutions. Details regarding the future consultation events will be advertised as the study progresses. In addition, information on the study will be posted on the Town of East Gwillimbury's website and can be found under the 'Our Town News' section at <http://www.eastgwillimbury.ca>.

The study is currently planned as a 'Master Plan' project in compliance with the Municipal Engineers Association document "*Municipal Class Environmental Assessment*," (October 2000, amended 2007).

We are interested in hearing any comments or input that you may have about this study. Comments and information regarding the study are being collected to assist the Town of East Gwillimbury in meeting the requirements of the Environmental Assessment Act. This material will be maintained on file for use during the study and may be included in study documentation.

If you require further information, would like to be added to the mailing list, or if you have specific comments related to this study, please email your comments to EastGwillimburySWMMasterPlan@coleengineering.ca or contact either of the following:

Christopher Kalimootoo, BA, P.Eng.
Director of Engineering and
Environmental Services
Community Programs & Infrastructure
Town of East Gwillimbury
19000 Leslie Street
Sharon, ON L0G 1V0
Tel: (905) 478-4282 Ext. 3815

Edward Graham, M.A.Sc., P.Eng.
Project Manager

Cole Engineering Group Ltd.
Consultant
70 Valleywood Drive
Markham, ON L3R 4T5
Tel: (905) 940-6161

This Notice was first issued on December 20, 2010.

APPENDIX M
Notice Of Public Information Centre



Town of
East Gwillimbury

**TOWN OF EAST GWILLIMBURY
CLASS ENVIRONMENTAL ASSESSMENT
STORMWATER MANAGEMENT MASTER PLAN
NOTICE OF PUBLIC INFORMATION CENTRE**

The Town of East Gwillimbury has initiated a Class Environmental Assessment (Class EA) Master Plan to define all anticipated works necessary to maintain, expand and improve the existing storm drainage system (including ponds) while protecting the valued natural resources both within and beyond Town limits. The objective of the study is to improve the management of stormwater for both existing and planned development which is based on changes in land use as outlined in the Consolidated Official Plan 2031 for the Town of East Gwillimbury (June 2010) while meeting all requirements as set out in the Lake Simcoe Protection Plan (June 2009).

A key component of the study will be consultation with interested stakeholders (public, landowners and regulatory agencies). The Public Information Centre (PIC) will provide interested parties with an opportunity to review the work completed to date, discuss issues related to the project, and review potential alternative solutions. A PIC is scheduled for:

Date: Thursday, March 3, 2011

Location: Town of East Gwillimbury Town Office
Holland Landing Room
19000 Leslie St.
Sharon, ON L0G 1V0
Time: 4:00 p.m. to 7:00 p.m.

The study will be conducted as a 'Master Plan' project in compliance with the Municipal Engineers Association document "*Municipal Class Environmental Assessment*," (October 2000, amended 2007) which will address Phases 1 and 2 of the Class EA Process. We are interested in hearing any comments or input that you may have about this study. Comments and information regarding the study are being collected to assist the Town of East Gwillimbury in meeting the requirements of the Environmental Assessment Act. This material will be maintained on file for use during the study and may be included in study documentation.

If you require further information, or if you have specific comments related to this study, please contact either of the following:

Christopher Kalimootoo, BA. P.Eng.
Director of Engineering and
Environmental Services
Community Programs & Infrastructure
Town of East Gwillimbury
19000 Leslie Street
Sharon, ON L0G 1V0
Tel: (905) 478-4282 Ext. 3815

Edward Graham, M.A.Sc., P.Eng.
Project Manager
Cole Engineering Group Ltd.
Consultant
70 Valleywood Drive
Markham, ON L3R 4T5
Tel: (905) 940-6161

APPENDIX N
Public Information Centre Boards

**Town of East Gwillimbury
Stormwater Management Master Plan
Municipal Class EA
Public Information Centre
19000 Leslie Street, Sharon ON L0G 1V0
Town of East Gwillimbury Town Offices
Holland Landing Room
March 3, 2011**



**Welcome to the
East Gwillimbury Stormwater Management
Master Plan
Public Information Centre**

19000 Leslie Street, Sharon ON L0G 1V0
Town of East Gwillimbury Town Offices
Holland Landing Room

March 3, 2011

- ◆ Please sign in on the sheet provided, then feel free to walk around and view the displays.
- ◆ If you have any questions, our representatives will be pleased to discuss the project with you.
- ◆ Comment sheets are provided for those who wish to provide comments in writing. Please either place your completed sheets in the Comment Box, email to EastGwillimburySWMMasterPlan@ColeEngineering.ca, or mail/fax them to one of the identified Project Team Members (see below) by **March 17, 2011**.
- ◆ Thank-you for your involvement in this project.
- ◆ For additional information, please contact one of the following Team Members:

Christopher Kalimootoo, BA., P.Eng.

**Director of Engineering and
Environmental Services**

Town of East Gwillimbury

19000 Leslie Street,

Sharon, ON L0G 1V0

Phone: 905-478-4282 Ext. 3815

Fax: 905-478-2808

Email: ckalimootoo@eastgwillimbury.ca

Geoff Masotti, P.Eng.

Project Engineer

Cole Engineering Group Ltd.

70 Valleywood Drive

Markham, ON L3R 4T5

Phone: 905-940-6161 Ext. 254

Fax: 905-940-2064

E-mail: gmasotti@coleengineering.ca



Town of
East Gwillimbury

Purpose of the Public Information Centre

The purpose of this Public Information Centre (PIC) is to introduce you to this project, inform you of progress to date and obtain your comments.

The major elements presented today are:

- ◆ Study Overview & Background;
- ◆ Problem / Opportunity Statement
- ◆ Overview of the Municipal Class Environmental Assessment Process;
- ◆ Alternative Solutions Being Considered; and
- ◆ Next Steps.



Town of
East Gwillimbury

Study Overview

Problem

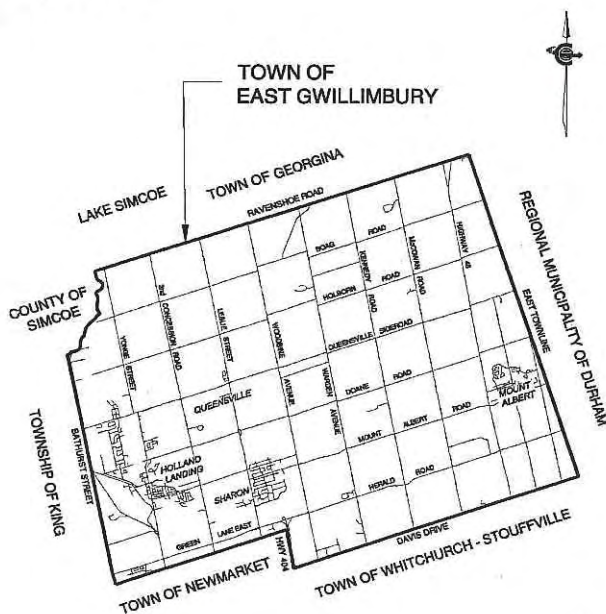
The Town of East Gwillimbury has retained Cole Engineering to develop a Stormwater Management Master plan in accordance to *Section 4.5-SA* of the *Lake Simcoe Protection Plan (June 2009)*. The plan will define all anticipated works necessary to maintain, expand and improve the existing storm drainage system.

Opportunity

An opportunity exists to improve stormwater management within the Town of East Gwillimbury by implementing a drainage strategy. While implementing drainage improvements, there will be the opportunity to mitigate ongoing erosion and sedimentation, phosphorus loadings and changes in water balance.

This study is following the Master Plan requirements of the Municipal Class EA (2007) planning process.

Key Plan



Town of East Gwillimbury

Background

- ◆ A Stormwater Management (SWM) Master Plan is to define all anticipated works necessary to maintain, expand and improve the existing storm drainage system (including ponds) while protecting the valued natural resources both within and beyond Town limits.
- ◆ The anticipated growth of the Town as outlined in the Town of East Gwillimbury Consolidated Official Plan 2031 (June 2010), provides the framework necessary for the development of the SWM Master Plan.
- ◆ The study area is part of four (4) subwatersheds:
 1. West Holland River Subwatershed;
 2. East Holland River Subwatershed;
 3. Maskinonge River Subwatershed; and
 4. Black River Subwatershed.
- ◆ High phosphorus levels in Lake Simcoe is a current issue, thus efforts will be made to mitigate phosphorus loadings generated from existing and planned development.



Town of
East Gwillimbury

Overview of the Municipal Class Environmental Assessment Process

- ◆ The *Municipal Class Environmental Assessment (2007)* (Class EA) process, which is approved under the *Environmental Assessment Act*, enables the planning of municipal infrastructure projects in accordance with a proven procedure for protecting the environment.
- ◆ The study is being undertaken in accordance with the first two Phases of the Class EA process for a **Master Plan** project.
- ◆ The Master Plan Class EA process includes public and review agency consultation, an evaluation of alternatives, and an assessment of the effects on the environment.
- ◆ There is an opportunity at any time during the Class EA process for public input, including this Public Information Centre (PIC).
- ◆ Upon completion of the Master Plan Class EA, a Project File Report will be available for public review.



Town of
East Gwillimbury

Alternative Stormwater Management Strategies

1. Do Nothing

- ◆ Existing stormwater management works are left in place for rural and urban lands.
- ◆ The Town continues to grow and expected flooding and erosion issues may arise.

2. Traditional Stormwater Management Strategy

- ◆ This alternative is the implementation of conventional stormwater management in proposed development or redevelopment areas.
- ◆ Stormwater management works would consist of stormwater management ponds for quality, quantity and erosion control as per the Ministry of Environment (MOE) guidelines.

3. Traditional SWM with BMP Implementation Strategy

- ◆ This alternative is consistent with the Traditional Stormwater Management Strategy as it will apply to proposed development and redevelopment areas.
- ◆ Best Management Practices of source controls, conveyance controls and end of pipe measures will be implemented for all proposed areas of development.
- ◆ Alternative LIDs (Low Impact Development technologies) would also be considered where applicable.

4. Traditional SWM with Urban Retrofits Strategy

- ◆ This alternative is consistent with the Traditional Stormwater Management Strategy as it will apply to proposed development and redevelopment areas.
- ◆ In addition, a variety of source, conveyance and end of pipe retrofit measures will be implemented in existing urban areas.

5. Traditional Stormwater Management with Rural Retrofits Strategy

- ◆ This alternative is consistent with the Traditional Stormwater Management Strategy as it will apply to proposed development and redevelopment areas.
- ◆ In addition, a variety of source, conveyance, and end of pipe measures will be implemented in existing rural areas.



Town of
East Gwillimbury

Alternative Solutions: Source Controls

Roof Downspout Disconnection



- ◆ Involves directing flow from roof downspouts to a pervious area that drains away from the building.
- ◆ Will prevent stormwater from directly entering the storm sewer system or flowing across an impervious surface, such as a driveway that drains into a storm sewer.

Bioretention



- ◆ Bioretention are landscaping features that temporarily stores, treats, and infiltrates runoff.
- ◆ Primary component is a filter bed composed of a mixture of sand and organic material. Other components include mulch ground cover and plants adapted to the conditions of stormwater practice.
- ◆ Designed to capture small storm events.

Green Roofs (i.e. Roof Gardens)



- ◆ Consist of a thin layer of vegetation and growing medium on top of a conventional flat or sloped roof.
- ◆ Improve energy efficiency, water quality, water balance, and peak flow control.
- ◆ Excess rainfall enters underdrain and overflow points and is conveyed in the building drainage system.
- ◆ Stored water is evapotranspired by the plants and evaporates or slowly drains.

Alternative Solutions: Source Controls



Soakaway Pits, Infiltration Trenches

- ◆ Soakaway pits and infiltration trenches are rectangular or circular excavations with geotextile fabric, filled with clean granular stone.
- ◆ They will receive runoff from a perforated pipe inlet and allow it to infiltrate into the native soil.
- ◆ Typically service individual lots and receive only roof and walkway runoff.



Permeable Pavement

- ◆ Allows stormwater to drain through the pavement and into a stone reservoir where it infiltrates into underlying native soil or temporarily detained.
- ◆ Can be used for low traffic roads, parking lots, driveways, pedestrian walkways. Ideal for sites with limited space for surface stormwater BMPs.



Rainwater Harvesting

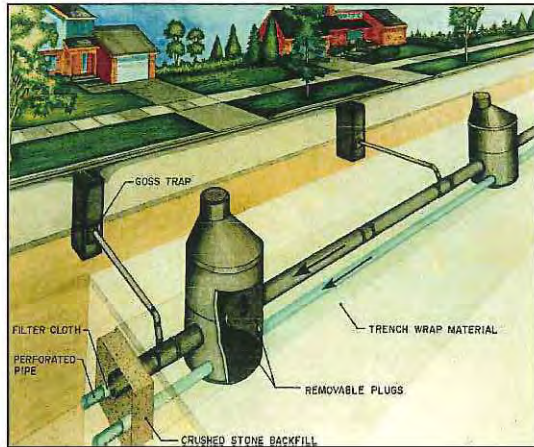
- ◆ The process of intercepting, conveying and storing rainfall for future use.
- ◆ Potable water can be used in rural and urban areas to irrigate landscaped areas and for other uses.
- ◆ Rainwater is typically collected from roof drains into rain barrels or containment units.

Alternative Solutions: Conveyance Controls



Grassed Swales

- ◆ Vegetated open channels designed to convey, treat and attenuate stormwater runoff.
- ◆ Check dams and vegetation in the swale slows the water to allow sedimentation, filtration through the root zone, evapotranspiration, and infiltration into the underlying native soil.



Perforated Pipe Systems

- ◆ Long infiltration trenches or linear soakaway pits that are designed for both conveyance and infiltration of stormwater runoff.
- ◆ Composed of perforated pipes lined with geotextiles to allow infiltration.
- ◆ Perforated pipe systems can be used in place of conventional storm sewer pipes where topography, and water table depth conditions are suitable.



Vegetated Filter Strips

- ◆ Gently sloped, densely vegetated areas that treat runoff as sheet flow from adjacent impervious areas.
- ◆ Function by slowing runoff velocity and filtering out suspended sediment and pollutants, while providing some infiltration.
- ◆ Vegetation may be comprised of a variety of trees, shrubs and native plants.

Alternative Solutions: End of Pipe Measures



Stormwater Management Ponds (Wet Ponds)

- ◆ Less land intensive than wetland systems and normally reliable in operation.
- ◆ Can efficiently provide for water quality, erosion, and quantity control.
- ◆ Can be designed with extensive landscaping and associated recreational amenities.



Dry Ponds

- ◆ Dry ponds have no permanent pool of water and rely on retention time for contaminant removal.
- ◆ Designed for erosion control, flood control and the removal of stormwater contaminants.



Constructed Wetlands

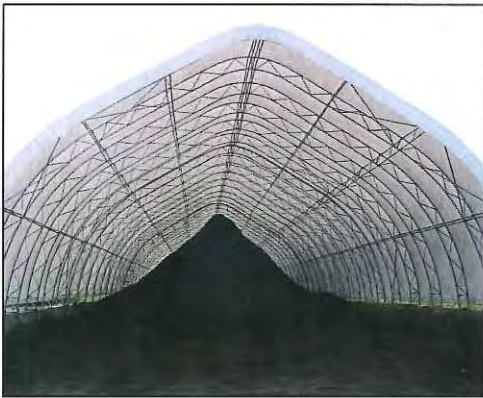
- ◆ Suitable for providing storage needed for erosion control and phosphorus removal but limited in flood control.
- ◆ Normally more land intensive than wet ponds due to shallow depth.

Alternative Solutions: Rural Measures



Restoration Measures

- ◆ Includes the restoration or enhancement of existing habitats.
- ◆ Stream restoration programs help develop the implementation of this alternative.
- ◆ Aquatic and terrestrial habitat enhancement.



Structural Measures

- ◆ Designed to reduce the loss of topsoil containing contaminants and their deposition into the lake and watershed.
- ◆ Examples include proper manure storage, milkhouse waste management, private septic systems, bypass channels and bottom draw structures for online ponds.



Non-Structural Measures

- ◆ Best Management Practices in rural areas which do not require structures.
- ◆ Examples include livestock fencing, buffer strips, conservation tillage and nutrient management.

The Next Steps . . .

- ◆ Comments received from this PIC will be considered along with those received from review agencies and landowners in order to identify the preferred Alternative Solution(s).
- ◆ The study team will identify alternative design concepts associated with the preferred solution and evaluate those concepts while considering the anticipated environmental effects.
- ◆ A Notice of Completion will be mailed to the contact list, advertised, and the project file will be available for viewing at the Town Hall.
- ◆ Further comments can be provided after the PIC or when the Notice of Completion is published.

Thank you for your participation!



Town of
East Gwillimbury

APPENDIX O
Public Information Centre Sign-In Sheet And
Comment Form

REGISTRATION SHEET

Public Information Centre – Town of East Gwillimbury
Class Environmental Assessment and Stormwater Management Master Plan

March 3, 2011 – Holland Landing Room – 19000 Leslie Street Sharon

Name (Please Print)	Address / E-mail	Phone Number
1. GEDDF MASOTTI	g.masotti@coleengineering.ca	905-940-6161
2. JOHN EATON	jeaton@eastgwillimbury	
3. Dale Pegg	dale.pegg@rogers.com	905-478-4128
4. Don Sander		
5. Virginia Hucks		
6. Java Roy-Dillente		
7. Madeline Holtz		
8.		
9.		
10.		

APPENDIX P
Indian And Northern Affairs Canada Letter

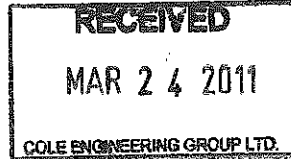


Votre référence - Your file

Notre référence - Our file

March 14, 2011

Edward Graham
Project Manager
Cole Engineering
70 Valleywood Drive
Markham, Ontario L3R 4T5



Dear Mr. Graham,

Re: Town of East Gwillimbury, Class Environmental Assessment Storm water Management Master Plan - Notice of Public Information Centre

I am writing in response to your letter of February 22, 2011 addressed to Josée Beauregard inquiring about any claims that may affect the subject property. I regret that we were unable to respond earlier. Thank you for your invitation to the Public Information Centre, held on March 3, 2011. Unfortunately, we were unable to attend; however, the following information regarding active litigation may be useful to you as it could affect the lands that you are concerned with.

We can inform you that our inventory includes active litigation in the vicinity of this property. They are, *Alderville Indian Band, Beausoleil Indian Band, Chippewas of Georgina Island Indian Band, Chippewas of Rama Indian Band, Curve Lake Indian Band, Hiawatha Indian Band, Mississaugas of Scugog Indian Band v. HTMQ and Ontario (Third Party), Federal Court of Canada, filed in Montreal, Court file reference # T-195-92; and*

Moose Deer Point First Nation, Chief Edward Williams suing on his own behalf and on behalf of the members of Moose Deer Point First Nation v. Her Majesty the Queen in Right of Ontario, Superior Court of Justice File #01-CV-220612CM.

I am unable to comment with respect to the possible effect of these claims as the cases have not yet been adjudicated and any statement regarding the outcome of the litigation would be speculative at this point. It is recommended that you consult legal counsel as to the effect these actions could have on the lands you are concerned with.

If you are interested in further details about these claims, copies of the pleadings can be obtained from the Court for a fee. Please contact the appropriate Court Registry Office and make reference to the court file numbers listed above.

We cannot make any comments regarding claims filed under other departmental policies. For information on any claims you should also contact Don Boswell of the Specific Claims Branch at (819) 953-1940 to inquire about any Specific Claims. To inquire about any current Comprehensive Claims, please contact Nicole Cheechoo of Treaty and Aboriginal Government Central Operations at (819) 997-3499.

If you have any further questions please do not hesitate to contact me at (819)994-1947. Also, please note that all future requests of this nature should no longer be addressed to Josée Beauregard. Instead, could you kindly modify your distribution list to send these requests to the following destination:

Marc-André Millaire, Ontario Team
Indian and Northern Affairs
LITIGATION MANAGEMENT AND RESOLUTION BRANCH
25 Eddy Street
Gatineau, Quebec
K1A 0H4

Sincerely,


for Marc-André Millaire
Litigation Team Leader
Eastern Litigation Directorate
Litigation Management and Resolution Branch

DISCLAIMER: In this Disclaimer, "Canada" means Her Majesty the Queen in right of Canada and the Minister of Indian Affairs and Northern Development and their servants and agents. Canada does not warrant or assume any legal liability or responsibility for the accuracy, completeness, or usefulness of any data or information disclosed with this correspondence or for any actions in reliance upon such data or information or on any statement contained in this correspondence. Data and information is based on information in departmental records and is disclosed for convenience of reference only. Canada does not act as a representative for any Aboriginal group for the purpose of any claim. Information from other government sources and private sources (including Aboriginal groups) should be sought, to ensure that the information you have is accurate and complete.

APPENDIX Q
Lake Simcoe Regional Conservation Authority
Meeting Minutes

PROGRESS MEETING #:	LSRCA Meeting #1	DATE:	January 14th, 2011
		TIME:	10:00 AM
PROJECT NAME:	Town of East Gwillimbury Stormwater Management Master Plan	PROJECT #:	W10-487
LOCATION:	LSRCA Offices 120 Bayview Parkway, Newmarket		
PURPOSE:	Discuss East Gwillimbury SWM Master Plan		
PRESENT:	REGRETS:		
Tom Hogenbirk (TH), LSRCA			
Geoff Masotti (GM), Cole Engineering			
Alan Manlucu (AM), Cole Engineering			

ITEM	DESCRIPTION	ACTION BY
1.	<p>Review of Sections in the LSPP</p> <ul style="list-style-type: none"> Reviewed each section of SWM Master Plan Guidelines Matrix with respect to the LSPP Document Existing GIS Layers/Models/Data required from LSRCA TH to provide GIS and Models by request with Agreement Form Contact for information or clarifications regarding the SWM Master Plan is Tom Hogenbirk, LSRCA 	
2.	<p>Specific Sections Required to be Addressed</p> <p>Phosphorus</p> <ul style="list-style-type: none"> If > 750 ha, model required Refer to Report on Phosphorus Loads to Lake Simcoe 2004-2007 for pre-development loading rates Refer to Humber River Watershed phosphorus study regarding urban phosphorus loading rates <p>Climate Change</p> <ul style="list-style-type: none"> Address based on SWM Facility improvements for changes in weather (i.e. droughts or floods) and if retrofit opportunities exist Vegetation within SWM Facility to be improved based on climate change Based on frequency of flooding, what are the impacts to existing SWM Facilities and infrastructure? What improvements need to be made? Refer to Dave Lemkey (Dave L.?) from LSRCA for retrofit strategy (2007) <p>Erosion Analysis</p> <ul style="list-style-type: none"> Erodibility Index Analysis of reaches within EG and analysis to meet Index Possible survey required with assistance from fluvial 	

PLEASE NOTE: If your records of this meeting do not agree with this document, or if there are any omissions, please advise the writer at once, otherwise the contents of this document shall be assumed accurate and correct.

ITEM	DESCRIPTION	ACTION BY
3.	Alternatives <ul style="list-style-type: none"> • If proposing BMPs, may require by-law or implementation of permanent BMPs • Abide by LSPP as well as EA Process with respect to alternatives and public consultation 	
4.	Review <ul style="list-style-type: none"> • Circulation to LSRCA, but ultimate approval by Town required • Final Project File to be organized by headings outlined in Section 4.5 of LSPP (SWM Master Plan Guidelines Matrix) with Alternatives based on EA Process 	

Next Meeting: TBD
 Minutes Recorded By: Alan Manlucu
 Distribution: Town of East Gwillimbury

APPENDIX R
Ministry Of Environment Letter

Ministry of the Environment

Central Region
Technical Support Section

5775 Yonge Street, 8th Floor
North York, Ontario M2M 4J1

Tel.: (416) 326-6700
Fax: (416) 325-6347

Ministère de l'Environnement

Région du Centre
Section d'appui technique

5775, rue Yonge, 8^{ème} étage
North York, Ontario M2M 4J1

Tél. : (416) 326-6700
Télééc. : (416) 325-6347



March 8, 2011

File: EA02-03-04

Christopher Kalimootoo, P.Eng
Community Programs and Infrastructure
Town of East Gwillimbury
19000 Leslie Street
Sharon, ON L0G 1V0

**RE: TSS Comments:
Stormwater Management Master Plan
Town of East Gwillimbury
Class Environmental Assessment
Response to Notice of Commencement**

Dear Mr. Kalimootoo:

This letter is our response to the Notice of Study Commencement for the above noted project. This response acknowledges that the Town of East Gwillimbury has indicated that its study is following the approved environmental planning process under the *Municipal Engineers Association Municipal Class Environmental Assessment (Class EA)*.

Based on the information submitted, we have identified the following issues of concern with respect to the proposed undertaking:

- Ecosystem Protection and Restoration
- Surface Water
- Groundwater
- Air Quality
- Servicing and Facilities
- Contaminated Soils
- Mitigation and Monitoring
- Planning and Policy
- Class EA Process
- First Nations Consultation

We are providing the following general comments to assist you and your project team members in effectively addressing these issues:

Ecosystem Protection and Restoration

- Any impacts to ecosystem form and function must be avoided where possible. The Project File/ESR should describe any proposed mitigation measures and how project planning will protect and enhance the local ecosystem.
- All natural heritage features should be identified and described in detail to assess potential impacts and to develop appropriate mitigation measures. Our records confirm that the following sensitive environmental features are located within or adjacent to the Study Area:
 - Areas of Natural and Scientific Interest (ANSIs)
 - Environmentally Sensitive Areas (ESAs)

- Rare Species of flora or fauna
- Watercourses
- Wetlands
- Woodlots

We recommend consulting with the Ministry of Natural Resources (MNR), Fisheries and Oceans Canada (DFO) and your local conservation authority to determine if special measures or additional study will be necessary to preserve and protect these sensitive features.

Surface Water

- The Project File/ESR must include a sufficient level of information to demonstrate that there will be no negative impacts on the natural features or ecological functions of any watercourses within the Study Area. Measures should be included in the planning and design process to ensure that any impacts to watercourses from construction or operational activities (e.g. spills, erosion, pollution) are mitigated as part of the proposed undertaking. The MOE Guideline B-6, *Evaluating Construction Activities Impacting on Water Resources* should be used to plan and construct this project.
- Additional stormwater runoff from new pavement can impact receiving watercourses and flood conditions. Quality and quantity control measures to treat stormwater runoff should be considered for all new impervious areas and, where possible, existing surfaces. MOE's *Stormwater Management Planning and Design Manual (2003)* should be referenced in the Project File/ESR and utilized when designing stormwater control methods. We recommend that a Stormwater Management Plan should be prepared as part of the Class EA process that includes:
 - Strategies to address potential water quantity and erosion impacts related to stormwater draining into streams or other sensitive environmental features, and to ensure that adequate (enhanced) water quality is maintained
 - Watershed information, drainage conditions, and other relevant background information
 - Future drainage conditions, stormwater management options, information on erosion and sediment control during construction, and other details of the proposed works
 - Information on maintenance and monitoring commitments
- Ontario Regulation 60/08 under the Ontario Water Resources Act (OWRA) applies to the Lake Simcoe Basin, which encompasses Lake Simcoe and the lands from which surface water drains into Lake Simcoe. The proposed WWTP is listed in Table 1 of the regulation. The EA Document should describe how the proposed project and its mitigation measures are consistent with the requirements of this regulation and the OWRA.

Groundwater

- The status of, and potential impacts to any well water supplies should be addressed. If the project involves groundwater takings or changes to drainage patterns, the quantity and quality of groundwater may be affected due to drawdown effects or the redirection of existing contamination flows. In addition, project activities may infringe on existing wells such that they must be reconstructed or sealed and abandoned. Appropriate information to define existing groundwater conditions should be included in the Project File/ESR.
- If the potential construction or decommissioning of water wells is identified as an issue, the Project File/ESR should refer to Ontario Regulation 903, Wells, under the *Ontario Water Resources Act*.
- Potential impacts to groundwater-dependent natural features should be addressed. Any changes to groundwater flow or quality from groundwater taking may interfere with the ecological

processes of streams, wetlands or other surficial features. In addition, discharging contaminated or high volumes of groundwater to these features may have direct impacts on their function. Any potential effects should be identified, and appropriate mitigation measures should be recommended. The level of detail required will be dependent on the significance of the potential impacts.

- Any potential approval requirements for groundwater taking or discharge should be identified in the Project File/ESR. In particular, a Permit to Take Water (PTTW) under the *Ontario Water Resources Act* will be required for any water takings that exceed 50,000 litres per day. A PTTW application must be accompanied by an assessment of potential effects as noted above, and may require a higher level of detail than what is provided in the Project File/ESR. Please note that when significant long-term water taking is proposed, the maximum rate identified in the Project File/ESR must not be exceeded in any subsequent PTTW applications. For more information on the application and approval process, we suggest you refer to the MOE *Permit to Take Water Manual* (April 2005).

Air Quality

- An air quality or odour impact assessment may be required for this project to evaluate alternatives, determine impacts and identify appropriate mitigation measures. The scope of the assessment should be determined based on the potential effects of the proposed alternatives, and typically includes source and receptor characterization, a quantification of air quality impacts by determining emission rates and conducting dispersion modelling, and an assessment of effects. This assessment should compare to all available standards for any contaminants of concern. We recommend that you contact this office during the scoping process to confirm the appropriate level of assessment.

Dust and Noise

- Dust and noise control measures should be addressed and included in the construction plans to ensure that nearby residential and other sensitive land uses within the Study Area are not adversely affected during construction activities. If dust suppressants are proposed to be used, we recommend the use of non-chloride based compounds to protect water quality.
- The Project File/ESR should consider the potential impacts of increased noise levels during the operation of the undertaking. The proponent should explore all potential measures to mitigate significant noise impacts during the assessment of alternatives.

Servicing and Facilities

- Any facility that releases emissions to the atmosphere, discharges contaminants to ground or surface water, provides potable water supplies, or stores, transports or disposes of waste must have a Certificate of Approval before it can operate lawfully. Please consult with the Environmental Assessment and Approvals Branch to determine whether a new or amended Certificate of Approval will be required for any proposed infrastructure.
- We recommend referring to MOE's "D-Series" guidelines – *Land Use Compatibility* to ensure that all applicable Ministry procedures are followed in planning for any infrastructure or facilities related to wastewater, pipelines, landfills or industrial uses.

Contaminated Soils

- Since the removal or movement of soils may be required, appropriate tests to determine contaminant levels from previous land uses or dumping should be undertaken. If the soils are

contaminated, you must determine how and where they are to be disposed of, consistent with *Part XV.1 of the Environmental Protection Act (EPA)* and Ontario Regulation 153/04, Records of Site Condition, which details the new requirements related to site assessment and clean up. We recommend contacting the MOE York Durham District Office in Ajax for further consultation if contaminated sites are present.

- The location of any underground storage tanks should be included in the Project File/ESR. Measures should be identified to ensure the integrity of these tanks and to ensure an appropriate response in the event of a spill. The MOE Spills Action Centre must be contacted in such an event.
- Any current or historical waste disposal sites should be identified in the Project File/ESR. The status of these sites should be determined to confirm whether approval pursuant to Section 46 of the *Environmental Protection Act* may be required for land uses on former disposal sites.
- The Project File/ESR should identify any underground transmission lines in the Study Area. The owners should be consulted to avoid impacts to this infrastructure, including potential spills.

Mitigation and Monitoring

- Design and construction reports and plans should be based on a best management approach that centres on the prevention of impacts, protection of the existing environment, and opportunities for rehabilitation and enhancement of any impacted areas.
- All waste generated during construction must be disposed of in accordance with MOE requirements.
- Contractors must be made aware of all environmental considerations so that all environmental standards and commitments for both construction and operation are met. Mitigation measures should be clearly referenced in the Project File/ESR and regularly monitored during the construction stage of the project. In addition, we encourage proponents to conduct post-construction monitoring to ensure all mitigation measures have been effective and are functioning properly. The proponent's construction and post-construction monitoring plans should be documented in the Project File/ESR.

Planning and Policy

- The *Lake Simcoe Protection Plan (LSPP), 2009*, contains policies to protect, improve or restore the elements that contribute to the ecological health of the Lake Simcoe Watershed. Since part of the Study Area is within the Lake Simcoe Watershed, applicable policies should be referenced in the Project File/ESR for this proposed project, and the Town of East Gwillimbury should demonstrate how the project is consistent with these policies.
- The *Oak Ridges Moraine Conservation Plan, 2001*, contains policies that protect the Moraine's ecological and hydrological features and functions. Since the Study Area is within the Oak Ridges Moraine planning area, the proponent should consider and document how the project adheres to the relevant sections of the *Oak Ridges Moraine Conservation Plan*, including section 41, which contains policies for Transportation, infrastructure and utilities. A description of measures that prevent and minimize potential impacts should also be included. You may wish to consider consulting with the Ministry of Municipal Affairs & Housing in this matter.
- The *Greenbelt Plan* contains policies that protect the Greenbelt's agricultural land base and the ecological features and functions occurring on this landscape. Since the Study Area is within the Greenbelt planning area, the Project File/ESR should demonstrate how the project adheres

to the relevant sections of the *Greenbelt Plan*, including Section 4.2.1 - General Infrastructure Policies. A description of measures that prevent and minimize potential impacts should also be included. You may wish to consider consulting with the Ministry of Municipal Affairs & Housing in this matter.

- The 2005 *Provincial Policy Statement* contains policies that protect Ontario's Natural Heritage. Applicable policies should be referenced in the Project File/ESR, and the proponent should demonstrate how this proposed project is consistent with these policies. You may wish to consider consulting with the Ministry of Municipal Affairs & Housing in this matter.
- The *Places to Grow Plan* contains policies which guide decisions on a range of issues such as infrastructure planning and land-use planning to ensure that stronger and more prosperous communities are built in the Greater Golden Horseshoe. The Project File/ESR should demonstrate how this project adheres to the relevant policies of the *Places to Grow Plan*, including Section 3, which contain policies for Infrastructure to Support Growth.

Class EA Process

- There are several different approaches that can be used to conduct a Master Plan, examples of which are outlined in Appendix 4 of the Class EA. The Project File/ESR should clearly indicate the selected approach for conducting the plan, in particular by identifying whether the levels of assessment, consultation and documentation are sufficient to fulfill the requirements for Schedule B or C projects. Please note that any Schedule B or C projects identified in the plan would be subject to Part II Order Requests under the *Environmental Assessment Act*, although the plan itself would not be.
- The Project File/ESR should provide clear and complete documentation of the planning process in order to allow traceability of decision-making. It must also demonstrate how the consultation provisions of the Class EA have been fulfilled, including documentation of all public consultation efforts undertaken during the planning process. Additionally, it should identify all concerns that were raised and how they have been addressed throughout the planning process. The Class EA also directs proponents to include copies of comments submitted on the project by interested stakeholders, and the proponent's responses to these comments.
- The Class EA requires the consideration of the effects of each alternative on all aspects of the environment. The Project File/ESR should include a level of detail (e.g. hydrogeological investigations, terrestrial and aquatic assessments) such that all potential impacts can be identified and appropriate mitigation measures can be developed. Any supporting studies conducted during the Class EA process should be referenced and included as part of the Project File/ESR.
- Please include in the Project File/ESR a list of all subsequent permits or other approvals that may be required for the implementation of the preferred alternative, including Permits to Take Water, Certificates of Approval or other ministerial approvals, approval under the *Canadian Environmental Assessment Act* (CEAA), and conservation authority permits.
- Please note that MOE guidelines and other information related to the issues noted above are available at www.ene.gov.on.ca under the publications link. We encourage the proponent to review all the available guides and to reference any relevant information in the Project File/ESR.

First Nations Consultation

- Please note that as part of the required stakeholder and agency consultation, proponents are advised to contact the Ministry of Aboriginal Affairs and the Department of Indian and Northern

Affairs to determine potentially affected Aboriginal peoples in the project area. Please refer to the website <http://www.ene.gov.on.ca/en/eaab/aboriginal-resources.php> for a list of appropriate government contacts.

- Once identified, you are advised to provide notification directly to the Aboriginal peoples who may be affected by the project and provide them with an opportunity to participate in any planned public consultation sessions and comment on the project.

Thank you for the opportunity to comment on this project. Please ensure that Nisha Shirali, MOE Central Region **EA and Planning Coordinator**, is placed on the project mailing list. We recommend a draft copy of the Project File/ESR be circulated to this office prior to the filing of the final draft, allowing approximately 30-days review time for the ministry's technical reviewers to provide comments. Please also forward our office the Notice of Completion and Project File/ESR when completed. Should you or any members of your project team have any questions regarding the above, please contact me at (416) 326-3577.

Yours sincerely,



Nisha Shirali
Environmental Resource Planner and EA Coordinator
Air, Pesticides and Environmental Planning

- c. Dave Fumerton, York Durham District Office, MOE
Central Region EA File
A & P File

APPENDIX S
Statement Of Limiting Conditions And Assumptions

Statement of Limiting Conditions and Assumptions

1. This Report/Study (the “Work”) has been prepared at the request of, and for the exclusive use of, the Owner, and its affiliates (the “Intended Users”). No one other than the Intended Users has the right to use and rely on the Work without first obtaining the written authorization of Cole Engineering Group Ltd. (Cole Engineering) and its Owner.
2. Cole Engineering expressly excludes liability to any party except the Intended Users for any use of, and/or reliance upon, the Work.
3. Cole Engineering notes that the following assumptions were made in completing the Work:
 - a) the land use description(s) supplied to us are correct;
 - b) the surveys and data supplied to Cole Engineering by the Owner are accurate;
 - c) market timing, approval delivery and secondary source information is within the control of Parties other than Cole Engineering; and
 - d) there are no encroachments, leases, covenants, binding agreements, restrictions, pledges, charges, liens or special assessments outstanding, or encumbrances which would significantly affect the use or servicing.

Investigations have not been carried out to verify these assumptions. Cole Engineering deems the sources of data and statistical information contained herein to be reliable, but we extend no guarantee of accuracy in these respects.

4. Cole Engineering accepts no responsibility for legal interpretations, questions of survey, opinion of title, hidden or inconspicuous conditions of the property, toxic wastes or contaminated materials, soil or sub-soil conditions, environmental, engineering or other factual and technical matters disclosed by the Owner, the Client, or any public agency, which by their nature, may change the outcome of the Work. Such factors, beyond the scope of this Work, could affect the findings, conclusions and opinions rendered in the Work. We have made disclosure of related potential problems that have come to our attention. Responsibility for diligence with respect to all matters of fact reported herein rests with the Intended Users.
5. Cole Engineering practices engineering in the general areas of infrastructure and transportation. It is not qualified to and is not providing legal or planning advice in this Work.
6. The legal description of the property and the area of the site were based upon surveys and data supplied to us by the Owner. The plans, photographs, and sketches contained in this report are included solely to aide in visualizing the location of the property, the configuration and boundaries of the site, and the relative position of the improvements on the said lands.
7. We have made investigations from secondary sources as documented in the Work, but we have not checked for compliance with by-laws, codes, agency and governmental regulations, etc., unless specifically noted in the Work.
8. Because conditions, including capacity, allocation, economic, social, and political factors change rapidly and, on occasion, without notice or warning, the findings of the Work expressed herein, are as of the date of the Work and cannot necessarily be relied upon as of any other date without subsequent advice from Cole Engineering.
9. The value of proposed improvements should be applied only with regard to the purpose and function of the Work, as outlined in the body of this Work. Any cost estimates set out in the Work are based on construction averages and subject to change.
10. Neither possession of the Work, nor a copy of it, carries the right of publication. All copyright in the Work is reserved to Cole Engineering. The Work shall not be disclosed, produced or reproduced, quoted from, or referred to, in whole or in part, or published in any manner, without the express written consent of Cole Engineering and the Owner.
11. The Work is only valid if it bears the professional engineer’s seal and original signature of the author, and if considered in its entirety. Responsibility for unauthorized alteration to the Work is denied.